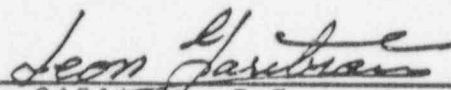


April 12, 1981

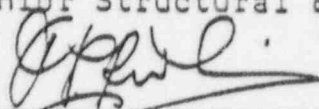
OYSTER CREEK NUCLEAR STATION
REEVALUATION OF SAFETY-RELATED
CONCRETE MASONRY WALLS
NRC IE BULLETIN 80-11

JERSEY CENTRAL POWER & LIGHT COMPANY
Subsidiary of General Public Utilities Nuclear Corp.

Prepared By:


L. GARIBIAN, P.E.
Senior Structural Engineer

Approved By:


A. P. ROCHINO
Manager, Engineering Mechanics

8106050 064

TABLE OF CONTENTS

<u>Section</u>	<u>Title</u>	<u>Page</u>
1.0	Purpose and Summary	1
2.0	Methods	1
3.0	Results	2
4.0	Conclusions	3
5.0	Recommendations	3
6.0	References	4
7.0	Appendices	

.1 JCP&L/GPU letter to NRC, dated
September 19, 1980

.2 JCP&L/GPU letter to NRC, dated
November 14, 1980

.3 Summary of Results

.4 Reinforcing Details

.5 Boundary Support Requirements

.6 Schedule for Implementation of
Design Modifications

.7 Floor Response Spectra

1.0 PURPOSE AND SUMMARY

The purpose of the reevaluation is to determine the structural adequacy of the concrete masonry walls as required by the NRC IE Bulletin 80-11.

The reevaluation shall determine whether the walls will perform their intended function under all postulated loads and load combinations specified in the "Criteria for the Reevaluation of Concrete Masonry Walls" in Appendix 7.2, which is consistent with the requirements outlined in item 2b of the Bulletin.

2.0 METHODS

The twenty-seven walls, reported in Appendix 7.2, that were affected by the Bulletin have been analyzed for all postulated loads and load combinations specified in Section 3 of the Reevaluation Criteria in Appendix 7.2. The procedure used in this analysis is described in Section 6 of the Reevaluation Criteria. The Criteria was developed in accordance with the rules and guidelines outlined in the NRC IE Bulletin 80-11.

The analysis was performed using the ANSYS Computer Program to determine the frequency and resultant stresses in the block walls. None of the walls were intended to resist impact or pressurization load, nor would they be subjected to a significant thermal load to be of any concern.

In the analysis, all support edges of the block walls were assumed to be simple. In reviewing the wall support details shown in the construction drawings it was determined that some supports are inadequate to transfer the Seismic Shear Load to the main structure. In order to be consistent with the analysis, the Wall Support edges, shown in Appendix 7.5, will have to be reinforced to be able to carry the Seismic Shear Load.

3.0 RESULTS

The results of the block wall analysis are summarized in Appendix 7.3 as listed below:

- .1 Stresses. See pages 7.3-1 to 7.3-43
- .2 Out-of-plane shear. See page 7.3-44
- .3 In-plane strain. See pages 7.3-45 & 7.3-46
- .4 Results of Operability Analysis. See page 7.3-47

4.0 CONCLUSIONS

The results of the concrete block wall analysis indicate that all twenty-seven walls are qualified by satisfying either the stress or operability acceptance criteria.

The following walls were qualified by operability. Wall Nos. 8, 15, 19, 20, 21, 22, 23, 24 and 29. In the analysis, it was assumed that:

- .1 The support edges of all walls are capable of transferring the Seismic Shear Load to the main structure.
- .2 The excess equipment loads on the block walls will be either removed or transferred to another support point, other than the block wall, so that, the wall can be qualified.
- .3 Both surfaces of the block walls have no visible cracks.

5.0 RECOMMENDATIONS

The modifications listed below are to be implemented in order for the block walls to be consistent with the assumptions in the analysis.

- .1 Reinforce the support edges of the walls shown in the "Wall Boundary Support Requirements" table in Appendix 7.5.
- .2 Remove the excess equipment load from the wall Nos. 21, 33 and 43 shown in Appendix 7.5.
- .3 Repair all visible cracks on both sides of the concrete block walls.

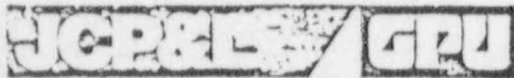
6.0 REFERENCES

- .1 ACI 531-79 "Building Code Requirements for Concrete Masonry Structures "
- .2 Burns & Roe Inc., Technical Specification No. 45, Section 4A.7 (see Appendix 7.2 ~~Enclosure~~).
- .3 ASTM-C90-75 "Hollow Load-Bearing Concrete Masonry Units "
- .4 Burns & Roe Inc., drawing No. 4514-3 " Misc. Plans Sections and Details (Masonry) "

SECTION 7.0

APPENDICES

Appendix 7.1
JCP&L/GPU letter to NRC
dated Sept. 19, 1980



Jersey Central Power & Light Company
Madison Avenue at Punch Bowl Road
Morristown, New Jersey 07960
(201) 455-8200

September 19, 1980

Mr. Boyce H. Grier, Director
Office of Inspection and Enforcement
Region I
U.S. Nuclear Regulatory Commission
651 Park Avenue
King of Prussia, Pennsylvania 19406

Dear Mr. Grier:

Subject: Oyster Creek Nuclear Generating Station
Docket No. 50-219
I.E. Bulletin 80-11

Reference: Ivan R. Finfrock, Jr. letter to Boyce H. Grier
dated July 7, 1980

The referenced letter provided information required in Items 1, 2a and 3 of the subject bulletin. Attachment No. 1 has been revised per the recommendations of the NRC during a site inspection on August 6 and 7, 1980. Please replace Attachment 1 in its entirety with Attachment 1, Rev. 1 enclosed.

JERSEY CENTRAL POWER & LIGHT COMPANY

By Ivan R. Finfrock, Jr.
Ivan R. Finfrock, Jr.
Vice President

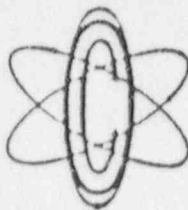
Sworn and subscribed to before me this 19th day of September, 1980.

David C. Shadle
Notary Public
DAVID C. SHADLE

cc: Director
Office of Inspection and Enforcement
Division of Reactor Operations Inspection
Washington, D.C. 20555
NOTARY PUBLIC OF NEW JERSEY
My Commission Expires Oct. 3, 1983

Appendix 7.2
JCP&L/GPU letter to NRC
dated Nov. 14, 1980

OYSTER CREEK



NUCLEAR GENERATING STATION



Jersey Central Power & Light
Company is a Member of the
General Public Utilities System

(609) 693-1951 P.O. BOX 388 • FORKED RIVER • NEW JERSEY • 08731

November 14, 1980

Mr. Boyce H. Grier, Director
Office of Inspection and Enforcement
Region I
U. S. Nuclear Regulatory Commission
631 Park Avenue
King of Prussia, Pennsylvania 19406

Dear Mr. Grier:

Subject: Oyster Creek Nuclear Generating Station
Docket No. 50-219
I. E. Bulletin 80-11

Our letters of July 7, 1980 and September 19, 1980 supplied the preliminary information as required by I. E. Bulletin 80-11. In accordance with the request for information described in item 2b of the bulletin, the attached represents the majority of our response. The final reevaluation of the design adequacy of all identified walls to perform their intended function under all postulated loads and load combination is however, not yet complete.

The delay in meeting our original schedule, enclosed in our July 7, 1980 submittal, is attributed primarily to difficulties encountered in the development of the reevaluation criteria. The final reevaluation for all walls will be submitted on or before May 1, 1981. If additional information or further clarification is needed, please contact Mr. J. Knubel of my staff at 201-455-8753.

Very truly yours,

Ivan R. Finfrock, Jr.
Ivan R. Finfrock, Jr.
Vice President

Sworn and subscribed to before me this 14th day of November, 1980.

David C. Hodge
Notary Public

cc: NRC Office of Inspection and Enforcement
Division of Reactor Operations Inspection
Washington, D.C. 20555.

IRF/lcp
1922

Appendix 7.3

Summary of Results

GENERAL NOTES

Type of Construction	<p>1. All blocks are ASTM-C-90, Hollow Block</p> <p>2. Walls that are reinforced have vertical rebar and horizontal dur-o-wall as shown below. For reinforcing details see App. 7.4.</p> <p>Vertical: Rebar $F_y = 40,000$ psi</p> <p>Horizontal: Dur-O-Wall $F_y = 70,000$ psi (ASTM-A-82)</p>				
Frequency Range - Hz	<p>1. All boundary conditions are Simple-Supports.</p> <p>2. For 3-Edge support, the boundary condition that has not counted for has been pointed out.</p>				
Flexural Tensile Stresses - psi	ALLOWABLES AS GIVEN IN THE CRITERIA				
1. Uncracked Section	Load Combination	Normal to Bed Jt		Parallel to Bed Joint	
		Running Bond	Stack Bond	Running Bond	Stack Bond
a. Hollow Block	OBE+DL	25	25	50	16.7
	SSE+DL	41.5	41.5	83	27.7
b. Hollow Blk - Fully Grouted	OBE+DL	50	40	75	26.7
	SSE+DL	83.5	67	125	44.5
2. Cracked Section		Vertical Rebar		Horizontal Dur-o- wall	
a. Steel	OBE-DL	20,000		30,000	
	SSE+DL	36,000		63,000	
b. Concrete Compressive Stresses-psi	OBE+DL	396			
	SSE+DL	1020			

SUMMARY OF RESULTS

Location	Turbine Building, Control Room South Wall, West Half Floor Elev. 46'-6"	
Dimensions of Model	Height: <u>14</u> Ft <u>6</u> In. Thickness: <u>8</u> In. Width: <u>11</u> Ft <u>4</u> In.	
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond (X) Reinforced () Other _____ () Unreinforced	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>8.31</u> To <u>10.17</u> () Horizontal Span (X) Two Way <u>22.35</u> To <u>27.37</u> (X) 4-Edge Support () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.50</u> g, (OBE) <u>0.75</u> g, (SSE)	(X) Two Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, $\frac{187.9}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{270.3}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ () Parallel to Bed Joint, _____ vs _____ _____ vs _____ (X) Two Way: (X) Normal to Bed Joint, $\frac{15.4}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{21.5}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (X) Parallel to Bed Joint, $\frac{24.3}{(OBE+DL)}$ vs $\frac{16.7}{(Allowable)}$ $\frac{38.6}{(SSE+DL)}$ vs $\frac{27.7}{(Allowable)}$	
Remarks	Continue for cracked section model analysis	Wall No. <u>2-1</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	<div style="display: flex; justify-content: space-between;"> <div> (x) One Way (x) Vertical Span () Horizontal Span </div> <div style="text-align: right;"> <u>4.18</u> To <u>5.12</u> </div> </div> <hr/> <div> () Two Way () 4-Edge Support () 3-Edge Support () Top, () Side, Missing </div>	
Response Acceleration (Cracked Section)	(X) One Way: $\frac{0.30}{(OBE)}$ g, $\frac{0.45}{(SSE)}$ g,	() Two Way: $\frac{\quad}{(OBE)}$ g, $\frac{\quad}{(SSE)}$ g,
Flexural Tensile Stresses - psi (Cracked Section)	<div> (X) One Way: (X) Normal, () parallel, To Bed Joint </div> <div style="margin-top: 10px;"> $f_{(Steel)}: \frac{13,238}{(OBE+DL)} \text{ vs } \frac{20,000}{(Allowable)}$ $\frac{19,587}{(SSE+DL)} \text{ vs } \frac{36,000}{(Allowable)}$ $f_{(Conc.):} \frac{-370.0}{(OBE+DL)} \text{ vs } \frac{-396.0}{(Allowable)}$ (Compression) $\frac{-547.0}{(SSE+DL)} \text{ vs } \frac{-1,020.0}{(Allowable)}$ </div> <hr/> <div> () Two Way: () Normal To Bed Joint </div> <div style="margin-top: 10px;"> $f_{(Steel)}: \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $f_{(Conc.):} \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div> <div style="margin-top: 10px;"> () Parallel To Bed Joint </div> <div style="margin-top: 10px;"> $f_{(Steel)}: \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div>	
Remarks	Wall is good for one way model cracked section	Wall No. <u>2-1</u>

SUMMARY OF RESULTS

Location	Turbine Building, Control Room South Wall West Half Floor Elev. 46'-6"	
Dimensions of Model	Height: <u>14</u> Ft <u>6</u> In. Width: <u>8</u> Ft <u>5</u> In.	Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced	() Running Bond (X) Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>6.23</u> To <u>7.63</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>22.55</u> To <u>27.62</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.38</u> g, (OBE) <u>0.56</u> g, (SSE)	(X) Two Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>174.2</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>247.7</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>27.3</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>34.4</u> vs <u>41.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>34.5</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>48.4</u> vs <u>27.7</u> (SSE+DL) (Allowable)	
Remarks	Continue for cracked section model analysis	Wall No. <u>2-2</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	(X) One Way (X) Vertical Span <u>3.39</u> To <u>4.15</u> () Horizontal Span	
	(X) Two Way (X) 4-Edge Support <u>12.27</u> To <u>15.03</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Cracked Section)	(X) One Way: $\frac{0.30}{(OBE)}$ g, $\frac{0.45}{(SSE)}$ g,	(X) Two Way: $\frac{0.99}{(OBE)}$ g, $\frac{1.33}{(SSE)}$ g,
Flexural Tensile Stresses - psi (Cracked Section)	(X) One Way: (X) Normal, () parallel, To Bed Joint $f_{(Steel)}: \frac{10,450}{(OBE+DL)} \text{ vs } \frac{20,000}{(Allowable)}$ $\frac{15,580}{(SSE+DL)} \text{ vs } \frac{36,000}{(Allowable)}$ $f_{(Conc.)}: \frac{-441.0}{(OBE+DL)} \text{ vs } \frac{-396.0}{(Allowable)}$ $(Compression) \frac{-658.0}{(SSE+DL)} \text{ vs } \frac{-1020.0}{(Allowable)}$	
	(X) Two Way: (X) Normal To Bed Joint $f_{(Steel)}: \frac{3,628}{(OBE+DL)} \text{ vs } \frac{20,000}{(Allowable)}$ $\frac{4,880}{(SSE+DL)} \text{ vs } \frac{36,000}{(Allowable)}$ $f_{(Conc.)}: \frac{-154.0}{(OBE+DL)} \text{ vs } \frac{-396.0}{(Allowable)}$ $(Compression) \frac{-206.4}{(SSE+DL)} \text{ vs } \frac{-1020.0}{(Allowable)}$ (X) Parallel To Bed Joint $f_{(Steel)}: \frac{26,108}{(OBE+DL)} \text{ vs } \frac{30,000}{(Allowable)}$ $\frac{35,080}{(SSE+DL)} \text{ vs } \frac{63,000}{(Allowable)}$	
Remarks	Model 2 of Wall #2 is good for Two-Way Model - Cracked Section	Wall No. <u>2-2</u>

SUMMARY OF RESULTS

Location	Turbine Building, Control Room, South Wall, East Half Floor Elev. 46'-6"	
Dimensions of Model	Height: <u>14</u> Ft <u>6</u> In. Width: <u>8</u> Ft <u>7</u> In.	Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced	() Running Bond (X) Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>6.56</u> To <u>8.04</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>23.85</u> To <u>29.20</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.38</u> g, (OBE) <u>0.56</u> g, (SSE)	(X) Two Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, $\frac{133.5}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{191.5}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ () Parallel to Bed Joint, $\frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ (X) Two Way: (X) Normal to Bed Joint, $\frac{24.2}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{31.1}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (X) Parallel to Bed Joint, $\frac{29.7}{(OBE+DL)}$ vs $\frac{16.7}{(Allowable)}$ $\frac{50.0}{(SSE+DL)}$ vs $\frac{27.7}{(Allowable)}$	
Remarks	Continue for cracked section model analysis	Wall No. <u>2-3</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	(X) One Way (X) Vertical Span <u>3.56</u> To <u>4.37</u> () Horizontal Span () Two Way () 4-Edge Support _____ To _____ () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Cracked Section)	(X) One Way: <u>0.30</u> g, (OBE) <u>0.45</u> g, (SSE)	() Two Way: _____ g, (OBE) _____ g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	(X) One Way: (X) Normal, () parallel, To Bed Joint <div style="text-align: right;"> $f_{(Steel)}:$ $\frac{8.106}{(OBE+DL)}$ vs $\frac{20,000}{(Allowable)}$ $\frac{12,140}{(SSE+DL)}$ vs $\frac{36,000}{(Allowable)}$ $f_{(Conc.)}:$ $\frac{-342.0}{(OBE+DL)}$ vs $\frac{-396.0}{(Allowable)}$ (Compression) $\frac{-513.0}{(SSE+DL)}$ vs $\frac{-1,020.0}{(Allowable)}$ </div> () Two Way: () Normal To Bed Joint <div style="text-align: right;"> $f_{(Steel)}:$ _____ vs _____ (OBE+DL) (Allowable) _____ (SSE+DL) (Allowable) $f_{(Conc.)}:$ _____ vs _____ (OBE+DL) (Allowable) _____ (SSE+DL) (Allowable) </div> () Parallel To Bed Joint <div style="text-align: right;"> $f_{(Steel)}:$ _____ vs _____ (OBE+DL) (Allowable) _____ (SSE+DL) (Allowable) </div>	
Remarks	Wall is good for one way model * cracked section	Wall No. <u>2-3</u>

SUMMARY OF RESULTS

Location	Turbine Building, Control Room, South Wall, East Half Floor Elev. 46'-6"		
Dimensions of Model	Height: <u>14</u> Ft <u>6</u> In. Width: <u>14</u> Ft <u>6</u> In.	Thickness: <u>6</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced	() Running Bond (X) Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(X) One Way <u>6.43</u> To <u>7.88</u> (X) Vertical Span () Horizontal Span (X) Two Way <u>12.88</u> To <u>15.78</u> (X) 4-Edge Support () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.38</u> g, (OBE) <u>0.56</u> g, (SSE)	(X) Two Way: <u>1.52</u> g, (OBE) <u>1.99</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, $\frac{138.0}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{197.4}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ () Parallel to Bed Joint, $\frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ (X) Two Way: (X) Normal to Bed Joint, $\frac{119.4}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{154.3}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (X) Parallel to Bed Joint, $\frac{176.5}{(OBE+DL)}$ vs $\frac{16.7}{(Allowable)}$ $\frac{232.5}{(SSE+DL)}$ vs $\frac{27.7}{(Allowable)}$		
Remarks	Continue for cracked section Model Analysis		Wall No. <u>2-4</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	(X) One Way (X) Vertical Span <u>2.65</u> To <u>3.25</u> () Horizontal Span	
	() Two Way () 4-Edge Support _____ To _____ () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Cracked Section)	(X) One Way: <u>0.28</u> g, (OBE) <u>0.45</u> g, (SSE)	() Two Way: _____ g, (OBE) _____ g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	(X) One Way: (X) Normal, () parallel, To Bed Joint <div style="text-align: right;"> $f_{(Steel)} : \frac{7.841}{(OBE+DL)}$ vs $\frac{20,000}{(Allowable)}$ $\frac{12,520}{(SSE+DL)}$ vs $\frac{36,000}{(Allowable)}$ $f_{(Conc.)} : \frac{-331.0}{(OBE+DL)}$ vs $\frac{-396.0}{(Allowable)}$ (Compression) $\frac{-530.0}{(SSE+DL)}$ vs $\frac{-1020.0}{(Allowable)}$ </div> () Two Way: () Normal To Bed Joint <div style="text-align: right;"> $f_{(Steel)} : \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $f_{(Conc.)} : \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> () Parallel To Bed Joint <div style="text-align: right;"> $f_{(Steel)} : \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div>	
Remarks	Model 4 of Wall No. 2 is good for one wall model cracked section	Wall No. <u>2-4</u>

SUMMARY OF RESULTS

Location	Turbine Building, Observation Room Enclosure, South Wall. Floor Elev. 49'-8"	
Dimensions of Model	Height: <u>10</u> Ft <u>10</u> In. Thickness: <u>8</u> In. Width: <u>14</u> Ft <u>11</u> In.	
Type of Construction	Block: ASTM - C-90 (X) Running Bond Mortar: Type "M" () Stacked Bond (X) Reinforced (X) Other: Fully grouted block () Unreinforced	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>13.51</u> To <u>16.55</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>20.18</u> To <u>24.72</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>1.52</u> g, (OBE) <u>1.99</u> g, (SSE)	(X) Two Way: <u>0.40</u> g, (OBE) <u>0.76</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>229.3</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>299.0</u> vs <u>83.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, vs (Allowable) (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>34.5</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>59.4</u> vs <u>83.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>18.4</u> vs <u>75.0</u> (OBE+DL) (Allowable) <u>34.3</u> vs <u>125.0</u> (SSE+DL) (Allowable)	
Remarks	Wall is good for two way model.	Wall No. <u>5</u>

SUMMARY OF RESULTS

Location	Turbine Building, Observation Room Enclosure, South-East to North-West Wall Floor Elev. 49'-8"	
Dimensions of Model	Height: <u>10</u> Ft <u>10</u> In. Thickness: <u>8</u> In. Width: <u>7</u> Ft <u>6</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced (X) Running Bond () Stacked Bond (X) Other: <u>Fully grouted block</u>	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>13.48</u> To <u>16.51</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>39.37</u> To <u>48.22</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>1.52</u> g, (OBE) <u>1.99</u> g, (SSE)	(X) Two Way: <u>0.18</u> g, (OBE) <u>0.35</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>241.4</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>314.0</u> vs <u>83.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, vs (OBE+DL) (Allowable) vs (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>14.4</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>16.6</u> vs <u>83.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>11.8</u> vs <u>75.0</u> (OBE+DL) (Allowable) <u>19.7</u> vs <u>125.0</u> (SSE+DL) (Allowable)	
Remarks	Wall is good for two way model Wall No. <u>6</u>	

SUMMARY OF RESULTS

Location	Turbine Building, Observation Room Enclosure West Wall Floor Elev 49'-8"		
Dimensions of Model	Height: <u>10</u> Ft <u>10</u> In. Width: <u>7</u> Ft <u>0</u> In.		Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced (X) Running Bond () Stacked Bond (X) Other: <u>Fully grouted block</u>		
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>13.98</u> To <u>17.12</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>45.30</u> To <u>55.48</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>1.52</u> g, (OBE) <u>1.99</u> g, (SSE)	(X) Two Way: <u>0.18</u> g, (OBE) <u>0.35</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>224.0</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>291.7</u> vs <u>83.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>10.0</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>11.7</u> vs <u>83.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>5.9</u> vs <u>75.0</u> (OBE+DL) (Allowable) <u>11.8</u> vs <u>125.0</u> (SSE+DL) (Allowable)		
Remarks	Wall is good for Two Way Model		Wall No. <u>7</u>

SUMMARY OF RESULTS

Location	Office Building, Cable Tray Area, East Wall, Intermediate Section Elev. 46'-6"	
Dimensions of Model	Height: <u>13</u> Ft <u>11</u> In. Thickness: <u>6</u> In. Width: <u>21</u> Ft <u>3</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced () Running Bond (X) Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>5.8</u> To <u>7.5</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>10.66</u> To <u>13.76</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.38</u> g, (OBE) <u>0.56</u> g, (SSE)	(X) Two Way: <u>1.1</u> g, (OBE) <u>1.5</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, $\frac{192.0}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{266.0}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ () Parallel to Bed Joint, $\frac{--}{(OBE+DL)}$ vs $\frac{--}{(Allowable)}$ $\frac{--}{(SSE+DL)}$ vs $\frac{--}{(Allowable)}$ (X) Two Way: (X) Normal to Bed Joint, $\frac{132.4}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{178.7}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (X) Parallel to Bed Joint, $\frac{122.2}{(OBE+DL)}$ vs $\frac{16.7}{(Allowable)}$ $\frac{165.2}{(SSE+DL)}$ vs $\frac{26.7}{(Allowable)}$	
Remarks	Wall is not good for both models. Wall is ok for operability (See pg. 7.3-47) <div style="float: right;">Wall No. <u>8</u></div>	

SUMMARY OF RESULTS

Location	Office Building, Monitor and Change Room, South Wall, Intermediate Section. Elev. 46'-6"	
Dimensions of Model	Height: <u>13</u> Ft <u>4</u> In. Thickness: <u>6</u> In. Width: <u>15</u> Ft <u>10</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced () Running Bond (X) Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>6.61</u> To <u>8.53</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>11.53</u> To <u>14.89</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.43</u> g, (OBE) <u>0.60</u> g, (SSE)	(X) Two Way: <u>1.52</u> g, (OBE) <u>1.99</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>162.5</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>223.3</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, -- vs -- (OBE+DL) (Allowable) -- vs -- (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>121.5</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>158.6</u> vs <u>41.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>103.0</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>134.8</u> vs <u>27.7</u> (SSE+DL) (Allowable)	
Remarks	Wall is not good for both models wall is O.K. for operability (See pg. 7.3-47) Wall No. <u>15</u>	

SUMMARY OF RESULTS

Location	Office Bldg. Battery Room South Wall, West Section Fl. El. 35'-0"	
Dimensions of Model	Height: <u>11</u> Ft <u>0</u> In. Width: <u>14</u> Ft <u>11</u> In.	Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	(X) Running Bond () Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>10.19</u> To <u>13.15</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>15.22</u> To <u>19.65</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.22</u> g, (OBE) <u>0.41</u> g, (SSE)	() Two Way: <u>0.34</u> g, (OBE) <u>0.50</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u>68.3</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>25.1</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>38.7</u> vs <u>41.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>19.1</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>27.0</u> vs <u>83.0</u> (SSE+DL) (Allowable)	
Remarks	O.K. for Two-Way Model	Wall No. <u>17</u>

SUMMARY OF RESULTS

Location	Office Bldg. Battery Room West Wall, South Section Fl. El. 35'-0"		
Dimensions of Model	Height: <u>11</u> Ft <u>0</u> In. Width: <u>13</u> Ft <u>5</u> In.		Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced (X) Running Bond () Stacked Bond () Other _____		
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>10.23</u> To <u>13.21</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>16.73</u> To <u>21.60</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.22</u> g, (OBE) <u>0.41</u> g, (SSE)	(X) Two Way: <u>0.33</u> g, (OBE) <u>0.60</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>65.2</u> vs <u>41.5</u> (OBE+DL) vs (Allowable) (SSE+DL) vs (Allowable) () Parallel to Bed Joint, <u>16.30</u> vs <u>50.0</u> (OBE+DL) vs (Allowable) (SSE+DL) vs (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>18.24</u> vs <u>25.0</u> (OBE+DL) vs (Allowable) <u>36.4</u> vs <u>41.5</u> (SSE+DL) vs (Allowable) (X) Parallel to Bed Joint, <u>16.30</u> vs <u>50.0</u> (OBE+DL) vs (Allowable) <u>28.6</u> vs <u>83.0</u> (SSE+DL) vs (Allowable)		
Remarks	O.K. for Two-Way Model		Wall No. <u>18</u>

SUMMARY OF RESULTS

Location	Office Bldg. Elect. Tray Room North Wall Fl. El. 35'-0"	
Dimensions of Model	Height: <u>11</u> Ft <u>0</u> In. Width: <u>10</u> Ft <u>6</u> In.	Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	() Running Bond (X) Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>10.25</u> To <u>13.23</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>20.79</u> To <u>26.84</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.22</u> g, (OBE) <u>0.41</u> g, (SSE)	(X) Two Way: <u>0.44</u> g, (OBE) <u>0.65</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u>67.7</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>17.4</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>26.2</u> vs <u>41.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>22.2</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>32.2</u> vs <u>27.7</u> (SSE+DL) (Allowable)	
Remarks	N. C. for Both Models O. K. for Operability (See pg. 7.3-47)	Wall No. <u>19</u>

SUMMARY OF RESULTS

Location	Office Bldg. Elect. Tray Room East Wall Fl. El. 35'-0"	
Dimensions of Model	Height: <u>11</u> Ft <u>0</u> In. Width: <u>16</u> Ft <u>2.5</u> In.	Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	() Running Bond (X) Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>9.33</u> To <u>12.05</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>14.34</u> To <u>18.51</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.24</u> g, (OBE) <u>0.45</u> g, (SSE)	(X) Two Way: <u>0.34</u> g, (OBE) <u>0.45</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> (X) One Way: (X) Normal to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u>59.9</u> vs <u>41.5</u> (SSE+DL) (Allowable) </div> <div> () Parallel to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable) </div> <div> (X) Two Way: (X) Normal to Bed Joint, <u>21.8</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>31.1</u> vs <u>41.5</u> (SSE+DL) (Allowable) </div> <div> (X) Parallel to Bed Joint, <u>19.2</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>25.4</u> vs <u>27.7</u> (SSE+DL) (Allowable) </div>	
Remarks	N. G. for Both Models O. K. for Operability (See pg. 7.3-47) Wall No. <u>20</u>	

SUMMARY OF RESULTS

Location	Office Bldg. Switchgear Room North Wall, West Section Fl. El. 23'-6"	
Dimensions of Model	Height: <u>10</u> Ft <u>2.5</u> In. Width: <u>23</u> Ft <u>0</u> In.	Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	(X) Running Bond () Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way () Vertical Span () Horizontal Span	<u>16.16</u> To <u>20.86</u>
	(X) Two Way () 4-Edge Support (X) 3-Edge Support () Top, (X) Side, Missing	<u>17.35</u> To <u>22.40</u>
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.35</u> g, (OBE) <u>0.50</u> g, (SSE)	(X) Two Way: <u>0.35</u> g, (OBE) <u>0.50</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, () Parallel to Bed Joint, (X) Two Way: (X) Normal to Bed Joint, () Parallel to Bed Joint, N.G. for Both Models O.K. for Operability (See pg. 7.3-47)	<u>84.2</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>115.0</u> vs <u>41.5</u> (SSE+DL) (Allowable) <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable) <u>N.A.</u> vs <u> </u> (OBE+DL) (Allowable) <u>114.0</u> vs <u>41.5</u> (SSE+DL) (Allowable) <u>N.A.</u> vs <u> </u> (OBE+DL) (Allowable) <u>N.A.</u> vs <u> </u> (SSE+DL) (Allowable)
Remarks	Wall No. <u>21</u>	

SUMMARY OF RESULTS

Location	Office Bldg. Switchgear Room Partition Wall, East Section Fl. El. 23'-6"	
Dimensions of Model	Height: <u>9</u> Ft <u>0</u> In. Thickness: <u>8</u> In. Width: <u>17</u> Ft <u>8.5</u> In.	
Type of Construction	Block: ASTM - C-90 (X) Running Bond Mortar: Type "M" () Stacked Bond () Reinforced () Other _____ (X) Unreinforced	
Frequency Range - Hz (Uncracked Section)	(X) One Way <u>21.61</u> To <u>27.90</u> (X) Vertical Span () Horizontal Span (X) Two Way <u>22.61</u> To <u>29.19</u> () 4-Edge Support (X) 3-Edge Support () Top, (X) Side, Missing	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.44</u> g, (OBE) <u>0.70</u> g, (SSE)	(X) Two Way: <u>0.44</u> g, (OBE) <u>0.70</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>26.0</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>43.5</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>25.7</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>43.1</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, <u>N.A.</u> vs _____ (OBE+DL) (Allowable) <u>N.A.</u> vs _____ (SSE+DL) (Allowable)	
Remarks	N.G. for Both Models O. K. for Operability (See pg. 7.3-47) Wall No. <u>22</u>	

SUMMARY OF RESULTS

Location	Office Bldg. Switchgear Room Partition Wall Fl. El. 23'-6"	
Dimensions of Model	Height: <u>11</u> Ft <u>0</u> In. Width: <u>23</u> Ft <u>0</u> In.	Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	(X) Running Bond () Stacked Bond () Other _____
Frequency Range - Hz (Uncracked Section)	(X) One Way <u>14.09</u> To <u>18.19</u> (X) Vertical Span () Horizontal Span (X) Two Way <u>14.85</u> To <u>19.17</u> () 4-Edge Support (X) 3-Edge Support () Top, (X) Side, Missing	
Response Acceleration (Uncracked Section)	(X) One-Way: <u>0.30</u> g, (OBE) <u>0.45</u> g, (SSE)	(X) Two Way: <u>0.35</u> g, (OBE) <u>0.50</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> (X) One Way: (X) Normal to Bed Joint, $\frac{\text{---}}{(\text{OBE}+\text{DL})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ <u>53.2</u> vs <u>41.5</u> $\frac{(\text{SSE}+\text{DL})}{(\text{Allowable})}$ () Parallel to Bed Joint, $\frac{\text{---}}{(\text{OBE}+\text{DL})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ $\frac{(\text{SSE}+\text{DL})}{(\text{Allowable})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ </div> <div> (X) Two Way: (X) Normal to Bed Joint, $\frac{\text{---}}{(\text{OBE}+\text{DL})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ <u>53.2</u> vs <u>41.5</u> $\frac{(\text{SSE}+\text{DL})}{(\text{Allowable})}$ () Parallel to Bed Joint, $\frac{\text{N.A.}}{(\text{OBE}+\text{DL})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ $\frac{\text{N.A.}}{(\text{SSE}+\text{DL})}$ vs $\frac{\text{---}}{(\text{Allowable})}$ </div>	
Remarks	N.G. for Both Models O. K. for Operability (See pg. 7.3-47) <div>Wall No. <u>23</u></div>	

SUMMARY OF RESULTS

Location	Turbine Building, North East Stairwell West Wall, Lower Part Floor Elev. 23'-6"		
Dimensions of Model	Height: <u>17</u> Ft <u>11</u> In. Width: <u>13</u> Ft <u>9</u> In.	Thickness: <u>8</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	(X) Running Bond () Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>4.78</u> To <u>6.17</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>12.72</u> To <u>16.42</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.37</u> g, (OBE) <u>0.55</u> g, (SSE)	(X) Two Way: <u>0.85</u> g, (OBE) <u>1.14</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, $\frac{220.0}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{309.0}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ () Parallel to Bed Joint, $\frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ (X) Two Way: (X) Normal to Bed Joint, $\frac{64.6}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{77.1}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (X) Parallel to Bed Joint, $\frac{72.0}{(OBE+DL)}$ vs $\frac{50.0}{(Allowable)}$ $\frac{105.1}{(SSE+DL)}$ vs $\frac{83.0}{(Allowable)}$		
Remarks	Wall is not good for both models Wall is O.K. for operability (See pg. 7.3-47)		Wall No. <u>24-1</u>

SUMMARY OF RESULTS

Location	Turbine Building, North East Stairwell West Wall, Upper Part. Floor Elev. 23'-6"		
Dimensions of Model	Height: <u>8</u> Ft <u>6</u> In. Width: <u>13</u> Ft <u>9</u> In.	Thickness: <u>8</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced	(X) Running Bond () Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(x) One Way <u>23.03</u> To <u>29.73</u> (x) Vertical Span () Horizontal Span () Two Way _____ To _____ () 4-Edge Support () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.17</u> g, (OBE) <u>0.32</u> g, (SSE)	() Two Way: _____ g, (OBE) _____ g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(x) One Way: (x) Normal to Bed Joint, $\frac{25.0}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ <u>35.7</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, $\frac{--}{(OBE+DL)}$ vs $\frac{--}{(Allowable)}$ <u>--</u> vs <u>--</u> (SSE+DL) (Allowable) () Two Way: () Normal to Bed Joint, $\frac{--}{(OBE+DL)}$ vs $\frac{--}{(Allowable)}$ <u>--</u> vs <u>--</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, $\frac{--}{(OBE+DL)}$ vs $\frac{--}{(Allowable)}$ <u>--</u> vs <u>--</u> (SSE+DL) (Allowable)		
Remarks	Wall is good for one way model		Wall No. <u>24-2</u>

SUMMARY OF RESULTS

Location	Turbine Building, Cable Spread Room, West Wall, South Section Floor Elev. 36'-0"	
Dimensions of Model	Height: <u>9</u> Ft <u>0</u> In. Width: <u>9</u> Ft <u>3</u> In.	Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (x) Unreinforced	(X) Running Bond () Stacked Bond (X) Other: <u>Wall reinforced with unistrut on both sides thru bolts</u>
Frequency Range - Hz (Uncracked Section)	(x) One Way <u>26.16</u> To <u>33.77</u> (x) Vertical Span () Horizontal Span (x) Two Way <u>49.76</u> To <u>64.24</u> (x) 4-Edge Support () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(x) One Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)	(x) Two Way: <u>0.20</u> g, (OBE) <u>0.35</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	(x) One Way: (x) Normal to Bed Joint, <u>25.6</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>36.0</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable)	
Stress on concrete block	(X) Two Way: (x) Normal to Bed Joint, <u>9.7</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>11.9</u> vs <u>41.5</u> (SSE+DL) (Allowable) (x) Parallel to Bed Joint, <u>5.2</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>7.9</u> vs <u>81.0</u> (SSE+DL) (Allowable)	
Remarks	Wall is good for Two Way Model.	Wall No. <u>25</u>

SUMMARY OF RESULTS

Location	Turbine Building Cable Spread Room, West Section of North Wall. Floor Elev. 36'-0"		
Dimensions of Model	Height: <u>9</u> Ft <u>0</u> In. Thickness: <u>8</u> In. Width: <u>30</u> Ft <u>8</u> In.		
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced (X) Running Bond () Staked Bond (X) Other <u>Wall reinforced with unistrut in both sides with thru bolts</u>		
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>26.09</u> To <u>33.68</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>28.02</u> To <u>36.18</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE) (X) Two Way: <u>0.25</u> g, (OBE) <u>0.50</u> g, (SSE)		
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>38.1</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>51.4</u> vs <u>41.5</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, <u> </u> vs <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> (SSE+DL) (Allowable)		
Stress on concrete block	(X) Two Way: (X) Normal to Bed Joint, <u>24.1</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>35.5</u> vs <u>41.5</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>3.8</u> vs <u>50.0</u> (OBE+DL) (Allowable) <u>11.6</u> vs <u>83.9</u> (SSE+DL) (Allowable)		
Remarks	Wall is good for two way model		Wall No. <u>26</u>

SUMMARY OF RESULTS

Location	Turbine Building, Cable Spread Room North-South Wall on Column Line H. Floor Elev. 36'-0"		
Dimensions of Model	Height: <u>9</u> Ft <u>0</u> In. Width: <u>3</u> Ft <u>4</u> In.		Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" <input type="checkbox"/> Reinforced <input checked="" type="checkbox"/> Unreinforced <input checked="" type="checkbox"/> Running Bond <input type="checkbox"/> Stacked Bond <input checked="" type="checkbox"/> Other: <u>Wall reinforced with unistrut on both sides with thru bolts</u>		
Frequency Range - Hz (Uncracked Section)	<input checked="" type="checkbox"/> One Way <input checked="" type="checkbox"/> Vertical Span <u>26.29</u> To <u>33.94</u> <input type="checkbox"/> Horizontal Span <input type="checkbox"/> Two Way <input type="checkbox"/> 4-Edge Support _____ To _____ <input type="checkbox"/> 3-Edge Support _____ <input type="checkbox"/> Top, <input type="checkbox"/> Side, Missing		
Response Acceleration (Uncracked Section)	<input checked="" type="checkbox"/> One Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)		<input type="checkbox"/> Two Way: _____ g, (OBE) _____ g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<input checked="" type="checkbox"/> One Way: <input checked="" type="checkbox"/> Normal to Bed Joint, <u>20.5</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>29.6</u> vs <u>41.5</u> (SSE+DL) (Allowable) <input type="checkbox"/> Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable)		
Stress on concrete block	<input type="checkbox"/> Two Way: <input type="checkbox"/> Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) <input type="checkbox"/> Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable)		
Remarks	Wall is good for One Way Model.		Wall No. <u>27</u>

SUMMARY OF RESULTS

Location	Turbine Building, North East Stairwell from Turbine operating floor, West Wall Floor Elev. 46'-6"	
Dimensions of Model	Height: <u>8</u> Ft <u>3</u> In. Width: <u>21</u> Ft <u>4</u> In.	Thickness: <u>8</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (x) Unreinforced (x) Running Bond () Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(x) One Way (x) Vertical Span <u>25.1</u> To <u>32.4</u> () Horizontal Span () Two Way () 4-Edge Support _____ To _____ () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	(x) One Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)	() Two Way: _____ g, (OBE) _____ g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> (x) One Way: (x) Normal to Bed Joint, <u>23.4</u> vs <u>25.0</u> (OBE+DL) (Allowable) <u>32.2</u> vs <u>41.5</u> (SSE+DL) (Allowable) </div> <div> () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> () Two Way: () Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div>	
Remarks	Wall is good for one way model	Wall No. <u>28</u>

SUMMARY OF RESULTS

Location	Reactor Bldg. Southeast Stairwell North Wall Fl. El. (-) 19'±6"	
Dimensions of Model	Height: <u>36</u> Ft <u>9.5</u> In. Thickness: <u>8</u> In. Width: <u>16</u> Ft <u>6</u> In.	
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond (X) Reinforced () Other _____ () Unreinforced	
Frequency Range - Hz (Uncracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span (X) Two Way <u>6.92</u> To <u>8.47</u> () 4-Edge Support (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.42</u> g, (OBE) <u>0.67</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> (X) Two Way: () Normal to Bed Joint, <u>N.A.</u> vs _____ (OBE+DL) (Allowable) <u>N.A.</u> vs _____ (SSE+DL) (Allowable) (X) Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) <u>199.0</u> vs <u>26.7</u> (SSE+DL) (Allowable) </div>	
Remarks	Continue for Cracked Section Model Analysis	Wall No. <u>29</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	<div style="display: flex; justify-content: space-between;"> <div> <input type="checkbox"/> One Way <input type="checkbox"/> Vertical Span <input type="checkbox"/> Horizontal Span </div> <div>_____ To _____</div> </div> <hr/> <div> <input checked="" type="checkbox"/> Two Way <input type="checkbox"/> 4-Edge Support <input checked="" type="checkbox"/> 3-Edge Support <input checked="" type="checkbox"/> Top, <input type="checkbox"/> Side, Missing </div> <div style="display: flex; justify-content: space-between;"> <div>_____</div> <div>2.48 To 3.03</div> <div>_____</div> </div>	
Response Acceleration (Cracked Section)	<div> <input type="checkbox"/> One Way: _____ g, <div style="text-align: center;">(OBE)</div> <div>_____ g, <div style="text-align: center;">(SSE)</div> </div> </div>	<div> <input checked="" type="checkbox"/> Two Way: _____ g, <div style="text-align: center;">(OBE)</div> <div>_____ g, <div style="text-align: center;">(SSE)</div> </div> </div>
Flexural Tensile Stresses - psi (Cracked Section)	<div> <input type="checkbox"/> One Way: <input type="checkbox"/> Normal, <input type="checkbox"/> parallel, To Bed Joint </div> <div style="margin-top: 20px;"> $f_{(Steel)} : \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div> <div style="margin-top: 20px;"> $f_{(Conc.)} : \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div> <hr/> <div> <input checked="" type="checkbox"/> Two Way: <input checked="" type="checkbox"/> Normal To Bed Joint </div> <div style="margin-top: 10px;"> <div style="display: flex; align-items: center;"> <div style="margin-right: 10px;">①</div> <div> $f_{(Steel)} : \frac{3300}{(OBE+DL)} \text{ vs } \frac{20000}{(Allowable)}$ $\frac{4520}{(SSE+DL)} \text{ vs } \frac{36000}{(Allowable)}$ </div> </div> <div style="margin-top: 10px;"> <div style="display: flex; align-items: center;"> <div style="margin-right: 10px;">①</div> <div> $f_{(Conc.)} : \frac{-234}{(OBE+DL)} \text{ vs } \frac{-396}{(Allowable)}$ $\frac{-321}{(SSE+DL)} \text{ vs } \frac{-1020}{(Allowable)}$ </div> </div> </div> <hr/> <div style="margin-top: 20px;"> <div style="display: flex; align-items: center;"> <div style="margin-right: 10px;">②</div> <div> $f_{(Steel)} : \frac{33360}{(OBE+DL)} \text{ vs } \frac{30000}{(Allowable)}$ $\frac{45840}{(SSE+DL)} \text{ vs } \frac{63000}{(Allowable)}$ </div> </div> </div> </div>	
Remarks	<div style="display: flex; justify-content: space-between;"> <div> N.G. for Both Models, however wall will not fail (See pg. 7.3-47) </div> <div style="border: 1px solid black; padding: 5px;"> Wall No. 29 </div> </div>	

SUMMARY OF RESULTS

Location	Reactor Bldg. Southeast Stair Well West Wall Fl. El. (-) 19'-6"	
Dimensions of Model	Height: <u>38</u> Ft <u>2.5</u> In. Thickness: <u>8</u> In. Width: <u>8</u> Ft <u>3</u> In.	
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond (X) Reinforced () Other _____ () Unreinforced	
Frequency Range - Hz (Uncracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span (X) Two Way <u>26.67</u> To <u>32.66</u> () 4-Edge Support (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.30</u> g, (OBE) <u>0.48</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, $\frac{\text{_____}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ () Parallel to Bed Joint, $\frac{\text{_____}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ () Parallel to Bed Joint, $\frac{\text{_____}}{\text{(SSE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ </div> <div> (X) Two Way: () Normal to Bed Joint, $\frac{\text{N.A.}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ () Normal to Bed Joint, $\frac{\text{N.A.}}{\text{(SSE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ (X) Parallel to Bed Joint, $\frac{28.1}{\text{(OBE+DL)}} \text{ vs } \frac{16.7}{\text{(Allowable)}}$ (X) Parallel to Bed Joint, $\frac{44.6}{\text{(SSE+DL)}} \text{ vs } \frac{27.7}{\text{(Allowable)}}$ </div>	
Remarks	Continue for Cracked Section Model Analysis	Wall No. <u>30</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span	
	(X) Two Way () 4-Edge Support 20.00 To 24.49 (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Cracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: 0.25 g, (OBE) 0.43 g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	<p>() One Way: () Normal, () parallel, To Bed Joint</p> <div style="text-align: right;"> $f_{(Steel)} : \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div> <div style="text-align: right;"> $f_{(Conc.)} : \frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$ </div> <hr/> <p>(X) Two Way: (X) Normal To Bed Joint</p> <div style="text-align: right;"> $\textcircled{1} f_{(Steel)} : \frac{1389}{(OBE+DL)} \text{ vs } \frac{20000}{(Allowable)}$ $\frac{2390}{(SSE+DL)} \text{ vs } \frac{36000}{(Allowable)}$ </div> <p>(X) Parallel to Bed Joint</p> <div style="text-align: right;"> $\textcircled{1} f_{(Conc.)} : \frac{-52}{(OBE+DL)} \text{ vs } \frac{-396}{(Allowable)}$ (Compression) $\frac{-89}{(SSE+DL)} \text{ vs } \frac{-1020}{(Allowable)}$ </div> <div style="text-align: right;"> $\textcircled{2} f_{(Steel)} : \frac{7360}{(OBE+DL)} \text{ vs } \frac{30000}{(Allowable)}$ $\frac{12650}{(SSE+DL)} \text{ vs } \frac{63000}{(Allowable)}$ </div>	
Remarks	O.K. for Cracked Section Model	Wall No. 30

SUMMARY OF RESULTS

Location	Reactor Bldg. Elevator Shaft West Wall Fl. El: 23'-6"	
Dimensions of Model	Height: 26 Ft 7 In. Thickness: 8 In. Width: 9 Ft 9.5 In.	
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond (X) Reinforced () Other _____ () Unreinforced	
Frequency Range - Hz (Uncracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span (X) Two Way 19.46 To 23.83 () 4-Edge Support (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: 0.44 g, (OBE) 0.65 g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> (X) Two Way: () Normal to Bed Joint, N.A. vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) (X) Parallel to Bed Joint, 45.9 vs 16.7 (OBE+DL) (Allowable) 67.5 vs 27.7 (SSE+DL) (Allowable) </div>	
Remarks	Continue for Cracked Section Model Analysis Wall No. 31	

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span	
	(X) Two Way () 4-Edge Support _____ 9.00 To _____ 11.00 (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Cracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: _____ 0.23 g, (OBE) _____ 0.46 g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	<p>() One Way: () Normal, () parallel, To Bed Joint</p> <div style="text-align: right;"> $f_{(Steel)} = \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> <div style="text-align: right;"> $f_{(Conc.)} = \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> <hr/> <p>(X) Two Way: (X) Normal To Bed Joint</p> <div style="text-align: right;"> $\textcircled{1} f_{(Steel)} = \frac{157}{(OBE+DL)}$ vs $\frac{20000}{(Allowable)}$ $\frac{302}{(SSE+DL)}$ vs $\frac{36000}{(Allowable)}$ </div> <p>(x) Parallel to Bed. Joint</p> <div style="text-align: right;"> $\textcircled{1} f_{(Conc.)} = \frac{-74}{(OBE+DL)}$ vs $\frac{-396}{(Allowable)}$ (Compression) $\frac{-146}{(SSE+DL)}$ vs $\frac{-1026}{(Allowable)}$ </div> <hr/> <div style="text-align: right;"> $\textcircled{2} f_{(Steel)} = \frac{15160}{(OBE+DL)}$ vs $\frac{30000}{(Allowable)}$ $\frac{29910}{(SSE+DL)}$ vs $\frac{63000}{(Allowable)}$ </div>	
Remarks	O.K. for Cracked Section Model	All No. 31

SUMMARY OF RESULTS

Location	Reactor Bldg, Southeast stairwell. North Wall, East Section. FL.EL. 23'-6"	
Dimensions of Model	Height: <u>25</u> Ft <u>9.5</u> In. Thickness: <u>8</u> In. Width: <u>8</u> Ft <u>9.5</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced () Running Bond (X) Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span (X) Two Way (X) 4-Edge Support <u>24.51</u> To <u>30.02</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.45</u> g, (OBE) <u>0.65</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> (X) Two Way: () Normal to Bed Joint, <u>N.A.</u> vs _____ (OBE+DL) (Allowable) <u>N.A.</u> vs _____ (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>38.0</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>54.3</u> vs <u>27.7</u> (SSE+DL) (Allowable) </div>	
Remarks	Continue for Cracked Section Model Analysis	Wall No. <u>32</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	<div> <input type="checkbox"/> One Way <input type="checkbox"/> Vertical Span <input type="checkbox"/> Horizontal Span </div> <div> <input checked="" type="checkbox"/> Two Way <input checked="" type="checkbox"/> 4-Edge Support <input type="checkbox"/> 3-Edge Support <input type="checkbox"/> Top, <input type="checkbox"/> Side, Missing </div> <div> <div></div> <div>17.96</div> <div>To</div> <div>22.00</div> </div>	
Response Acceleration (Cracked Section)	<div> <input type="checkbox"/> One Way: <div></div> g, (OBE) </div> <div> <div></div> g, (SSE) </div>	<div> <input checked="" type="checkbox"/> Two Way: <div>0.28</div> g, (OBE) </div> <div> <div>0.45</div> g, (SSE) </div>
Flexural Tensile Stresses - psi (Cracked Section)	<div> <input type="checkbox"/> One Way: <input type="checkbox"/> Normal, <input type="checkbox"/> parallel, To Bed Joint </div> <div> <div> $f_{(Steel)}$: <div></div> vs <div></div> <div>(OBE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Steel)}$: <div></div> vs <div></div> <div>(SSE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Conc.)}$: <div></div> vs <div></div> <div>(OBE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Conc.)}$: <div></div> vs <div></div> <div>(SSE+DL)</div> <div>(Allowable)</div> </div> </div> <div> <input checked="" type="checkbox"/> Two Way: <input checked="" type="checkbox"/> Normal To Bed Joint </div> <div> <div> $f_{(Steel)}$: <div>707</div> vs <div>20000</div> <div>(OBE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Steel)}$: <div>1140</div> vs <div>36000</div> <div>(SSE+DL)</div> <div>(Allowable)</div> </div> <div> <input checked="" type="checkbox"/> Parallel to Bed Joint </div> <div> $f_{(Conc.)}$: <div>-72</div> vs <div>-396</div> <div>(OBE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Conc.)}$: <div>-115</div> vs <div>-1020</div> <div>(SSE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Steel)}$: <div>14910</div> vs <div>30000</div> <div>(OBE+DL)</div> <div>(Allowable)</div> </div> <div> $f_{(Steel)}$: <div>23680</div> vs <div>62000</div> <div>(SSE+DL)</div> <div>(Allowable)</div> </div> </div>	
Remarks	<div> O. K. for Cracked Section Model </div> <div> Wall No. <div>32</div> </div>	

SUMMARY OF RESULTS

Location	Reactor Bldg, Southeast stairwell. West wall Fl. El. 23'-6"	
Dimensions of Model	Height: <u>25</u> Ft <u>3.5</u> In. Thickness: <u>8</u> In. Width: <u>8</u> Ft <u>0</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" (X) Reinforced () Unreinforced () Running Bond (X) Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span (X) Two Way (X) 4-Edge Support <u>28.24</u> To <u>34.59</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.30</u> g, (OBE) <u>0.50</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> () Parallel to Bed Joint, _____ vs _____ (OBE+DL) (Allowable) _____ vs _____ (SSE+DL) (Allowable) </div> <div> (X) Two Way: () Normal to Bed Joint, <u>N.A.</u> vs _____ (OBE+DL) (Allowable) <u>N.A.</u> vs _____ (SSE+DL) (Allowable) </div> <div> (X) Parallel to Bed Joint, <u>29.2</u> vs <u>16.7</u> (OBE+DL) (Allowable) <u>47.4</u> vs <u>27.7</u> (SSE+DL) (Allowable) </div>	
Remarks	Continue for cracked section model analysis Wall No. <u>33</u>	

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	() One Way	
	() Vertical Span _____ To _____ () Horizontal Span	
	(X) Two Way (X) 4-Edge Support <u>17.80</u> To <u>21.80</u> () 3-Edge Support () Top, () Side, Missing	
Response Acceleration (Cracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.25</u> g, (OBE) <u>0.40</u> g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	() One Way: () Normal, () parallel, To Bed Joint	
	$f_{(Steel)}$:	$\frac{\quad}{(OBE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$
		$\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$
	$f_{(Conc.)}$:	$\frac{\quad}{(OBE-DL)} \text{ vs } \frac{\quad}{(Allowable)}$
		$\frac{\quad}{(SSE+DL)} \text{ vs } \frac{\quad}{(Allowable)}$
	(X) Two Way: (X) Normal To Bed Joint	
	① $f_{(Steel)}$: $\frac{839}{(OBE+DL)} \text{ vs } \frac{20000}{(Allowable)}$	
(X) Parallel to Bed Joint	$\frac{1280}{(SSE+DL)} \text{ vs } \frac{36000}{(Allowable)}$	
	① $f_{(Conc.)}$: $\frac{-72}{(OBE+DL)} \text{ vs } \frac{-396}{(Allowable)}$	
(Compression)	$\frac{-114}{(SSE+DL)} \text{ vs } \frac{-1020}{(Allowable)}$	
	② $f_{(Steel)}$: $\frac{14840}{(OBE+DL)} \text{ vs } \frac{30000}{(Allowable)}$	
	$\frac{23540}{(SSE+DL)} \text{ vs } \frac{63000}{(Allowable)}$	
Remarks	OK for cracked section Model	Wall No. <u>33</u>

SUMMARY OF RESULTS

Location	Office Bldg. Corridor # 5. East Wall. North Section Fl. El. 46'-6"		
Dimensions of Model	Height: <u>13</u> Ft <u>6</u> In. Thickness: <u>6</u> In. Width: <u>9</u> Ft <u>2.5</u> In.		
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (X) Unreinforced (X) Running Bond () Stacked Bond (X) - With face Tiles		
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>5.88</u> To <u>7.59</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>17.68</u> To <u>22.83</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.93</u> g, (OBE) <u>1.20</u> g, (SSE)	() Two Way: <u>0.35</u> g, (OBE) <u>0.65</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	<div> (X) One Way: (X) Normal to Bed Joint, $\frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ <u>355.0</u> vs <u>41.5</u> $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> <div> () Parallel to Bed Joint, $\frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> <div> (X) Two Way: (X) Normal to Bed Joint, $\frac{16.4}{(OBE+DL)}$ vs $\frac{25.0}{(Allowable)}$ $\frac{30.9}{(SSE+DL)}$ vs $\frac{41.5}{(Allowable)}$ (x) Parallel to Bed Joint, $\frac{31.4}{(OBE+DL)}$ vs $\frac{50.0}{(Allowable)}$ $\frac{55.4}{(SSE+DL)}$ vs $\frac{83.0}{(Allowable)}$ </div>		
Remarks	O.K. for two way model		Wall No. <u>42A</u>

SUMMARY OF RESULTS

Location	Office Bldg. Corridor # 5 East Wall, Interm. Section Fl. El. 46'-6"		
Dimensions of Model	Height: <u>13</u> Ft <u>6</u> In. Thickness: <u>6</u> In. Width: <u>8</u> Ft <u>6</u> In.		
Type of Construction	Block: ASTM - C-90 (X) Running Bond Mortar: Type "M" () Stacked Bond () Reinforced (X) <u>With face</u> (X) Unreinforced Tiles		
Frequency Range - Hz (Uncracked Section)	(X) One Way <u>5.88</u> To <u>7.69</u> (X) Vertical Span () Horizontal Span (X) Two Way <u>20.36</u> To <u>26.29</u> (X) 4-Edge Support () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.93</u> g, (OBE) <u>1.20</u> g, (SSE)	(X) Two Way: <u>0.45</u> g, (OBE) <u>0.65</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	<div> (X) One Way: (X) Normal to Bed Joint, vs <u> </u> <div> <u> </u> (OBE+DL) (Allowable) <u>317.0</u> vs <u>41.5</u> <u> </u> (SSE+DL) (Allowable) </div> </div> <div> () Parallel to Bed Joint, vs <u> </u> <div> <u> </u> (OBE+DL) (Allowable) <u> </u> vs <u> </u> <u> </u> (SSE+DL) (Allowable) </div> </div> <div> (X) Two Way: (X) Normal to Bed Joint, vs <u>25.0</u> <div> <u>12.6</u> vs <u> </u> <u> </u> (OBE+DL) (Allowable) <u>20.2</u> vs <u>41.5</u> <u> </u> (SSE+DL) (Allowable) </div> </div> <div> (X) Parallel to Bed Joint, vs <u>50.0</u> <div> <u>32.1</u> vs <u> </u> <u> </u> (OBE+DL) (Allowable) <u>46.3</u> vs <u>83.0</u> <u> </u> (SSE+DL) (Allowable) </div> </div>		
Remarks	O.K. for two way model		Wall No. <u>42B</u> <u>42D</u>

SUMMARY OF RESULTS

Location	Office Bldg. Corridor #5 East Wall, Interm, Section Fl. El. 46'-6"		
Dimensions of Model	Height: <u>13</u> Ft <u>6</u> In. Width: <u>11</u> Ft <u>3.5</u> In.		Thickness: <u>6</u> In.
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" <input type="checkbox"/> Reinforced <input checked="" type="checkbox"/> Unreinforced <input checked="" type="checkbox"/> Running Bond <input type="checkbox"/> Stacked Bond <input checked="" type="checkbox"/> With face Tiles		
Frequency Range - Hz (Uncracked Section)	<input checked="" type="checkbox"/> One Way <input checked="" type="checkbox"/> Vertical Span <u>5.88</u> To <u>7.67</u> <input type="checkbox"/> Horizontal Span <input checked="" type="checkbox"/> Two Way <input checked="" type="checkbox"/> 4-Edge Support <u>13.83</u> To <u>17.86</u> <input type="checkbox"/> 3-Edge Support <input type="checkbox"/> Top, <input type="checkbox"/> Side, Missing		
Response Acceleration (Uncracked Section)	<input checked="" type="checkbox"/> One Way: <u>0.93</u> g, (OBE) <u>1.20</u> g, (SSE)		<input checked="" type="checkbox"/> Two Way: <u>0.35</u> g, (OBE) <u>0.45</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<input checked="" type="checkbox"/> One Way: <input checked="" type="checkbox"/> Normal to Bed Joint, <u>(OBE+DL)</u> vs <u>(Allowable)</u> <u>422,0</u> vs <u>41.5</u> <u>(SSE+DL)</u> vs <u>(Allowable)</u> <input type="checkbox"/> Parallel to Bed Joint, <u>(OBE+DL)</u> vs <u>(Allowable)</u> <u>(SSE+DL)</u> vs <u>(Allowable)</u> <input checked="" type="checkbox"/> Two Way: <input checked="" type="checkbox"/> Normal to Bed Joint, <u>20.6</u> vs <u>25.0</u> <u>(OBE+DL)</u> vs <u>(Allowable)</u> <u>28.1</u> vs <u>41.5</u> <u>(SSE+DL)</u> vs <u>(Allowable)</u> <input checked="" type="checkbox"/> Parallel to Bed Joint, <u>33.4</u> vs <u>50.0</u> <u>(OBE+DL)</u> vs <u>(Allowable)</u> <u>43.0</u> vs <u>83.0</u> <u>(SSE+DL)</u> vs <u>(Allowable)</u>		
Remarks	O.K. for Two WAY Model		Wall No. <u>42C</u>

SUMMARY OF RESULTS

Location	Reactor Bldg Shutdown Heat Exchanger Room. North Wall, Interm. Section Fl. El. 51'-3"		
Dimensions of Model	Height: <u>9</u> Ft <u>11</u> In. Thickness: <u>24</u> In. Width: <u>14</u> Ft <u>11</u> In.		
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond () Reinforced (X) fully grouted double (X) Unreinforced Wythe 12 in. each wythe		
Frequency Range - Hz (Uncracked Section)	(X) One Way (X) Vertical Span <u>25.03</u> To <u>30.65</u> () Horizontal Span (X) Two Way (X) 4-Edge Support <u>34.45</u> To <u>42.19</u> () 3-Edge Support () Top, () Side, Missing		
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.32</u> g, (OBE) <u>0.52</u> g, (SSE)	(X) Two Way: <u>0.26</u> g, (OBE) <u>0.42</u> g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(X) One Way: (X) Normal to Bed Joint, <u>25.4</u> vs <u>40.0</u> (OBE+DL) (Allowable) <u>43.1</u> vs <u>67.0</u> (SSE+DL) (Allowable) () Parallel to Bed Joint, vs (OBE+DL) (Allowable) (SSE+DL) (Allowable) (X) Two Way: (X) Normal to Bed Joint, <u>12.7</u> vs <u>40.0</u> (OBE+DL) (Allowable) <u>20.4</u> vs <u>67.0</u> (SSE+DL) (Allowable) (X) Parallel to Bed Joint, <u>9.3</u> vs <u>26.7</u> (OBE+DL) (Allowable) <u>13.8</u> vs <u>44.5</u> (SSE+DL) (Allowable)		
Remarks	O.K. for both models		Wall No. <u>43</u>

SUMMARY OF RESULTS

Location	Turbine Building, North East Stairwell from turbine Operating Floor, North Wall Floor Elev. 46'-6"			
Dimensions of Model	Height: <u>8</u> Ft <u>3</u> In. Width: <u>5</u> Ft <u>10</u> In.		Thickness: <u>8</u> In.	
Type of Construction	Block: ASTM - C-90 Mortar: Type "M" () Reinforced (x) Unreinforced		(x) Running Bond () Stacked Bond () Other _____	
Frequency Range - Hz (Uncracked Section)	(x) One Way (x) Vertical Span () Horizontal Span		<u>25.15</u> To <u>32.47</u>	
	() Two Way () 4-Edge Support () 3-Edge Support () Top, () Side, Missing		_____ To _____	
Response Acceleration (Uncracked Section)	(X) One Way: <u>0.33</u> g, (OBE) <u>0.54</u> g, (SSE)		() Two Way: _____ g, (OBE) _____ g, (SSE)	
Flexural Tensile Stresses - psi (Uncracked Section)	(x) One Way:			
	(x) Normal to Bed Joint,		<u>23.1</u> (OBE+DL)	vs <u>25.0</u> (Allowable)
			<u>33.7</u> (SSE+DL)	vs <u>41.5</u> (Allowable)
	() Parallel to Bed Joint,		_____ vs _____ (OBE+DL) (Allowable)	_____ vs _____ (SSE+DL) (Allowable)
	() Two Way:			
	() Normal to Bed Joint,		_____ vs _____ (OBE+DL) (Allowable)	_____ vs _____ (SSE+DL) (Allowable)
			_____ vs _____ (OBE+DL) (Allowable)	_____ vs _____ (SSE+DL) (Allowable)
	() Parallel to Bed Joint,		_____ vs _____ (OBE+DL) (Allowable)	_____ vs _____ (SSE+DL) (Allowable)
Remarks	Wall is good for one way model		Wall No. <u>44</u>	

SUMMARY OF RESULTS

Location	Reactor Bldg. Elevator Shaft North Wall Fl. El. 23'-6"	
Dimensions of Model	Height: <u>26</u> Ft <u>7</u> In. Thickness: <u>8</u> In. Width: <u>10</u> Ft <u>5</u> In.	
Type of Construction	Block: ASTM - C-90 () Running Bond Mortar: Type "M" (X) Stacked Bond (X) Reinforced () Other _____ () Unreinforced	
Frequency Range - Hz (Uncracked Section)	() One Way _____ To _____ () Vertical Span _____ () Horizontal Span _____ (X) Two Way _____ To <u>17.45</u> To <u>21.37</u> () 4-Edge Support (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Uncracked Section)	() One Way: _____ g, (OBE) _____ g, (SSE)	(X) Two Way: <u>0.35</u> g, (OBE) <u>0.50</u> g, (SSE)
Flexural Tensile Stresses - psi (Uncracked Section)	<div> () One Way: () Normal to Bed Joint, $\frac{\text{_____}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ $\frac{\text{_____}}{\text{(SSE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ </div> <div> () Parallel to Bed Joint, $\frac{\text{_____}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ $\frac{\text{_____}}{\text{(SSE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ </div> <div> (X) Two Way: () Normal to Bed Joint, $\frac{\text{N.A.}}{\text{(OBE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ $\frac{\text{N.A.}}{\text{(SSE+DL)}} \text{ vs } \frac{\text{_____}}{\text{(Allowable)}}$ </div> <div> (X) Parallel to Bed Joint, $\frac{43.7}{\text{(OBE+DL)}} \text{ vs } \frac{16.7}{\text{(Allowable)}}$ $\frac{61.9}{\text{(SSE+DL)}} \text{ vs } \frac{27.7}{\text{(Allowable)}}$ </div>	
Remarks	Continue for cracked section model analysis	Wall No. <u>45</u>

SUMMARY OF RESULTS - (Cont'd)

Frequency Range - Hz (Cracked Section)	() One Way () Vertical Span _____ To _____ () Horizontal Span	
	(X) Two Way () 4-Edge Support 8.04 To 9.85 (X) 3-Edge Support (X) Top, () Side, Missing	
Response Acceleration (Cracked Section)	() One Way: _____ g, (OBE) _____, (SSE)	(X) Two Way: 0.40 _____ g, (OBE) 0.60 _____ g, (SSE)
Flexural Tensile Stresses - psi (Cracked Section)	() One Way: () Normal, () parallel, To Bed Joint <div style="text-align: right;"> $f_{(Steel)} : \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $f_{(Conc.)} : \frac{\quad}{(OBE+DL)}$ vs $\frac{\quad}{(Allowable)}$ $\frac{\quad}{(SSE+DL)}$ vs $\frac{\quad}{(Allowable)}$ </div> (X) Two Way: (X) Normal To Bed Joint <div style="text-align: right;"> $f_{(Steel)} : \frac{1337}{(OBE+DL)}$ vs $\frac{20000}{(Allowable)}$ $\frac{2018}{(SSE+DL)}$ vs $\frac{36000}{(Allowable)}$ $f_{(Conc.)} : \frac{-144}{(OBE+DL)}$ vs $\frac{-396}{(Allowable)}$ (Compression) $\frac{-216}{(SSE+DL)}$ vs $\frac{-1020}{(Allowable)}$ $f_{(Steel)} : \frac{29820}{(OBE+DL)}$ vs $\frac{30000}{(Allowable)}$ $\frac{44660}{(SSE+DL)}$ vs $\frac{63000}{(Allowable)}$ </div>	
Remarks	O.K. for cracked section model	Wall No. 45

OYSTER CREEK NUCLEAR STATION
SUMMARY OF OUT OF PLANE SHEARS
FOR UNREINFORCED WALLS

Wall	Thickness	Construction	Max Calculated Shear Force		Shear Capacity of Walls		Results
			Model	Nx or Ny lb/ft	SSE+DL lb/ft	OBE+DL lb/ft	
17 18 42A 42B & 42D 42C	6in.	Running Bond	Two Way	86 90 131 114 99	706	457	All Walls satisfy shear requirement
25 26	8in.	Running Bond	Two Way	87 315	831	573	
24-2 27 28 44	8in	Running Bond	One Way	112 220 104 89	883	573	

Notes: For all unreinforced Walls which satisfy the stress requirements.

OYSTER CREEK NUCLEAR STATION
IN PLANE STRAIN OF BLDG. MODELS @ OBE

Bldg. Model	Flr. Mass No.	Flr. Elev.	Max. Disp.	Story Height	Diff. Disp.	In-Plane Strain	Remarks	Results
Reactor Bldg.	5	75.25	2.795×10^{-3}	24.00	0.868×10^{-3}	3.62×10^{-5}	Allowable Strain $\gamma_c \leq 8 \times 10^{-4}$	All Walls Satisfy In-Plane Strain Req't
	6	51.25	1.927	27.75	0.994	3.58		
	7	23.50	0.933	23.50	0.473	2.01		
	8	0.00	0.460	19.00	0.460	2.42		
	Base	-19.00	0.000	--	--			
Turbine Bldg.	1	46.50	0.599	23.00	0.282	1.23		
	2	23.50	0.317	23.50	0.317	1.35		
	Base	0.00	0.000	--	--			

- NOTES: 1. All masonry walls investigated are within the scope of the tabulated elev. and classified as confined walls.
2. All linear units in ft.

OYSTER CREEK NUCLEAR STATION
IN PLANE STRAIN OF BLDG. MODELS @ SSE

BLDG Model	FLR MASS No.	FLR ELEV.	MAX DISP.	Story Height	Diff. DISP.	IN PLANE STRAIN	REMARKS	RESULT
Reactor BLDG	5	75.25	5.591×10^{-3}	24.00	1.738×10^{-3}	7.24×10^{-5}	Allowable Strain $\gamma_c \leq 1.33 \times 10^{-3}$	All Wall: satio: In-Pl strai: requi ment
	6	51.25	3.853	27.75	1.986	7.16		
	7	23.50	1.867	23.50	0.947	4.03		
	8	0.00	0.920	19.00	0.920	4.84		
	Base	-19.00	0.000	—	—	—		
Turbine Bldg	2	46.50	1.197	23.00	0.564	2.45		
	1	23.50	0.633	23.50	0.633	2.69		
	Base	0.00	0.000	—	—	—		

For Notes see the Preceeding page.

OYSTER CREEK NUCLEAR STATION
SUMMARY FOR OPERABILITY RESULTS

Wall Type	Wall No.	Height of Wall	Nominal Thickness of wall in.	DISPLACEMENTS		AXIAL FORCE		Remarks	Result
				Actual Δ_a in.	Allowable $\Delta = t/3$ in.	Actual H_a lb/ft	Allowable H lb/ft		
Unreinforced	8	13'-4"	5 ⁵ / ₈	0.22	1.88	745.7	3621.0		OK
	15	13'-4"	5 ⁵ / ₈	0.17	1.88	619.0	3621.0		OK
	19	11'-0	5 ⁵ / ₈	0.007	1.88	353	3621		OK
	20	11'-0	5 ⁵ / ₈	0.007	1.88	353	3621		OK
	21	11'-2 $\frac{1}{2}$ "	7 ⁵ / ₈	0.001	2.54	173	4208	Steel Beam to be provided @ El 31.0	Wall is O.K. with the Steel Beam
	22	9'-0"	7 ⁵ / ₈	0.002	2.54	220	4208		OK
	23	11'-0"	7 ⁵ / ₈	0.004	2.54	341	4208		OK
	24-1	17'-11"	7 ⁵ / ₈	0.028	2.54	1010.0	4208		OK
Reinforced	29	16'-6"	7 ⁵ / ₈	The Dur-O-wall is approximately 11.2% over stressed and is less than 50% of the yield stress therefore the wall will not fail					OK

APPENDIX 7.4
Reinforcing Details

APPENDIX 7.5
Boundary Support Requirements

OYSTER CREEK NUCLEAR STATION
WALL BOUNDARY SUPPORT REQUIREMENTS

Wall No.	Top	Side		Side		Remarks
		North	East	South	West	
2			yes		yes	* North-West ** South-East
5			yes		yes	
6		yes *		yes **		
7		yes		yes		
8	yes					+ Additional supports to be provided for equipment
15	yes				yes	
17	yes				yes	
18	yes			yes		
19	yes					
20	yes					
21	yes	+				
22	yes					
23	yes					
24	yes					
25				yes		
26						
27						
28						
29			yes			
30				yes		
31				yes		
32	yes		yes		yes	
33	yes		+	yes		
42	yes	yes		yes		
43				+		
44						
45						

APPENDIX 7.6
Schedule for Implementation
of Design
Modifications

CONSTRUCTION SCHEDULE

The implementation of the design modifications for items listed below will be completed prior to the restart from our 1981/1982 Refueling Outage.

1. Pre-emptive modification to 19 walls by removal of the effected sections of the concrete block walls to eliminate potential missile hazard to the vital systems. These walls are wall Nos. 1, 3, 4, 9, 10, 11, 12, 13, 14, 16, 34, 35, 37, 38, 39, 40, 41, 46 and 47.
2. Repair all visible cracks on both sides of the concrete block walls.
3. Installation of wall boundary supports.
4. Modification of equipment supports for wall Nos. 21, 33 and 43.

APPENDIX 7.7
Floor Response Spectra
Reactor Building
Turbine Building

Floor Responses Spectra

Building	Floor Elevation	Damping Value			
		OBE		SSE	
		2%	4%	4%	7%
R.B.	23'-6	x	x	x	x
	51'-3	x	x	x	x
T.B.	23'-6	x	x	x	x
	46'-6	x	x	x	x

FLOOR SPECTRA R.S. 05E

SI. 20.50 (MS) DF=2%

ACCELERATION (G)

1.00

0.90

0.80

0.70

0.60

0.50

0.40

0.30

0.20

0.10

0.50

1.00

2.00

5.00

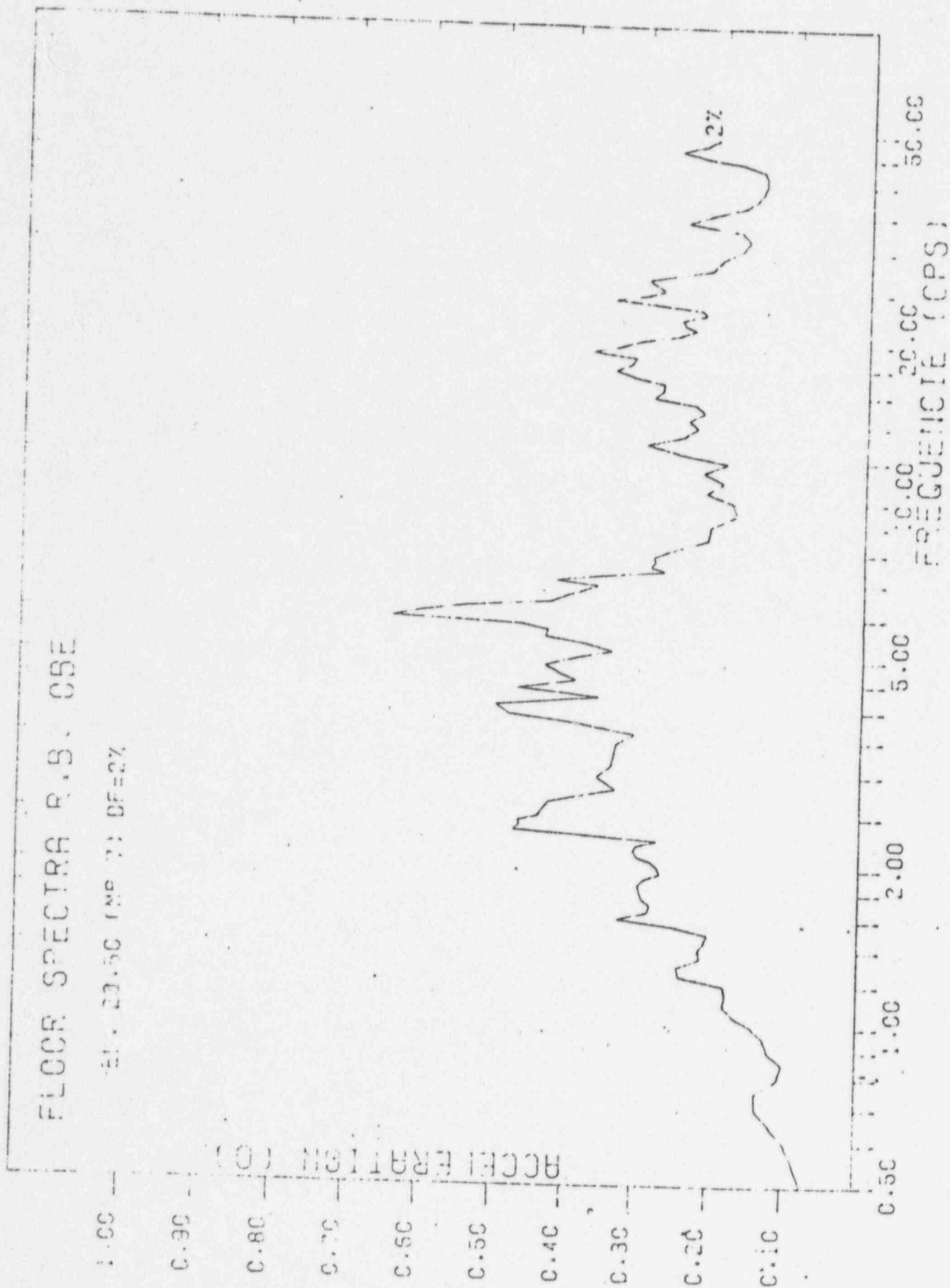
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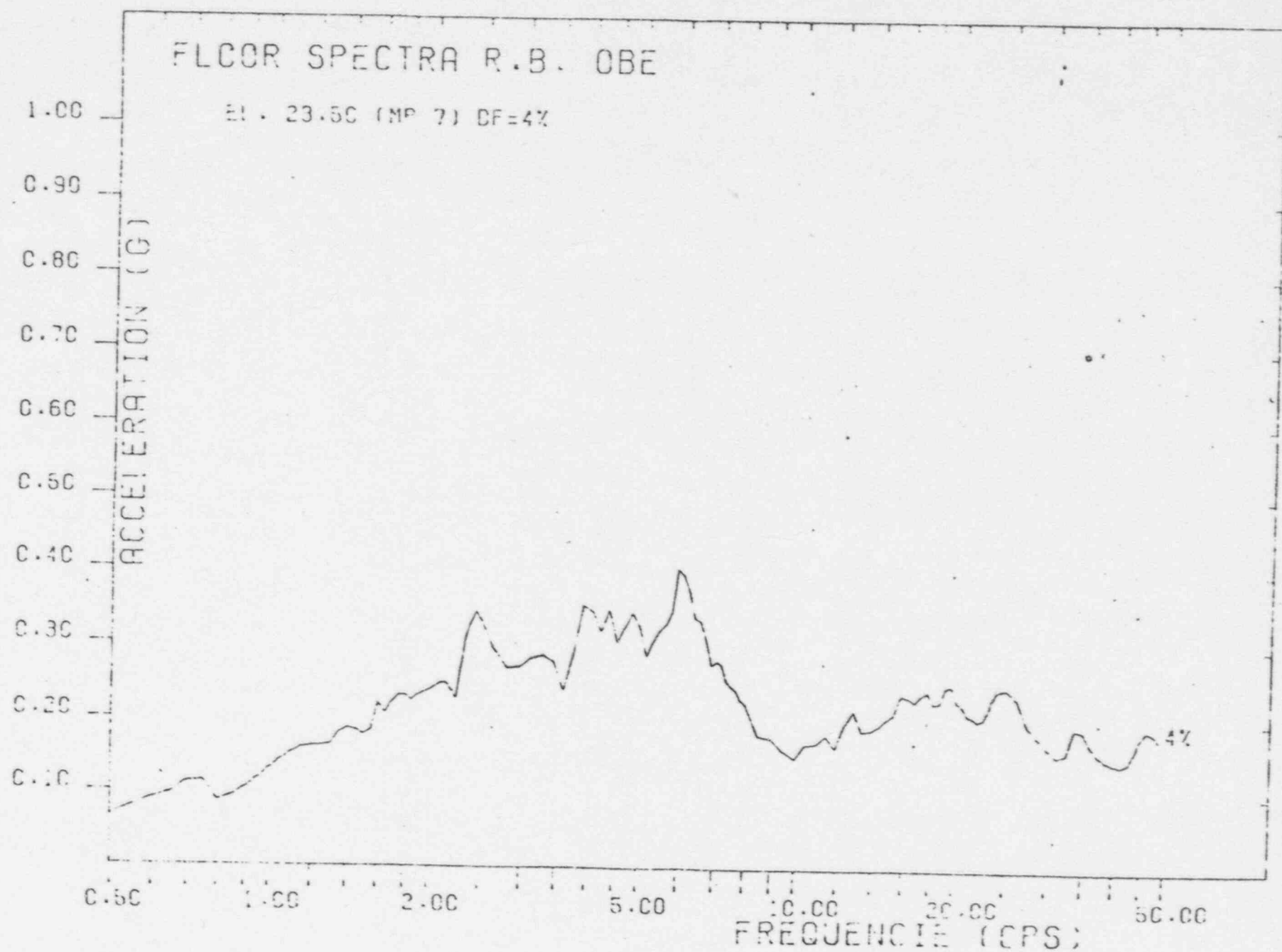
20.00

50.00

FREQUENCY (CPS)

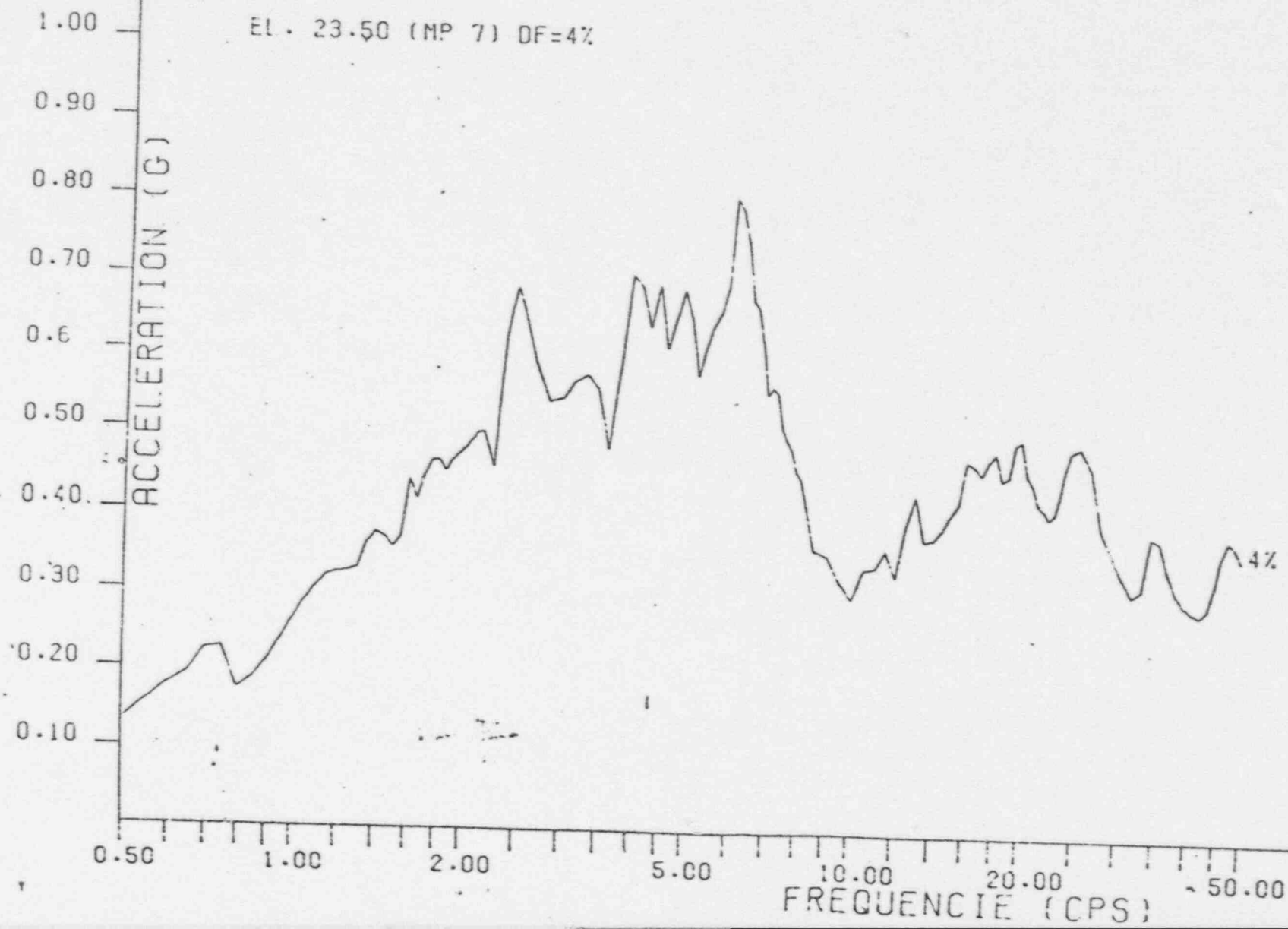
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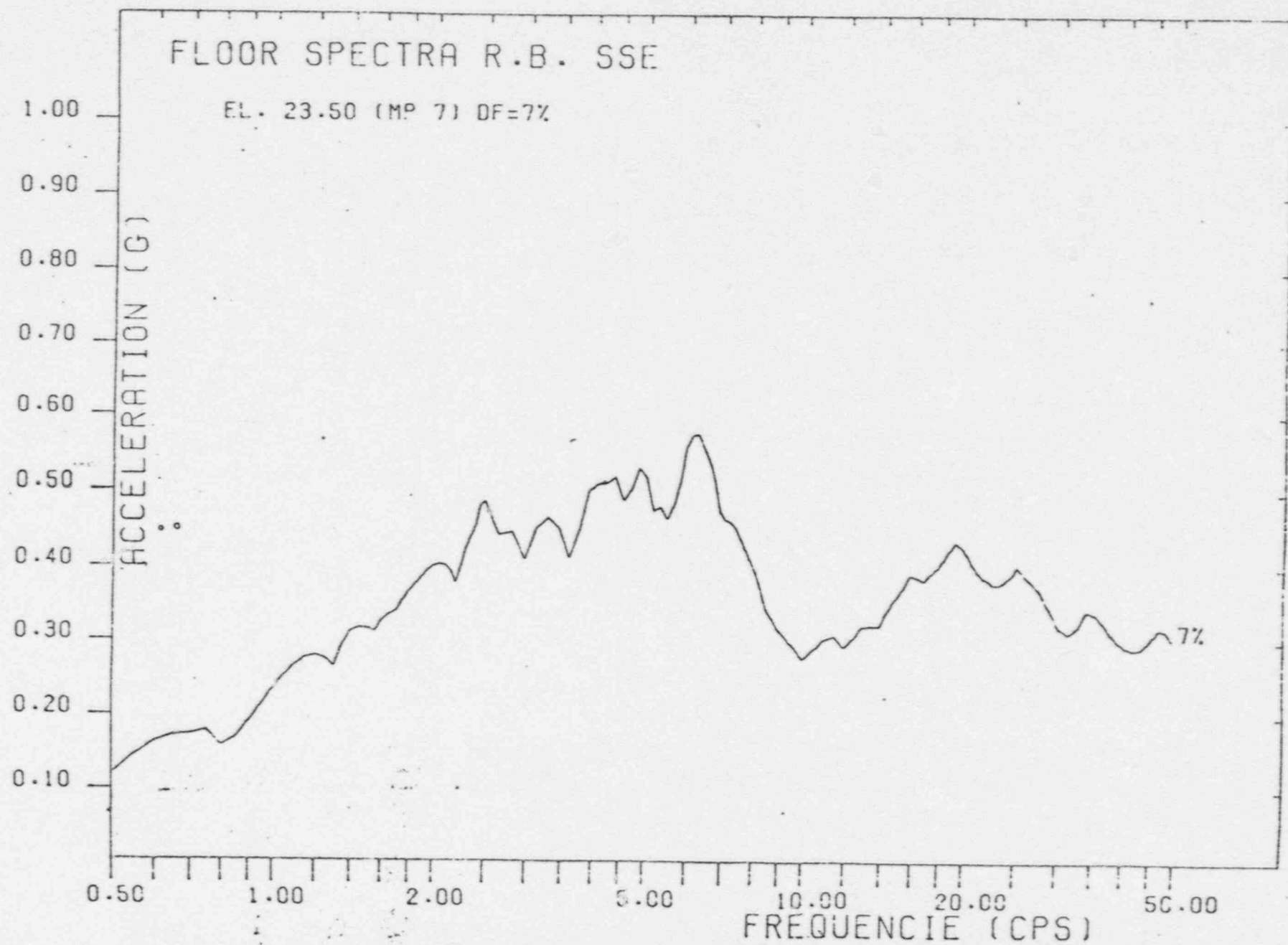




FLOOR SPECTRA R.B. SSE .

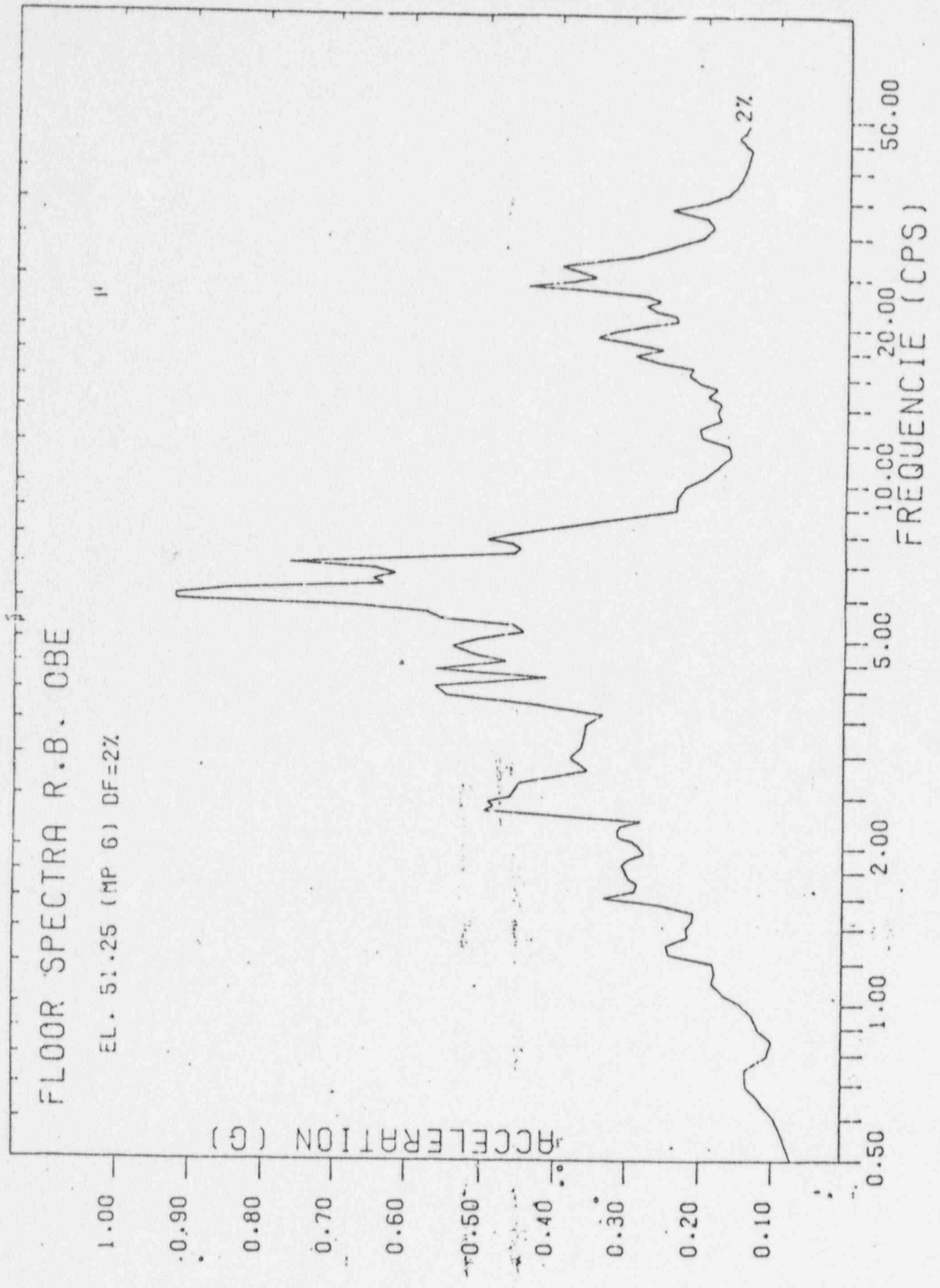
EL. 23.50 (MP 7) DF=4%





FLOOR SPECTRA R.B. OBE

EL. 51.25 (MP 6) DF=2%



FLOOR SPECTRA R.B. OBE

EL. 51.25 (MP 6) DF=4%

1.00

0.90

0.80

0.70

0.60

0.50

0.40

0.30

0.20

0.10

ACCELERATION (G)

0.50

1.00

2.00

5.00

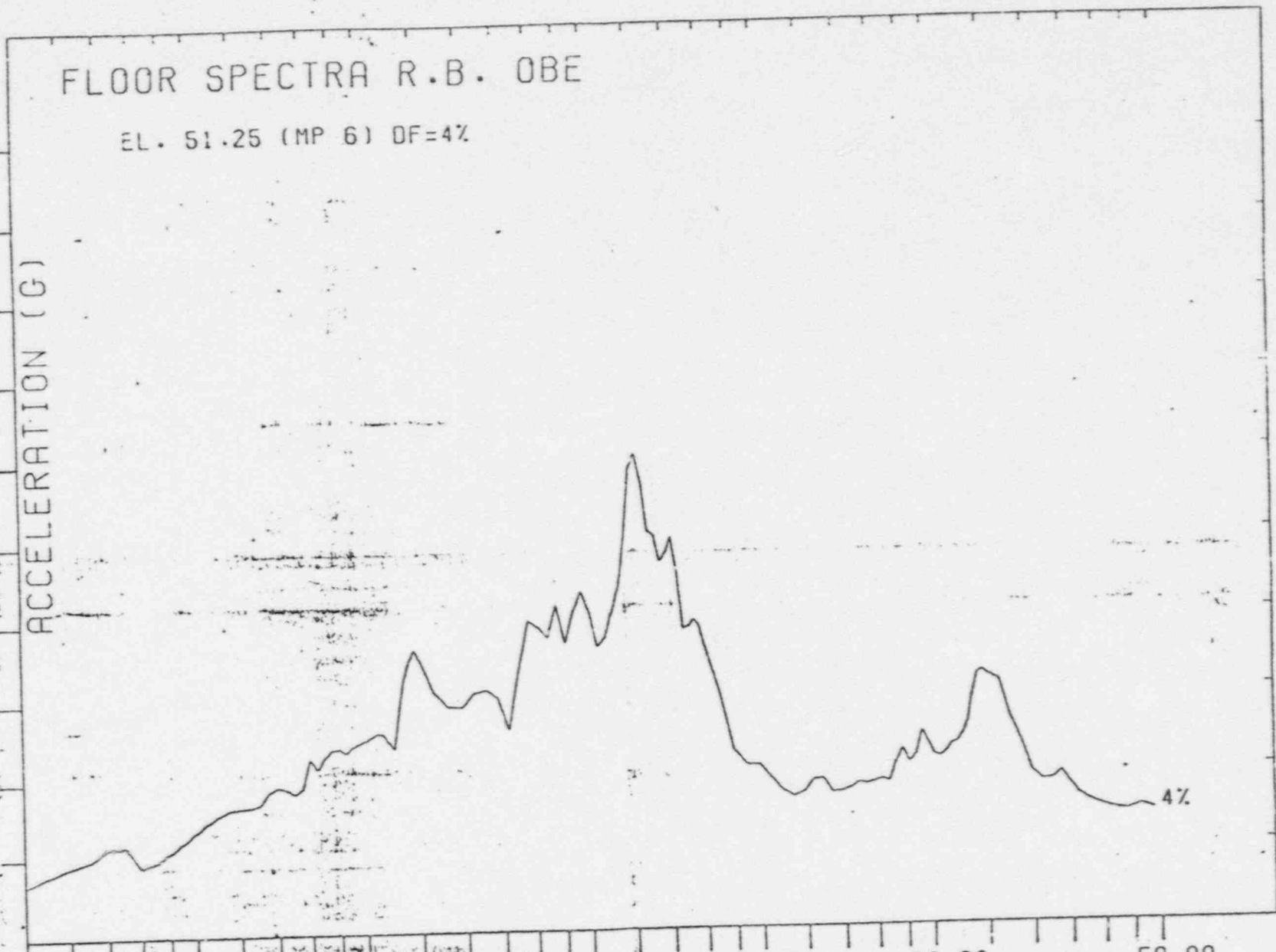
10.00

20.00

50.00

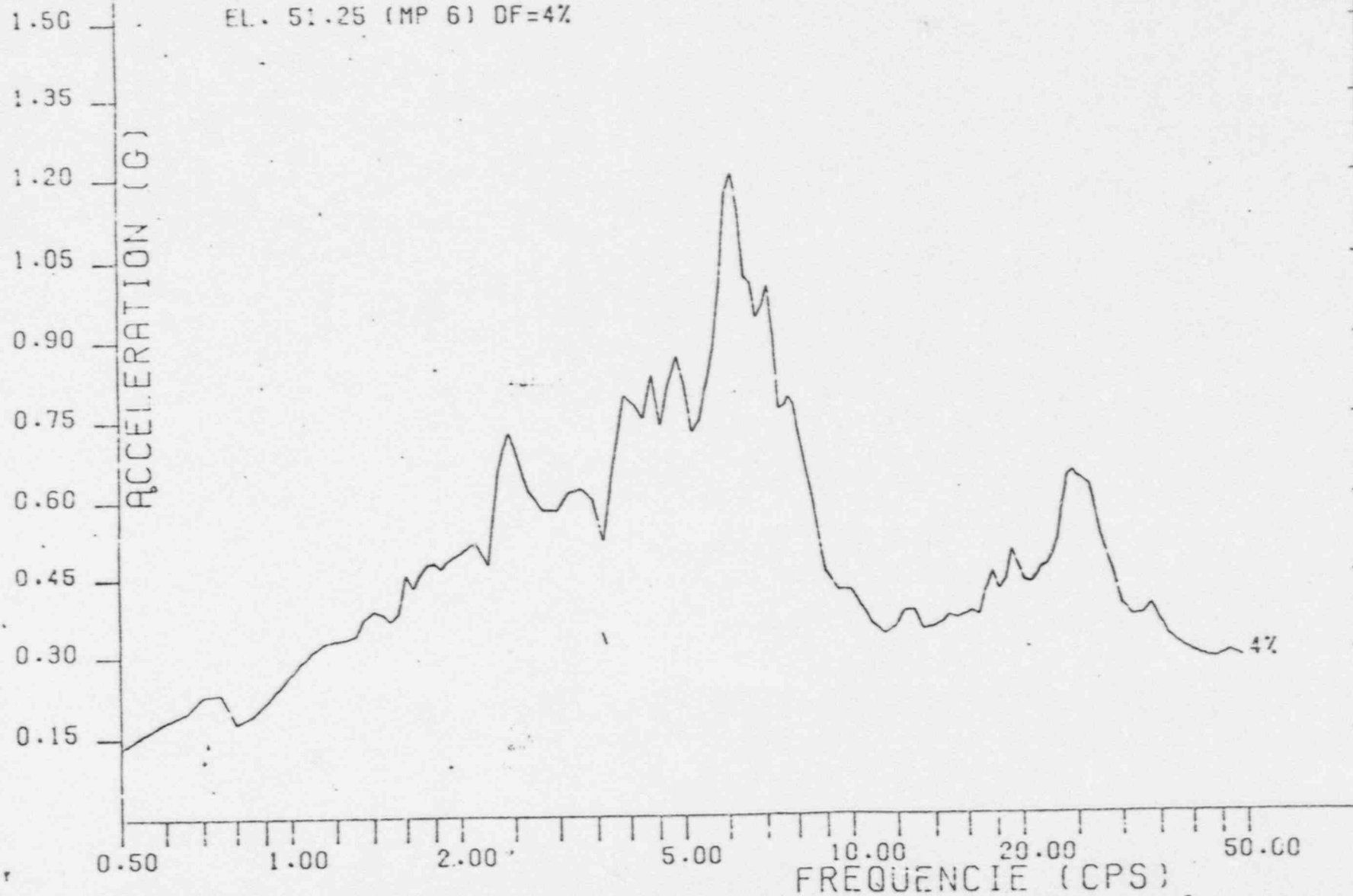
FREQUENCY (CPS)

4%



FLCOR SPECTRA R.B. SSE

EL. 51.25 (MP 6) DF=4%



FLOOR SPECTRA R.B. SSE

EL. 51.25 (MP 6) DF=7%

1.00

0.90

0.80

0.70

0.60

0.50

0.40

0.30

0.20

0.10

ACCELERATION (G)

0.50

1.00

2.00

5.00

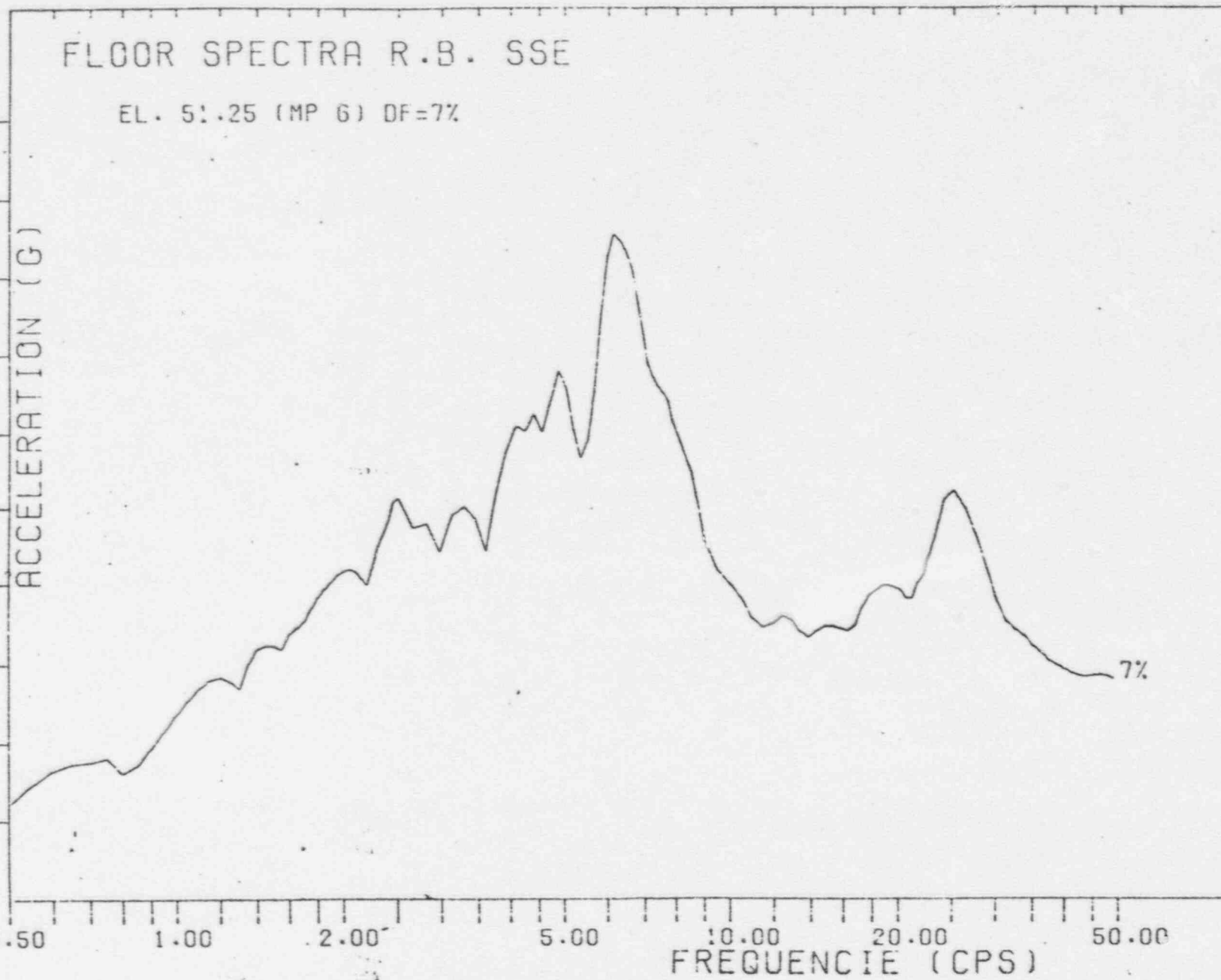
10.00

20.00

50.00

FREQUENCY (CPS)

7%



FLOOR SPECTRA T.B. CBE

EL. 23.5 (MP 2) DF=2.3%

ACCELERATION (G)

1.00

0.90

0.80

0.70

0.60

0.50

0.40

0.30

0.20

0.10

0.50

1.00

2.00

5.00

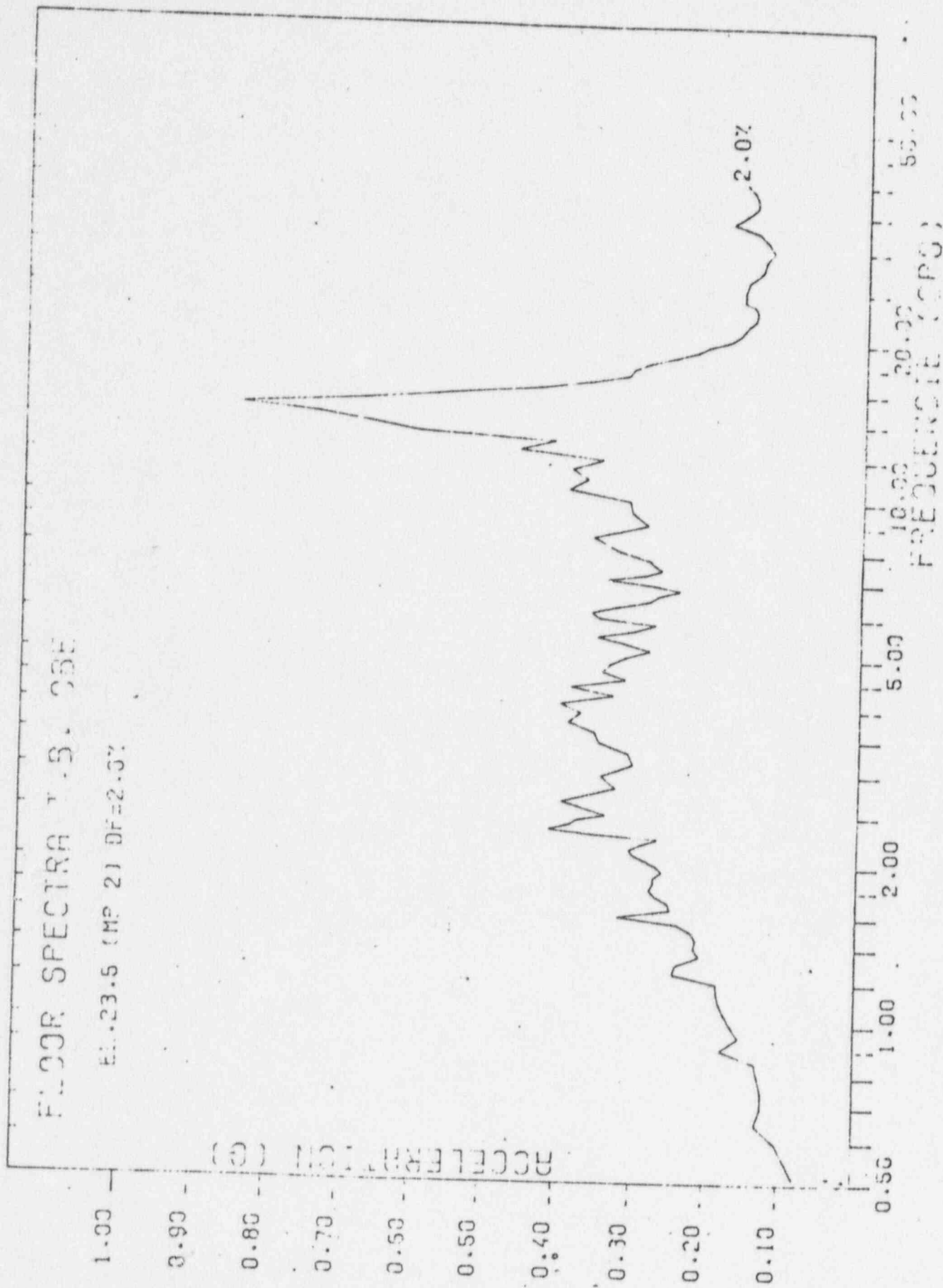
10.00

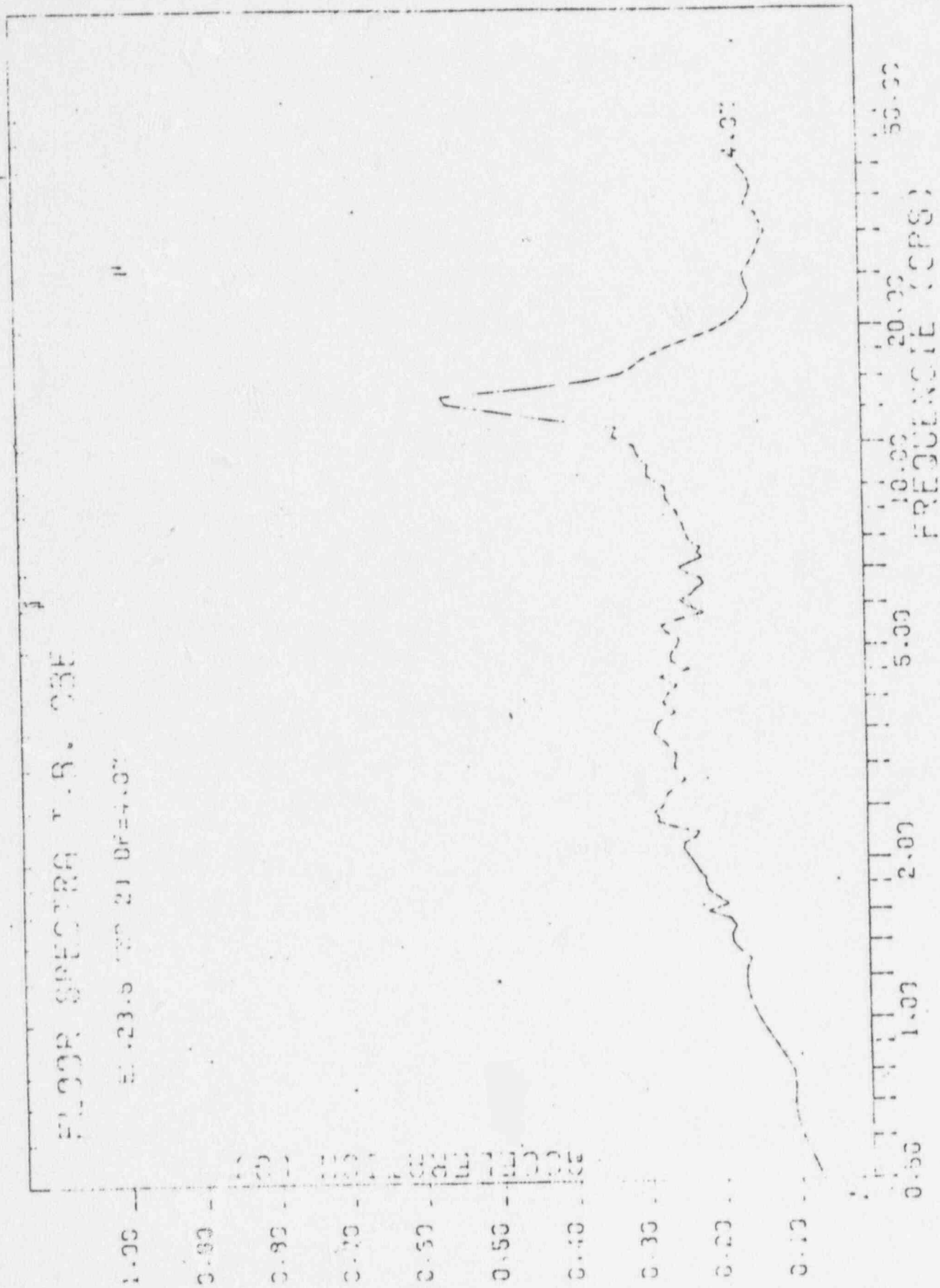
20.00

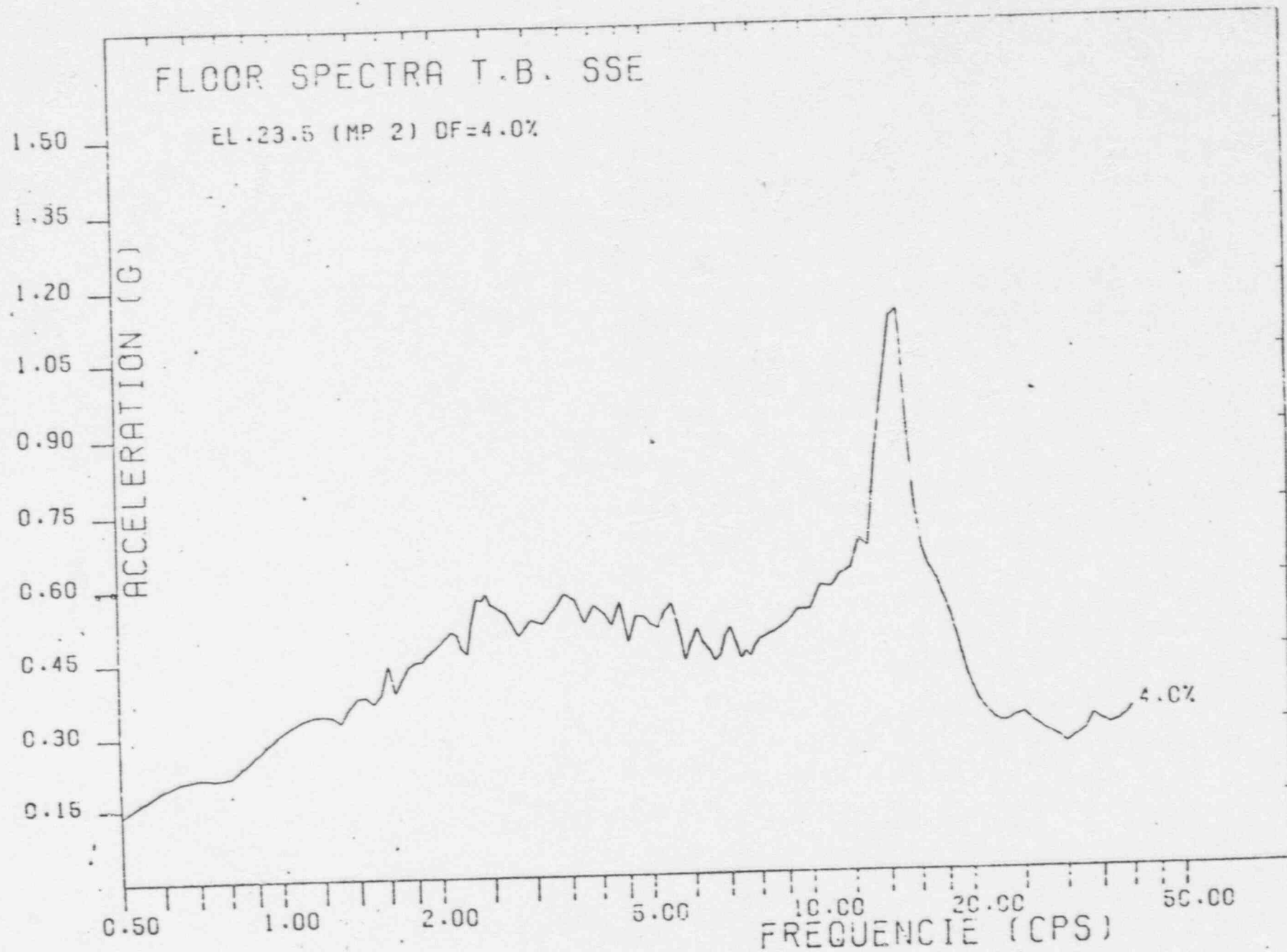
50.00

FREQUENCY (CPS)

2.0%





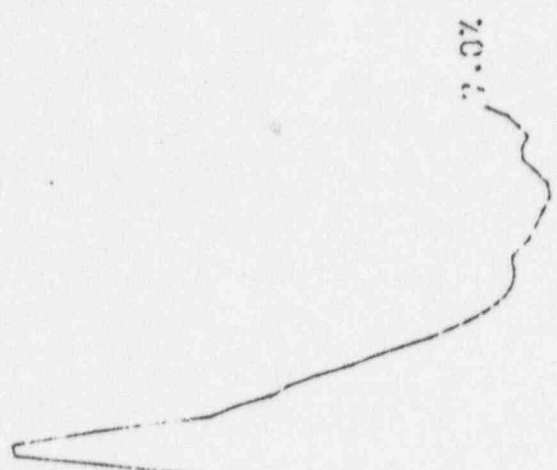


FLOOR SPECTRA T.B. SSE

23.5 (MF 2) DF=7.3%

ACCELERATION (G)

1.00
0.90
0.80
0.70
0.60
0.50
0.40
0.30
0.20
0.10



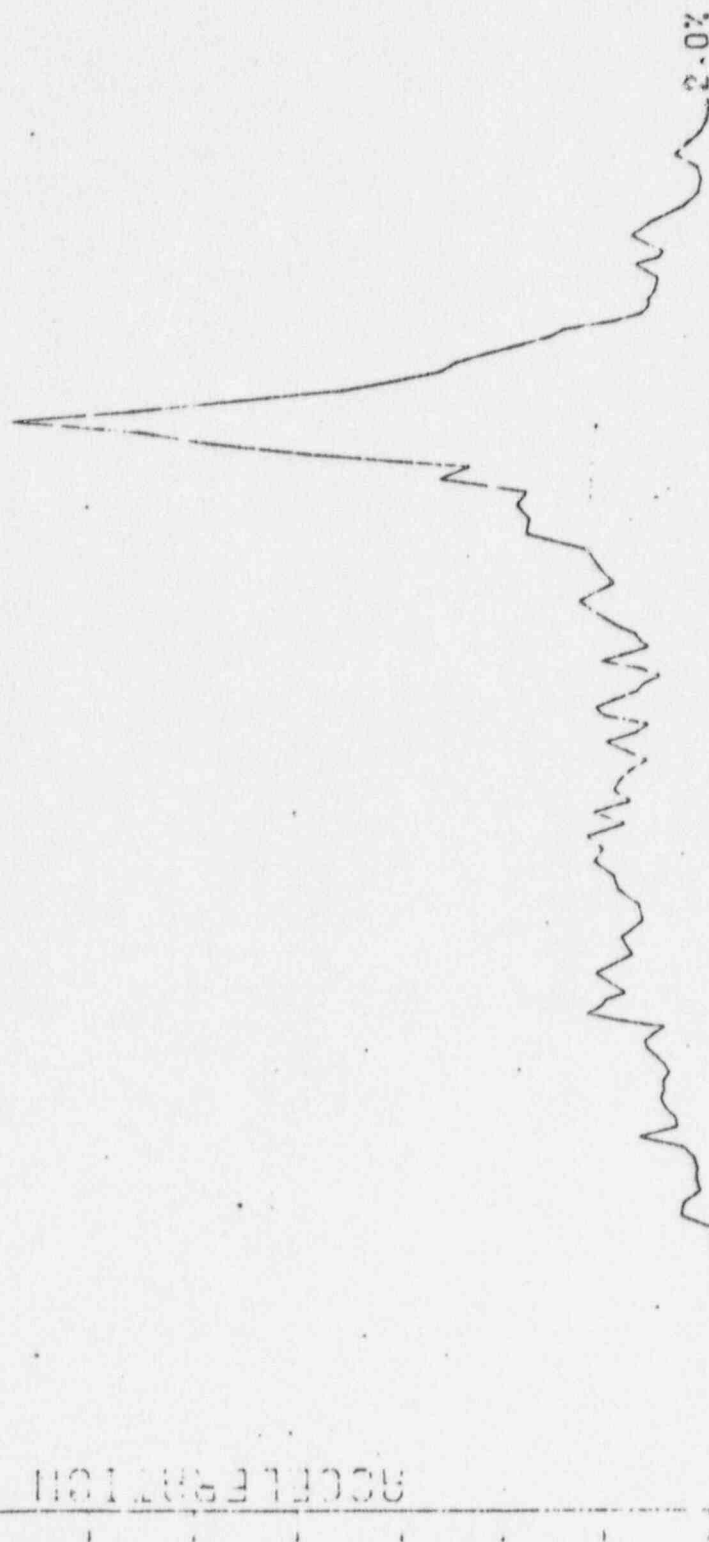
FREQUENCY (CPS)
50.00
20.00
10.00
5.00
2.00
1.00
0.50

FLOOR SPECTRA T.S. 33E

EL. 48.5 (MP) OF 2.0%

SOCKET POSITION (G)

2.00 -
1.80 -
1.60 -
1.40 -
1.20 -
1.00 -
0.80 -
0.60 -
0.40 -
0.20 -



50.00

20.00

10.00

5.00

2.00

1.00

0.50

FREQUENCY (CPS)

FLOOR SPECTRA T.S. JRE

11.45.0 (22.1) DF=4.37

ACCELERATION (G)

1.00
0.90
0.80
0.70
0.60
0.50
0.40
0.30
0.20
0.10

0.50

1.00

2.00

5.00

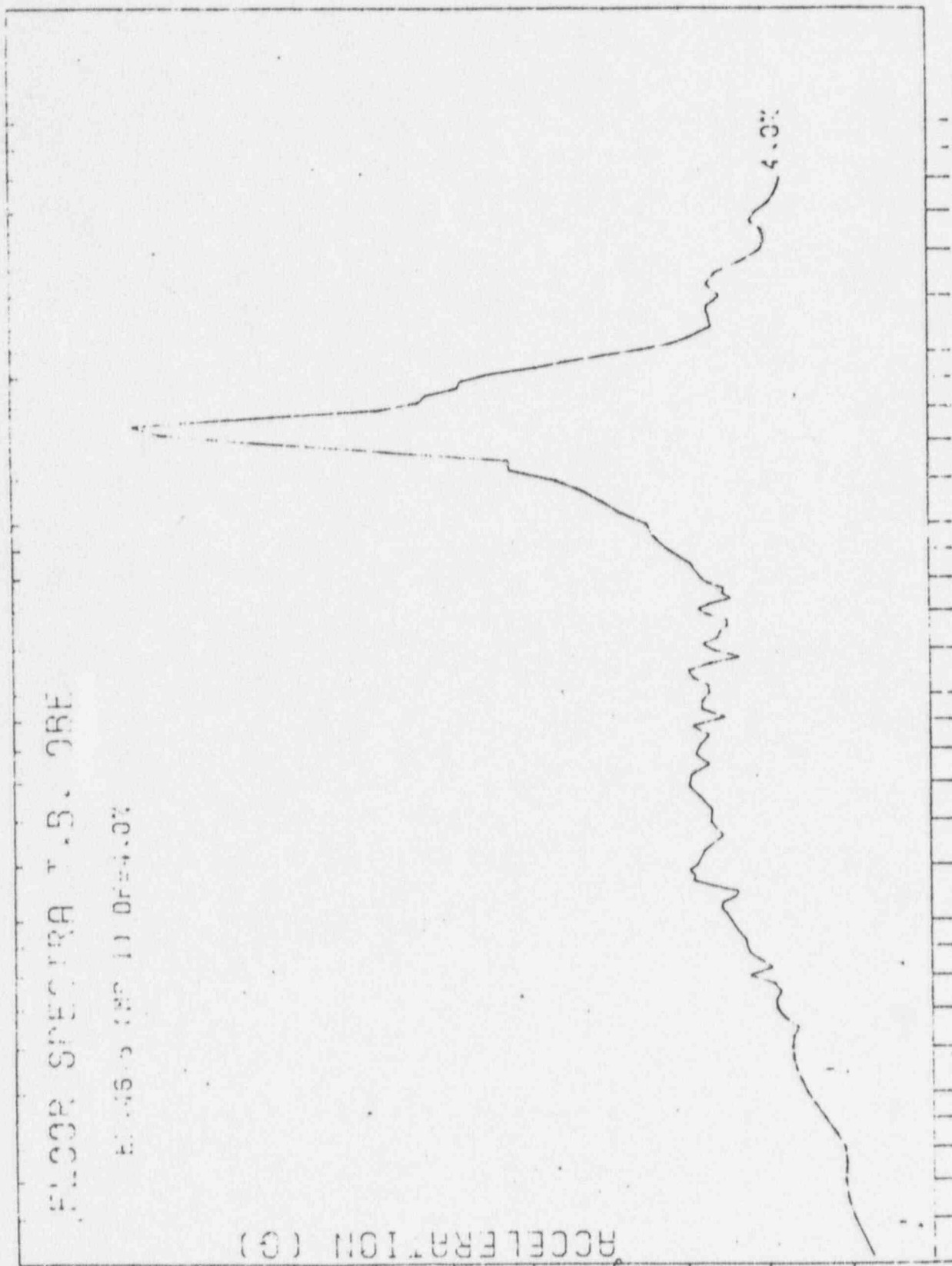
10.00

20.00

50.00

FREQUENCY (CPS)

4.37



FLOOR SPECTRA T.B. SSE

EL. 46.5 (MP 1) DF=4.0%

ACCELERATION (G)

FREQUENCY (CPS)

