



GULF STATES UTILITIES COMPANY

RIVER BEND STATION POST OFFICE BOX 220 ST. FRANCISVILLE, LOUISIANA 70775
AREA CODE 504 635-6094 346-8651

July 8, 1985
RBG- 21487
File Nos. G9.5, G9.8.6.2

Mr. Harold R. Denton, Director
Office of Nuclear Reactor Regulation
U.S. Nuclear Regulatory Commission
Washington, D.C. 20555

Dear Mr. Denton:

River Bend Station - Unit 1
Docket No. 50-458

Enclosed is Gulf States Utilities' (GSU) final response to the NRC Staff's Request for Additional Information 210.110 delineated in Mr. A. Schwencer's letter of April 30, 1985 (RBC-31,879). Pursuant to our discussions with your Mechanical Engineering Branch, a confirmatory analysis has been completed and is provided as Enclosure 2 to supplement our initial response of May 20, 1985 (RBG-21,042). Enclosure 1 will be incorporated into a future Final Safety Analysis Report amendment.

Sincerely,

J. E. Booker

J. E. Booker
Manager-Engineering,
Nuclear Fuels & Licensing
River Bend Nuclear Group

JEB/ERG/je

Enclosures

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RESPONSE

The original calculation demonstrating that the feedwater check valves can perform their intended function following a feedwater line break outside the containment (See Question 210.82) was based on an inelastic analysis assuming no strain hardening.

The conclusions from this approach have been confirmed by an inelastic analysis done in accordance with Appendix F of the ASME III Code (1977) for Class 1 service, using the ANSYS computer program (Appendix 3A of the FSAR). The non-linear stress/strain relationship will be conservatively approximated by a bilinear curve with the strain at ultimate stress equal to $2/3$ the elongation at temperature as provided in ASME II, adjusted for strain rate and temperature effects. This analysis has verified that structural integrity of the feedwater check valves is maintained. Any long term leakage through the check valves is controlled by redundant motor-operated valves which are closed and sealed by the penetration valve leakage control system at approximately 25 min. after the accident. Note that, as discussed in FSAR Section 15.6.6, a feedwater line break outside containment is less limiting than other postulated LOCA's.

ENCLOSURE 2

Calculation for River Bend Station Feedwater Check Valve
Integrity Following Pipe Rupture Outside Containment

Appendix F has been added to the calculation which confirms that both feedwater isolation valves will withstand rapid closure and that the pressure boundary will be maintained.

RBS FSAR

QUESTION 210.82 (3.6.2)

Provide the basis for assuring that the feedwater isolation check valves can perform their function following a postulated pipe break of the feedwater line outside containment.

RESPONSE

- 11 | The response to this request is provided in revised Appendix 3C, Section 3C.2.2. See also Question and Response 210.110.

QUESTION 210.110

Your response to Q 210.82 regarding the basis for assuring that the feedwater check valves can perform their intended function following a feedwater line break outside containment is not totally acceptable. In your letter from J. E. Booker to H. Denton dated December 17, 1984, you provided a table which summarized the results of your feedwater check valve analysis. The staff finds that additional information is needed to justify the allowable values used for the disk shear load and strain limits.

In Note (1) to Enclosure 2 of your letter you state:

"Strain due to absorption of energy must be less than 0.7 times the ultimate elongation (based on Table CB-3700-1 of the ASME Code, Section III, Division II, 1983 Edition)."

The staff does not find the use of Table CB-3700-1 to be acceptable for Division 1 components including valves. Table CB-3700-1 is applicable to Division 2 components (i.e., concrete reactor vessels and containment).

Furthermore, in Note (1) you state:

"It is assured that strain rates are sufficiently high to warrant doubling the material allowables (Ref. Juvinal, R. C., Stress, Strain, and Strength - McGraw-Hill, 1967, pg-168)."

The staff finds that the figure provided in the referenced text book shows the effect of strain rate on tensile properties of typical mild steel at room temperature. However, the text book figure provides only general information not suitable for specific design application without more details. The use of the figure in the text book is not appropriate for the specific valve design because it does not represent the yield strength of the disk material (i.e., SA-216 WCC) at temperature. Furthermore, the text book states that because it is difficult to predict the conditions of impact strain rate, it is common to design parts for impact based on empirically determined stress impact factors together with the static strength of the materials. It appears that the shear load on the disk for check valve 1B21*A0VF032A, B is near the allowable value. (Factor of Safety equal to 1.05). Thus, the staff is concerned that the acceptability of doubling the material allowables at temperature is not well-justified, and because of the small design margin and the uncertainties involved, the shear load on the disk might be unacceptable. Additionally, it appears the calculated values were established assuming strain hardening effects. Provide the basis for this assumption if used. Furthermore, it has been shown for mild steel at elevated temperatures that a negative slope occurs in the stress-strain rate curve at the higher strains which is generally associated with strain aging.

The staff requests that you provide a detailed discussion of the valve parts which do not satisfy linear stress limits per the ASME Code and the basis for concluding that the leakage of the reactor coolant pressure boundary through the feedwater check valves following the postulated break in the feedwater line outside containment is acceptable using your calculated strain values.

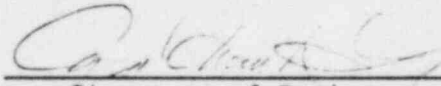
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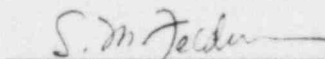
REVIEW STATEMENT

This calculation has been reviewed in accordance with EMDM 82-12 and was found to be adequate. The method of review utilized was (circle one):

- a. Comparison with a similar previous Calculation No. _____
- ☒ b. Review of calculation.
- c. Alternate Calculation No. _____



Signature of Reviewer *REV. 1 C.T.M.*
 REV. 2 C.T.M.



Signature of Independent
Reviewer (where different
from Reviewer) *REV 1 S.M.F.*

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APPENDIX

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A-Plastic Strain Energy for Beams of Various Cross-section

B-Plastic Strain Energy for a Circular Plate in Bending

C-Miscellaneous Reference Material

D-Computer Log and Microfiche

E-Voided Pages from Revisions

(1)

F-Confirmation of Conclusions

(2)

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REVISION STATUS TABLE

REV NO.	PAGES REVISED		DESCRIPTION & REASON	ORIGINATOR & DATE	REVIEWER & DATE
	CHANGED	ADDED			
0	-	-	Original Issue	J. Gwinn 10/26/84	Cam Ly 10/27/84
(1)	Title Page 2,4,5 7,9,13 15 17-19 21-30 D-8,-9 Sect'ns 9.&10.		Revisions to clarify the results of analysis and data input sources.	J. Gwinn 12-7-84	Cam Ly 12-7-84
(2)			Appendix F added to confirm the conclusions of Revision 1.	J. Gwinn	Cam Ly

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REV.2

SUMMARY

The reactor vessel water is protected from blowdown following a postulated rupture of the feedwater piping outside the containment, by the Velan check valve 1B21*F010A,B inside the containment and by the Atwood/Morrill testable check valve 1B21*AOVF32A,B outside the containment. Breaks are not postulated in the piping between the valves because that region is classified as break exclusion.

The reverse flow caused by the sudden pressure reduction at the break rapidly closes both valves. This calculation examines the ability of these valves to withstand the impact of the disk on the seat without excessive leakage thereafter.

Loads on the critical elements, i.e., the disk, tail link, rockshaft and seat, were computed by simulation of the impact dynamics using the STARDYNE and GTSTRUDL computer programs. Both uniform and point impact were considered in search of the worst case kickback loads generated by the point of impact not being at the center of percussion. Seismic, hydrodynamic and dead loads were not considered because of their insignificant magnitude compared to impact loads.

In those cases where linear stresses were not below their allowables, a strain energy analysis or a non-linear time history strain analysis was conducted to demonstrate integrity. (1)

It is concluded that both valves will remain intact and that the leakage will be within the make-up capability of the High Pressure Core Spray (HPCS) or the Reactor Core Injection Cooling (RCIC) system, following rupture of the feedwater piping outside the containment.

Revision 2 adds Appendix F, an inelastic systems and component analysis using the non-linear option of the ANSYS computer program to confirm the conclusions. (2)

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1. OBJECTIVE/INTRODUCTION

The purpose of this calculation is to evaluate the ability of River Bend ASME III class 1 feedwater check valves 1B21*AOVF32(specification 228.218) and 1B21*F010(specification 228.211) to withstand rapid closure following the postulated pipe break outside the containment shown in Figure 1.1. The acceptance criterion is that gross leak rates do not occur because of disk rupture, serious fracture of the seat/disk interface or misalignment of the disk with respect to the seat from this faulted event.

There are no consequences defined for a High Energy Line Break(HELB) outside the containment. The other feedwater system is not available for make-up because loss-of-off-site power is postulated and this system is not on the emergency diesel generators. Single active failure of the HPCS(465 GPM) or the RCIC(600 GPM) systems is assumed, leaving at least one of these systems available as a source of water to the vessel required to make-up for any leakage.

Feedwater pump trip from high water level in the vessel, a normal operating condition, was found by the analysis of Reference 8 not to cause high energy impact of the disk on the seat. The inertia of the motor was sufficiently high that the pump took approximately 1000 seconds to completely stop. From the stand point of the valve, two(2) seconds were required to generate a reverse flow rate equal to the normal flow rate, resulting in a very gentle closing of these check valves. Pump trip is not considered a critical loading condition and is, therefore, not treated in this analysis.

Added to pipe rupture loads on a square-root-sum- of-squares basis are the seismic and hydrodynamic loads. However, they are not considered in this analysis because of insignificant magnitudes as compared to the impact forces generated by reverse flow following rupture of the feedwater line. For the same reason, dead load and opening loads are not considered in this analysis.

Figures 1.2 and 1.3 show a cross-section of these valves from the vendor drawing. Figure 1.4 is a sketch labelling the dimensions and elements of importance to the integrity of swing check valves. The coordinate system used to identify the loading directions is also shown therein.

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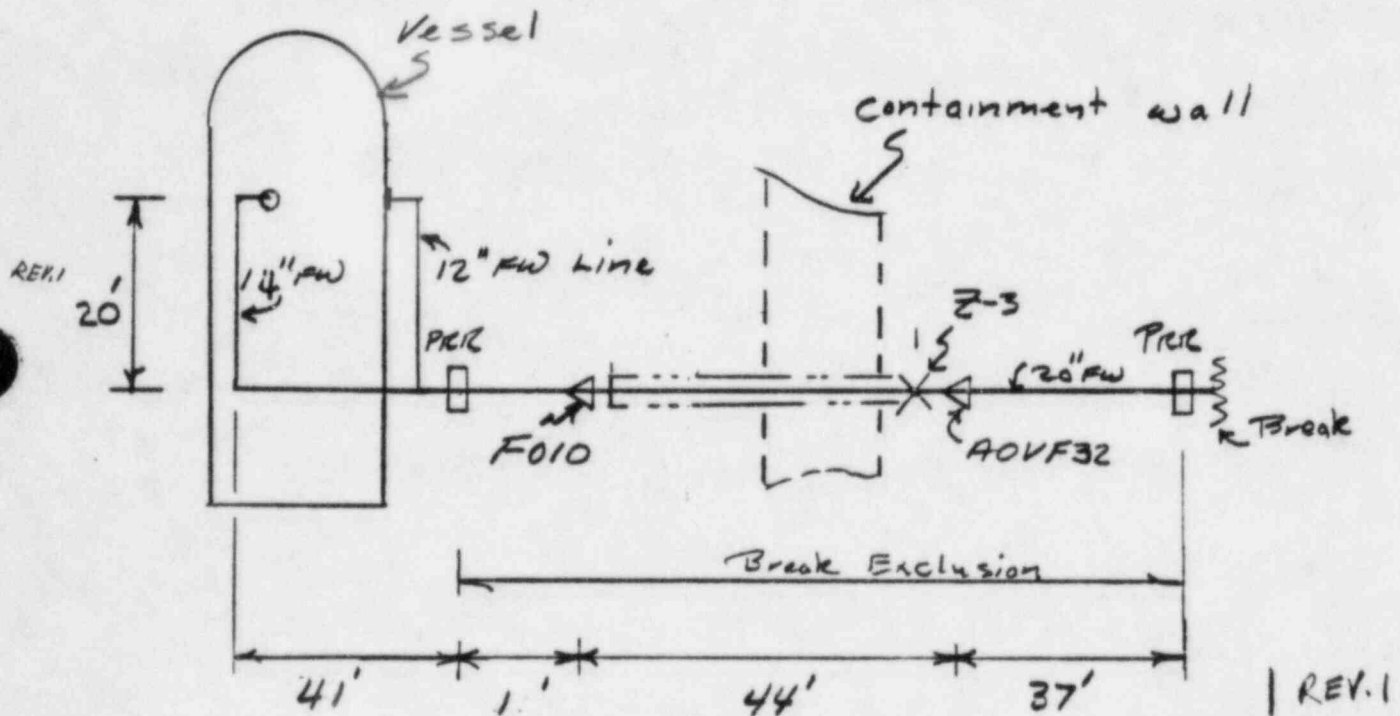


Figure 1.1 Sketch of Piping Model (Ref.17)

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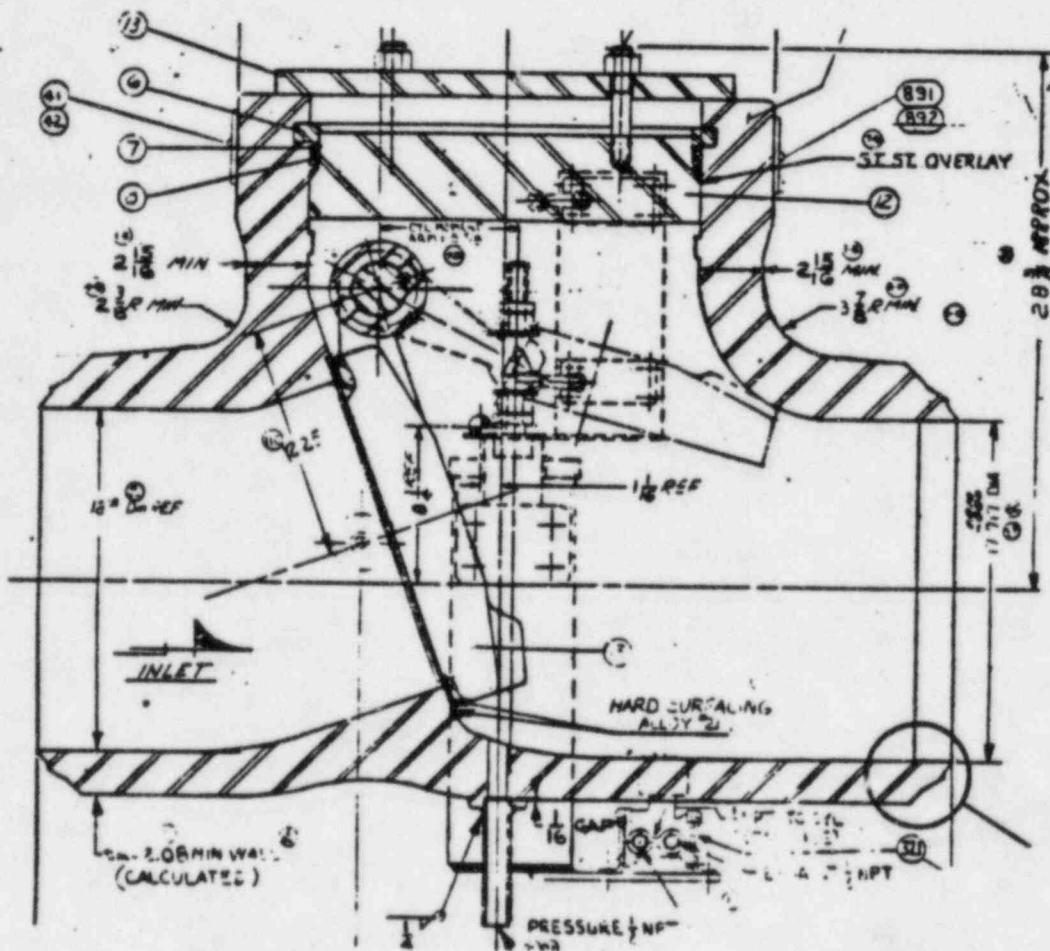


Figure 1.2 Cross-section of the AOVF32 Check Valve(Ref.2)

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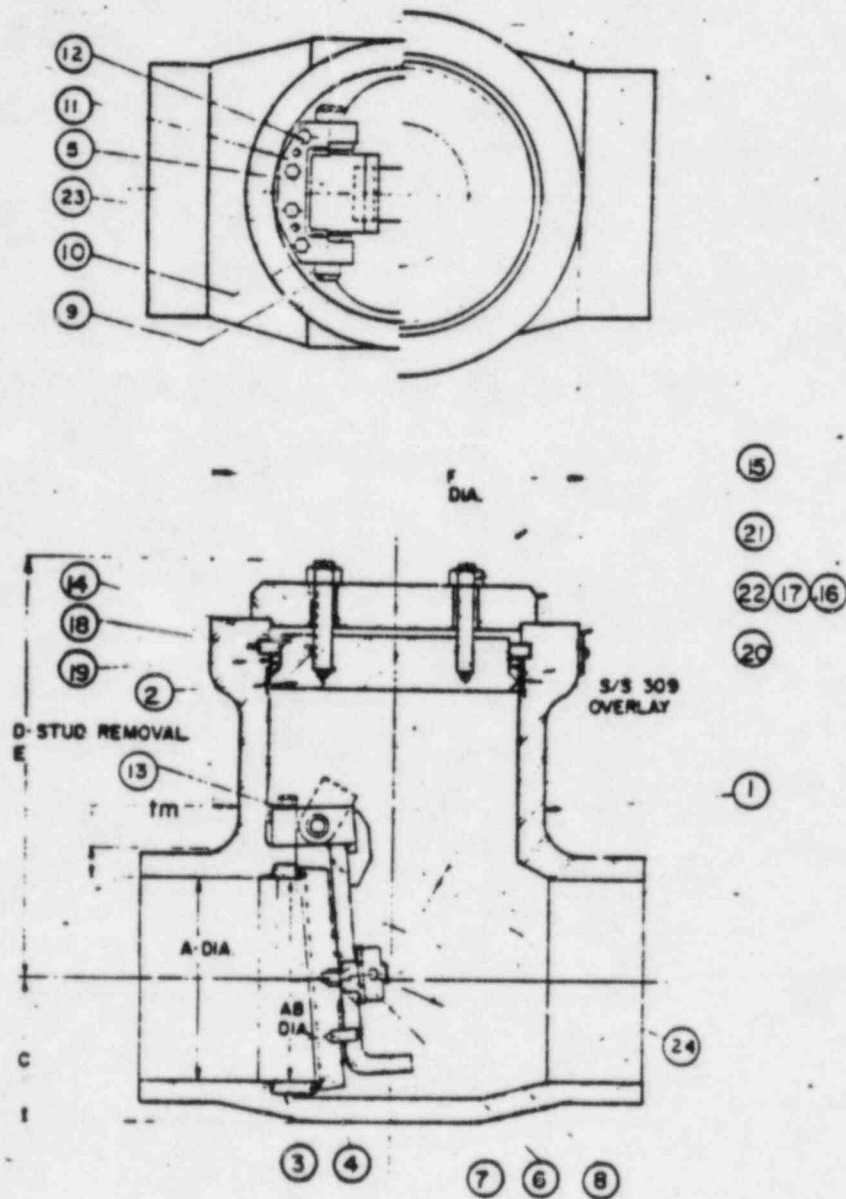


Figure 1.3 Cross-section of the F010 Check Valve(Ref.3)

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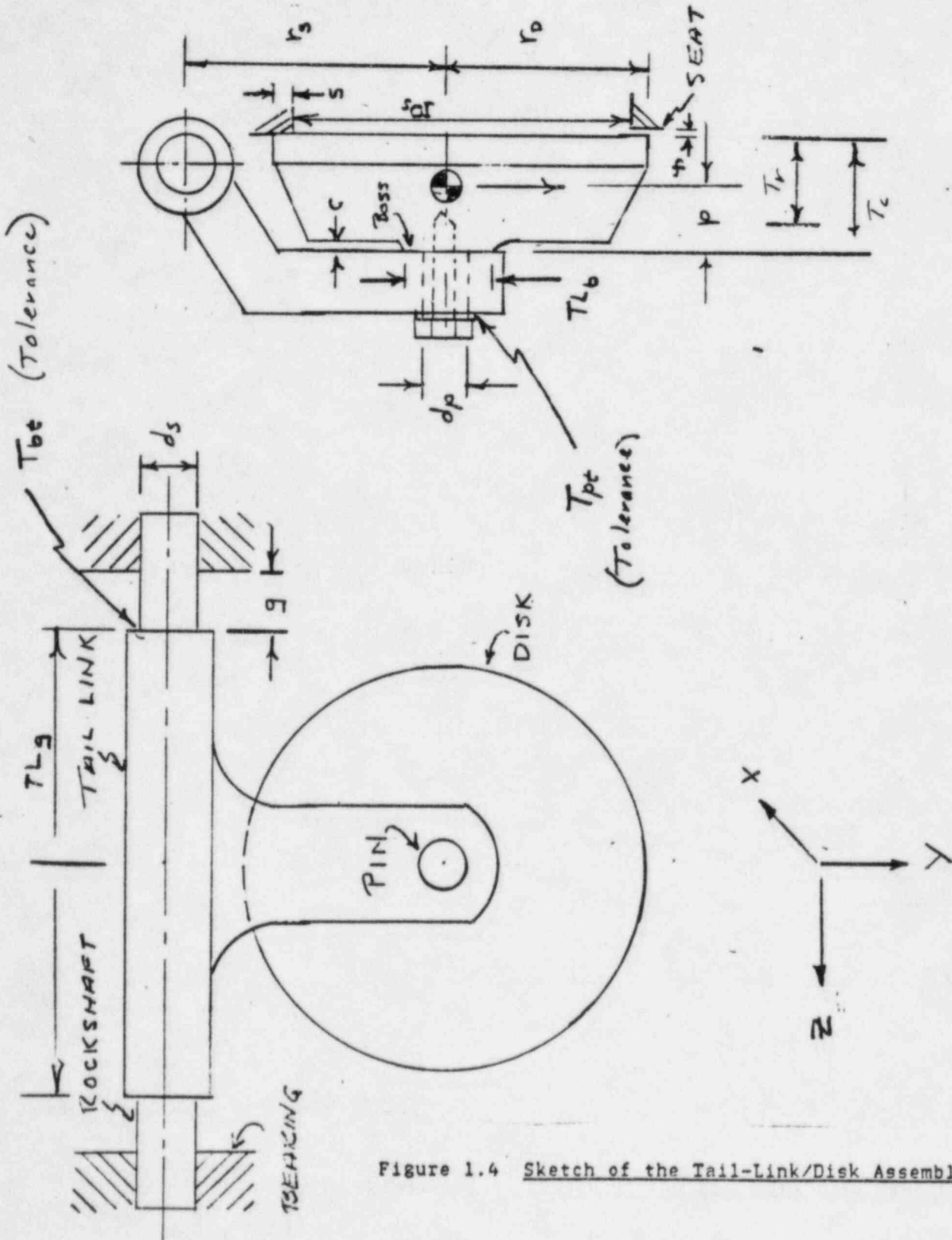


Figure 1.4 Sketch of the Tail-Link/Disk Assembly

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2. METHOD

Elements critical to the integrity of a check valve identified in Figure 1.4, are the disk, tail link, the pin which connects the tail link to the disk(F010 only - AOVF32 tail link and disk are integral), the rockshaft and the seat/disk interface.

The dynamic loading just before impact is purely centrifugal, reacted by the rockshaft, pin and tail link. Acting simultaneously with this load are kickback forces arising from the disk impacting the seat at a single arbitrary point(non-uniform impact) not at the center of percussion. These loads may be worst case for the tail link, pin and rockshaft. For the seat and disk, worst case loads are generated by a uniform impact of the disk on the seat combined with the pressure surge from water hammer.

Kickback loads are neither obvious nor easily calculated manually and were therefore modelled on GTSTRUDL and STARDYNE , using the integration option for specified initial conditions. Typical initial conditions were the impact speed at all nodes except a node suffering impact, its speed being set equal to zero. Note that all dynamic loads are conservatively calculated based on a linear stress-strain relationship.

The details associated with calculating these loads are presented in Section 8. The integrity analyses of Sections 9 & 10 are linear elastic for faulted service, supplemented as necessary by a plastic strain energy analysis to treat limited energy loading. Although not an ASME III code analysis, certain methods and allowables from Appendix F(Ref.9) were used in this analysis for convenience.

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3. ASSUMPTIONS

- 1) The strain rates are sufficiently high to warrant doubling the material allowables, as provided in reference 6, page 168.
(confirmation not required)

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4. DESIGN INPUT

Mass Properties

TABLE 4.1	Atwood/Morrill Check Valve 1B21*AOVF32A,B		Velan Check Valve 1B21*F010A,B	
		REF.		REF.
Disk - wt.	260	12	185#	13
Tail Link - wt.	71 (Integral/Disk)	12	20	13
Mass Moment of Inertia/Rockshaft	13.9 slug-ft**2	2	6.8	13

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Materials

TABLE 4.2	Atwood/Morrill Check Valve 1B21*AOVF32A,B		Velan Check Valve 1B21*F010A,B	
		REF.		REF.
Disk	cs SA216 WCC	2	cs SA105 Stellite #6 facing	3
Rock Shaft	ss A182 F6a	2	ss SA564 630 Hardened(1100 degF)	3
Seat	cs SA216 WCC Stellite #21 facing	2	cs SA105 Stellite #6 facing	3
Tail Link	cs SA216 WCC	2	cs SA105	3
Pin & Stud	N/A	N/A	cs SA 105	3
Nut	N/A	N/A	ss SA564 630	3

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Dimensions

TABLE 4.3	Atwood/Morrill 1B21*AOVF32A,B		Velan alve 1B21*F010A,B	
		SYMBL	REF.	REF.
Diam of Disk	19. in.	2*Rd	16	17.5 11
Dist - Rkshft/Dsk Ctr	14.	Rs	16	11.25 3
Diam of Pin	N/A	Dp	N/A	2.5 (welded to disk) 3
Diam - Stud for Tlk	N/A	Dsd	N/A	1.0 (Threaded) 3
Diam - TailLink Nut	N/A		N/A	3.5 3
Diam of Rockshaft	1.88	Ds	16	1.0 3
Gap - Tlk/Bearing	.56	g	16	.1 or less 3
Dist - Dsk CG/Pin Seat	N/A	P	N/A	1.375 11
Clearance - Disk/Tlk	N/A	c	N/A	.125 3
Thickness of Disk	3.75ctr, 2.5rim	Tc/Tr	16	2.75 11
Seat Annulus	.5+	s	2	.56 3
Tail Link Section	2.9 x 15.0	h x w	16	1.0 x 4.5 3
Tail Link Grip	16.4	TLg	16	5.25 3
Seat Inside Diam	16.75	IDs	13	15.75 3
Diam of Tlk Boss	N/A	TLb	N/A	3.5 3
Fit - Seat/Disk Face	*	f		.015 (at rim) 3
Tolerance - Brg/Rkshft	*	Tbr		+.004, -0.0 3
Tolerance - Pin/Tlk	N/A	Tpt	N/A	.010 (nut prevents rot'n) 3
Tolerance - Rkshft/Tlk	*	Trt		+0.0, -.005 3

* Assumed same as Velan

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Material Properties at 500F (Ref. 9, Appendix I)

TABLE 4.4	Sy (ksi)	Su (ksi)	Elong.* (%)	E (psi*e-6)	Remarks
cs SA 216 WCC	40	70	22	27	
cs SA 105	29.1	70	22	27	
ss SA 564 630	95.2	133	16	27	
ss A 182 F6a	34.7	66.3	18	27	

* Ref.10(ambient temperatures) *REV. 1*

Section Properties(calculated from Table 4.3)

TABLE 4.5	AOVF32	F010
Tail Link - Area	43.0 in**2	4.5(2.@pin)
- Mom of Inert/z	30.0 in**4	.375(.167@pin)
- Torsion Const/y	119.0 in**4	1.5
Rockshaft - Area	2.8	.79
- Mom of Inert/z	.61	.049
Pin - Area	N/A	4.9
- Mom of Inert/z	N/A	1.9
Stud - Area	N/A	.60(root)

Impact Speed and Hammer Pressure(Ref. 1, page 31)

TABLE 4.6	AOVF32	F010
Impact Speed(rad/sec)	42	58
Max Pressure(psi) - six(6) milliseconds after impact	3100	3200

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Allowables for Faulted Service

The primary stress from a steady-state load must be less than .7 times the ultimate strength(S_u) of the material(Ref.9, App.F). Stresses acting for a very short time(less than .5 milliseconds) must be less than the ultimate strength of the material. If not, then it is necessary to recognize that the real question is the ability of the configuration to absorb the energy of impact at acceptable values of strain, taken here to be .7 times the ultimate elongation, based on Table CB-3700-1 of Ref.9A. Strains acting for a very short time(.5 ms) must be less than the ultimate elongation. Because the specific reduction of elongation with temperature was not available, the ambient values of elongation in Table 4.4 were used without benefit of the increase from high strain rate loading.

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5. SUMMARY OF RESULTS

TABLE 5.1		AOVF32				F010			
		Calc'd	Allow.	FS	PG10-	Calc'd	Allow.	FS	PG9-
Disk	-shear #	7.9E6	8.3E6	1.05	2	2.7E6	7.7E6	2.9	2
	-strain %	2.1	15.4	7.3	3	3.6	15.4	4.3	4
Rockshaft	-stress ksi	N/A	N/A	-	-	13.6	53.8	4.0	12
	-ratio(1)	.65	1.0	1.5	11	N/A	N/A	-	-
Tail Link	-stress ksi	46.4	98.	2.1	16	68.1	70.0	1.03	8
	-ratio(1)	.48	1.0	2.1	17	N/A	N/A	-	-
Pin	-stress ksi	N/A	N/A	-	-	23.	49.	2.1	7
Stud	-stress ksi	N/A	N/A	-	-	41.	49.	1.2	6
Seat/Disk Interface	-stress ksi	74.	98.	1.3	18	98.8	140.	1.4	15

(1) Appendix A9231, Ref. 9

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6. CONCLUSIONS

It is concluded that both 1B21*AOVF32A(specification 228.218) and 1B21*F010A(specification 228.211) valves will withstand rapid closure and remain intact. In addition, any leakage will be within the make-up capability of the HPCS or RCIC system, following rupture of the feedwater piping outside the containment.

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7. REFERENCES

- 1) SWEC Calculation 12210-NP(B)-1007-PX, Feedwater Check Valve Closure Following a Pipe Rupture Event, April 1984
- 2) Atwood/Morrill Drawing 13464-04-H11 (SWEC File No. 228.218-020-4i)
- 3) Velan Drawing 7890-015 (SWEC File No. 228.211-049-26D)
- 4) SWEC Specification 228.211 for Check Valves, July 1983
- 5) SWEC Specification 228.218 for Reactor Feed and Testable Check Valves, Oct 1983
- 6) Juvinal, R.C., Stress, Strain and Strength, McGraw-Hill, 1967
- 7) Wambold, J.C. & Ju, F., Safe Impact Speeds for Hollow Circular Cylinders, Journal of Applied Mechanics, June 1970
- 8) SWEC Calculation 12210-NP(C)1000-PX, Water Hammer Analysis of Feedwater Piping for Pump Trip, May 1984.
- 9) ASME B & PV code, 1974 through winter 1976, Section III
- 9A) ASME B & PV code, 1983, Section III, Division II
- 10) Manual of ASTM Standards, 1977
- 11) Velan Design Report for Class 1 20" Forged Carbon Steel Check Valve, ASME class 900#, DR-1204, February 1981
- 12) Atwood/Morrill Design Report for Class 1 Feedwater Check Valve, Procedure No. 205-13464-04, December 1976
- 13) Telex No. 05-826599 from Velan dated 2-14-84 (App. C herein)
- 14) Atwood/Morrill Report No. 300-15202-00 dated 4-17-84, Valve Data for Performing Closure Analysis (App. C herein)
- 15) Rourke, R.J., Formulas for Stress and Strain, McGraw-Hill, 4th Edition
- 16) SWEC Internal Correspondence (EM# 2697) dated 17 October 1984 from J. MacIntosh, FW Check Valve AOVF32B Measurements (App. C herein).
- 17) Feedwater piping drawings 12210 EP 17C-7 and EP 17F-5

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8. LOADS ANALYSIS

8.1 Pre-Impact Loads

The centrifigal force(F_c) arising from the angular speed(Ω) of the disk just prior to impact is given by

$$F_c = W \cdot \Omega^2 \cdot R / g$$

where R is the distance from the rockshaft to the disk/tail link center of gravity and W is its weight. Table 8.1 below contains the pre-impact loads on the rockshaft calculated from the above expression:

<u>TABLE 8.1</u>	<u>AOVF32</u>	<u>F010</u>	<u>KEF</u> <i>REV. 1</i>
Omega - rad/sec	45	60	1
W - #	331	205	Table 4.1
R - in.	14.0	11.25	Table 4.3
F_c - #	24400	21500	calculated

8.2 Kickback Loads

Kickback loads arise when a body rotating about one point is impacted at any other point not at the center of percussion. Although the anticipated angle between the plane of the seat and the disk is very small, nonetheless, extremely high loads are possible by such a point contact(non-uniform) of the disk rim on the seat. They disappear in less than one(1) millisecond because after that time, the disk rim is in uniform contact with the seat. They are much reduced if sufficient articulation is provided by the tolerances in the shaft bearing and/or a pinned attachment of the disk to the tail link, because this limits the kickback forces to those arising only from uniform impact of the disk rim on the seat. Under this condition, the forces from rotation of the large mass of the disk about center-line tail link after impact do not occur.

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Computer Model

The Velan valve 1B21*F010A has a separate tail link(Figure 1.2) attached to the disk by a pin whereas the tail link and disk are integral(Figure 1.3) in the design of the Atwood/Morrill valve 1B21*AOVF32A.

Both tail-link/disk assemblies were modelled as shown in Figures 8.1 and 8.2, for computation of the kickback loads under conditions of uniform and point impact of the disk rim on the seat. The postulated impact points(IP) are identified as a, b, c, d, e, f, & g therein.

Results

Tables 8.2 and 8.3 give the kickback loads at the attachment of the tail link to the rockshaft and to the pin at the disk center for non-uniform impact at several points(IP) on the seat, as shown in Figure 8.1. These loads for the F010 valves are much smaller than those for the AOVF32 valves because its tail link(see sketch in Ref. 14, App. C herein) is extremely rigid compared to the F010 tail link(see sketch in section 9.2 herein). Tables 8.4 and 8.5 provide kickback loads at these points plus the load on the seat for the case of uniform impact. *REV 1*

The kickback loads from point impact are small compared to uniform impact loads because the fit of the disk to the seat in the closed position is tight for both valves, the specified tolerance(Table 4.3) being .015 inches. The loads in Tables 8.2 and 8.3 are based on a tolerance of twice that amount, .030 inches.

The 7200 kip load on the disk rim in Table 8.4 is the sum of the loads appearing at each node on the rim. Table 8.6 presents the loads taken from the computer run which is plotted in Figure 8.3. *REV 1*

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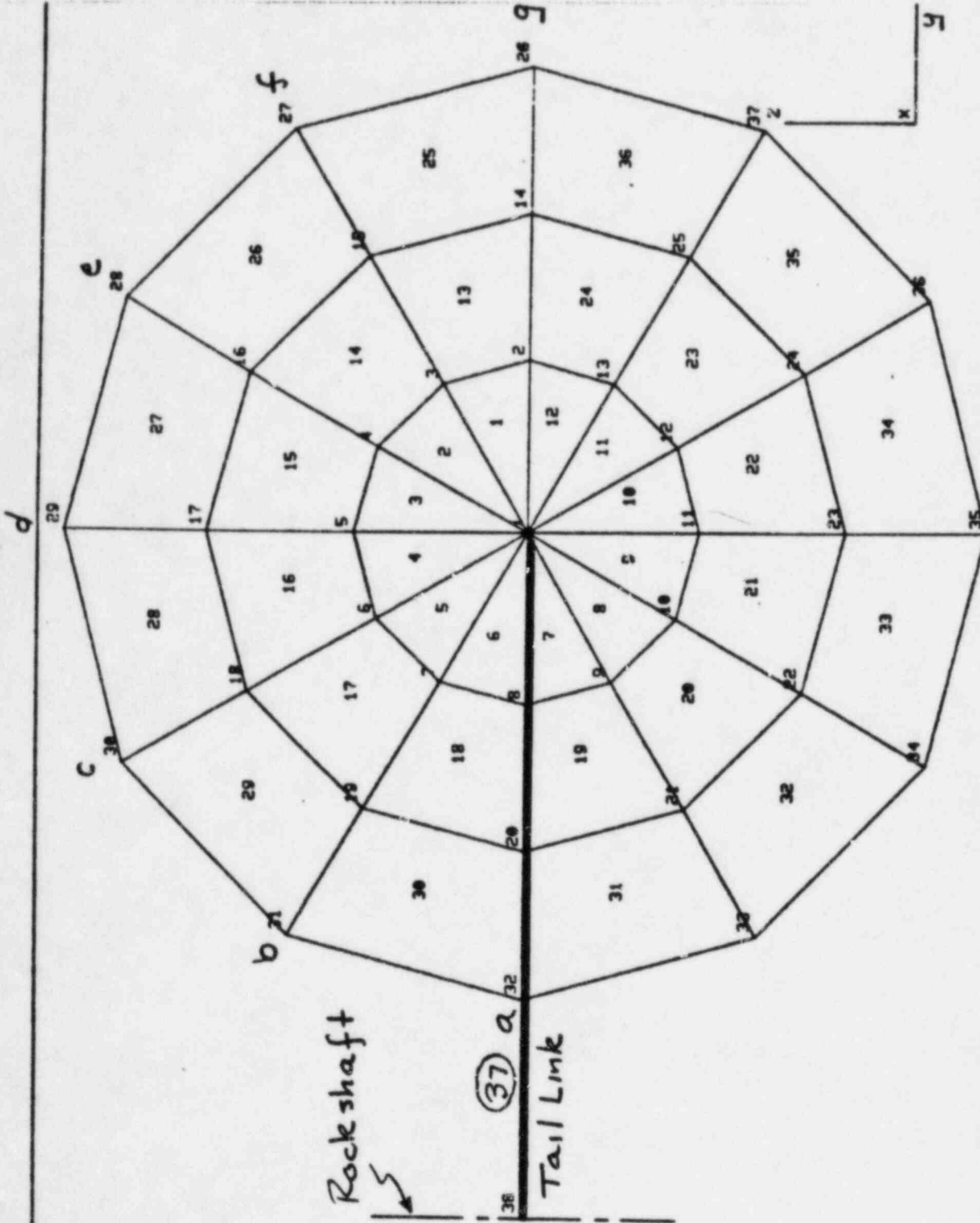


Figure 8.1 Computer Model - F010 Tail-Link/Disk Kickback Loads

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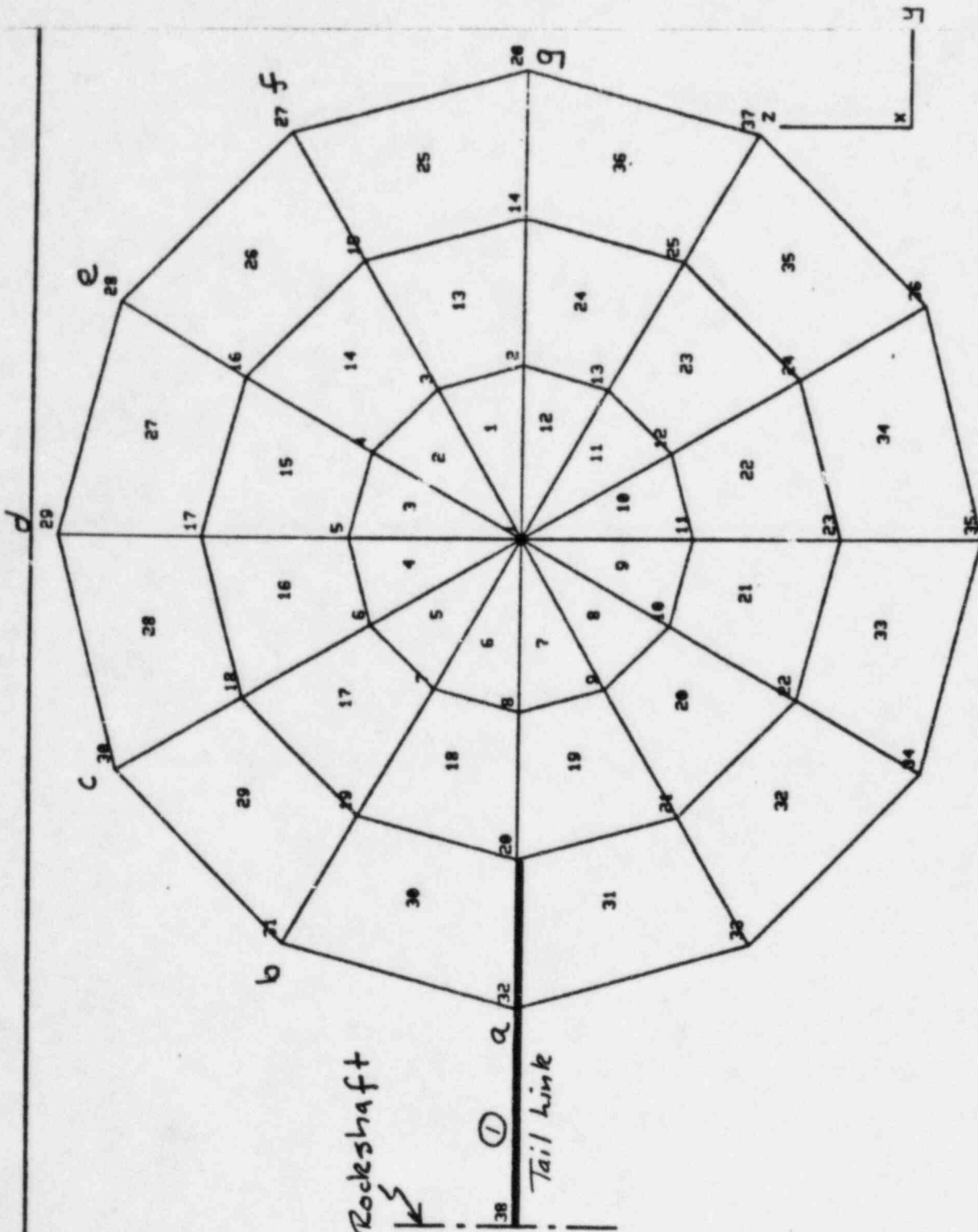


Figure 8.2 Computer Model - AOVF32 Tail-Link/Disk Kickback loads

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TABLE 8.2		POINT IMPACT - KICKBACK LOADING TABLE						Equipment AOVF32
IP	LOAD POINT	FXK (kips)	FYK (kips)	FZK (kips)	MXK (in-k)	MYK (in-k)	MZK (in-k)	REMARKS
a	R O C K S H A F T	60	-	-	-	0.0	-	
b		20	-	-	-	160	-	
c		1	-	-	-	30	-	
d		1	-	-	-	60	-	
e		5	-	-	-	10	-	
f		10	-	-	-	15	-	
g		10	-	-	-	0.0	-	
Worst Case		60	-	-	-	160	-	
a	Tail Link Pairing to Disk	70	-	-	-	0.0	450	
b		20	-	-	-	160	130	
c		1	-	-	-	30	10	
d		1	-	-	-	60	60	
e		5	-	-	-	10	30	
f		10	-	-	-	15	50	
g		10	-	-	-	0.0	60	
Worst Case		70	-	-	-	160	450	

These loads are based on an impact speed of 60 rad/sec.

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Calculation Identification Number				PAGE 27 <i>REV 1</i>
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

TABLE 8.3		POINT IMPACT - KICKBACK LOADING TABLE						Equipment F010
IP*	LOAD POINT	FXK (kips)	FYK (kips)	FZK (kips)	MXK (in-k)	MYK (in-k)	MZK (in-k)	REMARKS
a	R O C K S H A F T	.01	-	-	-	0.0	-	
b		.01	-	-	-	.04	-	
c		.01	-	-	-	.13	-	
d		.01	-	-	-	.23	-	
e		.03	-	-	-	.28	-	
f		.06	-	-	-	.20	-	
g		.07	-	-	-	0.0	-	
Worst Case		.07	-	-	-	.28	-	
a	D I S K C E N T E R (Pin)	.01	-	-	-	0.0	.10	
b		.01	-	-	-	.04	.12	
c		.01	-	-	-	.13	.13	
d		.01	-	-	-	.23	.01	
e		.03	-	-	-	.28	.28	
f		.06	-	-	-	.20	.67	
g		.07	-	-	-	0.0	0.0	
Worst Case		.07	-	-	-	.28	.67	

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TABLE 8.4	UNIFORM IMPACT - KICKBACK LOADING TABLE						Equipment AOVF32
LOAD POINT	FXK (kips)	FYK (kips)	FZK (kips)	MXK (in-k)	MYK (in-k)	MZK (in-k)	REMARKS
Rockshaft -Unif. Impact	240	-	-	-	-	-	
Tail Link/Disk -Unif. Impact	240	-	-	-	-	960*	
Disk Rim -Unif. Impact	7200	-	-	-	-	-	

* Weakest section of the tail link(intersection with disk)

These loads are based on an impact speed of 60 rad/sec.

TABLE 8.5	UNIFORM IMPACT - KICKBACK LOADING TABLE						Equipment F010
LOAD POINT	FXK (kips)	FYK (kips)	FZK (kips)	MXK (in-k)	MYK (in-k)	MZK (in-k)	REMARKS
Rockshaft -Unif. Impact	5	-	-	-	-	-	
Tail Link/Disk -Unif. Impact	5	-	-	-	-	35	
Disk Rim -Unif. Impact	2070	-	-	-	-	-	

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10210	N11(C)		2043		SQE						

Seconds (1/10)	34	0.2	3.8	1.0	1.0	1.0	16.	15.	.4	2.	4	6	6.8	8.
26 (Kips)	576	503	505	400	400	400	481	481	374	100	525	180	216	158
27	365	505	490	410	410	410	242	242	335	45	489	194	409	186
28	420	285	625	610	610	610	2	2	283	347	521	258	532	205
29	290	210	605	716	716	716	180	180	206	484	604	247	500	116
30	215	105	525	693	693	693	448	448	102	349	521	277	362	309
31	410	100	370	525	525	525	741	741	98	401	388	578	311	77
32	825	325	885	810	810	810	1385	1385	320	900	384	922	554	8
SUM.	4851	3328	6326	7178	7178	7178	5087	5085	2722	4252	6095	4628	5098	2032

OFF RUN # 35N, TDB 10/24/84

DISK IMPACT LOADS ARE CALCULATED BY ABSOLUTE SUM OF ALL THE NODAL LOADS WITH THE CONSIDERATION OF THE SYMMETRICAL OF THE NODES:

SUM = 1 * (LOAD OF NODE 26) + 2 * (SUMMATION OF LOADS FROM NODE 26 TO NODE 31) + 1/4 (LOAD OF NODE 32)

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TABLE 8.6 DISK RIM NODAL IMPACT LOADS OF 18214 ANK7032A (CONT.)

SECTIONS NODE	8	16	24	44	5.6	6.4	7.6	8.4	9.2
26 (KIPS)	449	481	323	306	55	246	76	287	257
27	415	242	258	267	38	250	116	218	195
28	333	2	56	177	1	172	200	128	271
29	320	180	89	97	30	179	218	169	341
30	413	448	29	94	74	280	119	197	253
31	278	741	47	366	167	486	145	116	442
32	128	1380	149	940	330	932	409	107	821
SUM	4105	5087	2148	8205	1005	3962	2081	2070	4882

REF RUN # 35N, TAB 10/04/84

SUM = 14 (LOADS OF NODE 26) + 24 (SUMMATION OF LOADS FROM NODE 26 TO NODE 31) + 1 (LOADS TO NODE 32)

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Calculation Identification Number				PAGE 31 REV 1
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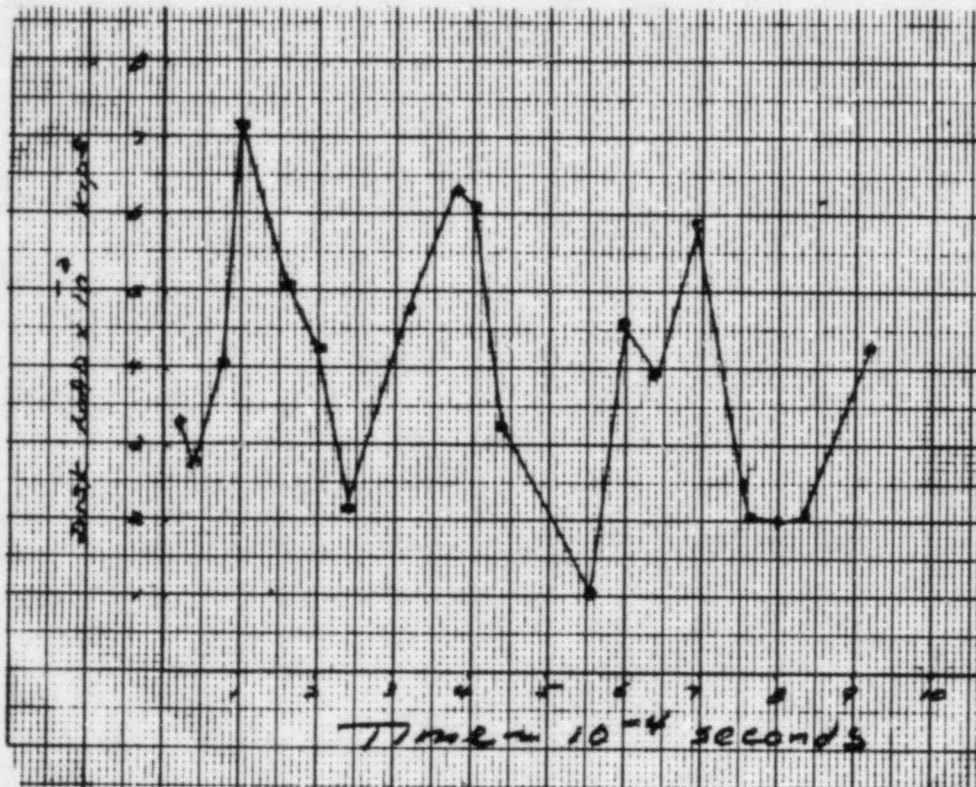


Figure 8.2 Disk Impact Load vs Time for AOVF32

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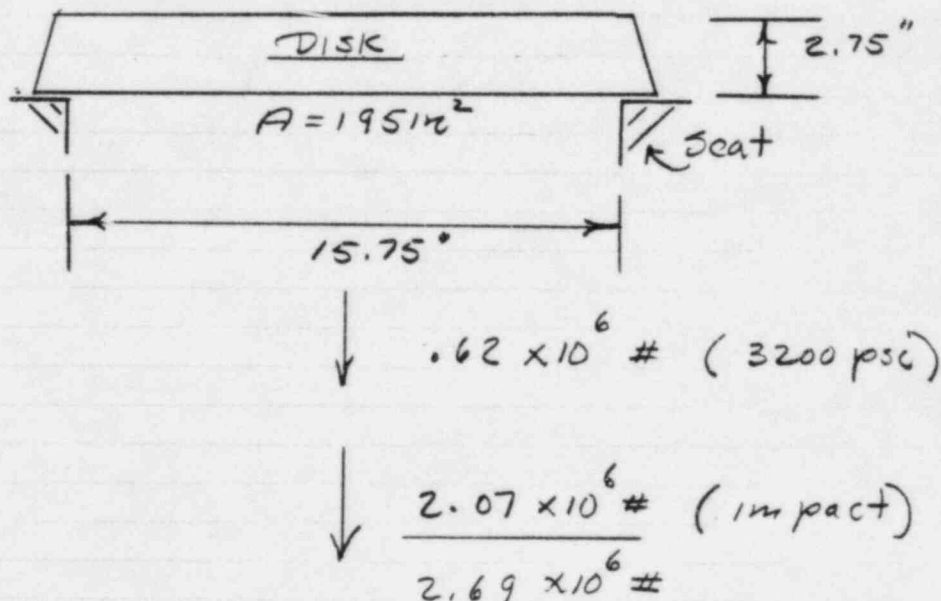
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9. INTEGRITY OF VELAN CHECK VALVE 1B21* F010

9.1 DISK (SA105)

Uniform impact of the disk on the seat generates a reaction of $2.07 \times 10^6 \#$, as given by FXK in Table 8.5. Acting simultaneously with impact is the pressure surge from rapidly closing the valve. The maximum pressure is used although it occurs several milli seconds after impact.



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Shear Out

The allowable shear load in the disk at the inside diameter of the seat where the bending stress in the disk is zero, is given by

$$P_u = .7 \frac{\sigma_u}{\sqrt{3}} \cdot \text{shear area} \cdot \text{strain rate factor}$$

$$= .7 \left(\frac{70000}{1.73} \right) (15.75 \pi 2.75) (2)$$

$$= 7.7 \times 10^6 \#$$

Since $7.7 \times 10^6 \# > 2.7 \times 10^6$, $F_s = 2.9$ OK
the integrity of the disk in shear out is demonstrated.

Rupture

The disk must absorb the impact energy at strains below the rupture level. The impact energy

$$= \frac{192}{32} (60)^2 \frac{12}{2} = 129,000 \text{ in} \#$$

$$\text{where effective disk weight} = 175 + \frac{20}{3} = 192 \#$$

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The plastic strain energy of a circular disk from Appendix B

$$= \frac{\pi}{2} \left(\frac{\pi}{2} - 1 \right) \sigma_y t^2 y_0$$

$$= .89 (29100) (2.75)^2 y_0 = 196,000 y_0$$

Equating to the impact energy and solving for the deflection at the center of the disk

$$y_0 = \frac{129,000}{196,000} = .66''$$

From Appendix B, the maximum strain associated with this deflection is

$$\epsilon = \frac{t}{2} \left(\frac{\pi}{D} \right)^2 y_0 = \frac{2.75}{2} \left(\frac{\pi}{15.75} \right)^2 (.66)$$

$$= .036 \text{ in/in}$$

which neglects the deflection of the disk from the pressure, it being in the elastic range and therefore negligible.

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FS = 4.3

which shows that the disk will not rupture under impact and pressure surge loading combined.

9.2 Pin (SA105)

4 1/2"

2 1/2" hole for pin

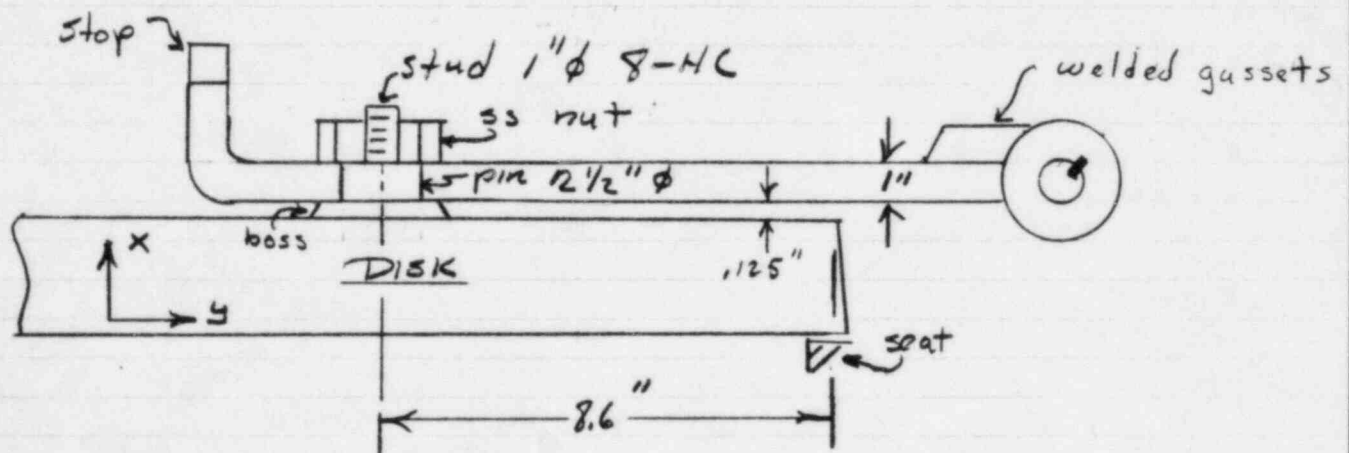
1" ϕ

11.25"

rock shaft
1" ϕ

x

z



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The pin and stud are integral and welded to the disk. A ss nut locked to the stud keeps the tail link tight to the boss.

Pre-impact

The centrifugal force, F_c , is reacted by shear on the pin. The shear stress on the pin

$$= F_c / \text{Area of pin}$$

$$= 21500 / 4.9 = 4400 \text{ psi}$$

the allowable shear stress

$$= .75 S_u / \sqrt{3} = .7 (70000) / 1.73$$

$$= 28300 \text{ psi} > 4400 \quad F_s = 6.14 \text{ OK}$$

the centrifugal force is eccentric to the tail link, generating a moment which is reacted by the stud in tension and the boss in compression,

$$= F_c \left(\frac{2.75}{2} + .125 + \frac{1}{2} \right)$$

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CALCULATION IDENTIFICATION NUMBER				PAGE <u>9-6</u> REV. <u>1</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>N/M(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQ/E</u>	
1				
2				
3	$= 21,500 (2.0) = 43,000 \text{ in}^{\#}$			
4				
5				
6	The distance between the edge of the boss			
7	and the stud = $3.5/2 = 1.75 \text{ in}$. Dividing			
8	into the moment yields the force on			
9	the stud			
10				
11				
12				
13				
14				
15	$= 43,000 / 1.75 = 24,600^{\#}$			
16				
17	which corresponds to a tensile stress			
18				
19				
20				
21	$= 24,600 / .60$			
22				
23	$= 41,000 \text{ psi}$			
24				
25				
26				
27				
28	The allowable stress in tension on the <u>stud</u>			
29				
30				
31				
32	$= .7 S_u = .7 (70000)$			
33				
34				
35	$= 49,000 \text{ psi} > 41,000$			
36	$F_s = 1.2$ <u>OK</u>			
37				
38	Hence, the integrity of the stud is demonstrated			
39	for pre-impact loads.			
40				
41				
42				
43				
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45				
46				

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>N/M(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQIE</u>	

Uniform Impact

From Table 8.5, a force $F \times K = 5$ Kips acts in compression at the disk center which is in no way carried by the pin or stud. Also acting is a moment $M \times K = 35$ in-k, resulting in a bending stress on the pin,

$$= \frac{M \times C}{I} = 35000 \left(\frac{2.5}{2} \right) / 1.9$$

$$= 23,000 \text{ psi} < 49000, \quad F_s = 2.1 \quad \text{OK}$$

which demonstrates the integrity of the pin under uniform impact loads.

Point Impact

These forces and moments from Table 8.3 are much lower than those resulting from pre-impact and uniform impact, because of the flexibility of the tail link in bending and

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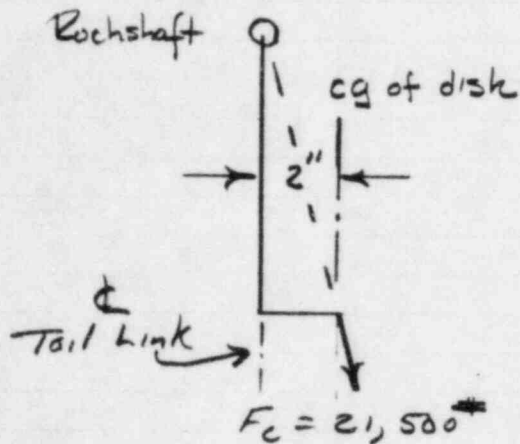
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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NA7(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	REV. <u>1</u>

torsion. Hence, integrity of the pin is demonstrated for point impact loading.

9.3 Tail Link (SA105)

Pre-Impact



The centrifugal force, F_c , loads the tail link in direct tension and bending. Assuming that the nut makes the full tail link section effective in bending, the total

tensile stress

$$= \frac{F_c}{Area} + F_c(2'' \text{ arm}) C/I$$

$$= \frac{21,500}{2} + 21,500(2)(.5)/.375$$

$$= 10,800 + 57,300 = 68,100 \text{ psi}$$

which is greater than the tensile allowable, but less than the ultimate stress of 70,000 psi. (OK)

FS = 1.03

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▲ 5010.65

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE
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Referring to pg A4 of Ref. 1, it is observed that the duration of this stress is less than 1 ms which is not of sufficient duration to develop the strain necessary for serious failure.

The deflection of the disk off the seat just prior to impact

$$= PL/AE = 21500(11.25)/4.5 \times 27E6$$
$$= .002 \text{ inches}$$

which is insignificant compared to the allowable deflection of .25". Hence, the integrity and operability of the tail link under pre-impact loading is demonstrated.

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<u>12210</u>	<u>NAT(C)</u>	<u>2043</u>	<u>SQE</u>	

Uniform Impact

The worst case bending for the tail link is caused by deflection of the disk center following impact with the seat, calculated in section 9.1 to be .66 inches including the contribution from the pressure surge. From Appendix A, the



plastic strain corresponding to a deflection, y_0 , of a simply supported beam of length = 2×11.25

$$\epsilon = \frac{\pi^2}{2} \frac{D y_0}{L^2} = \frac{\pi^2}{2} \frac{(1)(.66)}{(2 \times 11.25)^2}$$

$$FS = 26$$

$$= .006 < .7(22) = .154$$

(OK)

Since this occurs after the initial kickback loads, the contribution from shear is negligible.

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

The worst case shear loads, from Table 8.5, are 5 Kips at the rock shaft end and 5 Kips at the pin end. Maximum shear stress

$$= 5000 / \text{area} = 5000 / 4.5$$

$$= 1100 \text{ psi}$$

The allowable shear stress

$$= .75 S_u / \sqrt{3} = .7(70000) / 1.7$$

$$= 28,300 > 1100 \text{ psi}$$

$$FS = 26 \text{ (OK)}$$

Hence, integrity of the tail link is demonstrated for uniform impact loading.

Point Impact

Point impact loads, as seen from Table 8.5, are much smaller than pre- or uniform impact. Hence, integrity for point impact loads is demonstrated.

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9.4 Rock shaft (SS S7564 630, 1100°F draw)

Pre-impact

The rock shaft reacts the centrifugal force in double shear. The stress

$$= 21,500 / 2 (\text{Area})$$

$$= 21,500 / 2 (.79) = 13,600 \text{ psi}$$

which is below the allowable

$$= .7(133,000) / \sqrt{3}$$

$$= 53,800 > 13,600 \text{ psi} \quad FS = 4.0 \text{ OK}$$

Uniform Impact

From Table 8.5, the kick back load is 5 kips in double shear, resulting in a stress

$$= 5000 / (2 \times \text{area}) = 2500 / .79$$

$$= 3200 \text{ psi}$$

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

the allowable shear stress

$$= .754/\sqrt{3} = .7(133,000)/1.73$$

$$= 53,800 \text{ psi} > 3200, \quad F_s = 17 \text{ OK}$$

Point Impact

Referring to Table 8.3 shows that the point impact loads on the rock shaft are much smaller than the loads from pre-impact and uniform impact.

~

Hence, the rockshaft integrity is demonstrated for all critical loads.

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	REV. <u>1</u>

9.5 Seat/Disk Interface

The stress caused by impact in the seat and disk material at the interface is calculated from the hammer equation

$$\Delta \sigma = \frac{\Delta u}{c} E$$

where c is the speed of sound in steel.
Thus

$$\Delta \sigma = \frac{60}{16,400} 27E6 = 98,800 \text{ psi}$$

The allowable stress for

$$= .75 \sigma_u (\text{strain rate factor})$$

$$= .7 (70000) \sqrt{2} = 98000 \text{ psi}$$

Although slightly above the allowable, it is below ultimate and since this stress acts for a time less than $1/2$ millisecond (Fig. 8.2), very little damage to the

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CALCULATION IDENTIFICATION NUMBER				PAGE <u>9-15</u> REV. <u>1</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>MM(13)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQIE</u>	
1	<p>seat/disk interface is indicated. Hence,</p> <p>the leakage will remain within the</p> <p>limits of the RCIC or HPCS systems.</p> <p>Thus, the allowable becomes <u>$F_s = 1.4$</u></p> <p>$2 S_u = 2(70,000) = 140,000 > 98,800 \text{ psi}$ (OK)</p>			
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STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

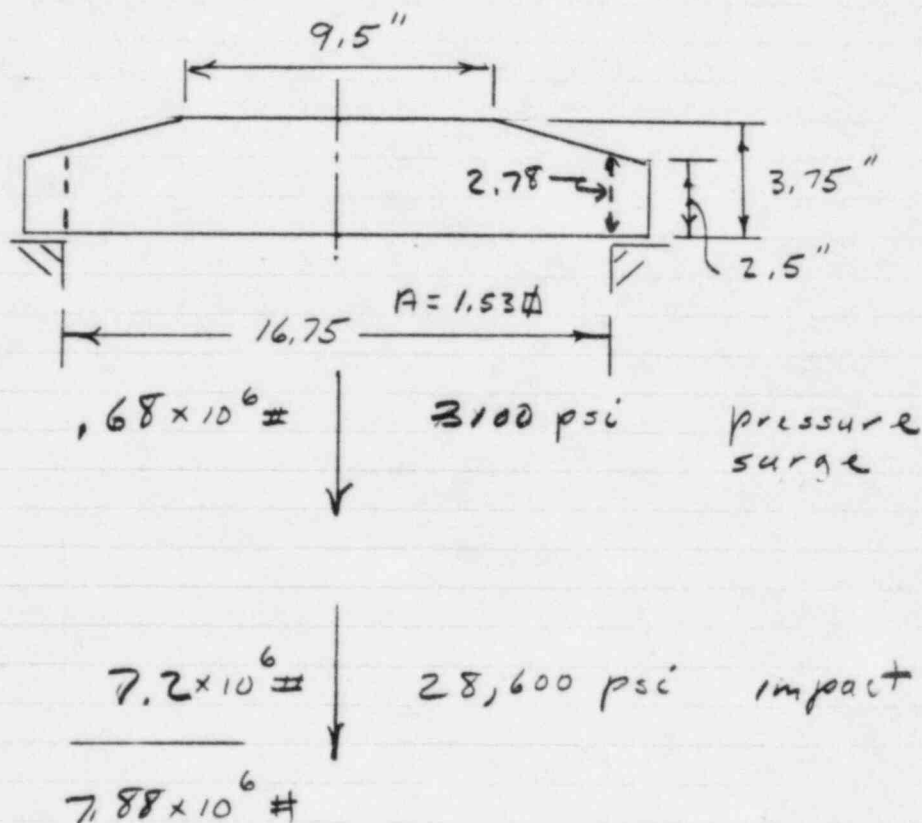
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CALCULATION IDENTIFICATION NUMBER				REV. 1
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE	PAGE 10-1 of 18

10. INTEGRITY OF ATWOOD/MORRILL CHECK VALVE 11321X ADVF32

10.1 Disk (SA 216 WCC)

Uniform impact of the disk on the seat generates a reaction of $7.2 \times 10^6 \#$, as given by $F \times K$ in Table 8.4. Acting simultaneously with impact is the pressure surge from rapidly closing the check valve.



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Shear Out

The allowable shear load in the disk at the seat inside diameter where the bending stress is zero

$$= .7 (S_u / \sqrt{3}) \cdot \text{shear area} \cdot \text{strain rate factor}$$

$$= .7 \left(\frac{70000}{1.73} \right) \cdot (16.75 \pi 2.78) (2)$$

$$= 8.28 \times 10^6 \text{ \#} > 7.88 \times 10^6 \quad F_s = 1.05 \text{ OK}$$

Rupture

The disk must absorb the impact energy at strains below the rupture level. The impact energy equals

$$= \frac{417}{32} (45)^2 \frac{12}{2} = 158,000 \text{ in \#}$$

The plastic strain energy of a circular disk from Appendix B, equation 5

$$= \frac{\pi}{2} \left(\frac{\pi}{2} - 1 \right) \sigma_0 t^2 y_0$$

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1				
2				
3	$= .89 (40000) (3.75)^2 y_0$			
4				
5				
6	$= 501,000 y_0$			
7				
8				
9	Equating to the impact energy and solving for			
10	the deflection, y_0 , at the disk center			
11				
12				
13				
14				
15	$y_0 = 158000 / 501000$			
16				
17	$= .32 "$			
18				
19				
20				
21	From Egn. 6, Appendix B, the maximum strain			
22	associated with this deflection is			
23				
24				
25				
26	$\epsilon = \frac{t}{2} \left(\frac{\pi}{15} \right)^2 y_0 = \frac{3.75}{2} \left(\frac{\pi}{16.75} \right)^2 (.32)$			
27				
28				
29				
30	$= .021$			
31				
32				
33	which includes the small contribution from the			
34	pressure surge by overstating the incoming			
35	energy (mass of disk).			
36				
37				
38				
39				
40	The allowable strain			
41				
42				
43	$= .7 \times \text{elongation} = .7 (.22)$			
44				
45	$= .154 > .021$			
46	$FS = 7.3 \quad \text{OK}$			

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$$= .89 (40000) (3.75)^2 y_0$$

$$= 501,000 y_0$$

Equating to the impact energy and solving for the deflection, y_0 , at the disk center

$$y_0 = 158000 / 501000$$

$$= .32$$

From Eqn. 6, Appendix B, the maximum strain associated with this deflection is

$$\epsilon = \frac{t}{2} \left(\frac{H}{D} \right)^2 y_0 = \frac{3.75}{2} \left(\frac{11}{16.75} \right)^2 (.32)$$

$$= .021$$

which includes the small contribution from the pressure surge by overstating the impact energy (mass of disk).

The allowable strain

$$= .7 \times \text{elongation} = .7 (.22)$$

$$= .154 > .021$$

$$FS = 7.3$$

(21)

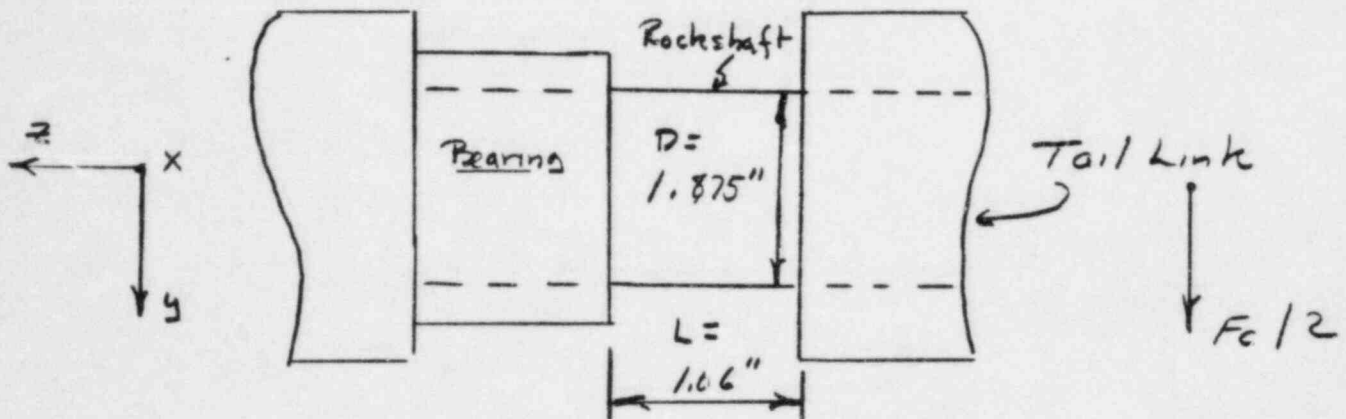
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Hence, the integrity of the disk is demonstrated.

9.2 Rock Shaft (A182 F6a)



Pre-Impact

The shear stress from the centrifugal force

$$= F_c / (2 \cdot \text{area of rock shaft})$$

$$= 26,300 / 2 (2.8)$$

$$= 4700 \text{ psi}$$

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J.O. OR W.O. NO. 12210	DIVISION & GROUP NMCC1	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE
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The allowable shear stress

$$= \frac{.7 S_u}{1.3} = .7 (70000) / 1.73$$

$$= 28300 \text{ psi} > 4700$$

$$F_s = 6$$

OK

The bending stress

$$= M c / I = \frac{26300}{2} (1.06) \frac{1.875}{2} \frac{1}{.61}$$

$$= 21,400 \text{ psi}$$

The allowable bending stress

$$= .7 S_u = .7 (70000)$$

$$= 49000 > 21,400 \text{ psi}$$

$$F_s = 2.3$$

OK

Combining via the stress interaction formulas of Reference 9, Appendix A9231

$$R_b = 21,400 / 49000 = .44$$

$$R_s = 4700 / 28300 = .17$$

Since $R = \sqrt{R_b^2 + R_s^2} = .47 < 1$, integrity under pre-impact loads is demonstrated

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12210	NAC	2043	SQE	

Uniform Impact

The rock shaft must carry a rich back load of 240 kips, as given in Table 8.4 for 1-XK. The resulting stress

$$= \frac{F \times K}{2} / \text{Area}$$

$$= \frac{240000}{2} (2.8)$$

$$= 33,600 \text{ psi}$$

The allowable shear stress

$$= .7 S_u \times \text{strain rate factor} / \sqrt{3}$$

$$= .7 (70000) (2) / 1.73$$

$$= 56,600 \text{ psi} > 33,600$$

$$F_s = 1.7 \text{ OK}$$

The bending stress

$$= M C / I = F \times K (2L) (C) / 8 I$$

$$= 242000^+ (1.06) \left(\frac{1.875}{2} \right) / 4 (.61)$$

$$= 98,600 \text{ psi}$$

* Used actual value from computer run.

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CALCULATION IDENTIFICATION NUMBER				
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the allowable bending stress

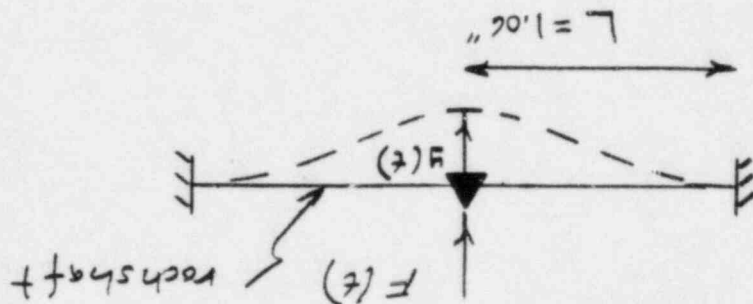
$$= 0.75 \times \text{strain rate factor}$$

$$= 0.7(70000)(2)$$

$$= 98,000 \text{ psi} < 99,000 \text{ allowed}$$

NG

A non-linear strain analysis is required to demonstrate integrity of the rockshaft.



The rockshaft is modelled as shown in the sketch. All the mass of the rockshaft and the tail link bushing are lumped at the point shown by the black triangle, driven by the time varying rockback force $F(t)$, obtained from the rockback loads analysis. Figure 10-1 shows the damped trace. Only the first peak will be used as shown on the figure. Both the shear and bending strains are

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computed as a function of time. Following the procedure of section 9.2.3.1 of Appendix A, reference 9, the interactive ratio is formed

$$I = \sqrt{K_s^2 + K_b^2} < 1$$

where, here

$$K_s = \text{shear strain / allowable}$$

$$K_b = \text{bending strain / allowable}$$

The differential equation governing the rockshaft response to load is given by

$$m''y + ky = F(t)$$

as long as primarily elastic conditions prevail. After essentially plastic conditions are achieved, the equation is switched by Fortran logic to

$$m''y + R_p = F(t)$$

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where R_p is the constant resistance to displacement. The maximum bending moment is given by

$$MOM = \frac{F(2L)}{8} = FL/4$$

The resistance R_p is the value of F when $MOM = M_p$, the plastic moment. Thus

$$R_p = 4M_p/L$$

For a circular section (Eqn. 4, App. A) this reduces to

$$R_p = \frac{8}{3\pi} \sigma_y A D/L$$

The elastic stiffness, k , for combined shear and bending, since this a short beam, was obtained from the expression for deflection under a concentrated load at the center

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$$\delta = \delta_{\text{bending}} + \delta_{\text{shear}}$$

From reference 15

$$\delta_{\text{bending}} = w(2L)^3 / 192 EI = wL^3 / 24 EI$$

For direct shear

$$\tau = \tau Q = \frac{S_s}{L} Q = w/2A$$

from which is obtained

$$\delta_{\text{shear}} = wL/2Aq$$

Combining bending with shear

$$\delta = wL^3/24EI + wL/2Aq$$

which provides the stiffness

$$k = w/\delta = \frac{1}{L^3/24EI + L/2Aq}$$

for elastic conditions.

From Appendix A, the relation between deflection y and the maximum strain is, from eqn. 14 therein

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1	$E = \frac{D}{2} \frac{1}{P} = \frac{\pi^2}{4} \frac{P_y}{L^2}$ <p>These equations were coded for computation using the IBM CSMP III simulation program. Details of the coding and results are provided in the following pages. The results show that the interactive strain ratio is less than one, i.e.</p> $R = .65 < 1$ <p>Hence, integrity under uniform impact loading is demonstrated.</p>			
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J.O. OR W.O. NO.

DIVISION & GROUP

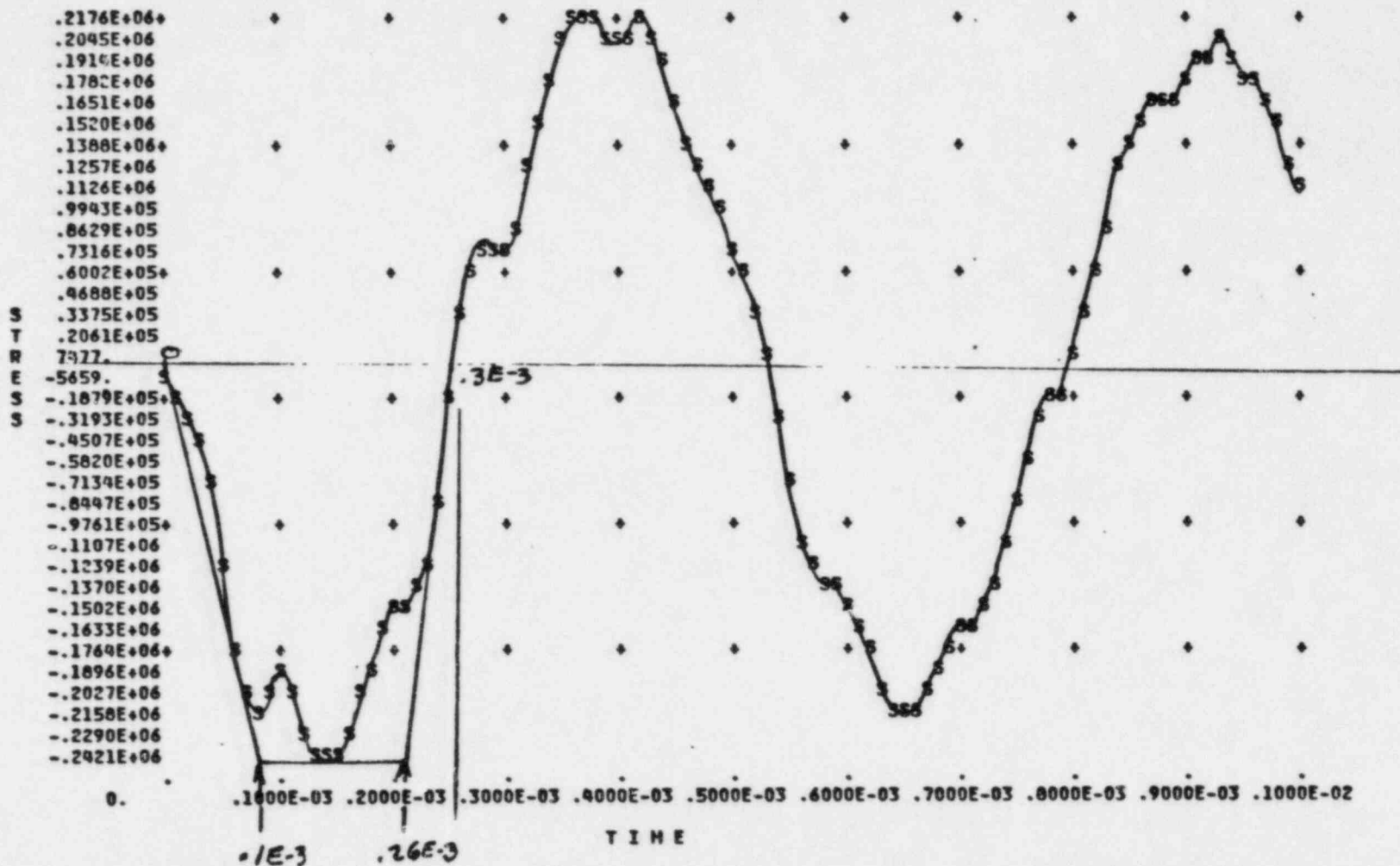
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FIG. 10-1

(STRESS VECTOR NO. 36) EQUILB CHK RESPONSE FOR ELEMENT NUMBER 30, STRESS TYPE 1 (MOD/DOF X1)



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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SOE</u>							
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6	***CONTINUOUS SYSTEM MODELING PROGRAM III VIH1 TRANSLATOR OUTPUT***									
7	TITLE TIME HISTORY STRAIN IN THE ROCKSHAFT FROM KICKBACK LOAD									
8	TITLE 1B21*AOV032A CHECK VALVE									
9	TITLE GSU/RIVER BEND POWER STATION - UNIT 1									
10	* STORED JMG.RS032A:210									
11	FUNCTION TH=(0.,0.),(.1E-3,.242E6),(.26E-3,.242E6),(.3E-3,0.),... (.5E-3,0.)									
12	INITIAL									
13	PARAM MTG=71.,E=27.E6,GS=11.E6,L=1.06,A=2.8,IRS=.61,D=1.88									
14	* ROCKSHAFT MATERIAL IS SS A182 F6A, TEMP=500 F									
15	* SIGY=34,700*1.2 = 41,600 PSI FROM APP. F FOR SUPPORTS									
16	PARAM EL=.18,SIGY=41600.,PI=3.14									
17	ALEL3=EL									
18	ALEL3=AELB/1.73									
19	CONST G=386.									
20	H=MTG/G									
21	RP=.84*SIGY*A*D/L									
22	EK=PI**2*D/(4.*L**2)									
23	K=1./(L**3/(24.*E*IRS)+L/(2.*A*GS))									
24	DYNAMIC									
25	FT=AFGEN(TI,TIME)									
26	DDY=(FT-RY)/H									
27	PROC RY=FUNC(RP)									
28	RY=H*Y									
29	IF(RY.GT.RP)RY=RP									
30	ENDPROC									
31	DY=INTGRL(0.,DDY)									
32	Y=INTGRL(0.,DY)									
33	BSTRN=E*K*Y									
34	SIG1=E*BSTRN									
35	SIG2=H*H*D/(2.*IRS)									
36	HOM=FT*L/Q.									
37	SSTRN=Y/L									
38	* 'RATIO' IS CALCULATED IN ACCORDANCE WITH APPENDIX A9231 OF THE									
39	* ASME III B&PV CODE									
40	RATIO=SQRT((BSTRN/ALELB)**2+(SSTRN/ALEL3)**2)									
41	TAU1=GS*SSTRN									
42	TAU2=FT/(2.*A)									
43	METHOD RKSFX									
44	* RANGE BSTRN,SSTRN,SIG1,SIG2,TAU1,TAU2,Y,MP,RP,RE,RATIO									
45	TIHER FINTIH=.00043,PROEL=.00001,DELT=.0000001									
46	PRINT RATIO,Y,FT,RY									
	- TERMINAL									
	WRITE(6,10)K									
	10 FORMAT(/5X,'STIFFNESS OF ROCKSHAFT(K) =',E15.3)									
	END									
	STOP									
	OUTPUT VARIABLE SEQUENCE									
	ALELB	ALEL3	H	RP	EK	K	FT	RY	DDY	DY
	-Y	BSTRN	SIG1	HOM	SIG2	SSTRN	RATIO	TAU1	TAU2	ZZ1004

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12210

DIVISION & GROUP

H/H(C)

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REV. 1

1

TIME HISTORY STRAIN IN THE ROCKSHAFT FROM KICKBACK LOAD
 1B21MAOV032A CHECK VALVE
 GSU/RIVER BEND POWER STATION - UNIT 1

TIME	RATIO	Y	FT	RY
.0	.0	.0	6.2500E-02	.0
1.000000-05	5.3960E-05	2.1898E-06	24200.	108.29
2.000000-05	4.2994E-04	1.7448E-05	48400.	862.86
3.000000-05	1.4413E-03	5.8491E-05	72600.	2892.6
4.000000-05	3.3844E-03	1.3735E-04	96800.	6792.3
5.000000-05	6.5305E-03	2.6502E-04	1.2100E+05	13106.
6.000000-05	1.1119E-02	4.5121E-04	1.4520E+05	22314.
7.000000-05	1.7349E-02	7.0406E-04	1.6940E+05	34818.
8.000000-05	2.5378E-02	1.0299E-03	1.9360E+05	50932.
9.000000-05	3.5314E-02	1.4331E-03	2.1780E+05	70873.
1.000000-04	4.7213E-02	1.9160E-03	2.4200E+05	94755.
1.100000-04	6.1027E-02	2.4766E-03	2.4200E+05	1.2248E+05
1.200000-04	7.6438E-02	3.1020E-03	2.4200E+05	1.5341E+05
1.300000-04	9.3037E-02	3.7757E-03	2.4200E+05	1.7353E+05
1.400000-04	.11057	4.4870E-03	2.4200E+05	1.7353E+05
1.500000-04	.12901	5.2356E-03	2.4200E+05	1.7353E+05
1.600000-04	.14838	6.0215E-03	2.4200E+05	1.7353E+05
1.700000-04	.16866	6.8446E-03	2.4200E+05	1.7353E+05
1.800000-04	.18986	7.7050E-03	2.4200E+05	1.7353E+05
1.900000-04	.21198	8.6026E-03	2.4200E+05	1.7353E+05
2.000000-04	.23501	9.5372E-03	2.4200E+05	1.7353E+05
2.100000-04	.25895	1.0509E-02	2.4200E+05	1.7353E+05
2.200000-04	.28382	1.1518E-02	2.4200E+05	1.7353E+05
2.300000-04	.30960	1.2564E-02	2.4200E+05	1.7353E+05
2.400000-04	.33630	1.3648E-02	2.4200E+05	1.7353E+05
2.500000-04	.36391	1.4768E-02	2.4200E+05	1.7353E+05
2.600000-04	.39245	1.5926E-02	2.4200E+05	1.7353E+05
2.700000-04	.42176	1.7116E-02	1.8150E+05	1.7353E+05
2.800000-04	.45119	1.8310E-02	1.2100E+05	1.7353E+05
2.900000-04	.47990	1.9476E-02	60501.	1.7353E+05
3.000000-04	.50711	2.0580E-02	.0	1.7353E+05
3.100000-04	.53212	2.1595E-02	.0	1.7353E+05
3.200000-04	.55481	2.2516E-02	.0	1.7353E+05
3.300000-04	.57518	2.3342E-02	.0	1.7353E+05
3.400000-04	.59322	2.4074E-02	.0	1.7353E+05
3.500000-04	.60893	2.4712E-02	.0	1.7353E+05
3.600000-04	.62232	2.5255E-02	.0	1.7353E+05
3.700000-04	.63339	2.5704E-02	.0	1.7353E+05
3.800000-04	.64213	2.6059E-02	.0	1.7353E+05
3.900000-04	.64854	2.6319E-02	.0	1.7353E+05
4.000000-04	.65264	2.6485E-02	.0	1.7353E+05
4.100000-04	.65440	2.6557E-02	.0	1.7353E+05
4.200000-04	.65384	2.6534E-02	.0	1.7353E+05
4.300000-04	.65096	2.6417E-02	.0	1.7353E+05
4.400000-04	.64575	2.6206E-02	.0	1.7353E+05
4.500000-04	.63822	2.5900E-02	.0	1.7353E+05

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Point Impact

Point impact stresses occur before uniform impact stresses and therefore are treated independently. From Table 8, 2, the worst case loads - the rock shaft are 60 kips (F_{xx}) and 160 in-kips (M_{yx}). These combine to load the rock shaft

$$= \frac{F_{xx}}{2} + \frac{M_{yx}}{GIP} \quad (\text{maximum})$$

$$= \frac{60}{2} + 160 / 16.4$$

$$= 40 \text{ in Kips}$$

Since this is much smaller than the load from uniform kick back, no further analysis is required.

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J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SPE
			PAGE 10-16 REV. 1

10.3 Tail link

From Table 8.4, the moment acting on the weakest section of the tail link, where it joins the dish 4.5" from the rock shaft, the bending moment is 960 in-kips from uniform impact. The bending stress is calculated

$$\sigma_b = \frac{M c}{I} = 960000 \left(\frac{2.9}{2} \right) / 30.$$

$$= 46,400 \text{ psi}$$

The allowable tensile stress

$$= .7 S_u (\text{strain rate factor})$$

$$= .7 (70000) (2) = 98000 > 46400 \text{ psi (OK)}$$

The shear stress acting on this section

$$= 240000 / 43 = 5600 \text{ psi}$$

the allowable shear stress

$$= .7 \left(\frac{70000}{1.73} \right) (\text{strain rate factor})$$

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J.O. OR W.O. NO. <u>17210</u>	DIVISION & GROUP <u>NA9(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

$$= 56,600 \text{ psi} > 5600$$

$$F_s = 10 \quad \text{OK}$$

Applying section 9231 of appendix A, ref. 9,

$$R = \sqrt{R_z^2 + R_b^2} < 1$$

$$= \left[\left(\frac{5600}{56,600} \right)^2 + \left(\frac{46400}{98000} \right)^2 \right]^{1/2}$$

$$= .48 < 1$$

$$F_s = 2.1 \quad \text{OK}$$

Hence, the integrity of the tail link is demonstrated. For a strain rate factor of 1 instead of 2, this ratio becomes

$$R = .48(2) = .96 < 1$$

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10.4 Seat/Dish Interface

The stress in the dish and seat at their interface is calculated using the familiar water hammer equation

$$\Delta \sigma = \frac{\Delta u}{c}$$

where Δu = the impact speed and c = the sonic speed in steel. Thus

$$\Delta \sigma = \frac{45}{16,400} 27 \times 10^6 = 74,000 \text{ psi compression}$$

The allowable tensile stress

$$= .7 S_u (\text{strain rate factor})$$

$$= .7(70000)2 = 98,000 \text{ psi} > 74000$$

FS=1.3

(OK)

Hence, the integrity of the seat/dish interface is established, from which it is concluded that the leakage will be within the make-up capability of the RCIC or HPCS systems.

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APPENDIX A

Plastic Strain Energy for Beams of Various Cross-section

PLASTIC STRAIN ENERGY FOR BEAMS OF VARIOUS CROSS-SECTION

Ignoring the initial elastic stage, the plastic strain energy is given by the expression

$$E = \int_0^L M_p d\theta \quad (1)$$

where M_p is the plastic bending moment and $d\theta$ is the angle through which this moment is rotated. For physical reasons, the integral is always positive.

Referring to Figure 2 showing a differential element in the line

$$M_p = Fa = \sigma_y A a/2$$

since the line stress is at yield, σ_y (assumed for simplicity). The distance 'a' between the center of area above and below the neutral axis is given by the integral expression (Figure 3).

$$\begin{aligned} \frac{A}{2} \frac{a}{2} &= \int_0^\pi z dA = \int_0^\pi (rs \sin \alpha) (rt d\alpha) \\ \frac{a}{2} &= \frac{2}{A} r^2 t \int_0^\pi \sin \alpha d\alpha = -\frac{2r^2 t}{A} \cos \alpha \Big|_0^\pi = \frac{4r^2 t}{A} \\ &= \frac{4r^2 t}{2\pi r t} = \frac{D}{\pi} \end{aligned} \quad (2)$$

Thus, the plastic bending moment becomes

$$M_p = \sigma_y A \frac{D}{\pi} \quad (3)$$

for a thin circular tube. By a similar analysis, it is shown that the plastic moment for a solid circular section is given by

$$M_p = \frac{4}{3} r^3 \sigma_y = \frac{2}{3} \sigma_y A D/\pi \quad (4)$$

For a rectangular section of height, h , and width, b

$$M_p = \frac{1}{4} \sigma_y A h$$

To determine $d\theta$, assume that the deflection curve shown by the dashed line in Figure 1 is approximated by

$$y = y_0 \cos \pi x/L \quad (5)$$

Differentiating twice, we obtain

$$y' = \theta = -y_0(\pi/L) \sin \pi x/L \quad (6)$$

$$y'' = \theta' = 1/\rho = -y_0(\pi/L)^2 \cos \pi x/L \quad (7)$$

where $1/\rho$ = curvature of the beam. Noting that $d\theta = \theta' dx$, the expression for energy given by equation 1 becomes

$$E_B = \int M_p d\theta = \int M_p y_0 (\pi/L)^2 \cos \pi x/L dx$$

Calling $\pi x/L = \xi$, the integration reads

$$E_B = (\pi/L) y_0 M_p \int_{-\pi}^{\pi} \cos \xi d\xi$$

Remembering that the integrand is never less than zero since energy is always absorbed, change the limits to $\pi/2$ and zero, and multiply by 4

$$E_B = (4y_o/L) \pi M_p \sin \left| \frac{\pi}{2} \right|_0$$

which yields for the plastic strain energy in bending

$$E_B = 4 \pi M_p y_o/L \quad (8)$$

The displacement, y_o , must now be related to strain, ϵ , in order to determine acceptable damage. Referring to Figure 2

$$\epsilon = (ds-dx)/dx = ds/dx - 1$$

Since $ds = (\rho + r) d\theta$, the expression for strain can be written

$$\epsilon = (\rho + r) d\theta/dx - 1$$

But, from equation 7, $d\theta/dx = 1/\rho$ so that

$$\epsilon = (\rho + r)/\rho - 1 = r/\rho$$

Replacing $1/\rho$ by its equivalent from equation 7

$$\epsilon = r \left(\frac{\pi}{L} \right)^2 y_o \cos \frac{\pi x}{L}$$

which has for its maximum

$$\epsilon = \frac{\pi^2}{2} \frac{Dy_o}{L^2} \quad (9)$$

$$\frac{y_o}{L} = \frac{2}{\pi^2} \frac{L}{D} \epsilon \quad (10)$$

Now, replacing y_o by ϵ in equation 8 yields

$$E_B = \frac{8 L}{\pi D} M_p \epsilon \quad (11)$$

for the strain energy of a beam pinned at both ends.

For a beam fixed at both ends, the assumed deflection curve is

$$y = \frac{y_o}{2} \left(\cos \frac{2\pi x}{L} + 1 \right) \quad (12)$$

By a similar analysis to the simply supported case, the plastic strain energy in bending for a beam with both ends fixed

$$E_B = 4\pi Mp \frac{y_o}{L} \quad (13)$$

$$\epsilon = \frac{D}{2} \frac{1}{\rho} = \pi^2 \frac{Dy_o}{L} \quad (14)$$

$$\frac{y_o}{\pi} \frac{1}{2} \frac{L}{D} \epsilon \quad (15)$$

which yields for the plastic strain energy in terms of strain

$$E_B = \frac{4}{\pi} Mp \frac{L}{D} \epsilon \quad (16)$$

For a beam with one end fixed and the other end pinned, the assumed deflection curve for uniform loading is given by the elastic curve obtained from any edition of Rourk's Formulas for Stress and Strain

$$y = 3.86 y_o \left[3 \left(\frac{X}{L} \right)^3 - 2 \left(\frac{X}{L} \right)^4 - \frac{X}{L} \right] \quad (17)$$

where X is measured from the pinned end. Again, a similar analysis leads to the result for plastic strain energy in bending

$$E_B = 3\pi Mp \frac{y_o}{L} \quad (18)$$

$$\epsilon = 1.2 \pi^2 \frac{Dy_o}{L^2} \quad (19)$$

$$\frac{y_o}{L} = \frac{.83}{\pi^2} \frac{L}{D} \epsilon \quad (20)$$

which yields for the plastic strain energy in terms of strain

$$E_B = \frac{2.5}{\pi} M_p \frac{L}{D} \epsilon \quad (21)$$

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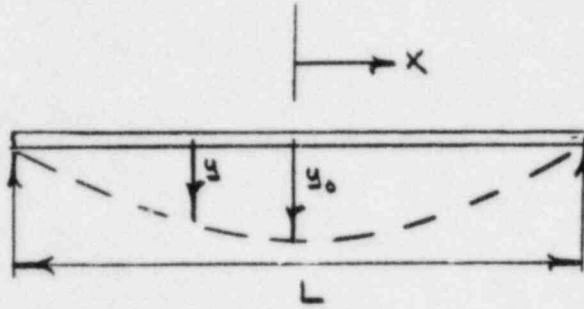


FIGURE 1

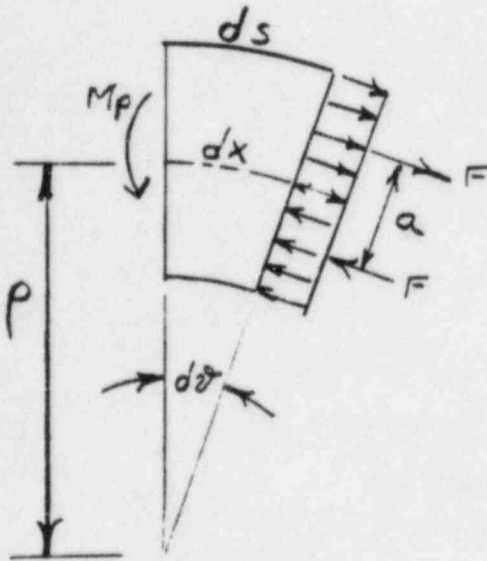


FIGURE 2

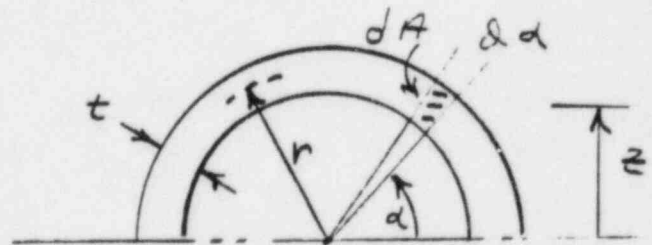


FIGURE 3

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APPENDIX B

Plastic Strain Energy for a Circular Plate in Bending

PLASTIC STRAIN ENERGY FOR A CIRCULAR PLATE

The strain energy of a plate in bending is derived below for a purely plastic response to a uniform loading. Ignoring the elastic contribution greatly simplifies the analysis and is conservative.

A deflection curve that satisfies the simply-supported boundary conditions is assumed

$$y = y_0 \cos \frac{\pi r}{2R} \quad (1)$$

$$y' = y_0 \left(\frac{\pi}{2R} \right) \sin \frac{\pi r}{2R} \quad (2)$$

$$y'' = y_0 \left(\frac{\pi}{2R} \right)^2 \cos \frac{\pi r}{2R} = \frac{1}{\rho} = \frac{d\theta}{dr}$$

Referring to the Figure, the strain energy is given by

$$E_s = \int M_p d\theta \quad (3)$$

summed over the plate where M_p is the moment for fully plastic loading. Referring to the figure, because of symmetry, the plastic moment acting at radius, r , is simply the unit moment, M_p times the circumference at radius, r .

$$M_p = \left(\frac{t}{2} \right)^2 \sigma_y (2\pi r) \quad (4)$$

Inserting into the integral

$$E_s = \int_0^R \left(\frac{t}{2} \right)^2 \sigma_y (2\pi r) y_0 \left(\frac{\pi}{2R} \right)^2 \cos \frac{\pi r}{2R} dr$$

which reduces to

$$E_s = \left(\frac{t}{2}\right)^2 \sigma_y y_o 2\pi \int_0^{\pi/2} \xi \cos \xi d\xi$$

with $\xi = \frac{\pi r}{2R}$

Integrating,

$$\begin{aligned} E_s &= 2\pi \sigma_y \left(\frac{t}{2}\right)^2 y_o [\cos \xi + \xi \sin \xi]_0^{\pi/2} \\ &= \frac{\pi}{2} \left(\frac{\pi}{2} - 1\right) \sigma_y t^2 y_o \end{aligned} \quad (5)$$

The plastic strain by definition equals

$$\begin{aligned} \epsilon &= \frac{ds-dr}{dr} = \frac{ds}{dr} - 1 \\ &= \frac{(\rho+t/2)d\theta}{\rho d\theta} - 1 = t/2\rho \end{aligned}$$

Since $1/\rho = \theta'$, this becomes

$$\epsilon = \frac{\pi^2}{8} \frac{ty_o}{R^2} \quad (6)$$

which can be used to eliminate y_o from the energy equation

$$E_s = \frac{2}{\pi} \left(1 - \frac{2}{\pi}\right) \sigma_y V\epsilon \quad (7)$$

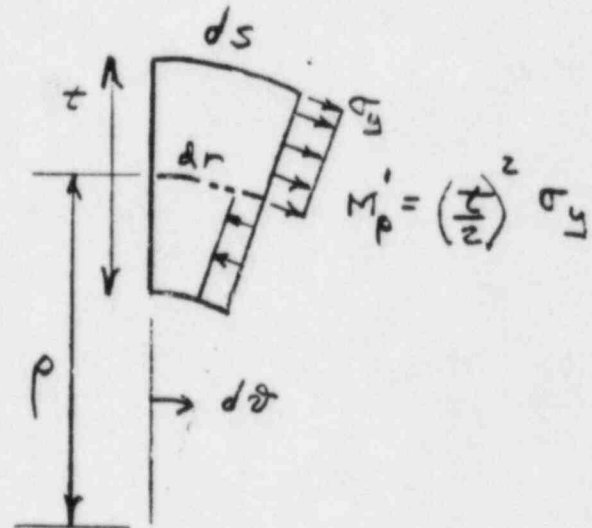
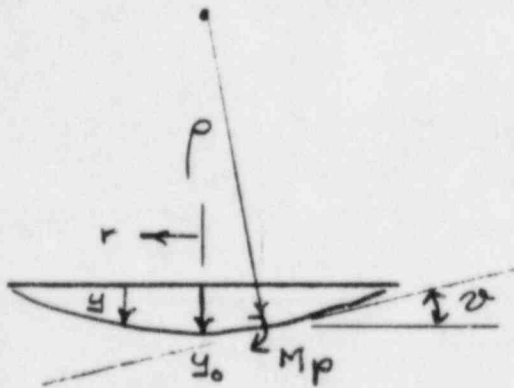
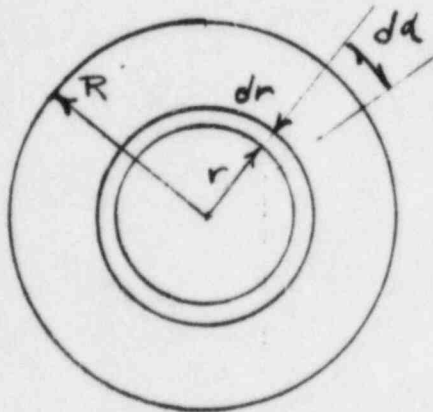
where V = volume of the circular plate ($\pi R^2 t$) and ϵ is the maximum strain.

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APPENDIX C

Miscellaneous Reference Material

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Safe Impact Speed for Hollow Circular Cylinders

An analog simulation of stress wave propagation in hollow circular cylinders with, and without, end caps for the purpose of determining safe impact velocity. The results were checked with experimental tests run by Sandia Corporation.

Introduction

THE PRESENT paper deals with problems of axisymmetric elastic wave propagation in moderately thick-walled hollow cylinders with a flat end cap, Fig. 1. The analysis was done to provide design engineers with a program with which to

1. Check the safety of cylinders from fracture due to end-on impact.

2. Predict the safe speed of end-on impact without fracture for various designs to establish the effects of each variable—i.e., material, length, width, etc.

The impact speed is of the order of hundreds ft/sec so that the shear effect¹ is predominant and any initiation of fracture is to be considered failure. Thus the fracture could be caused by the shear wave, as observed from many geological phenomena such as "shear cones" of volcanoes and meteor impact. With the foregoing objectives and restricting this work to polycrystalline material of low ductility, it is postulated that the initiation of fracture will occur before there is time for plastic flow [1]² and the linear theory of elastic applies.

The mathematical model for the fracture initiation of a closed-end hollow cylinder is essentially the Pochhammer-Chree model [2, 3]. The problem of uncapped hollow circular cylinders was treated by Ghosh [4], McFadden [5], Hermann and Mirsky [6], and more recently, by Gazis [7] whose work Fitch [8] verified with experiments on 5052 aluminum-alloy cylinders.

¹At high pressure, the shock wave is hydrodynamic in nature. Fracture then is mostly due to the "spalling" effect.

²Numbers in brackets designate References at end of paper.

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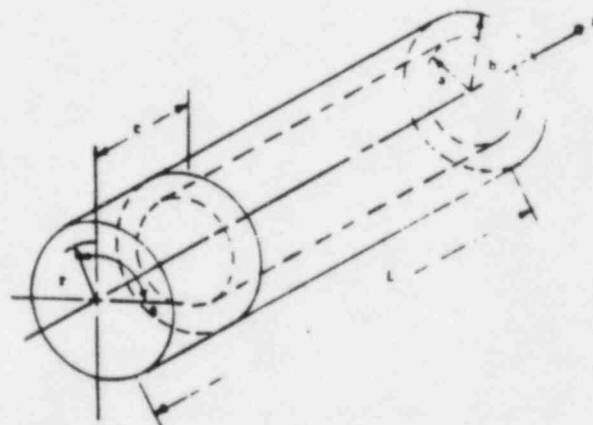


Fig. 1 Region of analysis and definition of coordinate system

The Pochhammer-Chree model may be treated by (a) analytical methods or by (b) numerical methods. The analytical methods may involve the use of transform calculus (e.g., DeVault and Curtis [9]) or where displacements are assumed to be harmonic (e.g., Pochhammer-Chree [2, 3], and Gazis [7]). The analytical methods are generally restricted to pressure bars of relatively simple geometry such as a solid bar or a hollow cylinder.

The numerical methods involve use of (a) the digital computer (e.g., Thorne and Hermann [10]) or (b) the simulation technique. The digital programming, due to intrinsic stability problem has, in the past, introduced artificial damping terms either in the governing differential equation or in the numerical method. The criticism is that in order to gain stability, the basic characteristic of the dynamic system is altered. Lax and Wendroff [12] offered a method of getting stable numerical solutions and later, after this work was started, Clifton [13], using the Lax-Wendroff

method, offers a method of solving plane problems. Still later, Berthoff [14] offers further work on the solutions of two-dimensional wave propagation of finite bars. The present paper employs a simulation technique to solve the problem of closed-end hollow cylinder which does not necessitate the introduction of any artificial damping terms. In addition, and perhaps the main advantage of simulating by analog computer is that the designer can vary parameters to obtain an optimum design. Parameters can be varied on both digital and analog but with the following tradeoff—Berthoff indicates a solution time on the digital of approximately 1 hr, the analog solutions are of the order of 3 sec. Resetting of new parameters takes seconds on the digital and less than 5 min on the analog. Since many solutions are required, the analog offered a great savings in both cost and time.

Mathematical Model

Interior. The basic governing differential equation for the displacement field, $u(r, z, t)$ in the body is

$$c_d^2 \text{grad div } u - 2c_s^2 \text{curl } \omega = \ddot{u} \quad (1)$$

where $\omega(r, z, t)$ is the rotation field such that $\omega = \frac{1}{2} \text{curl } u$, c_d and c_s are the dilatational and shear wave speeds, respectively, and $\ddot{u} = (\partial^2 u / \partial t^2)$. In terms of axisymmetric cylindrical coordinates (r, z) , equation (1) may be written

$$c_d^2 \ddot{u}_r = \frac{1}{r} (ru_{r,r})_r - \frac{u_r}{r^2} + \left(\frac{c_s}{c_d}\right)^2 u_{r,z,z} + \left(1 - \frac{c_s^2}{c_d^2}\right) u_{r,z,z} \quad (1a)$$

$$c_d^2 \ddot{u}_z = u_{r,r} + \left(\frac{c_s}{c_d}\right)^2 \frac{1}{r} (ru_{r,z})_z + \left(1 - \frac{c_s^2}{c_d^2}\right) u_{r,z} + \frac{u_{z,z}}{r} \quad (1b)$$

where (u_r, u_z) are the radial and longitudinal components of u , the comma in the subscripts denotes partial differentiation with respect to the coordinates following the comma.

Boundaries. On the boundaries, the following constraints must be met, Fig. 1:

On $r = a$, $z > c$, $\sigma_{rz} = 0$ ($i = r, \theta, z$). Therefore

$$\left[2\alpha^2 u_{r,z} + (1 - \alpha^2) \left(u_{r,z} + \frac{u_r}{r} + u_{z,z} \right) \right] = 0 \quad (2a)$$

$$[u_{r,z} + u_{z,z}] = 0 \quad (2b)$$

On $r = b$, $z < 0$ ($i = r, \theta, z$). Therefore

$$\left[2\alpha^2 u_{r,z} + (1 - \alpha^2) \left(u_{r,z} + \frac{u_r}{r} + u_{z,z} \right) \right] = 0 \quad (3a)$$

$$[u_{r,z} + u_{z,z}] = 0 \quad (3b)$$

On $z = c$, $r < a$, $\sigma_{rz} = 0$ ($i = r, \theta, z$). Therefore

$$\left[2\alpha^2 u_{r,z} + (1 - \alpha^2) \left(u_{r,z} + \frac{u_r}{r} + u_{z,z} \right) \right] = 0 \quad (4a)$$

$$[u_{r,z} + u_{z,z}] = 0 \quad (4b)$$

At the corner $r = a$, $z = c$, $\sigma_{rz} = 0$, $i = r, \theta, z$ where r and z are rotated about c by 45 deg. Therefore

$$\sigma_{rz} = \frac{\sigma_{rz} + \sigma_{zz}}{2} - \sigma_{rr} \quad \text{and} \quad \sigma_{zz} = \frac{\sigma_{rz} - \sigma_{rr}}{2}$$

This makes the following restrictions on displacements:

$$(1 - \alpha^2)(u_{r,z} + u_{z,z}) + (1 - 2\alpha^2) \frac{u_r}{r} - \alpha^2(u_{r,z} + u_{z,z}) = 0 \quad (5a)$$

$$u_{r,z} - u_{z,z} = 0 \quad (5b)$$

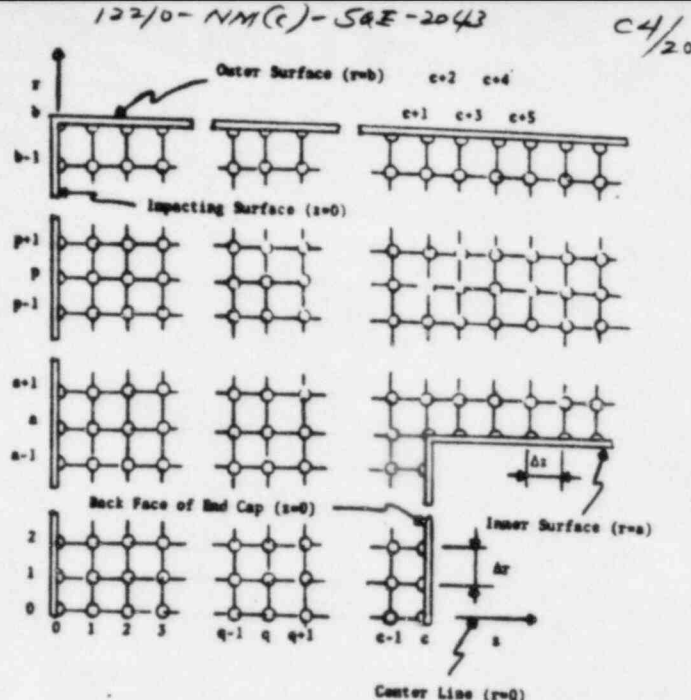


Fig. 2 Finite-difference network

where $\alpha = c_s/c_d$. The equations of motion (1a) and (1b) subject to the boundary values (2)–(5) along with the input loading specification completely define the mathematical model.

Analog Model

Adaptions of Equations to Analog. Before the partial differential equations can be solved by the analog simulation, the problem must be converted to a system of linear difference-differential equations by converting spatial variables (r, z) to discrete variables, while time (t) is retained as the continuous variable. The spatial position in (r, z) is therefore designated as junction points (p, q) , respectively, Fig. 2, in a two-dimensional net. The governing equations (1) become the following second-order difference-differential equations.

$$\left(\frac{\Delta r}{c_d}\right)^2 \frac{d^2}{dt^2} (u_r^{p,q}) = 2Au_r^{p,q} + B_1u_r^{p+1,q} + B_2u_r^{p-1,q} + D_1(u_r^{p,q+1} + u_r^{p,q-1}) + E_1(u_r^{p+1,q+1} - u_r^{p-1,q+1} - u_r^{p+1,q-1} + u_r^{p-1,q-1}) \quad (6a)$$

$$\left(\frac{\Delta r}{c_d}\right)^2 \frac{d^2}{dt^2} (u_z^{p,q}) = 2Ku_z^{p,q} + u_r^{p,q+1} + u_r^{p,q-1} + D_2(B_1u_r^{p+1,q} + B_2u_r^{p-1,q}) + E_2(u_r^{p+1,q+1} - u_r^{p-1,q+1} - u_r^{p+1,q-1} + u_r^{p-1,q-1}) + M(u_r^{p,q+1} + u_r^{p,q-1}) \quad (6b)$$

where Δr and Δz are mesh sizes in r, z , respectively, and $u_r^{p,q}$ ($i = r, z$) is the value of u_i at $r = p\Delta r$ and $z = q\Delta z$.

$$A = 1 + \alpha^2\beta^2 + \frac{1}{2}\gamma, \quad \beta \text{ (mesh ratio)} = \frac{\Delta r}{\Delta z},$$

$$\gamma \text{ (radial size ratio)} = \frac{\Delta r}{r}$$

$$B = 1 + \frac{1}{2}\gamma, \quad B_1 = 1 - \frac{1}{2}\gamma, \quad D_1 = \alpha^2\beta^2, \quad D_2 = \alpha^2\beta^2$$

$$E_1 = \frac{1}{4}\beta(1 - \alpha^2), \quad E_2 = \frac{1}{4}\beta(1 - \alpha^2),$$

$$K = 1\alpha^2\beta^{-2}, \quad M = \frac{(1 - \alpha^2)\gamma}{2\beta}$$

The boundary conditions are also described by the difference

$$u_r^{p+1,q} = u_r^{p,q} + (1 - 2\alpha^2)2\gamma u_r^{p,q}$$

$$+ \beta(u_r^{p,q+1} - u_r^{p,q-1}) \quad (7a)$$

$$p = a$$

$$q = c$$

$$u_r^{p+1,q} = u_r^{p,q} + \beta(u_r^{p,q+1} - u_r^{p,q-1}) \quad (7b)$$

$$u_r^{p+1,q} = u_r^{p,q} + (1 - 2\alpha^2)2\gamma u_r^{p,q}$$

$$+ \beta(u_r^{p,q+1} - u_r^{p,q-1}) \quad (8a)$$

$$p = b$$

$$q = 0$$

$$u_r^{p+1,q} = u_r^{p,q} - \beta(u_r^{p,q+1} - u_r^{p,q-1}) \quad (8b)$$

$$u_r^{p+1,q} = u_r^{p,q} - \frac{1 - 2\alpha^2}{\beta} (u_r^{p,q+1} - u_r^{p,q-1})$$

$$= u_r^{p,q} + 2\gamma u_r^{p,q} \quad (9a)$$

$$p = a$$

$$q = c$$

$$u_r^{p+1,q} = u_r^{p,q} - \frac{1}{\beta} (u_r^{p,q+1} - u_r^{p,q-1}) \quad (9b)$$

$$u_r^{p+1,q} = u_r^{p,q} - \frac{4(1 - \alpha^2)\Delta r}{\alpha^2 G_1} u_r^{p,q} + \frac{2\beta}{G_1} (u_r^{p,q+1} - u_r^{p,q-1})$$

$$- \frac{G_2}{G_1} (u_r^{p+1,q+1} - u_r^{p+1,q-1}) - \frac{2\beta}{G_1} (u_r^{p,q+1} - u_r^{p,q-1}) \quad (10a)$$

$$p = a$$

$$q = c$$

$$u_r^{p+1,q} = u_r^{p,q} + \frac{4\beta(1 - 2\alpha^2)}{\alpha^2 G_1} u_r^{p,q} - \frac{G_2}{G_1} (u_r^{p+1,q+1} - u_r^{p+1,q-1})$$

$$- \frac{2\beta}{G_1} (u_r^{p,q+1} - u_r^{p,q-1}) + \frac{G_2}{G_1} (u_r^{p,q+1} - u_r^{p,q-1}) \quad (10b)$$

where

$$G_1 = 1 + \beta^2 + \frac{2\beta}{\alpha^2} (1 - \alpha^2) \quad \gamma_c = \gamma \text{ with } c = a$$

$$G_2 = 1 + \beta^2 - \frac{2\beta}{\alpha^2} (1 - \alpha^2) \quad \gamma_c = \gamma \text{ with } c = b$$

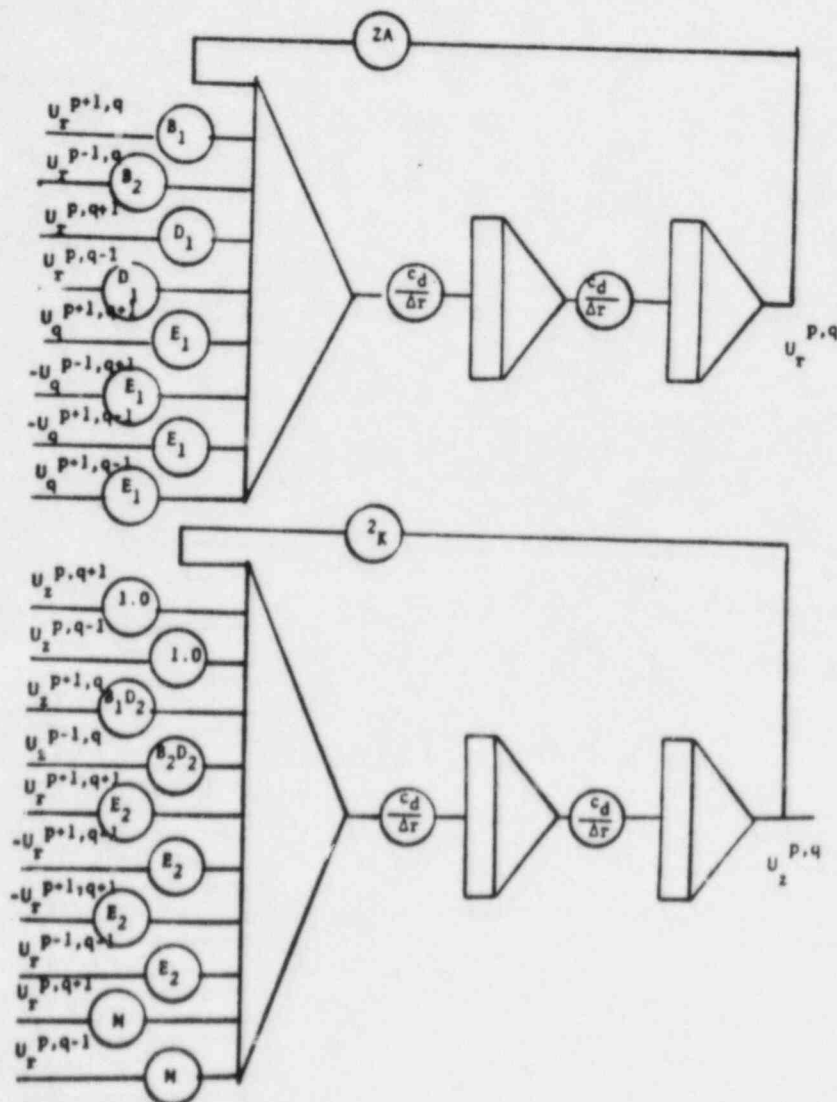


Fig. 3 Analog circuit for interior node point p, q , equation (6). The foregoing equation can also be used on the boundaries if the points outside are artificially created by summing and multiplying interior points as indicated by the boundary equations (7)-(10).

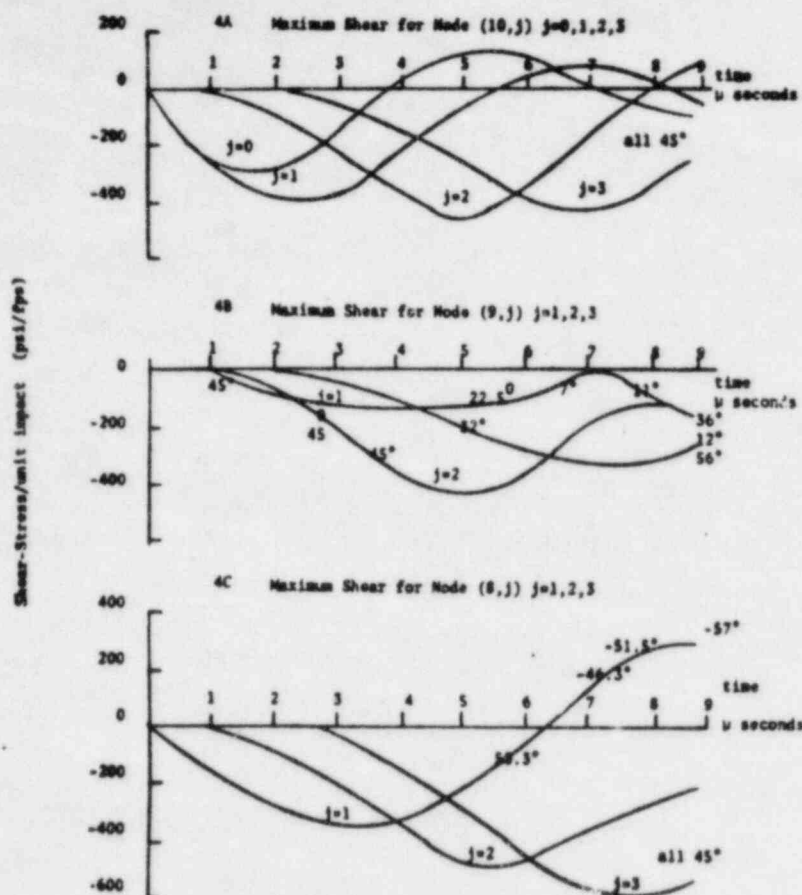


Fig. 4 Stress-time history for maximum shear of problem 3; $\Delta r = 0.1$, $\beta = 1/2$ (refer to Fig. 2 for node numbers)

General Program. The boundary conditions (7)-(10) are combined with the governing difference-differential equations (6) to give a pair of difference-differential equations at each junction point, which relates the radial and axial acceleration there with the surrounding displacements. Thus, by summing the proper surrounding displacement and by integrating twice, the radial and axial accelerations, velocities, and displacements are obtained,² Fig. 3.

After one decides on the sizes (end cap thickness, inside and outside diameters), mesh size, and material in terms of modulus of elasticity (E), Poisson's ratio (ν), and density (ρ), the data are put into a digital program and the pot setting for the problem will be given.

Using these pot settings (if digital set pots are available, they can be set directly as was done by the authors) in the analog circuits, one obtains the solution in terms of outputs of 1 volt = $1.2\Delta r \sqrt{\frac{(1-\nu-2\nu^2)}{E}}$ in (fps of impact) with 10 volts of initial velocity applied to the front surface. Real time is equal to $\Delta r \sqrt{\frac{(1-\nu-2\nu^2)}{E}} N$ (computer time).

Discussion and Results

To check for convergence and stability, the problem was run three times, each run with a finer mesh size. All runs were made for model size of $b = 1.0$ in., $a = 0.8$ in., $c = 0.4$ in., a time scaling of $(\Delta r/c_s)$ and stainless steel material. All of the results³ are

² Complete simulation program is referred to Ju and Wambold (1971).

³ The analog simulation was performed on the computer at Holloman AFB.

Table 1 Maximum shear stresses

Position		Problem I		Problem II		Problem III	
Radial	Axial	Stress	Time	Stress	Time	Stress	Time
1.0	0.2	400	2.54	400	3.50	400	3.40
1.0	0.4	-	-	500	4.90	460	4.90
1.0	0.6	380	5.53	380	7.36	410	7.00
1.0	0.8	-	-	320	9.80	-	-
0.9	0.2	-	-	-	-	160	4.20
0.9	0.4	-	-	-	-	440	4.90
0.9	0.6	-	-	-	-	360	7.36
0.8	0.2	-	-	280	3.86	360	5.50
0.8	0.4	-	-	-	-	480	5.06
0.8	0.6	480	5.01	480	7.01	500	7.50
0.8	0.8	540	5.50	-	-	-	-

Shear Stress in psi/fps of impact speed
Time in Microseconds

summarized in Tables 1 and 2, where problem 3 is the finest mesh, problem 2 is the next, and problem 1 the largest mesh. For problem 3, maximum shear stress-time curves at several critical points are shown in Fig. 4. The results indicate that for low velocity levels the critical shear stress is reached on the inside surface and thus the initiation of a crack at this point, Fig. 4(c). Then at about a 50 percent increase in velocity, the point of initiation of a crack is transferred to the outside surface, Fig. 4(a).

It should be noted that these results apply to a cylinder with



Fig. 5 Fracture initiation in a cylinder as a result of end-on impact. Tests were performed at Sandia Corporation, Albuquerque, N. Mex.

the end cap used and that a different end cap could alter the results. The results of velocity magnitude (given in Table 2) and the locations of fracture were in agreement with experiment.⁴ Fig. 5 shows a fractured cylinder as a result of end-on impact. For 1018 and 4130 steel, the experimental results give a threshold velocity of 244 and 242, respectively, for visible fracture initiation. The fracture velocity as predicted by the analog simulation are 150 and 220 fps, respectively. The velocities predicted by this work are lower limits since the Tresca's failure theory (maximum shear stress) is generally considered conservative for fracture initiation. In fact, the higher velocity required to initiate fracture on the outside surface is generally needed. Thus, if a high degree of safety is not required, the safe velocity could be taken as the one related to the outside surface fracture velocity rather than that causing fracture on the inside surface.

Having established the feasibility of this technique, one can

⁴ Re: Sandia Corporation.

⁵ Impact fracture of the capped cylinders was performed at Sandia Corporation, Albuquerque, N. Mex.

Table 2 Predicted maximum safe impact velocities for cylinders made of several stainless steels

Material	Shear Strength Range*	Maximum Impact Velocity Range for	
		Outside Surface	Inside Surface
AISI-304	107 to 120 kpsi	260 to 300 fps	220 to 240 fps
17-1PH	110 to 136 kpsi	275 to 340 fps	220 to 270 fps
17-4PH	123 kpsi	308 fps	250 fps
AISI	80 to 100 kpsi	200 to 250 fps	160 to 200 fps

*Values from Military Handbook Five

now take the general program developed and test a particular model to establish a safe impact speed or for a given impact speed find the geometrical and the mechanical characteristics of the model for safe impact without fracture.

In design, a voltage representing the safe impact velocity may be referenced on a comparator and compared to points of maximum stress giving a "go-no-go" test. Thus the design engineer would have an immediate answer as to the safety of his design.

References

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11. Love, A. E. H., *Mathematical Theory of Elasticity*, Dover, 4th ed., 1944, p. 143.
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Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE C-8 0720
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

REFERENCE 13

STONEBUST CHI

VELAN P2 MTL
EB. 14/84
SG 5072

12210-ND(3)-224-100-PX-1007-PA

REF 5.2

P5
116

TN.: MR. R. J. MCMORLAND

SUBJECT: PURCHASE ORDER NOS.: RBS-228.211-049 + RBS-228.221-068
VALVES GENERAL - CAT. I AND II
RIVER BEND STATION - UNIT 1
GULF STATES UTILITIES CO.

THESE ARE THE ANSWERS TO YOUR QUESTIONS IN YOUR TWX RBTX-84-049:

- 1) CLOSED ANGLE OF DISC = 10 DEGREES.
FULL OPEN ANGLE OF DISC = 61 DEGREES.
- 2) MINIMUM PIPE VELOCITY REQUIRED FOR FULL DISC LIFT -
ITEM 79 $V = .49 \times \text{SQUARE ROOT OF FLUID SPECIFIC VOLUME.}$ $FO101A \& B$
ITEM 57 $V = .52 \times \text{SQUARE ROOT OF FLUID SPECIFIC VOLUME.}$ $V \neq 5.6$
FLOW TO KEEP DISC AT CRACK OPEN POSITION IS INDETERMINATE
(UNSTABLE).

- 3) PLOT SMOOTH CURVE FROM FOLLOWING -

DISC ANGULAR POSITION
PERCENT OF FULL OPEN

VALVE CV
PERCENT OF FULL OPEN ($C_v = 7000$)

0
10
30
50
75
100

0
14
40
64
86
100

- 4) TO BE DETERMINED BY TEST LATER.

- 5) MASS OF DISC = 205 LBS
MASS OF HANGER = 24 LBS.

REFER TO DRAWING: C.G. OF DISC PLUS HANGER IS LOCATED 10.6
INCHES BELOW AND .38 INCHES TO THE RIGHT OF PIVOT CENTRELINE
WITH DISC CLOSED.

- 6) M.I. DISC W.R.T. PIVOT = 30161
M.I. HANGER W.R.T. PIVOT = 1019

REGARDS

J. M. FARRELL, ENG.
LX NO.: 05-826399

STONEBUST CHI

VELAN P2 MTL

Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE C-10 07 20
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

REFERENCE 14



ATWOOD & MORRILL CO., INC.
SALEM, MASS. 01970

Report No.
300-15202-00

B 113

ENGINEERING DESIGN REPORT

Date
April 17, 1984

TITLE

12210 - NPCG) - 228-800-PX-1007-FD

RBS-5

20" - 900# FEEDWATER CHECK VALVE

VALVE DATA FOR PERFORMING CLOSURE
TIME ANALYSIS

FOR

Stone & Webster Engineering Corporation
Cherry Hill, N.J. 08034

P.O. No. RBS-228.218-062-30
Atwood & Morrill S.O. No. 15202-01

Revisions

Original Issue (Rev. 0)			(1)	(2)	(3)	(4)
Signature	Title	Date	Initials Date			
Originator: <i>Carl Schmidt</i>	DESIGN ENG	4/17/84	- - - -	- - - -	- - - -	- - - -
Reviewer: (MANAGEMENT) <i>AD</i>	ENG SECT MGA	4/17/84	- - - -	- - - -	- - - -	- - - -
Reviewer:			- - - -	- - - -	- - - -	- - - -
Engineering Management:			- - - -	- - - -	- - - -	- - - -

CI 8404270002

PROC. NO. 300-15202-00

122/0-NP(3)-228-800-PX-1007-PA

Pg 114

REVISION SHEET

REF 55

Rev. No.	Date	Revision
0	4/17/84	Original Issue

TABLE OF INFORMATION

- 1.) The Center of Gravity of the disc about the shaft centerline is: $CG_x = 1.21$ in. and $CG_y = 9.65$ in. (See Fig. 1).
- 2.) The Mass of the disc is 12.95 slugs.
- 3.) The Mass moment of inertia of the disc about the shaft centerline is 12.16 slug - ft².
- 4.) Disc angle versus fluid velocity - See Fig. 3.

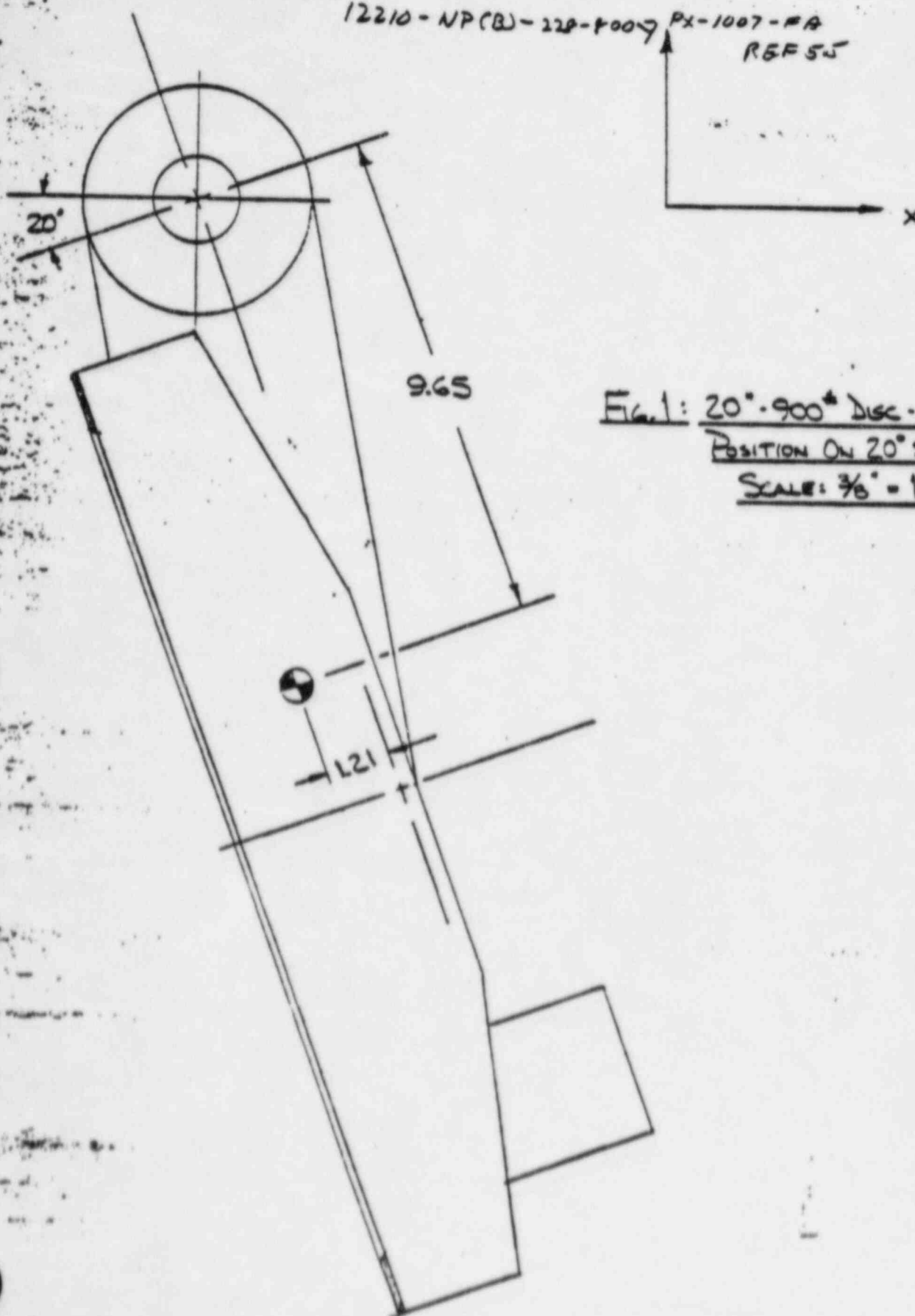
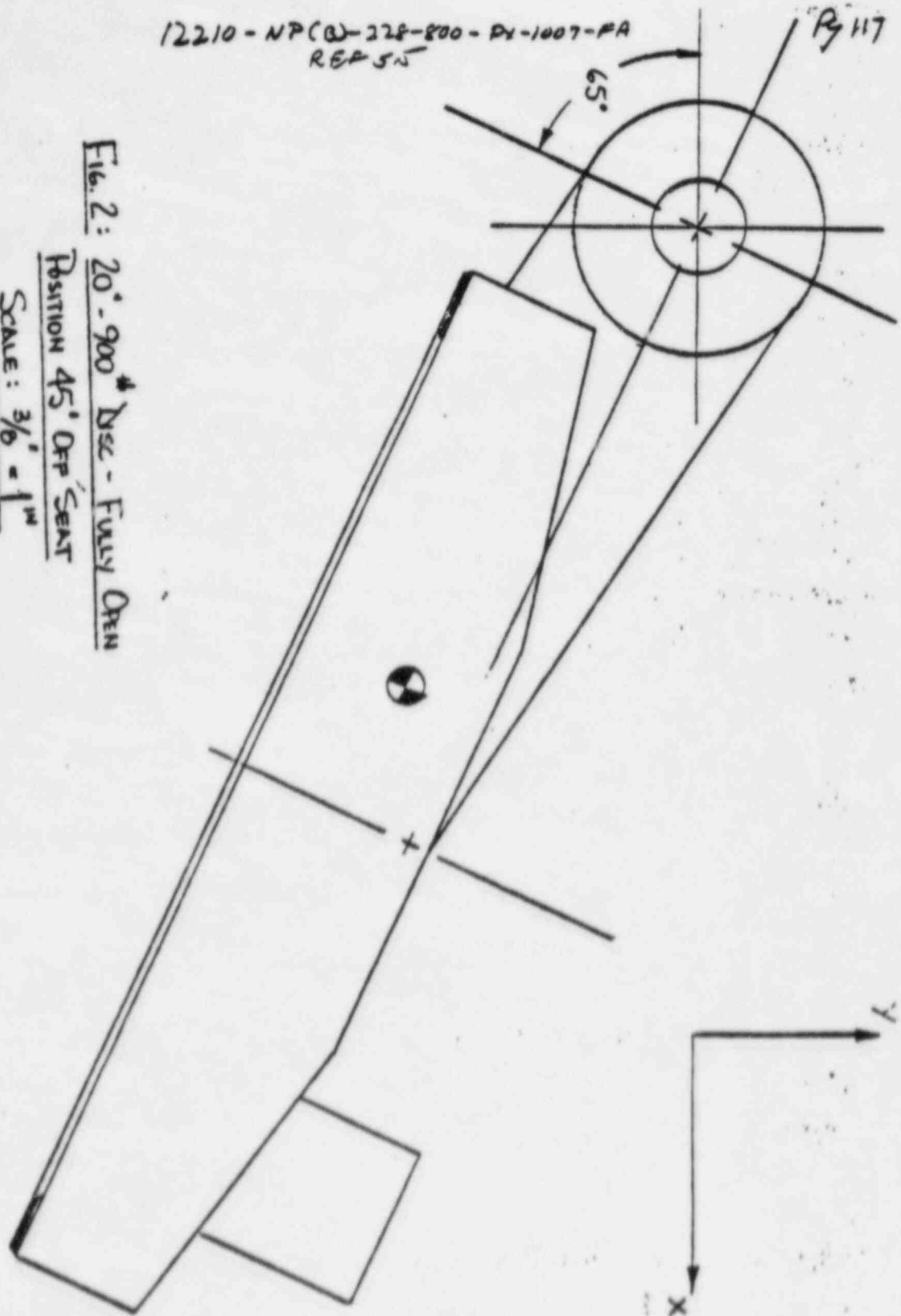


Fig. 1: 20°-900° Disc - Closed
Position on 20° Seat
Scale: 3/8" = 1"

PROC. NO 300-15202-00

12210 - NP(C) - 228-800 - PY-1007-PA
REP 5

Fig. 2: 20°-900° Desc - Fully Open
Position 45° Off Seat
Scale: $\frac{3}{8}$ " = 1"



PROC. NO. 300-1520

FIG. 3: DISC ANGLE VERSUS FLUID VELOCITY

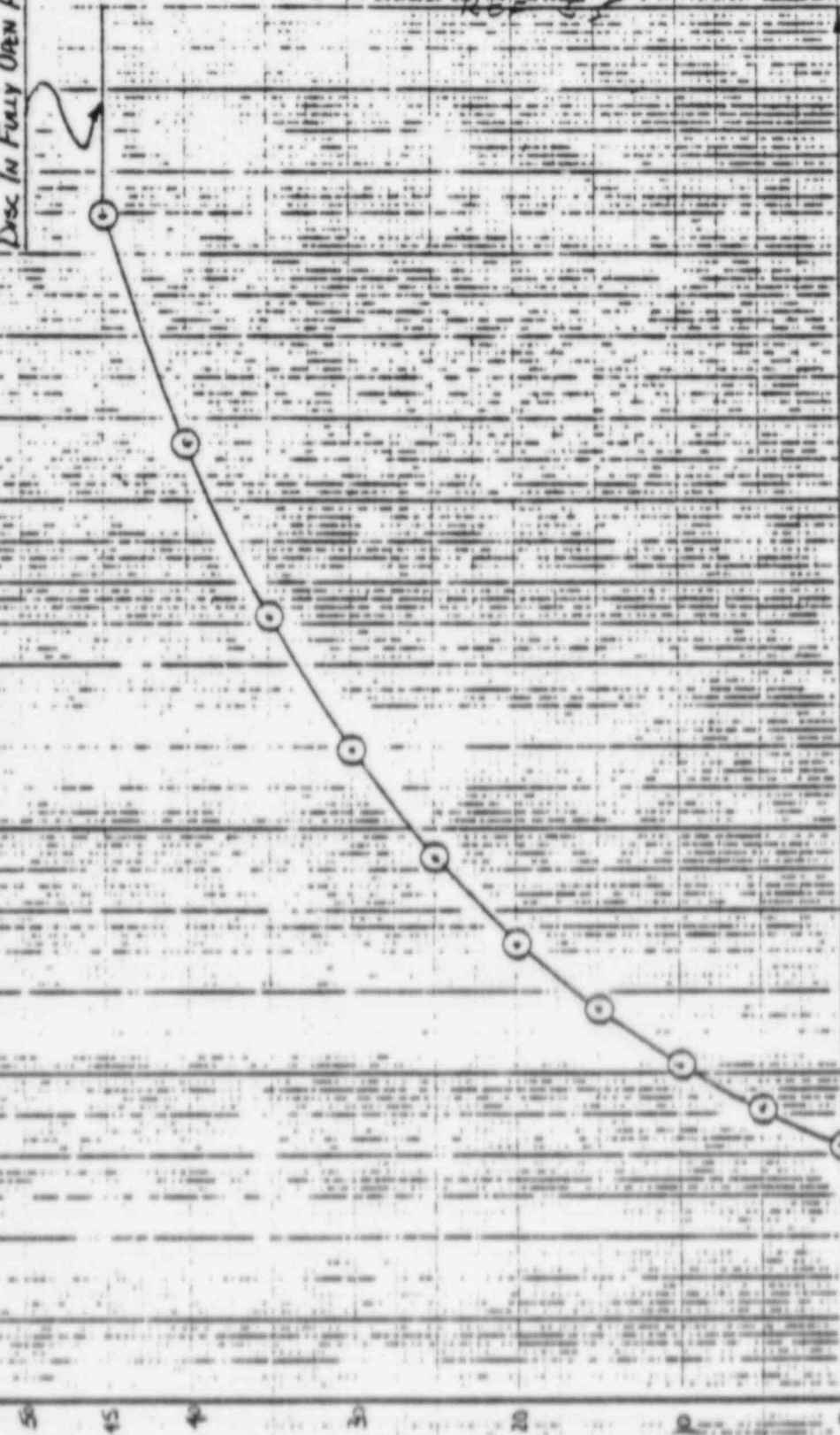
TEMPERATURE = 450°F (REF. SMEC SPEC # 220 210, APP. 7, VALUE DATA 1-11)
DENSITY OF WATER @ 450°F = 5.467 #/FT³

Disc in Fully Open Position

Disc Opening Off of 20° Seat (Degrees)

FLUID VELOCITY IN 20" SCHEDULE 100 PIPE (FT/SEC)

12210-NP(C)-22E-800-PA-100T-FM
REF. 3



Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE C-17 of 20
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

REFERENCE 16

EM#-2697

INTEROFFICE CORRESPONDENCE

TO:	C. LY	LOCATION	6Y	SUBJECT / REFERENCE / J.O. NO.
FROM:	J. MACINTOSH	LOCATION	SEG/4	FW CHECK VALVE MEASUREMENTS / 12210
MESSAGE: —				
<p>ATTACHED PLEASE FIND TABLE AND DRAWING OF FEEDWATER CHECK VALVE 1B21*A0V032B WHICH WAS MEASURED IN PLACE. DIMENSIONS GIVEN ARE APPROXIMATE DUE TO ACCESSIBILITY OF VALVE. NO SEAT BEARING AREA COULD BE OBTAINED.</p>				
CC. D. BRAY				
10/17/84		J. MacIntosh		4555
DATE		SIGNATURE		TELEPHONE
REPLY:				
Thank you very much				
10/20/84		C. Ly		3617
DATE		SIGNATURE		TELEPHONE

Stone and Webster Engineering Corporation
CALCULATION SHEET

Page 2 of 3

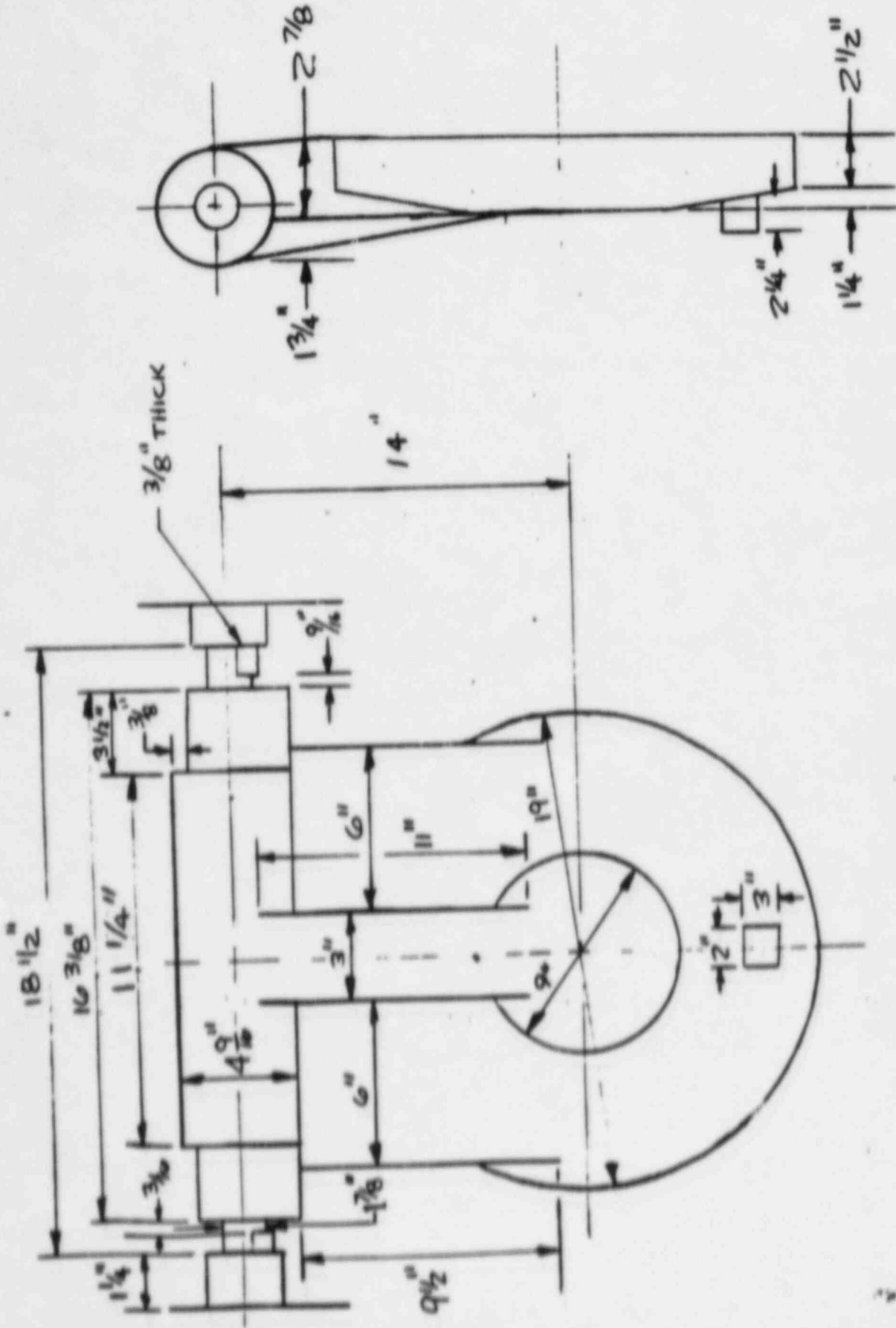
Calculation Identification Number				PAGE 15
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL CODE	
14764.07	MN(C)	2043	SQE	

Dimensions

TABLE 4.3	Atwood/Morrill Check Valve 1B21°ACVQ32A,B			Velan Check Valve 1B21°P010A,B		
	MEASURED					
			REP.			REP.
Diam of Disk(in)	18.75	19"	12	17.6		11
Dist from Rockshaft to Disk Center	12.3	14"	2	11.7		3
Diam of Pin	---	1 7/8"	--	2.6		3
Diam of Rockshaft	2.7	4 9/16"	2	2.0		3
Gap between Tail Disk & Bearing	.1 or less	SEE DWG	2	.1 or less		3
Dist between CG of Disk & Pin Seat(boss)	2.2	CANNOT MEAS.	12	1.5		11
Clearance - Disk/Tlk	--	NA	--	.5 (7)		
Thickness of Disk	3.75 ctr, 1.9 rim	2 1/2"	12	3.6 ctr, 2.75 rim		11
Seat Annulus	.5+	CANNOT MEAS.	2	.56		3
Tail Link Section	4.5 x 2.0	SEE DWG	2	1.8 x 3.0		3
Tail Link Grip	14.4	16 3/8"	2	6.1		3
Seat Inside Diam	15.75	CANNOT MEAS.	2	15.75		3
Diam of Tlk Boss	---	NA	--	4.0		3

Material Properties at 500F (Ref. 9, Appendix I)

TABLE 4.4	Sy (ksi)	Su (ksi)	Elong.* (%)	E (psi*10 ⁻⁶)	Sm (ksi)	
CS SA 216 WCC	40	70	22	27	21.6	
CS SA 105	36	70	22	27	19.5	
SS SA 564 630	105	135	18	27	42.8	
SS A 182 F6a	40	70	25	27	22.0	
			*Ref. 10			



FEEDWATER VALVE

NTS

Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE D-1 of 9
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

APPENDIX D

Computer Log and Microfiche

1722

CALCULATION NO. 2043
ORDER NO. 12210

D-24

* COMPUTER GENERATED JOB NUMBER

** COMPUTER USED (S & W OR CDC)

COMPUTER LOG

D-3/9

7729

S & W AUTH. NO. 4737
CDC CHARGE NO. B3091 NCCALCULATION NO. 5QE-2043
JOB ORDER NO. 12210

RUN NO.	JOB NO. #	FICHE LOC.		PREPARED BY		REVIEWED BY		COMP. **	COMMENTS
		PAGE	SECT	SIGNATURE	DATE	SIGNATURE	DATE		
300	3320			<i>Carol Black & Ly</i>	10/26/84	<i>Joe Quinn</i>	10/30/84		NSR 7A PT A (10/26/84)
302	422			"	"	"	"		NSR 7A PT A (10/27/84)
303	3326			"	"	"	"		NSR 7A PT B (10/28/84)
303	424			"	"	"	"		NSR 7A PT B (10/27/84)
30C	114			"	"	"	"		NSR 7A PT C (10/28/84)
30E	3137			"	"	"	"		NSR 7A PT C (10/29/84)
30D	41			"	"	"	"		NSR 7A PT D (10/28/84)
30D	429			"	"	"	"		NSR 7A PT D (10/27/84)
302	467			"	"	"	"		NSR 7A PT E (10/28/84)
30E	3133			"	"	"	"		NSR 7A PT E (10/29/84)
30F	116			"	"	"	"		NSR 7A PT F (10/28/84)
30Z	432			"	"	"	"		NSR 7A PT F (10/27/84)

* COMPUTER GENERATED JOB NUMBER

** COMPUTER USED (S & W OR CDC)

D-4/9

3724

** COMPUTER USED (S & W OR CDC)

1129

D-5

CALCULATION NO. 12210-SOE-2043

JOB ORDER NO. 12210

[illegible]

** COMPUTER USED (S & W OR CDC)

COMPUTER LOG

YEAR 1984 JOA CHECK VALVES (FNOA)
POINT IMPACT ANALYSIS

D-6/9

7729

S&W AUTH. NO. 4737
CDC CHARGE NO. B382 JLC

CALCULATION NO. 12210-58Z 21-43
JOB ORDER NO. 12210

RUN NO.	JOB NO. *	FICHE LOC.		PREPARED BY		REVIEWED BY		COMP. **	COMMENTS
		PAGE	SECT	SIGNATURE	DATE	SIGNATURE	DATE		
100				Cam E. Hanky	11/1/84	J. M. Durin	11/8/84	CDC	HOK FOR PT A TAPE # 12556
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10b				Cam E. Hanky	11/7/84	"	"	:	HOK FOR PT B TAPE # 12557
10b				Cam E. Hanky	11/7/84	"	"	:	DYRE 1 FOR PT B
10c				Cam E. Hanky	11/7/84	"	"	:	HOK FOR PT C TAPE # 12562
10c				Cam E. Hanky	11/7/84	"	"	:	DYRE 1 FOR PT C
10d				:	:			:	HOK FOR PT D TAPE # 12569
10d				:	:			:	DYRE 1 FOR PT D
10e				:	:			:	HOK FOR PT E TAPE # 12650
10e				:	:			:	DYRE 1 FOR PT E
10f				:	:			:	HOK FOR PT F TAPE # 12672
10f				:	:			:	DYRE 1 FOR PT F

* COMPUTER GENERATED JOB NUMBER

** COMPUTER USED (S&W OR CDC)

1729

D-7

CALCULATION NO. 12210-SQE-2043
JOB ORDER NO. 12210

** COMPUTER USED (S & W OR CDC)

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION TITLE PAGE

CL8412130001

A 5010 54 (FRONT)

CLIENT & PROJECT GULF STATES UTILITIES - RIVER BEND UNIT 1				PAGE 1 OF 104/145 WITH 38 PAGES OF APPENDIX	
CALCULATION TITLE (indicative of the Objective): FEEDWATER CHECK VALVE INTEGRITY FOLLOWING PIPE RUPTURE (O.C.)				QA CATEGORY (✓) 4/P <input checked="" type="checkbox"/> I - NUCLEAR SAFETY RELATED <input type="checkbox"/> II <input type="checkbox"/> III <input type="checkbox"/> OTHER	
CALCULATION IDENTIFICATION NUMBER					
J.O. OR W.O. NO.	DIVISION & GROUP	CURRENT CALC. NO.	OPTIONAL TASK CODE	Calculation Status	
12210	NM(C)	2043	SQE	FINAL	
APPROVALS - SIGNATURE & DATE					
PREPARER(S)/DATE(S)	REVIEWER(S)/DATE(S)	INDEPENDENT REVIEWER(S)/DATE(S)	REV. NO. OR NEW CALC. NO.	SUPERSEDES CALC. NO. OR REV. NO.	CONFIRMATION *REQUIRED (✓) YES NO
J. M. Gwinn J. M. Gwinn 10/30/84	C. M. T. LY 10/30/84	S. M. FELDMAN 10/30/84 S. M. FELDMAN	New	N/A	✓
J. M. Gwinn J. M. Gwinn 12/7/84	C. M. T. LY 12/7/84	S. M. FELDMAN 12-10-84 S. M. FELDMAN	1	Rev. 0	✓
J. M. Gwinn J. M. Gwinn 6/24/85	C. M. T. LY 7/3/85	S. M. FELDMAN 7/3/85 S. M. FELDMAN	2	1	✓
<p>REV. 1 APPROVAL <i>[Signature]</i> 12/10/84 *As listed in this block.</p> <p>REV. 2 APPROVAL <i>[Signature]</i> 7/3/85</p>					
CALCULATION DISTRIBUTION			LOADS TRANSMITTALS		
GROUP	NAME & LOCATION	COPY SENT (✓)	GROUP	NAME & LOCATION	COPY SENT (✓)
Project Files	DDMartin (4R)				
Fire Files (microfilm)	Boston 401 Summer Street				
PPF	Site				
EQ (SEISMIC)	N. MUNI (6Y)				
EMP/BOB	J. Gwinn 2/6/89				
EMD/CHOL	S. FELDMAN (2PR)				

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

▲ 5010 65

CALCULATION IDENTIFICATION NUMBER				PAGE ____
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

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page D-8

Mr. A. P. Chae

CALCULATION SHEET

▲ 5010.65

CALCULATION IDENTIFICATION NUMBER				PAGE <u>E107</u>
J.O. OR W.O. NO. <u>122/0</u>	DIVISION & GROUP <u>NMCC)</u>	CALCULATION NO. <u>2643</u>	OPTIONAL TASK CODE <u>S&E</u>	
1	<p style="text-align: center;"><u>APPENDIX E</u></p> <p style="text-align: center;">VOID PAGES</p>			
2				
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Stone and Webster Engineering Corporation
CALCULATION SHEET

APPENDIX E PAGE 24 OF 38

Calculation Identification Number				PAGE 9-1 OF 16
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

9. INTEGRITY OF VELAN CHECK VALVE 1B21*F010

VOID

Stone and Webster Engineering Corporation
CALCULATION SHEET

APPENDIX E PAGE E297
38

- Calculation Identification Number				PAGE 19
J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

5. SUMMARY OF RESULTS

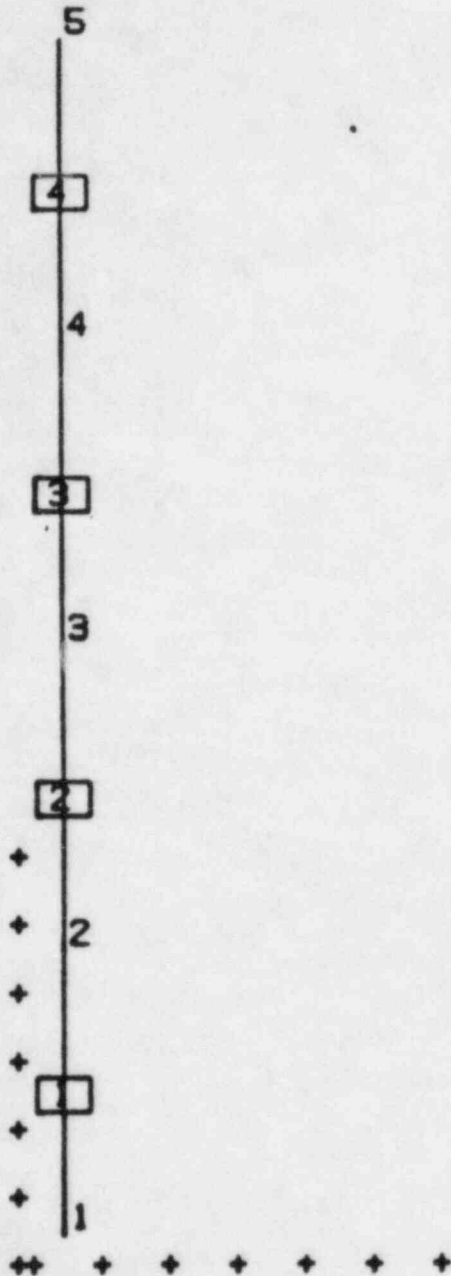
VOID

TABLE 5.1	AOVF32			F010		
	Calculated	Allowable	FS	Calculated	Allowable	FS
Disk -shear #	7.88E6	8.28E6	1.05	4.2E6	7.8E6	1.9
-strain %	2.3	15.4	6.7	2.9	15.4	5.3
Rockshaft -stress ksi	N/A	N/A		28.5	54.6	1.9
-ratio	.35	1.0	2.9	N/A	N/A	
Tail Link -stress ksi	N/A	N/A		68.1	70.0	1.03
-ratio	.89	1.0	1.1	N/A	N/A	
Pin -stress ksi	N/A	N/A	N/A	4.8	28.8	6.0
Stud -stress ksi	N/A	N/A	N/A	41.0	49.	1.2
Seat/Disk Interface -stress ksi	74.	98.	1.3	98.8	70.	1.4

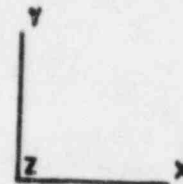
Stone and Webster Engineering Corporation
CALCULATION SHEET

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J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	



VOID



UNIFORM IMPACT MODEL OF CHECK VALVE 1B21-FO10A TAIL-LINK

▲ 5010 85

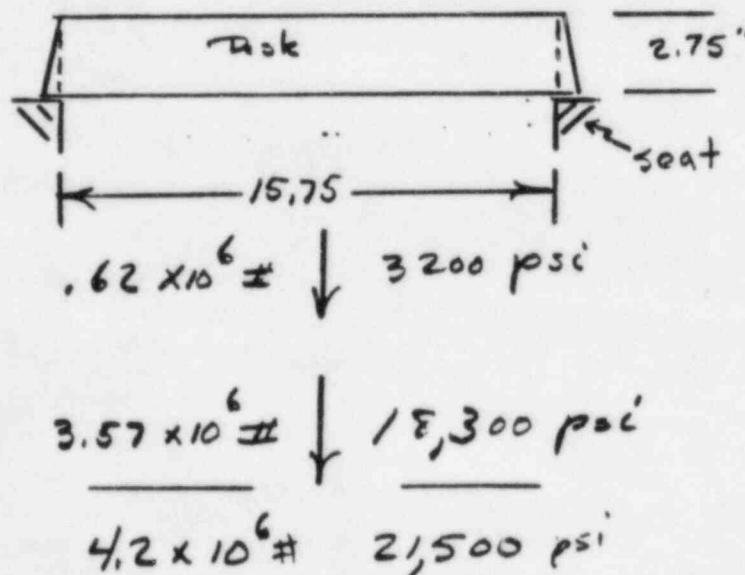
CALCULATION IDENTIFICATION NUMBER				PAGE <u>9-2</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

9.1 Disk (SA 105)

VOID

Loading

Uniform impact of the disk on the seat generates a reaction of $3.57 \times 10^6 \#$, as given by FKK in Table 8.5. In reality, the impact speed varies linearly with the distance from the rock-shaft but the total is the same for a constant impact speed equal to the average speed at the disk center at mass. Acting simultaneously with impact is the pressure surge from rapidly closing the check valve. The maximum pressure is used although it occurs several milliseconds after impact.



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12210	NM(C)	2043	SQE	

Shear Out

VOID

The allowable shear load in the disk at the inside diameter of the seat where the bending stress in the disk is zero, is given by

$$P_u = .7 \frac{\sigma_u}{\sqrt{3}} \cdot \text{shear area} \cdot \text{strain rate factor}$$

$$= .7 \left(\frac{70,000}{1.7} \right) (15.75 \pi 2.75) (2)$$

$$= 7.7 \times 10^6 \#$$

Since $7.7 \times 10^6 \# > 4.2 \times 10^6 \#$,
the integrity of the disk in shear out is demonstrated.

Rupture

The disk must absorb the impact energy at strains below the rupture level. The impact energy

$$= \frac{190}{32} (60)^2 \frac{12}{2} = 129,000 \text{ in}\#$$

where effective disk weight = $185 + \frac{20}{4} = 190 \#$

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▲ 5010 85

CALCULATION IDENTIFICATION NUMBER				PAGE 9.4
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	M17(C)	2043	5QE	

VOID The plastic strain energy of a circular disk from Appendix E

$$= \frac{\pi}{2} \left(\frac{\pi}{2} - 1 \right) \sigma_y t^2 y_0$$

$$= .89 (34000) (2.75)^2 y_0 = 244,000 y_0$$

Equating to the impact energy and solving for the deflection at the center of the disk

$$y_0 = \frac{128,000}{244,000} = .53''$$

From Appendix B, the maximum strain associated with this deflection is

$$\epsilon = \frac{t}{2} \left(\frac{\pi}{D} \right)^2 y_0 = \frac{2.75}{2} \left(\frac{\pi}{15.75} \right)^2 (.53) = .029 \text{ in/in}$$

which neglects the deflection of the disk from the pressure, it being in the elastic range and therefore negligible.

CALCULATION SHEET

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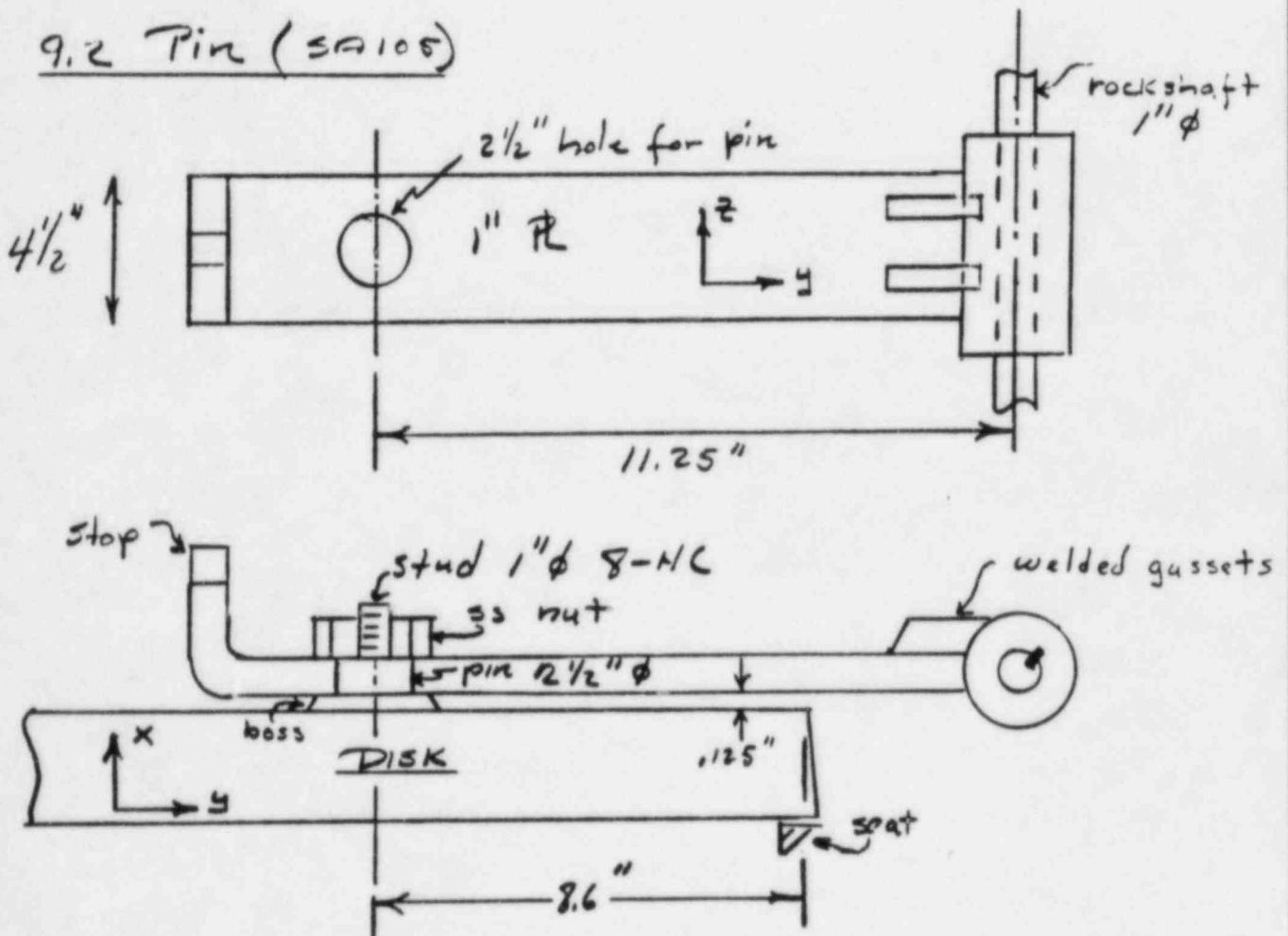
CALCULATION IDENTIFICATION NUMBER				PAGE 4-5
J.O. OR W.O. NO. 12210	DIVISION & GROUP A/A9(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE	

The allowable strain VOID

$$= .7 \cdot \text{Elongation} = .7 (.22) = .154 > .029 \quad \text{OK}$$

which shows that the disk will not rupture under impact loading.

9.2 Pin (SA105)



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12610	NAP(C)	2043	SQE	

The pin and stud are VOID integral and welded to the disk. A ss nut locked to the stud keeps the tail link tight to the boss.

Pre-impact

The centrifugal force, F_c , is reacted by shear on the pin. The shear stress on the pin

$$= F_c / \text{Area of pin}$$

$$= 21500 / 4.5 = 4800 \text{ psi}$$

the allowable shear stress

$$= .75 S_u / \sqrt{3} = .7 (70000) / 1.7$$

$$= 28300 \text{ psi} > 4800$$

(OK)

The centrifugal force is eccentric to the tail link, generating a moment which is reacted by the stud in tension and the boss in compression.

$$= F_c \left(\frac{2.75}{2} + .125 + \frac{1}{2} \right)$$

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>N/M(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQI</u>	
$= 21,500 (2.0) \quad \text{VOID} \quad = 43,000 \text{ in}^{\#}$				
<p>The distance between the edge of the boss and the stud = $3.5/2 = 1.75 \text{ in}$. Dividing into the moment yields the force on the stud</p>				
$= 43,000 / 1.75 = 24,600^{\#}$				
<p>which corresponds to a tensile stress</p>				
$= 24,600 / .60$				
$= 41,000 \text{ psi}$				
<p>the allowable stress</p>				
$= .7 S_u = .7 (70000)$				
$= 49,000 \text{ psi} > 41,000 \quad \text{OK}$				
<p>Hence, the integrity of the pin is demonstrated for pre-impact loads.</p>				

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12210	N/M(C)	2043	SQIE	

Uniform ImpactVOID

From Table 8.5, a force $F_{XK} = 55 \text{ Kips}$ acts in compression at the disk center which is in no way carried by the pin or stud. Also acting is a moment $M_{ZK} = 100 \text{ inK}$ which is not real since the plastic moment, from Appendix B

$$= \frac{1}{4} \sigma_y A t = \frac{1}{4} (36,000)(7.5)(1) = 40,500$$

$$= 40,500 \text{ inK}$$

is exceeded. What really happens is the tail link deflects in bending until it strikes the disk, $1/8"$ away, thus limiting the strain.

Point Impact

These forces and moments from Table 8.5 are much lower than those resulting from pin impact and uniform impact, because of the flexibility of the tail link in bending and

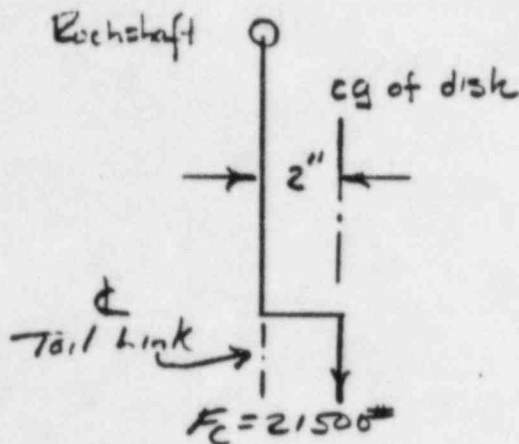
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CALCULATION IDENTIFICATION NUMBER				PAGE 9-9
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NA 9(C)	2043	SQE	

torsion. Hence, integrity of the pin is demonstrated for point impact loading.

VOID

9.3 Tail LinkPre-Impact

The centrifugal force, F_c , loads the tail link in direct tension and bending. Assuming that the nut makes the full tail link section effective in bending, the total

tensile stress

$$= \frac{F_c}{2} + F_c(2'' \text{ arm}) C/I$$

$$= \frac{21,500}{2} + 21,500(2)(.5)/.375$$

$$= 10,800 + 57,300 = 68,100 \text{ psi}$$

which is greater than the tensile allowable, but less than the ultimate stress of 70,000 psi.

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12210	NAT(C)	2043	SQE	

VOID

Referring to Ref. 1, it is observed that this stress lasts for one or two milliseconds which is not of sufficient duration to develop the strain necessary for failure.

The deflection of the disk off the seat just prior to impact

$$= PL/AE = 21500(11.25)/4.5 \times 27E6$$

$$= .002 \text{ inches}$$

which is insignificant compared to the allowable deflection of .25". Hence, the integrity and operability of the tail link under pre-impact loading is demonstrated.

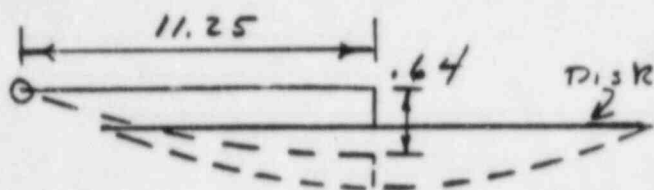
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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NAT(C)	2043	SQE	

VOID

Uniform Impact

The worst case bending for the tail link is caused by deflection of the disk center following impact with the seat, calculated in section 9.1 to be .64 inches including the contribution from the pressure surge. From Appendix A, the



plastic strain corresponding to a deflection, y_0 , of a simply supported beam of length = 2×11.25

$$\epsilon = \frac{\pi^2}{2} \frac{D y_0}{L^2} = \frac{\pi^2}{2} \frac{(1)(.64)}{(2 \times 11.25)^2}$$

$$= .0062 < .7(22) = .154$$

OK

Since this occurs after the initial kickback loads, the contribution from shear is negligible.

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	SQE	

The worst case shear loads from Table 7.5 are 50 Kips at the each shaft end and 60 Kips at the pier ends. Also max shear stress:

VOID

$$= 50,000 / \text{area} = 50,000 / 4.1$$

$$= 12,200 \text{ psi}$$

the allowable shear stress

$$= .75u/\sqrt{2} = .7(70,000)/1.7$$

$$= 28,300 > 12,200 \text{ psi}$$

OK

Hence, integrity of the tail link is demonstrated for uniform impact loading.

Point Impact

Point impact loads, as seen from Table 7.5, are much smaller than those for uniform impact. Hence, integrity for point impact loads is demonstrated.

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NAT(C)	20V3	SQE	

9.4 Rock shaft

~~VOID~~

Pre-impact

The rock shaft reacts the centrifugal force in double shear. The stress

$$= 21,500 / 2 (\text{Area})$$

$$= 21,500 / 2 (.79) = 13,600 \text{ psi}$$

which is below the allowable

$$= .7(105,000) / \sqrt{3}$$

$$= 42,500 > 13,600 \text{ psi}$$

OK

Uniform Impact

From Table 8.5, the kick back load is 45 kips in double shear, resulting in a stress

$$= \frac{45,000}{2} / \text{area} = \frac{45,000}{2} / .79$$

$$= 28,500 \text{ psi}$$

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	N11(C)	2043	EQE	

1
2
3 the allowable shear stress VOID
4
5
6 $= .754/\sqrt{3} = .7(135,000)/1.7$
7
8
9 $= 54,600 \text{ psi} > 28,500$ (OK)
10
11
12
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16 Point Impact
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19 Referring to Table 8.3 shows that the
20 point impact loads on the rock shaft
21 are much smaller than the loads from
22 pre-impact and uniform impact.
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30 Hence, the rockshaft integrity is demonstrated
31 for all critical loads.
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9.5 Seat/Disk Interface VOID

The stress caused by impact in the seat and disk material at the interface is calculated from the hammer equation

$$\Delta \sigma = \frac{\Delta u}{c} E$$

where c is the speed of sound in steel.
Thus =

$$\Delta \sigma = \frac{60}{16,400} 27E6 = 98,800 \text{ psi}$$

The allowable stress for

$$= .7 S_u (\text{strain rate factor})$$

$$= .7 (70000)(2) = 98000 \text{ psi}$$

Although slightly above the allowable, it is below ultimate and since this stress acts for a time less than $1/2$ milliseconds (Fig. 8.2), very little damage to the

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<u>12210</u>	<u>NA1(15)</u>	<u>2043</u>	<u>SQIE</u>	

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3 seat/disk interface is indicated. Hence,
4
5 the leakage will remain within acceptable
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7 limits.
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VOID

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10. INTEGRITY OF ATWOOD/MORRILL CHECK VALVE 1B21*AOV032

VOID

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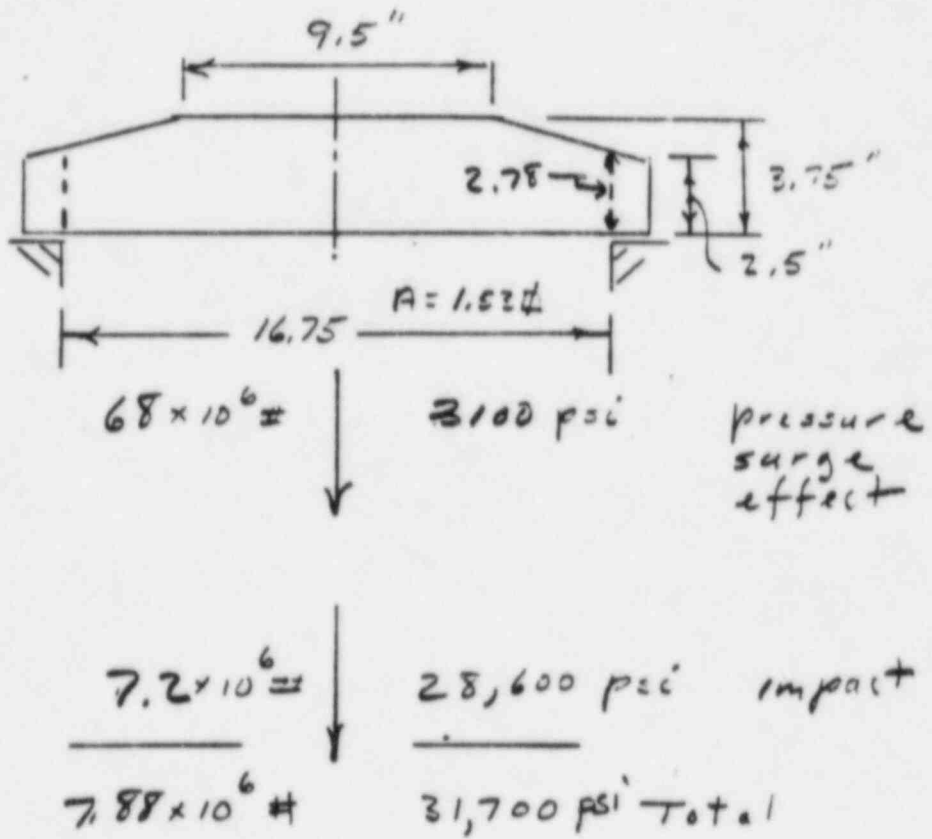
CALCULATION IDENTIFICATION NUMBER				PAGE 10-2
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	SQE	

10.1 Disk (SA 216 WCC)

VOID

Loading

Uniform impact of the disk on the soil generates a reaction of $7.2 \times 10^6 \#$, as given by F&H in Table 5.1. Acting simultaneously with impact is the pressure surge from rapidly closing the closed valve.



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CALCULATION IDENTIFICATION NUMBER				PAGE 10-3
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NIA(C)	2043	SQE	

Shear Out

VOID

The allowable shear load in the disk at the seat inside diameter where the bending stress is zero

$$= .7 (S_u / \sqrt{3}) \cdot \text{shear area} \cdot \text{strain rate factor}$$

$$= .7 \left(\frac{20000}{1.73} \right) \cdot (16.75 \pi 2.75) (2)$$

$$= 8.28 \times 10^6 \text{ lb} > 7.88 \times 10^6$$

(015)

Rupture

The disk must absorb the impact energy at strains below the rupture level. The impact energy

$$= \frac{1}{2} \cdot \frac{1}{2} (45)^2 \frac{1}{2} = 158,000 \text{ in-in}$$

The plastic strain energy of a circular disk from Appendix E

$$= \frac{\pi}{2} \left(\frac{\pi}{2} - 1 \right) \epsilon_0 \epsilon^2 y_0$$

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	IV/C	2043	SQT	

$$= .89 (36000) (3.75)^2 y_0$$

VOID

$$= 450000 y_0$$

Equating to the impact energy and solving for the deflection, y_0 , at the disk center

$$y_0 = 158000 / 450000$$

$$y_0 = .35$$

From Appendix B, the maximum stress associated with this deflection is

$$\epsilon = \frac{t}{2} \left(\frac{H}{L} \right)^2 y_0 = \frac{3.75}{2} \left(\frac{1.1}{6.75} \right)^2 (.35) = .023$$

which includes the small contribution from the pressure surge by overstating the impact energy (mass of disk).

The allowable strain

$$= .7 \times \text{elongation} = .7 (.22) = .154 > .023$$

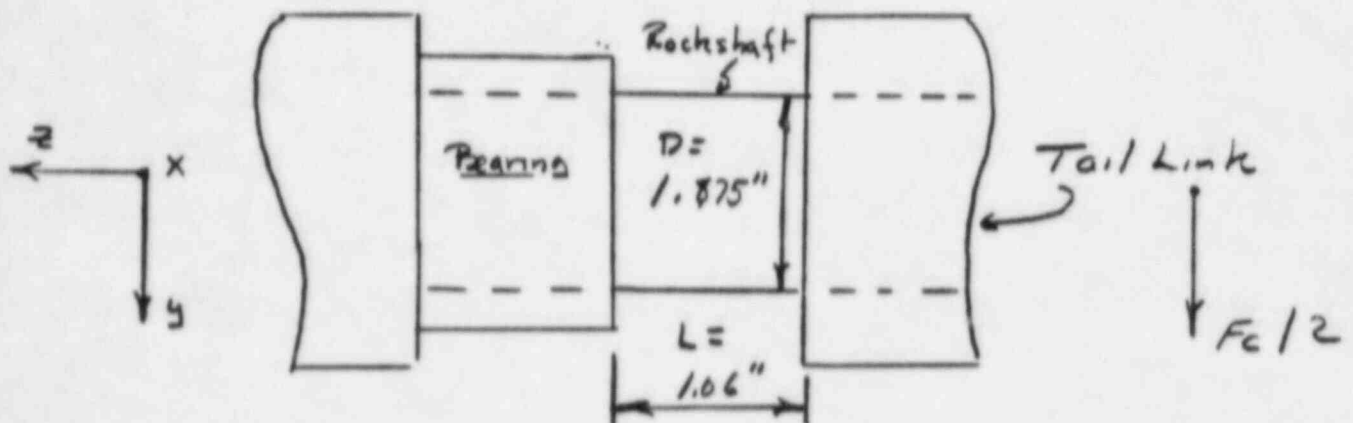
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▲ 5010 65

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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(c)	2043	SQE	

Hence, the integrity of the disk is demonstrated.

9.2 Rock Shaft (A182 F6a)



Pre-Impact

The shear stress from the centrifugal force

$$= F_c / (2 \cdot \text{area of rock shaft})$$

$$= 24,300 / 2 (2.8)$$

$$= 4700 \text{ psi}$$

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J.O. OR W.O. NO. 12210	DIVISION & GROUP NMCC1	CALCULATION NO. 2043	OPTIONAL TASK CODE SQZ	

The allowable shear stress

VOID

$$= \frac{.7 S_u}{\sqrt{3}} = .7 (70000) / 1.7$$

$$= 28300 \text{ psi} > 4700$$

(51)

The bending stress

$$M c / I = \frac{26300 (1.06) (151)}{2} \frac{1}{.61}$$

$$= 21,400 \text{ psi}$$

The allowable bending stress

$$= .7 S_u = .7 (70000)$$

$$= 49000 > 21,400 \text{ psi}$$

(51)

Combining via the stress interaction formula
of Reference 7, Appendix F79231

$$R_L = 21,400 / 49000 = .44$$

$$R_S = 4700 / 28300 = .17$$

Since $R = \sqrt{R_L^2 + R_S^2} = .47 < 1$, integrity
under pre-impact loads is demonstrated

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12210	NACE	2043	S&E	

Uniform ImpactVOID

The rock shaft must carry a rich back load of 240 kips, as given in Table 8.4 for 1-XK. The resulting stress

$$= \frac{F \times K}{2} / \text{Area}$$

$$= \frac{240,000}{2} (2.8)$$

$$= 33,600 \text{ psi}$$

The allowable shear stress

$$= .75 S_u \times \text{strain rate factor} / \sqrt{3}$$

$$= .7 (70,000) (2) / 1.7$$

$$= 57,600 \text{ psi} > 33,600$$

OK

The bending stress

$$= M C / I = F \times K (2L) (C) / 8 I$$

$$= 242,000 (1.06) (1.875) / 4 (.61)$$

$$= 99,000 \text{ psi}$$

* Used actual value from computer run.

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12210	NMCC)	2042	5QZ	

the allowable bending stress:

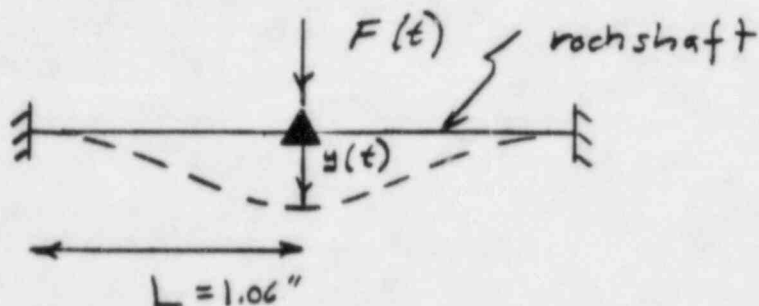
$$= .7 S_u \cdot \text{strain rate factor}$$

$$= .7(70000)(2)$$

$$= 98,000 \text{ psi} < 99,000$$

NG

A non-linear strain analysis is required to demonstrate integrity of the rockshaft.



The rockshaft is modelled as shown in the sketch. All the mass of the rockshaft and the tail link bushing are lumped at the point shown by the black triangle, driven by the time varying kickback force $F(t)$, obtained from the kickback loads analysis. Figure — shows the damped trace. Only the first peak will be used as shown on the Figure. Both the shear and bending strains are

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12-10	MM(C)	2043	50E

Computed as a function of time. Following the procedure of Section 9.2.31 of Appendix A, Reference 9, the interactive ratio is formed

$$R = \sqrt{R_s^2 + R_b^2} < 1$$

where, here

$$R_s = \text{shear stress / allowable}$$

$$R_b = \text{bending stress / allowable}$$

The differential equation governing the rock shaft response to load is given by

$$m\ddot{y} + ky = F(t)$$

as long as primarily elastic conditions prevail. After essentially plastic conditions are achieved, the equation is switched by toggle logic to

$$m\ddot{y} + R_p = F(t)$$

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12210

N/A(C)

2043

SEE

where R_p is the constant resistance to displacement. The maximum bending moment is given by

VOID

$$MOM = \frac{F(2L)}{8} = FL/4$$

The resistance R_p is the value of F when $MOM = M_p$, the plastic moment. Thus

$$R_p = 4M_p/L$$

For a circular section (App. F), this reduces to

$$R_p = \frac{8}{3\pi} \sigma_y A D$$

The elastic stiffness, k , for combined shear and bending, since this is a short beam, was obtained from the expression for deflection under a concentrated load at the center

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12216	N/A (C)	2043	SQT	

$$\delta = \delta_{\text{bending}} + \delta_{\text{shear}}$$

From reference 15

$$\delta_{\text{bending}} = \frac{w(2L)^3}{192EI} = \frac{wL^3}{24EI}$$

For direct shear

$$T = \tau Q = \frac{\delta_s Q}{L} = w/A$$

from which is obtained

$$\delta_{\text{shear}} = wL/AQ$$

Combining bending with shear

$$\delta = \frac{wL^3}{24EI} + \frac{wL}{AQ}$$

which provides the stiffness

$$k = w/\delta = \frac{1}{\frac{L^3}{24EI} + \frac{L}{AQ}}$$

for elastic conditions.

From Appendix A, the relation between deflection y and the maximum strain is

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12210	NY(C)	2043	SCZ	

$$\epsilon = \frac{D}{2} \frac{1}{p} = \frac{\pi^2}{4} \frac{P_H}{L^2}$$

VOID

These equations were coded for computation using the IBM CSMP III simulation program. Details of the coding and results are provided in the following pages. The results show that the interactive strain ratio is less than one, i.e.

$$R = 1.35 < 1$$

(OK)

Hence, integrity under uniform impact loading is demonstrated.

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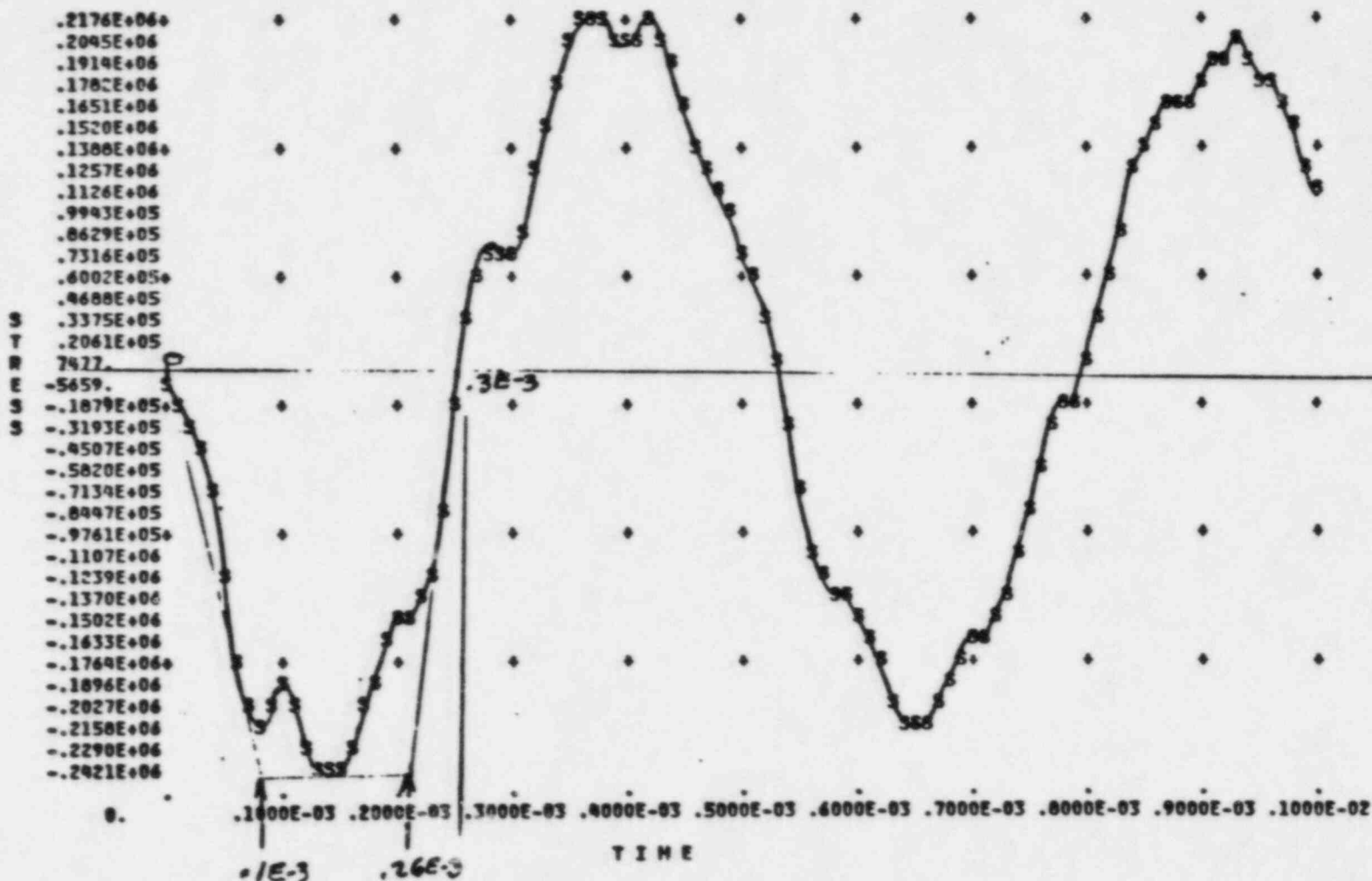
CALCULATION NO.

OPTIONAL TASK CODE

CALCULATION IDENTIFICATION NUMBER

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(STRESS VECTOR NO. 34) EQUILB CHN RESPONSE FOR ELEMENT NUMBER 30, STRESS TYPE 1 (MOD/DOF X1)



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J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

000CONTINUOUS SYSTEM MODELING PROGRAM III V1H1 TRANSLATOR OUTPUT000

TITLE TIME HISTORY STRAIN IN THE ROCKSHAFT FROM KICKBACK LOAD
TITLE 1821-A0V032A CHECK VALVE
TITLE 6SU/RIVER BEND POWER STATION - UNIT 1

* STORED JMG.RS032A:210

FUNCTION TH=(0.,0.),(.1E-3,.242E6),(.24E-3,.242E6),(.3E-3,0.),...
(.5E-3,0.)

INITIAL

PARAM MTG=71.,E=27.E6,GS=11.E6,L=1.06,A=2.8,IRS=.61,D=1.88

PARAM EL=.25,SIGY=90000.

ALELB=.7*EL

ALELS=ALELB/1.7

CONST G=386.

H=MTG/G

MP=.21*SIGY*A*D

RP=4.*MP/L

EK=(3.14/L)*2*D/(4.*L*D*2)

K=1./(L*D*3/(24.*E*IRS)+L/(2.*A*GS))

DYNAMIC

FT=AFGEN(TH,TIME)

PROC DDY=FUNC(MP)

DDY=(FT-K*Y)/H

IF(HOM.GT.MP)DDY=(FT-RP)/H

ENDPROC

DY=INTGRL(0.,DDY)

Y=INTGRL(0.,DY)

BSTRN=E*K*Y

SIG1=E*BSTRN

SIG2=HOM*D/(2.*IRS)

HOM=FT*L/4.

SSTRN=Y/L

RE=K*Y

* 'RATIO' IS CALCULATED IN ACCORDANCE WITH APPENDIX A9231 OF THE

* ASME III B&PV CODE

RATIO=SQRT((BSTRN/ALELB)**2+(SSTRN/ALELS)**2)

TAU1=GS*SSTRN

TAU2=FT/(2.*A)

METHOD RKFX

* RANGE BSTRN,SSTRN,SIG1,SIG2,TAU1,TAU2,Y,MP,RP,RE,RATIO

TIER FINTIH=.0003,PRDEL=.00001,DELT=.0000001

PRINT BSTRN,SSTRN,Y,FT

NGSORT

IF(Y.LY.0.)CALL FINISH

TERMINAL

WRITE(6,10)H

10 FORIAT(/5X,'STIFFNESS OF ROCKSHAFT(K) =',E15.3)

WRITE(6,11)RATIO

11 FORIAT(/5X,'INTERACTIVE STRAIN RATIO =',F15.3)

END

STOP

OUTPUT VARIABLE SEQUENCE

ALELB ALELS H MP RP EK K DDY DY Y
BSTRN SIG1 FT HOM SIG2 RE SSTRN RATIO TAU1 TAU2

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CALCULATION IDENTIFICATION NUMBER

J.O. OR W.C. NO.

DIVISION & GROUP

CALCULATION NO.

OPTIONAL TASK CODE

PAGE *10-15*VOIDTIME HISTORY STRAIN IN THE ROCKSHAFT FROM KICKBACK LOAD
1B21-A0V032A CHECK VALVE
GSU/RIVER BEND POWER STATION - UNIT 1

TIME	BSTRN	SSTRN	Y	FT
.0	.0	.0	.0	6.2500E-02
1.000000-05	7.9778E-06	2.0504E-06	2.1735E-06	24200.
2.000000-05	6.3804E-05	1.6399E-05	1.7383E-05	48400.
3.000000-05	2.1416E-04	5.5043E-05	5.8346E-05	72400.
4.000000-05	5.0321E-04	1.2933E-04	1.3709E-04	96800.
5.000000-05	9.7136E-04	2.4966E-04	2.6464E-04	1.2100E+05
6.000000-05	1.6542E-03	4.2516E-04	4.5067E-04	1.4520E+05
7.000000-05	2.5802E-03	6.4315E-04	7.0294E-04	1.6940E+05
8.000000-05	3.6209E-03	9.3064E-04	9.8648E-04	1.9360E+05
9.000000-05	4.7149E-03	1.2118E-03	1.2645E-03	2.1780E+05
1.000000-04	5.9104E-03	1.5191E-03	1.6102E-03	2.4200E+05
1.100000-04	7.2476E-03	1.8626E-03	1.9745E-03	2.6200E+05
1.200000-04	8.7349E-03	2.2450E-03	2.3797E-03	2.8200E+05
1.300000-04	1.0372E-02	2.6658E-03	2.8257E-03	2.9200E+05
1.400000-04	1.2159E-02	3.1251E-03	3.3126E-03	2.9200E+05
1.500000-04	1.4096E-02	3.6229E-03	3.8403E-03	2.9200E+05
1.600000-04	1.6183E-02	4.1592E-03	4.4087E-03	2.9200E+05
1.700000-04	1.8419E-02	4.7339E-03	5.0180E-03	2.9200E+05
1.800000-04	2.0805E-02	5.3472E-03	5.6681E-03	2.9200E+05
1.900000-04	2.3341E-02	5.9990E-03	6.3590E-03	2.9200E+05
2.000000-04	2.6027E-02	6.6894E-03	7.0908E-03	2.9200E+05
2.100000-04	2.8863E-02	7.4183E-03	7.8634E-03	2.9200E+05
2.200000-04	3.1849E-02	8.1857E-03	8.6769E-03	2.9200E+05
2.300000-04	3.4985E-02	8.9917E-03	9.5312E-03	2.9200E+05
2.400000-04	3.8271E-02	9.8362E-03	1.0426E-02	2.9200E+05
2.500000-04	4.1706E-02	1.0719E-02	1.1362E-02	2.9200E+05
2.600000-04	4.5292E-02	1.1641E-02	1.2339E-02	2.9200E+05
2.700000-04	4.9008E-02	1.2596E-02	1.3352E-02	1.8150E+05
2.800000-04	5.2458E-02	1.3483E-02	1.4292E-02	1.2100E+05
2.900000-04	5.4773E-02	1.4078E-02	1.4922E-02	60501.
3.000000-04	5.5739E-02	1.4326E-02	1.5185E-02	.0

*** SIMULATION HALTED FOR FINISH CONDITION TIME 3.0000E-04

STIFFNESS OF ROCKSHAFT(N) = 0.495E+08

INTERACTIVE STRAIN RATIO = 0.348

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CALCULATION IDENTIFICATION NUMBER				PAGE 10-16
J.O. OR W.O. NO. 12210	DIVISION & GROUP NINCC)	CALCULATION NO. 2042	OPTIONAL TASK CODE SGE	

Point ImpactVOID

Point impact stresses occur before uniform impact stresses and therefore are treated independently. From Table 8.2, the worst case loads on the rock shaft are 100 kips (F_{AK}) and 290 in kips (M_{YK}). These combine to load the rock shaft

$$= \frac{F_{AK}}{2} + \frac{M_{YK}}{Grip} \quad (\text{maximum})$$

$$= 50 + 290 / 16.4$$

$$= 68 \text{ in Kips}$$

Since this is much smaller than the load from uniform kick back, no further analysis is required.

A 5010 65

CALCULATION IDENTIFICATION NUMBER				PAGE 10-11
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	SQE	

10.3 Tail LinkVOID

From Table 8.4, the moment acting on the weakest section of the tail link, where it joins the dish 4.5" from & rock shaft, the bending moment is 960 in-kips. The bending stress is calculated as

$$\sigma_b = \frac{M C}{I} = 960000 \left(\frac{2.9}{2} \right) / 16.$$

$$= 87000 \text{ psi}$$

The allowable tensile stress

$$= .7 S_u (\text{strain rate factor})$$

$$= .7 (70000) (2) = 98000 > 87000 \text{ psi (OK)}$$

The shear stress acting on this section

$$= 240000 / 43 = 5600 \text{ psi}$$

the allowable shear stress

$$= .7 \left(\frac{70000}{1.7} \right) (\text{strain rate factor})$$

A 5010.65

CALCULATION IDENTIFICATION NUMBER				PAGE 10-18
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
11210	NA9(C)	2043	SQIE	

$$= 57,000 \text{ psi} > 5600 \text{ VOID } (612)$$

Applying section 9231 of appendix A, ref. 9,

$$R = \sqrt{R_z^2 + R_b^2} < 1$$

$$= \left[\left(\frac{5600}{57000} \right)^2 + \left(\frac{87000}{98000} \right)^2 \right]^{1/2}$$

$$= .89 < 1$$

(OK)

therefore, the integrity of the tail link is demonstrated.

▲ 5010 85

CALCULATION IDENTIFICATION NUMBER				PAGE 10-19
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	30E	

10.4 Seat/Disk Interface

VOID

The stress in the disk and seat at their interface is calculated using the familiar water hammer equation

$$\Delta \sigma = \frac{\Delta u}{c}$$

where Δu = the impact speed and c = the sonic speed in steel. Thus

$$\Delta \sigma = \frac{45.27 \times 10^6}{16,400} = 74,000 \text{ psi compression}$$

The allowable tensile stress

$$= .7 S_u (\text{strain rate factor})$$

$$= .7(70,000) = 98,000 \text{ psi } > 74,000$$

Hence, the integrity of the seat/disk interface is established, from which it is concluded that the leakage will be within acceptable limits.

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CALCULATION IDENTIFICATION NUMBER				PAGE <u>F-1</u> of <u>07</u>
J.O. OR W.O. NO. <u>15471.10</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

REV. 2

APPENDIX F

Confirmation of Conclusion

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CALCULATION SHEET

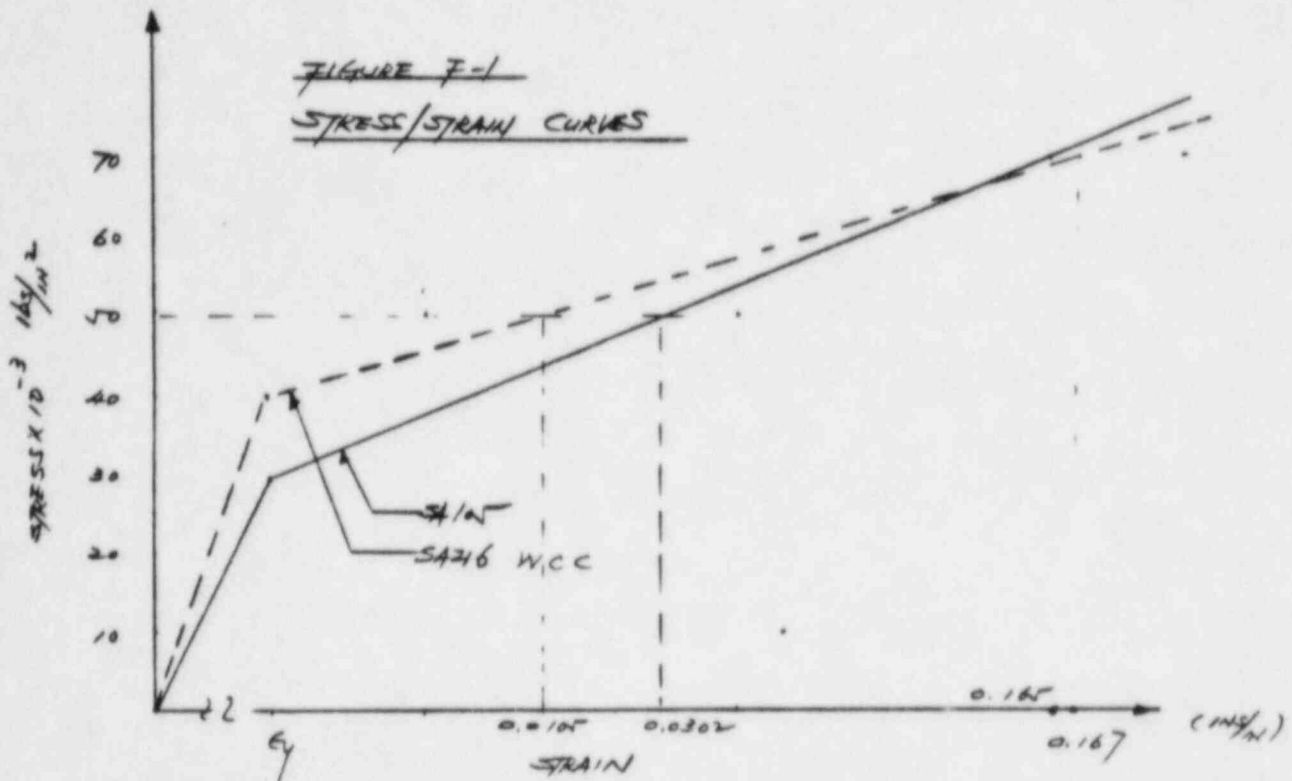
▲ 5010 85

CALCULATION IDENTIFICATION NUMBER				PAGE <u> </u> F-2
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>N19(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>3Q1E</u>	
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6				
7	F.1 <u>OBJECTIVE</u>			
8	The objective of this Appendix is to confirm the conclusions of Section 5 above.			
9				
10	F.2 <u>METHOD</u>			
11				
12	Inelastic systems and component analyses are carried out using the ANSYS			
13	computer program(Ref.18), for comparison of stresses to ASME III Class 1			
14	allowables for faulted service. The stress/strain relationship was approximated			
15	by a bilinear curve(Fig.F-1) adjusted for strain rate and temperature effects.			
16	The pin and stud are not considered since original stresses therein were below			
17	ASME III allowables. Deflection of the disks from their centered position on the			
18	seat is not significant.			
19				
20	The dynamic loading just before impact is purely centrifugal, reacted by the			
21	rockshaft, stud and tail link. The resulting forces on the rockshaft and stud,			
22	the stress in the tail link were calculated by the ANSYS(Ref.18) program. Shear			
23	stresses in the rockshaft and stud were obtained manually.			
24				
25	A second run was coded to calculate the force on the rockshaft and the stress in			
26	the disk from impact of the F010 tail link on its disk. Shear stress in the			
27	rockshaft was, again, determined manually.			
28				
29	A third run was coded for computation of the seat force/deflection curve and the			
30	seat stress arising from impact.			
31				
32	A fourth run was coded to compute AOVF032 disk stress and seat force generated			
33	by impact and pressure surge, using the seat force/deflection curve obtained			
34	from the third program.			
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CALCULATION IDENTIFICATION NUMBER				
J.O. OR W.O. NO. 12210	DIVISION & GROUP N/A(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SDE	PAGE F-3



F.3 ASSUMPTIONS

- 1) The clearance between the tail link bushing and the stud on the disk is sufficient to preclude transfer of significant moment at the instant of impact. This allows analysis of the disk and tail link to be carried out independently.
(confirmation not required)
- 2) The clearance between the AOVFO32 rockshaft and integral tail link is sufficient to preclude transfer of any significant force to the rockshaft from impact of the disk assembly at point not on its center of percussion.

F.4 DESIGN INPUT

Same as Section 4 above.

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CALCULATION IDENTIFICATION NUMBER				PAGE <u>F-4</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>N/A(2)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

F.5 SUMMARY OF RESULTS

TABLE F5.1	AOVF32				F010			
	Calc'd	Allow.	FS	PG#	Calc'd	Allow.	FS	PG#
Disk -stress ksi	38.7	49.	1.27	F-17	40.	49.	1.2	F-15
Rockshaft -stress ksi	4.4	28.3	6.5	F-8	3.7	28.3	7.7	F-6
Tail Link -stress ksi	N/A	N/A	-	-	32.6	49.0	1.5	F-8
Seat -stress ksi	46.6	49.	1.0	F-23	45.	49.	1.07	F-25

F.6 CONCLUSIONS

The results of Section F.5 confirms the conclusion of Section 6 above, that both valves will withstand rapid closure and that the pressure boundary will be maintained.

F.7 REFERENCES (added to the list in Section 7)

18. ANSYS Engineering Analysis System User's Manual, Revision 4,
3-1-1983, SWEC Program ST-348.04/1C

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CALCULATION IDENTIFICATION NUMBER				PAGE
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	EQE	F-5

F8.0 TAIL LINK / ROCKSHAFT

Two loading conditions are considered below:
pre-impact and impact. Critical elements
are the stress in the rockshaft, the stress
in the tail link section itself & the stress
in the stud which secures the dish to
the tail link.

F010 VALVE

Pre-Impact, (Run # 1)



The dish is accelerated about the rockshaft
by the reversed flow until impact with the sent.
The variation of centrifugal
force, F_c , with time ^{was} obtained from the time
history angular velocity derived in Ref. 1.

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CALCULATION IDENTIFICATION NUMBER				PAGE K-6
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SDE	

The tail link was modelled for AHSYS (Ref. 18) computation as shown in the sketch. All of the disk weight (260^{lb}) plus 1/2 the tail link weight was lumped at node 7.

Results were as follows, from run[#] 1 :

Maximum stress intensity in the tail link

$$= 29300 \text{ psi} < 49000 \quad FS = 1.7$$

(OK)

(Maximum deflection of the disk from its centered position

$$= .004" \ll .625$$

which demonstrates its insignificance.)

Rockshaft

Stress in double shear

$$= 20500 / 2.7(2) = 3,660 \text{ psi} < \frac{49000}{1.8} = 28300 \quad FS = 7.7$$

(OK)

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CALCULATION SHEET

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CALCULATION IDENTIFICATION NUMBER				PAGE
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	SQE	F-7

Impact, (Run # 2)

At impact, the critical elements are stress in the tail link and the rock shaft. The same model as described above was used for ANSYS computation except that disk mass is not included and the stud was replaced by a zero gap element of stiffness equal to $1.5E7$, the elastic stiffness of the disk. This is conservative because, since the disk is actually deflecting, its stiffness as seen by the tail link is reduced.

The maximum reaction at the rock shaft, from the computer run, was 23800 lb , which results in a shear stress

$$= \frac{23800}{2(2.8)} = 4,300 \text{ psi} < \frac{49000}{\sqrt{3}} < 28350$$

FS = 6.6

(OK)

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CALCULATION SHEET

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CALCULATION IDENTIFICATION NUMBER				PAGE F8
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SOE	

From the computer run, the maximum stress in the tail link

$$= 33,600 \text{ psi} < 49,000$$

$$F_s = 15$$

(OK)

AOVF32 VALVE (No tail link)

Pre-Impact

From Table 8.1, the maximum force on the rock shaft results in a shear stress

$$= \frac{24,400}{2(2.8)} = 4,400 < 28,300 \quad F_s \leq 6.5$$

(OK)

Impact

The clearance between the disk attachment and the rockshaft precludes any significant reaction.

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CALCULATION SHEET

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CALCULATION IDENTIFICATION NUMBER				PAGE
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	SOE	F-9

F9.0 DISK ANALYSIS

Pre-impact loads on the disk are not significant, and therefore not addressed. At impact, the critical stresses are bending and membrane as a plate and the local stresses from contact with the seat. However, the disk material being the same as the seat, it is expected that the local stresses will be similar to those computed for the seat in section 10.

Figures F.2 and F.3 show the nodes and elements of the disk as modelled for ANSYS computation. Attached to the rim nodes are elements which represent the non-linear force/deflection character of the seat, obtained from the seat analysis in Section F.10, reduced by a factor of 2 to account for the contribution from local yielding of the disk.

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CALCULATION SHEET

Calculation Identification Number				PAGE
J.O./W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL CODE	
12210	NM(C)	2043	SQE	F-10

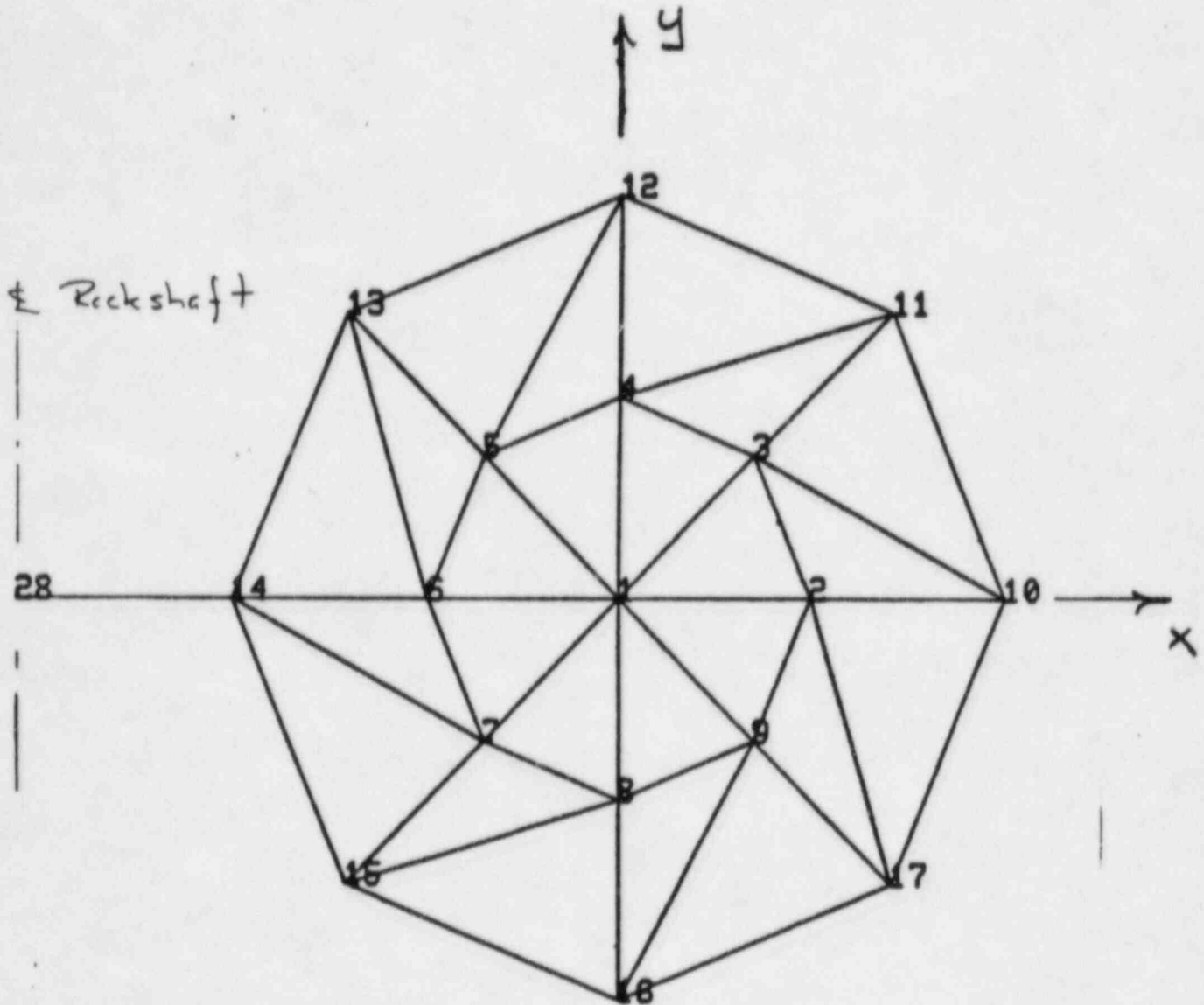


Figure F-2 ANSYS Computer Model for Tail-Link/Disk Impact Loads(Nodes)

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CALCULATION SHEET

Calculation Identification Number				PAGE
J.O./W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL CODE	
12210	MM(C)	2043	SQE	I-11

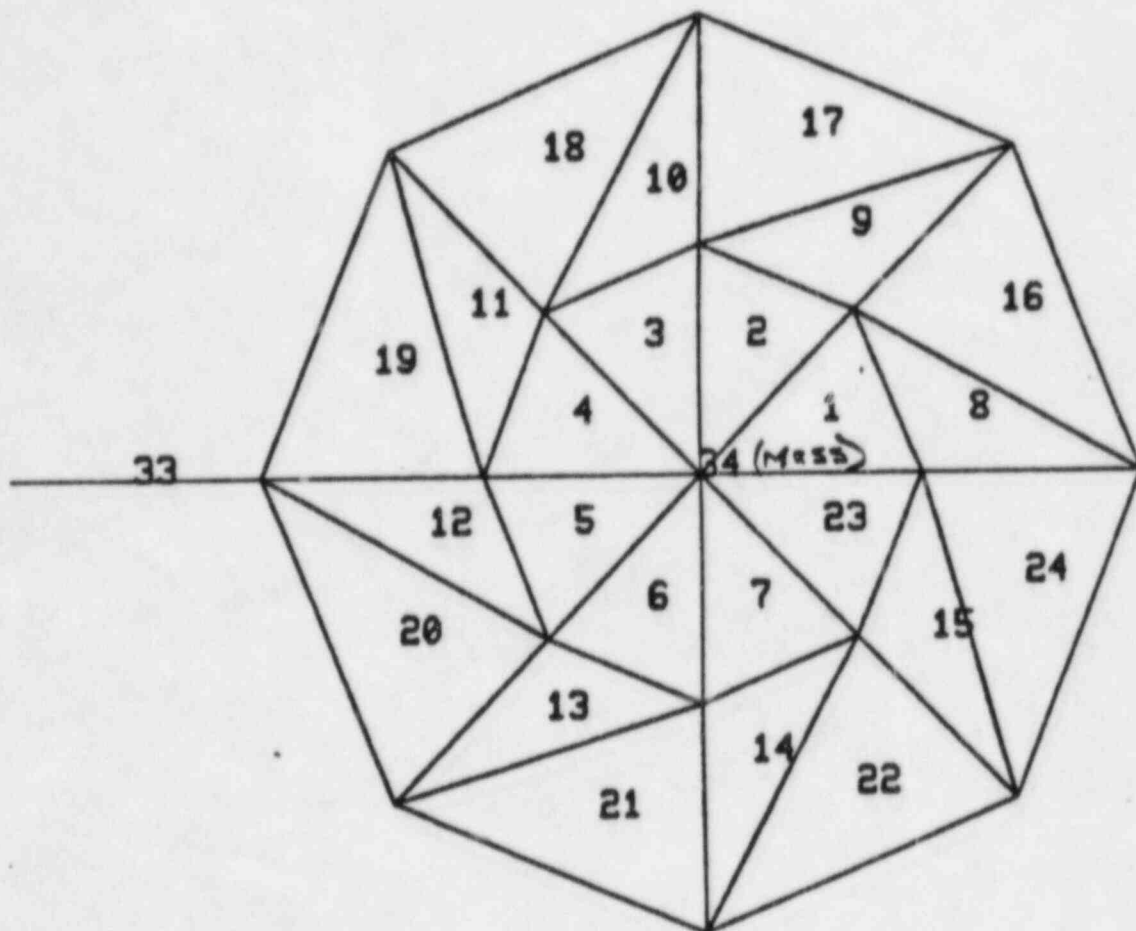


Figure F-3 ANSYS Computer Model for Tail-Link/Disk Impact Loads (Elements)

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CALCULATION SHEET

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CALCULATION IDENTIFICATION NUMBER				PAGE F-12
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE	

Stresses are printed for element 23, representing the worst case for the center elements, and for element 24, representing the worst case for the rim elements.

The seat force is printed out for later use in calculating the seat stress.

The seat analysis of section F.10 shows that after the seat force from disk impact is $1.2EG^\#$, the stiffness of the seat is greatly reduced by plasticity. This was not modelled in the ANSYS run because two non-linearities, gap stiffness representing the seat and the dish material, occurring in the same time frame could not be made to converge. Thus, the seat force from the ANSYS runs is greatly overstated.

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CALCULATION IDENTIFICATION NUMBER				PAGE F-13
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	N/M(C)	2043	SQE	

FOIO DISK (Run # 6)

The FOIO disk is a 2.75 inch circular plate 17.5 inches in diameter, modelled as a 2.75" circular plate 16.8 inches in diameter to simulate the effective seat diameter. Actual seat diameter is 15.75".

Initial nodal velocities input to DNSVS from Ref. 1 are shown in Table F-2.

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TABLE F-2 - INITIAL AURAL VELOCITIES FOR
GNSSys IN RT

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J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NM(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SQE</u>	

Maximum stress intensity in the disk, as
computed by the ANSYS program, element 24

$$= 40,000 \text{ psi} < 49000$$

$$FS = 1.23$$

OK

from disk response as a plate.
Local stress from impact is expected to
be similar to the local stress in the
seat, as described in section 10 below.

Maximum seat force from the ANSYS
run, variable 21

$$= 2.1 \text{ E } 6 \text{ \#}$$

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CALCULATION IDENTIFICATION NUMBER				PAGE <u>16</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>NA(C)</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SPE</u>	

A0VF032 DISK (Run # 5)

the A0VF032 disk is a tapered circular plate of 19" diameter, 2.5" thick at the rim and 3.75" diameter at the center. It was modelled as a 3" circular plate to simulate the average thickness of the outer elements in the model, with a concentrated mass at its center

$$= 260 - 3\pi 8.8^2 (.283) = 260 - 207$$

$$= 53^{\#} (.138 \text{ in/slugs})$$

the 17.6" dimension is the estimated diameter of the seat pressure. The seat ID is 16.75"

Initial modal velocities obtained from Ref. 1 are shown in Table F-3.

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CALCULATION IDENTIFICATION NUMBER				PAGE
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NAT(C)	2043	5QE	F-17

Maximum stress intensity in the disk, as
computed by the ANSYS program, element 24

$$= 38,700 \text{ psi} < 49000$$

$$FS = 1.27$$

OK

from disk response as a plate.
Local stress from impact is expected to
be similar to the local stress in the
seat, as described in section 10 below.

Maximum seat force from the ANSYS
run, variable 21

$$= 2.0 \text{ EG } \#$$

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▲ 9010.05

CALCULATION IDENTIFICATION NUMBER			
J.O. OR W.O. NO. 12210	DIVISION & GROUP NMCC	CALCULATION NO. 2043	OPTIONAL TASK CODE SDE
			PAGE 15-18

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	NOISE NUMBER			
NOVF32				
42				
End/sec	14	13, 15	6	5, 7
Radius (in)	5	7.6	9.5	10.8
Vel. (ips)	210	307	460	533
				588
				722
				777
				911
				966

TABLE F.3 - INITIAL NORMAL VELOCITIES FOR
ANALYSIS

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CALCULATION IDENTIFICATION NUMBER				PAGE F-19
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE 3QE	

F.10 SEAT ANALYSIS

A 12° sector of the seat annulus was modelled for ANSYS computation of stress intensity and deflection as a function of the impact pressure. Figure F-4 shows the elements while Figures F-5 and F-6 identify the nodes on the left and right hand faces respectively.

The loading was on elements 12, 18 and 24 in the z direction, varied from .5EG to 2EG # over an area of 22.3 in^2 .

Deflection was calculated as the average of nodes 25, 26, 29, 30, 73-76. Maximum stress was selected from the loaded elements and adjacent elements 10-12, 17, 18 & 24.

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Calculation Identification Number				PAGE
J.O./W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL CODE	
12210	MM(C)	2043	SQE	F-20

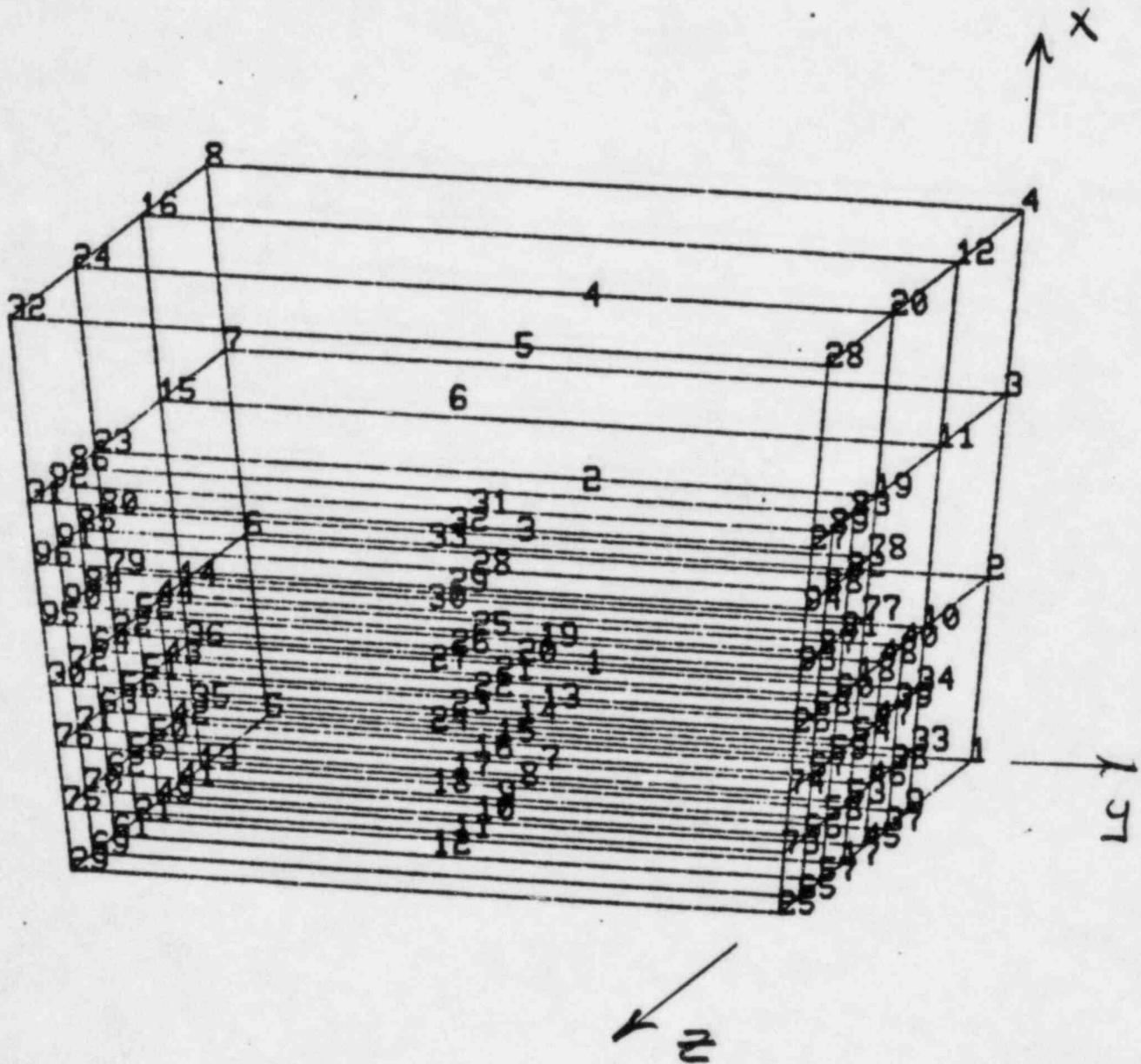


Figure F-4 Computer Model for Seat Analysis

Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE
J.O./W.O. NO.	DIVISION & GROUP NM(C)	CALCULATION NO.	OPTIONAL CODE SQE	
12210		2043		F-21

32			24			16		8
31	92	86	23			15		7
96	91	85	80					
95	90	84	79					
30	72	64	22	52	44	14		6
76	71	63	56	51	43	36		
75	70	62	55	50	42	35		
29	69	61	21	49	41	13		5

Figure F-5 - Nodes for Left Face of Model

Stone and Webster Engineering Corporation
CALCULATION SHEET

Calculation Identification Number				PAGE
J.O./W.O. NO.	DIVISION & GROUP MM(C)	CALCULATION NO.	OPTIONAL CODE SQE	
12210		2043		15-2E

28				20				12				4
27	89	83	19					11				3
94	88	82	78									
93	87	81	77									
26	68	60	18	48	40	10						2
74	67	59	54	47	39	34						
73	66	58	53	46	38	33						
25	65	57	17	45	37	9						1

Figure 5.6 - Nodes for Right Face of Model

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

▲ 5010.65

CALCULATION IDENTIFICATION NUMBER				PAGE F-23
J.O. OR W.O. NO. 12210	DIVISION & GROUP N/M(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE	

170VF032 SEAT (Run # 1302)

Results of the seat analysis are summarized in Table F-4 below.

From the dish analysis of Section F.9, the maximum seat force is calculated

$$= 2.0 \text{ E6 } \#$$

which is overstated because the seat stiffness, as shown in that Table, drops drastically below its elastic value after the seat force is above $1.2 \text{ E6 } \#$. Hence, the actual seat force is closer to

$$1.2 \text{ E6 } + \frac{(2.0 - 1.2) \text{ E6}}{4} = 1.4 \text{ E6 } \#$$

which corresponds to a seat stress from the Table by interpolation.

$$= 46,600 \text{ psi} < 49,000 \quad F.S. = 1.05$$

* SEE PAGE F-27 FOR ALTERNATE CALCULATION

OK

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

▲ 5010 85

CALCULATION IDENTIFICATION NUMBER				PAGE <u>F-24</u>
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	
12210	NM(C)	2043	59E	

Total Force # (WOB)	812270	1001430	1224000	1446500	1669000	1891600	2114100	2225400
Element Pressure (Ksi)	36.5	45	55	65	75	85	95	100
Average Disp. (in.)	.225E-2	.2926E-2	.4429E-2	.7219E-2	.1162E-1	.1792E-1	.2674E-1	.3602E-1
Stiffness (lb/in)	361E8	342E8	276E8	200E8	144E8	106E8	791E7	618E7
Stress (Ksi)	34.3	43.5	45.8	47.4	49.3	51.5	54	56.5

TABLE F-4 - RESULTS OF ANALYSIS SEAT ANALYSIS
FOR 170V F 032
(Radial Free)

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

▲ 5010 65

CALCULATION IDENTIFICATION NUMBER				PAGE F-25
J.O. OR W.O. NO. 12210	DIVISION & GROUP NA1(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SQE	

F010 SEAT (Run # 4)

Results of the ANSYS seat analysis are summarized in Table F-10.2 below.

From the disk analysis of Section F.9, the maximum seat force = $2.0 \text{ EG}^{\#}$, which results in a seat stress

$$= 45,000 \text{ psi} < 49 \text{ ksi} \quad FS = 1.09 \quad \textcircled{\text{OK}}$$

obtained by interpolation of Table F-4.

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

5010.65

CALCULATION IDENTIFICATION NUMBER				PAGE F-26
J.O. OR W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL TASK CODE SPE	

Total Force ~ # (NOV)	578,600	778,890	890,160	1,001,430	1,112,700	1,325,240	1,517,780	2,002,800
Element Pressure (KSI)	26	35	40	45	50	60	70	90
Average Displ. (IN.)	.1072E-2	.1513E-2	.2082E-2	.2981E-2	.4021E-2	.697E-2	1.0647E-2	1.773E-2
Stiffness (LB/IN)	5.4E8	5.1E8	4.3E8	3.4E8	2.6E8	1.9E8	1.5E8	1.1E8
Stress (KSI)	25.9	32.3	33.5	34.5	35.7	37.5	40.0	44.1

TABLE F-5 - RESULTS OF ANALYSIS SEAT ANALYSIS
FOR F010
(Radial Held)

STONE & WEBSTER ENGINEERING CORPORATION
CALCULATION SHEET

▲ 5010 65

CALCULATION IDENTIFICATION NUMBER				PAGE <u>F27</u>
J.O. OR W.O. NO. <u>12210</u>	DIVISION & GROUP <u>EMU/EQS</u>	CALCULATION NO. <u>2043</u>	OPTIONAL TASK CODE <u>SOE</u>	

ATTACHMENT TO APPENDIX F

ALTERNATE CALCULATION FOR BOX 032 SEAT FORCE

THIS ATTACHMENT IS INTENDED TO SUBSTANTIATE THE REASONING FOR ADJUSTING THE MAX. SEAT FORCE IN PAGE F-23 AS FOLLOWS

FROM PAGE F24, THE LIMIT FORCE IS APPROXIMATELY:

$$F_y = \underline{812,300 \text{ \#}}$$

FROM PAGE F23, THE MAX. ELASTIC SEAT FORCE IS

$$F_e = \underline{2.026 \text{ \#}}$$

FROM EQ. II.7, THE WORKING DUCTILITY RATIO IS

$$\mu = \left(1 - \frac{1}{\xi}\right) + \left[\frac{1}{\xi} \left(\frac{1}{\xi} - 1 + \left(\frac{D}{C}\right)^2\right)\right]^{\frac{1}{2}}$$

WHERE

$$\xi = \text{RATIO OF PLASTIC STIFFNESS AND ELASTIC STIFFNESS} \\ = \frac{k_2}{k_1} = \frac{1.7 \text{ E7}}{3.6 \text{ E8}} = \underline{0.139}$$

$$D = \text{DEMAND LOAD} = F_e = 2.026 \text{ \#} \Rightarrow \frac{D}{C} = \underline{2.5}$$

$$C = \text{CAPACITY} = F_y = 812,300 \text{ \#}$$

$$\mu = \left(1 - \frac{1}{0.139}\right) + \left[\frac{1}{0.139} \left(\frac{1}{0.139} - 1 + (2.5)^2\right)\right]^{\frac{1}{2}}$$

$$\mu = \underline{3.3}$$

FROM EQ. III.3 THE DYNAMIC IMPACT LOAD, IF THE SEAT IS MODELED AS ELASTO-PLASTIC ELEMENT IS

$$F_d = F_y [1 + \xi (\mu - 1)]$$

$$= 812,300 [1 + 0.139 (3.3 - 1)]$$

$$= \underline{1.07 \text{ E6 \#}} < \underline{1.14 \text{ E6}} \text{ AS USED IN THIS CALCULATION}$$

THEREFORE IT'S CONSERVATIVE TO USE 1.14 E6 \#

(SEE
F-25)

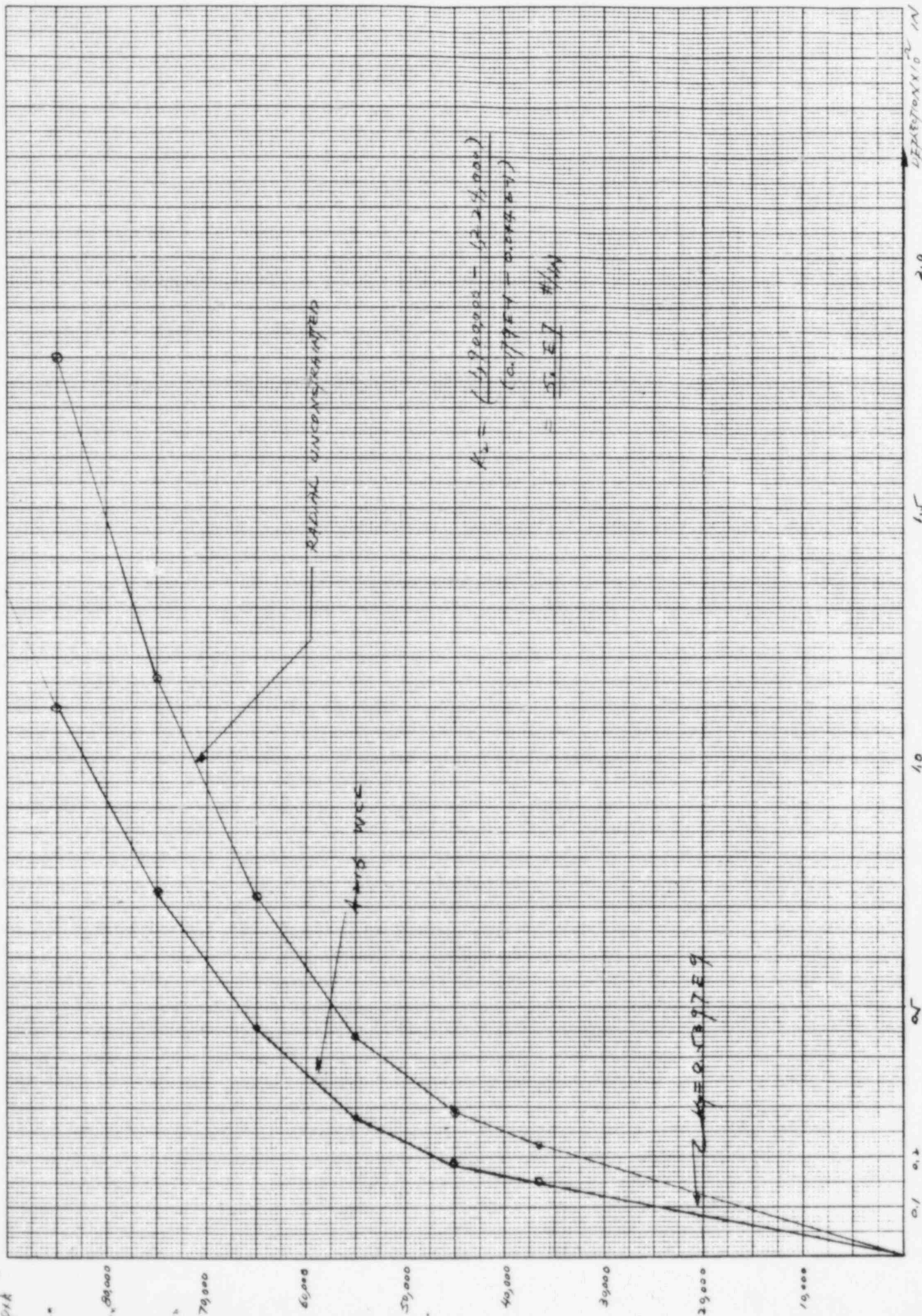
K-E

10 X 10 TO THE CENTIMETER 18 X 25 CM
KEUFFEL & ESSER CO. MADE IN U.S.A.

46 1512

$A = 0.7418 \text{ IN}$

$A_0 F_{52}$

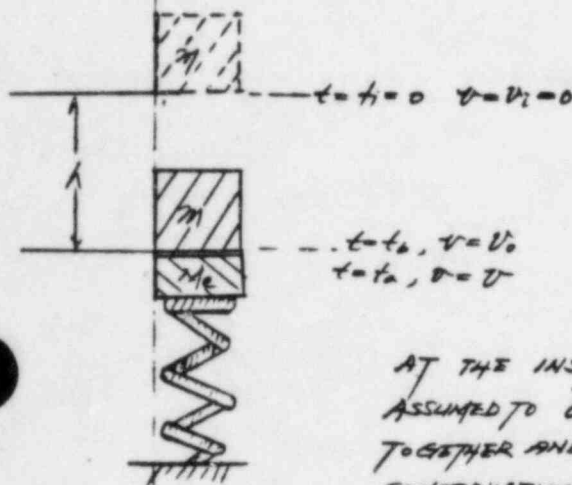


SITUATION A MASS m IS DROPPED VERTICALLY DOWNWARD FROM A HEIGHT h AND IS STOPPED BY THE ELASTO-PLASTIC ELEMENT REPRESENTED AS A SPRING MASS SYSTEM. THE MASS OF THE ELASTO-PLASTIC ELEMENT IS LUMPED AS M_0

METHOD ENERGY METHOD

ANALYSIS

1. MATERIAL IS PERFECT-PLASTIC



FROM THE TIME t_1 THAT THE MASS m IS BEGINNING TO FALL, THE VELOCITY THEN IS ZERO. AFTER MOVING A DISTANCE h THE VELOCITY INCREASES TO v_0 . FROM FIG. THE K.E. OF THIS MASS, m , JUST PRIOR TO IMPACT, IS

$$(K.E.)_0 = \frac{1}{2} m v_0^2 \quad \text{--- (1)}$$

THE K.E. IS GAINED FROM THE POTENTIAL ENERGY

$$\frac{1}{2} m v_0^2 = mgh \quad \text{--- (2)}$$

AT THE INSTANT OF IMPACT, LOCAL INELASTIC DEFORMATION IS ASSUMED TO OCCUR, AFTER WHICH BOTH MASSES ARE ASSUMED TO MOVE TOGETHER AND SPRING COMPRESSION TAKES PLACE. FROM MOMENTUM CONSERVATION, ONE HAS

$$m v_0 = (m + M_0) v \quad \text{--- (3)}$$

$$v = \frac{m v_0}{(m + M_0)} = \frac{1}{(1 + \frac{M_0}{m})} v_0$$

LET
$$C = \frac{1}{(1 + \frac{M_0}{m})}$$

$$v = C v_0 \quad \text{--- (4)}$$

THE TOTAL KINETIC ENERGY OF BOTH BODIES AT THE TIME LOCAL INELASTIC DEFORMATION IS COMPLETE

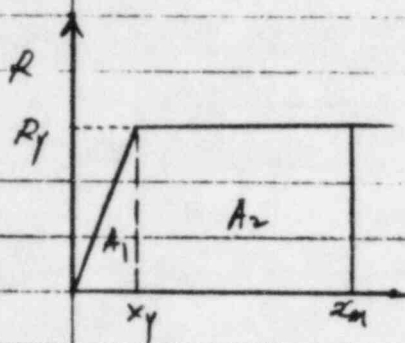
$$(K.E.)_1 = \frac{1}{2} (m + M_0) v^2 \quad \text{--- (5)}$$

$$= \frac{1}{2} (m + M_0) C^2 v_0^2 = \frac{1}{2} m (1 + \frac{M_0}{m}) C^2 v_0^2$$

$$(K.E.)_1 = \frac{1}{2} m C v_0^2 \quad \text{--- (6)}$$

FROM (4), (6) BECOMES

$$(K.E.)_1 = mgh C \quad \text{--- (7)}$$



THE TOTAL WORK DONE IN COMPRESSING THE SPRING IS EQUAL TO THE TOTAL K.E. AVAILABLE AT THE TIME LOCAL INELASTIC DEFORMATION IS COMPLETE PLUS THE POTENTIAL ENERGY AVAILABLE RESULTING FROM THE DISPLACEMENT OF SPRING WILL COMPRESS

$$\begin{aligned} & \frac{1}{2}(m+M_0)v^2 + (m+M_0)g x_m \\ &= mgh_c + (m+M_0)g x_m \\ &= mgh_c + m\left(1 + \frac{M_0}{m}\right)g x_m \\ &= mgh_c + \frac{m g x_m}{\mu} \quad \text{--- (8)} \end{aligned}$$

THE INTERNAL STRAIN ENERGY IS

$$(P.E.) = A_1 + A_2 = R_y x_m - R_y \frac{x_y}{2} \quad \text{--- (9)}$$

BY ENERGY BALANCE, THE TOTAL STRAIN ENERGY STORED IN THE SPRING MUST EQUAL THE TOTAL WORK DONE.

$$R_y x_m - R_y \frac{x_y}{2} = mgh_c + \frac{m g x_m}{\mu} \quad \text{--- (10)}$$

BUT

$$R_y = k x_y \quad \text{and} \quad x_m = \mu x_y = \mu \frac{R_y}{k} \quad \text{--- (11)}$$

$$\left(\frac{\mu}{k}\right) R_y^2 - \frac{R_y^2}{2k} = mgh_c + \frac{m g \mu}{k} R_y$$

$$\left(\frac{\mu}{k} - \frac{1}{2k}\right) R_y^2 = mgh_c + \frac{m g \mu}{k} R_y$$

$$\left(\frac{2\mu - 1}{2k}\right) R_y^2 = mgh_c + \frac{m g \mu}{k} R_y$$

$$R_y^2 - \frac{m g \mu}{(2\mu - 1)} R_y - \frac{2 m g k h_c}{(2\mu - 1)} = 0$$

$$\text{LET } a = \frac{m g}{(2\mu - 1)}$$

$$R_y^2 - 2a\mu R_y - 2akh_c = 0$$

$$R_y = a\mu + \sqrt{(a\mu)^2 + 2akh_c}$$

$$R_y = a\mu \left[1 + \sqrt{1 + \frac{2kh_c}{\mu^2 a}} \right] \quad \text{--- (12)}$$

THE PERIOD OF THE SYSTEM IS

$$T = 2\pi \sqrt{\frac{m+M}{k}} \quad \text{----- (2)} \Rightarrow \frac{k}{(m+M)} = \frac{4\pi^2}{T^2}$$

$$R_y = \frac{mg\mu}{(2\mu-1)} \left[1 + \sqrt{1 + \frac{2kh(2\mu-1)}{\mu^2 mg}} \right]$$

$$= mg \frac{\mu}{(2\mu-1)} \left[1 + \sqrt{1 + \frac{2kh(2\mu-1)}{\mu^2 (m+M)g}} \right]$$

$$\text{NOTE } h = \frac{V_0^2}{2g}$$

$$= mg \frac{\mu}{(2\mu-1)} \left[1 + \sqrt{1 + \frac{2kV_0^2(2\mu-1)}{2\mu^2(m+M)g^2}} \right]$$

$$= mg \frac{\mu}{(2\mu-1)} \left[1 + \sqrt{1 + \frac{4\pi^2 \mu^2 (2\mu-1)}{\mu^2 T^2 g^2}} \right]$$

$$R_y = mg \left(\frac{\mu}{2\mu-1} \right) \left[1 + \sqrt{1 + \left(\frac{2\mu-1}{\mu^2} \right) \left(\frac{2\pi \mu}{gT} \right)^2} \right]$$

$$2\pi \left(\frac{2\mu-1}{\mu^2} \right) \left(\frac{2\pi \mu}{gT} \right)^2 \gg 1$$

$$\text{THEN } 1 + \sqrt{1 + \left(\frac{2\mu-1}{\mu^2} \right) \left(\frac{2\pi \mu}{gT} \right)^2} = \left(\frac{2\pi \mu}{gT} \right) \left(\frac{2\mu-1}{\mu} \right) \sqrt{\frac{1}{2\mu-1}}$$

$$\therefore R_y = \frac{2\pi \mu V_0}{T} \sqrt{\frac{1}{2\mu-1}}$$

$$\text{BUT } I = \mu V_0 = \text{MOMENTUM}$$

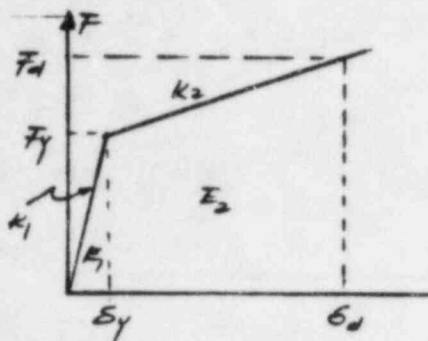
$$W = \frac{2\pi}{T} = \sqrt{\frac{k}{m+M}}$$

$$R_y = \frac{2IW}{\sqrt{2\mu-1}}$$

IN ELASTIC IMPACT, ONE HAS

$$R_e = IW$$

$$\Rightarrow \frac{R_y}{R_e} = \frac{1}{\sqrt{2\mu-1}}$$

I. MATERIAL IS ELASTO PLASTICII.1 DERIVATION OF ABSORBED ENERGY, E_a , I.E. THE INTERNAL STRAIN ENERGY

THE ENERGY ABSORBED BY A DEFORMATION OF THE TARGET IS

$$E_a = \int_0^{\delta_d} F_R(x) dx \quad \text{--- (I.1)}$$

THE INTEGRATION TERM IN THE ABOVE EQUATION CAN BE CARRIED OUT IF WE ASSUME THE MATERIAL OF THE TARGET HAS A BILINEAR FORCE-DISPLACEMENT RELATION AS SHOWN, WHERE

k_1 = ELASTIC STIFFNESS, KIPS/IN

k_2 = PLASTIC STIFFNESS, KIPS/IN

F_y = YIELD FORCE, KIPS

F_d = LOAD AT MAX. DISPLACEMENT, δ_d , KIPS

δ_y = YIELD DISPLACEMENT, IN.

δ_d = MAX. DISPLACEMENT, IN.

INTEGRATE THE ABSORBED ENERGY PIECEWISELY OVER THE FORCE-DISPLACEMENT CURVE, ONE HAS :

$$E_a = E_1 + E_2 = \int_0^{\delta_y} F_R(x) dx + \int_{\delta_y}^{\delta_d} F_R(x) dx \quad \text{--- (II.2)}$$

$$\text{SINCE } F_R(x) = k_1 x \quad \text{--- (II.3)}$$

$$F_R(x) = F_y + k_2(x - \delta_y) \quad \text{--- (II.4)}$$

$$\Rightarrow E_a = \int_0^{\delta_y} k_1 x dx + \int_{\delta_y}^{\delta_d} [F_y + k_2(x - \delta_y)] dx \quad \text{--- (II.5)}$$

$$= k_1 \frac{\delta_y^2}{2} + F_y(\delta_d - \delta_y) + k_2 \left(\frac{\delta_d^2}{2} - \delta_y \delta_d \right) - k_2 \left(\frac{\delta_y^2}{2} - \delta_y^2 \right)$$

$$= k_1 \frac{\delta_y^2}{2} + F_y \delta_d - F_y \delta_y + k_2 \frac{\delta_d^2}{2} (\delta_d - 2\delta_y) + k_2 \frac{\delta_y^2}{2}$$

$$= k_1 \frac{\delta_y^2}{2} + k_1 \delta_y \delta_d - k_1 \delta_y^2 + k_2 \frac{\delta_d^2}{2} - k_2 \delta_d \delta_y + k_2 \frac{\delta_y^2}{2}$$

$$= k_1 \frac{\delta_y^2}{2} + k_1 \delta_y \delta_d - k_1 \delta_y^2 + k_2 \frac{\delta_d^2}{2} - k_2 \delta_d \delta_y + k_2 \frac{\delta_y^2}{2}$$

$$= -(k_1 - k_2) \frac{\delta_y^2}{2} + (k_1 - k_2) \delta_y \delta_d + k_2 \frac{\delta_d^2}{2}$$

$$= -(k_1 \delta_y - k_2 \delta_y) \frac{\delta_y}{2} + \frac{\delta_d}{2} [2k_1 \delta_y - 2k_2 \delta_y + k_2 \delta_d]$$

$$= -(k_1 \delta_y - k_2 \delta_y) \frac{\delta_y}{2} + \frac{\delta_d}{2} [(k_1 \delta_y - k_2 \delta_y) + (k_1 \delta_y + k_2(\delta_d - \delta_y))]$$

$$= -(k_1 \delta_y - k_2 \delta_y) \frac{\delta_y}{2} + \frac{\delta_d}{2} [(k_1 \delta_y - k_2 \delta_y) + F_d]$$

$$E_a = \frac{1}{2} \delta_y (k_1 - k_2) (\delta_d - \delta_y) + \frac{1}{2} F_d \delta_d \quad \text{--- (II.6)}$$

USING THE DEFINITION OF DUCTILITY RATIO $\mu = \frac{\delta_d}{\delta_y}$

$$\Rightarrow E_d = \frac{1}{2} \delta_y^2 (k_1 - k_2) (\mu - 1) + \frac{1}{2} \delta_y F_d \mu \quad \text{--- (2.7)}$$

II.2. DERIVATION OF IMPACT LOAD BY APPLYING RAYLEIGH'S PRINCIPLE

WHEN THE TOTAL KINETIC ENERGY OF THE TWO BODIES IS TRANSFORMED INTO THE POTENTIAL ENERGY OF THE COMPRESSED SPRING, THE WEIGHT COMES TO REST AND THE FORCE COMPRESSING THE SPRING ATTAINS ITS MAX. VALUE.

ACCORDING TO RAYLEIGH'S PRINCIPLE, ONE HAS :

$$(K.E.)_{(min)}_{max} = (P.E.)_{s,max} \quad \text{--- (2.8)}$$

$$\frac{1}{2} (m+M) v^2 = \frac{1}{2} \delta_y^2 (k_1 - k_2) (\mu - 1) + \frac{1}{2} \delta_y F_d \mu$$

FROM ELASTIC IMPACT CASE, WE HAVE SHOWN THAT

$$v^2 = \left(\frac{m}{m+M} \right)^2 v_0^2 \quad \text{(2.9)}$$

THEN

$$\frac{1}{2} (m+M) \left(\frac{m}{m+M} \right)^2 v_0^2 = \frac{1}{2} \delta_y^2 (k_1 - k_2) (\mu - 1) + \frac{1}{2} \delta_y F_d \mu$$

$$\frac{m^2}{(m+M)} v_0^2 = \delta_y^2 (k_1 - k_2) (\mu - 1) + \delta_y F_d \mu$$

OR

$$F_d \delta_y \mu = \frac{m^2}{(m+M)} v_0^2 - (k_1 - k_2) (\mu - 1) \delta_y^2$$

$$F_d = \frac{(m^2 v_0^2)}{(m+M) \mu \delta_y} + (k_1 - k_2) \left(1 - \frac{1}{\mu}\right) \delta_y$$

$$F_d = \frac{(m^2 v_0^2)}{(m+M) \mu \delta_y} + \left(1 - \frac{k_2}{k_1}\right) \left(1 - \frac{1}{\mu}\right) k_1 \delta_y \quad \text{--- (2.10)}$$

$$\text{USING BILINEAR SLOP RATIO} = \xi = \frac{k_2}{k_1} \quad \text{--- (2.11)}$$

$$F_d = \frac{(m^2 v_0^2)}{(m+M) \mu \delta_y} + (1 - \xi) \left(1 - \frac{1}{\mu}\right) k_1 \delta_y \quad \text{--- (2.12)}$$

$$\text{IF } k_1 = k_2 = \text{ELASTIC} \quad F_d = F_1 = k_1 \delta_y, \text{ SO (2.12) BECOMES}$$

$$F = \frac{m^2 v_0^2}{\mu \delta_y (m+M)} \Rightarrow F^2 = \frac{m^2 v_0^2}{\mu \delta_y (m+M)} = \frac{m^2 v_0^2}{\mu}$$

$$\Rightarrow \boxed{F = \frac{m v_0 v_1}{\sqrt{\mu}}} \quad \text{IT'S AGREE WITH ELASTIC RESULT}$$

EQUATION (2.12) CAN BE REWRITTEN TO INCLUDE $K.E. = \frac{1}{2} m v_0^2$

$$F_d = \frac{\frac{1}{2} (m v_0^2) m}{\frac{1}{2} (m+M) \mu \delta_y} + (1 - \xi) \left(1 - \frac{1}{\mu}\right) F_y$$

$$\Rightarrow \boxed{F_d = \frac{2 (K.E.)}{\mu \delta_y (1 + \frac{m}{M})} + (1 - \xi) \left(1 - \frac{1}{\mu}\right) F_y} \quad \text{--- (2.12a)}$$

II.2. DERIVATION OF IMPACT LOAD BY APPLYING CONSERVATION OF ENERGY

$$(K.E.) + (P.E.) = \text{CONSTANT}$$

THE TOTAL WORK DONE IN COMPRESSING THE SPRING IS EQ. TO THE TOTAL K.E. AVAILABLE AT THE TIME LOCAL ELASTIC DEFORMATION IS COMPLETE PLUS THE POTENTIAL ENERGY AVAILABLE RESULTING FROM THE DISPLACEMENT OF SPRING. THIS ADDITIONAL P.E. TO THE INPUT ENERGY IS NOT CONSIDERED IN THE RAYLEIGH'S PRINCIPLE.

THE TOTAL AVAILABLE INPUT ENERGY IS

$$E_i = K.E. + P.E. \quad \text{--- II.3.1}$$

$$E_i = \frac{1}{2} (m+M) v^2 + (m+M) g \delta_d \quad \text{--- II.3.2}$$

$$E_i = \frac{1}{2} \frac{m^2}{(m+M)} v_0^2 + (m+M) g \delta_d \quad \text{--- II.3.3}$$

THEN, BY ENERGY BALANCE

$$E_i = E_o$$

$$\frac{1}{2} \frac{m^2}{(m+M)} v_0^2 + (m+M) g \delta_d = \frac{1}{2} (k_1+k_2) (\mu-1) \delta_y^2 + \frac{1}{2} \delta_y F_d \mu$$

$$F_d \mu \delta_y = \frac{m^2 v_0^2}{(m+M)} + 2(m+M) g \delta_d - (k_1+k_2) (\mu-1) \delta_y^2$$

$$F_d = \frac{m^2 v_0^2}{(m+M) \mu \delta_y} + 2(m+M) g \frac{1}{\mu} \cdot \frac{\delta_d}{\delta_y} - (k_1+k_2) \left(\frac{1}{\mu} - 1\right) \delta_y$$

$$\begin{aligned} F_d &= \frac{m^2 v_0^2}{(m+M) \mu \delta_y} + 2(m+M) g - (k_1+k_2) \left(\frac{1}{\mu} - 1\right) \delta_y \\ &= \frac{m^2 v_0^2}{(m+M) \mu \delta_y} + 2(m+M) g - \left(1 - \frac{k_2}{k_1}\right) \left(\frac{1}{\mu} - 1\right) k_1 \delta_y \end{aligned}$$

$$F_d = \frac{m^2 v_0^2}{(m+M) \mu \delta_y} + 2(m+M) g - \left(1 - \xi\right) \left(\frac{1}{\mu} - 1\right) k_1 \delta_y \quad \text{II.3.4}$$

IF THIS IS PERFECT PLASTIC, THEN $F_d = F_y = k_1 \delta_y$ $\xi = 0$

$$F_d = \frac{m^2 v_0^2}{(m+M) \mu \delta_y} + 2(m+M) g + \left(\frac{1}{\mu} - 1\right) F_d$$

$$F_d = \frac{2mg\delta_c}{\delta_d} + 2(m+M) g + \left(\frac{\delta_y}{\delta_d} - 1\right) F_d$$

$$= \frac{2mg\delta_c}{\delta_d} + 2(m+M) g + \frac{(\delta_y - \delta_d)}{\delta_d} F_d$$

$$F_d \delta_d = 2mg\delta_c + 2(m+M) g \delta_d + (\delta_y - \delta_d) F_d$$

$$F_d \delta_d = mg\delta_c + (m+M) g \delta_d + \frac{F_d \delta_y}{2}$$

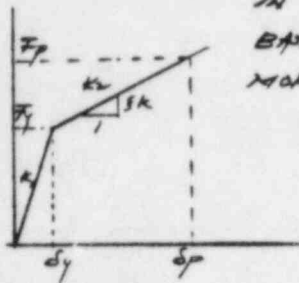
$$F_d \delta_d - \frac{F_d \delta_y}{2} = mg\delta_c + (m+M) g \delta_d \quad \text{--- II.3.5}$$

IS THE SAME AS EQ. (10) WHICH IS FOR PERFECT PLASTIC CASE,
AND THE IMPACT IS PROVED TO BE :

$$F_H = \frac{27mU_0}{T} \sqrt{\frac{1}{3A-1}}$$

III CORRELATION BETWEEN EQUIVALENT ELASTIC IMPACT LOAD AND PLASTIC LOAD

RELATIONSHIP BETWEEN EQUIVALENT ELASTIC IMPACT LOAD F_e OR DEMAND D , AND PERFECT PLASTIC IMPACT LOAD F_p OR CAPACITY C IN TERMS OF BILINEAR SLOPE RATIO, ξ , AND DUCTILITY RATIO, μ . IS BASED ON EQUATING THE ENERGY ABSORBED BY THE ELASTIC CAPACITY MODEL WITH AN ASSUMED EQUIVALENT PERFECT ELASTIC DEMAND MODEL



FOR ELASTIC DEMAND MODEL

$$E_e = \frac{1}{2} \frac{F_p^2}{k_1} \quad \text{--- (2.1)}$$

FOR THE INELASTIC CAPACITY MODEL

$$F_p = \frac{1}{2} \frac{F_p^2}{k_1} + \left(\frac{F_p - F_y}{2} \right) (\delta_p - \delta_y) \quad \text{--- (2.2)}$$

$$\begin{aligned} \text{BUT } F_p &= k_1 \delta_y + k_2 (\delta_p - \delta_y) \\ &= k_1 \delta_y + \xi k_1 (\delta_p - \delta_y) = k_1 \delta_y + \xi k_1 \delta_y (\mu - 1) \\ &= k_1 \delta_y [1 + \xi (\mu - 1)] \end{aligned}$$

$$F_p = k_1 \delta_y [1 + \xi (\mu - 1)] \quad \text{--- (2.3)}$$

$$\begin{aligned} E_p &= \frac{1}{2} k_1 \delta_y^2 + \frac{1}{2} (F_p + F_y) (\delta_p - \delta_y) \\ &= \frac{1}{2} k_1 \delta_y^2 + \frac{1}{2} (F_p + F_y) (\mu - 1) \delta_y \\ &= \frac{1}{2} k_1 \delta_y^2 + \frac{1}{2} [k_1 \delta_y + k_1 \delta_y + k_1 \delta_y \xi (\mu - 1)] (\mu - 1) \delta_y \\ &= \frac{1}{2} k_1 \delta_y^2 + \frac{1}{2} \{ k_1 \delta_y [2 + \xi (\mu - 1)] \} (\mu - 1) \delta_y \\ &= \frac{1}{2} k_1 \delta_y^2 + \frac{1}{2} \{ k_1 \delta_y [2(\mu - 1) + \xi (\mu - 1)^2] \} \delta_y \\ E_p &= \frac{1}{2} k_1 \delta_y^2 [1 + 2(\mu - 1) + \xi (\mu - 1)^2] \\ F_p &= \frac{1}{2} \frac{F_p^2}{k_1} [1 + 2(\mu - 1) + \xi (\mu - 1)^2] \quad \text{--- (2.4)} \end{aligned}$$

EQUATE (2.1) TO (2.4), YIELDS

$$\frac{1}{2} \frac{F_e^2}{k_1} = \frac{1}{2} \frac{F_p^2}{k_1} [1 + 2(\mu - 1) + \xi (\mu - 1)^2] \quad \text{--- (2.5)}$$

$$\left(\frac{F_e}{F_p} \right)^2 = 1 + 2(\mu - 1) + \xi (\mu - 1)^2$$

$$\begin{aligned} \left(\frac{T_c}{T_f}\right)^2 &= 1 + 2(\mu - 1) + \xi(\mu^2 - 2\mu + 1) \\ &= 1 + 2\mu - 2 + \mu\xi - 2\mu\xi + \xi = \xi\mu^2 + 2(1-\xi)\mu + (\xi - 1) \end{aligned}$$

$$\Rightarrow \xi\mu^2 + 2(1-\xi)\mu + (\xi - 1) - \left(\frac{T_c}{T_f}\right)^2 = 0$$

$$\mu^2 + 2\left(\frac{1}{\xi} - 1\right)\mu + \left(1 - \frac{1}{\xi}\right) - \left(\frac{T_c}{T_f}\right)^2 \frac{1}{\xi} = 0$$

$$\begin{aligned} \Rightarrow \mu &= \left(1 - \frac{1}{\xi}\right) + \sqrt{\left(\frac{1}{\xi} - 1\right)^2 - \xi\left(1 - \frac{1}{\xi}\right) - \left(\frac{T_c}{T_f}\right)^2 \frac{1}{\xi}} \\ &= \left(1 - \frac{1}{\xi}\right) + \sqrt{2\left(1 - \frac{1}{\xi} - 1\right)\left(1 - \frac{1}{\xi}\right) + \left(\frac{T_c}{T_f}\right)^2 \frac{1}{\xi}} \\ &= \left(1 - \frac{1}{\xi}\right) + \sqrt{\left(\frac{1}{\xi} - 1\right)\frac{1}{\xi} + \left(\frac{T_c}{T_f}\right)^2 \frac{1}{\xi}} \end{aligned}$$

$$\boxed{\mu = \left(1 - \frac{1}{\xi}\right) + \left[\frac{1}{\xi}\left(\frac{1}{\xi} - 1 + \left(\frac{T_c}{T_f}\right)^2\right)\right]^{\frac{1}{2}}} \quad \text{--- (III.6)}$$

$$\text{SINCE } \frac{T_c}{T_f} = \frac{D}{C}$$

$$\boxed{\mu = \left(1 - \frac{1}{\xi}\right) + \left[\frac{1}{\xi}\left(\frac{1}{\xi} - 1 + \left(\frac{D}{C}\right)^2\right)\right]^{\frac{1}{2}}} \quad \text{--- (III.7)}$$

FOR ELASTIC PLASTIC CASE $\xi = 0$

$$\boxed{\mu = \frac{1}{2} \left[\left(\frac{D}{C}\right)^2 + 1 \right]} \quad \text{--- (III.8)}$$

$$\text{OR } \left(\frac{D}{C}\right) = \sqrt{2\mu - 1}$$

$$\boxed{C = \frac{D}{\sqrt{2\mu - 1}}} \quad \text{--- (III.9)}$$



ENGINEERING MECHANICS DIVISION
COMPUTER LOG

CALCULATION NUMBER: SD E - 2043 - 2S&M COMP. AUTH: XXTCHARGE NUMBER: ***OTHER:

PROGRAM NAME	LIBRARY REF. NUMBER	VERSION / LEVEL	RUN NO.	JOB SUBMITTAL NUMBER	FICHE LOC		PREPARED BY:		COMPUTER USED **	COMMENTS OTHER PERTINENT INFORMATION
					SECT	PAGE	NAME	DATE		
ANSYS	ST-348	04/10	1	6270		737	J. GUINNY	6/24/85	3xW	PRE-IMPACT
:	:	:	2	910		737	:	6/24/85	:	AFTER IMPACT
:	:	:	3	2009		:	:	6/26/85	:	SEAT-32 FACE DEFLECTION CURVE
:	:	:	4	3297		:	:	6/26/85	:	SEAT-10
:	:	:	5	1839		:	:	6/30/85	:	DATA 329
:	:	:	6	1486		:	:	7/3/85	:	DISK 10

* = COMPUTER GENERATED JOB SUBMITTAL NUMBER
 ** = COMPUTER USED (SHEC OR OTHER)
 *** = IF OTHER THAN SHEC COMPUTER IS USED

CALCULATION SHEET

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FOR DETAILS PLEASE SEE
THE ATTACHED HARD COPIES OF
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J.O./W.O. NO. 12210	DIVISION & GROUP NM(C)	CALCULATION NO. 2043	OPTIONAL CODE SQE	

APPENDIX

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B-Plastic Strain Energy for a Circular Plate in Bending

C-Miscellaneous Reference Material

D-Computer Log and Microfiche

E-VOID PAGES (30 PAGES)

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12210 EMD/EGS SQ E 2043 APPENDIX F F 40

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SUMMARY

VOID

The reactor vessel water is protected from blowdown following a postulated rupture of the feedwater piping outside the containment, by the Velan check valve 1B21*F010A,B inside the containment and by the Atwood/Morrill testable check valve 1B21*AOVF32A,B outside the containment. Breaks are not postulated in the piping between the valves because that region is classified as break exclusion.

The reverse flow caused by the sudden pressure reduction at the break rapidly closes both valves. This calculation examines the ability of these valves to withstand the impact of the disk on the seat without excessive leakage thereafter.

Loads on the critical elements, i.e., the disk, tail link, rockshaft and seat, were computed by simulation of the impact dynamics using the STARDYNE and GTSTRUDL computer programs. Both uniform and point impact were considered in search of the worst case kickback loads generated by the point of impact not being at the center of percussion. Seismic, hydrodynamic and dead loads were not considered because of their insignificant magnitude compared to impact loads.

In those cases where linear stresses were not below their allowables, a strain energy analysis or a non-linear time history strain analysis was conducted to demonstrate integrity.

It is concluded that both valves will remain intact and that the leakage will be within the make-up capability of the High Pressure Core Spray (HPCS) or the Reactor Core Injection Cooling (RCIC) system, following rupture of the feedwater piping outside the containment.