



ENERGY
SERVICES

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415 397-5600

March 15, 1984
84042.07

50-445/446

Mrs. Juanita Ellis, President
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1426 South Polk
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Subject: Comanche Peak Steam Electric Station Independent Assessment Program -
Response to CASE Questions

Reference: (1) Brief Summary of Generic Problems from CASE Witness Jack Doyle,
2/22/84.

(2) Brief Summary of Cross-examination Questions from CASE Witness
Mark Walsh, 2/22/84.

Dear Mrs. Ellis:

Enclosed please find our responses to reference (1) item 10; and reference (2) items 2 and 11.

Further responses will be forthcoming.

Very truly yours,

N. H. Williams

Nancy H. Williams
Project Manager

NHW:eam

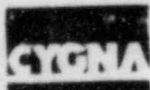
Enclosures: Attachment A, Partial Responses to
CASE Questions

cc: See attachment

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PDR ADOCK 05000445
A PDR

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1/1

San Francisco Boston Chicago Richland



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Mrs. J. Ellis
Response to CASE Questions

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March 15, 1984
Attachment

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1.0 CASE Question

Changing from a flare weld to a fillet weld induces flange bending. Has this been addressed by Cygna?

2.0 Cygna Interpretation

The calculation sheet attached to Exhibit 893 states that the weldment between the rear bracket and the beam flange was changed from a fillet to a flare/bevel weld. This fillet-to-flare change results in a 90 degree re-orientation of the weld lines, from perpendicular-to-parallel to the web of the wide flange. Did Cygna evaluate the additional loads on the flange?

3.0 Response

Cygna judged that this re-orientation would not cause an overstress in the flange. The following calculation verifies that judgement:

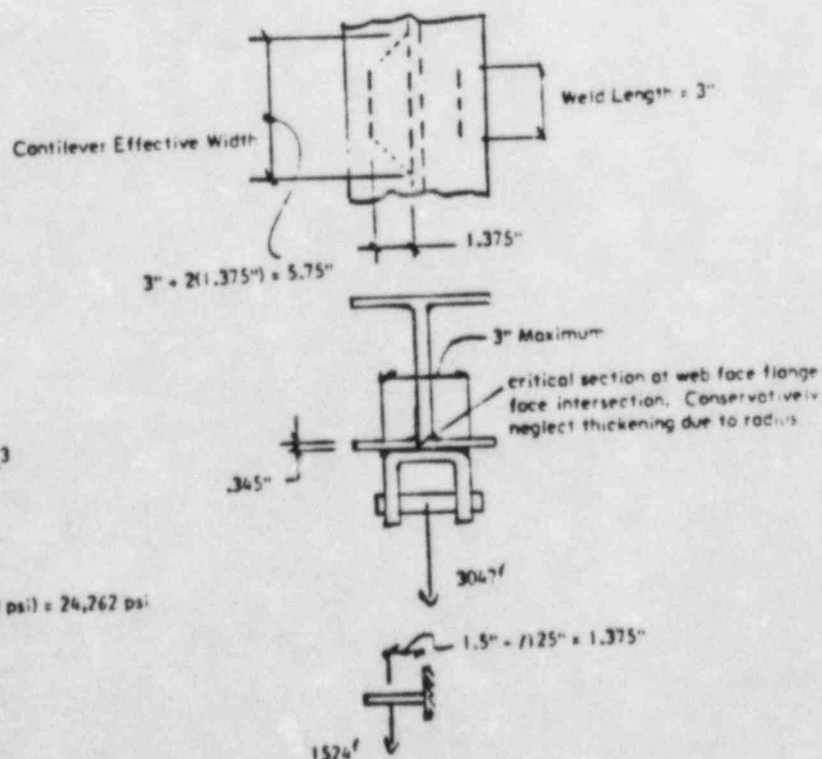
Check Flange Bending
 Support No. SI-1-079-001-5325

$$S = \frac{b^3}{6} = \frac{5.75}{6} (1.345)^2 = .114 \text{ in}^3$$

$$= \frac{M}{S} = \frac{1524(1.375)}{.114} = 18,382 \text{ psi}$$

$$\text{Allowable stress} = .75F_y = .75(32,350 \text{ psi}) = 24,262 \text{ psi}$$

$$18,382 < 24,262 \text{ psi}$$



Comanche Peak ASLB Hearings
Response to CASE Questions
Question No.: Walsh #2
Exhibit No.: None

1.0 CASE Question

Observation Record PS-02-01. The applicant did not consider shear cone interaction of adjacent bolts.

2.0 Cygna Interpretation

Cygna Observation PS-02-01 was written to evaluate an apparent discrepancy between drawing information and calculations, as related to anchor bolt embedment lengths. Was shear cone interaction also addressed?

3.0 Response

Observation PS-02-01 identifies a concern with the calculation of bolt embedment lengths and the direction provided to the field for the installation. Further investigation revealed that the embedment was provided to the constructor as a function of total bolt length which is specified on the drawing. In addition, the greater of the two embedments derived from either the construction specification or the drawing governs.

Although not related to this concern, Cygna did check both the analyses and construction to ensure that bolt spacing requirements were met. Minimum bolt spacing criteria are necessary to assure full development of bolt capacity as specified by the manufacturer. Maximum bolt capacity is realized when the concrete shear cone is fully developed without interferences. Interaction or overlapping between adjacent bolt shear cones will reduce bolt capacity as a function of bolt diameters. The applicant properly considered these effects.



1.0 CASE Question

Pipe stress checklist, note 3, item a:

- 1) What is the basis for considering that the effects were negligible?
- 2) What pipe stress run did Cygna look at, since the inclined load was used in the design of support RH-1-010-003-S22R?

2.0 Cygna Interpretation

Pipe stress checklist (PI-02), note 3, states the following:

3. The following supports were modeled along the coordinate axis rather than inclined. The impact is negligible.

- a. FH-1-010-003-S22R @ data point 1253 (9.6 degrees)
- b. SI-1-042-001-S22R at data point 793 (7.5 degrees)

What was the basis for concluding that support RH-1-010-003-S22R was adequate?

3.0 Response

Support RH-1-010-003-S22R is a simple restraint, inclined 8.6 degrees from a line drawn perpendicular to the pipe. Cygna judged that this small inclined angle would not significantly affect the support design or the piping analysis. This judgement was based on the following considerations:

- the fabrication tolerance is 5 degrees, therefore the misalignment is only 3.6 degrees beyond tolerance.
- the axial restraint load will increase by $1/\cosine\ 8.6\ degrees = 1.011$ or 1%.

In order to verify the adequacy of this judgement, Cygna requested that Gibbs & Hill reanalyze piping segment AB-1-70. For this reanalysis, the piping model was revised to include the following:



- Supports RH-1-010-003-S22R and SI-1-042-001-S22R were modeled with skew angles of 8.6 and 7.5 degrees, respectively.
- Support RH-1-010-003-S22R was modeled as two trunnions with snubbers located 7 inches from the pipe centerline.
- Support RH-1-064-010-S22R was modeled 1'-4" west of the elbow.

The results of this reanalysis are summarized below:

Maximum System Stress (psi)

ASME Equation

	Old	New
8	9039	9039
9 (upset)	21094	21103
9 (emergency)	24451	24463
10	22883	22883
11	27881	27881

Support Loads (lbs)

	Normal		Upset		Emergency		Allow.
	Old	New	Old	New	Old	New	
RH-1-010-003-S22R	1705	1459	3534	4519	3967	5189	15700
•	105	164	-1724	-2894	-2756	-3565	-15700



Comanche Peak ASLB Hearings
Response to CASE Questions
Question No.: Walsh #11
Exhibit No.: None
Page 3

Nozzle Loads (lbs)

	<u>Old</u>	<u>New</u>
Load	3084	2541
Allowat	3120	3120
Ratio	0.98	0.81

Therefore, the correctness of the original engineering judgement is verified. Enclosure W11-1 contains the computer output for this reanalysis.



Gibbs & Hill, Inc. Job No. 2323 Client TUSI/CPSES
 Subject AS BUILT STRESS VERIFICATION - ANALYSIS DATA/SPECIAL CALCULATIONS
 Calculation Number AB-1-70 Sheet No. 1 of 2

Revision	Original Issue	Date	Rev. #	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method #			1							
Preparer			AKP	3-1-84						
Checker			CV	3-1-84						
DR			AKP	3-2-84						

* REANALYSIS OF STRESS PROB. AB-1-70, ISSUE 0 HAS BEEN PERFORMED STRICTLY TO INCORPORATE THE FOLLOWING COMMENTS MADE BY CYGNA INC. DURING THEIR AUDIT OF ISSUE 0 OF THE ANALYSIS.

- (1) SUPPORT NO. RH-1-010-003-S22R AT NODE POINT 1253 IS SKEWED AT 8.6° AND SUPPORT NO. SI-1-042-001-S22R AT NODE POINT 793 IS SKEWED AT 7.5° PER THE RESPECTIVE BRH SKETCHES.
- (2) SUPPORT NO. RH-1-010-004-S22X AT NODE POINT 252 IS A PAIR OF SHUBBERS. THESE HAVE BEEN REMODELED AT THE TWO STUNCHIONS AT DATA POINT 8252 AND 6252 PER THE BRH SKETCH.
- (3) SUPPORT NO. RH-1-064-010-S22R AT DATA POINT 2245 WAS EARLIER CALLED SUPPORT NO. RH-1-062-001-S22R AND HAS BEEN MODELED AT 1'-4" WEST OF THE ELBOW INSTEAD OF 2'-6" WEST OF THE ELBOW AS MODELED EARLIER.
- (4) THE INPUT AND OUTPUT OF THIS REANALYSIS IS STORED AS SHOWN BELOW.
 INPUT: FILE NAME: TUSI.AB\$1\$70.CYGNA
 TAPE # T 02630, LABEL # 023
 OUTPUT: FILE NAME: TUSI.AB\$1\$70.CYGNA
 TAPE # T13039 LABEL 001

THE NOZZLE LOADS FOR THE INLET NOZZLE OF THE RHR HEAT EXCHANGER TBX-RHAHRS-02 AT N.P. 1255 ARE TABULATED AND COMPARED WITH THE ALLOWABLE NOZZLE LOADS OF THE SAME AND ARE FOUND TO BE ACCEPTABLE.

Checking Method #

1 Line-by-line checking
 2 Alternative Calculation Results compared
 3 Special Calculation Results compared
 4 Compare inputs and results of computer with corresponding inputs and results of similar codes

F-166, 7-82

Gibbs & Hill, Inc. Job No. 11-2323-030 Client TUSI PLS

Subject AB-1-70 VERIFICATION OF DESIGN STRESS ANALYSIS

Calculation Number: AB-1-70 Sheet No. 2 of 2

Rev	Date	Rev	Date	Rev	Date	Rev	Date	Rev	Date
1		1		1		1		1	
2		2		2		2		2	
3		3		3		3		3	
4		4		4		4		4	
5		5		5		5		5	
6		6		6		6		6	
7		7		7		7		7	
8		8		8		8		8	
9		9		9		9		9	
10		10		10		10		10	
11		11		11		11		11	
12		12		12		12		12	
13		13		13		13		13	
14		14		14		14		14	
15		15		15		15		15	
16		16		16		16		16	
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99		99		99		99		99	
100		100		100		100		100	

RHR HT. EXCHANGER (INLET)

BRP/01111 RH-1-SB-003 NODE: 1255

IDENT: TBX-RHAHRS-02

	NORMAL		UPSET		EMERGENCY		FAULTED	
	AS-BU.	ALL	AS-BU.	ALLO.	AS-BU.	ALLO.	AS-BU.	ALLO.
F _x	+84	1560	+1075	3120	+1445	6240		
	-81		-1072		-1442			
F _y	+441		+2363		+3000		BOUNDED	
	-118		-2040		-2678			
F _z	+164		+935		+1105		BY	
	-119		-890		-1060			
FR/Shear	471	1560	2541	3120	3197	6240	EMERGENCY	
	168		2226		2880			
M _x	+187	7811	+1596	15622	+1969	31244	CONDITION.	
	-468		-1877		-2250			
M _y	+298		+2021		+2359			
	-475		-2197		-2535			
M _z	+772		+2503		+2906			
	+165		-1565		-1969			
MR/Bend	+828	3905	3217	7811	3743	15622		
	503		2697		3210			

BRP/01111

NODE:

IDENT:

	NORMAL		UPSET		EMERGENCY		FAULTED	
	AS-BU.	ALL	AS-BU.	ALLO.	AS-BU.	ALLO.	AS-BU.	ALLO.
F _x								
F _y								
F _z								
FR/Shear								
M _x								
M _y								
M _z								
MR/Bend								

Notes: 1) Forces are in lbs, moments in Ft-lbs

2) For allowable loads see *** (3) For anchors, complimentary problem results must be

tabulated.

F-165, 4-81

*** CALCULATION BOOK OF AB-1-70

Checking Method #

1. Use the following
2. After the calculation results are checked
3. Compare the calculation results with the
4. Compare the results and results of the calculation with the results of the calculation

DESIGN REVIEW

RECORD FORM

Texas Utilities Services, Inc. Comanche Peak S.E.S.
CLIENT PROJECT

2323
O&H JOB NO.

Title: AB-1-70 ISSUE #

☐ Drawing

☒ Calculation

☐ Specification

AB-1-70
DOCUMENT NO.

*
REVISION NO.

3/1/84
DATE

COMMENTS ARE AS NOTED ON DOCUMENT SHEETS LISTED BELOW EXCEPT AS
STATED HEREIN:

NOTE: * Q.A Design Review has been
performed only on the incorporation of the items
defined on sheet 1 of the attached calculation of
load AB-1-70. All other input data are as per
issue 0 of the analysis AB-1-70

REQUIRED ACTION NONE

Walter D. Rogers
DESIGN REVIEW ENGINEER

3/2/84
REVIEW DATE

REQUIRED ACTION SATISFACTORILY COMPLETED YES

☐ NO ☐

COMMENTS

DESIGN REVIEW ENGINEER

REVIEW DATE