



101 California Street, Suite 1000, San Francisco, CA 94111-5894

415/397-5600

August 29, 1984
84042.028

Mrs. Juanita Ellis
President, CASE
1426 S. Polk
Dallas, Texas 75224

Subject: Communications Report Transmittal #11
Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3
Texas Utilities Generating Company
Job. No. 84042

Dear Mrs. Ellis:

Enclosed please find telecons associated with the Phase 3 Independent Assessment Program.

If you have any questions or desire to discuss any of these documents, please do not hesitate to call either me or Donna Oldag.

Very truly yours,

N. H. Williams
Project Manager

Attachments

cc: Mr. D. Wade (TUGCO) w/attachments
Mr. S. Treby (USNRC) w/attachments
Ms. J. Van Amerongen (TUGCO/EBASCO) w/attachments
Mr. D. Pigott (Orrick, Herrington & Sutcliffe) w/o
Mr. S. Burwell (USNRC) w/attachments

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Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station	Job No.	84042
	Independent Assessment Program - Phase 3	Date:	6/14/84
Subject:	Oversize Bolt Holes (Pipe Supports)	Time:	9:20
	(Item 25 in 5/24 Telecon)	Place:	Site
Participants:	R. Sievers	of	B&R
	J. Minichiello		Cygna

Item	Comments	Required Action By
	<p>Cygna called the CPSES N-STAMP holder to understand their position on the oversize bolt holes. Mr. Sievers, who is responsible for site ASME QC and a member of the Code Committee, stated that both his and the ANI's interpretation of NF-4721 is that no code violation exists. Their basis is that paragraph NF-4721(b) does <u>not</u> prohibit the use of oversize holes in bearing connections. Since the code has no prohibition, no violation exists. If engineering needs a smaller tolerance hole to meet their design criteria, that is up to engineering. For fabrication up to 1/4" oversize would be acceptable.</p>	

so as to avoid harmful thermal gradients. This procedure may also be used for postweld heat treatment after repairs.

NF-4624.4 Heating Components Internally. The component support or item may be heated internally by any appropriate means and with adequate indicating and recording temperature devices to aid in the control and maintenance of a uniform distribution of temperature in the component support or item. Previous to this operation, the component support or item should be fully enclosed with insulating material.

BON STEVENS DAR
X204

NF-4660 HEAT TREATMENT OF ELECTROSLAG WELDS

Electroslag welds in ferritic material over $1\frac{1}{2}$ in. (38 mm) in thickness at the joints shall be given a grain refining heat treatment.

NF-4700 REQUIREMENTS FOR BOLTED CONSTRUCTION

NF-4710 BOLTING AND THREADING

NF-4711 Thread Engagement

The threads of all bolts or studs shall be engaged for the full length of thread in the nut.

NF-4712 Thread Lubricants

Any lubricant or compound used in threaded joints shall be suitable for the service conditions and shall not react unfavorably with any support element material. Contact surfaces within friction type joints shall be free of oil, paint, lacquer, or galvanizing.

NF-4713 Removal of Thread Lubricants

All threading lubricants or compounds shall be removed from surfaces which are to be welded.

NF-4720 BOLTING

NF-4721 Bolt Holes

For the purpose of this Article, high strength bolts shall be considered those with yield strength greater than 80.0 ksi (552 MPa). Bolt holes shall meet the requirements of (a) through (e) below.

(a) Holes for nonfitted bolts shall meet the requirements of Table NF-4721(a)-1. For anchor bolts or concrete expansion anchors having a specified minimum tensile strength of 80.0 ksi (552 MPa) or less, the hole sizes indicated in this Subsection may be increased by $\frac{1}{8}$ in. (1.6 mm). When the bolt hole size is $\frac{1}{8}$ in. (3.2 mm) larger than the bolt, and the bolt is $\frac{1}{2}$ in. (13 mm) or smaller, standard washers shall be used.

(b) Oversized or slotted bolt holes may be used with high strength bolts $\frac{1}{2}$ in. (13 mm) in diameter and larger, except as restricted in (1), (2), and (3) below.

(1) Oversized holes shall not exceed the requirements of Table NF-4721(b)-1. They may be used in any or all plies of friction-type connections. Hardened washers shall be installed over exposed oversized holes.

(2) Short-slotted holes shall not be wider than permitted by (a) above and shall not have a length exceeding the oversize diameter allowed in (1) above by more than $\frac{1}{8}$ in. (1.6 mm). They may be used in any or all plies of friction-type or bearing-type connections. The slots may be used without regard to direction of loading in friction-type connections but shall be normal to the direction of the load in bearing-type connections. Hardened washers shall be installed over exposed short-slotted holes.

(3) Long-slotted holes shall not be wider than permitted by (a) above and shall not have a length which exceeds $2\frac{1}{2}$ times the bolt diameter. In friction-type connections, the long-slotted holes may be used without regard to direction of loading, provided the stress on the bolts does not exceed 75% of the allowable working stress given in NF-3000. In bearing-type connections, the long diameter of the slot shall be normal to the direction of loading. Long-slotted holes may be used in only one of the connected parts of either a friction-type or bearing-type connection at an individual faying surface. Structural plate washers or a continuous bar not less than $\frac{1}{8}$ in. (3 mm) in thickness shall be used to cover long slots that are in the outer plies of joints. These washers or bars shall have a size sufficient to cover the slot completely after installation and shall meet the requirements of NF-3000.

(c) Except as specified in (d) below, holes may be

TABLE NF-4721(a)-1
BOLT HOLE SIZES

Bolt Size	Hole Size
≤ 1 in.	Bolt diameter plus $\frac{1}{16}$ in.
$1\frac{1}{8}$ in. to 2 in.	Bolt diameter plus $\frac{1}{8}$ in.
> 2 in.	Bolt diameter plus $\frac{3}{16}$ in.

punched, provided the thickness of the material is not greater than the nominal diameter of the bolt plus $\frac{1}{8}$ in. (3.2 mm). When the thickness of the material is greater than the nominal diameter of the bolt plus $\frac{1}{8}$ in. (3.2 mm), holes shall be drilled, subpunched, and reamed, or thermally cut. Thermal cutting shall not be used unless the load bearing surfaces are machined or ground smooth. For subpunched holes, the die shall be at least $\frac{1}{16}$ in. (1.6 mm) smaller than the nominal diameter of the bolt.

(c) Bolt holes in material over $\frac{1}{2}$ in. (13 mm) thick having a specified minimum yield strength greater than 80.0 ksi (552 MPa) shall be drilled.

(e) For bolts not subjected to shear, the limits for oversized and slotted holes in (d) above may be increased if structural plate washers or continuous bars which meet the requirements of NF-3000 are provided.

NF-4722 Bolted Connections

(a) Surfaces of bolted parts in contact with the bolt head and nut shall not have a slope of more than 1:20 with respect to a plane normal to the bolt axis. Where the surface of high strength bolted part has a slope of more than 1:20, a beveled washer shall be used to compensate for the lack of parallelism.

(b) Bolts loaded in pure shear shall not have threads located in the load bearing part of the shank unless permitted by the Design Specifications.

NF-4723 Precautions Before Bolting

All parts assembled for bolting shall have contact surfaces free from scale, chips, or other deleterious material. Surfaces and edges to be joined shall be smooth, uniform, and free from fins, tears, cracks, and other defects which would degrade the strength of the joint.

TABLE NF-4721(b)-1
TOLERANCES ON OVERSIZED HOLES

Bolt Size	Max. Hole Size
≤ $\frac{7}{8}$ in.	Bolt diameter plus $\frac{1}{16}$ in.
1 in.	Bolt diameter plus $\frac{1}{8}$ in.
> 1 in.	Bolt diameter plus $\frac{3}{16}$ in.

NF-4724 Bolt Tension

All high strength structural bolts shall be preloaded to a value not less than that given in the Design Specifications. Preloading shall be monitored by the turn of nut method by properly calibrated wrenches, or by direct extension indicators. Bolts preloaded by means of a calibrated wrench shall be installed with a hardened washer under the nut or bolt head, whichever is the element turned in preloading. Hardened washers are not required when bolts are preloaded by the turn of nut method, except that hardened washers are required under the nut and bolt head when the bolts are used to connect material having a specified yield point less than 40.0 ksi (275 MPa).

NF-4725 Locking Devices

All threaded fasteners shall be provided with locking devices to prevent loosening during service. Elastic stop nuts (when compatible with service temperature), lock nuts, jam nuts, and drilled and wired nuts are all acceptable locking devices. Upset threads (by peening, tack welding, or other means) may serve as locking devices. Torquing and other preloading methods may be used as locking devices on threaded parts made of material with a yield strength of 80 ksi (552 MPa) and greater, loaded in tension, provided the resulting preload is at least 20% above the maximum load on the fastener for the specified loading conditions, but is limited to 70% of the specified minimum tensile strength of the fastener. The threaded assembly shall be tested for the dynamic loading conditions specified in the Design Specification, and the established preload shall be verified on the assembly by properly calibrated wrenches, direct extension indicators, or the turn of the nut method. The results of the test, required preload, and specified thread lubrication shall be provided in the Design Report. Disk and helical spring lock washers shall not be used as locking devices.

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Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project:

Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No: 84042

Date: 4/2/84

Subject:

Mechanical Review Document Request

Time: 12:45 pm

Place: CPSES Site

Participants:

B. Wood

of

TUGCO

J. Russ

Cygna

Item

Comments

Required
Action By

Reference: Conference Report dated 2 April 1984, "Mechanical Review Document Request," B. Wood and J. Russ participating

Cygna received the following from Mr. Wood:

1. All documents requested on the referenced conference report.
2. Calculation Package CC-1-028-004-A33K.

Signed:

/pm Page 1 of 1

Distribution: D. Wade, N. Williams, G. Grace, J. Minichiello, C. Wong, Project File



Communications Report

Company:	Texas Utilities	<input checked="" type="checkbox"/> Telecon	<input type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	5/31/84
Subject:	Action Items	Time:	10:15 a.m.
		Place:	Boston
Participants:	G. Grace	of	TUSI
	S. Bibb		Cygna

Item	Comments	Required Action By
	<p>I received a call from George Grace inquiring whether there were any QA action items that I was waiting for. Specifically, he was referring to some information that I requested from Tony Vega. I told George that I was currently discussing these issues with Susan Spencer and Debra Anderson of TUGCO (Dallas) and that he didn't have to get involved at this time.</p> <p>In addition, I requested George to send me a copy of DCP-3, latest revision.</p> <p>I asked George to make sure that the list of Cygna personnel on the gate log for May, 1984 was maintained. He told me that it was taken care of.</p>	


Signed: *NH Williams* Page 1 of 1
Distribution: N. Williams, D. Wade, G. Grace, D. Smedley, S. Bibb, S. Treby, J. Ellis, Project

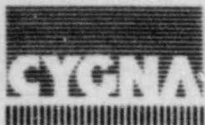


Communications Report

Company:	Texas Utilities	<input checked="" type="checkbox"/> Telecon	<input type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	3/27/84
Subject:	Response Time for the Pipe Stress Questions	Time:	11:15 am
		Place:	SF
Participants:	S. Lim	of	Gibbs & Hill
	L. Weingart		Cygna

Item	Comments	Required Action By
	I contacted Steve to discuss his expected response time for the pipe stress questions. Steve is handling the main steam inside containment and feels that he will have his answers ready for discussion in about a week. He is going to discuss the other problems (MSOC and CCW) with Henry Mentel and call me back later today. I told Steve to consider the possibility of my going to the Gibbs & Hill offices in New York in order to expedite the process.	

Signed:  /pm Page 1 of 1
Distribution: D. Wade, N. Williams, L. Weingart, R. Hess, Project File, G. Grace



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 3/21/84

Subject: Cygna Questions on Main Steam

Time: 10:00 am

Place: Fort Worth

Participants: S. Lim

of Gibbs & Hill

L. Weingart

Cygna

Item	Comments	Required Action By
	<p>Steve and I discussed all of the questions which have arisen from the review of the stress analyses of the main steam inside and outside containment and the component cooling water system. The main purpose of this discussion was to assure that Gibbs & Hill understood both the nature and intent of the questions such that excessive iterations of response could be avoided.</p> <p>Only one direct action item resulted from this conversation. Steve will send a plot of the time history response of some of the axial restraints on the main steam inside containment. A comparison of these with the plots of the input forcing function along those runs will provide further assurance that the proper forcing functions were indeed applied for each system.</p>	

Signed:

/pm Page 1 of 1

Distribution: D. Wade, N. Williams, J. Minichiello, L. Weingart, Project File, G. Grace



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 3/23/84

Subject: Data Request

Time: 8:30 am

Place: Comanche Peak

Participants: D. Rencher

of Texas Utilities

J. Minichiello

Cygna

Item	Comments	Required Action By
•	<p>Cygna requested the following:</p> <p>Add. Load Case 4 in STRUDL run - get output</p> <p>Change sign of X coordinate in Node 24 -- get output</p> <p>We need 2 runs to determine what was used in design</p> <p>(i.e., node 24 with: Run 1. $\div Y = -.5$, $x = .374$; Run 2. $\div Y = -.5$, $x = -.374$)</p>	

Signed:

J. Minichiello

/pm

Page 1

of

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Distribution: D. Wade, N. Williams, J. Minichiello, G. Grace, J. Russ, Project File



Communications Report

Company:	Texas Utilities	<input checked="" type="checkbox"/> Telecon	<input type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No:	84042
		Date:	6/22/84
Subject:	Lateral Displacement, MS-1-004-003-S72R	Time:	3:30 p.m.
		Place:	Boston/Washington
Participants:	Dr. Robert Iotti	of	Ebasco
	Dr. Gordon Bjorkman		Cygna

Item	Comments			Required Action By
	Dr. Iotti confirmed that the lateral displacements of support MS-1-004-003-S72R were small.			
	<u>Lateral</u>	<u>Axial</u>	<u>Vertical</u>	
DL	0	0	0	
DL+TH1	0	1.223	0	
DL+TH2	0	1.324	0	
1/2 SSE	0.0017	0.0064	0	
SAM	0.002	0.207	0	
SSE	0.003	0.0108	0	
SAM	0.003	0.346	0	
Dr. Iotti had made a very preliminary calculation of the dynamic load factor for the stability bumpers and determined that it was less than 1.0. The calculation was based on the minimum frequency of the stability bumpers and assured the application of a triangular pulse at peak static bumper load.				
Dr. Björkman commented that based on the uncertainties in the frequency calculation (support stiffener and mass distribution), pulse shape and duration one should use the maximum dynamic load factor.				
Dr. Iotti agreed that this might be true. He mentioned that the impulse force is displacement dependent and that the problem was nonlinear. He recognized that the Cygna static calculation showed that the stability bumpers are not adequate. However, he believes that the bumpers can be qualified dynamically.				

Signed: *N. Williams* /ms Page 1 of 2
Distribution: N. Williams, D. Wade, G. Grace, G. Bjorkman, J. Minichiello, S. Treby, J. Ellis,
S. Burwell, Project File



Communications Report

Item	Comments	Required Action By
	<p>Dr. Bjorkman commented that this would probably be a first (i.e., qualify a support dynamically which a calculation shows is not adequate statically). This would require an extensive nonlinear time history analysis.</p> <p>Dr. Iotti mentioned that dynamic qualification of the support was a possibility.</p> <p>Dr. Bjorkman noted that dynamic qualification would be discussed internally at Cygna.</p>	



Communications Report

Company: Texas Utilities

☒ Telecon

☐ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 7/11/84

Subject: Pipe Support Questions

Time: 3:15 p.m.

Place: S.F.

Participants: J. Finneran

of TUGCO

J. Minichiello

Cygna

Item	Comments	Required Action By
	<p>In response to Cygna's question on composite section weld calculations (earlier telcon 7/11/84), TUGCO reviewed the 7 calculations with Cygna and agreed that the weld calculations were incomplete (did not include tensile effect) or inappropriate (wrong method used). Cygna's recalculations did show all these welds acceptable, so there is no design impact. TUGCO has reviewed the 18" feedwater line supports for composite sections and found one section. TUGCO is checking the weld for that section. In addition, TUGCO will check supports on the 30" service water lines for composite sections. Since these systems represent the largest, most highly loaded systems in the plant, this check by TUGCO will ensure that there is no design impact plant-wide on this topic. N. Williams will telephone D. Wade to determine the appropriate documentation required for this commitment.</p>	

Signed:

NH Williams

/ms Page 1 of 1

Distribution: N. Williams, D. Wade, G. Grace, J. Minichiello, G. Wong, S. Treby, J. Ellis,

S. Burwell, Project File



Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input checked="" type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Election Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	June 27, 1984
Subject:	Specification Report	Time:	8:05 am
		Place:	CPSES
Participants:	J. Kimble	of	B&R - P&R Room
	J. Russ		CES

Item	Comments	Required Action By
	<p>Cygna requested and received one copy of the following specification:</p> <p>Specification No.: G-952124, Rev. 4 WPT No.: 4514, dated February 17, 1982 Shop Order No.: TBX-120</p>	


Signed: *N. Williams* /ss Page 1 of 1
Distribution: N. Williams, D. Wade, G. Grace, R. Hess, J. Russ, S. Treby, J. Ellis, S. Burwell,



Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input checked="" type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program, Phase 3	Job No.	84042
		Date:	March 16, 1984
Subject:	Fire Protection and Hardware Weights for Cable Trays	Time:	3:30 pm
		Place:	CPSES (site)
Participants:	Doug Hunt	of	Gibbs & Hill
	D. Nandi		Gibbs & Hill
	John Russ		Cygna

Item	Comments	Required Action By
	<p>Reference: Conference Report of 15 March 1984 "Fire Protection and Hardware Weights for Cable Trays," D. Hunt, D. Nandi and J. Russ participating</p> <p>In response to the questions in the referenced conference report, Mr. Nandi gave me the attached sheets, marked A1 to A10. The information on sheets A1 to A9 is a summary of data from Binder 2323-E1-1700, Issue 322, as noted on each sheet. The information on Sheet A10 is taken from manufacturer's catalogs or as noted. Mr. Nandi told me that the weights of the splice plates and bolts used in the tray connections are included in the tray weight. He also stated that "Z" clips and friction clips are normal connections of the tray to the supports. Heavy duty clamps are used at a support, if a tray joint occurs there.</p> <p>Doug told me that cover plates are not used at Comanche Peak because the insulating material used is rigid. The cable tray is not used to support the weight of the insulation.</p>	

Signed:  /pm Page 1 of 11

Distribution: D. Wade, N. Williams, R. Hess, Project File, G. Grace

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____

(2) Electrical 2323-81-718-11 Rev 3

(3) Structural _____

* (4) Other 2323-81-1700 Issue 322
Pg. 30-14.2

3. LOCATION:

(1) Building: SAFEGUARDS(2) Elevation: 052'-6"(3) Room No: 103(4) Tray No: T11GEAB

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY*Ht (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES (E1-1700)	TOTAL WT.	DESIGN LOAD (TABLE 1)	REMARKS
27	36" x 4"	12 #Ht		16.8 #Ht	28.8 #Ht	105.0 #Ht	
26	36" x 4"	12 #Ht		16.8 #Ht	28.8 #Ht	105.0 #Ht	
29	36" x 4"	12 #Ht		12.6 #Ht	24.6 #Ht	105.0 #Ht	
30	36" x 4"	12 #Ht		4.2 #Ht	16.2 #Ht	105.0 #Ht	
22	30" x 4"	10.5 #Ht		16.8 #Ht	27.3 #Ht	87.5 #Ht	
23	30" x 4"	10.5 #Ht		16.8 #Ht	27.3 #Ht	87.5 #Ht	
24	30" x 4"	10.5 "		16.8 "	27.3 "	87.5 "	
25	30" x 4"	10.5 "		16.8 "	27.3 "	87.5 "	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

SHEET A1

LENGTH OF TRAY	FROM	T11GEAB 25	TO	T11GEAB 26	= 6 ft
LENGTH OF TRAY	"	T11GEAB 26	TO	T11GEAB 27	= 4 ft
"	"	T11GEAB 27	TO	T11GEAB 29	= 1 ft
"	"	T11GEAB 29	TO	T11GEAB 28	= 4 ft
"	"	T11GEAB 28	TO	T11GEAB 30	= 10 ft
"	"	T11GEAB 22	TO	T11GEAB 23	= 26 ft
"	"	T11GEAB 23	TO	T11GEAB 24	= 8 ft
"	"	T11GEAB 24	TO	T11GEAB 25	= 9 ft

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____

(2) Electrical 2323-EI-0602-14 Rev. 2

(3) Structural _____

(4) Other 2323-EI-1700 ISSUE 322
Pg. 30-14.1

3. LOCATION:

(1) Building: SAFEGUARDS 1(2) Elevation: 831'-6"(3) Room No: 88 NORTH(4) Tray No: T11G5AB

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES (EI-1700)	TOTAL WT.	DESIGN LOAD (TABLE 1)	REMARKS
44	24" x 4'	9.5 #/ft	29.5 #/ft	16.8	55.8 #/ft	70 #/ft	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

SHEET A2

LENGTH OF TRAY FROM T11G5AB44 TO T11G5AB45 = 24 ft.

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____
 (2) Electrical 2323-E1-603-11 Rev. 3
 (3) Structural _____
 (4) Other 2323-E1-1700 Issue 322
 Pg. 30-19.2

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY #/ft (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES #/ft (E1-1700)	TOTAL WT. #/ft	DESIGN LOAD #/ft (TABLE 1)	REMARKS
45	24" x 4"	9.5		16.8	26.3	70	
46	24" x 4"	9.5		16.8	"	"	
47	24" x 4"	9.5		16.8	"	"	
48	30" x 4"	10.5		16.8	27.3	87.5	
49	30" x 4"	10.5		16.8	"	"	
50	30" x 4"	10.5		8.4	18.9	"	

5. APPROVAL SIGNATURES:

SHEET A3

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

LENGTH OF TRAY				FROM	T1143AB-45	TO	T1143AB-46 =	8.0 ft	
					2)	-46	1)	-47 =	12.0 ft
1)	1)	2)	2)		2)	-47	2)	-48 =	16.0 ft
1)	2)	1)	1)		2)	-48	2)	-49 =	4.0 ft
2)	1)	2)	2)		1)	-49	1)	-50 =	9.0 ft
2)	2)	1)	1)		2)	-50	2)	-51 =	5.0 ft

ATTACHMENT 2

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____
 (2) Electrical E1-0602-11 Rev 1
 (3) Structural _____
 (4) Other 2323-E1-1700 Issue 322
Pg. 30-16.1

3. LOCATION:

(1) Building: SAFEGUARDS-1
 (2) Elevation: 810'-6"
 (3) Room No: 77 NORTH
 (4) Tray No: T11G5AB

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES (E1-1700)	TOTAL WT.	DESIGN LOAD (TABLE 1)	REMARKS
07	24" x 4"	9.5 #/ft		16.8 #/ft	26.3 #/ft	70	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
 Attach calculations and reference CMC and/or
 DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

SHEET A4

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

LENGTH OF TRAY FROM T11G5AB 07 TO T11G5AB 44 = 15 ft

ATTACHMENT 2

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA.

2323-EI-0600-11 Rev. 3

(2) Electrical

2323-EI-0600-12 Rev. 2

(3) Structural

(4) Other

2323-EI-1700 Issue 322
Pg. 30-14.1

3. LOCATION:

(1) Building:

SAFEGUARDS

(2) Elevation:

773'-6"

(3) Room No:

(4) Tray No:

T11GSAB

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES (EI-1700)	TOTAL WT.	DESIGN LOAD (TABLE 1)	REMARKS
03	24" x 4"	9.5 #/ft		16.8	26.3 #/ft	70 #/ft	2323-EI-0600-12 Rev. 2
04	24" x 4"	9.5 #/ft		16.8	26.3 #/ft	70 #/ft))
05	24" x 4"	9.5 #/ft		12.6	22.1 #/ft	70 #/ft	2323-EI-0600-11 Rev. 3

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

SHEET A5

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

TRAY LENGTH FROM T11GSAB 03 TO T11GSAB 04 = 5 ft.

)))) T11GSAB 05 TO T11GSAB 06 = 10 ft.

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____

(2) Electrical 2323-EI-0601-11 Rev 3

(3) Structural _____

(4) Other 2323-EI-1700 Issue 322
Pg. 30-1 G.1

3. LOCATION:

(1) Building: SAFEGUARDS-1(2) Elevation: 790'-6"

(3) Room No: _____

(4) Tray No: T11G5AB

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY #Ht (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1) #Ht	WT. OF CABLES #Ht (E1-1700)	TOTAL WT. #Ht	DESIGN LOAD #Ht (TABLE 1)	REMARKS
01	24" x 4"	9.5		16.8		70	
02	"	9.5		16.8		70	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

SHEET A6

LENGTH OF TRAY FROM T11G5AB01 TO T11G5AB02 = 24 ft

LENGTH OF TRAY " T11G5AB01 " T11G5AB07 = 27 ft.

" " " T11G5AB02 " T11G5AB03 = 19 ft.

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____

(2) Electrical E1-0600-11 Rev. 3

(3) Structural _____

(4) Other 2323-E1-1700 Issue 322
Pg 30-1G.1

3. LOCATION:

(1) Building: SAFEGUARDS-1(2) Elevation: 773'-6"

(3) Room No: _____

(4) Tray No: T11GSE

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES (E1-1700)	TOTAL WT.	DESIGN LOAD (TABLE 1)	REMARKS
007	12" x 4"	6 #/ft		4.2 #/ft	10.2 #/ft	35 #/ft	
008	12" x 4"	6 #/ft		4.2 #/ft	10.2 #/ft	35 #/ft	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

SHEET A7

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

TRAY LENGTH FROM T11GSE007 TO T11GSE008 = 4ft

ATTACHMENT 2

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE: 0

2. REFERENCE DRAWINGS: (1) SK. TFHA. EI-0600-11 Rev. 3
 (2) Electrical EI-0600-12 Rev. 2
 (3) Structural 2323-EI-1700 Issue 322
 (4) Other Pg. 30-10.1

3. LOCATION: (1) Building: SAFEGUARDS-1
 (2) Elevation: 773'-6"
 (3) Room No: _____
 (4) Tray No: T1105A11

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY #Ht (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1)	WT. OF CABLES #Ht (EI-1700)	TOTAL WT. #Ht	DESIGN LOAD #Ht (TABLE 1)	REMARKS
28	24" x 4"	9.5		16.8	26.3	70	
29	24" x 4"	9.5		16.8	26.3	70	
30	24" x 4"	9.5		12.6	22.1	70	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
 Attach calculations and reference CMC and/or
 DCA No. in Remarks column.

5. APPROVAL SIGNATURES:

SHEET AB

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

TRAY LENGTH FROM T1105A11-28 TO T1105A11-29 = 5 ft
 " " " " - 30 " " - 31 = 7 ft.

ATTACHMENT 2

CABLE TRAY THERMOLAG EVALUATION FORM

1. DATE:

2. REFERENCE DRAWINGS: (1) SK. TFHA. _____

(2) Electrical 2323-EI-601-11 Rev.3

(3) Structural _____

(4) Other 2323-EI-1700 ISSUE 322
Pg. 30-1Φ.1

3. LOCATION:

(1) Building: SAFEGUARDS-1(2) Elevation: 790'-6"

(3) Room No: _____

(4) Tray No: T11ΦSAA

4. (a) EVALUATION AS PER SECTION 3-2-2-1:

SEGMENT NO.	TRAY SIZE	SELF WT. OF TRAY #ft (TABLE 1)	WT. OF FIRE BARRIER (TABLE 1) #ft	WT. OF CABLES #ft (EI-1700)	TOTAL WT. #ft	DESIGN LOAD #ft (TABLE 1)	REMARKS
23	24" x 4"	9.5		16.8	26.3	70	
24	24" x 4"	9.5		16.8	26.3	70	
25	24" x 4"	9.5		16.8	26.3	70	
26	24" x 4"	9.5		16.8	26.3	70	
27	24" x 4"	9.5		16.8	26.3	70	

4. (b) Evaluation as per section 3-2-2.2 or 3-2-2.3.
Attach calculations and reference CMC and/or
DCA No. in Remarks column.

SHEET A9

5. APPROVAL SIGNATURES:

Electrical Engineering:

Civil/Structural Engineering:

Civil/Structural Check:

Civil/Structural Review:

TRAY LENGTH FROM T11ΦSAA-23 TO T11ΦSAA-24 = 29 ft

" " " " -24 " " -25 = 7 ft

" " " " -26 " " -27 = 5 ft

" " " " -27 " " -28 = 22 ft

TEXAS UTILITIES SERVICES INC.
COMANCHE PEAK S.E.S.

Agent For

DALLAS POWER & LIGHT COMPANY
TEXAS ELECTRIC SERVICE COMPANY
TEXAS POWER & LIGHT COMPANY

Date _____

Filing Code _____

Calc. By _____

Sheet No. _____ Of _____

Checked/Approved By _____

G & H Job No. _____

Subject _____

Ref. Dwg./Spec. No. _____

① WT. OF 'Z' CLIP

a) For 4" High Tray = .47 [#] EACH

b) For 6" High Tray = .60 [#] EACH

② WT. OF FRICTION TYPE CLIP

a) For 4" High Tray = .92 [#] EACH

b) For 6" High Tray = 1.20 [#] EACH

③ WT. OF HEAVY DUTY CLAMP

a) 12 CLAMP $\frac{1}{2} \times 4 \times 6$ = 3.5 EACH (For 4" High Tray)

b) 12 CLAMP $\frac{1}{2} \times 6 \times 6$ = 5.10 EACH (For 6" High Tray)

④ STANDARD SPLICE PLATE

SPLICE PLATE USED IN EVERY 12' IN STAIRWAY RUN. (MAX)

a) For 4" High Tray 3" WIDE = .85 [#] EACH

b) For 6" High Tray 3" WIDE = 1.06 [#] EACH

c) For 6" High Tray 6" WIDE = 1.70 [#] EACH

⑤ WT. OF $\frac{5}{8}$ A 307 BOLT (2" long) = .47 ^{#/each}

⑥ WT. OF BAYEL WASHER USED IN BOLT = .22 ^{#/each}

⑦ WT. OF ROUND WASHER USED IN CASE OF MULTI SUPPORT ONLY = .036 ^{#/each}

⑧ For _____



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program, Phase 3

Job No. 84042

Date: March 15, 1984

Subject: CPAA's for Fire Protected Cable Trays

Time: 2:15 pm

Place: CPSES (site)

Participants: Sarah Donald

of Brown & Root

John Russ

Cygna

Item	Comments	Required Action By
	I requested copies of the CPPA's listed on the attached sheet from Sarah of Site Document Control Center.	Brown & Root

Signed: 

/pm

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of 2

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TFHA FIRE BARRIER PENV LOG *

JEFF SPEDMAN
15 MARCH '89

BUILD. / ELEV.	FIRE ZONE	PENV DRAWING #	CPPA #
SG - 873	17B 17C	1 2	30,117 , 21,082
AB - 852	21F	4	25,883 , 25,432 , 23,716 , 22,096 33,137 <hr/> 35,616
SG - 852	18 17A	5 6	30,182 , 30,118 , 26,005 33,136 <hr/> 34,961 36,628
AB - 831	21D	10	24,362 , 22,345 <hr/> 35,620
SG - 831	15 16 144	7 8 9	26,763 , 26,731 , 26,014 <hr/> 35,147
AB - 790	21A	12	26,588 <hr/> 35,594
SG - 790	4C 1C & 2C	13 14	26,706 <hr/> 35,359
AB - 810	21B, C, & 31	15	29,835 <hr/> 35,832
SG - 810	8 9 142 & 143	20 21 22	30,104 <hr/> 35,171
EC - 778	153 43, 47, & 57	17 18	31,636 , 30,510 <hr/> 35,547

* = (Original)
(Update)



Communications Report

Company: Texas Utilities

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☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program, Phase 3

Job No. 84042

Date: March 15, 1984

Subject: Fire Protection for Cable Trays

Time: 2:30 pm

Place: CPSES (site)

Participants: Ed Bezkor

of Gibbs & Hill

John Russ

Cygna

Item	Comments	Required Action By
	I spoke to Ed about the cable trays of the RHR system which may have fire protection applied to them. Ed told me that that information was available only on site. He suggested that I speak to the Site Civil Group regarding fire protection.	

Signed

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Communications Report

Company: Texas Utilities

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☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program, Phase 3

Job No. 84042

Date: March 15, 1984

Subject: Fire Protection and Hardware
Weights for Cable Trays

Time: 1:30 pm

Place: CPSES (site)

Participants: Doug Hunt

of Gibbs & Hill

D. Nandi

Gibbs & Hill

John Russ

Cygna

Item	Comments	Required Action By
	<p>I spoke to Doug and Mr. Nandi (both of Gibbs & Hill's Site Civil Group) about Mr. Walsh's request for weights of fire protection insulation and cover plates, splice plates and tray-support attachment hardware. Doug told me that the Site Hazards Group is responsible for determining which cable trays will require fire protection. This information is prepared by Mr. Jeff Spiegleman of TUGCO and sent by CPPA to the Civil Group for evaluation of effects due to any additional weight. This evaluation is performed by adding the additional weight into the existing cable tray weight and comparing the total value to the design weight of 35 psf.</p> <p>I gave Doug a copy of the RHR tray segments. To expedite our response to Mr. Walsh's question, I asked Doug to provide the following information:</p> <ol style="list-style-type: none">1. Tray segment unit weight for the size of each tray segment.2. Cable fill weight per tray segment.3. Weight of insulation per tray segment.4. If any tray cover, and if so, its unit weight.5. The combined weight of Items 1 through 4.6. Weight of splice ("doubler") plates for the size of each tray segment.7. Weight of the tray attachment clips and bolts for each attachment.	<p>Gibbs & Hill</p> <p>Gibbs & Hill</p> <p>Gibbs & Hill</p> <p>Gibbs & Hill</p> <p>Gibbs & Hill</p> <p>Gibbs & Hill</p> <p>Gibbs & Hill</p>

Signed:

/pm Page 1 of 1

Distribution: D. Wade, N. Williams, R. Hess, Project File, G. Grace



Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input checked="" type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	3/21/84
Subject:	Data Request	Time:	10:30 am
		Place:	Comanche Peak
Participants:	D. Rencher	of	TUSI
	J. Minichiello		Cygn

Item	Comments	Required Action By
4	<p>Cygn requested the following items:</p> <p>Baseplate output re-run dated 10:58:11.12...02/23/84 for calc CC-1-087-004-A33</p> <p>Revision 3 of CC-1-077-013-S435</p> <p>Documents SA-4124 and SA-4125 (Calc. MS-1-004-004-S72)</p> <p>STRU DL frame output for support CC-1-028-023-S33R</p> <p>STRU DL frame output for support MS-1-004-004-C72K</p>	

Signed: JC Minichiello /pm Page 1 of 1

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Communications Report

Company: Texas Utilities

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☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 3/20/84

Subject: Cygna Comment on MS

Time: 8:00

Place: Comanche Peak

Participants: G. Krishnan

of Gibbs & Hill

J. Minichiello

Cygna

Item	Comments	Required Action By
1.	<p>Cygna requested response to the following comment:</p> <p>For the MS lines, Cygna has not found any hydrotest analysis. Where is the documentation showing hydrotest stresses acceptable?</p>	

Signed:

J. Minichiello

/pm Page 1 of 1

Distribution: D. Wade, N. Williams, J. Minichiello, G. Grace, Project File



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 3/20/84

Subject: PUS U-Bolt Allowables

Time: 4:00 pm

Place: Comanche Peak

Participants: D. Rencher

of TUSI

J. Minichiello

Cygna

Item	Comments	Required Action By
	Cygna had requested allowables for PUH U-bolts above PUH-160. According the Mr. Rencher, allowables do not exist. Instead, the designers are directed to use appropriate calculations to qualify the U-bolts, based on the cross-section. The same is true for PUS allowables not shown.	

Signed

J. C. Minichiello

/pm

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of 1

Distribution: D. Wade, N. Williams, J. Minichiello, G. Grace, Project File



Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input checked="" type="checkbox"/> Conference Report
Project:	Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	3/14/84 - 3/17/84
Subject:	Tube Steel Allowable	Time:	1:00 p.m./ 8:00 p.m.
		Place:	Site
Participants:	D. Rencher	of	TUSI
	J. Minichiello		Cygna
	J. Finneran		TUSI/PSE

Item	Comments	Required Action By
	Cygna has noted that the allowables used for tube steel do not reflect Revision 10 of the N-71 Code Case, but only Revision 9. Mr. Rencher explained that TUSI was aware of the differences and suggested Cygna contact Mr. Finneran concerning TUSI's position. On 3/17, Cygna discussed the allowable with Mr. Finneran. Mr. Finneran stated that TUSI had documentation from the ASME Code permitting the use of the earlier revision. Cygna requested a copy of the documentation for review.	JF/TUSI

Signed: J. Minichiello /eam Page 1 of 1
Distribution: D. Wade, N. Williams, J. Minichiello, G. Grace, Project File



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 3/22/84

Subject: Mechanical Review Comments

Time:
Place: Comanche Peak

Participants: D. Rencher

of Texas Utilities

J. Minichiello

Cygna

Item	Comments	Required Action By
1.	<p>Cygna requested response to the following two questions.</p> <p>In CC-1-031-008-S33R, Cygna has noted another instance of a "fixed" joint at an embed plate, rather than a "pin" joint (see comments on MS-1-004-001-C72S, dated 3/21). In this case, a "fixed" assumption is needed, since self-weight was considered. The Civil Spec (SS-30) requires stiffeners be provided (see attached).</p> <p>a. Is piece 7 considered to be the stiffener?</p> <p>b. Has the Civil Group provided guidelines for stiffeners?</p> <p>c. What direction has TUSI given the support designers in using embed plates as "fixed" connections?</p> <p>d. The designer appears to use conservative calcs in determining plate loads. Then, he uses the highest plate allowables. What is TUSI's direction for determining embed plate allowables?</p>	

Signed: *J. Minichiello*

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AS-BUILT



- 4.) Locking devices for high strength bolts are not required per OCA 7607

VENDOR CERTIFIED
DRAWING REV. NO. 6
BY JD DATE 7-17-80

ITEM NO.	MATERIALS & OPERATIONS	QUAN	SHIP	PBS	L	CSS	PHIM	SEC	AI				
	SEISMIC SWAY STRUT ASSEMBLY CONSISTING OF:	ONE											
1	1" x 1/2" Hilti Kirk Concrete Anchors, TW	8				X							
2	Carbon Steel (SA-315 OR 65 or SA-36) Plate	2											
3	Welding (SA-36) 2" x 7 1/4" Long, TW-340	1				X	X						
4	Fig. 211 of Sway Brace Assembly, 18" O.D. Pipe, Carbon Steel, W-11", Load=3055#	1											
5	W-11 (SA-36) 1" O.D. x 7 1/8" Long, TW-400 As Shown Shop Center and Weld to (1) One of Legs	1				X	X						
6	MAX 1/2" (FIELD CUT AS SHOWN) SA 36	1											
	SEISMIC ASSEMBLY SKETCH AND ENGINEERING	1											
	BUNDLE AND TAG	1											
	MARK # CC-1-031-008-5338												
7	C.S. PLATE 2' x 2' SA 36	2											
	All material to be painted with Carbo-Zinc #11 except												
	Excess which shall be treated with a rust preventative.												
8	C.S. 3" x 6" x 6" SA 36	1											
9	MAX 1/2" x 1/4" x 3" SA 36	1											
REV	DATE	BY	CHK	APP	DESCRIPTION	QUAN	SHIP	PBS	L	CSS	PHIM	SEC	AI
1	10/1/80	1	1	1	AS BUILT, VENDOR CERTIFICATION								
2	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
3	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
4	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
5	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
6	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
7	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
8	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
9	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
10	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
11	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
12	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
13	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
14	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
15	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
16	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
17	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
18	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
19	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
20	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
21	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
22	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
23	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
24	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
25	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
26	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
27	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
28	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
29	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
30	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
31	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
32	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
33	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
34	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
35	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
36	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
37	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
38	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
39	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
40	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
41	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
42	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
43	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
44	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
45	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
46	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
47	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
48	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
49	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
50	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
51	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
52	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
53	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
54	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
55	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
56	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
57	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
58	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
59	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
60	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
61	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
62	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
63	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
64	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
65	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
66	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
67	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
68	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
69	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
70	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
71	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
72	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
73	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
74	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
75	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
76	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
77	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
78	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
79	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
80	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
81	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
82	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
83	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
84	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
85	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
86	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
87	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
88	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
89	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
90	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
91	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
92	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
93	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
94	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
95	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
96	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
97	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
98	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
99	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								
100	10/1/80	1	1	1	REV'D VENDOR CERTIFICATION								

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a concrete free edge (i.e., openings, face of beam, etc.) in any direction.

- 3.4 "Pin Connections" shall be assumed for load transfer to the embedded plates. Only forces normal to the embedment (P) and forces in the plane of the embedment (S) may be transferred to the embedment. Moments due to cantilever action or from any other source may be transferred to the embedment only when the embedment is stiffened for the calculated moment.
- 3.5 The loading pattern on sheet A4-1 and A4-3 assumes that the embedment is loaded at the midpoint of every span between pairs of studs for A4-1 and at every pair of studs for A4-3. In cases in which the load is distributed on more than one pair of studs, the full normal load (P) and only half of the plane load (S) should be considered when using the figures on sheet A4-2 and A4-4.
- 3.6 For capacity of embedded plate strips for loads acting on stud line see Cases 3 and 4 (A4-5 through A4-9)
- 4.0 CAPACITY OF EMBEDDED PLATE STRIPS FOR LARGE ECCENTRIC LOADING REV. 1
- 4.1 Tension and shear forces generated on the stud anchors by loads applied eccentric to the supporting stud group should be calculated to insure no failure of the stud anchors.
- 4.2 Ultimate tension and shear capacities of the stud anchors shall be taken from reference 2 of this Appendix.
- 4.3 The number of participating stud anchors may be increased by welding stiffeners to the embedded plate strips and to the support structure to ensure that the loading is spread to all the selected stud anchors. The embedded plate strip shall be checked for bending and shear. REV. 1
- 4.4 Steel plate material is A-36 Nuclear Safety Related as defined on Drawing 2323-S-0786 for embedded plate details.

TEXAS UTILITIES SERVICES INC.

COMANCHE PEAK S.E.S.

Agent For

DALLAS POWER & LIGHT COMPANY

TEXAS ELECTRIC SERVICE COMPANY

TEXAS POWER & LIGHT COMPANY

Filing Code

Sheet No. 9 of 12

G & H Job. No.

Sub'd/App'd. By

APB 4/2/52

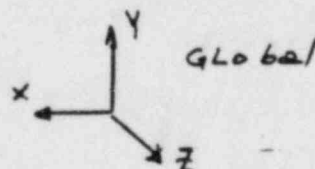
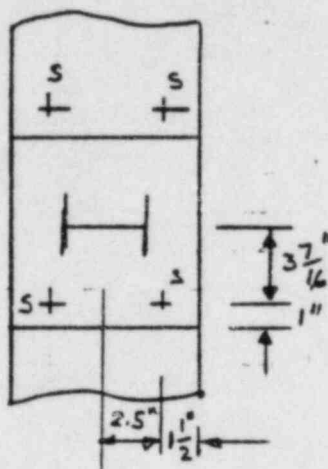
Subject **# CC-1-031-008-533R**

Rel. Dwg./Spec. No.

REF

AG

Mach plate at embedded pl.

Joint 3 Support STRUDL

$$\begin{aligned} F_x &= 131^\# \\ F_y &= 51^\# \\ F_z &= 2236^\# \\ M_x &= 1020^\# \\ M_y &= 2968^\# \\ M_z &= 2 \sim \text{NIL} \end{aligned}$$

$$F_{\text{shear}} = [131^2 + 51^2]^{1/2} = 141 \text{ #/60/t}$$

$$F_{\text{Tension}} = 2236 + \frac{1020}{3.94} + \frac{2968}{4} = 3237 \text{ N}$$

per SS-30 Appendix 4 page 4 of 6 Allow:

$$F_S = 21000^{\text{N}} > 141^{\text{N}} \quad \therefore \text{OK}$$

$$F_T = 6640^{\text{N}} > 3237^{\text{N}} \therefore \text{OK}$$

kicker plate at embedd. pl

$$\begin{array}{ll} L_x = 2903 & M_x = 391 \\ F_y = 39 & M_y = 300 \\ F_z = 2236 & M_z = 502 \end{array}$$

$$F_s = [2903^2 + 39^2]^{1/2} + 502 = 3405 \text{ #}$$

$$F_z = 2236 + \frac{391}{3.94} + \frac{300}{4} = 2410$$

$$3405^\circ < 21000^\circ \therefore OK ; 2410^\circ < 6640^\circ \therefore OK$$

Conservative
assumption
in both cases

sub
11/15



Communications Report

Item	Comments	Required Action By
2.	<p>In reviewing CC-1-028-022-S33K, Cygna has not found any checks of the hardware, other than references to previous design calcs. Cygna has checked piece 3 (24" special riser clamp) against the Grinnell load sheets (attached). Using the "max operating temp" of <200°F from the line list, the emergency allowable for the clamp is (@4' c-c):</p> $E = 1.25 (16800)$ $= 21000 \text{ lb}$ <p>Since $E_{act} = 21964 \text{ lb}$, this support may fail, assuming the above calc is OK.</p> <p>a. Since the calc states the original design loads were higher, what is the correct procedure for calculating clamp allowables?</p> <p>b. Has TUSI considered that one snubber may be more highly loaded than the other, due to pipe rotation?</p>	

ITT GRINNELL
PIPE HANGER DIVISION
QUALIFIED PRODUCT
LOAD RATINGS
FIG. 40N

Size	C to C IN.	Maximum Load Rating (Lbs.) at 650°F		
		Design Loading & Level A & B	Level C	Level D
18" PIPE	34	13300	17730	25000
	36	11700	15870	22370
	42	9200	12270	17300
	48	7900	10530	14850
20" PIPE	37	14600	19467	27448
	42	11650	15530	21900
	48	9400	12530	17670
	54	7900	10530	14850
24" PIPE	43	15750	21000	29610
	48	12600	16800	17770
	54	10400	13870	19550
	60	8800	11730	16540
28" PIPE	51	15300	20350	28765
	54	13780	18325	25905
	60	11500	15295	21620
	66	9865	13120	18545

LOAD RATINGS NOTES

1. AT REDUCED SERVICE TEMPERATURES, THE LOAD RATINGS MAY BE INCREASED BY THE FOLLOWING FACTORS:

<u>CLAMP TEMP. °F</u>	<u>LOAD FACTOR</u>
100	1.37
200	1.25
300	1.22
400	1.18
500	1.11
600	1.01
650	1.00

2. IF THE REQUIRED C TO C IS BETWEEN LISTED C TO C, USE THE LOAD RATING FOR THE GREATER C TO C. NO INTERPOLATION IS ALLOWED. C TO C'S OUTSIDE THE RANGES SHOWN ARE NOT ALLOWED.
3. IF UNEQUAL E DIMENSIONS ARE REQUIRED, USE THE LOAD VALUE LISTED FOR THE EQUIVALENT C TO C CALCULATED BY DOUBLING THE LONGER E DIMENSION. E DIMENSIONS OUTSIDE THE RANGES SHOWN ARE NOT ALLOWED. SEE NOTE 2. UNEQUAL E DIMENSIONS ARE ALLOWED ON VERTICAL PIPING ONLY.
4. THE LOAD RATINGS ARE FOR RIGID HANGERS; FOR SPRING HANGERS THE LOAD RATINGS MAY BE DOUBLED.

LIT GREENWELL CORPORATION
VENDOR CERTIFICATION
COMANCHE PEAK STEAM ELECTRIC STATION

Sheet 1 of 5

REV	ENGR	DATE	CHK'D DATE
0			
1	BG	4/13	AT 4/27
2			
3			

MR#: CC-1-028-022-533E
 SUBJECT: VENDOR CERTIFICATION
 CUSTOMER: T U S I
 PROJECT: CPSES

BRH REV: 7 LIT REV: 2
 DATA PT: 772
 GTN NO: 62995
 PROB NO: AB-1-6A R 2
 BRHL #: CC-1-63-05 R 4
 CPPA #:

CONDITION	Fx	Fy	Fz	Nx	My	Nz
DESIGN						
NORMAL/ UPSET	± 7539					
EMERGENCY	± 2124					
FAULTED						

SUPPORT DESIGN CODE

☒ ASME III THROUGH WINTER
 1974 ADDENDA

☐ B31.1 THROUGH
 ADDENDA

PIPELINE CLASS I, 2, (3) S

MR# CC-1-028-022-533E REV 7 is certified for the above
 specified loads.

DESIGN TEMP 200 °F AMBIENT TEMP 104 °F

1. DATE REV.....
2. REV. FILE # 1 REV.....
3. ASME III - NF.....
4. ASME III - APPENDICES.....
5. ASME III - APPENDIX XVII.....
6. PROJECT DANGER SPEC.....
7. ASME - STEEL MANUAL.....
8. ASME - STRUCT. WELDING - II, I.....
9. DESIGN OF WELDED STRUCTURES -
 BLACKSMITH.....
10. DESIGN OF WELDED FRAMES -
 WELDING.....
11. MAY 199/CHEM. STRESS REPORT.....
12. OTHER.....
13. ENGR. SIGN..... REV.....
14. ENGR. SIGN..... REV.....
15. ENGR. SIGN..... REV.....
16. ENGR. SIGN..... REV.....
17. STD. PROCS..... REV.....
18. STD. PROCS..... REV.....
19. STD. PROCS..... REV.....
20. S.A.S..... REV.....
21. S.A.S..... REV.....

ADDITIONAL INFORMATION		REV.
REF. TYPING		
RE: A-ENG		
RE: FUD IX		2
RE: S.A.S		
RE:		
RE:		
RE: CALC		

CHECK ALSO APPLY TO		REV.

CHE NO. _____ REV. _____ P. 1/1

10-1A AS-SIGMA CORP. PROJECT
THE GEORGINA CORPORATION

ENGL BY: <u>BG</u>	DATE: <u>4/19/83</u>	SK#	_____	MS#	<u>CC-1.028-022-533 K</u>
CHECK BY: <u>AT</u>	DATE: <u>4/20/83</u>	SK#	_____	MS#	_____
PROJECT: <u>CPSES</u>		SK#	_____	MS#	_____
PROB/ISO: <u>AB-1-GIA R2</u>		SK#	_____	MS#	_____
OTHER I.D.: <u>D.A. 772</u>		SK#	_____	MS#	_____

*These items only to be checked when loads and movements decrease.

ENGINEERING CHECK LIST**INITIAL CHECK****FINAL CHECK**

	YES	NO	N/A	YES	NO	N/A
PIPE SIZE ELEVATION	✓			✓		
STEEL SIZE ELEVATION						
DIMENSIONS						
INTERFERENCES						
OTHER PIPES						
ELECT TRAY						
DUCTS						
OTHER HANGERS						
INSULATION THICKNESS						
TRACEABILITY REQUIREMENTS						
WELD SIZES & TYPES						
HANGER COMPONENT SIZES	✓			✓		
NUCLEAR CODE CLASS						
TYPE OF SUPPORT						
TYPE OF MEMBER						
COORDINATE SYSTEM						
LOADINGS & CONDITIONS	✓			✓		
MOVEMENTS AND ROTATIONS	✓			✓		
NCE REQUIREMENTS						
MATERIAL IDENTIFICATION						
IMPACT TEST REQUIREMENTS						
ENGINEERING PARTS LIST						
DIMENSIONAL TOLERANCES						
FABRICATION PROCEDURES						
PAINTING REQUIREMENTS						
P.E. SIGN-OFF						
GENERAL NOTES			✓			✓
LITERATURE SEARCHES DOCUMENTED						
REFERENCE MATERIAL IS SPECIFIED						
ASSUMPTIONS DOCUMENTED						
ANALYSIS DOCUMENTATION IS LEGIBLE						
CALCULATIONS IDENTIFIED BY SYMBOL						
ACCEPTANCE CRITERIA IDENTIFIED						
DESIGN INPUT CRITERIA IS CORRECT						
ASSUMPTIONS ARE REASONABLE						
THE APPROPRIATE DESIGN METHOD						
WAS USED (YES)						
SPECIFIED EQUIPMENT IS SUITABLE						
FOR THE DESIGN APPLICATION						
DESIGN ANALYSIS RESULTS ARE REASON-						
ABLE COMPARED TO THE DESIGN INPUT						

SUPPORT LOAD TABLE

10/22/89

4 of 5

REV.	CALC./DATE	CHK'D./DATE	DESCRIPTION
0	GARY COVELL 8-12-82	MIKE SMITH 8-17-82	AS-BUILT LOADS PER ORIGINAL ISSUE OF PROBLEM
1	GARY COVELL 9-23-82	RC WOOD 9-25-82	TRANSFERRED SIGNATURES AND DATES, AS-BUILT LOADS PER REV.1 OF AB-1-61A
2	J.MCDONALD 1-10-83	<i>JK</i> /1-11-83	TRANSFERRED SIGNATURES AND DATES, AS-BUILT LOADS PER REV.2 OF AB-1-61A

PROBLEM NUMBER/REVISION AB-1-61A / REV.2 ; GTN # 62995

BRHL/REVISION CC-1-AB-007/REV.4 CC-1-SB-001/REV.3
CC-1-AB-013/REV.7 CC-1-SB-022/REV.2
CC-1-AB-049/REV.5 CC-1-SB-003/REV.4

CC-1-028-026-S33R NORMAL (A)

MODE PT. 842 UPSET (B)

EMERGENCY (C) / FAULTED (D)

"NORMAL OPERATING MODE" DISPLACEMENTS

Fx	Fy	Fz	ΔX	ΔY	ΔZ
—	-4307 -2690	-106 +49	+0.204 -0.401	—	—
—	-7482 +485	-4063 +4007	Fx	Fy	Fz
—	-8537 +1540	-5503 +5447	—	—	—

 $\Delta X =$ $\Delta Y =$ $\Delta Z =$

CC-1-028-024-S33R NORMAL (A)

MODE PT. 83 UPSET (B)

EMERGENCY (C) / FAULTED (D)

"NORMAL OPERATING MODE" DISPLACEMENTS

Fx	Fy	Fz	ΔX	ΔY	ΔZ
—	-4956 -964	-497 +1035	+0.222 -0.437	—	—
—	-11144 +5224	-3752 +4291	Fx	Fy	Fz
—	-12503 +6583	-4444 +4982	—	—	—

 $\Delta X =$ $\Delta Y =$ $\Delta Z =$

CC-1-028-023-S33R NORMAL (A)

MODE PT. 773 UPSET (B)

EMERGENCY (C) / FAULTED (D)

"NORMAL OPERATING MODE" DISPLACEMENTS

Fx	Fy	Fz	ΔX	ΔY	ΔZ
—	-7273 -4255	-1453 +695	+0.257 -0.508	—	—
—	-11880 +352	-6006 +5248	Fx	Fy	Fz
—	-13081 +1552	-7240 +6482	—	—	—

 $\Delta X =$ $\Delta Y =$ $\Delta Z =$

CC-1-028-022-S33K NORMAL (A)

MODE PT. 772 UPSET (B)

EMERGENCY (C) / FAULTED (D)

"NORMAL OPERATING MODE" DISPLACEMENTS

Fx	Fy	Fz	ΔX	ΔY	ΔZ
—	—	—	+0.255 -0.521	+0.003 -0.001	+0.001 -0.001
-17535 +17535	—	—	Fx	Fy	Fz
-21964 +21964	—	—	—	—	—

 $\Delta X = -0.402$ $\Delta Y = +0.002$ $\Delta Z = +0.001$

Y C S DRAFTING INSTRUCTIONSENG: BC DATE: 4/19/83 CHECK: AT DATE: 4/20/83MARK NUMBER: CC-1-028-022-533K

- ☒ Revise sketch revision level 7.
- ☒ Revise problem number AB-1-COIA R 2.
- ☒ Revise loads.
- ☒ Include BRHL number _____.
- ☒ Include GTN number 62995.
- ☒ Include CPPA number _____.
- ☒ Revise data point to _____.
- ☒ Revise movements X +.565/- .581 Y +.003/- .001 Z +.001/- .001.
- ☐ Add/remove one jam nut on item number _____.
- ☐ Change weld inspection to NF _____.
- ☐ Revise snubber setting on sketch AC _____, AH(1) _____, AH(2) _____.
- ☐ Remove C-C from sketch.
- ☐ Remove items _____ from B.O.M.
- ☐ Revise load in B.O.M. for item(s) # _____ to _____ #, # _____ to _____ #.
- ☒ Include normal operating movements, DX - .402", DY + .002", DZ + .001".
- ☐ Incorporate survey comments per CPP number _____.
- ☐ Revise base plate dimension as per TSDR number _____.
- ☐ Revise welds to meet min-weld requirement _____.

☒ Erase signature and date _____
 from vendor certified stamp _____

☐ Revise B.O.M. as shown below.

ITEM NO.	MATERIALS & OPERATIONS	QUAN.	SHIP.	PAS	L	CSS	PRIN.	SEC.	ATSC.

☐ OBTUSE AND/OR ACUTE ANGLE WELD NOT USED FOR ENGINEERING EVALUATION

TEXAS UTILITIES SERVICES INC.
COMANCHE PEAK S.E.S.

Agent For

DALLAS POWER & LIGHT COMPANY
TEXAS ELECTRIC SERVICE COMPANY
TEXAS POWER & LIGHT COMPANYDate 4/19/83Calc By EGChk'd/App'd. By AT 4/20/83Subject CC-1-028-022-533K

Filing Code _____

Sheet No 5 OF 5

G & H Job No. _____

Ref. Desig./Spec. No. _____

V.C. P.1

New load sheet as per CATN 62995.
However, loads are less than those used
in the design review, and snubber settings
are OK as per V.C.P.O. calc. Therefore
no further calcs req'd.

(NEW LOAD SHEET MOVEMENTS ARE SAME
AS OLD ONE A.T.)

AS-BUILT

INFORMATION COPY

THIS DOCUMENT IS FOR INFORMATION ONLY.
CONTACT DOCUMENT CONTROL FOR CURRENT
STATUS AND REVISION.

VENDOR CERTIFIED
DRAWING REV. NO. 7
BY *[Signature]* DATE 4/12/83

30MMES
TYP.
PLACES

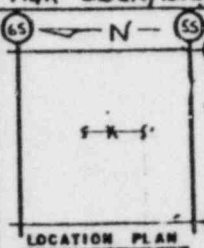
24-CC-1-28-152-3

PLAN VIEW @ EL. 806'-0"

NOTE:
4) LOCKING DEVICES FOR
HIGH STRENGTH BOLTS
ARE NOT REQUIRED
PER UCS 1001

MOVEMENT	
X	+ .752 / - .521
Y	+ .003 / - .001
Z	+ .001 / - .001
NORM. OPER. DISPL.	
DX	- .402
DY	+ .002
DZ	+ .002

SEE BRHL FOR
MGR LOCATION



THIRD PARTY INSPECTION
CODE CLASS: ASME III - 3

T.O. 1101

BRHL Iso. CC-1-58-03 R.4
I.P.D. Iso. CC-1-58-03
Data Point 772 / Pipe AB-1-141A R.2
Pipe Mat'l. SA 106 GRB
Insul. 1/2" Bldg. SB

REV	DATE	OWN	CHK	APP	DESCRIPTION
2-41	81	FW	HC		REV'D AS NOTED: REF CMC 37330 1-14 1298 & DELETED F.W.'S 1-10 & DELETED NOTES 1-3 & ADDED SHT. 2 OF 2
11-3	81	JED			REV'D AS NOTED: REF CMC 52813 DCA 7607 SEE HT. 4 AS BUILT
12-21	82	FW	HC		REV'D AS BUILT: REF CMC 7727 R.1 MIDW. 1-14 1298 & DELETED F.W.'S 1-10

FOR OFFICE AND ENGINEERING USE ONLY

MATERIALS & OPERATIONS

MECHANICAL SHOCK SUPPRESSOR
ASSEMBLY CONSISTING OF:
MATERIAL EXISTS
No. 35, 6" Stroke, Fig. 307N Mechanical
Shock Suppressor, W-0-8 374

Lead=10430 "Ordered in Pairs"
1 3/4"x4" Carbon Steel (SA315 Gr. 65) Lugs
0-5" Long, TW-79#
1 1/2" Hex Nuts with concrete Anchors (1144)
1 1/4"x1 3/8" Carbon Steel (SA 515 Gr. 65 or
SA 36) Plate, 1 3/8" Long, TW-160#

NEW MATERIAL REQUIRED
3 Special 24" Fig. 40N Riser Clamp, C-C=4'-0"
S=2" Load Stud=1.497/1.499 x 1 1/2"
W/Thread Both Ends W/3" Unthreaded Section
W/ (2) Hex Nuts & (2) Jam Nuts W/Washers
for Use with #35, 6" Stroke, Fig. 307N
6 1/4" C.S. R / PER SECT. A-A SA-36
7 1/4" C.S. R / PER SECT. A-A SA-36

SEISMIC ASSEMBLY SKETCH AND ENGINEERING
BUNDLE AND TAG
MARK # CC-1-028-022-S33K
8 1"x12" SUPER HILTI'S CONCRETE ANCHORS

Apply one coat of Carbo-Tite #11		DESCRIPTION	
Q	Q	REV. VENDOR CERTIFICATION.	
Q	Q	REV. VENDOR CERTIFICATION.	
Q	Q	REF: GTH 62995	

FOR MATERIALS AND OPERATIONS SEE SKETCH NO.

Brown & Root, Inc.					CONDITIONS		Fz	Fy	Fz	Mx	My
					DESIGN						
REF. DRAWING NUMBERS					NORMAL & UPRGT		*11555				
PIPE: MI-0604-REV. ELECTRIC-0601-REV. 1					EMERGENCY		*2174				
STEEL: SI-0602-REV. 12, H.V.A.C. MI-0601-REV. 1					FAULTED						
REV	DATE	OWN	CHK	APP	DESCRIPTION		CUSTOMER Texas Utilities Service, Inc.				
△	2-41	FW	HC		ISSUED FOR CONST		ORDER OR CONT. NO. CP-0046				
					FW 1-10		JOB NAME Cosanche Peak 1 & 2				
△	11-3	FW	HC		REVISED PER ITT REV. 1		MARK NO. CC-1-028-022-S33K				
△	12-21	JW	HC		REV'D PER ITT REV. 2		SKETCH NO.				
					CONT		SHEET 1 OF 2				
							REV 7				



Communications Report

Company: Texas Utilities

☐ Telecon

☒ Conference Report

Project: Comanche Peak Steam Electric Station
Independent Assessment Program - Phase 3

Job No. 84042

Date: 6/12/84

Subject: Pipe Stress Review - Welded Attachments

Time: 12:45 p.m.

Place: Site

Participants: C. Ray

of G&H

J. Minichiello

Cygna

Item	Comments	Required Action By
	<p>In the original review of AB-1-61A, Cygna did not find a welded attachment calculation for CC-1-028-713-A33K. G&H stated that the calculation was referenced in the back of the QA books, under CC-1-028-113-A33K (by mistake). Cygna requested a copy of the G&H memo AM-WA-82 dated 5/12/82, (To: B. Roy/F. Colucci, From: P. Bogart/V. Chirila) and verified that support -713 is similar to support -004, as stated in the memo (713 has a slightly larger trunnion diameter of 16" vs. 14").</p>	

Signed:

N. Williams

/ms Page 1 of 1

Distribution: N. Williams, D. Wade, G. Grace, J. Minichiello, S. Treby, J. Ellis, S. Burwell,

Project File



Communications Report

Company:	Texas Utilities	<input type="checkbox"/> Telecon	<input checked="" type="checkbox"/> Conference Report
Project:	Comache Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	6/20/84
Subject:	Pipe Support Review	Time:	3:30 p.m.
		Place:	Site
Participants:	D. Rencher	of	TUEC
	J. Minichiello		Cygna

Item	Comments	Required Action By
	In response to Cygna's question on MS-1-004-006-C72K (see conf. report 6/15/84), Texas Utilities ran a finite element model of the connection. Their response is attached.	

Signed: *NH Williams* /ms Page 1 of 1
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REC'D 6/20

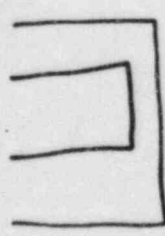
CYGNA RESPONSE

DATE OF QUESTION: 6/15/84

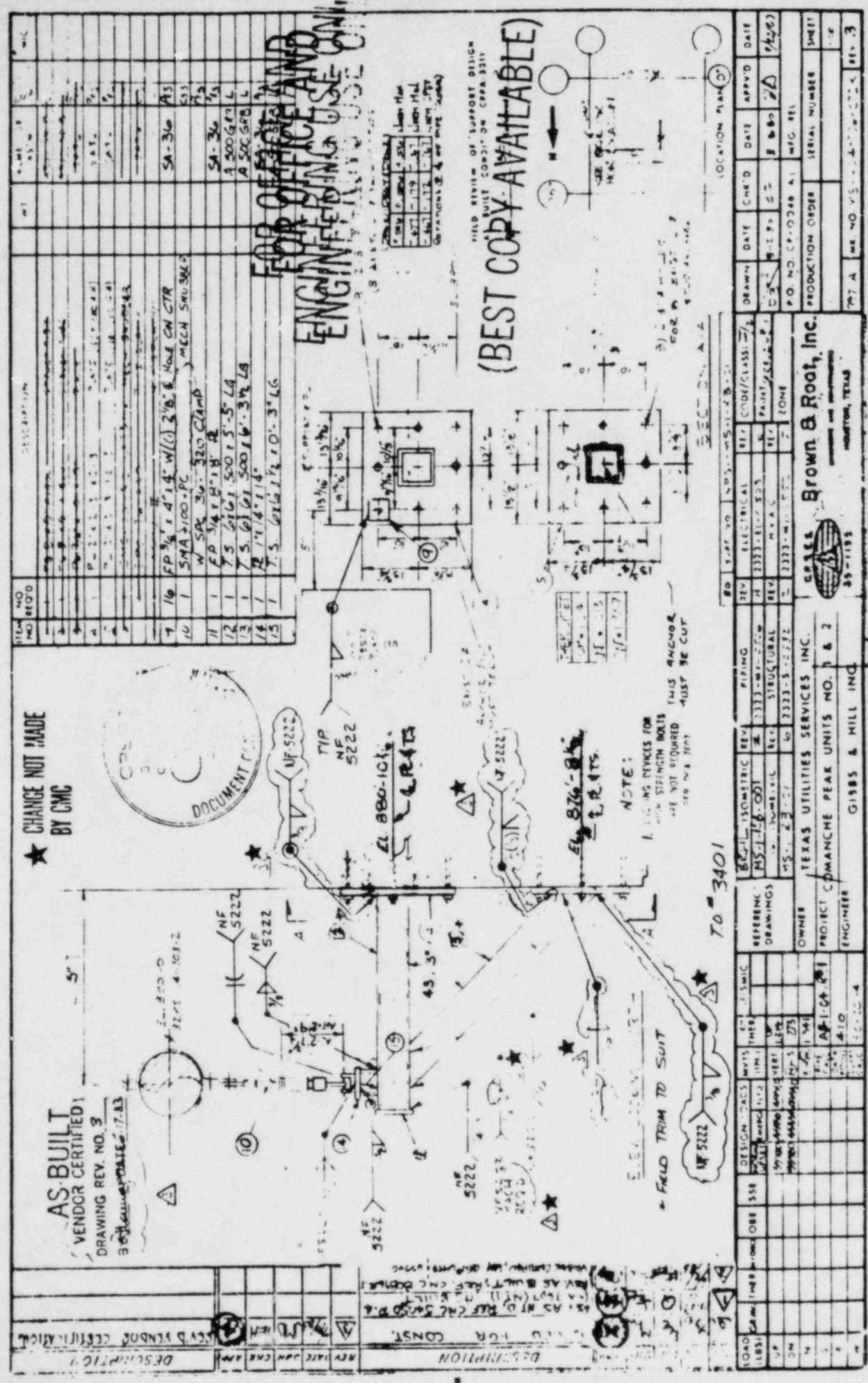
MSI-004-006-C72K

Calculation of 1" x 14" x 14" plate attached. Finite element analysis shows stress to be 22,540 psi in the upset condition (see p. 100 of output).

$$22,540 \text{ psi} \leq 0.75(32,040) = 24,030 \text{ psi. } \text{ok} \checkmark$$



12x 12 1/2 x 2
TO 1 = 8
L = 3



NO	REV	DESCRIPTION	DATE	BY	CHKD	DATE	APPD	DATE
1		AS-BUILT						
2		VENDOR CERTIFIED						
3		DRAWING REV. NO. 3						
4		BY DATE 11-17-83						
5								
6								
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14								
15								

NO	REV	DESCRIPTION	DATE	BY	CHKD	DATE	APPD	DATE
1		AS-BUILT						
2		VENDOR CERTIFIED						
3		DRAWING REV. NO. 3						
4		BY DATE 11-17-83						
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NO	REV	DESCRIPTION	DATE	BY	CHKD	DATE	APPD	DATE
1		AS-BUILT						
2		VENDOR CERTIFIED						
3		DRAWING REV. NO. 3						
4		BY DATE 11-17-83						
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NO	REV	DESCRIPTION	DATE	BY	CHKD	DATE	APPD	DATE
1		AS-BUILT						
2		VENDOR CERTIFIED						
3		DRAWING REV. NO. 3						
4		BY DATE 11-17-83						
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Communications Report

Company:	Texas Utilities	<input checked="" type="checkbox"/> Telecon	<input type="checkbox"/> Conference Report
Project:	Comache Peak Steam Electric Station Independent Assessment Program - Phase 3	Job No.	84042
		Date:	7/11/84
Subject:	Pipe Support Questions	Time:	11:00 a.m.
		Place:	SF
Participants:	J. Finneran	of	TUGCO
	J. Minichiello, N. Williams		Cygna

Item	Comments	Required Action By
	Cygna requested response or clarification on the following issues:	
1)	<p>For checking the welds for composite sections (i.e., a plate welded top and bottom to tube steel or a wide flange), Cygna noted errors in the following calculations.</p> <ul style="list-style-type: none">a) MS-1-003-005-S72R, Rev. 1b) MS-1-002-007-C72K, Rev. 2c) MS-1-004-004-S72R, Rev. 0d) MS-1-001-003-S72R, Rev. 2e) MS-1-002-004-S72R, Rev. 1f) MS-1-003-006-S72R, Rev. 3g) MS-1-004-003-S72R, Rev. 4 <p>Mr. Finneran will review these calculations and call Cygna back later today to discuss them.</p>	
2)	<p>For double trunnions, riser clamps, or trapeze arrangements with cinched U-bolts, TUGCO has answered that they use a 50/50 load split on the two sides, rather than, say, 66/33. For justification in terms of piping rotation, Mr. Finneran referenced his Affidavit on axial restraints and torsional loads due to piping rotation. Cygna will review this document as soon as we receive it. Cygna also referenced NUREG/CR-2175, which discusses the effect of clearance mismatch on load split for dual snubber arrangements. Mr. Finneran will obtain a copy for review. Mr. Finneran did state that the imbalance due to rotation was a self-limiting load, since one calculated it from the full piping rotation. Mr. Finneran also referenced his Affidavit on seismic safety factors. Cygna will also review this document.</p>	

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