

52-001

October 14, 1992

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U.S Nuclear Regulatory Commission
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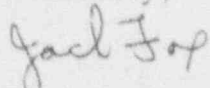
Dear Shou:

Enclosed are modified responses to the Piping Design Audit open items A-6 and A-26. Also included are the calculational summary for the SRV-quencher and pedestal weld stress analysis corresponding to open item A-4. In addition, a response is provided for SER item 5.18.

The remainder of the outstanding responses, with the exception of the open item A-18, should be available for transmittal to you next week. We are awaiting the piping environmental fatigue methodology for which Dave indicated the staff will take the lead.

If you have any questions, please call me (408-925-4824) or Maryann Herzog (408-925-1921).

Sincerely,



Jack N. Fox
Advanced Reactor Programs

cc: Chet Poslusny (NRC)
Giuliano DeGrassi (BNL)

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A PDR

DO50

GE-NE
ABWR PROGRAM
MECHANICAL SYSTEM DESIGN
FILE MH-A

DISTRIBUTION:
JBK, JW, EOS

DATE : OCTOBER 5, 1992

TO : M HERZOG
✓JACK N FOX

FROM : Henry Hwang

SUBJECT : SRV-QUENCHER AND PEDESTAL WELD STRESS ANALYSIS
(Response to NRC/BNL audit comment)

1.0 INTRODUCTION

The quencher hub and the pedestal welded region have been analyzed using ANSYS program. The configuration of the analysis model is shown in Figures 1 and 2. The analysis has two purposes. The first purpose is to calculate the effect of the SRV blowdown transient. The second purpose is to calculate the effect of the geometrical discontinuity at the quencher hub and the pedestal weld.

Since the SRV blowdown transient is the major temperature transient and has the major contribution to the fatigue usage factor. The quencher hub has the most severe effect due to the transient. The reason is that the hub is thicker than wetwell piping. Wetwell piping has more severe transient than drywell piping because stainless pipe has smaller thermal conductivity.

It is important to note that lug attachment should not be used for all the SRV pipings, otherwise, Code Case N-122 should be used for the detailed fatigue analysis which includes the thermal transient analysis of the pipe with the lug.

2.0 HEAT TRANSFER ANALYSIS

The heat transfer transient during relief valve blowdown create temperature transient with step change from 20 deg. C (68 F) to 166 deg. C (330 deg F) inside the quencher. At the outside of the quencher the water temperature is assumed to be the same as the air temperature of 20 deg. C. The transients are plotted in the figure below.

Figure 3a : Temperature transients.

The application of the heat transfer transients to the surface is shown in the figure below:

Figure 3b : Heat transfer surfaces

The heat transfer coefficients inside the quencher is calculated as follow:

assume $V=100$ ft

$$\begin{aligned} Re &= 100 \text{ (ft/sec)} \times 2 \text{ (ft)} / (1.06E-5 \text{ sq ft/sec}) \\ &= 1.88E7 \\ Pr &= 6.82 \\ h_1 &= 0.023(k/D) (Re)^{0.8} (Pr)^{0.33} \\ &= 0.023(0.347/2) (1.88E7)^{0.8} (6.82)^{0.33} \\ &= 0.023(0.173) (659669) (1.895) \\ &= 4975 \text{ use } 5000 \text{ btu/(hr-ft}^2\text{-F)} \end{aligned}$$

h_2 for outside surface:

Assume $V=2.5$ ft/sec

$$(h_0 D_o/k_f) = 1.1 (C) Pr^{0.31} (VD/\nu)^n$$

$$\begin{aligned} Re &= 2.5 \text{ (ft/sec)} \times 2 \text{ (ft)} / (1.06E-5 \text{ sq ft/sec}) \\ &= 4.7E5 \\ Pr &= 6.82 \\ C &= 0.0239 \text{ for } Re = 40000 \text{ to } 400,000 \\ n &= 0.805 \end{aligned}$$

$$\begin{aligned} h_2 &= (k/D) (1.1) (0.0239) 6.82^{0.31} * 470000^{0.805} \\ &= (0.173) (1.1) (0.0239) (1.81) (378350) \\ &= 3123 \text{ btu/hr-ft}^2 \text{ F} \end{aligned}$$

Inside natural convection heated surface face downward

$$h_L/k = 0.27(Gr Pr)^{0.25}$$

$$\begin{aligned} h_3 &= (0.347/2) * (0.27) * (3460,000 * 250 * 2^{0.25} * 6.82)^{0.25} \\ &= (0.174) (0.27) * (466) \\ &= 21.89 \text{ btu/hr-ft}^2\text{-deg F} \end{aligned}$$

The heat transfer outside the quencher:

Assume the water flow outside the pool is 2.5 ft/sec

$$h_2 = 1.1 (k/D) C Pr^{0.31} (VD/\mu)^n$$

$$\begin{aligned} n &= 0.805 \\ C &= 0.0239 \\ Pr &= 6.82 \end{aligned}$$

Substitute the values gives $h_c = 3123 \text{ Btu/hr-ft}^2\text{-F}$

The natural convection inside the pedestal is as follows:

$$\begin{aligned} h_3 &= 0.27(k/L) (Gr Pr)^{0.25} \\ &= 21.89 \text{ btu/hr-ft}^2\text{-F} \end{aligned}$$

The boundary condition of the quencher is considered to be the most severe condition in the SRV discharge system. The reason is that the water outside the quencher has cooling effect and the steam inside has heating effect.

The results of the analysis is shown in the following figures:

Figure 4 : Temperature distribution. degree C
(at time 1.00 min. after SRV blowdown)

Figure 5 : Temperature distribution through section A-A
(nodes 66-115, 1.00 min after SRV blowdown)

Figure 6 : Temperature distribution. degree C
(at time 2.00 min. after SRV blowdown)

Figure 7 : Temperature distribution through section A-A
(nodes 66-115, 2.00 min. after SRV blowdown)

Figure 8 : Temperature distribution through section B-B
(nodes 149-144, 2.00 min after SRV blowdown)

Figure 9 : Temperature distribution. degree C
(at time 3.00 min. after SRV blowdown)

Figure 10 : Temperature distribution through section A-A
(nodes 66-115, 3.00 min after SRV blowdown)

Figure 11 : Temperature distribution through section B-B
(nodes 149-144, 3.00 min after SRV blowdown)

3.0 STRESS ANALYSIS AND FATIGUE ANALYSIS

ANSYS program STIFF 42 axi-symmetric element is used for the stress analysis for all the temperature distributeion at 1.0, 2.0 and 3.0 minutes after SRV blowdown. The calculation of mechanical load input to the analysis are as follows:

Mechanical load calculations

quencher hub $R_o = 304.8$ mm
 $t = 55.1$ mm
 $R_i = 249.7$ mm
 $ID = 4$
 $OD =$
 $A_i = 1979$ mm sq

a) Pressure Load:

$$\begin{aligned}(P)(A_i) &= (0.38)(195878) \\ &= 74,434 \text{ kg} \\ F_p &= 23,693 \text{ kg/rad}\end{aligned}$$

Force is distributed among Nodes, 186, 187, 188, 189, 190, 191

$$\begin{aligned}F_{p, 187, 188, 189, 190} &= 23,693/5 \\ &= 4,739 \text{ lb}\end{aligned}$$

$$F_{p, 186, 191} = 2,370 \text{ lb}$$

b) 10 ton-meter unit moment load :

$$\begin{aligned}R_{avg} &= 277.25 \text{ mm} \\ I &= (3.1416/4) (304.8^4 - 249.7^4) \\ &= 3.725E9 \\ Z &= 3.725E9/304.8 \\ &= 12.22E06\end{aligned}$$

$$\begin{aligned}S_b &= M/Z \\ &= 1000000/12.22E6 \\ &= 0.08182 \text{ kg/mm}^2\end{aligned}$$

$$\begin{aligned}A_{metal} &= 3.1416(304.8^2 - 249.7^2) \\ &= 95985 \text{ mm}^2\end{aligned}$$

$$\begin{aligned}\text{Force per radian} &= 0.08182 \times (95985/6.2832) \\ &= 1250 \text{ kg/rad for 1 m-t moment}\end{aligned}$$

The input value is to 10 m-t as unit load input

$$\begin{aligned}F_{p, 187, 188, 189, 190} &= 10(\text{m-t}) \times 1250/5 \\ &= 2,500 \text{ lb/node-radian}\end{aligned}$$

$$F_{p, 186, 191} = 1.250 \text{ lb}$$

c) Shear load 10 ton unit load

10 ton as unit load input

$$\begin{aligned} F_p, 187, 188, 189, 190 &= 10,000 / (6.2832 \times 5) \\ &= 318 \text{ lb/node-radian} \end{aligned}$$

$$F_p, 186, 191 = 159 \text{ lb lb}$$

The pressure load, unit moment and unit force loads have also been calculated for load combinations. The stress intensity calculation are performed in accordance with ASME Section NB-3214 procedures. NB-3216.2 fatigue analysis procedures are used for Fatigue analysis.

d) Apply Load

The forces and moments due to thermal expansion is used for the fatigue analysis. It is conservative to combine torsion to the bending moment and the axial force to shear force for the finite element model. The shear force and moment due to thermal expansion are as follows:

Force = 10 tons
Moment = 6 meter-tons

The forces and moments for the primary load combinations are as follows. These forces and moments are obtained from calculated results of one line. Enveloping process for all the line has not been performed.

	Force(tons)	Moment(m-t)
Design	10.0	1.0
Upset	30.0	25.0
Level C	30.0	25.0
Level D	30.0	40.0

4.0 RESULTS

Thermal transient due to blowdown, pressure, external forces and moments for each service level have been included in the analyses. The results are tabulated below:

		Node	Calc'd Value (kg/mm ²)	Allowable (kg/mm ²)
Primary membrane	Maximum	150	1.7	10.3
	Weld	61	1.0	10.3
Primary membrane plus bending	Maximum	150	4.6	15.5
	Weld	66	3.4	15.5

Fatigue usage for quencher with low-low set SRV valve has 3064x1.5=4596 valve actuations for 60 years. These quenchers without low-low set SRV valve has 264x1.5=396 valve actuations for 60 years.

		Node		Usage Calc'd	Allowable Usage
Fatigue usage (with low-low set)	Maximum	144	4596	0.5380	1.0
	Weld	61	4596	0.0180	1.0
Fatigue usage (with ^{out} low-low set)	Maximum	144	396	0.0470	1.0
	Weld	61	396	0.0016	1.0

The attached Table 5-1 and 5-2 show the detailed results at various nodes for reference. The usage factors tabulated in Tables 5-2 are for these quenchers with 1800 valve actuation.

In general the fatigue usage factor due to the blowdown transient at the inside surface of the hub are higher than the weld which is at the outside surface. The stress intensification factor used in the analysis for the weld is 2.4, which is conservative because the finite element analysis results has included stress concentration due to geometry change. A factor of 1.1 was used for all other locations for conservatism.

5.0 CONCLUSION

The fatigue usage factor inside surface is higher than the outside surface of the quencher.

If the thermal expansion moment at the quencher is less than 6 m-t and the number of valve actuations is less a than 4596 the fatigue usage factor will be less than 0.60.

The fatigue usage factor for other locations of the safety relief valve discharge piping depends on the expansion moment and are not covered by this analysis.

TABLE 5-1 PRIMARY STRESS INTENSITY

Parts	Point	Material	Combination	Primary Membrane			Primary Membr+Bend		
				Limits	Intensity	Limits Evaluation	Intensity	Limits	Evaluation
ABWR-SRV N150	1	N150	Design	DESIGN	1.7	10.3	N150	2.3	15.5
			Operation II	OCCAS	1.7	10.3	N150	4.6	15.5
			Operation III	IIIAS	1.7	15.5	N150	4.6	23.2
			Operation IV	IVAS	1.7	21.6	N150	5.9	32.4
ABWR-QUEN N155	1	N155	Design	DESIGN	1.7	10.3	N155	1.4	15.5
			Operation II	OCCAS	1.7	10.3	N155	3.1	15.5
			Operation III	IIIAS	1.7	15.5	N155	3.1	23.2
			Operation IV	IVAS	1.7	21.6	N155	4.4	32.4
ABWR-QUEN N144	1	N144	Design	DESIGN	1.6	10.3	N144	2.1	15.5
			Operation II	OCCAS	1.6	10.3	N144	4.2	15.5
			Operation III	IIIAS	1.6	15.5	N144	4.2	23.2
			Operation IV	IVAS	1.6	21.6	N144	5.3	32.4
ABWR-QUEN N149	1	N149	Design	DESIGN	1.6	10.3	N149	1.3	15.5
			Operation II	OCCAS	1.6	10.3	N149	3.1	15.5
			Operation III	IIIAS	1.6	15.5	N149	3.1	23.2
			Operation IV	IVAS	1.6	21.6	N149	4.2	32.4
ABWR-QUEN N138	1	N138	Design	DESIGN	1.5	10.3	N138	1.9	15.5
			Operation II	OCCAS	1.5	10.3	N138	3.8	15.5
			Operation III	IIIAS	1.5	15.5	N138	3.8	23.2
			Operation IV	IVAS	1.5	21.6	N138	5.0	32.4
ABWR-QUEN N211	1	N211	Design	DESIGN	1.5	10.3	N211	1.2	15.5
			Operation II	OCCAS	1.5	10.3	N211	2.9	15.5
			Operation III	IIIAS	1.5	15.5	N211	2.9	23.2
			Operation IV	IVAS	1.5	21.6	N211	4.1	32.4
ABWR-QUEN N132	1	N132	Design	DESIGN	1.3	10.3	N132	1.8	15.5
			Operation II	OCCAS	1.3	10.3	N132	3.5	15.5
			Operation III	IIIAS	1.3	15.5	N132	3.5	23.2
			Operation IV	IVAS	1.3	21.6	N132	4.5	32.4

TABLE 5.1 PRIMARY STRESS INTENSITY

Parts	Point	Material	Combination	Primary Membrane			Primary Heat-Bond		
				Limits	Intensity	Limits	Intensity	Limits	Evaluation
ABWR-QUEN N206	1	N206	Design	DESIGN	1.3	10.3	N206-	1.0	15.5
			Operation II	OCCAS	1.3	10.3	N206-	2.6	15.5
			Operation III	IIIAS	1.3	15.5	N206-	2.6	23.2
			Operation IV	IVAS	1.3	21.6	N206-	3.7	32.4
ABWR-QUEN N120	1	N120	Design	DESIGN	1.0	10.3	N120-	1.8	15.5
			Operation II	OCCAS	1.0	10.3	N120-	2.3	15.5
			Operation III	IIIAS	1.0	15.5	N120-	2.3	23.2
			Operation IV	IVAS	1.0	21.6	N120-	2.6	32.4
ABWR-QUEN N125	1	N125	Design	DESIGN	1.0	10.3	N125-	0.5	15.5
			Operation II	OCCAS	1.0	10.3	N125-	1.2	15.5
			Operation III	IIIAS	1.0	15.5	N125-	1.2	23.2
			Operation IV	IVAS	1.0	21.6	N125-	1.6	32.4
ABWR-QUEN N115	1	N115	Design	DESIGN	1.0	10.3	N115-	1.7	15.5
			Operation II	OCCAS	1.0	10.3	N115-	2.0	15.5
			Operation III	IIIAS	1.0	15.5	N115-	2.0	23.2
			Operation IV	IVAS	1.0	21.6	N115-	2.2	32.4
ABWR-QUEN N 61	1	N 61	Design	DESIGN	1.0	10.3	N 61-	0.1	15.5
			Operation II	OCCAS	1.0	10.3	N 61-	0.8	15.5
			Operation III	IIIAS	1.0	15.5	N 61-	0.8	23.2
			Operation IV	IVAS	1.0	21.6	N 61-	1.1	32.4
ABWR-QUEN N109	1	N109	Design	DESIGN	0.9	10.3	N109-	1.6	15.5
			Operation II	OCCAS	0.9	10.3	N109-	1.6	15.5
			Operation III	IIIAS	0.9	15.5	N109-	1.6	23.2
			Operation IV	IVAS	0.9	21.6	N109-	1.7	32.4
ABWR-QUEN N114	1	N114	Design	DESIGN	0.9	10.3	N114-	0.3	15.5
			Operation II	OCCAS	0.9	10.3	N114-	0.4	15.5
			Operation III	IIIAS	0.9	15.5	N114-	0.4	23.2
			Operation IV	IVAS	0.9	21.6	N114-	0.6	32.4

TABLE 5-1 PRIMARY STRESS INTENSITY

Parts	Point	Material	Combination	Primary Membrane			Primary Bending		
				Limits	Intensity	Limits Evaluation	Intensity	Limits	Evaluation
ABWR-QUEN N125	1	N125	Design	DESIGN	1.0	10.3	N125	1.1	15.5
			Operation II	OCCAS	1.0	10.3	N125	1.4	15.5
			Operation III	IIIAS	1.0	15.5	N125	3.4	23.2
			Operation IV	IVAS	1.0	21.6	N125	4.9	32.4
ABWR-QUEN N196	1	N196	Design	DESIGN	1.0	10.3	N196	2	15.5
			Operation II	OCCAS	1.0	10.3	N196	1.5	15.5
			Operation III	IIIAS	1.0	15.5	N196	1.5	23.2
			Operation IV	IVAS	1.0	21.6	N196	5.0	32.4
ABWR-QUEN N 61	1	N 61	Design	DESIGN	1.0	10.3	N 61	1.1	15.5
			Operation II	OCCAS	1.0	10.3	N 61	1.4	15.5
			Operation III	IIIAS	1.0	15.5	N 61	3.4	23.2
			Operation IV	IVAS	1.0	21.6	N 61	4.8	32.4
ABWR-QUEN N 66	1	N 66	Design	DESIGN	1.0	10.3	N 66	1.1	15.5
			Operation II	OCCAS	1.0	10.3	N 66	1.4	15.5
			Operation III	IIIAS	1.0	15.5	N 66	3.4	23.2
			Operation IV	IVAS	1.0	21.6	N 66	4.8	32.4
ABWR-QUEN N 55	1	N 55	Design	DESIGN	0.9	10.3	N 55	1.2	15.5
			Operation II	OCCAS	0.9	10.3	N 55	1.4	15.5
			Operation III	IIIAS	0.9	15.5	N 55	3.4	23.2
			Operation IV	IVAS	0.9	21.6	N 55	4.8	32.4
ABWR-QUEN N 60	1	N 60	Design	DESIGN	0.9	10.3	N 60	0.8	15.5
			Operation II	OCCAS	0.9	10.3	N 60	1.1	15.5
			Operation III	IIIAS	0.9	15.5	N 60	3.1	23.2
			Operation IV	IVAS	0.9	21.6	N 60	4.5	32.4
ABWR-QUEN N001	1	N001	Design	DESIGN	0.6	10.3	N001	0.5	15.5
			Operation II	OCCAS	0.6	10.3	N001	1.9	15.5
			Operation III	IIIAS	0.6	15.5	N001	1.9	23.2
			Operation IV	IVAS	0.6	21.6	N001	2.8	32.4

TABLE 5-1 PRIMAP: STRESS INTENSITY

Parts	Point	Material	Combination	Primary Membrane			Primary Membrane Bend		
				Limits	Intensity	Limits	Intensity	Limits	Evaluation
ABWR-QUEEN N006		N006	Design	DESIGN	0.6	10.3	N006	0.7	15.5
			Operation II	OCCAS	0.6	10.3	N006	2.2	15.5
			Operation III	IIIAS	0.6	15.5	N006	2.2	23.2
			Operation IV	IVAS	0.6	21.6	N006	3.1	32.3

TABLE 5-2 PRIMARY PLUS SECONDARY STRESS INTENSITY RANGE + FATIGUE

Paris	Point	Material	Combination	Primary + Secondary Stress		Fatigue Analysis	
				Limits	Intensity Limits Evaluation	Usage Fac	Limits Evaluation
ABWR SRV N150	N150 1		Operation 1A, 11A	1811A 37.4	30.9 N150	0.105	1.000 N150
ABWR QUEN N155	N155 1		Operation 1A, 11A	1811A 33.2	30.9 N155	0.005	1.000 N155
ABWR QUEN N144	N144 1		Operation 1A, 11A	1811A 38.3	30.9 N144	0.211	1.000 N144
ABWR QUEN N149	N149 1		Operation 1A, 11A	1811A 27.1	30.9 N149	0.003	1.000 N149
ABWR QUEN N138	N138 1		Operation 1A, 11A	1811A 36.0	30.9 N138	0.110	1.000 N138
ABWR QUEN N211	N211 1		Operation 1A, 11A	1811A 25.5	30.9 N211	0.003	1.000 N211
ABWR QUEN N132	N132 1		Operation 1A, 11A	1811A 33.2	30.9 N132	0.042	1.000 N132
ABWR QUEN N206	N206 1		Operation 1A, 11A	1811A 24.2	30.9 N206	0.003	1.000 N206

TABLE 5-2 PRIMARY PLUS SECONDARY STRESS INTENSITY RANGE * FATIGUE

Parts	Point	Material	Combination	Primary+Secondary Stress		Fatigue Analysis	
				Limits	Intensity Limits Evaluation	Usage Fac	Limits Evaluation
ABWR-QUEN N120	1		Operation 1A, 11A	1811A 36.3	30.9 N120	0.165	1.000 N120
ABWR-QUEN N125	1		Operation 1A, 11A	1811A 22.5	30.9 N125	0.003	1.000 N125
ABWR-QUEN N115	1		Operation 1A, 11A	1811A 35.9	30.9 N115	0.154	1.000 N115
ABWR-QUEN N 61	1		Operation 1A, 11A	1811A 26.3	30.9 N 61	0.007	1.000 N 61
ABWR-QUEN N109	1		Operation 1A, 11A	1811A 36.6	30.9 N109	0.206	1.000 N109
ABWR-QUEN N114	1		Operation 1A, 11A	1811A 24.6	30.9 N114	0.003	1.000 N114
ABWR-QUEN N125	1		Operation 1A, 11A	1811A 10.6	30.9 N125	0.003	1.000 N125
ABWR-QUEN N196	1		Operation 1A, 11A	1811A 15.3	30.9 N196	0.003	1.000 N196

TABLE 5-2 PRIMARY PLUS SECONDARY STRESS INTENSITY RANGE + FATIGUE

Parts	Point	Material	Combination	Primary+Secondary Stress			Fatigue Analysis		
				Limits	Intensity	Limits	Evaluation	Usage Fa	Limits
									Evaluation
ABWR-QUEN N 61	N 61 1		Operation IA, IIA	1811A	14.2	30.9	N 61	0.003	1.000
ABWR-QUEN N 66	N 66 1		Operation IA, IIA	1811A	14.7	30.9	N 66	0.003	1.000
ABWR-QUEN N 55	N 55 1		Operation IA, IIA	1811A	10.6	30.9	N 55	0.003	1.000
ABWR-QUEN N 60	N 60 1		Operation IA, IIA	1811A	7.8	30.9	N 60	0.003	1.000
ABWR-QUEN N001	N001 1		Operation IA, IIA	1811A	0.7	30.9	N001	0.003	1.000
ABWR-QUEN N006	N006 1		Operation IA, IIA	1811A	0.7	30.9	N006	0.003	1.000

TABLE 9 MAXIMUM VALUE SORTING

ELEM	DESIGN M	ELEM	DESIGN B	ELEM	OP	IV M	ELEM	OP	IV B	ELEM	SN10	ELEM	SN13	ELEM	USAG
150	1 7	150	2 3	150		1 7	150		5 9	144	38 3	144	38 3	144	0 2106
155	1 7	144	2 1	155		1 7	144		5 3	150	37 4	150	37 4	150	0 2064
144	1 6	138	1 9	144		1 6	138		5 0	109	36 6	109	36 6	120	0 1649
149	1 6	132	1 8	149		1 6	196		5 0	120	36 3	120	36 3	115	0 1543
138	1 5	120	1 8	138		1 5	125		4 9	138	36 0	138	36 0	118	0 1100
211	1 5	115	1 7	211		1 5	55		4 8	115	35 9	115	35 9	150	0 1055
132	1 3	109	1 6	132		1 3	61		4 8	155	33 2	155	33 2	132	0 0417
206	1 3	155	1 4	206		1 3	66		4 8	132	33 2	132	33 2	61	0 0070
120	1 0	149	1 3	120		1 0	132		4 5	149	27 1	149	27 1	155	0 0051
125	1 0	211	1 2	125		1 0	60		4 5	61	26 3	61	26 3	125	0 0032
115	1 0	196	1 2	115		1 0	155		4 4	211	25 5	211	25 5	149	0 0032
61	1 0	55	1 2	61		1 0	149		4 3	114	24 6	114	24 6	206	0 0032
125	1 0	61	1 1	125		1 0	211		4 1	206	24 2	206	24 2	211	0 0032
196	1 0	66	1 1	196		1 0	206		3 7	125	22 5	125	22 5	114	0 0032
66	1 0	125	1 1	66		1 0	6		3 1	196	15 3	196	15 3	125	0 0032
55	0 9	206	1 0	55		0 9	66		2 8	66	14 7	66	14 7	196	0 0032
60	0 9	60	0 8	60		0 9	120		2 6	61	14 2	61	14 2	61	0 0032
109	0 9	1	0 5	109		0 9	115		2 2	125	10 6	125	10 6	66	0 0032
114	0 9	125	0 5	114		0 9	109		1 7	55	10 6	55	10 6	55	0 0032
1	0 6	61	0 3	1		0 6	125		1 6	60	7 8	60	7 8	60	0 0032
6	0 6	114	0 3	6		0 6	61		1 1	1	0 7	1	0 7	1	0 0032
							114		0 6	6	0 7	6	0 7	6	0 0032

TABLE 2 MAXIMUM VALUE SORTING

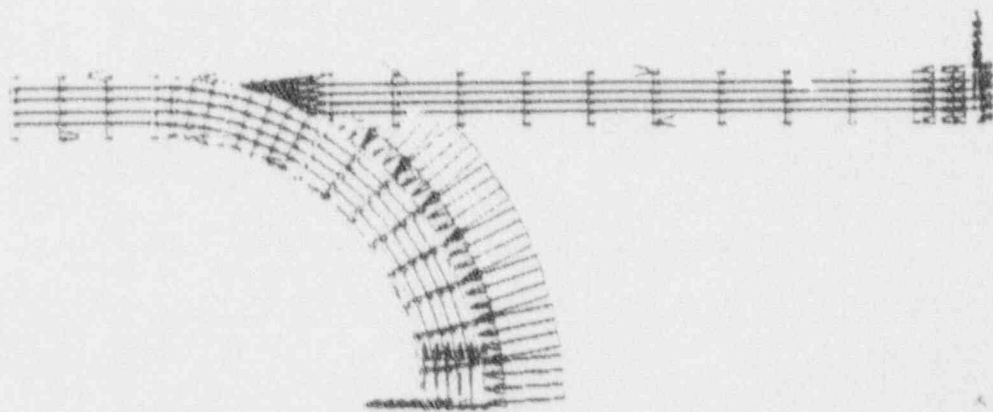
ELEM	OP-11-T	ELEM	OP-11-B	ELEM	OP-03-M	ELEM	OP-03-B	ELEM	OP	V-M	ELEM	OP	V-B	ELEM
150	1.7	150	4.6	150	1.7	150	4.6	150	0.0	0.0	150	0.0	0.0	150
155	1.7	144	4.2	155	1.7	144	4.2	155	0.0	0.0	155	0.0	0.0	155
144	1.6	138	3.8	144	1.6	138	3.8	144	0.0	0.0	144	0.0	0.0	144
149	1.6	196	3.5	149	1.6	196	3.5	149	0.0	0.0	149	0.0	0.0	149
138	1.5	132	3.5	138	1.5	132	3.5	138	0.0	0.0	138	0.0	0.0	138
211	1.5	55	3.4	211	1.5	55	3.4	211	0.0	0.0	211	0.0	0.0	211
132	1.3	125	3.4	132	1.3	125	3.4	132	0.0	0.0	132	0.0	0.0	132
206	1.3	61	3.4	206	1.3	61	3.4	206	0.0	0.0	206	0.0	0.0	206
120	1.0	66	3.4	120	1.0	66	3.4	120	0.0	0.0	120	0.0	0.0	120
125	1.0	155	3.1	125	1.0	155	3.1	125	0.0	0.0	125	0.0	0.0	125
115	1.0	60	3.1	115	1.0	60	3.1	115	0.0	0.0	115	0.0	0.0	115
61	1.0	149	3.1	61	1.0	149	3.1	61	0.0	0.0	61	0.0	0.0	61
125	1.0	211	2.9	125	1.0	211	2.9	109	0.0	0.0	109	0.0	0.0	109
196	1.0	206	2.6	196	1.0	206	2.6	114	0.0	0.0	114	0.0	0.0	114
61	1.0	120	2.3	61	1.0	120	2.3	125	0.0	0.0	125	0.0	0.0	125
66	1.0	6	2.2	66	1.0	6	2.2	196	0.0	0.0	196	0.0	0.0	196
55	0.9	115	2.0	55	0.9	115	2.0	61	0.0	0.0	61	0.0	0.0	61
60	0.9	1	1.9	60	0.9	1	1.9	66	0.0	0.0	66	0.0	0.0	66
109	0.9	109	1.6	109	0.9	109	1.6	55	0.0	0.0	55	0.0	0.0	55
114	0.9	125	1.2	114	0.9	125	1.2	60	0.0	0.0	60	0.0	0.0	60
1	0.6	61	0.8	1	0.6	61	0.8	1	0.0	0.0	1	0.0	0.0	1
6	0.6	114	0.4	6	0.6	114	0.4	6	0.0	0.0	6	0.0	0.0	6

SDRC I-DEAS 4.0: Pre/Post Processing
11-AUG-92 07:41:51
UNITS : GIN
DISPLAY : No stored OPTION
Associated Workset: 1-WORKING SET1

SDRC I-DEAS 4.0: Pre/Post Processing

DATABASE: K-5/7 QUENCHER TO PEDESTAL WELD
VIEW : No stored VIEW
Task: Model Preparation
Model: 1-FF MODEL1

Fig. 1 Quencher Model



SDRC I-DEAS 4.0: Pre/Post Processing

11 AUG 92 08:05:23

DATABASE: X-6-7 QUENCHER TO PEDESTAL WELD

VIEW: No stored VIEW

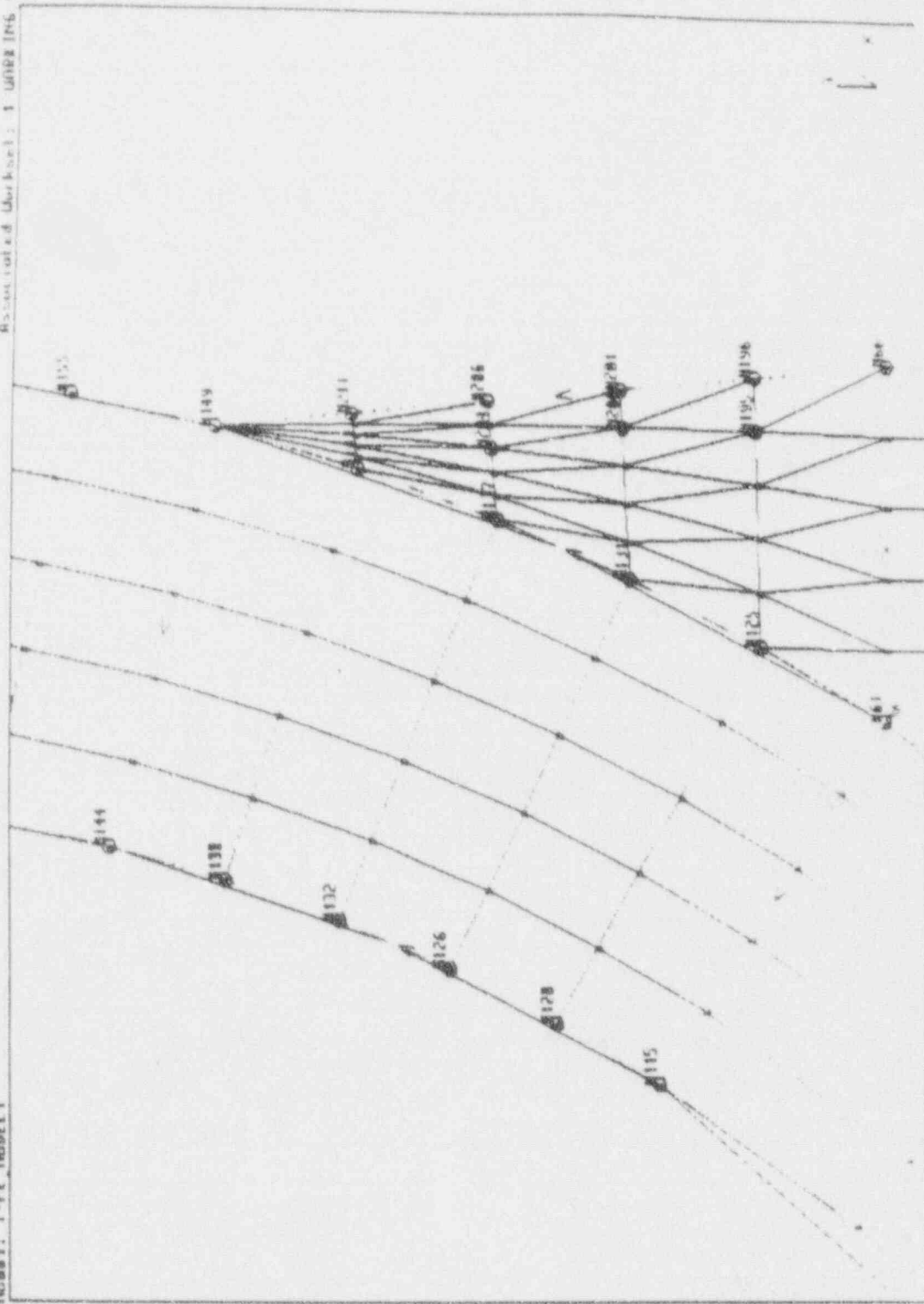
Task: Model Preparation

Model: 1-PE.MDDEL1

DISP: No stored DISP

UNIT: 1

Associated Workset: 1 WORKING SET





NUMBER _____ DATE _____
SUBJECT _____ BY _____ SHEET _____ OF _____

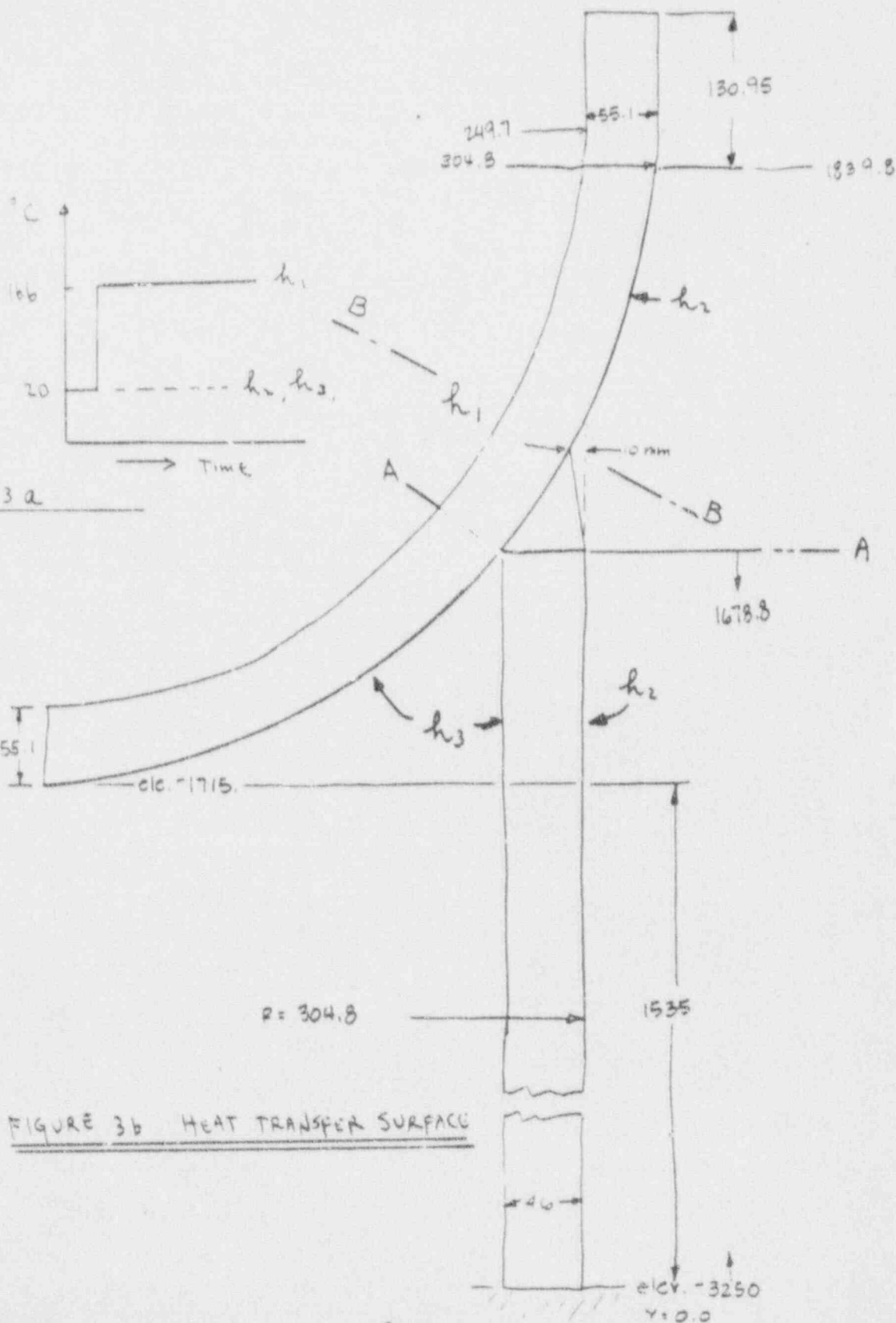


FIGURE 3b HEAT TRANSFER SURFACE

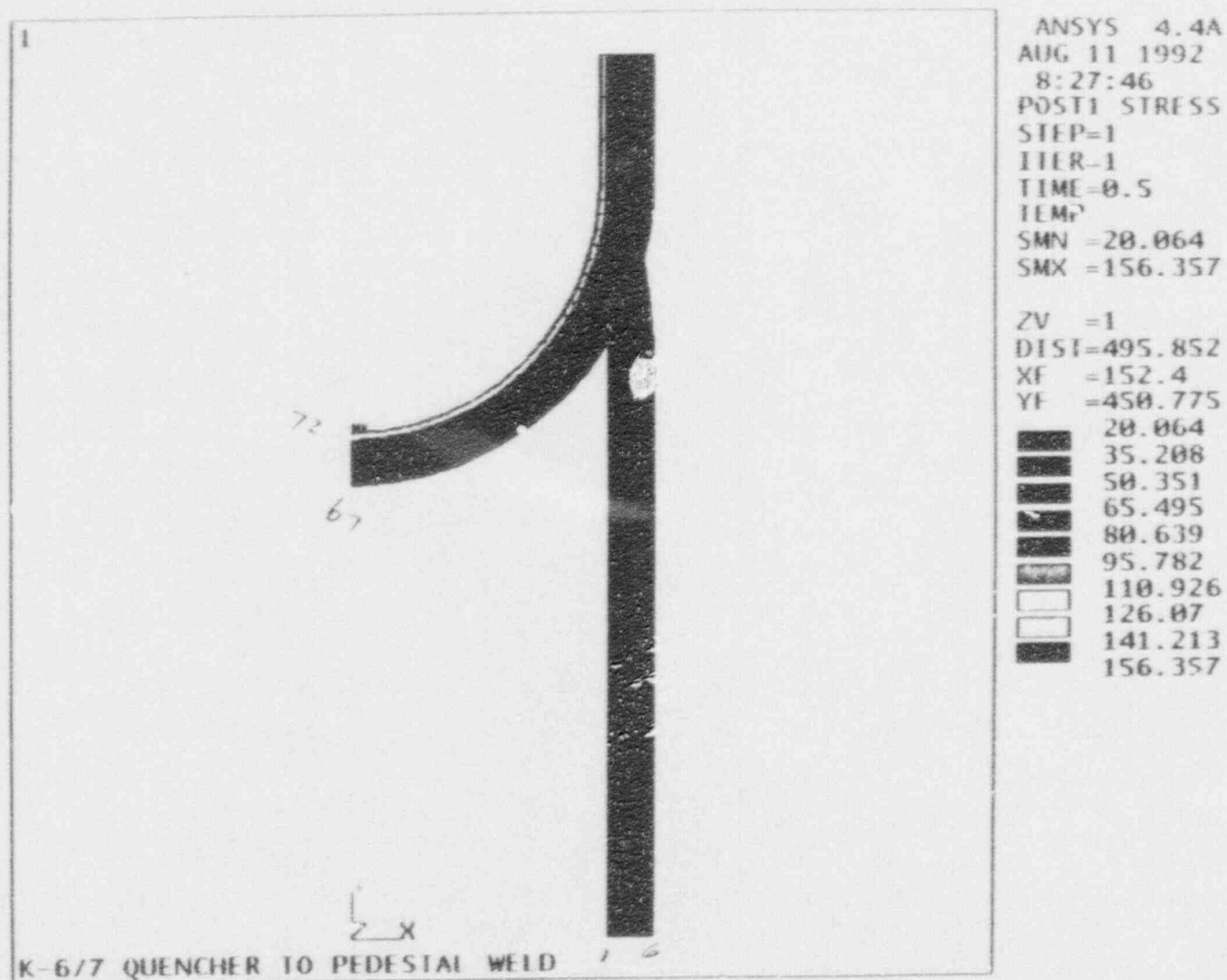


Figure 4 : Temperature distribution. degree C
(at time 1.00 min. after SRV blowdown)

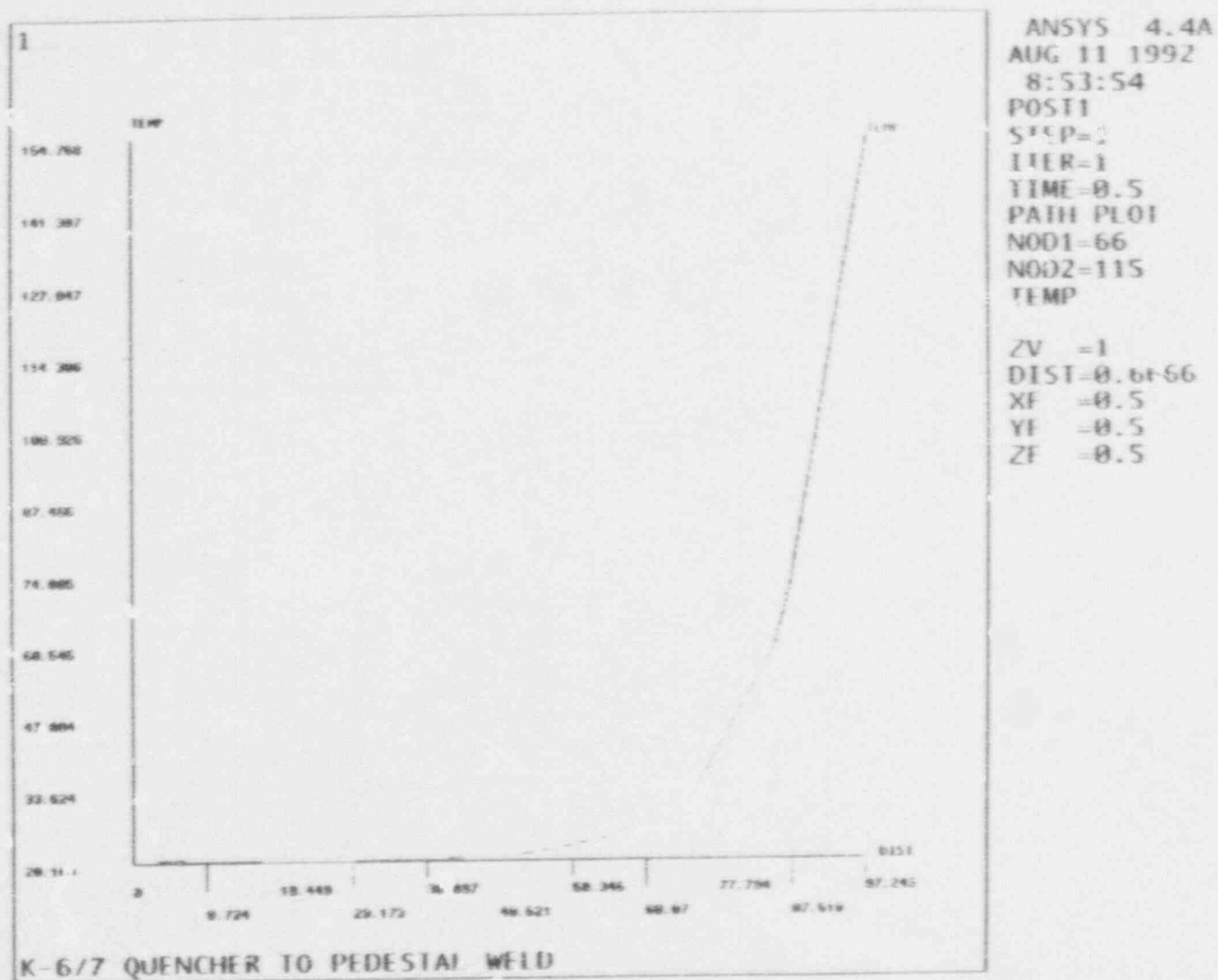


Figure 5 : Temperature distribution through section A-A
 (nodes 66-115, 1.00 min after SRV blowdown)

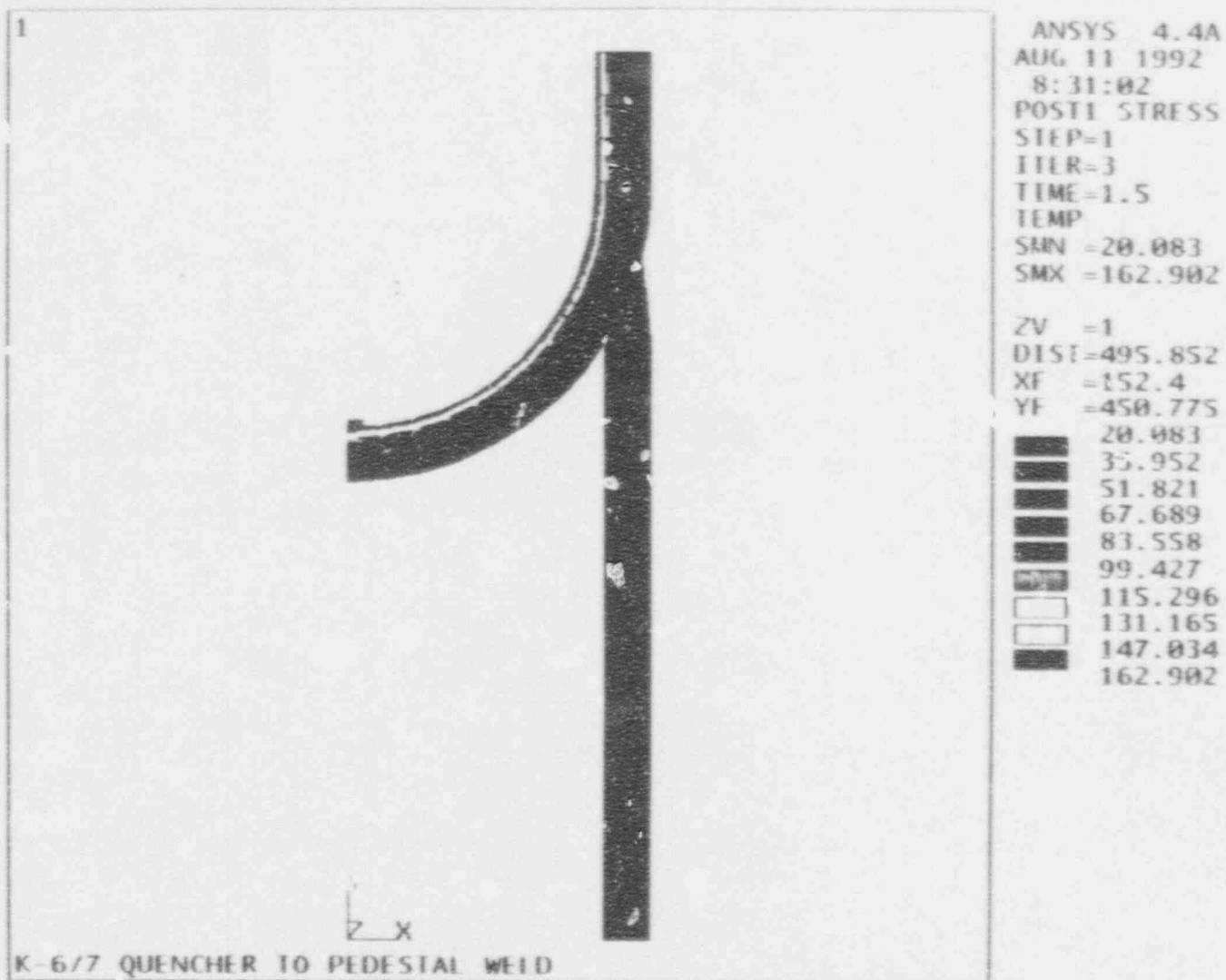
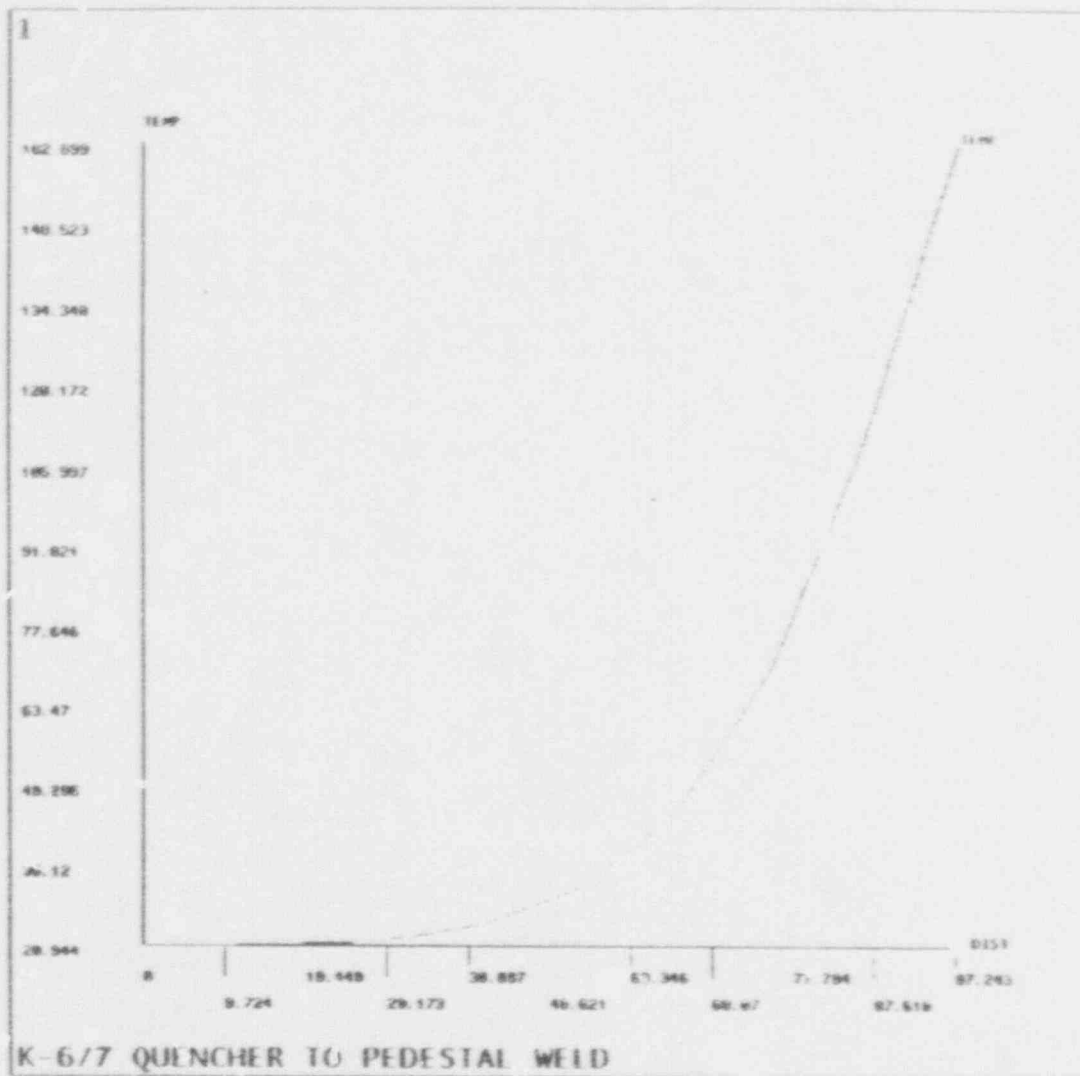


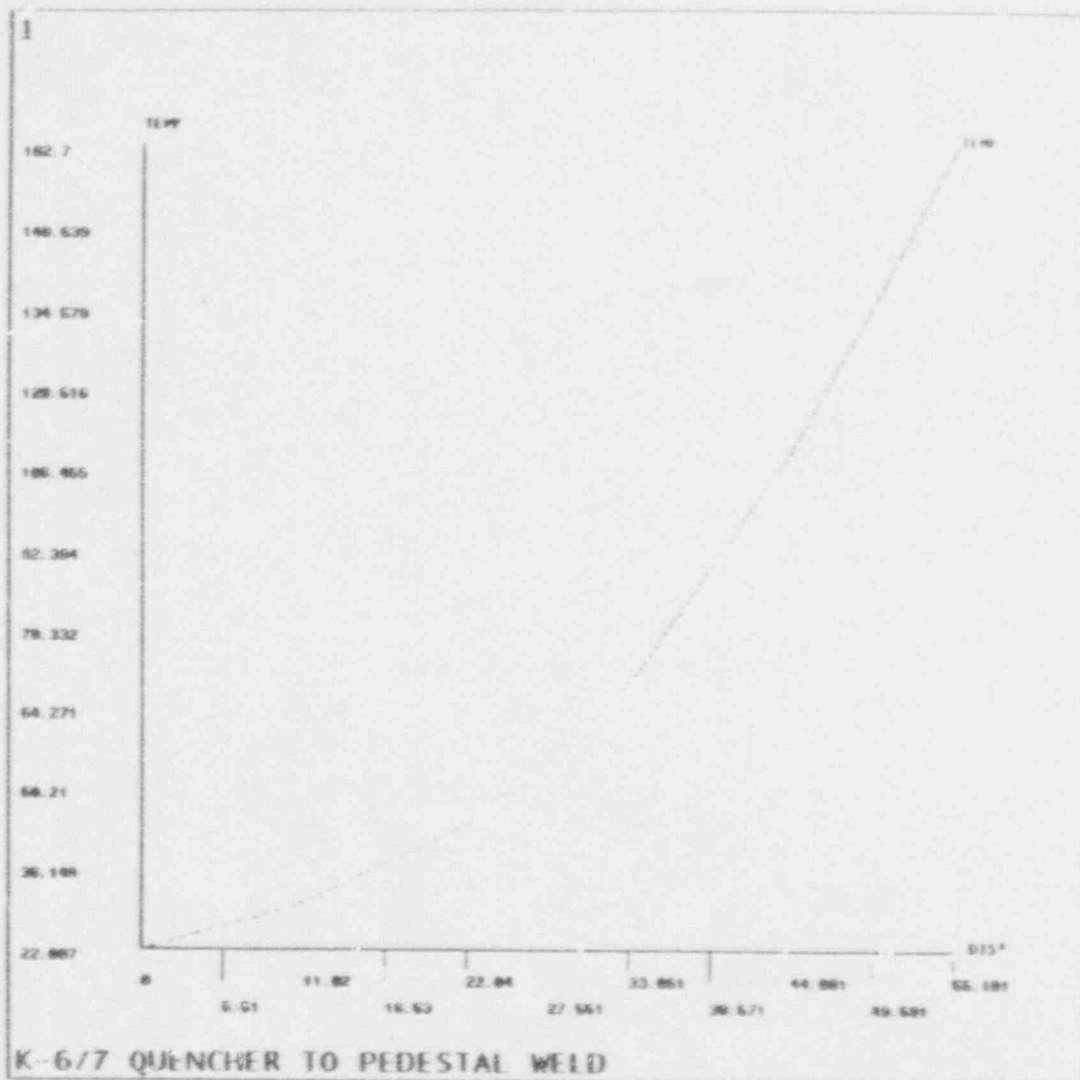
Figure 6 : Temperature distribution, degree C
(at time 2.00 min. after SRV blowdown)



ANSYS 4.4A
 AUG 11 1992
 8:47:00
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 STEP=1
 ITER=3
 TIME=1.5
 PATH PLOT
 NOD1=66
 NOD2=115
 TEMP

ZV =1
 DIST=0.6665
 XF =0.5
 YF =0.5
 ZF =0.5

Figure 7 : Temperature distribution through section A-A
 (nodes 66-115, 2.00 min after SRV blowdown)



ANSYS 4.4A
 AUG 11 1992
 8:50:01
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 STEP=1
 ITER=3
 TIME=1.5
 PATH PLOT
 NOD1=149
 NOD2=144
 TEMP

ZV =1
 DIST=0.6666
 XF =0.5
 YF =0.5
 ZF =0.5

Figure 8 : Temperature distribution through section B-B
 (nodes 149-144, 2.00 min after SRV blowdown)

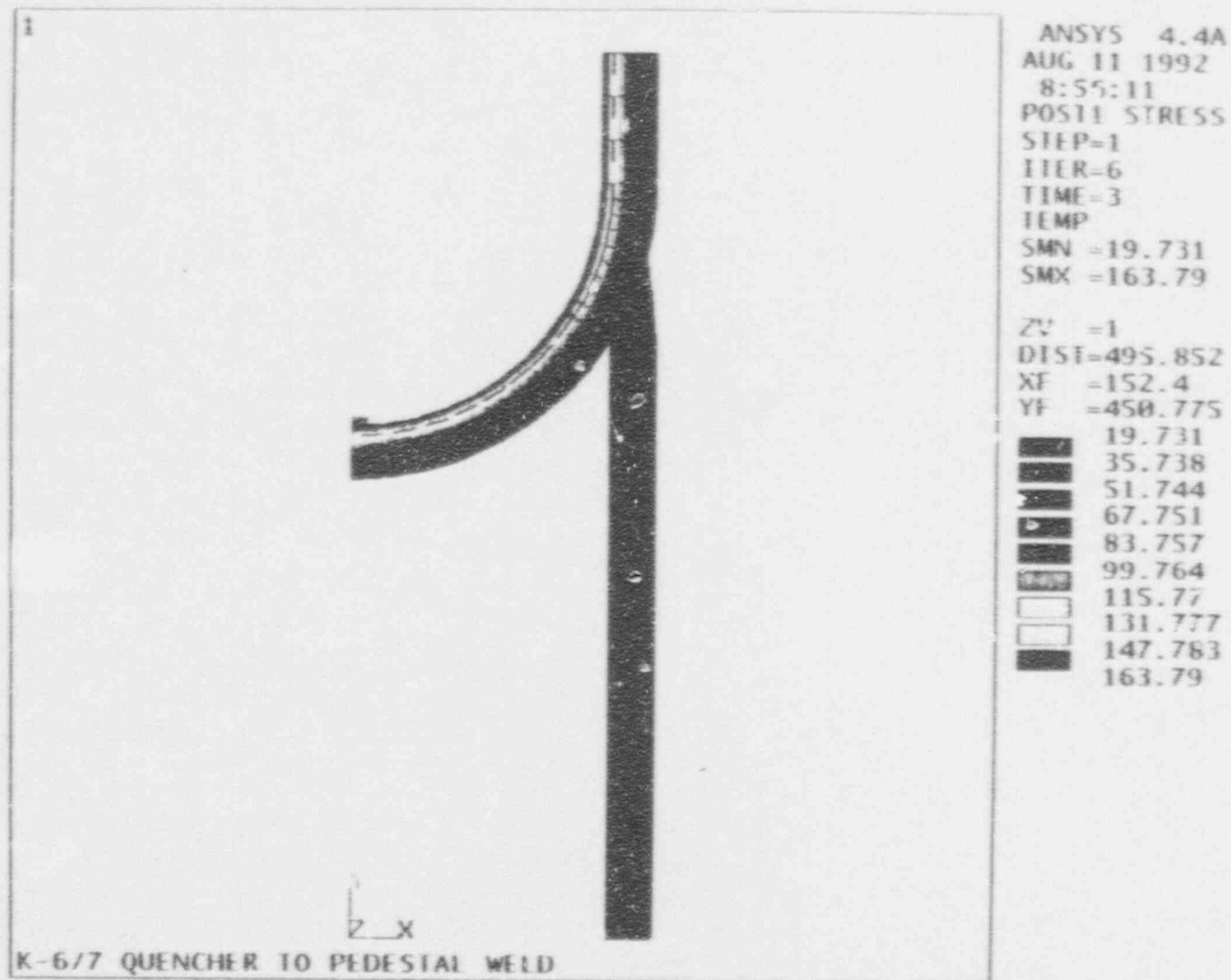
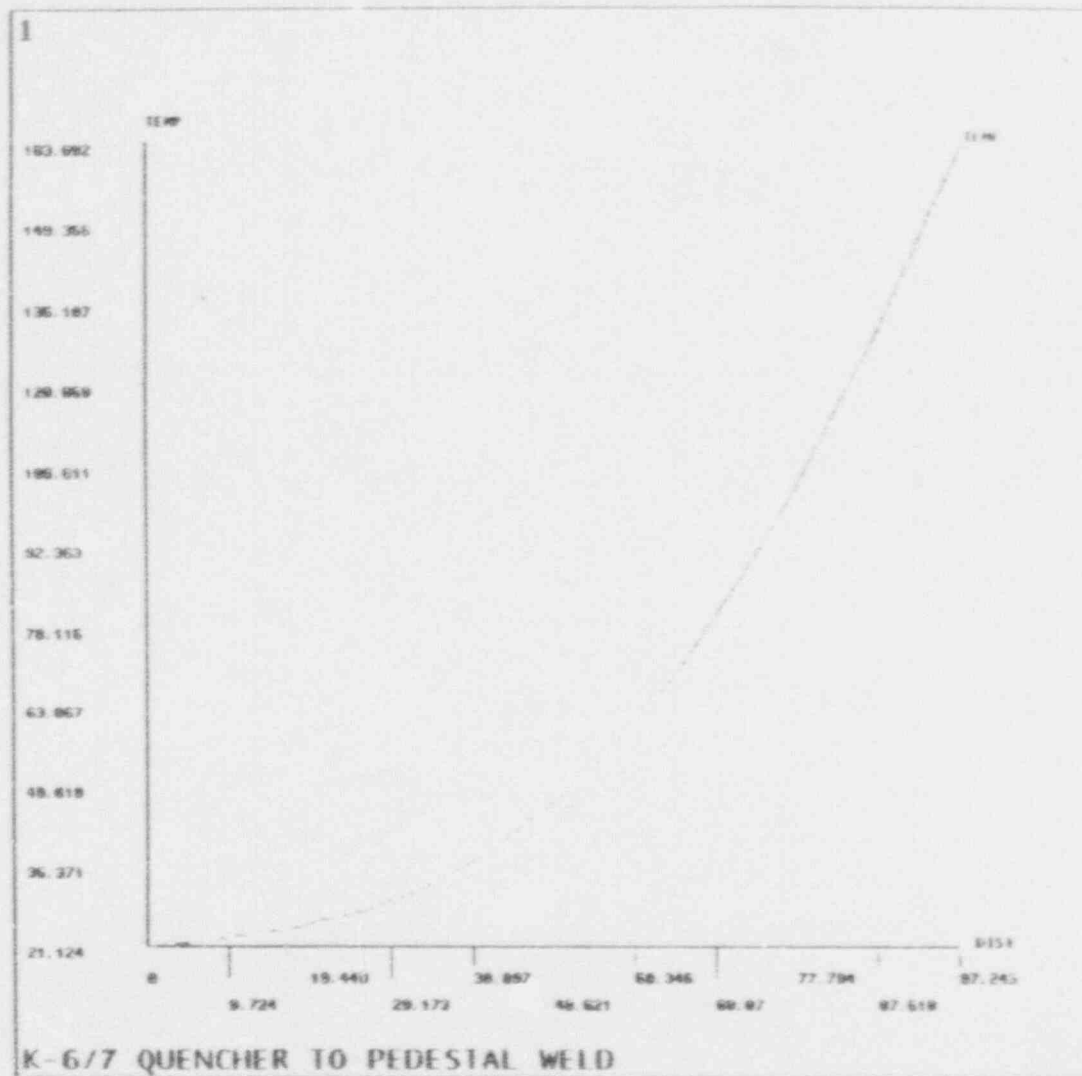


Figure 9 : Temperature distribution, degree C
 (at time 3.00 min. after SRV blowdown)



ANSYS 4.4A
AUG 11 1992
8:36:34
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ITER=6
TIME=3
PATH PLOT
NOD1=66
NOD2=115
TEMP

ZV =1
DIST=0.6666
XF =0.5
YF =0.5
ZF =0.5

Figure 10 : Temperature distribution through section A-A
(nodes 66-115, 3.00 min after SRV blowdown)

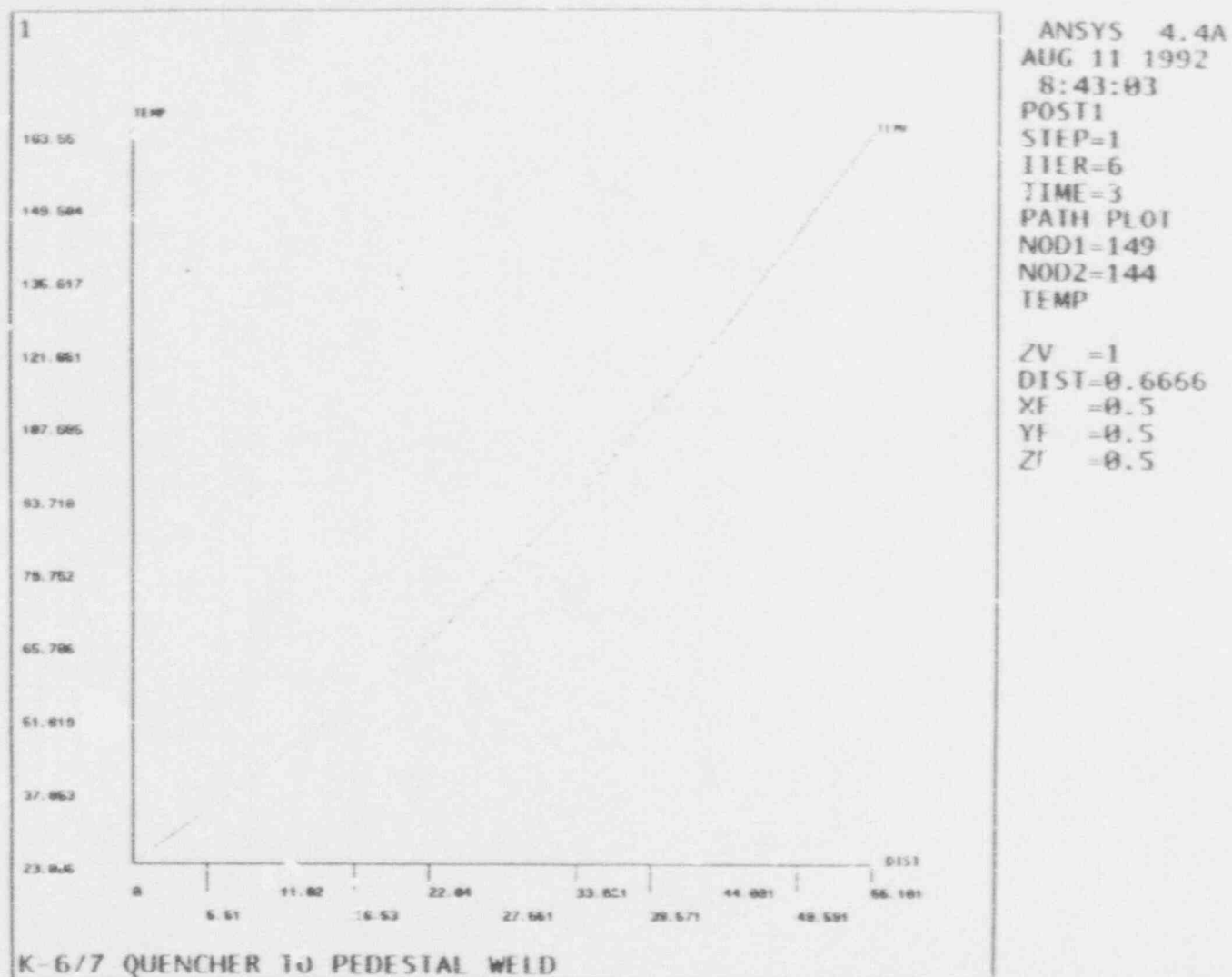


Figure 11 : Temperature distribution through section B-B
(nodes 149-144, 3.00 min after SKV blowdown)

the number of degrees of freedom are taken more than twice the number of modes with frequencies less than 33 Hz.

- (2) Mass is lumped at any point where a significant concentrated weight is located (e.g., the motor in the analysis of pump motor stand, the impeller in the analysis of pump shaft, etc).
- (3) If the equipment has free-end overhang span with flexibility significant compared to the center span, a mass is lumped at the overhang span.
- (4) When a mass is lumped between two supports, it is located at a point where the maximum displacement is expected to occur. This tends to lower the natural frequencies of the equipment because the equipment frequencies are in the higher spectral range of the response spectra. Similarly, in the case of live loads (mobile) and a variable support stiffness, the location of the load and the magnitude of support stiffness are chosen to yield the lowest frequency content for the system. This ensures conservative dynamic loads since the equipment frequencies are such that the floor spectra peak is in the lower frequency range. If not, the model is adjusted to give more conservative results.

3.7.3.3.3 Field Location of Supports and Restraints

Service Level 2

The field location of seismic supports and restraints for Seismic Category I piping and piping systems components is selected to satisfy the following two conditions:

- (1) the location selected must furnish the required response to control strain within allowable limits; and
- (2) adequate building strength and stiffness for attachment of the component supports must be available.

The final location of seismic supports and restraints for Seismic Category I piping, piping system components, and equipment, including the placement of snubbers, is checked against the drawings and instructions issued by the

A-6

engineer. An additional examination of these supports and restraining devices is made to assure that their location and characteristics are consistent with the dynamic and static analyses of the system.

3.7.3.3.4 Analysis of Frame Type Pipe Supports

The design loads on frame type pipe supports include (a) loads transmitted to the support by the piping response to thermal expansion, dead weight, and the inertia and anchor motion effects, and (b) support internal loads caused by the weight, thermal and inertia effects of loads of the structure itself, and (c) friction loads caused by the pipe sliding on the support. To calculate the frictional force acting on the support, dynamic loads that are cyclic in nature need not be considered. ~~The coefficient of friction used will be static coefficients and will be substantiated by actual test data covering the range of materials, geometry and loading condition. To determine the response of the support structure to applied dynamic loads, the equivalent static 1.2d method of analysis described in Paragraph 3.7.3.8.1.5 may be used. The loads transmitted to the support by the piping will be applied as static loads acting on the support.~~

As in the case of other supports, the forces the piping places on the frame-type support are obtained from an analysis of the piping. In the analysis of the piping the stiffness of the frame-type supports shall be included in the piping analysis model, unless the support can be shown to be rigid. The frame type supports may be modeled as rigid restraints providing they are designed so the maximum deflection in the direction of the applied load is less than 1/16 inch and providing the total gap or diametrical clearance between the pipe and frame support is between 1/16" and 3/16" when the pipe is in either the hot or cold condition. ~~← Add insert AA~~

3.7.3.4 Basis of Selection of Frequencies

Where practical, in order to avoid adverse resonance effects, equipment and components are designed/selected such that their fundamental frequencies are outside the range of 1/2 to twice the dominant frequency of the associated support structures. Moreover, in any case, the equipment is analyzed and/or tested to demonstrate that it is adequately designed for the applicable loads considering both its fundamental frequency and the forcing frequency of the applicable support structure.

3.7.3.12 Buried Seismic Category I Piping and Tunnels and Exterior Piping

All underground Category I piping systems are installed in tunnels. The following items are considered in the analysis:

- (1) The inertial effects due to an earthquake upon underground systems and tunnels will be adequately accounted for in the analysis. In case of buried systems sufficiently flexible relative to the surrounding or underlying soil, it is assumed that the systems will follow essentially the displacements and deformations that the soil would have if the systems were absent. When applicable, procedures, which take into account the phenomena of wave travel and wave reflection in compacting soil displacements from the ground displacements, are employed.
- (2) The design response spectra for the underground piping are the horizontal and vertical design spectra at the ground surface given in Figures 3.7-1 and 3.7-2. These design spectra are constructed in accordance with Regulatory Guide 1.60. The piping analysis is performed using one of the methods described in Subsection 3.7.3.1.
- (3) When applicable, the effects due to local soil settlements, soil arching, etc., are also considered in the analysis.

3.7.3.12 (cont'd)

All above grade level Category I piping outside the Reactor building, Control building and the Turbine building is enclosed within structures. The design response spectra for this enclosed piping are the horizontal and vertical design spectra at the ground surface given in Figures 3.7-1 and 3.7-2. The piping analysis is performed using one of the methods described in Subsection 3.7.3.1.

New third para.
in Section 3.9.3.1

SER 5.18

Piping loads due to the thermal expansion of the piping and thermal anchor movements at supports are included in the piping load combinations. All operating modes are evaluated and the maximum moment ranges are included in the fatigue evaluation. Piping systems with ^{maximum} operating temperatures of less than or equal to 150°F are not required to be analyzed for thermal expansion loading.