

ENVIRONMENTAL QUALIFICATION

FOR CLASS 1E LEAD-ACID

STORAGE BATTERIES

BYRON/BRAIDWOOD

COMMONWEALTH EDISON

SARGENT AND LUNDY ENGINEERS

P.O. 194757, 758

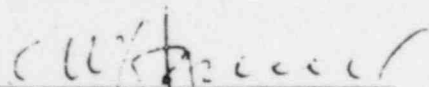
KE 5145, 5147, 5149, 5186

GOULD INC.

INDUSTRIAL BATTERY DIVISION

LANGHORNE, PA.

PREPARED BY:

  
R. W. HOPEWELL

APPROVED BY:

  
R. H. DESAI

PCT/112-0

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TEST PROCEDURES  
FOR THE  
GENERIC QUALIFICATION OF CLASS 1E LEAD-ACID STORAGE BATTERIES  
PLANTE & CALCIUM TYPES

Document No. GB-3454

Revision #11-Z November 19, 1982

Architectural/Engineering Firm:	Sargent & Lundy Engineers
Customer:	Commonwealth Edison Co.
Project:	Byron/Braidwood
P.O. Number:	194737, 758
Shop Order No.:	KE 5145, 5147, 5149, 5186

GOULD INC.  
INDUSTRIAL BATTERY DIVISION  
LANGHORNE, PA.

Appl. Eng'g. File No.  
-068399

Ref: IEEE Std. 323-1974

SECTION #1  
TEST PROCEDURE  
GB-3454

- A. Revisions #1 through #10 involved testing of accelerated heat aging of lead-acid storage batteries as outlined in IEEE 535-1979 Section 8.
- B. Revision #11 and any subsequent revision involves the use of naturally aged lead acid storage batteries as outlined in IEEE 535-1979 Section 8.

## 1.0 SCOPE

1.1 This test plan outlines the steps to be used by Gould Inc. Industrial Battery Division to establish the qualified life of their stationary battery cells, as described in Paragraph 1.2. The test program will be performed in accordance with the requirements of IEEE 323-1974, IEEE 344-1975 and as a guidance IEEE 450-1980 and IEEE 535-1979.

Seismic qualification of battery racks will be done by analysis on a job-by-job basis, and is not to be considered a part of this Test Procedure.

Cells to be tested are NCX-1680 type cells, which are in the mid-range size for this type. IEEE 535-1979 states "that qualification may be accomplished by type testing". It also states that "General practice is to qualify one or more sizes of a cell type and interpolate or extrapolate to other size cells of that type.

Definitions Section 2, Page 5 defines Cell Type as cells of identical design, for example, plate size, alloy, construction details, but that may have differences in the number of plates and spacers, quantity of electrolyte, or length of container.

The aging mechanism and seismic test portion of the test program shall consist of the following sequence.

1. Naturally Aged Cells
2. Pre-Seismic Capacity Test
3. Seismic Qualification Test
4. Post Seismic Capacity Test
5. Functional Analysis

The test specimens have the following history. Specimens were owned and operated by New Jersey Bell Telephone Company in their Bordentown, New Jersey Central Office for telephone standby service. Cells are ten years old and have been operated in a full float method of charge @ 2.17 V.P.C. Battery was located in an air-conditioned room with an average ambient temperature of 78°F. Relative humidity was 40%. Vibration level was not obtainable.

## 1.2 Seismic Test Cell Description (See Appendix II)

	<u>Type</u>	<u>Quantity</u>	<u>Size (WxLxH)</u>	<u>Weight (Approx.)</u>	<u>8 Hr. Capacity</u>
#1	MCX-510	3	11.125" x 6.5" x 18.25"	119 lbs. ea.	510
#2	NCX-1680	3	11.375" x 14.5" x 22.5"	332 lbs. ea.	1680
#3	FPR-23	3	13.0" x 12.5" x 19.875"	270 lbs. ea.	880
#4	*FPS-25	3	13.0" x 12.5" x 19.875"	287 lbs. ea.	996

\* FPR and FPS are of identical design. Change of designation to FPS was capacity rating only.

## 1.3 Test Fixtures

Special racks for holding the battery cells during seismic qualification will be provided by Gould IBD. Calculations demonstrating fitness of these racks for Gould's Required Response Spectra and comments or extrapolation of these racks to all seismic racks of similar design will be demonstrated in Section 5, "Seismic Calculations for Test Rack".

#### 1.4 Test Cells

The test cells used shall be naturally aged cells obtained from utility customer installations, thus exposing the cells to actual operating conditions and natural environmental influences. A description of each specimen source and operating conditions will be provided in the final report.

#### 2.0 PRE-SEISMIC CAPACITY TEST

- 2.1 Naturally aged test cells obtained from the utility customers will be transported to an outside test facility. Test specimens to be inspected and evaluated at customer installation to assure suitability for qualification purposes.
- 2.2 Test cells will be given an equalizing charge per IEEE 450-1980 to ensure a fully charged condition. Cells will then be placed on float charge at ambient temperature for 3 to 6 days prior to the Pre-Seismic qualification capacity test. MCX and NCX types will be on float charge @ 2.25 V.P.C. average and FPR and FPS types @ 2.19 V.P.C. average. Each three cell group voltage will be maintained through the float period within a tolerance of  $\pm 0.01$  volt per three cell group.
- 2.3 Test cell electrolyte level will be maintained within 1/4 inch below and at the high level line during the float charge period.
- 2.4 Following the float charge period, a Pre-Seismic capacity test will be conducted at the published 3 hour rate to 1.75 V.P.C. average per IEEE 535-1979 Para. 8.2.2 (1). If the capacity test is less than 80 percent of rated capacity, the cells may be recharged, returned to float at ambient temperature for a minimum period of 72 hours and retested. If the cells fail the second capacity test, the cells shall have failed.

### 3.0 SEISMIC QUALIFICATION (PER IEEE 344-1975)

#### 3.1 Seismic Mounting and Orientation

Each group of three fully recharged cells mounted in Gould-furnished racks, attached to a test lab-fabricated surface mounting fixture, hereinafter called the specimen, will be installed on a Multiaxis Simulator Table such that its longitudinal axis will be colinear with the longitudinal axis of the table. For the second axis of the test, the specimen will be rotated 90 degrees in the horizontal plane.

The racks are designed so that the natural frequency of the racks is in the rigid region (above 33 cps.).

Rack calculations will be provided in the final test report along with transmissibility plots to demonstrate rigidity of the rack design.

#### 3.2 Specimen Tie-down

The mounting of specimen will simulate the actual in-service configuration as closely as practical, using standard intercell connectors, terminal plates, terminal cables and terminal hardware. The test racks will be bolted to the test lab-fabricated surface mounting fixture. The surface mounting fixture will be welded to the shock table using 2" skip welds.

#### 3.3 Excitation

##### 3.3.1 Simultaneous Biaxial Excitation

Each horizontal axis will be excited separately, but each one will be excited simultaneously with the vertical axis (longitudinal simultaneously with vertical, then lateral simultaneously with vertical). The horizontal and vertical input acceleration levels will be phase incoherent during the multifrequency tests.

### 3.3.2 Low-Level Resonant Search

A low-level (approximately 0.2g both horizontally and vertically) sine sweep will be performed on each axis of the test configuration from 0.5 Hz to 35 Hz to 0.5 Hz at a sweep rate of one-half octave per minute to establish major resonances. Transmissibility plots of the resonant search will be provided in the test report.

### 3.3.3 Multifrequency Tests

The specimen shall be subjected to 30-second duration simultaneous horizontal and vertical inputs of random waveforms motions consisting of frequencies spaced one-third octave apart over the range of 0.5 Hz to 40 Hz. The amplitude of each one-third octave frequency band width will be independently adjusted in each axis until the Test Response Spectra (TRS) envelop the Required Response Spectra (RRS) (Appendix III, IV or V).

The resulting TRS will be analyzed by a spectrum analyzer at 0.5, 1, 2, 3 and 5 percent damping and plotted at one-third octave frequency intervals over the frequency range of interest. The Zero Period Acceleration (ZPA) of the RRS could be exceeded in order to meet the peak responses of the curves. The horizontal and vertical inputs will be phase incoherent.

### 3.3.4 Operational Basis Earthquake (OBE) Tests

Five OBE tests shall be performed in each test axis. Duration of the OBE tests will be 30 seconds. The TRS for the OBE tests will be analyzed at 0.5, 1, 2, 3 and 5 percent damping.

### 3.3.5 Safe Shutdown Earthquake (SSE) Tests

One SSE test will be performed in each test axis. Duration of the SSE test will be 30 seconds. The TRS for the SSE tests will be analyzed at 0.5, 1, 2, 3 and 5 percent damping.

#### 4.0 INSTRUMENTATION

##### 4.1 Excitation Control

Control accelerometers will be mounted on the table at locations near the base of the specimen.

##### 4.2 Specimen Response

Eight specimen mounted uniaxial accelerometers will be provided during the test. Placement of the accelerometers will be as directed by the Gould Technical Representative and the Test Engineer. Control and specimen accelerometer plots at the required damping of the SSE test in each axis will be provided in the test report.

Accelerometer placement will be similar to previous seismic tests performed by Gould at Wyle Labs, and used by many consulting engineers and utilities. All accelerometers will be placed in a proper position engineering-wise and will be located to provide the best possible, most meaningful results. Actual locations and photographs of accelerometer placement will be provided in the Final Test Report.

##### 4.3 Electrical Monitoring

One (1) electrical monitoring channel will be provided by the test lab. This channel will be used to continuously monitor the electrical discharge of the test cells during the seismic test. The discharge load will be approximately 2% of the three hour discharge rate of the largest cell being tested on the shock table at the time. Each 3 cell test group shall be connected in series using standard inter-cell connectors and terminal plates with terminal cables to simulate installed conditions. When more than one 3 cell group is tested on the shock table simultaneously, all 3 cell groups will be connected in series to provide electrical monitoring of all groups during the test.



## 5.0 POST-SEISMIC CAPACITY TEST

- 5.1 Following completion of the Seismic Test, each 3 cell group will be recharged to replace ampere hours withdrawn during the Seismic Test. Ampere hours charge will be equal to 110% of the ampere hours withdrawn.

Cells will then be given a Post Seismic Capacity Test at the published 3 hour rate to 1.75 V.P.C. average per IEEE 535-1979 Para. 8.2.2 (1).

## 6.0 INSPECTION

During and after seismic testing, the test specimens will be examined for abnormalities and the conditions recorded.

## 7.0 IN-PROCESS INSPECTION

The records of TRS plots will be compared with the RRS plots to assure compliance with requirements before proceeding with next test run. All vibration effects will be logged and analyzed.

- 7.1 The specimens will be examined for any damage following all tests. If necessary, a physical tightening of hardware will be performed after each seismic test event, with justification expressed in Section 7, "Test Series Summary".

- 7.2 During the seismic testing, the cells are connected in series and placed in a discharge mode. The series string voltage and current are monitored before, during and after seismic testing. Photographs will be taken of any noticeable physical damage that may occur as well as all specimen-mounted accelerometers.

## 8.0 FAILURE CRITERIA

A string of three cells must meet 80 percent of its 3-hour ampere hour capacity when discharged to an average final voltage of 1.75 V.P.C. If the initial Pre-Seismic capacity test indicates less than 80% of rated capacity, the cells may be recharged, returned to float at ambient temperature for a minimum period of 72 hours and the capacity test repeated. If the cells fail the second capacity test, the cells shall have failed.

- 8.1 During the seismic test, the cells shall be electrically monitored for current or voltage fluctuations which could indicate circuit interruptions.

If there is a failure, the failure shall be analyzed, the cause identified, and the designation of random or common mode justified.

- 8.2 If the failure is demonstrated to be random nature, the test will be repeated using a different group of naturally aged cells from the original or similar source. If the failure is of common mode origin, the equipment is not qualified and Gould has two choices. [See IEEE 535-1979 7.3 (1) and (2).]

8.2.1 The equipment can be redesigned.

8.2.2 A different group of naturally aged cells from the original or similar sources will be used for retesting. Retesting will use the same or lesser TRS. (Original TRS level was generically established to accommodate maximum anticipated future requirements.) The lower level TRS selected will satisfy the required RRS.

## 9.0 GENERIC & CUSTOMERS' TEST REPORTS

Customers' Test Reports (Environment Qualification Report, No. GB-3454) will be generated from the generic test program report to satisfy the requirements of individual contracts on a job-to-job basis. A comparison will be provided in the report between applicable RRS curves and the TRS at 2% and 5% damping. The report will comprise a summary, conclusions, and recommendations including a statement of qualified life.

Ten copies of a certification-type report will be issued subsequent to completion of generic testing. This report will be signed by a Registered Professional Engineer and will summarize the capacity test results, the response spectrum plots of the table motion, transmissibility plots of the resonant search, results and conclusions, details and recommendations concerning deficiencies and repairs, photographs of test setup, description and analysis of failures, etc. The report will also contain a list of test equipment used, functional capabilities, calibrations, calculations and instrumentation log sheets.

The Generic Qualification Test Report "Wyle Nuclear Environmental Qualification Program Report No. 44681-2", shall include the following:

1. The qualification information.
2. Identification of the specific feature(s) to be demonstrated by the test.
3. Test Procedure.
4. Report of test results. The report shall include:
  - A. Objective.
  - B. Equipment tested.
  - C. Description of test facility (test setup) and instrumentation used, including calibration records reference.
  - D. Test procedure, frequency of readings, and input data (for example, seismic acceleration and spectra).

9.0 GENERIC & CUSTOMERS' TEST REPORTS (Cont'd.)

- E. Test data and accuracy (results).
- F. Signature of test personnel and date.
- G. Supporting data.
- H. Approval signature and date.
- I. All malfunctions, whether or not they are detrimental.

Appendix I  
References

1. IEEE Std. 323-1974                      Qualifying Class 1E Equipment for  
Nuclear Power Generating Stations
2. IEEE Std. 344-1975                      Recommended Practices for Seismic  
Qualification of Class 1E Equipment  
for Nuclear Power Generating Stations
3. IEEE Std. 535-1979                      Qualifying Class 1E Lead Storage  
Batteries for Nuclear Power Generating  
Stations
4. IEEE Std. 450-1980                      Recommended Practice for Large Lead  
Storage Batteries for Generating  
Stations and Substations (ANSI 41.15)

## Appendix II

Gould Inc.  
Industrial Battery Division  
Langhorne, PA

### Generic Qualification Program Naturally Aged Cells Initial Program

A.	Test Group	Natural Age In Years
	Calcium & Plante	
	#1 MCX-510 (Calcium)	8 Years
	#2 NCX-1680 (Calcium)	10 Years
	#3 FPS-23 (Plante)	16 Years
	#4 FPS-23 (Plante)	10 Years

Note: Testing will be as outlined in Test Procedure.

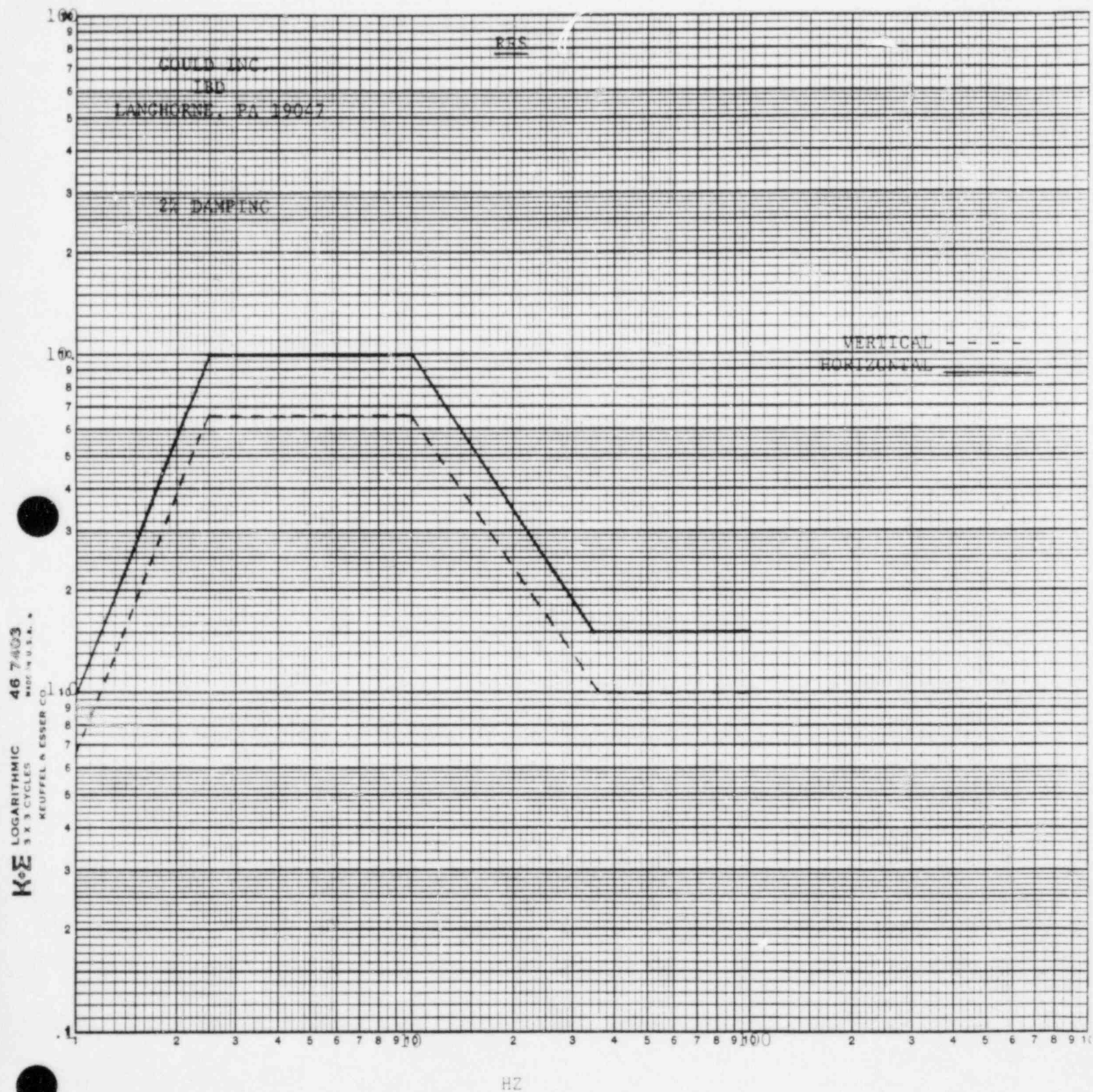
### Ongoing Qualification Program

B.	Test Group*	Natural Age In Years
	Calcium	
	#1 MCX-510 (Calcium)	15 Years
	#2 NCX-1680 (Calcium)	15 Years

Note: Test Group B will be composed of naturally aged cells which have attained the required age period in actual customer service. In the event additional test cells from the same source as in Test Group A are not available due to customer reluctance or abnormal degradation of original installation, alternate sources will be pursued. Attempt will be made to obtain test cells of the same size or as close to the same as possible.

Feasibility of ongoing qualification beyond 15 years will be determined following test results on the 15 year naturally aged groups.

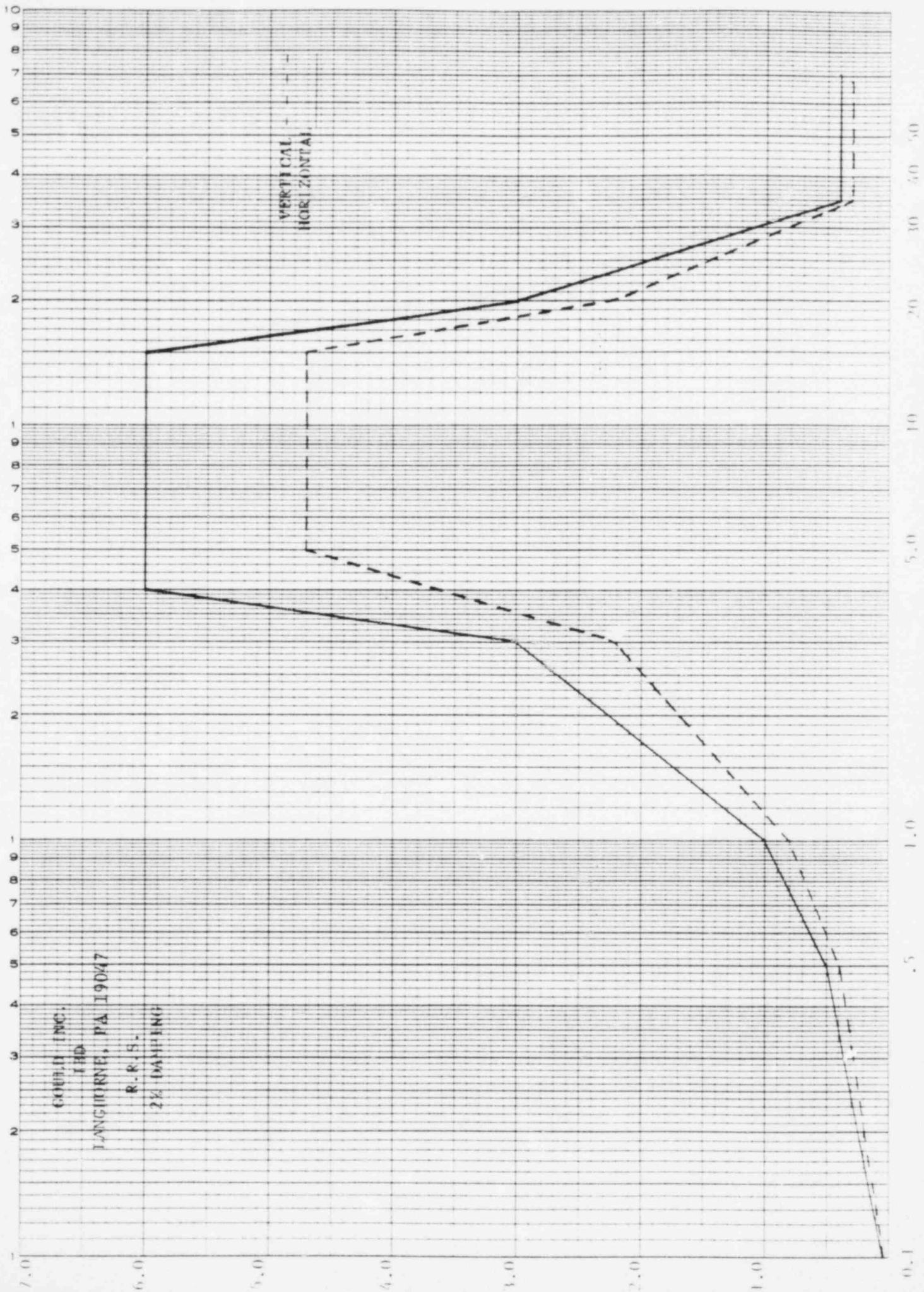
# APPENDIX III





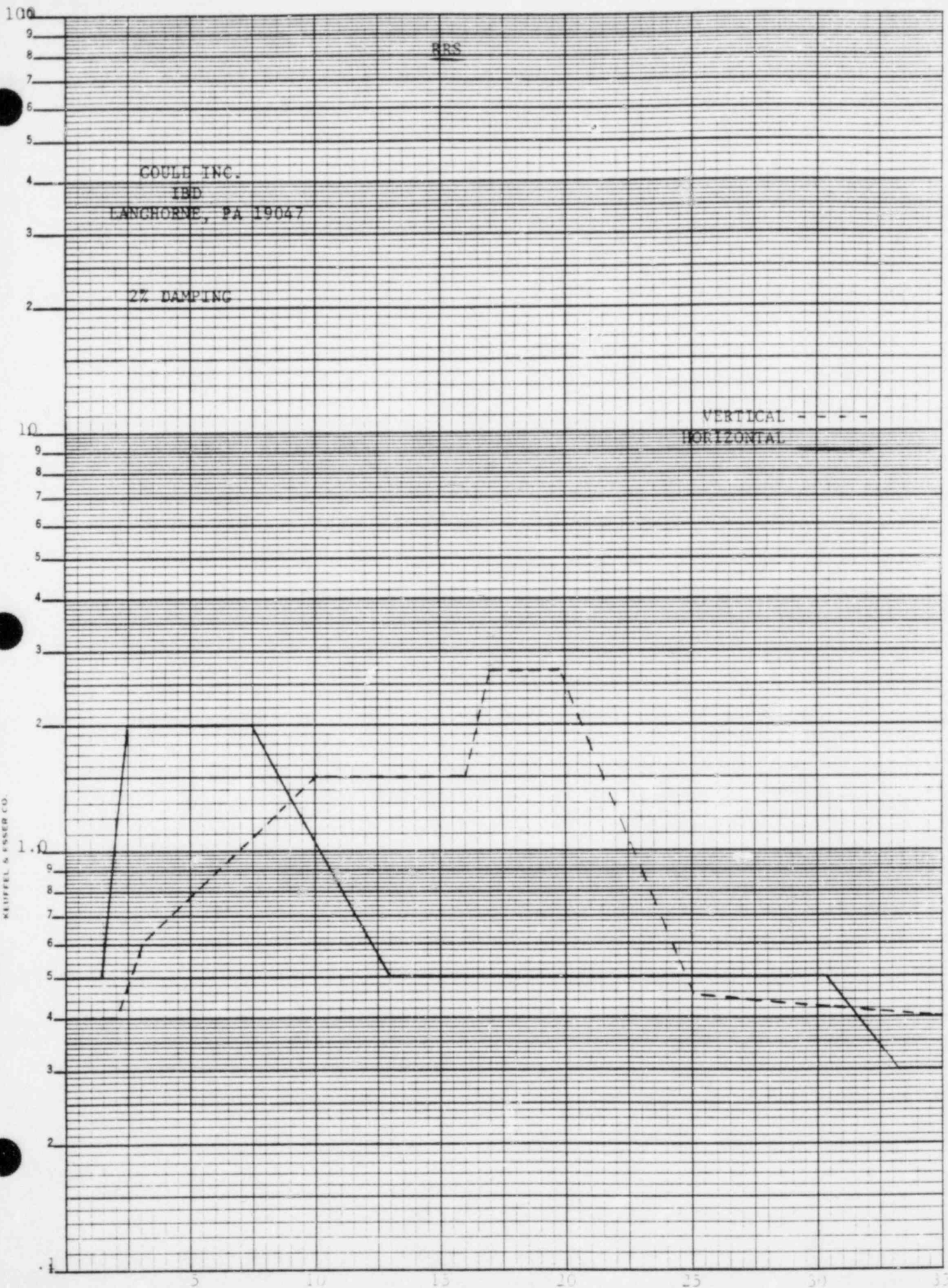
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MADE IN U.S.A.

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SEMI-LOGARITHMIC  
3 CYCLES X 10 DIVISIONS PER INCH





# APPENDIX V



SARGENT & LUNDY  
ENGINEERS  
CHICAGO

ATTACHMENT A  
(FORM 350)

RESPONSE SPECTRA  
• FOR  
STORAGE BATTERIES

SYRON/BRAIDWOOD STATION - UNITS 1 & 2  
SPECIFICATION F/L-2819

ATTACHMENT A

CONTENTS

- |  |                                  |
|--|----------------------------------|
| 1. Vertical Response Spectra<br>O.B.E.<br>El. 426'-0", 439'-0" & 451'-0"<br>Auxiliary Building Slab                                  | Dated 3-22-73<br>Sheet 28 of 150 |
| 2. Vertical Response Spectra<br>S.S.E./Blast<br>El. 426'-0", 439'-0" & 451'-0"<br>Auxiliary Building Slab                            | Dated 3-22-73<br>Sheet 32 of 150 |
| 3. Horizontal Floor Response Spectra<br>North-South Component (O.B.E.)<br>El. 451'-0"<br>Auxiliary-Turbine-Heater Bay Building       | Dated 3-22-73<br>Sheet 37 of 150 |
| 4. Horizontal Floor Response Spectra<br>East-West Component (O.B.E.)<br>El. 451'-0"<br>Auxiliary-Turbine-Heater Bay Building         | Dated 3-22-73<br>Sheet 38 of 150 |
| 5. Horizontal Floor Response Spectra<br>North-South Component (S.S.E./Blast)<br>El. 451'-0"<br>Auxiliary-Turbine-Heater Bay Building | Dated 3-22-73<br>Sheet 39 of 150 |
| 6. Horizontal Floor Response Spectra<br>East-West Component (S.S.E./Blast)<br>El. 451'-0"<br>Auxiliary-Turbine-Heater Bay Building   | Dated 3-22-73<br>Sheet 40 of 150 |

# SARGENT & LUNDY

ENGINEERS

CLIENT COMMONWEALTH EDISON CO.

PROJECT BYRON/BRAIDWOOD

JOB NO. 4391/46P2

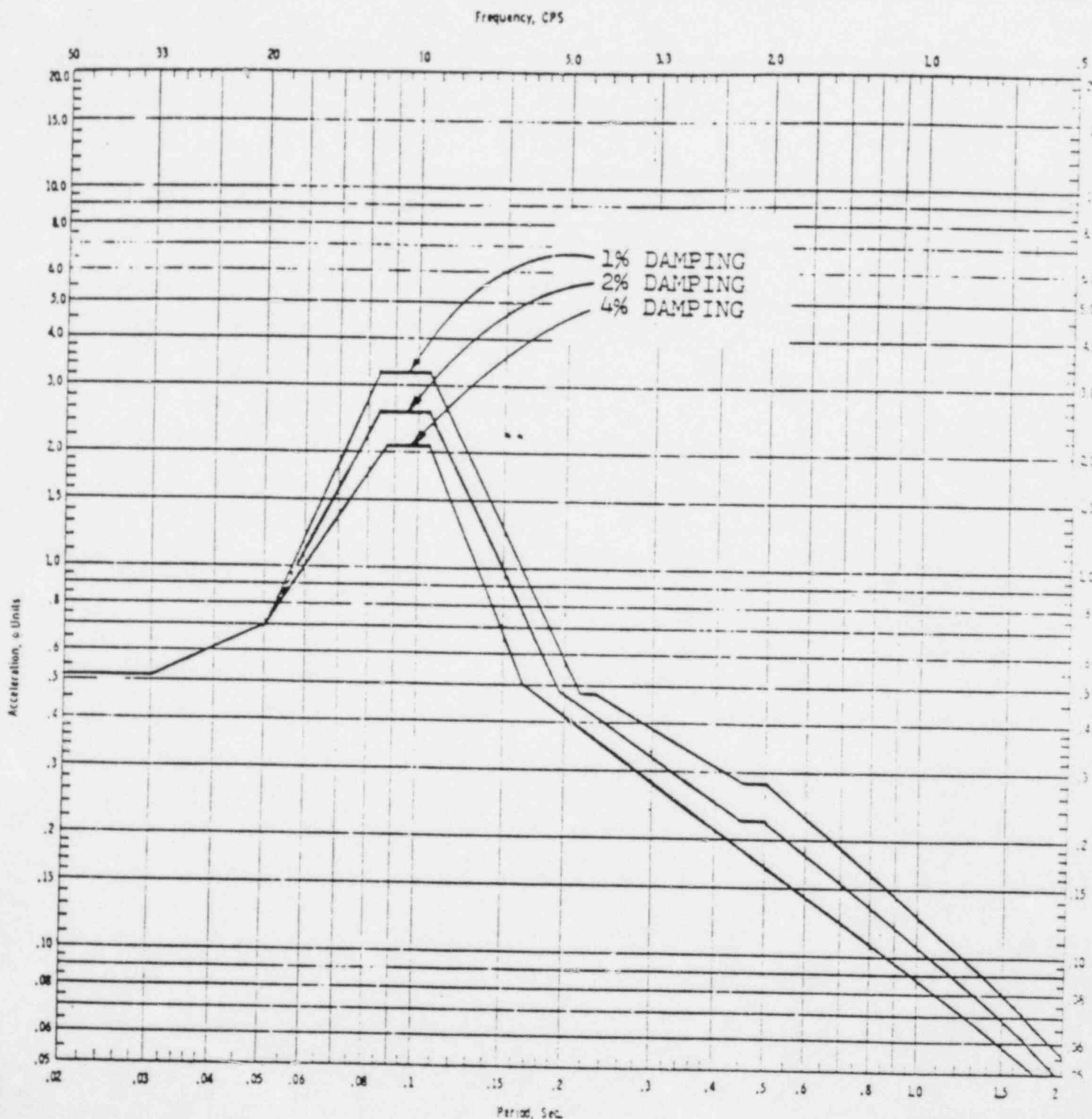
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DATE 3-22-73

CHECKED BY GSB

DATE 3-22-73 SHEET 28 OF 151

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DATE	2-7-74								
INITIALS	HS								

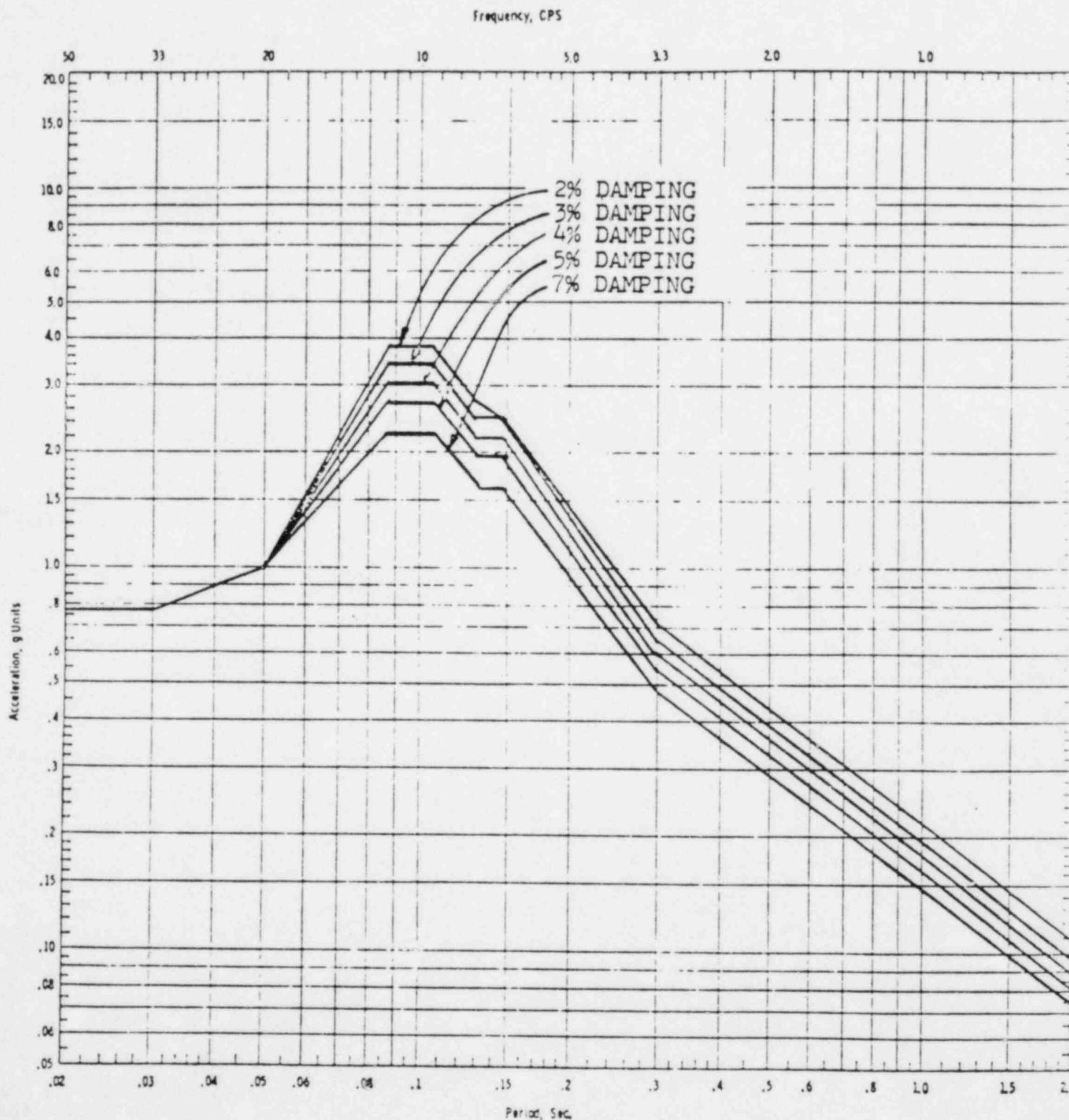


Vertical Response Spectra  
O.B.E.  
El. 426'-0", 439'-0" & 451'-0"  
Auxiliary Building Slab

Spectra No.  
108-CB-VS  
109-CB-VS  
110-CB-VS

**SARGENT & LUNDY****ENGINEERS**CLIENT COMMONWEALTH EDISON CO.PROJECT BYRON/BRAIDWOODJOB NO. 4391/460DESIGN BY HSDATE 3-22-73CHECKED BY GSBDATE 3-22-73 SHEET 32 OF 150

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DATE	2-7-74				
INITIALS	HS				



Vertical Response Spectra  
S.S.E./Blast  
El. 426'-0", 439'-0" & 451'-0"  
Auxiliary Building Slab

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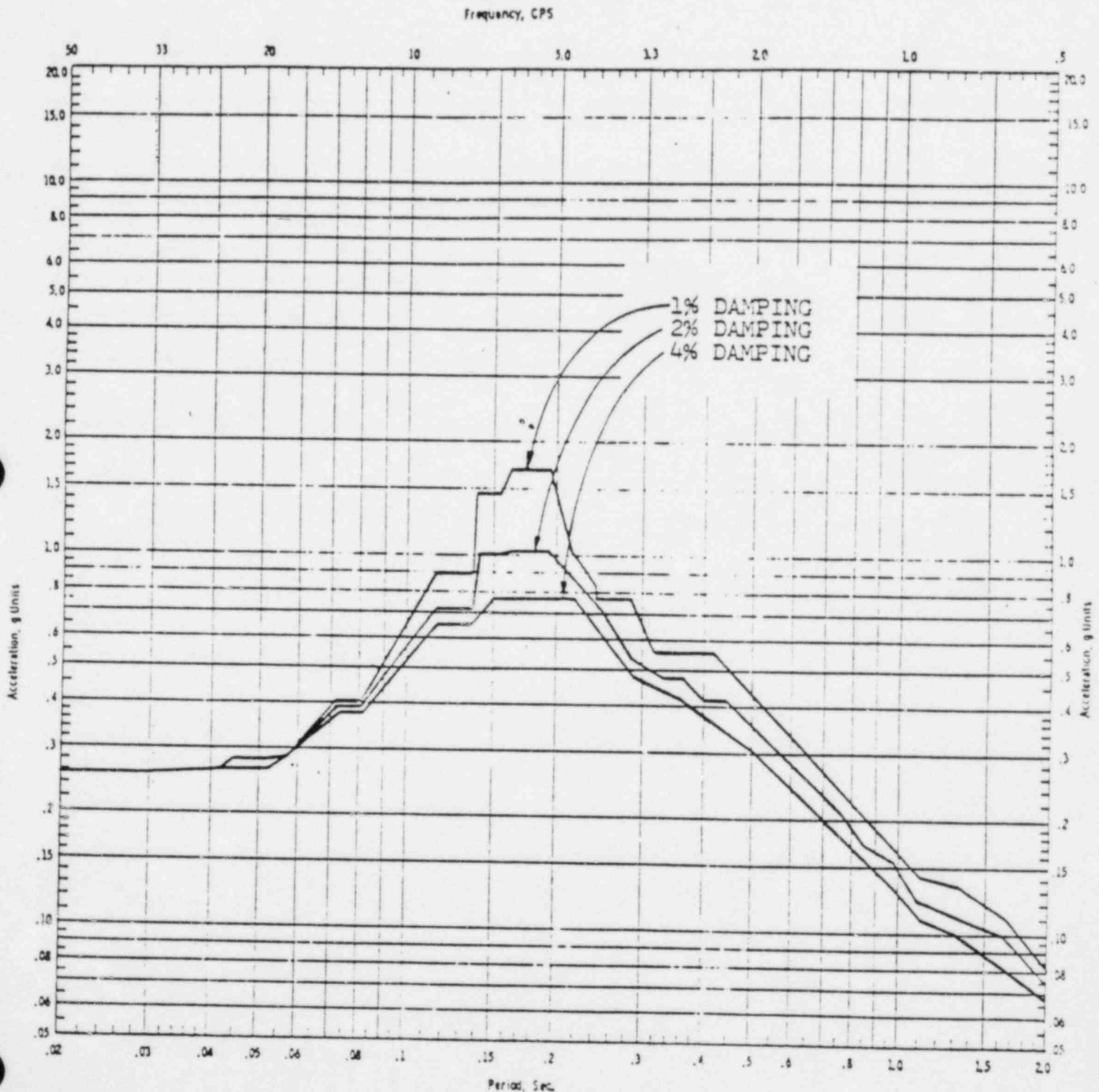


# SARGENT & LUNDY

ENGINEERS

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 CHECKED BY GSB DATE 3-22-73 SHEET 37 OF 150

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INITIALS	HS								



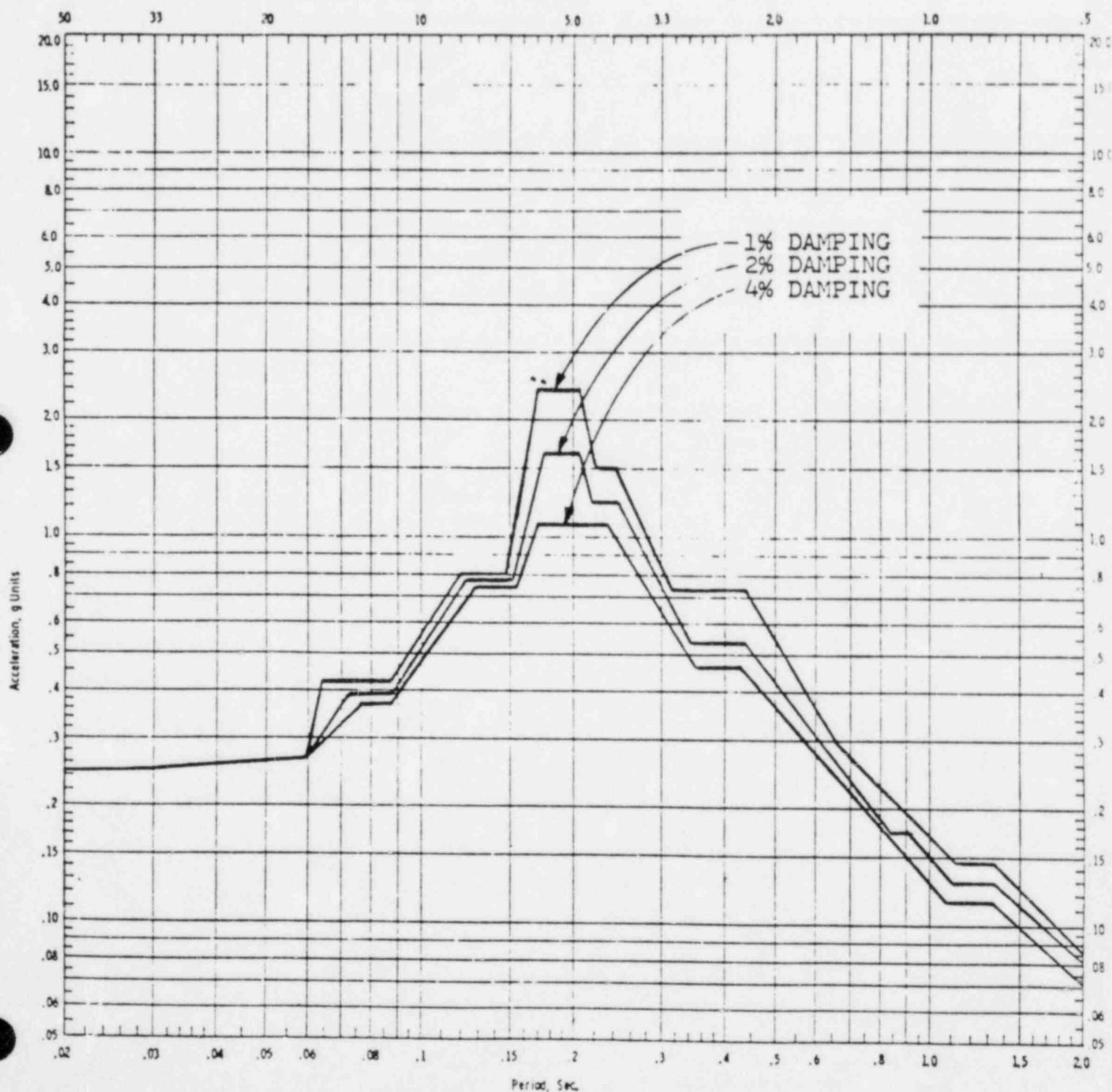
Horizontal Floor Response Spectra  
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 Auxiliary-Turbine-Heater Bay Building

Spectra No.  
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INITIALS	HS								

Frequency, CPS



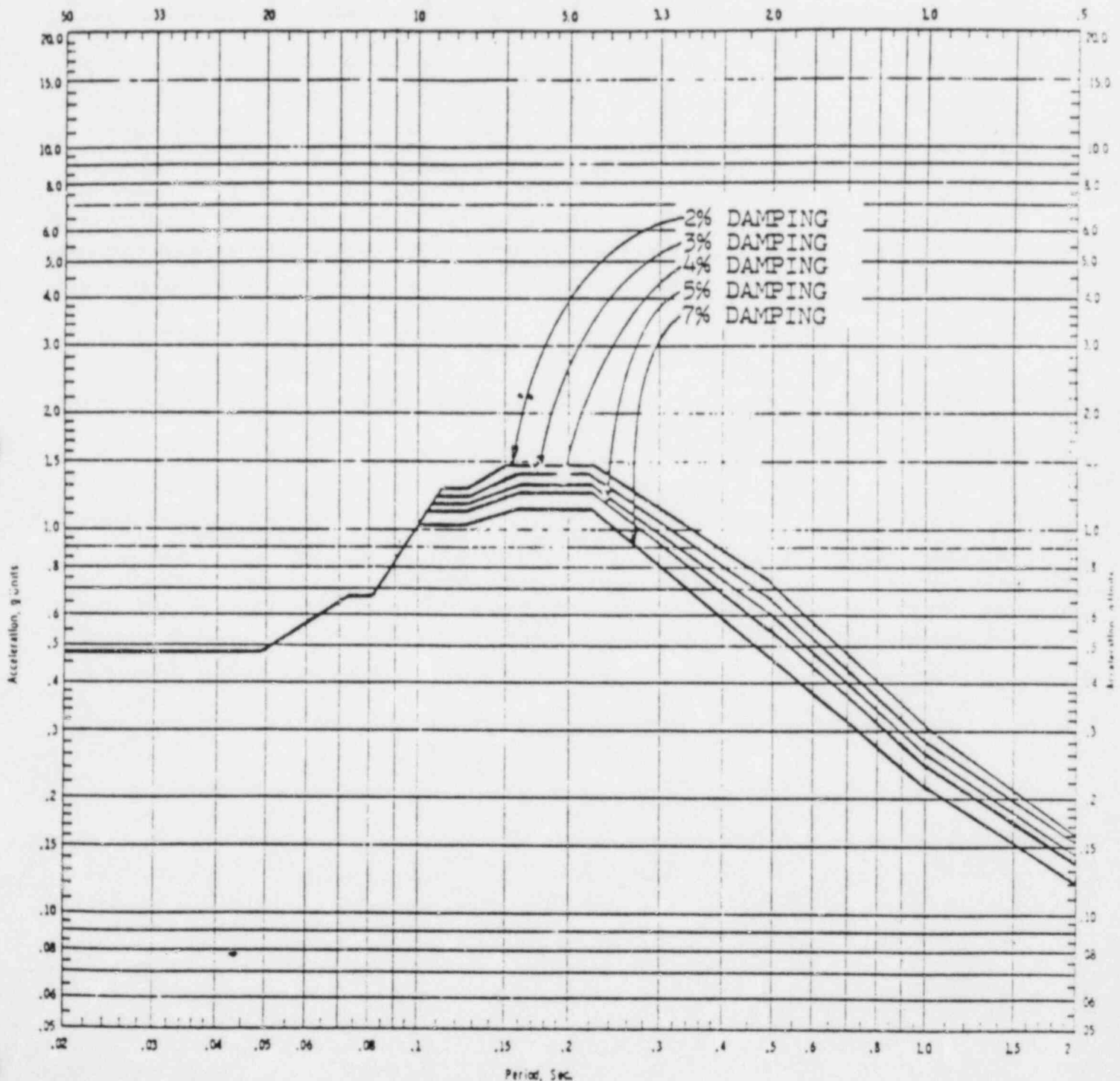
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Spectra No.  
110-OB-EW

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DATE	2-7-74								
INITIALS	HS								

Frequency, CPS



Horizontal Floor Response Spectra  
North-South Component (S.S.E./Blast)  
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Auxiliary-Turbine-Heater Bay Building

Spectra No.  
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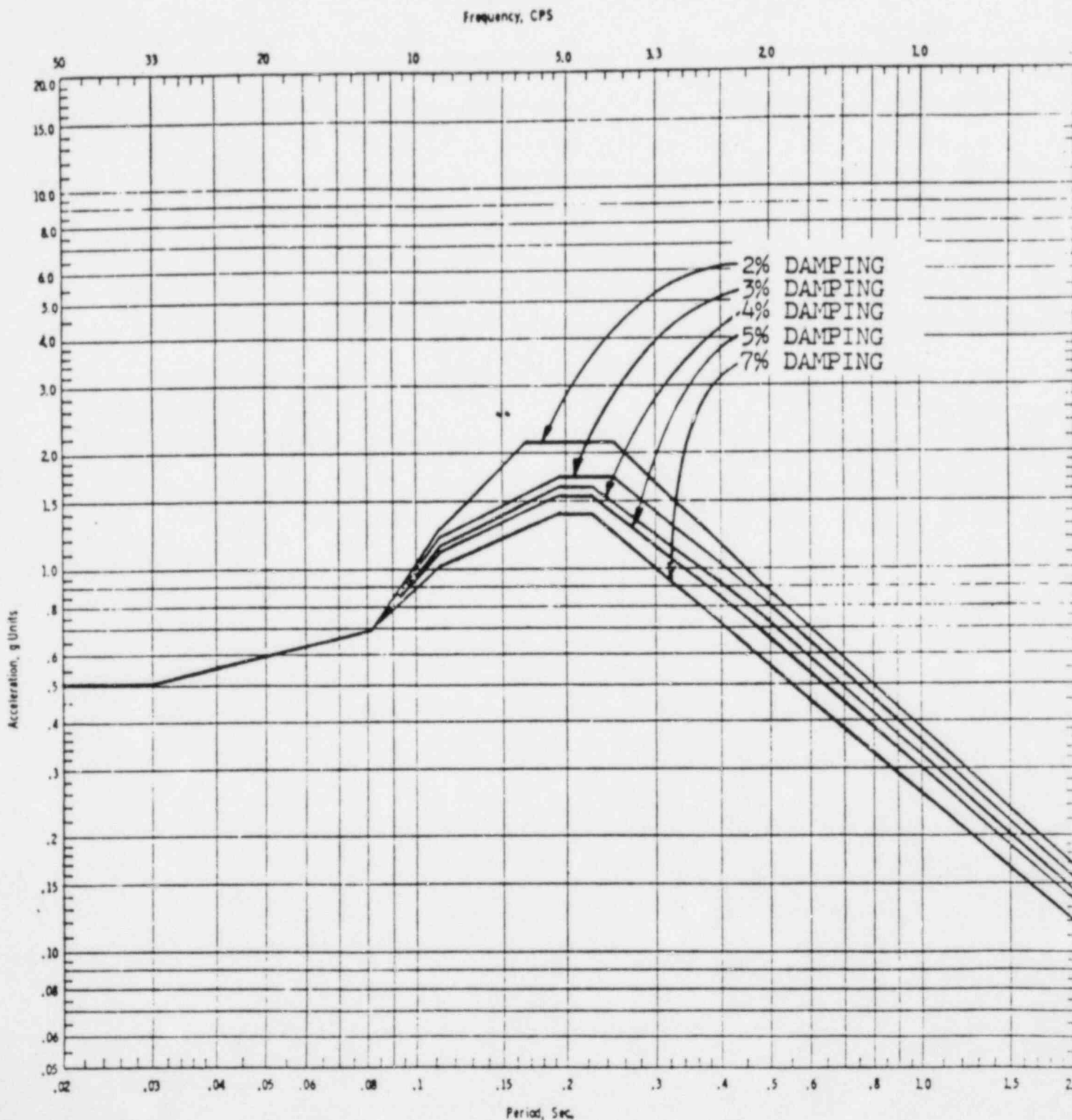


# SARGENT & UNDY

ENGINEERS

CLIENT COMMONWEALTH EDISON CO.  
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 CHECKED BY GSR DATE 3-22-73 SHEET 40 OF 150

REV. NO.	1								
DATE	2-7-74								
INITIALS	HS								



Horizontal Floor Response Spectra  
 East-West Component (S.S.E./Blast)  
 El. 451'-0"  
 Auxiliary-Turbine-Heater Bay Building

Spectra No.  
 110-SS-EW

## SECTION #2

### TEST SPECIMEN HISTORY

#### 1.0 SOURCE

The test specimens submitted to Wyle Laboratories for a seismic simulation test program were naturally aged cells obtained from New Jersey Bell Telephone Bordentown Central Office, Bordentown, New Jersey 08505.

1.1 Cells were manufactured in Gould manufacturing facilities located at 467 Calhoun Street, Trenton, New Jersey. Manufacturing date was 6/71 on Gould Order #T-62232. Shipment was made to customer the same month. Complete battery shipped consisted of 27 cells of NCX-1680 type. The NCX-1680 designation is the Gould commercial identification. This type cell when sold to Western Electric for resale to the Associated Bell Companies, is identified as a 27 KS-15544 List 508 battery. This is the Western Electric method of identifying equipment classification and specific size. There are no design differences between the commercial and Western Electric type designations.

Final negotiations for acquisition of the test cells was with George F. Hughes, Network Design, New Jersey Bell Telephone, Camden, New Jersey.

#### 2.0 LOCATION OF BATTERY

Battery was in service at the New Jersey Bell Telephone Central Office located at 195 Crosswicks Road, Bordentown, New Jersey 08505.

### 3.0 APPLICATION

Battery was used for telephone standby service in the event of commercial A.C. power failure. The battery would, therefore, assume the function of sustaining telephone equipment operation in such emergencies.

Battery was installed on a two-tier steel rack with cells #1-2 and #17-23 on the upper tier and cells #3 through 16 on the lower tier. Emergency cells were #1-4.

Twenty-three of the twenty-seven cells are used in the 48 volt main string. The remaining four cells are used as "End Cells" which are used during emergency discharges and are switched into use to sustain total system voltage requirements as the main string voltage declines during discharge.

### 4.0 CHARGING METHOD

The battery was maintained on a float method of charge at an average of 2.17 V.P.C. a Western Electric Model 302A voltage regulated power supply.

### 5.0 BATTERY RECORDS

Customer battery records are available for the first quarter of 1972 through the fourth quarter of 1980. See Battery Records Section #3. Battery is identified as 48 volt - String "B".

These records reflect quarterly individual cell float voltage readings on the 23 cell main string as well as cell voltage readings on the 4 cells in the emergency group #1.

#### 5.0 BATTERY RECORDS (Cont'd.)

In addition, the records include semi-annual specific gravity readings of cell in the main string as well as those in the emergency group #1.

Temperature readings of pilot cell electrolyte is also recorded.

#### 6.0 TEMPERATURE RANGE

The battery area temperature was controlled by using air conditioning. As reflected by the records, cell electrolyte temperature ranged from a low of 70°F to a high of 81°F. Based on temperature data available, the average yearly electrolyte temperature was 78°F. Relative humidity while not recorded was approximately 30 to 60%.

Recorded temperature values reflect no greater than a 2°F spread in battery temperature at any given time.

#### 7.0 OPERATING-MAINTENANCE PROCEDURES

Battery has been maintained in accordance with Bell System Practices which are comparable with Gould Installation and Operating Instructions GB-3384-B.

#### 8.0 RADIATION EXPOSURE

The test specimens were exposed to normal background radiation for their operating life span of 10 years. There are no environmental specification requirements for other than low level background radiation exposure, therefore, test specimens did not receive augmented radiation exposure prior to the seismic simulation test program.

9.0 BATTERY USAGE (EMERGENCY PERIODS)

Batteries of this type are normally sized to provide three to five hours of emergency power. Actual ampere load is a variable which is determined by total telephone subscribers handled as well as subscriber traffic which varies during a 24 hour period as well as from day to day.

### SECTION #3

#### CUSTOMER RECORDS

The customer records which are part of this section are actual record copies obtained from New Jersey Bell Telephone Company, for NCX-1680 type cells installed at their Bordentown Central Office located at 195 Crosswicks Road, Bordentown, New Jersey 08505.

The records represent quarterly individual cell voltages on the main string as well as the emergency group #1 cells. (Figs. # 3-1, 3-2, 3-3)

In addition, semi-annual specific gravity readings of cells referenced above are also included together with representative electrolyte temperatures on cells from the upper and lower tiers to reflect min.-max. as well as average battery temperatures prevailing.

As outlined in Section #2 Test History Item 1.1, the customer identification of the NCX-1680 is KS-15544 List 508 which is shown on the records. The design is exactly the same.

QUARTERLY INDIVIDUAL CELL VOLTAGE  
SEMI-ANNUAL CORRECTED SPECIFIC GRAVITY  
OFFICE

—486—

BATTERY NO.

[illegible]

Fig. 3-1



BATTERY NO.

QUARTERLY INDIVIDUAL CELL VOLTAGE  
SEMI-ANNUAL CORRECTED SPECIFIC GRAVITY

3 213 40

BATTERY NO.

PAGE 1 OF 3

CELL NO.	QUARTERLY INDIVIDUAL CELL VOLTAGE																SEMI ANNUAL CORRECTED SPECIFIC GRAVITY								BATTERY INFORMATION						
	YEAR 1975				YEAR 1976				YEAR 1977				YEAR 1978				YEAR 75		YEAR 76		YEAR 77		YEAR 78		TYPE & SIZE	SPECIFIC GRAVITY RANGE	NUMBER OF CELLS	PILOT CELL NO.	DATE INSTALLED	INITIAL CHARGE PILOT CELL VOLTAGE	INITIAL CHARGE PILOT CELL SPEC. GRAV.
	1 QTR	2 QTR	3 QTR	4 QTR	1 QTR	2 QTR	3 QTR	4 QTR	1 QTR	2 QTR	3 QTR	4 QTR	1 QTR	2 QTR	3 QTR	4 QTR	1 HLF	2 HLF	1 HLF	2 HLF	1 HLF	2 HLF									
1	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
2	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
3	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
4	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
5	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
6	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
7	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
8	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
9	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
10	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
11	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
12	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
13	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
14	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
15	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
16	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
17	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
18	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
19	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
20	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
21	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
22	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
23	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
24	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
25	2.17	2.18	2.15	2.17	2.14	2.16	2.15	2.16	2.15	2.15	2.16	2.15	2.16	2.15	2.16	2.15	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	12.05	
BATTERY RECORDS																															
NEW JERSEY REL. TELEPHONE																															
BORJENTOWN C. O.																															
BORJENTOWN, NEW JERSEY 08505																															
DATE BY REMARKS																															
WATER ADDED: DATE QUANTITY																															

Fig. 3-2



QUARTERLY INDIVIDUAL CELL VOLTAGE  
SEMI-ANNUAL CORRECTED SPECIFIC GRAVITY  
OFFICE

BSP 157-601-301

BATTERY NO. -48 B

MONTH		QUARTERLY INDIVIDUAL CELL VOLTAGE												SEMI-ANNUAL CORRECTED SPECIFIC GRAVITY				BATTERY INFORMATION															
		YEAR 1979				YEAR 1980				YEAR 1980				YEAR 1980				YEAR 1980															
		1 QTR	2 QTR	3 QTR	4 QTR	1 QTR	2 QTR	3 QTR	4 QTR	1 QTR	2 QTR	3 QTR	4 QTR	1 HRP	2 HRP	3 HRP	4 HRP	TYPE	SPECIFIC GRAVITY RANGE	NUMBER OF CELLS	PILOT CELL NO.	DATE INSTALLED	INITIAL CHARGE	PILOT CELL VOLTAGE	INITIAL CHARGE	PILOT CELL VOLTAGE	FROM RE. CO. RECORDS	BATTERY RECORDS	NEW JERSEY BILL TELEPHONE	BORDENTOWN C. O.	BORDENTOWN, NEW JERSEY 08805		
Pilot	CELL NO.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25							
	VOLTAGE	2.16	2.18	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	2.17	
	FM. CELL GRP1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25							
	FM. CELL GRP2	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25							
	DATE BY REMARKS																																
	WATER ADDED:																																
	DATE QUANTITY																																

#### SECTION #4

##### DESIGN CONTINUATION DOCUMENTATION

The cell design for batteries supplied for Byron/Braidwood as well as test cells submitted to Wyle Laboratories for seismic simulation testing are of the same design.

Included in this section are cell section drawings covering all cell sizes in the NCX line of cells.

<u>Types</u>	<u>Drawing #</u>	<u>Fig. #</u>
NCX-600 through 1200	107006D Rev. F	#4-1
NCX-1344 through 1500	107012D Rev. F	#4-2
NCX-1650 through 1950	106288D Rev. K	#4-3
NCX-2016 through 2550	106302D Rev. E	#4-4

These drawings demonstrate that cell type NCX-1200 & 1500 being supplied for Class 1E use is the same design as the NCX-1680 naturally aged test cells submitted to Wyle Laboratories for seismic qualification.

Additional documentation of design continuation is afforded by Gould Specification Sheets GB-3325 dated 6/71 (Fig. #4-5) current at the time the test cells were manufactured.

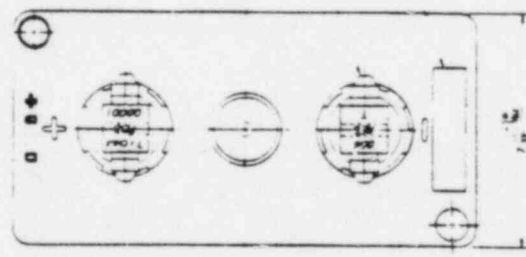
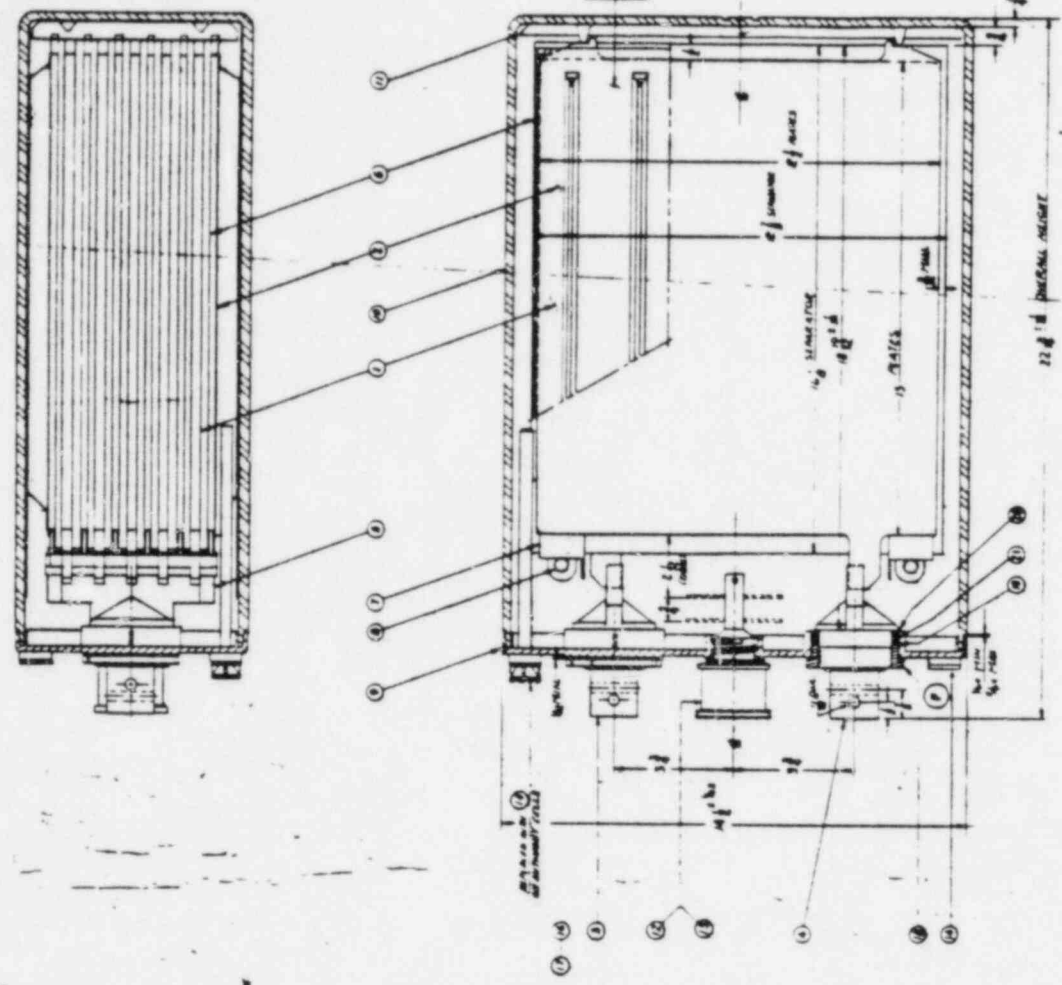
In addition, Gould Specification Sheet GB-3325 dated 2/80 (Fig. #4-6) is the latest issue and demonstrates design continuance for currently produced cells.

The revisions noted on the cell section drawings were minor dimensional changes, change of accessory options or redrawing of original drawing with no changes.

The materials used in cell construction have remained unchanged and traceability can be provided.

Part Name		Part Number	
1	WATER PUMP	1	WATER PUMP
2	WATER PUMP MOTOR	2	WATER PUMP MOTOR
3	WATER PUMP MOTOR	3	WATER PUMP MOTOR
4	WATER PUMP MOTOR	4	WATER PUMP MOTOR
5	WATER PUMP MOTOR	5	WATER PUMP MOTOR
6	WATER PUMP MOTOR	6	WATER PUMP MOTOR
7	WATER PUMP MOTOR	7	WATER PUMP MOTOR
8	WATER PUMP MOTOR	8	WATER PUMP MOTOR
9	WATER PUMP MOTOR	9	WATER PUMP MOTOR
10	WATER PUMP MOTOR	10	WATER PUMP MOTOR
11	WATER PUMP MOTOR	11	WATER PUMP MOTOR
12	WATER PUMP MOTOR	12	WATER PUMP MOTOR
13	WATER PUMP MOTOR	13	WATER PUMP MOTOR
14	WATER PUMP MOTOR	14	WATER PUMP MOTOR
15	WATER PUMP MOTOR	15	WATER PUMP MOTOR
16	WATER PUMP MOTOR	16	WATER PUMP MOTOR
17	WATER PUMP MOTOR	17	WATER PUMP MOTOR
18	WATER PUMP MOTOR	18	WATER PUMP MOTOR
19	WATER PUMP MOTOR	19	WATER PUMP MOTOR
20	WATER PUMP MOTOR	20	WATER PUMP MOTOR
21	WATER PUMP MOTOR	21	WATER PUMP MOTOR
22	WATER PUMP MOTOR	22	WATER PUMP MOTOR
23	WATER PUMP MOTOR	23	WATER PUMP MOTOR
24	WATER PUMP MOTOR	24	WATER PUMP MOTOR
25	WATER PUMP MOTOR	25	WATER PUMP MOTOR
26	WATER PUMP MOTOR	26	WATER PUMP MOTOR

See also drawing 100-117 and 100-118



Part Name	Part Number
WATER PUMP MOTOR	100-117
WATER PUMP MOTOR	100-118
WATER PUMP MOTOR	100-119
WATER PUMP MOTOR	100-120
WATER PUMP MOTOR	100-121
WATER PUMP MOTOR	100-122
WATER PUMP MOTOR	100-123
WATER PUMP MOTOR	100-124
WATER PUMP MOTOR	100-125
WATER PUMP MOTOR	100-126
WATER PUMP MOTOR	100-127
WATER PUMP MOTOR	100-128
WATER PUMP MOTOR	100-129
WATER PUMP MOTOR	100-130
WATER PUMP MOTOR	100-131
WATER PUMP MOTOR	100-132
WATER PUMP MOTOR	100-133
WATER PUMP MOTOR	100-134
WATER PUMP MOTOR	100-135
WATER PUMP MOTOR	100-136
WATER PUMP MOTOR	100-137
WATER PUMP MOTOR	100-138
WATER PUMP MOTOR	100-139
WATER PUMP MOTOR	100-140
WATER PUMP MOTOR	100-141
WATER PUMP MOTOR	100-142
WATER PUMP MOTOR	100-143
WATER PUMP MOTOR	100-144
WATER PUMP MOTOR	100-145
WATER PUMP MOTOR	100-146
WATER PUMP MOTOR	100-147
WATER PUMP MOTOR	100-148
WATER PUMP MOTOR	100-149
WATER PUMP MOTOR	100-150



DRAWING #107006-D REVISION F

REV. A            6-10-71   Added type and specification tabulation.   Revised  
Revised title to include commercial types.

REV. B            11-24-71   Revised mounting height dimension from 18-17/32"  
± 1/32" to 18-19/32" ± 1/32".

REV. C            1-24-72   Deleted reference to plastic plug used in withdrawal  
tube opening in cell cover.

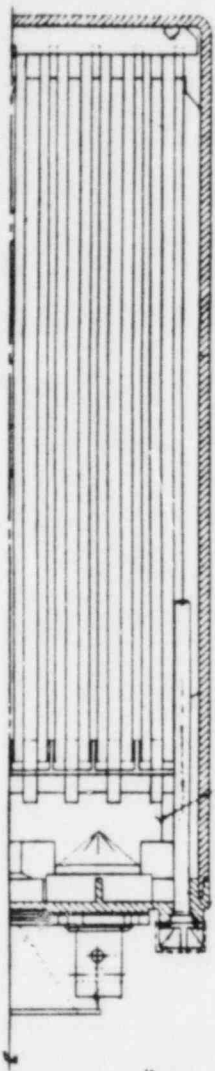
REV. D            2-2-72   Added outside negative plate shield.

REV. E            1-15-73   Overall height dimension changed from 22-1/8" ± 1/8"  
to 22-3/16" ± 1/16".   All general notes replaced with reference  
to Gould Engineering Specifications #D-217 and #D-229.

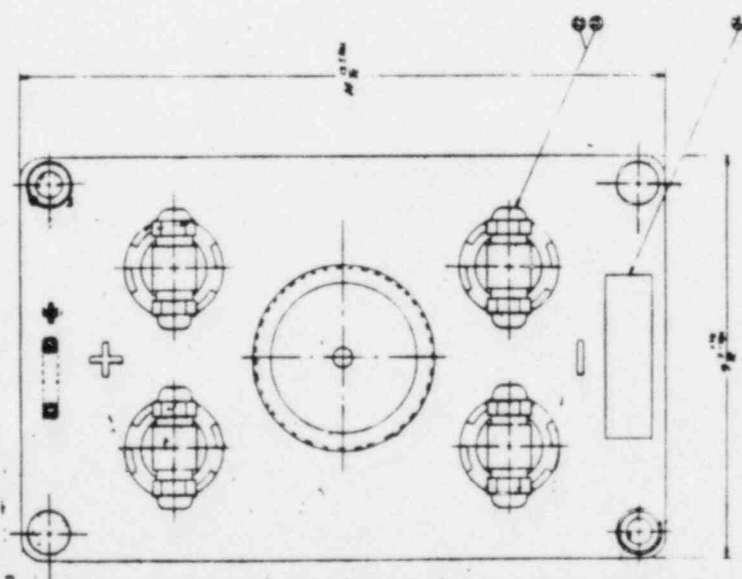
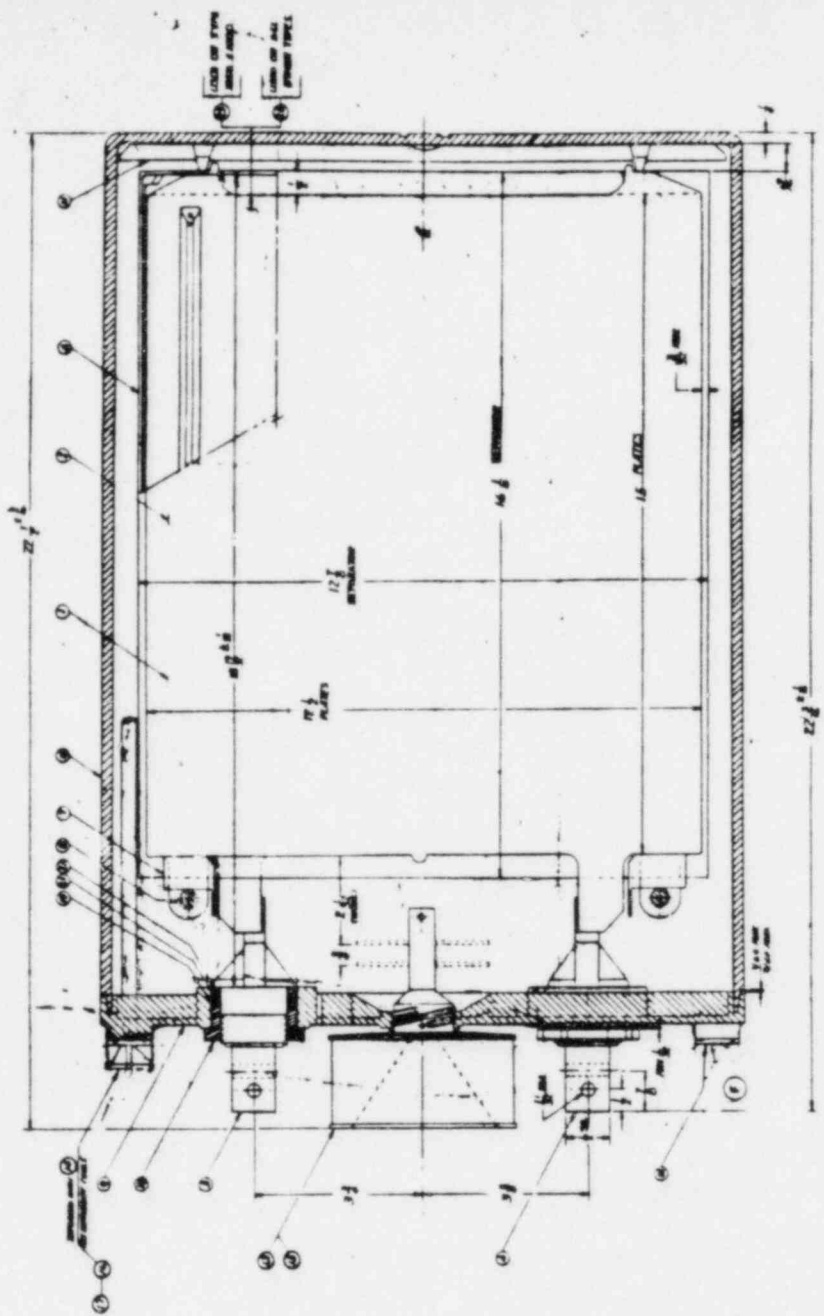
REV. F            1-5-77   Deleted Item #16 Parts Description with Item #14  
(Accessory Item).

DRAWING #107006-D REVISION F

<u>ITEM NO.</u>	<u>PART DESCRIPTION</u>	<u>REMARKS</u>
1	POSITIVE PLATE	-----STANDARD EQUIPMENT-----
2	NEGATIVE PLATE	
3	POS. TERMINAL BUSBAR	
4	NEG. TERMINAL BUSBAR	
5	BURNING LEAD	
6	SEPARATOR/MAT ASSY.	
7	POS. PLATE SUPPORT	
8	PLATE SUPPORT ROD	
9	COVER	
10	CONTAINER	
11	BRIDGE	
12	SCREW VENT	
13	FIRE ARRESTOR VENT ASSY. (SHOWN)	OPTIONAL REPLACEMENT FOR ITEM #12
14	DUMMY PLUG ASSY. (2 PER ANTIMONY, 1 PER CALCIUM)	-----STANDARD REQUIREMENT-----
15	SAMPLING TUBE ASSY. (SHOWN)	FURNISHED WITH CALCIUM CELLS ONLY (2 REQ'D)
16	(DELETED)	
17	DUST CAP (FOR SHIPPING PURPOSES ONLY)	USED ONLY WITH ITEM #15
18	SEAL NUT	-----STANDARD REQUIREMENT-----
19	WASHER	
20	SPACING RING	
21	"O" RING GASKET	
22	CONNECTOR STUD	
23	CONNECTOR NUT	
24	NAMEPLATE	
25	SHIM	USED WHERE INDICATED
26	OUTER PLATE SHIELD	USED WHERE INDICATED



GENERAL NOTE  
 10. Gould unit shown with Gould no. 1 and 2 components.  
 See also drawings 10-11 and 10-12.



GOULD	
SECTIONAL VIEW	10-11
SECTIONAL VIEW	10-12
SECTIONAL VIEW	10-13
SECTIONAL VIEW	10-14
SECTIONAL VIEW	10-15
SECTIONAL VIEW	10-16
SECTIONAL VIEW	10-17
SECTIONAL VIEW	10-18
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SECTIONAL VIEW	10-96
SECTIONAL VIEW	10-97
SECTIONAL VIEW	10-98
SECTIONAL VIEW	10-99
SECTIONAL VIEW	10-100

FIG. 10-2



DRAWING #107012-D REVISION F

- REV. A            6-10-71   Revised mounting height dimension from 18-17/32"  
                      $\pm 1/32"$  to 18-19/32"  $\pm 1/32"$ .
- REV. B            12-3-71   Revised cover ribs and vent opening boss.
- REV. C            1-14-72   Deleted reference to plastic plug used in withdrawal  
                     tube opening in cell cover.
- REV. D            2-2-72   Added outside negative plate shield.
- REV. E            1-15-73   Overall height dimension changed from 22-1/8"  $\pm 1/8"$   
                     to 22-3/16"  $\pm 1/16"$ . All general notes replaced with reference  
                     to Gould Engineering Specifications #D-217 and #D-229.
- REV. F            1-5-77   Deleted Item #16 Parts Description with Item #14  
                     (Accessory Item).



DRAWING #107012-D REVISION F

<u>ITEM NO.</u>	<u>PART DESCRIPTION</u>	<u>REMARKS</u>
1	POSITIVE PLATE	-----STANDARD EQUIPMENT-----
2	NEGATIVE PLATE	
3	POS. TERMINAL BUSBAR	
4	NEG. TERMINAL BUSBAR	
5	BURNING LEAD	
6	SEPARATOR/MAT ASSY.	
7	POS. PLATE SUPPORT	
8	PLATE SUPPORT ROD	
9	COVER	
10	CONTAINER	
11	BRIDGE	
12	SCREW VENT	
13	FIRE ARRESTOR VENT ASSY. (SHOWN)	OPTIONAL REPLACEMENT FOR ITEM #1
14	DUMMY PLUG ASSY. (2 PER ANTIMONY, 1 PER CALCIUM)	-----STANDARD REQUIREMENT-----
15	SAMPLING TUBE ASSY. (SHOWN)	FURNISHED WITH CALCIUM CELLS ONL (2 REQ'D)
16	(DELETED)	
17	DUST CAP (FOR SHIPPING PURPOSES ONLY)	USED ONLY WITH ITEM #15
18	SEAL NUT	-----STANDARD REQUIREMENT-----
19	WASHER	
20	SPACING RING	
21	"O" RING GASKET	
22	CONNECTOR STUD	
23	CONNECTOR NUT	
24	NAMEPLATE	
25	SHIM	USED WHERE INDICATED
26	OUTER PLATE SHIELD	USED WHERE INDICATED

DRAWING #106288-D REVISION K

<u>REV. A</u>	<u>4-28-70</u> Added Note 6, revised for lead antimony.
<u>REV. B</u>	<u>5-19-70</u> Cell length, width and overall height dimensions revised. Notes 8 and 9 added.
<u>REV. C</u>	<u>7-13-70</u> Dimension from top of separator to low level line changed from 1-1/2" to 1-13/32".
<u>REV. D</u>	<u>4-16-71</u> Added letter "X" to top of negative post for identification.
<u>REV. E</u>	<u>5-3-71</u> Letter "X" changed from top of negative post to top of positive post.
<u>REV. F</u>	<u>11/24/71</u> Revised mounting height dimension from 18-17/32" to 18-19/32".
<u>REV. G</u>	<u>8-18-72</u> Redrawn - no changes.
<u>REV. H</u>	<u>1-10-73</u> Overall height dimension changed from 22-1/8" $\pm$ 1/8" to 22-3/16" $\pm$ 1/16". Tolerance added to 18-19/32" mounting height ( $\pm$ 1/32"). All notes replaced with reference to Engineering Specification #D-229.
<u>REV. J</u>	<u>2-26-73</u> Separator height dimension changed from 16-1/16" to 16-1/8". Height of separator above plates was changed from 9/16" to 5/8".
<u>REV. K</u>	<u>1-5-77</u> Deleted Item #16 Parts Description with Item #14 (Accessory Item).

DRAWING #106288-D REVISION K

<u>ITEM NO.</u>	<u>PART DESCRIPTION</u>	<u>REMARKS</u>
1	POSITIVE PLATE	----STANDARD REQUIREMENT----
2	NEGATIVE PLATE	
3	POS. TERMINAL BUSBAR	
4	NEG. TERMINAL BUSBAR	
5	BURNING LEAD	
6	SEPARATOR/MAT ASSY.	
7	POS. PLATE SUPPORT	
8	PLATE SUPPORT ROD	
9	COVER	
10	CONTAINER	
11	BRIDGE	
12	SCREW VENT	
13	FIRE ARRESTOR VENT ASSY. (SHOWN)	OPTIONAL REPLACEMENT FOR ITEM #1
14	DUMMY PLUG ASSY. (4 PER ANTIMONY, 2 PER CALCIUM)	----STANDARD REQUIREMENT----
15	SAMPLING TUBE ASSY. (SHOWN)	FURNISHED WITH CALCIUM CELLS ONL
16	(DELETED)	
17	DUST CAP (FOR SHIPPING PURPOSES ONLY)	USED ONLY WITH ITEM #15
18	SEAL NUT	----STANDARD REQUIREMENT----
19	WASHER	
20	SPACING RING	
21	"O" RING GASKET	
22	CONNECTOR STUD	
23	CONNECTOR NUT	
24	NAMEPLATE	





DRAWING #106302-D REVISION E

- REV. A            11-29-71   Revised to include cell types using shim.   Revised mounting height from 18-17/32"  $\pm$  1/32" to 18-19/32"  $\pm$  1/32".
- REV. B            1-14-72   Deleted reference to plastic plug used in withdrawal tube opening in cell cover.
- REV. C            2-2-72   Added outside negative plate shield.
- REV. D            1-15-73   Overall cell height to post changed from 22-5/32"  $\pm$  1/8" to 22-7/32"  $\pm$  1/16".   All general notes replaced with reference to Gould Engineering Specification #D-229.
- REV. E            1-5-77   Deleted Item #16 Parts Description with Item #14 (Accessory Item).

DRAWING #106302-D REVISION E

<u>ITEM NO.</u>	<u>PART DESCRIPTION</u>	<u>REMARKS</u>
1	POSITIVE PLATE	----STANDARD REQUIREMENT----
2	NEGATIVE PLATE	
3	POS. TERMINAL BUSBAR	
4	NEG. TERMINAL BUSBAR	
5	BURNING LEAD	
6	SEPARATOR/MAT ASSY.	
7	POS. PLATE SUPPORT	
8	POS. PLATE SUPPORT ROD	
9	COVER	
10	CONTAINER	
11	BRIDGE	
12	SCREW VENT	
13	FIRE ARRESTOR VENT ASSY. (SHOWN)	OPTIONAL REPLACEMENT FOR ITEM #12
14	DUMMY PLUG ASSY. (4 PER ANTIMONY, 2 PER CALCIUM)	----STANDARD REQUIREMENT----
15	SAMPLING TUBE ASSY. (SHOWN)	FURNISHED WITH CALCIUM CELLS ONLY
16	(DELETED)	
17	DUST CAP (FOR SHIPPING PURPOSES ONLY)	USED ONLY WITH ITEM #15
18	SEAL NUT	----STANDARD REQUIREMENT----
19	WASHER	
20	SPACING RING	
21	"O" RING GASKET	
22	CONNECTOR STUD	
23	CONNECTOR NUT	
24	NAMEPLATE	
25	SHIM	USED WHERE INDICATED
26	OUTER PLATE SHIELD	USED WHERE INDICATED



# GOULD

## Stationary Power Cells —Calcium

### TYPE-NCX

**CAPACITIES—1650 A.H. TO 1950 A.H.  
@ 8 HOUR RATE TO 1.75 V.P.C. AVERAGE.**

#### SPECIFICATIONS

**Container**—Styrene-Acrylonitrile Plastic.<sup>1</sup>  
**Cover**—Acryl.-Buta.-Styr. Terpolym. Plastic.<sup>1</sup>  
**Separators**—Duropor-Microporous Material.  
**Retainers**—Fiberglass Mats.  
**Posts**—Four 1" Square.  
**Post Seals**—Floating O-Ring—Seal Nut.  
**Vents**—Screw Type—Spray Proof.<sup>2</sup>  
**Level Lines**—High and Low—All Jar Faces.  
**Electrolyte**—Height Above Plates—2-3/4".  
**Electrolyte Withdrawal Tube**—Each Cell.  
**Sediment Space**—1-1/16".  
**Specific Gravity**—1.215 @ 77°F. (25°C.).  
**Inter-Cell Connectors**—Lead Plated Copper.

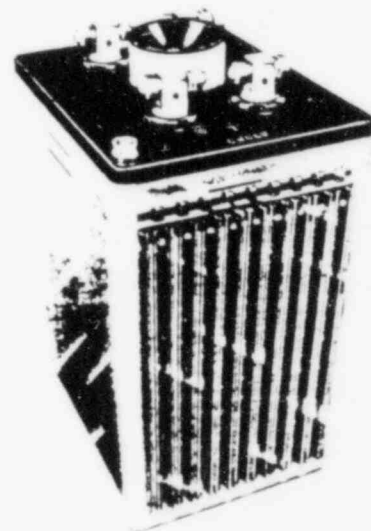


Plate Dimensions	Height	Width	Thick- ness
Positive Plate	15-1/2"	12"	320
Negative Plate	15-1/2"	12"	215

<sup>1</sup> Modified Polyvinyl Chloride which will not support combustion is available at additional cost.

<sup>2</sup> Combined Filling Funnel—Explosion resistant vent is available at additional cost. Specify Gould "Pre-Vent".\*\*

Type	Plates Per Cell	Ampere Hour Capacities to 1.75 V.P.C. Average*				1 Minute Rate in Amperes*		Overall Dimensions in inches			Approximate Wgt. in Lbs.		Elect. Gals. Per Cell
		8 Hr.	5 Hr.	3 Hr.	1 Hr.	To 1.75 V.P.C. Avg.	To 1.50 V.P.C. Avg.	L	W	H	Net Wgt.	Packed Wgt.	
NCX-1650	23	1650	1485	1287	825	1782	3390	11-3/8"	14-1/2"	22-1/2"	348	366	8.1
NCX-1680	21	1680	1470	1230	750	1530	2910	11-3/8"	14-1/2"	22-1/2"	332	350	8.3
NCX-1800	25	1800	1620	1404	900	1932	3675	11-3/8"	14-1/2"	22-1/2"	364	382	7.8
NCX-1950	27	1950	1755	1521	975	2080	3955	11-3/8"	14-1/2"	22-1/2"	380	398	7.6

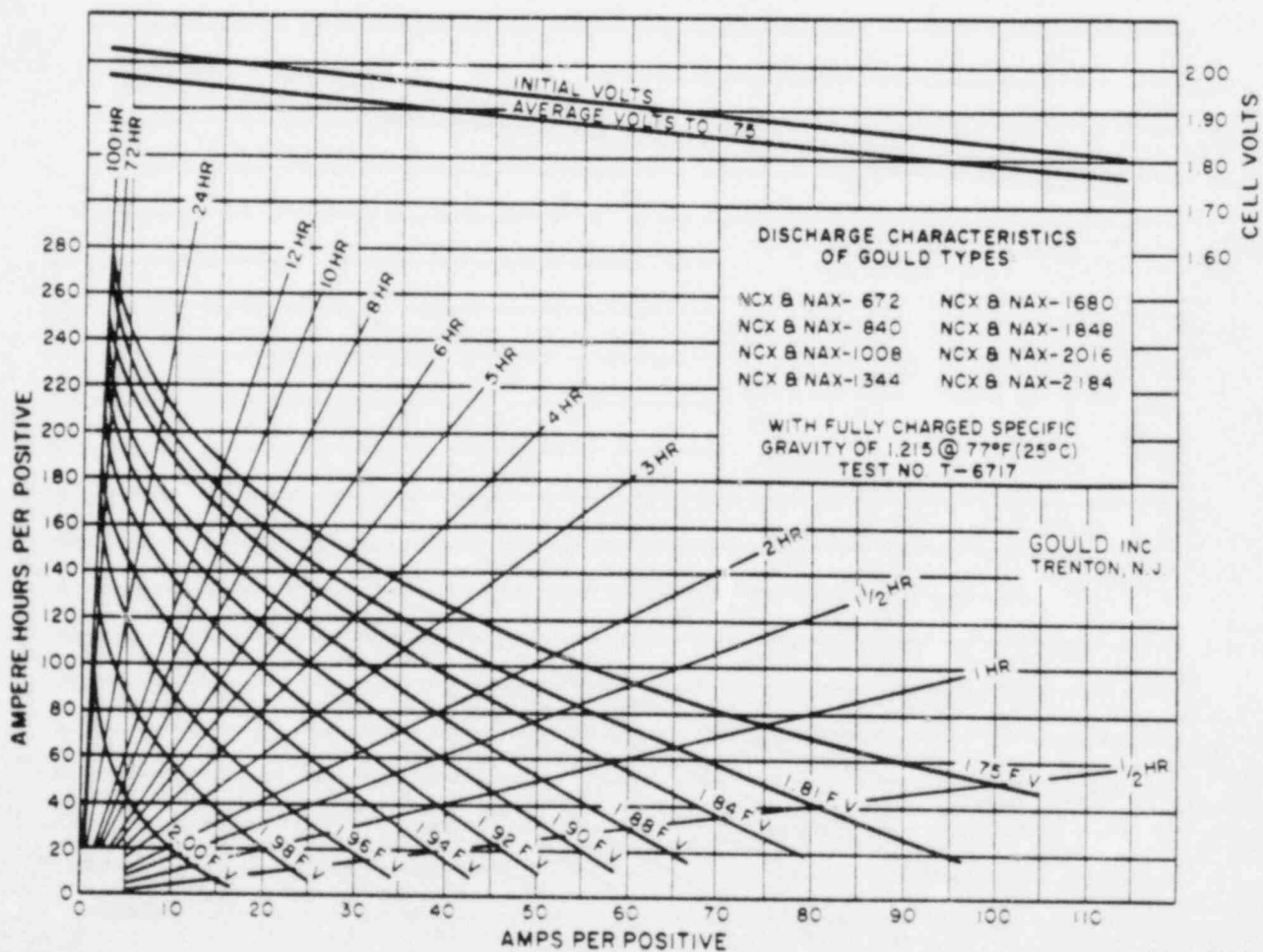
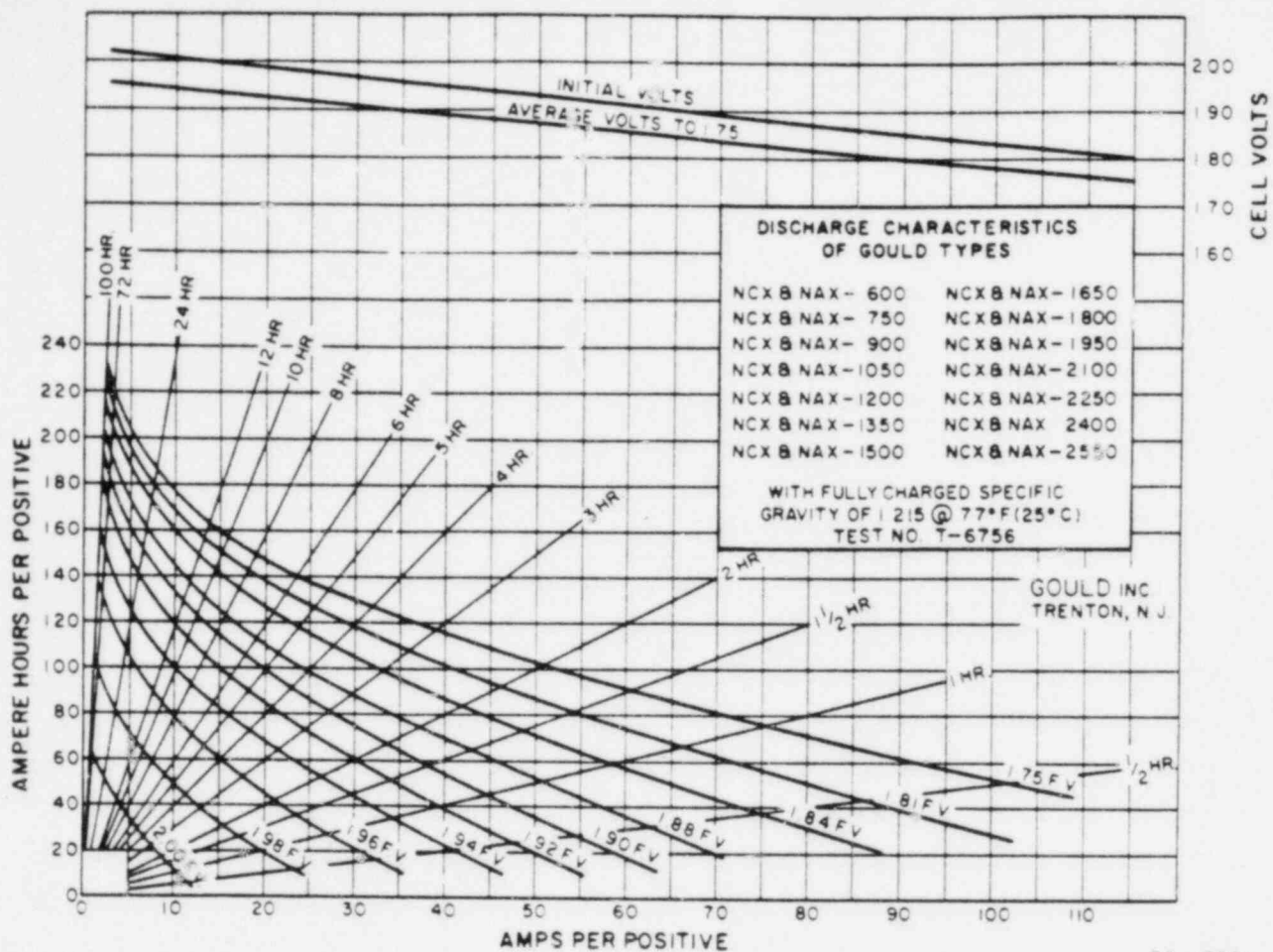
\* Includes voltage drop across intercell connections used in standard layouts.

\*\*™Gould, Inc.



# GOULD

Gould Inc., Industrial Battery Division  
 487 Calhoun Street, Trenton, New Jersey 08607  
 Telephone: (609) 392-3111





# GOULD

An Electrical/Electronics Company

## Stationary Power Cells (Calcium)

### TYPE: NCX

CAPACITIES—600 A.H. TO 2550 A.H.  
@ 8 HOUR RATE TO 1.75 V.P.C. AVERAGE

#### SPECIFICATIONS

Container — Styrene-Acrylonitrile Plastic.

Cover — Butadiene Styrene.

Separators — Microporous Material.

Retainers — Fiberglass Mats.

Posts — See Below.<sup>①</sup>

Post Seals — Floating O-Ring — Seal Nut.

Vents — Gould "Pre-Vent",\*\*

Level Lines — High and Low — All Jar Faces.

Electrolyte — Height Above Plates — 2-3/4".

Electrolyte Withdrawal Tubes — Each Cell.

Sediment Space — 1-1/16".

Specific Gravity — 1.215 @ 77°F. (25°C.).

Inter-Cell Connectors — Lead Plated Copper.



Plate Dimensions	Height	Width	Thickness
Positive Plate	15"	12 1/2"	.320
Negative Plate	15"	12 1/2"	.215

Posts—600 A.H. to 1200 A.H. Two—1 1/2" square 1344 A.H. to 1950 A.H. Four—1" square (Except 1848 A.H.) 1848 A.H. to 2550 A.H. Four—1 1/2" square

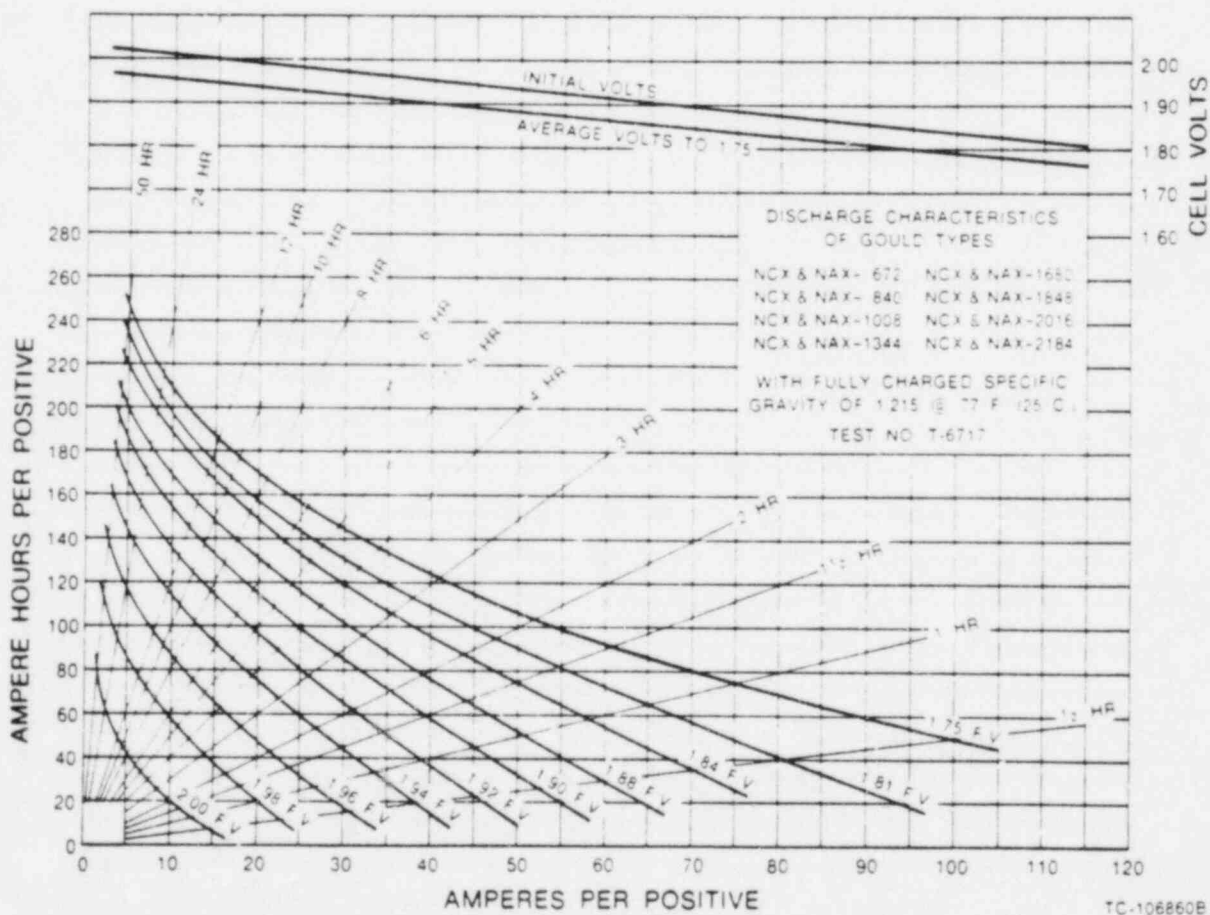
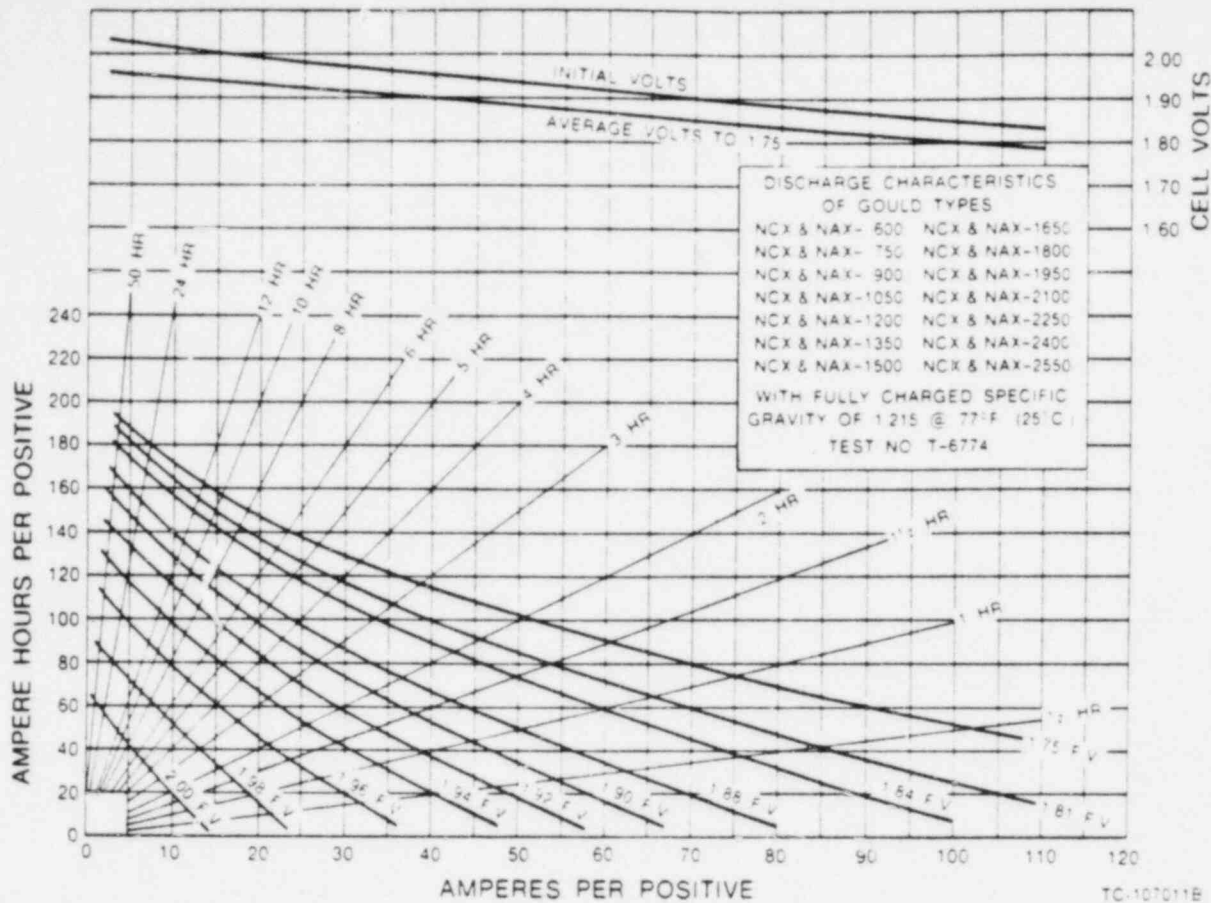
Type	Plates Per Cell	Ampere Hour Capacities to 1.75 V.P.C. Average*				1 Minute Rate in Amperes*		Overall Dimensions in inches			Approximate Wgt. in Lbs.		Elect. Gals. Per Cell
		8 Hr.	5 Hr.	3 Hr.	1 Hr.	To 1.75 V.P.C. Avg.	To 1.50 V.P.C. Avg.	L	W	H	Net Wgt.	Packed Wgt.	
NCX-600	9	600	540	468	300	712	1355	7-3/8	14-1/2	22-1/8	177	189	6.0
NCX-672	9	672	588	492	300	636	1210	7-3/8	14-1/2	22-1/8	178	190	6.0
NCX-750	11	750	675	585	375	880	1675	7-3/8	14-1/2	22-1/8	195	207	5.6
NCX-840	11	840	735	615	375	790	1500	7-3/8	14-1/2	22-1/8	196	208	5.6
NCX-900	13	900	810	702	450	1044	1985	7-3/8	14-1/2	22-1/8	213	225	5.1
NCX-1008	13	1008	882	738	450	942	1790	7-3/8	14-1/2	22-1/8	214	226	5.1
NCX-1050	15	1050	945	819	525	1204	2290	7-3/8	14-1/2	22-1/8	231	243	4.9
NCX-1200	17	1200	1080	936	600	1360	2585	7-3/8	14-1/2	22-1/8	249	261	5.0
NCX-1344	17	1344	1176	984	600	1240	2360	9-1/4	14-1/2	22-1/2	266	280	6.8
NCX-1350	19	1350	1215	1053	675	1494	2840	9-1/4	14-1/2	22-1/2	282	294	6.3
NCX-1500	21	1500	1350	1170	750	1620	3080	9-1/4	14-1/2	22-1/2	301	313	6.0
NCX-1650	23	1650	1485	1287	825	1782	3390	11-3/8	14-1/2	22-1/2	348	366	8.0
NCX-1680	21	1680	1470	1230	750	1530	2910	11-3/8	14-1/2	22-1/2	332	350	8.3
NCX-1800	25	1800	1620	1404	900	1932	3675	11-3/8	14-1/2	22-1/2	364	382	7.6
NCX-1848	23	1848	1617	1353	825	1661	3160	14-9/16	14-1/2	22-1/2	397	415	12.6
NCX-1950	27	1950	1755	1521	975	2080	3955	11-3/8	14-1/2	22-1/2	380	398	7.3
NCX-2016	25	2016	1764	1476	900	1788	3400	14-9/16	14-1/2	22-1/2	415	433	12.1
NCX-2100	29	2100	1890	1638	1050	2240	4260	14-9/16	14-1/2	22-1/2	446	464	11.5
NCX-2184	27	2184	1911	1599	975	1924	3660	14-9/16	14-1/2	22-1/2	433	451	11.5
NCX-2250	31	2250	2025	1755	1125	2400	4565	14-9/16	14-1/2	22-1/2	462	480	10.9
NCX-2400	33	2400	2160	1872	1200	2560	4865	14-9/16	14-1/2	22-1/2	479	497	10.3
NCX-2550	35	2550	2295	1989	1275	2720	5170	14-9/16	14-1/2	22-1/2	496	514	9.7

\* All values are approximate. These figures are based on standard conditions. Used in standard layouts. © 1977 Gould Inc.

G3-3325E 2/80 25M

File #4-6

Printed in U.S.A.



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Telephone (215) 752-0555

CABLE GOULNATBAT, LANGHORNE, PA - TWX GOULD LAHN 510-667 2056



**GOULD**

An Electrical/Electronics Company



SECTION #5  
TEST RACK DRAWING  
AND  
CALCULATIONS

SECTION #5

TEST RACK DRAWING  
AND  
CALCULATIONS

# ASSEMBLY

BLACK CELL CONNECTIONS TO BE TIGHTENED WITH A TORSIONAL FORCE OF 50 FT. LBS. UNLESS NOTED.

BATTERY AT ALL FIRST CONNECTIONS TO BE MADE WITH 5/16-18X1.25 LG.

SS HEX HD BOLT NUT & WASHERS.

B) TERM CONNS TO BE ATTACHED WITH 5/16-18X1.25 LG.

SS HEX HD BOLT NUT & WASHERS.

C) ABOVE CONNS TO BE TIGHTENED WITH A TORSIONAL FORCE OF 12-13 FT. LBS.

TERM. PLATE  
COP. L.P. L01-067511  
2 PER CONN.

TERM LUG COP. L.P.  
L07-086042  
(250MCM) 2 PER CONN.

INTERCELL CONN COP. L.P.  
1.25X.25  
2, L03-086966-002  
2, L03-086967-002

TERM CABLE CONN.  
L06-087484-048-11  
2 PER CONN.  
(NOT SHOWN)

TORSIONAL FORCE  
OF 25-40 FT. LBS.

5 APPROX

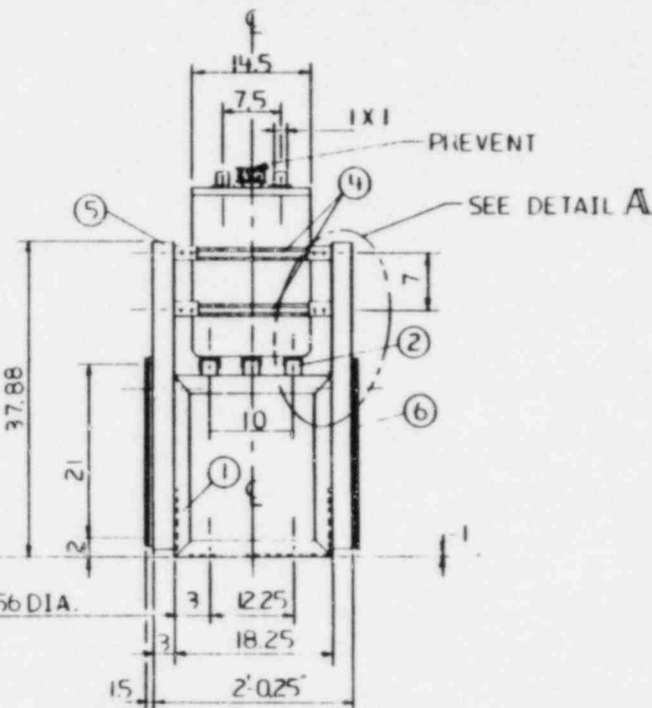
2'-11.125"

11.875

11.375

ANCHOR HOLE LOC'S .56 DIA.

MATERIAL REQ'D FOR 15070				15076 RACK	
PC#	QTY.	DESCRIPTION		PART NO.	
1	2	STEEL SUPPORT		S01-065899-009	
2	3	BASE STRINGER		S02-043301-002	
3	4	SIDE STRINGER		S02-043301-002	
4	4	END STRINGER		S02-043301-024	
5	4	UPRIGHT TUBING		S05-078917-073	
6	4	BRACING		S03-074940-018	
7	8	CORNER FITTING		S05-078440-002	
8	9	CELL STRIP		S04-106349-002	
9	2	CELL SPACER		S05-106333-007	
10	38	1/2-13 HEX HD BOLT	1.25"	W02-005330	
11	14		4"	W02-074552	
12	4		4.5"	W02-074385	
13	2		5"	W02-078931-002	
14	12	1/2 HEX NUT		W06-000383	
15	58	1/2 SPLIT LOCKWASHER		W03-005069	
16	46	1/2-13 CLAMPING NUT		S02-040124	
17	4	1/2 SPACING WASHER		S05-074400-009	



DETAIL A

GOULD		PLANT	DEPT
TITLE: LAYOUT FOR 3 CELLS NAX/NCX-1680 (ON SPECIAL TEST RACK S07076437676)			
DATE: 1-24	DRW: M.L.	CHKD: C.M.D.	APPD: [Signature]
SHEET 1 OF 1		0784376	

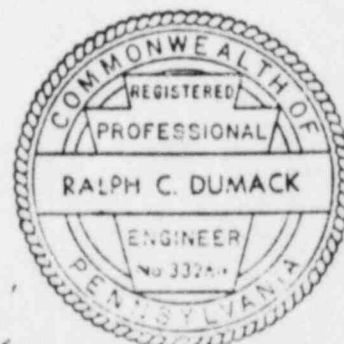
STRUCTURAL DESIGN OF TEST RACK  
FOR 3 CELLS OF NAX/NCX-1680  
GOULD BATTERIES

INDEX

DESCRIPTION

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SHOCK SPECTRUM, FRONT TO BACK / HORIZONTAL SSE	6
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DESIGN OF SIDE STRINGERS	7
DESIGN OF END STRINGERS	8
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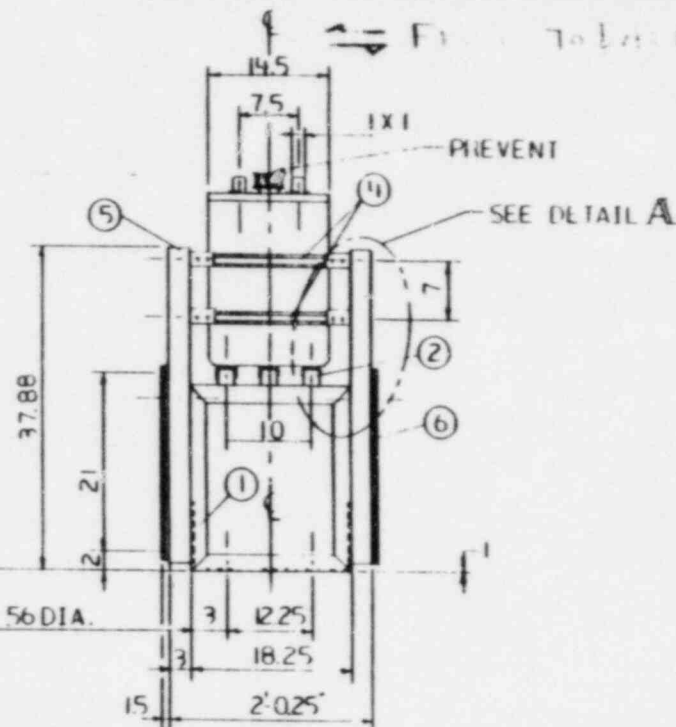
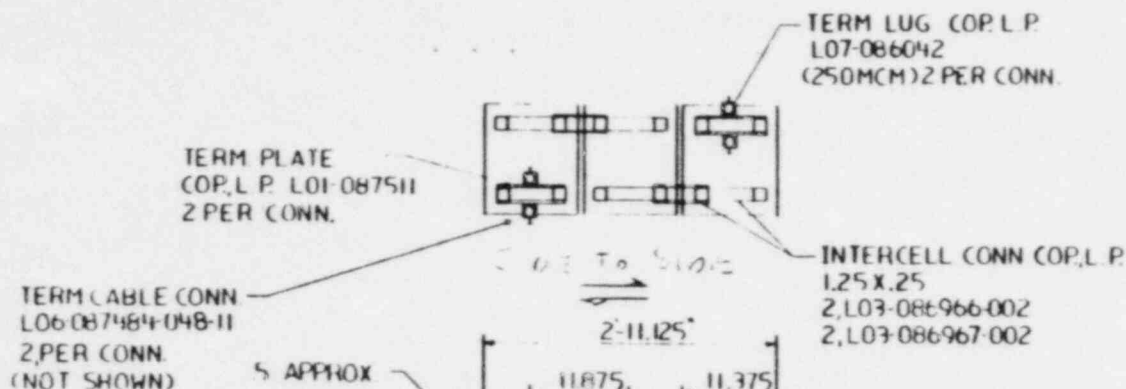


*Ralph C. Dumack*

# ASSEMBLY

- BACK CONNECTIONS TO BE TIGHTENED WITH A TORSIONAL FORCE OF 50 FT. LBS. UNLESS NOTED.
2. BATTERY A) ALL POST CONNECTIONS TO BE MADE WITH 5/16-18X2.5 LG. SS. HEX HD BOLT NUT & WASHERS.
- B) TERM CONNS TO BE ATTACHED WITH 5/16-18X1.25 LG. SS. HEX HD. BOLT NUT & WASHERS.
- C) ABOVE CONNS TO BE TIGHTENED WITH A TORSIONAL FORCE OF 12-13 FT. LB'S.

MATERIAL REQ'D FOR 1507				37-876 RACK
PC#	QTY	DESCRIPTION		PART NO
1	2	STEEL SUPPORT		S01-065899-009
2	3	BASE STRINGER		S02-043301-002
3	4	SIDE STRINGER		S02-043301-002
4	4	END STRINGER		S02-043301-024
5	4	UPRIGHT TUBING		S05-078971-073
6	4	BRACING		S03-074940-018
7	8	CORNER FITTING		S05-078440-002
8	9	CELL STRIP		S04-106347-002
9	2	CELL SPACER		S05-106333-007
10	38	1/2-13 HEX HD BOLT	1.25"	W02-005330
11	14		4"	W02-074552
12	4		4.5"	W02-074385
13	2		5"	W02-078931-002
14	12	1/2 HEX NUT		W06-000383
15	58	1/2 SPLIT LOCKWASHER		W03-005069
16	46	1/2-13 CLAMPING NUT		S02-074024
17	4	1/2 SPACING WASHER		S05-074400-009



S11#2

DETAIL A

GOULD				PLANT	DEPT
TITLE LAYOUT FOR 3CELL'S NAX/NCX 1680					
ON SPECIAL TEST RACK S070744-17876					
21	REV 1	DATE 6-1-74	BY 100	SHEET 2	OF 1

STRUCTURAL DESIGN OF TEST RACK  
FOR 3 CELLS OF NAX/NCX-1680  
GOULD BATTERIES

SHEET # 3

JOB 881

DESIGN DATA:

NATURAL FREQUENCIES:

23 Hz FRONT TO BACK/VERTICAL } REF. TO DYNAMIC  
70 Hz SIDE TO SIDE/VERTICAL } COMPUTER PRINTOUTS

DAMPING: 2% FOR BOLTED STEEL FRAMEWORK

SEISMIC LOADING FACTORS:

SSE (SAFE SHUTDOWN EARTHQUAKE) REF. GOULD FULL SCALE  
SHOCK SPECTRUM. FIG-8-5 & FIG. 8-6

HORIZ: GROUND ACCELERATION =  $2.3 \times g$   
VERT: " " =  $1.7 \times g$  } FRONT TO BACK/VERTICAL

HORIZ: GROUND ACCELERATION =  $2.3 \times g$   
VERT: " " =  $1.7 \times g$  } SIDE TO SIDE/VERTICAL

ALLOWABLE WORKING STRESSES

FOR NORMAL LOADINGS: AS PER AISC EIGHTH EDITION

FOR SSE: SHALL NOT EXCEED  $0.9 \times$  YIELD STRESS

WEIGHTS: (NAX/NCX-1680 BATTERIES)

3 CELLS =  $3 \times 332 = 996$  #

WT OF RACK (REF. GOULD DWG #078437C) = 265 #

MATERIALS:

STRUCTURAL STEEL SHAPES & PLATES: ASTM A-36,  $F_y = 36$  ksi

WELDED STEEL SHEET: ASTM A-570, GRADE C,  $F_y = 33$  ksi



DESIGN DATA (CONT'D):

SHEET #4

JUN 88

WELDING ELECTRODES: SMAW, AWS A5.1 OR A5.5 E70XX

SPECIFICATIONS:

A.I.S.C. 8TH EDITION

A.V.I.S. D1.1

TEST RACK COMPONENTS:

SUPPORT, SIDE & END STRINGERS -  $1\frac{5}{8}'' \times 1\frac{5}{8}'' \times 12$  GA. HUSKUT P100

SUPPORT FRAMES -  $L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$

VERTICAL TUBES - TS  $3 \times 3 \times 0.25$

DIAGONAL BRACING -  $\frac{1}{2}'' \times 3\frac{1}{2}''$

REFERENCE:  
WYLE REPORT PAGE III - 444  
FULL SCALE SHOCK SPECTRUM (g PEAK)  
100  
DAMPING 2%

K-E LOGARITHMIC  
5 x 3 CYCLES  
MADE IN U.S.A.  
46 7403  
KUPFFEL & EBER CO.

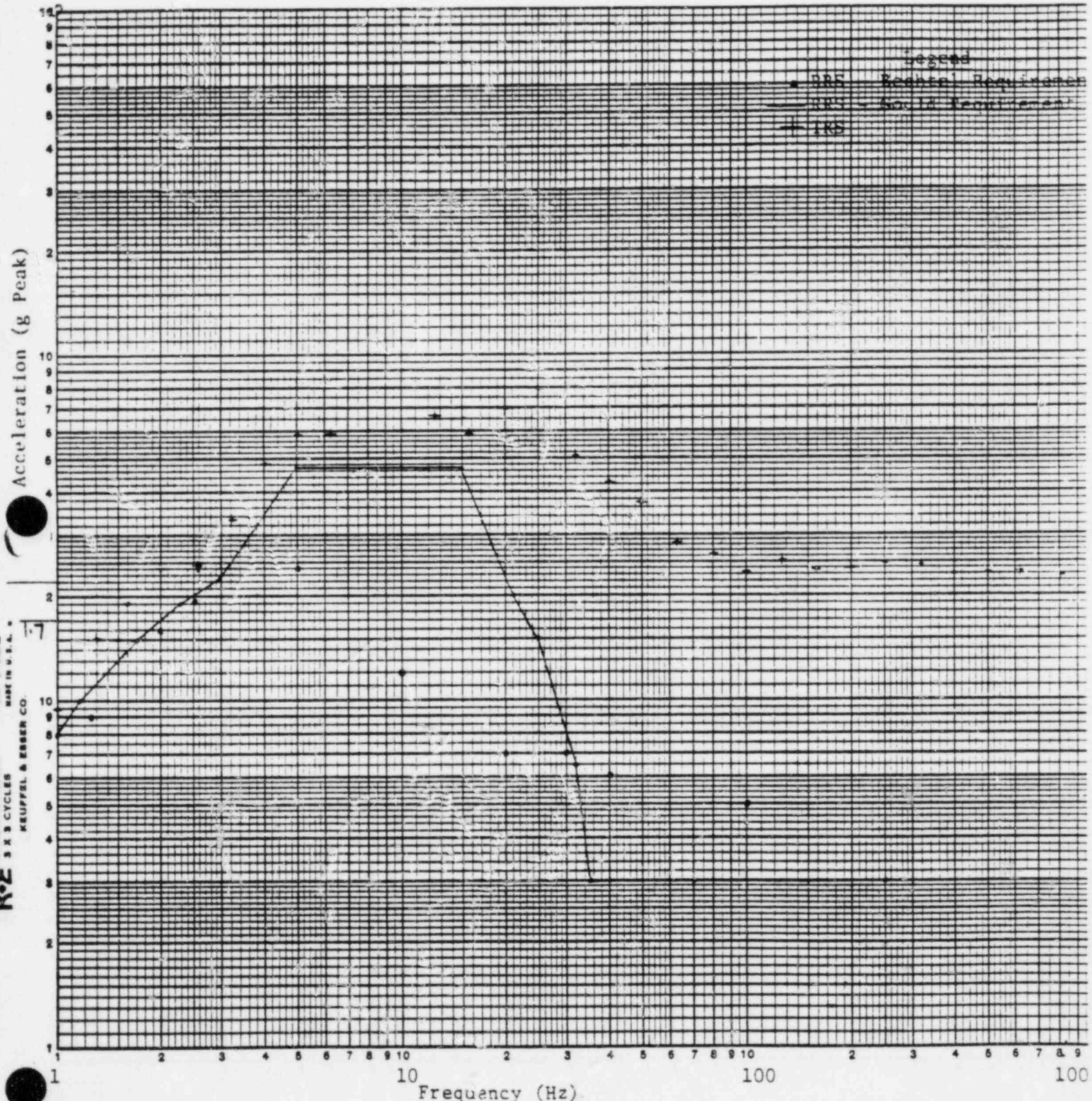


Fig. 3-5  
Vertical Side Shutdown Earthquake  
Front-to-back/vertical

REFERENCE:  
WYLE REPORT PAGE III - 439  
FULL SCALE SHOCK SPECTRUM (g PEAK)  
100  
DAMPING 2%

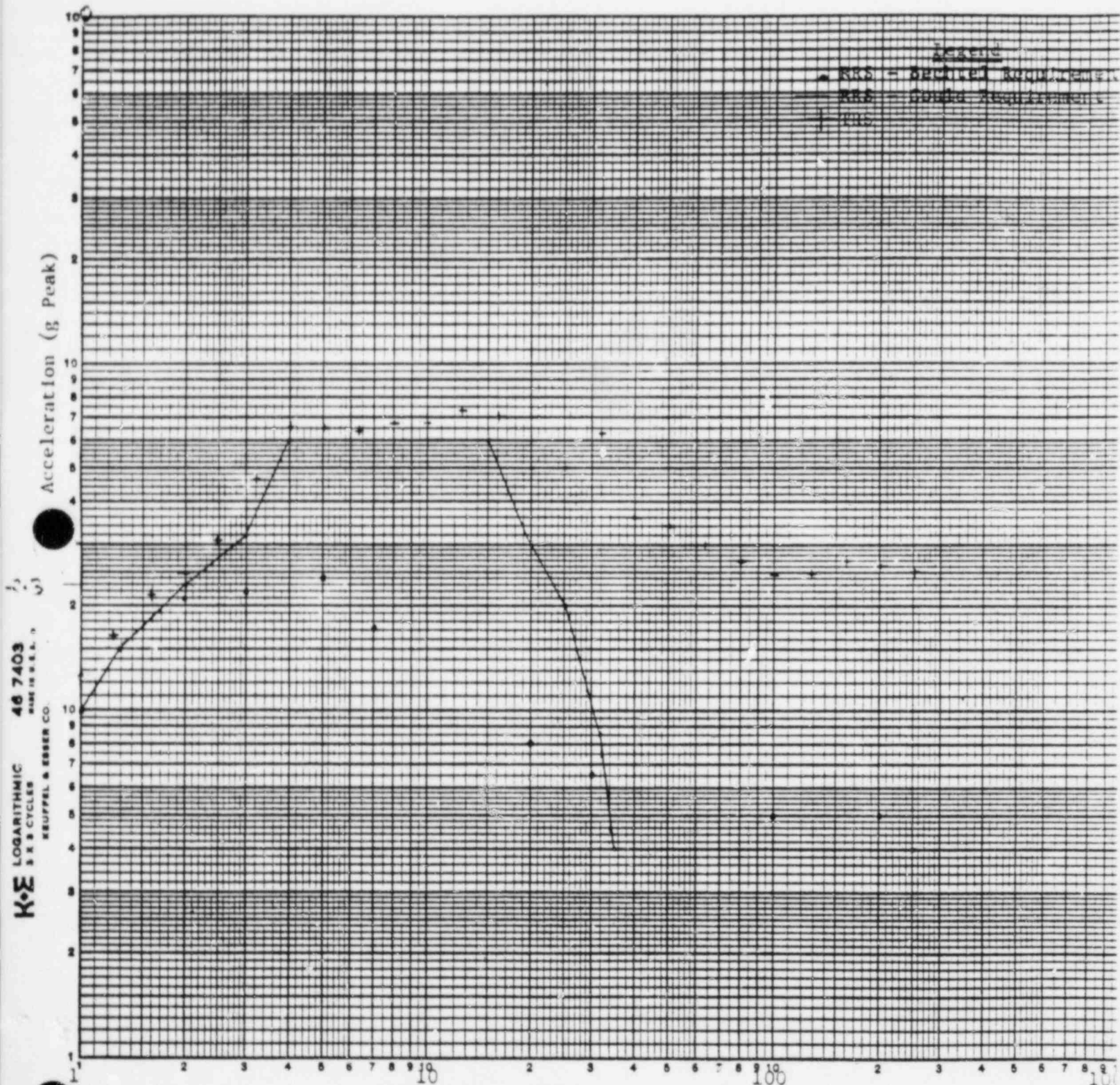


Fig. 8-6  
Horizontal Side Shutdown Earthquake  
Front-to-back/vertical

DESIGN OF MAIN STRINGERS:TOTAL WT. OF THREE NCX/NAX-1680 CELL =  $3 \times 332 = 996 \#$ 

THERE ARE THREE STRINGERS TO SUPPORT THIS LOAD. THEREFORE

LOAD FOR ONE STRINGER =  $996/3 = 332 \#$ U.D.L. ON ONE STRINGER =  $\frac{332}{2.93} = 113.4 + 1.9 = 115.3 \#/\text{FT}$  SELF WT

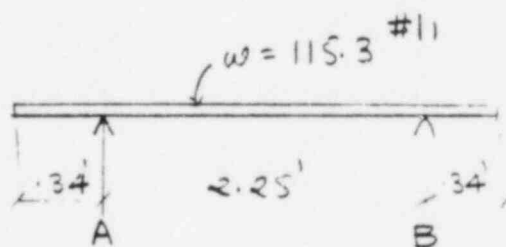
CASE I WT. OF STRINGER + WT OF CELL

$$w = 115.3 \#/\text{FT}$$

$$R_A = R_B = 115.3 \times \frac{2.93}{2} = 168.9 \#$$

$$M_A = M_B = -115.3 \times \frac{3.4^2}{2} = -6.67 \#-\text{FT}$$
$$= -80 \#-\text{IN.}$$

$$\text{MOMENT @ CENTER} = \frac{168.9 \times 2.25}{2} - 115.3 \times \frac{4.65^2}{2}$$
$$= 66.28 \#-\text{FT}$$
$$= 795.4 \#-\text{IN.}$$



STRINGERS ARE UNISTRUT P1000 - 1.63x1.63x12 GA.

$$A = 0.555$$

$$I_x = .126$$

$$I_y = .239$$

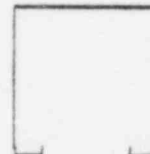
$$y_x = .579$$

$$S_x = .203$$

$$S_y = .294$$

$$y_y = .655$$

X



$$\therefore f_{bx} = \frac{795.4}{.203} = 3918 \text{ PSI} < .66 \times 33000 = 21780 \text{ PSI}$$

CASE II WT. OF STRINGER + WT OF CELL + SSE (FRONT TO BACK) - Y

$$w = 1.7 \times 115.3 + 115.3 = 311.3 \#/\text{FT}$$

$$M_A = M_B = -80 \times \frac{311.3}{115.3} = 216 \#-\text{IN.}$$

$$\text{MOMT @ CENTER} = 795.4 \times \frac{311.3}{115.3} = 2147.6 \#-\text{IN.}$$

$$f_{bx} = \frac{2147.6}{.203} = 10579 \text{ PSI} < 0.9 \times 33000 = 29700 \text{ PSI} \therefore \text{O.K.}$$

 $\therefore$  P1000 AS MAIN STRINGER IS O.K.DESIGN OF SIDE STRINGERS:

CASE II DEAD LOADS + SSE (FRONT TO BACK) + Z

$$w = 2.3 \times 115.3 \times 3/2 = 397.8 \#/\text{FT}$$

SIDE STRINGERS ARE UNISTRUT P1000 - 1.63x1.63x12 GA.

$$S_x = .203$$

$$A = .555$$



$$\text{MOMT } M_x (\text{MBX}) = 795.4 \times \frac{397.8}{115.3} = 2744.3 \text{ #-IN.}$$

$$\text{AXIAL LOAD} = 30^\# (\text{REF. COMP. OUTPUT 11CHB\#20 LOAD-2})$$

$$f_{bx} = \frac{2744.3}{.203} = 13518 \text{ PSI}$$

$$\frac{KL}{r_{\min}} = \frac{27}{.579} = 46.6 \quad F_a = 18.61 \times \frac{33}{36} = 17 \text{ KSI}$$

$$\therefore F_a \text{ FOR SSE} = 0.9 \times 33 \times \frac{17}{21.78} = 23.26 \text{ KSI}$$

$$f_a = 30\% \text{SSS} = 54 \text{ PSI}$$

CHECK AISC FORMULA 1.6-1a.

$$\frac{54}{23260} + \frac{13518}{29700} = .0023 + 0.46 = 0.457 < 1.0 \therefore \text{O.K.}$$

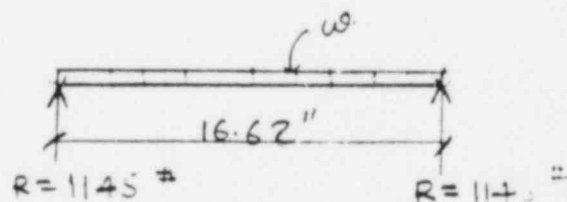
$\therefore$  P1000 AS A SIDE STRINGER IS O.K.

### DESIGN OF END STRINGERS:

$$\text{TOTAL WT OF CELLS} = 3 \times 332 = 996^\#$$

CASE II DEAD LOADS + SSE (SIDE TO SIDE)  
+X.

$$w = \frac{2.3 \times 996}{16.62} = 137.8 \text{ \#/IN.}$$



THERE ARE TWO STRINGERS TO RESIST THIS LOAD

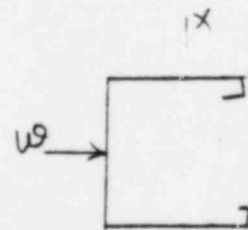
$$\therefore \text{LOAD } w \text{ PER STRINGER} = \frac{137.8}{2} = 68.92 \text{ \#/IN.}$$

$$\text{MOMT} = 68.92 \times \frac{16.62^2}{8} = 2379.56 \text{ #-IN.}$$

END STRINGERS ARE P1000 UNISTRUTS.

$$\therefore f_{bx} = \frac{2379.56}{.203} = 11722 \text{ PSI} < .9 \times 33000 = 29700$$

$\therefore$  O.K.



### CHECK VIBRATION:

$$f \text{ OF S.S. BEAM UNDER ITS OWN WT} = \frac{5}{324} \times \frac{1.9 \times 1.285 \times 12^3}{29 \times 10^6 \times .186} = 2.9 \times 10^{-5}$$

$$f = 657 \text{ CYCLES/SEC}$$

SHEET # 9

### DESIGN OF VERTICAL TUBES:

$$I_x = I_y = 3.16 \text{ in}^4 \quad \text{AREA} = 2.59 \text{ in}^2$$

$$C_2 = C_1 = 2.1 \times 10^3$$

CASE II DEAD WIT + SSE (SIDE TO SIDE) + X

P = REACTION FROM END STRINGER

$$= 68.92 \times 16.62/2 = 572.7 \text{ \#}$$

$$MOMT = 572.7(8+15) = 13172 \text{ #-IN.}$$

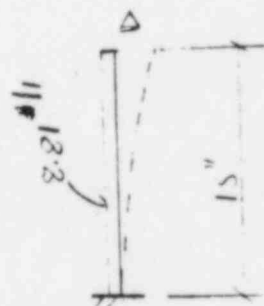
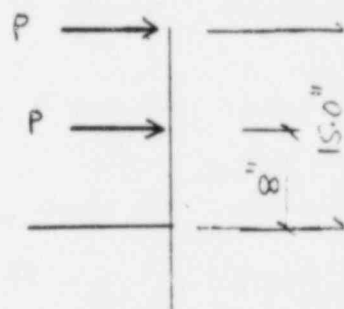
$$f_{bx} = \frac{13172}{2.1} = 6272 \text{ PSI} < .9 \times 36000 = 32400 \text{ PSI} \therefore \text{O.K.}$$

CHECK VIBRATION :

DEAD WT OF TUBE = 2.21 g

$$\Delta = \frac{wL^4}{8EI} = \frac{8.81 \times 1.25^4 \times 12^3}{8 \times 29 \times 10^6 \times 3.16} = 5.07 \times 10^{-5}$$

$$f = \frac{3.89}{\sqrt{\Delta}} = 546 \text{ CYCLES/sec}$$



CHECK THE BOLTS WHICH CONNECT SIDE STRINGERS TO END STR  
NGERS :

REACTION AT EACH END OF GIRD STRINGER = 1145 <sup>#</sup> ← S.S.E

THERE ARE TWO BOLTS AT EACH END

$\therefore$  LOAD PER BOLT = 573 #

FROM UNISTRUT CATALOG NO. 9 PAGE 14

EACH BOLT HAS RESISTANCE TO SLIP = 1500 #

З. О. К.

PULL OUT STRENGTH = 2000 #

∴ O.K.



I. J. P. 5-13-82

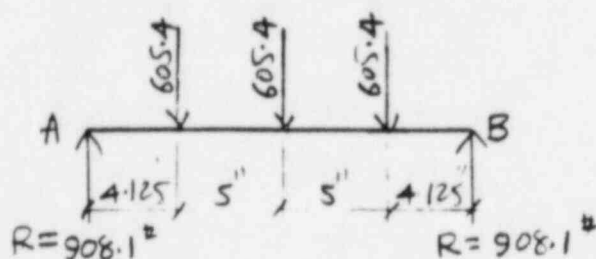
SHEET # 10

JOB 881

# DESIGN OF ANGLE FRAME: DESIGN OF HORIZ. MEMBER.

## CASE II DEAD LOADS + S S E (FRONT TO BACK)

$L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$   $A_x = 1.19$   $S_x = .394$   $\gamma = .491$



SIMPLE SUPP. MOMT AT 2.5" FROM A & B  
 $= 908.1 \times 2.5 = 2270 \text{ #-IN.}$

$\therefore$  DESIGN MOMT AT 2.5" FROM A  
 $= 16172 - 2270$   
 $= 13902 \text{ #-IN.}$

COMPUTER ANALYSIS WAS PERFORMED APPLYING THE RACK WT + BATT. WT. HORIZ. COMPONENT AT THE TOP. BUT RACK WT IS UNIFORMLY DISTRIBUTED THROUGH THE HEIGHT OF RACK. ADJUSTING FOR THIS EFFECT: NET HORIZ. LOAD AT THE TOP  $= 3 \times 332 \times 2.3 = 1145.4 \text{ #}$  AS COMPARED TO APPLIED LOAD  $= 1547.19 \text{ #}$

$\therefore$  REDUCTION FACTOR  $= \frac{1145.4}{1547.19} = 0.74$

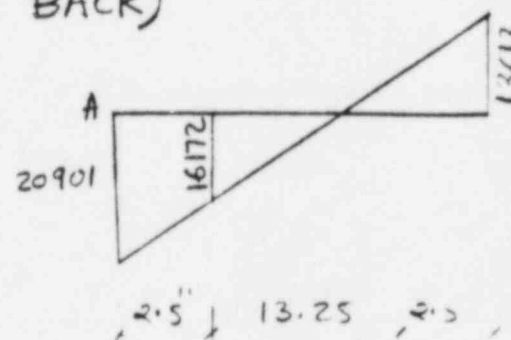
$\therefore$  NET DESIGN MOMENT  $= .74 \times 13902$   
 $= 10291.8 \text{ #-IN.}$

$\sigma_{bx} = \frac{10291.8}{.394} = 26121 \text{ PSI}$

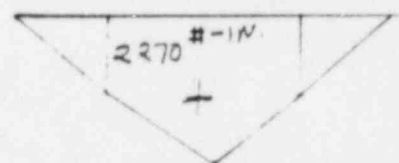
AXIAL LOAD  $= 844 \text{ #}$   $\therefore f_a = \frac{844}{1.19} = 709 \text{ PSI}$

$\frac{KL}{\gamma} = \frac{13.75}{.491} = 28.0$   $F_a = 20.15 \text{ KSI} = .56 F_y$

$\therefore$  FOR S S E  $= .9 \times 36 \times \frac{.4}{1.5} = 8.64 \text{ FT}$



MOMENT OF CONT. T.Y  
 REF. COMP. OUTPUT  
 MEMB # 35 LOAD # 1



SIMPLE SUPP MOMT.

I.J.P. 5-13-22

SHEET # 11

JOB 881

CHECK AISC FORMULA 1.6-1 c

$$\frac{26121}{.9136000} + \frac{709}{27500} = 0.80 + 0.025 = 0.825 < 1.0 \therefore \text{O.K.}$$

DESIGN OF VERTICAL MEMBER:

MOMENT = 13918 #-IN. (COMP OUTPUT MOMENT #2 LOAD #2)

AXIAL LOAD = 2019 #

$\therefore$  NET DESIGN MOMENT =  $.74 \times 13918 = 10299$  #-IN.

$$f_{bx} = \frac{10299}{.394} = 26140 \text{ PSI} \quad f_a = \frac{2019}{1.17} = 1697 \text{ PSI}$$

$$\frac{26140}{.7 \times 36000} + \frac{1697}{27500} = 0.80 + 0.062 = 0.86 < 1.0 \therefore \text{O.K.}$$

DRW 5-13-82

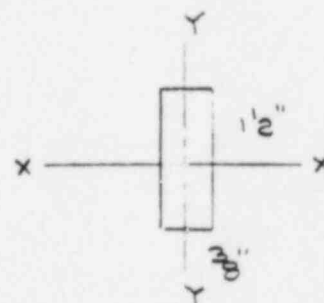
SHEET # 12  
JOB 981

# DESIGN OF BRACING:

MAX LOAD :  $\pm 1550^{\#}$

MEMBER -  $1\frac{1}{2}'' \times 3\frac{3}{8}''$  FLAT PL

$l = 17.4'$



$$\begin{aligned} A &= 0.563 \text{ in}^2 & J &= 0.006 \text{ in}^4 \\ I_x &= 0.106 \text{ in}^4 & I_y &= 0.0066 \text{ in}^4 \\ S_x &= 0.14 \text{ in}^3 & Z_x &= 0.0352 \text{ in}^3 \\ r_x &= 0.432 \text{ in} & r_y &= 0.108 \text{ in} \end{aligned}$$

$$\begin{aligned} \text{slend. ratio} &= 17.4 / 0.108 = 161.1 < 200 \quad \therefore \text{OK} \\ \rightarrow F_a &= 7.25 \text{ ksi} \quad \text{FROM AISC TABLE C-36} \end{aligned}$$

$$f_a = 1.55 / 0.563 = 2.75 \text{ ksi} < F_a \quad \therefore \text{OK (COMPRESSION)}$$

$$\begin{aligned} \text{NET AREA} &= 0.563 - 0.625 \times \frac{3}{8} = 0.329 \text{ in}^2 \leftarrow \text{CONVERTING} \\ &= 0.85 \times 0.563 = 0.479 \text{ in}^2 \end{aligned}$$

$$f_a = 1.55 / 0.329 = 4.71 \text{ ksi}$$

$$F_a = 0.5 (58) = 29 \text{ ksi}$$

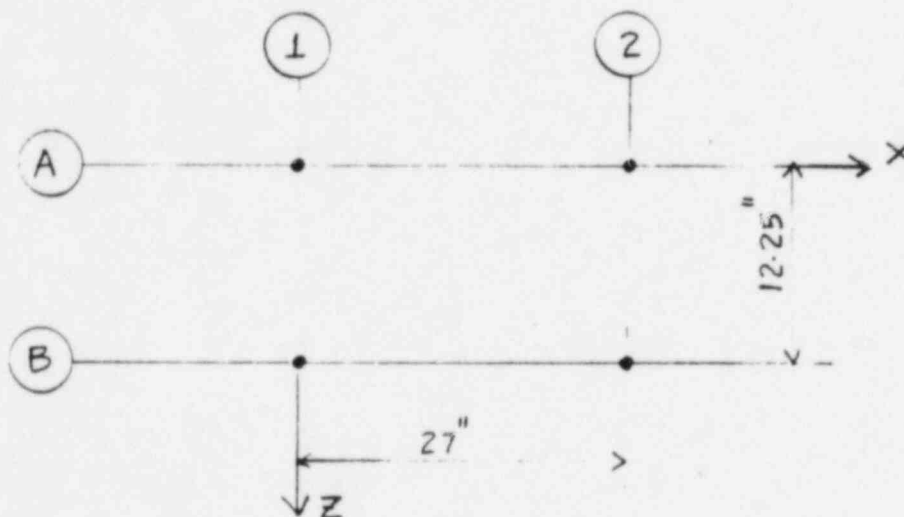
$$0.4 (36) = 22 \text{ ksi} > f_a \quad \therefore \text{OK}$$

I. J. P. 5-11-82

SH # 13

JOB 8-1

FOUNDATION LOADING TABLE: ALL LOADS ARE 111 LBS.



LOCATION		A-1, A-2 B-1, B-2
LOADING	VERTICAL	4662
	HORIZ. Z	1057
WT. OF RACK+CELLS +S S E (FRONT TO BACK/ VERT.)	X	100
WT. OF RACK+CELLS +S S E (SIDE TO SIDE/ VERT.)	VERTICAL	3029
	HORIZ. X	815
	Z	211
NET UPLIFT. S S E (FRONT TO BACK/VERT.)		3990
NET UPLIFT. S S E (SIDE TO SIDE/ VERT.)		2356

DESIGN OF ANCHOR BOLTS

$$\text{NET UPLIFT} = 3.989^k$$

$$\text{HORIZ. SHEAR, } x = 0.151^k$$

$$\text{" " " } z = 1.147^k$$

$$\text{RESULTANT SHEAR} = \sqrt{0.151^2 + 1.147^2} = 1.157^k$$

$$\text{ANCHOR BOLTS ARE } \frac{1}{2}'' \phi \text{ A-307 BOLTS. } A = 0.196 \text{ in}^2$$

$$f_v = 1.157 / 0.196 = 5.9 \text{ ksi} < F_v = \frac{0.166}{0.66} \times 60 \times 0.9 = 13.6 \text{ ksi}$$

$$f_t = 3.989 / 0.196 = 20.3 \text{ ksi}$$

$$(\text{SEISMIC}) \quad F_t = \frac{0.33}{0.66} \times 0.9 \times 60 = 27 \text{ ksi} \quad (\text{w/o SHEAR})$$

$$(\text{w/o SEISMIC}) \quad F_t = 26 - 1.8 \left( \frac{f_v}{5.4} \right) = 15.4 \text{ ksi} \quad (\text{w/ SHEAR})$$

$$(\text{w/ SEISMIC}) \quad F_t = \frac{0.75}{0.66} \times 0.9 \times 60 = 21 \text{ ksi} \quad (\text{w/ SHEAR})$$

$$f_t < F_t \quad \therefore \text{OK} \quad (\text{COMBINED STRESS})$$

ARITHMETIC FOR COMPUTER LOADINGS:

DYNAMIC ANALYSIS: WT. OF THE BATTERIES HAS BEEN TAKEN INTO ACCOUNT BY INCREASING THE DENSITY OF THE CORRESPONDING MEMBERS.

STRESS ANALYSIS:

MAIN STRINGER REACTION ON ANGLE FRAME DUE TO NORMAL (WT. OF STRINGER + WT. OF CELLS) LOADINGS =  $168.9 \#$  REF S11 #  
TOTAL NO OF STRINGER REACTIONS ON ANGLE FRAME =  $2 \times 3 = 6$   
WT. OF RACK =  $265 \#$  RACK WT IS TAKEN INTO ACCOUNT BY INCREASING THE STRINGER REACTIONS.

$\therefore$  ADDITIONAL LOAD AT REACTION POINTS =  $265/6 = 44.20 \#$   
 $\therefore$  MAIN STRINGER REACTION ON ANGLE FRAME DUE TO RACK WT + BATT. WT.  $P = 168.9 + 44.2 = 213.10 \# + 5\% \text{ MISC} = 224.23 \#$

LOAD-1 RACK WT. + BATT. WT. + S S E FRONT TO BACK | VERT. (-Y, +Z)

$$P = 224.23 \times 2.7 = 605.42 \#$$

$$\text{HORIZ. FORCE Z AT EACH ANGLE FRAME} = 3 \times 224.23 \times 2.3 = 1547.20$$

LOAD-2 RACK WT. + BATT. WT. + S S E FRONT TO BACK | VERT. (+Y, +Z)

$$P = 224.23 - 1.7 \times 224.23 = -156.96 \#$$

$$\text{HORIZ. FORCE Z AT EACH ANGLE FRAME} = 1547.20 \#$$

LOAD-3 RACK WT. + BATT. WT. + S S E SIDE TO SIDE | VERT. (+X, -Y)

$$P = 224.23 \times 2.7 = 605.42 \#$$

$$\text{HORIZ. FORCE X AT EACH END JOINT} = 1547.20 \#$$

LOAD-4 RACK WT. + BATT. WT. + S S E SIDE TO SIDE | VERT. (+X, +Y)

$$P = 224.23 - 1.7 \times 224.23 = -156.96 \#$$

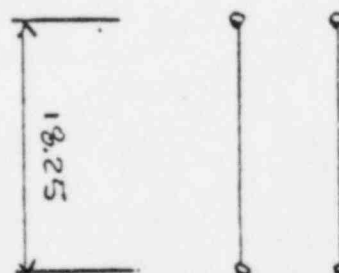
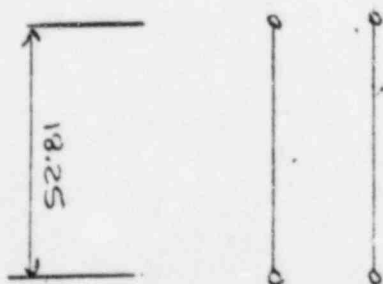
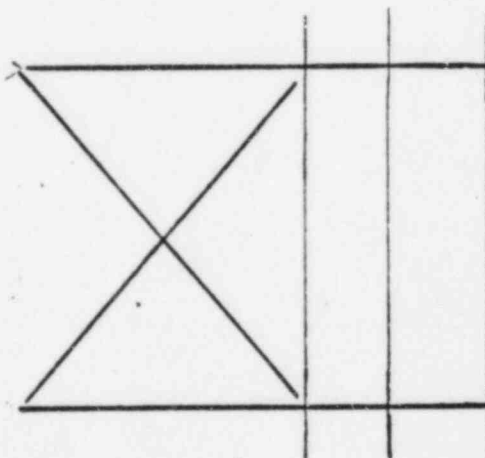
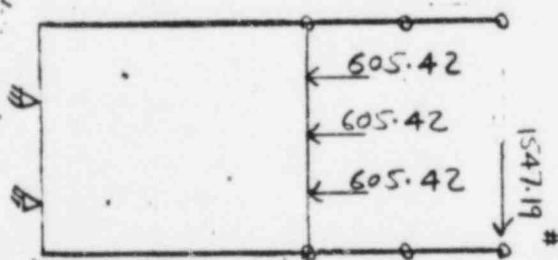
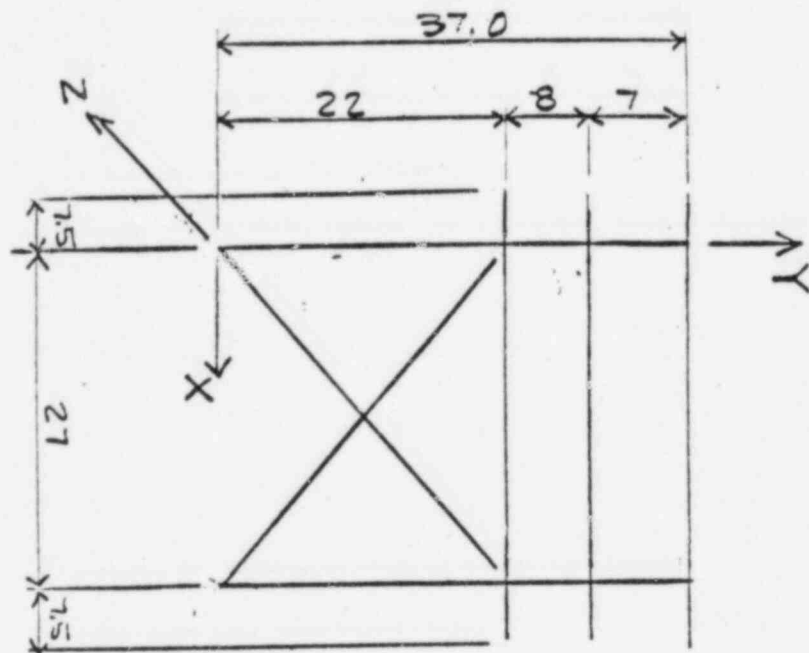
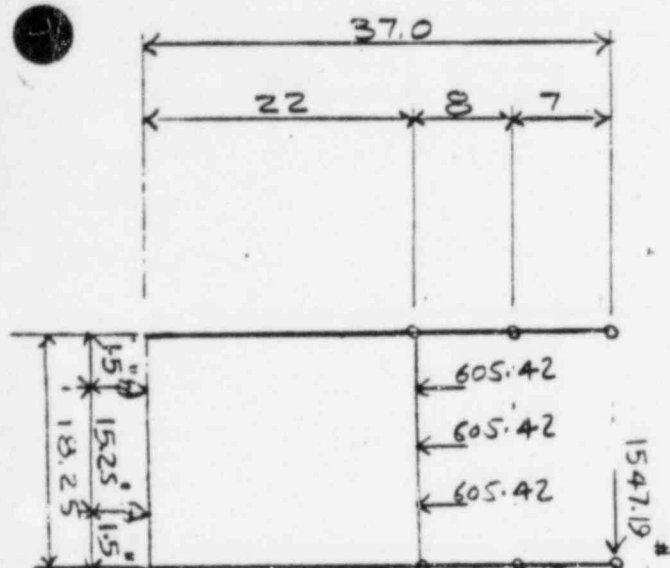
$$\text{HORIZ. FORCE X AT EACH END JOINT} = 1547.20 \#$$



# LOAD-1

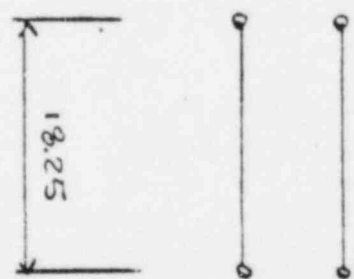
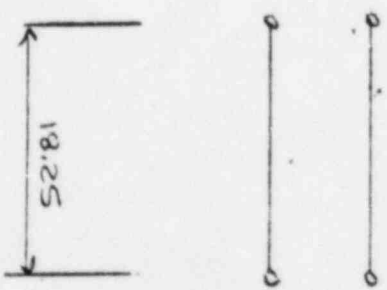
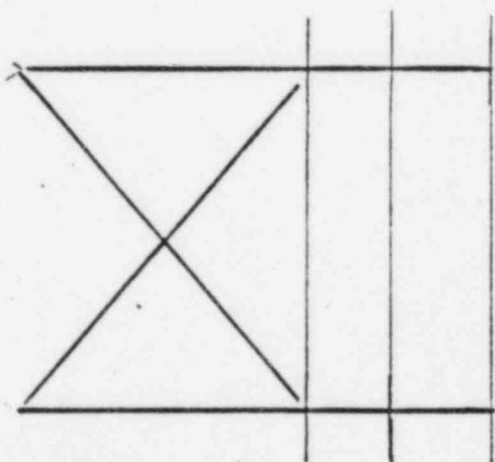
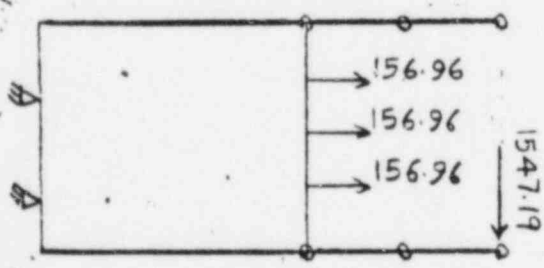
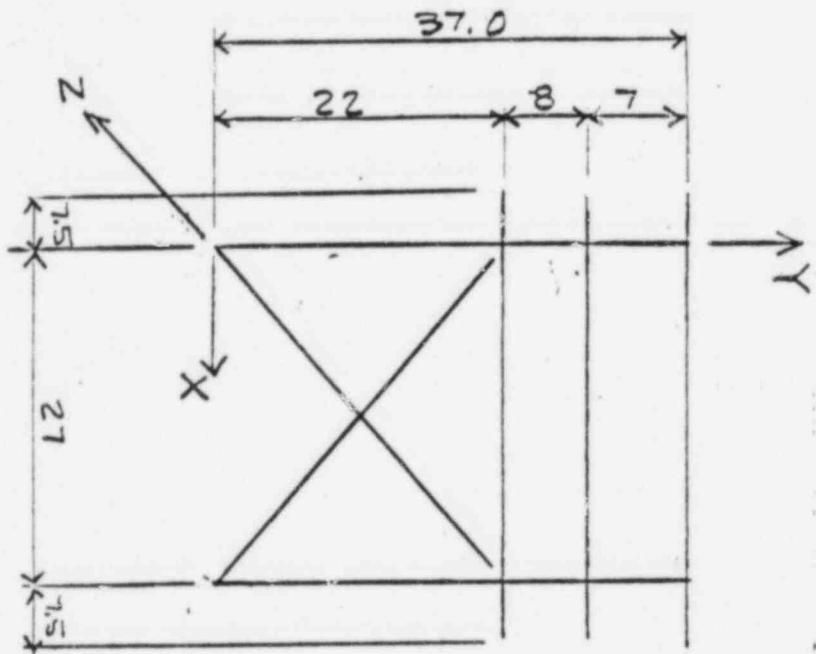
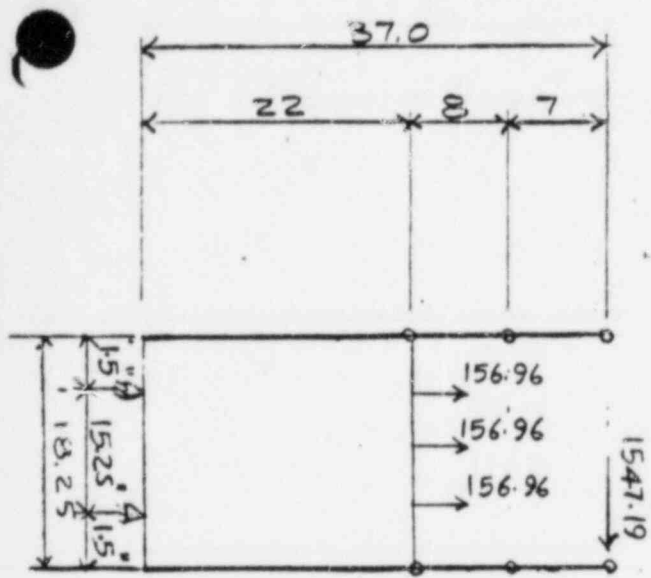
RACK WT. + BATT. WT + SSE FRONT TO BACK (VERT. (-Y, +Z))

SH#16



LOAD - 2  
 RACK WT + BATT. WT. + S<sup>-</sup>SE FRONT TO BACK | VERT. (+Y, +Z)

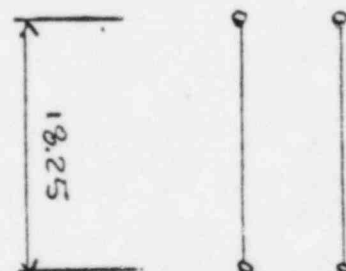
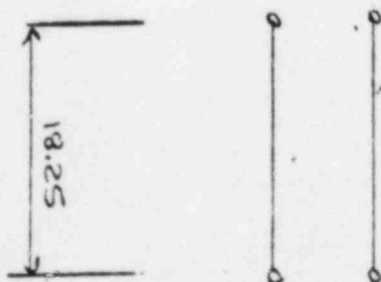
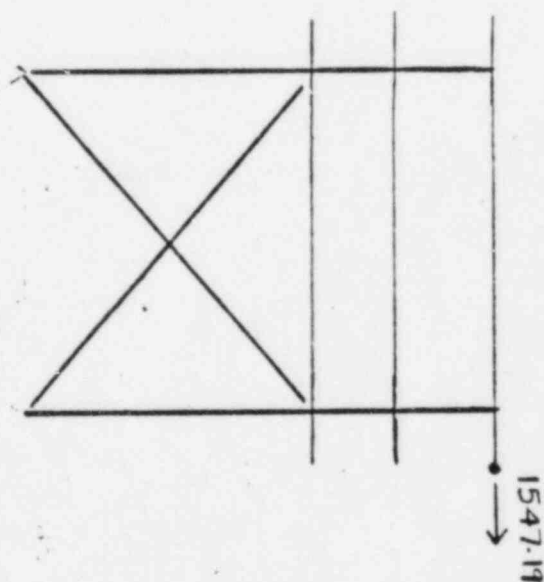
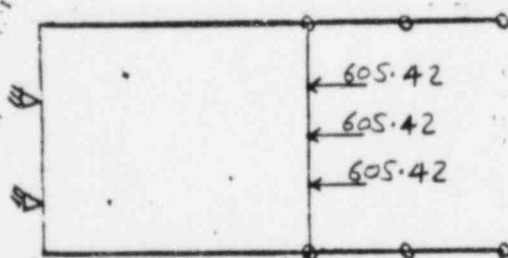
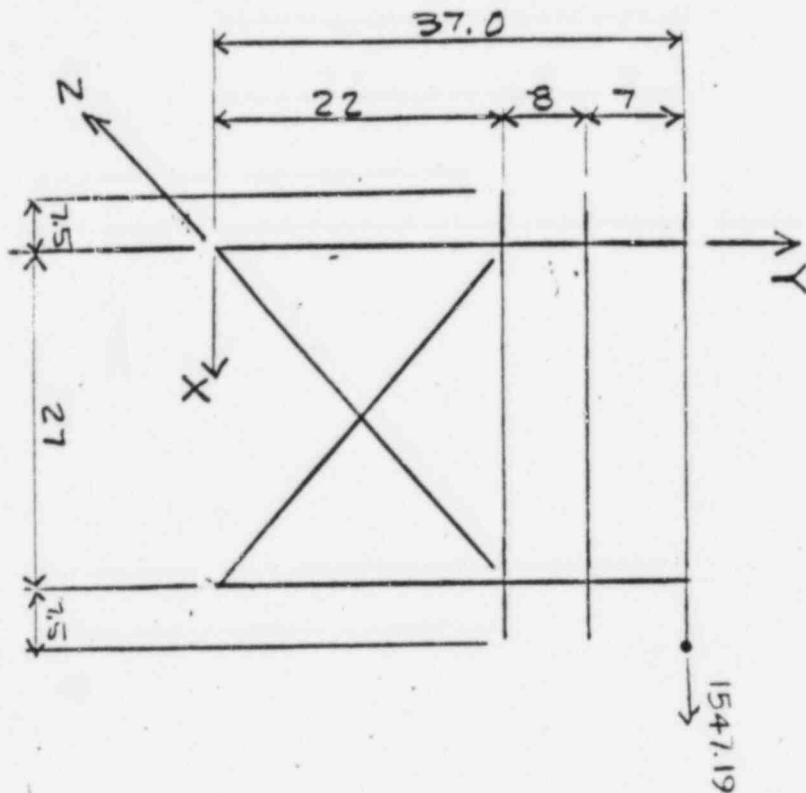
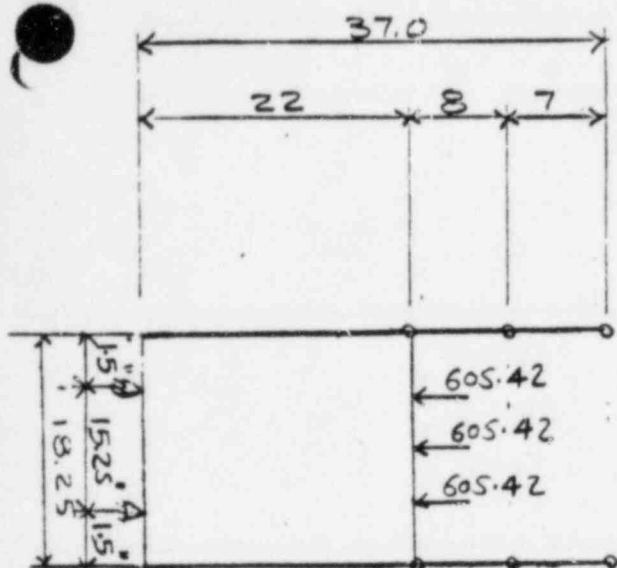
SH<sup>2</sup>T<sup>2</sup>



# LOAD-3

RACK WT + BATT. WT. + S S E SIDE TO SIDE VERT. (+X, -Y)

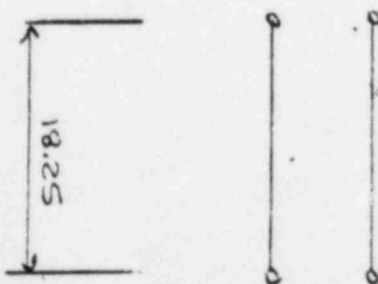
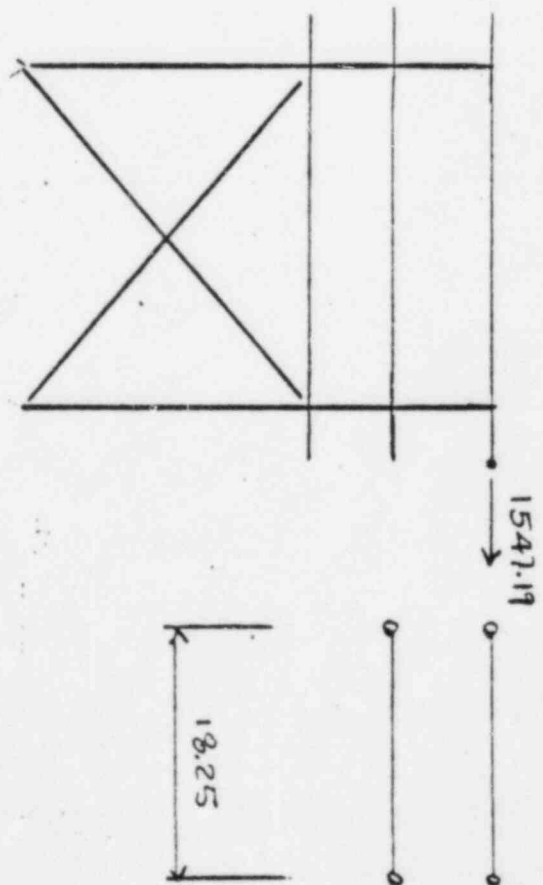
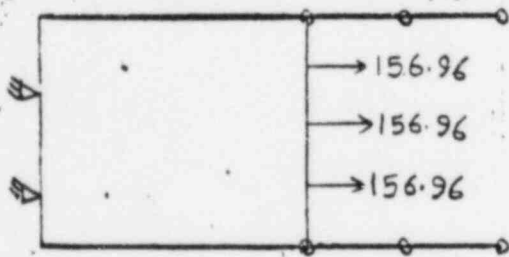
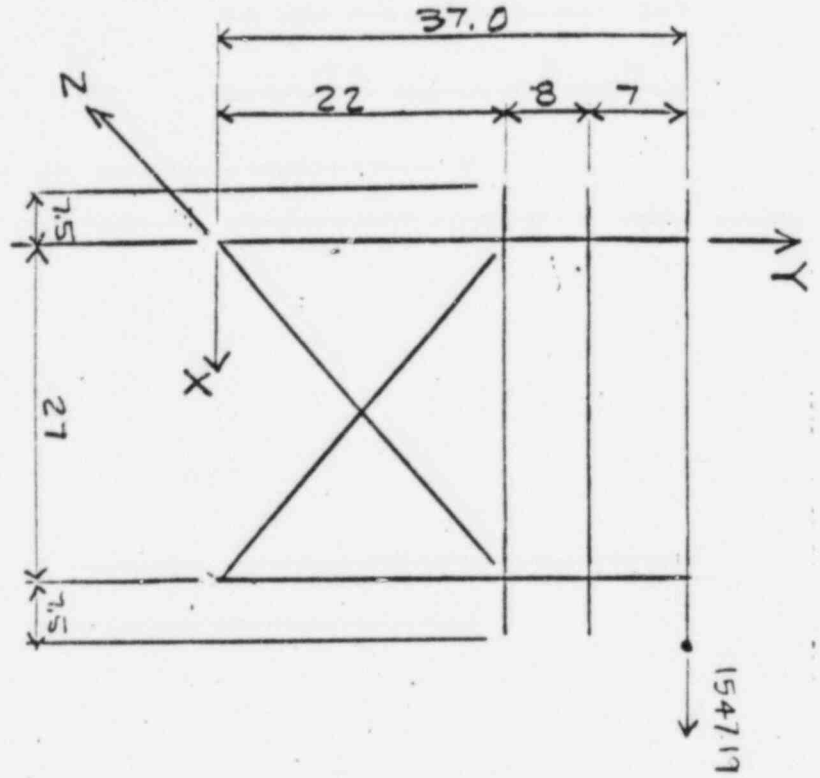
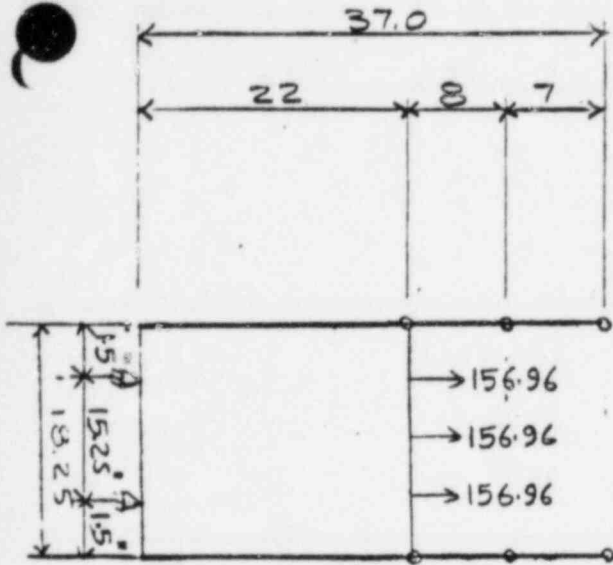
SH#75



# LOAD - 4

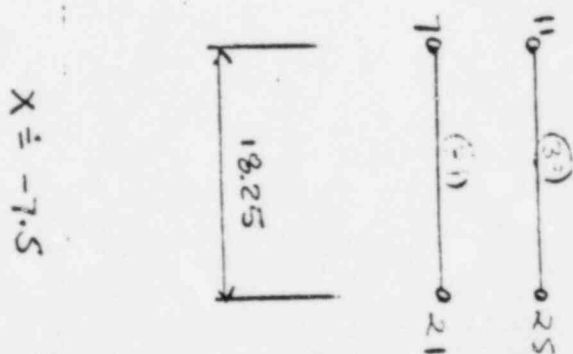
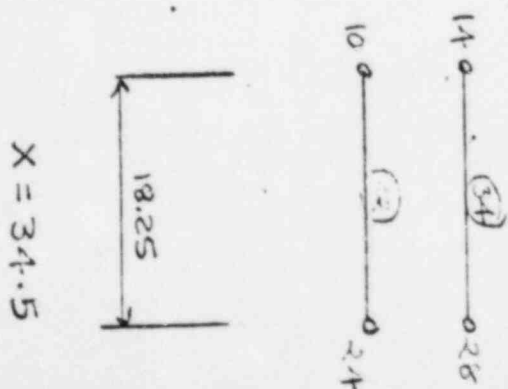
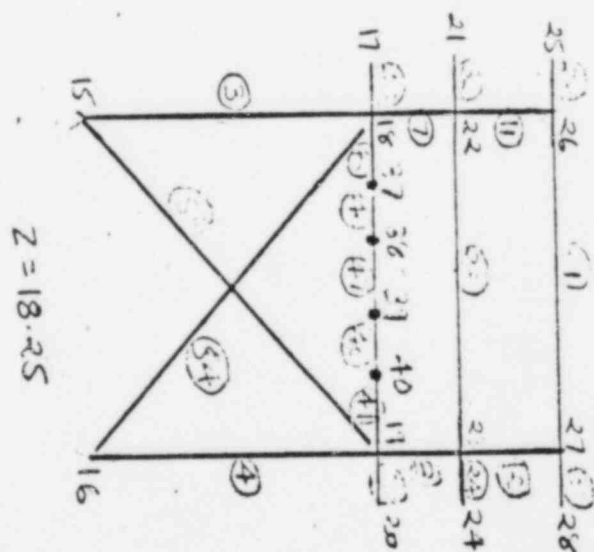
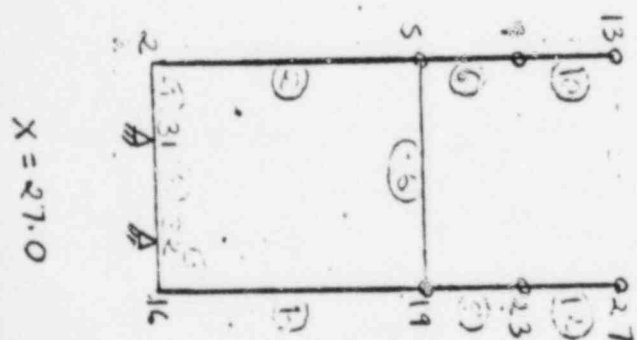
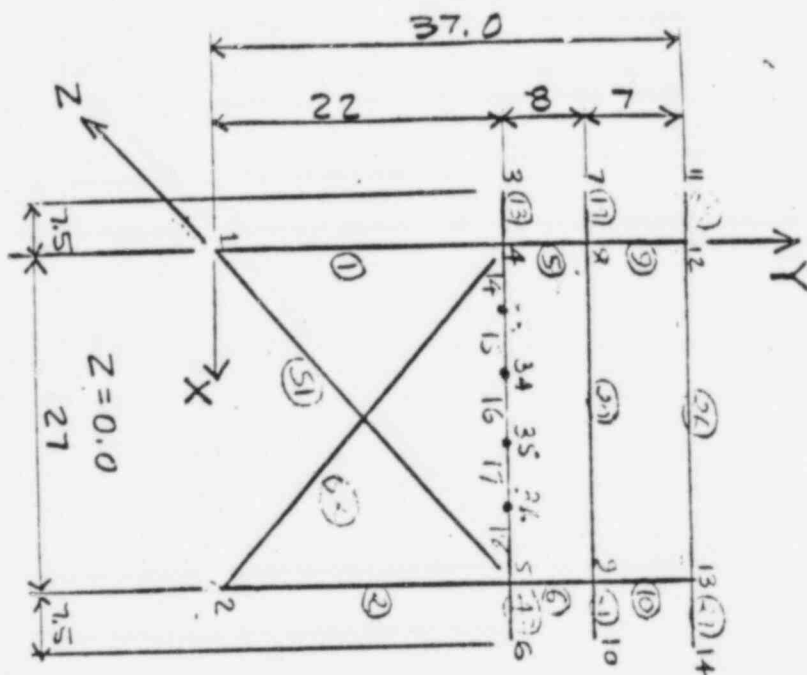
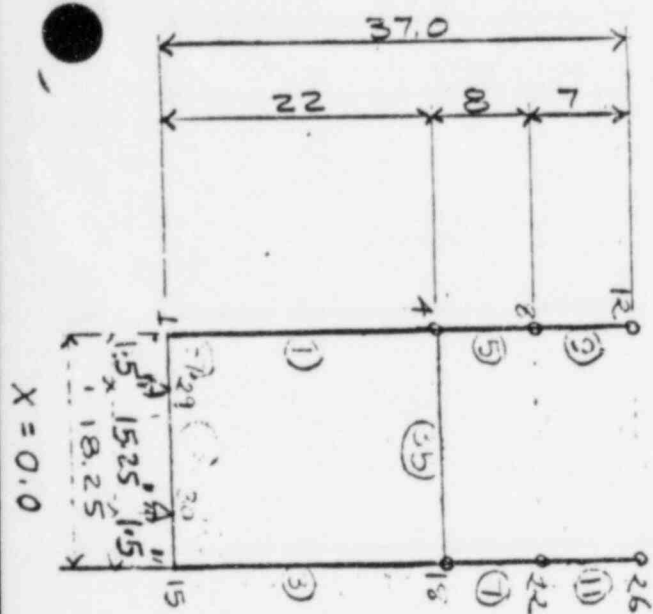
RACK WT. + BATT VIT. 7 S S E SIDE TO SIDE (VERT. (+X, +Y))

SII #19



I. J. P. 5-3-82

SH # 20  
JOB 881



R.C.D. 5-12-92

SHEET #21 OF #.

CONCLUSION:

JOB 881

A DYNAMIC ANALYSIS WAS MADE OF THE TEST RACK AS A WHOLE, CONSIDERING TWO LOADING CASES AND THEIR RESULTING MINIMUM NATURAL FREQUENCIES AS FOLLOWS:

1. COMBINED VERTICAL AND FRONT TO BACK DIRECTIONS  
MIN. 23 CYCLES PER SECOND.
2. COMBINED VERTICAL AND SIDE TO SIDE DIRECTIONS  
MIN. 73 CYCLES PER SECOND.

THE TEST RACK AS A WHOLE AS WELL AS THE MAIN SUPPORT STIFFNESS FREQUENCIES WERE DETERMINED BY THE GT STRUDL COMPUTER PROGRAM. THE NATURAL FREQUENCY OF OTHER COMPONENT PARTS WAS COMPUTED USING A STATIC ANALYSIS OF DEFLECTION.

A STRESS ANALYSIS OF THE TEST RACK WAS PERFORMED USING THE MAXIMUM VALUE OF GROUND ACCELERATION FROM THE RESPECTIVE FULL SCALE SHOCK SPECTRUM CURVES FOR THE MINIMUM NATURAL FREQUENCY OF 23 CYCLES PER SECOND. STRESSES WERE COMPUTED BY USING THE GT STRUDL STIFFNESS ANALYSIS FOR SEISMIC LOADS AND GROUND ACCELERATION LOADS FOR EACH OF THE TWO LOADING CASES. THE MAXIMUM STRESS EXPERIENCED BY ANY TEST RACK MEMBER IS LESS THAN THE YIELD STRESS VALUE FOR A SAFE SHUTDOWN EARTHQUAKE. THEREFORE THE TEST RACK IS CAPABLE OF SAFELY TRANSFERRING THE GROUND ACCELERATION FREQUENCIES TO THE BATTERIES

DESIGNED BY: Ishwar J. Patel

CHECKED BY: Ralph C. Dumack

ISHVAR J. PATEL, M.S.  
PROJECT STRUCTURAL ENGINEER  
RALPH C. DUMACK, P.E. & ASSOC.  
STRUCTURAL ENGINEERS

RALPH C. DUMACK, P.E.  
PRESIDENT  
RALPH C. DUMACK, P.E. & ASSOC.  
STRUCTURAL ENGINEERS



SECTION #6  
RACK DRAWINGS  
AND  
CALCULATIONS

INTERMEDIATE BATTERY CONNECTIONS TO BE SHOWN  
BY OTHERS. LOGS ONLY BY GOULD INC.

ASSEMBLY (1) RACK ALL CONNECTIONS TO BE TIGHTENED WITH  
A TORQUE FORCE OF 25-30 FT LBS.  
NOTES  
2) BATT. A ALL POST CONNECTIONS TO BE MADE WITH  
5/16-18 x 2 7/8 LG S.S. HEX. HD. BOLT, NUT & WASHERS

B) SEE SHEET #3 FOR DETAIL OF TERMINAL  
BUSHING CONNECTIONS.  
C) ABOVE CONNECTIONS TO BE TIGHTENED WITH A  
TORQUE FORCE OF 12-13 FT LBS. (100 IN LBS.)

APPROXIMATE WEIGHTS

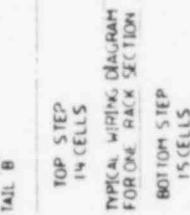
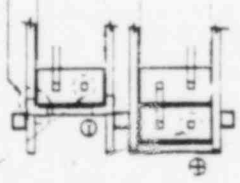
1 INCH-1000 CELL	261
1507-074520-B 6 RACK	1015
TERMINAL ASSEMBLY BATT	
& RACK (INCL. CONN.)	172

THIS DRAWING APPLIES WITH S&I  
SPEC. NO. E/L-2092

PUBLIC SERVICE CO. OF INDIANA  
PLAINFIELD, IND.  
BARGENT & LUNDY ENGINEERS  
CHICAGO, ILL.  
MARBLE HILL NUCLEAR GENERATING STATION  
NEW WASHINGTON, IND.  
CLASS 1E EQUIPMENT  
S&I PROJECT #A808/4923  
SPEC #7-2819  
UNIT #1, P.O. #1197-88-G CM #1  
ITEM #1, GOULD ORDER #807518  
UNIT #2, P.O. #2078-88-G CM #1  
ITEM #1, GOULD ORDER #807522

POS. & NEG. POLARITIES USE BUSHING & S.S. TYPE TERM. CONN.  
(FULL DETAIL SEE SHEET #2)

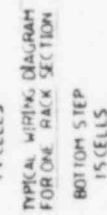
SEE DETAIL B



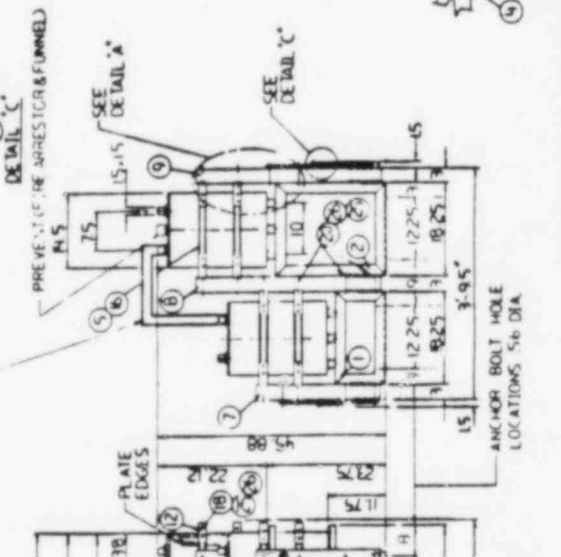
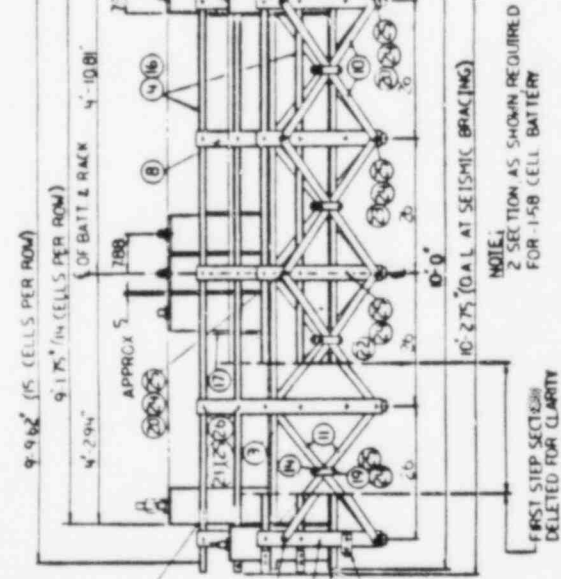
INTERCELL CONN. COP. LP  
1/25 x 25, 101-081945-004  
2 PER CONNECTION



12" STEP CONN. COP. LP 1/25 x 25  
101-081945-004  
2 PER CONNECTION



GOULD SUPPLY ITEM PARTIALS  
RACK 11-10000E  
RACK 12-10000E  
RACK 13-10000E  
RACK 14-10000E



NOTE:  
2 SECTION AS SHOWN REQUIRED  
FOR 1-18 CELL BATTERY

FIRST STEP SECTION  
DELETED FOR CLARITY

THIS DRAWING TO SUPERSEDE  
DWG. # 0628130

EQUIPMENT IDENTICAL TO COMMONWEALTH EDISON CO.  
BYRON & BRAIDWOOD STATIONS  
DWG. 084459C & 084459D

NOTE:  
SHEET 2 OF 2

GOULD

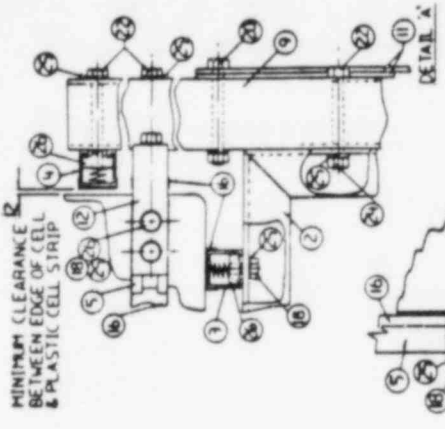
LAYOUT FOR SHELL NO. 1000 BATT.  
ON 2,507-074520-B 6 TWO STEP RACK

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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MATERIAL REQ. FOR 1507-074520-B 6

QTY	DESCRIPTION	PART NUMBER
5	STEEL SUPPORT 25 x 25 x 3/8	505-074520-006
5	STEEL STRINGER 1/2 x 1/2 x 12 GA	505-074520-007
5	STEEL UPRIGHT TUBING 3/4 x 25	505-074520-008
5	STEEL BRACE 1/2 x 3/8	505-074520-009
5	STEEL CORNER FITTING	505-074520-010
5	STEEL BRACING END	505-074520-011
5	PLASTIC CELL STRIP 9 LG	505-074520-012
5	PLASTIC CELL SPACER	505-074520-013
5	1/2-13 HEX HD BOLT	505-074520-014
5	1/2-13 LOCK WASHER	505-074520-015
5	1/2-13 CLAMPING NUT	505-074520-016
5	1/2-13 SPACING WASHER	505-074520-017
5	1/2-13 HEX NUT	505-074520-018
5	1/2-13 SPACING WASHER	505-074520-019
5	1/2-13 CLAMPING NUT	505-074520-020
5	1/2-13 SPACING WASHER	505-074520-021

MINIMUM CLEARANCE  
BETWEEN EDGE OF CELL  
& PLASTIC CELL STRIP



GROUNDING ASS'Y \*\*  
1.32-08792B002  
CONSISTS OF:  
1.07-CB602 (1/2-500 PM) GRD LUG  
1.01-08792300 COP LP ADAPT PLT  
NECESSARY HARDWARE  
(TWO REQUIRED PER RACK)  
(ONE AT EACH DIA. -OMAL. CHNL. 9)

SEISMIC CALCULATIONS  
FOR  
SARGENT & LUNDY, CHICAGO  
MARBLE HILL NUCLEAR GENERATING STATION  
UNIT 1 & 2

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Drawing No. 064745-C & 064745-D

## SECTION I. ABSTRACT

In these calculations the natural frequencies of the rack are computed in all principal directions, i.e., vertical transverse-horizontal and longitudinal-horizontal. Then the stresses in all members and connections of the rack structure are determined.

The calculations are based on a static analysis of deflections of the component parts of the rack. The deflection of the individual components are then added arithmetically to find the deflection of the assembly in either horizontal and vertical direction. One section of the rack is used, as the other sections are geometric repetitions. The sum of deflections of the individual components is always greater than the whole deflection of the entire structure. This is thus a conservative approach and hence the frequency calculated by this method results in a value on the safer side and adds to the safety of the rack design.

Based on the deflection calculations, an equivalent system with single degree of freedom is defined and the natural frequency is computed.

The frequency in all principal directions, vertical, transverse-horizontal and longitudinal-horizontal, is above 33 cycles per second.

The acceleration factors for seismic design are based on the Response Spectra given in Sargent and Lundy Specification Y-2819 for Marble Hill Nuclear Generating Station Unit 1 & 2.

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## SECTION II. DATA AND ASSUMPTIONS

### 1. Rack Components

The complete equipment consists of two (2) identical racks each containing 29 cells. Within each rack the cells are installed in two parallel steps, each step containing 15 cells and 14 cells respectively.

The battery arrangement is shown in Gould Drawing 064745-D titled:  
Layout for 58 cell NCX-1200 Battery on 2-S07-074520-846 Rack  
(Heavy Seismic Res.)

The approximate weights are listed on the Drawing as follows:

NCX-1200 Cell	261#
(1)-Rack S07-074520-846	1,015#
58 Cells NCX-1200 Battery	
& 2 Racks	17,275#

The weight of the 58 cells itself is  $58 \times 261 = 15,138\#$  equal to 87 percent of the total assembled rack and cell weight.

Each step of each rack consists of 4 repetitive units 26 inches long aligned in a row with out to out length of 10 ft.

The structural components of a unit are:

- Stringers: Made of Unistrut Section P-1000 roll formed of #12 gauge strip steel. In addition to the support stringers on which the cells are mounted, there are side- and end-stringers.
- Support Frames: Angles  $2\frac{1}{2}" \times 2\frac{1}{2}" \times \frac{3}{8}"$  welded with mitred joints, to form a four-sided rectangular frame.
- Uprights: Structural tubing TS  $3 \times 3 \times 0.25$
- Bracing: Flat Bar  $1\frac{1}{2}" \times \frac{3}{8}"$

The properties of these sections are tabulated below:

PROPERTIES OF SECTIONS								
Designation And Size	Weight Lbs/Ft	Area in <sup>2</sup>	X - X AXIS			Y - Y AXIS		
			I in <sup>4</sup>	S in <sup>3</sup>	r in	I in <sup>4</sup>	S in <sup>3</sup>	r in
Unistrut P-1000 1.625 X 1.625 X 0.105	1.90	.555	.186	.203	.579	.239	.294	.655
Angle 2 1/2 X 2 1/2 X 3/8"	5.90	1.73	.984	.566	.753	Same as X - X axis		
Structural Tube 3 X 3 X 0.25	8.80	2.59	3.16	2.10	1.10	Same as X- X axis		
Flat Bar 1.5 X 0.375	1.91	0.56	0.29	0.39	0.43	0.007	0.035	0.108

## 2. Seismic Acceleration

The rack is designed for Safe Shutdown Earthquake S.S.E. and Operating Basis Earthquake O.B.E. S.S.E. is the earthquake which produces the vibratory ground motion for which the battery must be able to perform its safety function. This earthquake is expected to be the largest which could rationally occur at the site during the life of the plant.

O.B.E. is the earthquake which produces the vibratory ground motion for which the battery must be able to perform its intended function.

TABLE OF ACCELERATION FACTORS			
	Horizontal Component		Vertical
	N-S	E-W	
O.B.E.	1.35	1.75	3.20
Reference: Specification Y-2819 Sheet No.	37	38	28
S.S.E.	1.90	2.40	4.50
Reference: Specification Y-2819 Sheet No.	39	40	29

In the design the maximum accelerations are used:

O.B.E.	Horizontal	1.75	Vertical	3.20
S.S.E.	Horizontal	2.40	Vertical	4.50

## 3. Natural Frequency

Based on the deflection calculations, an equivalent system with single degree of freedom is substituted in accordance with the method presented by M.I.T. Professor John M. Biggs in his book "Structural Dynamics" Chapter 5, pages 206 to 211. In this method transformation factors are used which convert the structural elements into the equivalent single degree system.

$\omega = \sqrt{g/y M_f}$  , where  $\omega$  (Omega) is the frequency in radians per second

$g$  = gravity acceleration = 386 lbs/sec<sup>2</sup>

$y$  = Sum of deflections

$M_f$  = Mass Transformation Factor from the Tables on Pages 209 and 210 of Biggs "Structural Dynamics"



### 3. Natural Frequency (cont'd.)

The value  $M_f$  depends on the loading and support conditions. For uniform loading in the elastic strain range.

$M_f$  = 0.50 for simply supported beams,  
 $M_f$  = 0.41 for fixed end beams

The value  $\omega$  is then divided by  $2 \pi = 6.28$  to obtain the natural frequency "f" in cycles per second (cps).

The natural frequencies of the rack are computed in vertical, transverse-horizontal and longitudinal-horizontal directions and in all cases this frequency is above 33 cps.

### 4. Codes & Allowable Stresses

The structural steel conforms to ASTM A-36. The allowable stresses for O.B.E. conditions are based on the ASCI Specification for rolled shapes and the Unistrut data for the cold formed members.

For S.S.E. conditions, a stress increase of 33% is permitted. The allowable stresses for bending ( $F_b$ ) tension ( $F_t$ ) and shear ( $F_v$ ) are listed in table below in kips per square inch.

Allowable Stresses KSI	OBE			SSE		
Steel Member	$F_b$	$F_t$	$F_v$	$F_b$	$F_t$	$F_v$
Cold Formed Unistrut	25.0	22.0	15.0	33.0	30.0	20.00
Rolled Shapes	24.0	22.0	15.0	32.0	30.0	20.00
Bolts A-307		20.00	15.00		25.6	20.00

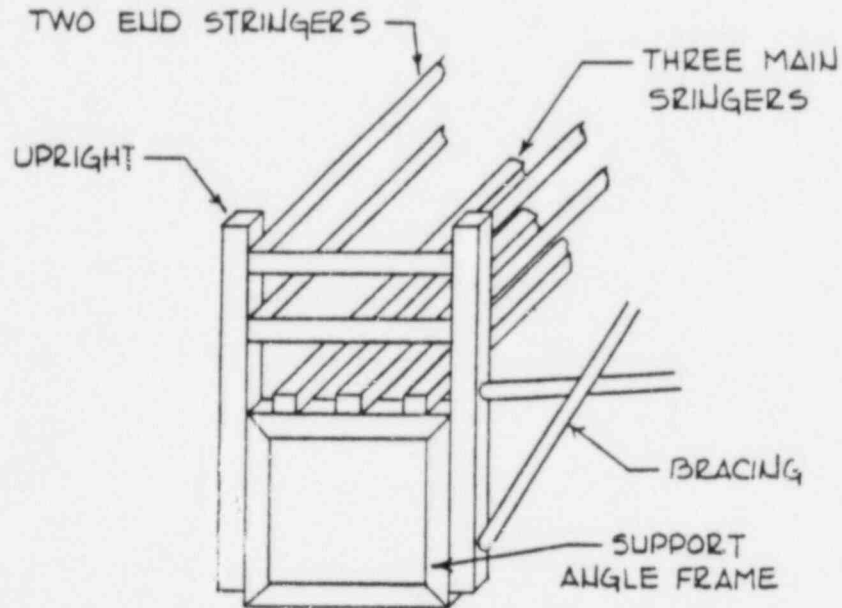
The design fabrication and installation of the rack conforms to the following codes and standards:

- IEEE Standard 484-1975-Recommended Practice for Installation and Design of Large Lead Storage Batteries for Generating Stations and Substations
- IEEE 344-1975-Recommended Practices for Seismic Qualification of Class IE Equipment and Nuclear Power Generating Stations
- ANSI C1-1978 National Electrical Code Article No. 480
- AISC.-Specification for the Design, Fabrication and Erection of Structural Steel
- AISI-American Iron and Steel Institute--Specification for the Design of Cold-Formed Steel Structural Members

### SECTION III. DEFLECTION ANALYSIS

#### 1. Principal Directions

The deflections in three directions are analyzed. The vertical deflection is the sum of the deflections of the main stringers and the top angle of the support frame.



#### STRUCTURAL MEMBERS OF RACK UNIT

The displacement in transverse horizontal direction is the sum of the deflections of the side stringers and the upright.

The displacement in longitudinal horizontal direction is the sum of the deflections of the end stringers, the upright and the bracing.

The weight of the unit consists of the weight of the cells and the weight of the supporting framing, accessories and hardware. There are 58 cells in two racks each consisting of 8 units. Thus the number of cells per unit is:  
 $n = 58/2 \times 8 = 3.625$ . The weight of cells per unit is  $3.625 \times 261 = 946.13$  lbs. The weight of the framing and accessories is 1015 lbs. per 8-unit rack, thus  $1015/8 = 126.87$  lbs. Total weight of rack unit is  $126.87 + 946.13 = \underline{1073 \text{ lbs.}}$

## 2. Vertical Deflection

### 2a. Main Stringer

The cells are placed on stringers spanning 26" between angle frames. There are 3 lines of stringers in each 4-span row. Thus each stringer is a continuous four-equal span beam uniformly loaded with the weight of cells.

There are 15 cells in one row

$$\text{Weight } 15 \times 261 = 3,915\#$$

Own weight of 3 stringers 10 ft. long,  
 $3 \times 10 \times 1.9 =$

Total

$$\begin{array}{r} 57\# \\ 3,972\# \end{array}$$

There are 4 units in a row

Load each unit  $3,972/4 =$

$$993\#$$

Uniform load  $w = 993/26 =$

$$38.19 \frac{\text{lbs.}}{\text{in.}}$$

Use uniform load 38 lbs/in

The deflection of a continuous beam is the result of a downward deflection  $y$ , as for a simply supported beam and upward deflections  $y_2$  and  $y_3$  from the support moments. The support moments are computed on next page by the Moment Distribution Method.

First interior support  $M = 2,651 \text{ in-lbs}$

Center interior support  $M = 1,886 \text{ in-lbs}$

The upward deflection is

$$y = \frac{3Ml^2}{48EI} \text{ where } l = 26" \quad E = 29 \times 10^6 \text{ psi}$$

$$I = 3 \times 0.186 = 0.558 \text{ in}^4$$

Call  $l^2/EI = k$

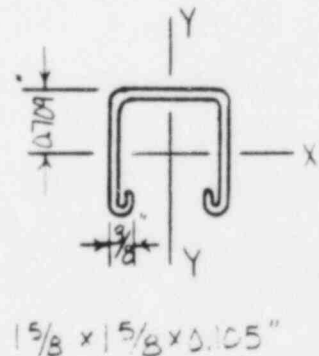
$$k = \frac{26^2}{(29 \times 10^6 \times 0.558)} = \frac{676}{(16.18 \times 10^6)} = 41.78 \times 10^{-6}$$

$$y = \frac{3Mk}{48} = \frac{Mk}{16} = M \times 41.78 \times 10^{-6} / 16 = 2.61M \times 10^{-6}$$

The deflection of a simply supported beam is

$$y_1 = (5/384) \times w l^4 / EI$$

$$y_1 = (1/76.8) \times 38 \times 26^2 \times k = 38 \times 676 \times 41.78 \times 10^{-6} / 76.8 = 13,973 \times 10^{-6}$$



# MAIN STRINGER - MOMENT DISTRIBUTION

Uniform load on 3 stringers

4-span continuous beam, each span

Fixed End Moment: F.E.M. =  $wl^2/12 = 38 \times 26^2/12 =$

Distribution Factor at intermediate support  
at end support

$w = 38 \text{ lbs/in}$

$l = 26''$

2141 in-lbs

0.5

1.0

Free span moment  $M_0 = wl^2/8 = 38 \times 26^2/8 =$

3211 in-lbs

Free shear  $V = wl/2 = 38 \times 26/2 =$

494 lbs

Symmetry  
← Axis

F.E.M. 1st Distrib.	+2141 -2141	-2141 0	+2141 0	-2141 0	+2141 0
Carry Over 2-nd Distrib.	0 0	-1020 +510	0 +510	0 0	0 0
Carry Over 3-d Distrib.	+255 -255	0 0	0 0	+255 0	-255 0
Final Moment	0	-2651	+2651	-1886	+1886
Free Span Mom 1/2 ( $M_1 + M_2$ ) Net Span Mom	+3211 -1325 +1886		+3211 -2268 +943		
Free Shear ( $M_1 - M_2$ )/l Final Shear	494 -102 392	494 +102 596	494 +29 523	494 -29 523	494 -29 523
Reaction	392	1119		1046	

+943

Summary:

$wl = 988 \text{ lbs;}$

$wl^2 = 25,688 \text{ in-lbs}$

Neg. Mom. First Int. Support	$M = 0.104$	$wl^2 =$	-2651 in-lbs
Neg. Mom. Center Int. Support	$M = 0.073$	$wl^2 =$	-1886 in-lbs
Posit. Mom. End-Span	$M = 0.073$	$wl^2 =$	+1886 in-lbs
Posit. Mom. Int. Span	$M = 0.037$	$wl^2 =$	+943 in-lbs
End Reaction	$R = 0.395$	$wl =$	392 lbs
Reaction First Int. Support	$R = 1.137$	$wl =$	1119 lbs
Reaction Center Support	$R = 0.937$	$wl =$	1046 lbs

2a. Main Stringer Deflection (cont'd.)

End Span  $M_L = 0$   $M_R = 2651$  in-lbs

Upward Deflection from  $M_R$

$$y_3 = 26.10 M \times 10^{-6} = 2651 \times 2.61 \times 10^{-6} = 6893 \times 10^{-6}$$

$$y = y_1 - y_3 = (13,973 - 6,893) \times 10^{-6} = 7,080 \times 10^{-6}$$

Interior Span - Upward Deflection

$$y_2 + y_3 = 2.61 \times 10^{-6} (2651 + 1886) \\ = 4537 \times 2.61 \times 10^{-6} = 11,842 \times 10^{-6}$$

$$y = y_1 - (y_2 + y_3) = (13,973 - 11,842) \times 10^{-6} = 2131 \times 10^{-6}$$

The End-Span deflection governs. Use for all spans of the main stringers  
 $y = 7080 \times 10^{-6}$  (on the safer side).

This is approximately half the deflection of a simply supported beam  
 $7080 \times 10^{-6} / 13,973 \times 10^{-6} = 0.5$ .



Downward Def. of Simple Beam.



Upward Def. Due to Support Moment.

2b. Angle Frame

The top angle is welded to the vertical members of the box frame and is treated as a rigidly supported beam with 3 point loads, symmetrically placed. The maximum deflection occurs at the center of span and is caused by the center load and the two loads placed 5' on either side of the center.

The load on the top angle is the reaction from the stringers. Due to continuity of the stringers, the maximum reaction is

$$R = 1.137 w_l$$

$$R = 1119 \text{ lbs (See Moment Distribution for Main Stringers).}$$

$$\text{Add weight of angle } 5.9 \times 18.25 / 12 = 9.0 \text{ lbs}$$

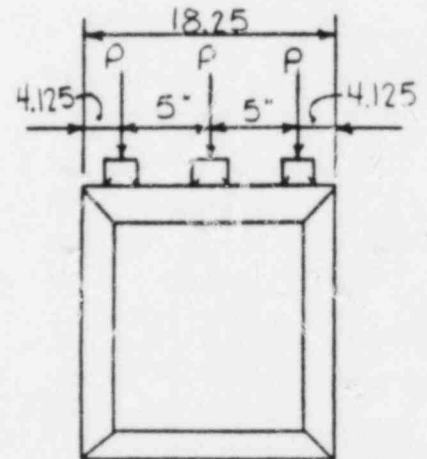
$$\text{Total load } 1119 + 9 = 1128 \quad P = 1128 / 3 = 376 \text{ Use } 376 \text{ lbs.}$$

From "Koark-Formulas for Stress and Strain" the deflection at point x from load  $P_1$  located at distance a from support is:

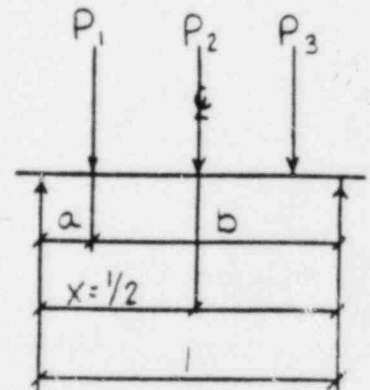
$$y = 1/6 [P_1 a^2 (1-x)^2] / (EI l^3) \times [(3b+a)(1-x) - 3bl]$$

$$\text{Here } P_1 = 376 \text{ lbs. } l = 18.25 \text{ in. } x = 9.125 \text{ in.}$$

$$a = 4.125 \text{ in } b = 14.125 \text{ in } E = 29 \times 10^6 \text{ psi; } I = 0.984 \text{ in}^4$$



ANGLES  $2\frac{1}{2} \times 2\frac{1}{2} \times \frac{3}{8}$ "



2b. Angle Frame (cont'd.)

Substitute  $x = 1/2$  The formula simplifies to:

$$y_1 = 1/48 (P_1 a^2)(a-3b)/EI$$

$$y_1 = 1/48 (376 \times 4.125^2)(4.125 - 3 \times 14.125)/(29 \times 10^6 \times 0.984)$$

$$y_1 = (7.83 \times 17.01) \times (-38.25)/(28.54 \times 10^6) = -178.7 \times 10^{-6}$$

Deflection at midspan from center load  $P_2$

$$y_2 = -1/192 (P_2 l^3)/EI = -1/192 (376 \times 18.25^3)/(29 \times 10^6 \times 0.984)$$

$$y_2 = -11,903.5/(28.54 \times 10^6) = -417.1 \times 10^{-6}$$

Deflection for load  $P_3$  is  $y_3 = -y_1 = -178.7 \times 10^{-6}$

$$\text{Total deflection } y = y_1 + y_2 + y_3 = -(178.7 + 417.1 + 178.7) \times 10^{-6}$$

$$y = -774.5 \times 10^{-6}$$

2c. Natural Frequency - Vertical Direction

The sum of stringer deflection and angle deflection is:

$$y = (7080 + 774.5) \times 10^{-6} = 7854.5 \times 10^{-6} \text{ Use } 7855 \times 10^{-6}$$

The circular frequency is:

$$\omega = \sqrt{g/(yM_f)} \text{ where } g = 386 \text{ } y = 7,855 \times 10^{-6} \text{ } M_f \text{ is } 0.41$$

$$\omega = \sqrt{386/(7,855 \times 10^{-6} \times 0.41)} = \sqrt{119,855} = 346.2 \text{ radians/sec}$$

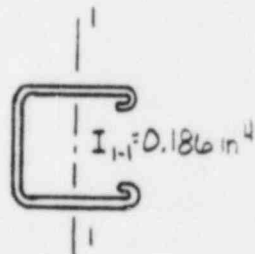
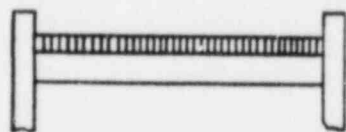
$$f = \omega / 2 \pi = 346.2/6.28 = 55.12 > 33 \text{ O.K.}$$

The rack is rigid in vertical direction.

3. Transverse Horizontal Deflection

3a. Side Stringers

The load is same as on the 3 main stringers,  $w = 38 \text{ lbs/in.}$  The two side stringers are continuous beams and the moment distribution is same as for the main stringers. The maximum deflection occurs in the end span of the continuous beam and is half of the deflection for a simply supported beam, (see item 2a. of this Section III).



15 3/4 x 1 1/2 x 0.075 - TYP

$$y = 0.5 \times (5/384) \times w l^4 / (EI) \text{ where } w = 38 \text{ lbs/in, } l = 26''$$

$$E = 29 \times 10^6 \text{ psi, } I = 2 \times 0.186 = 0.372$$

$$y = 0.5 \times (5/384) \times (38 \times 26^4) / (29 \times 10^6 \times 0.372) = 10,480 \times 10^{-6} \text{ in}$$



3b. Upright

The upright is treated as a cantilever beam with two concentrated loads  $P$  at distance 7" and 14" above the point of support of the cantilever. The load  $R$  is the reaction from the side stringer. Due to continuity, the max. reaction is:

$$R = 1.137 w l, \text{ (See item 2a - Moment Distribution). } w = 38 \text{ lbs/in, } l = 26''$$

$$R = 1.137 \times 38 \times 26 = 1119 \text{ lbs.}$$

$$P = R/2 = 559.5 \text{ lbs.}$$

The deflection of a cantilever from point load at distance  $b$  from base of cantilever is  $y = Pb^2(3l-b)/6EI$  where  $P = 559.5 \text{ lbs, } l = 15.5''$ ;  
 $E = 29 \times 10^6, I = 3.16 \text{ in}^4$

The dimension  $b_2 = 14''$  for upper load and  $b_1 = 7''$  for lower load.

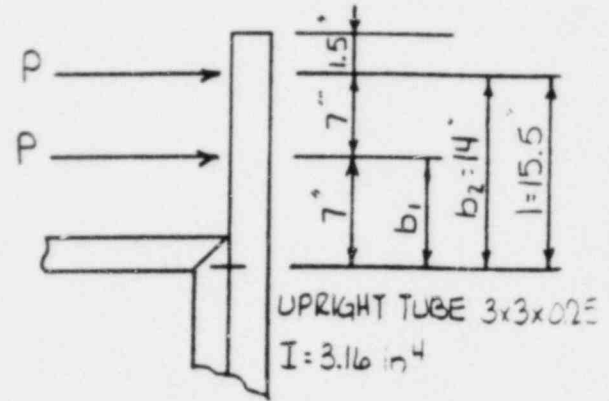
$$y = 559.5 b^2 (3 \times 15.5 - b) / (6 \times 29 \times 10^6 \times 3.16)$$

$$y = b^2 (46.5 - b) \times 1.01 \times 10^{-6}$$

$$\text{For } b_1 = 7'' \quad b^2 = 49, \quad 46.5 - 7 = 39.5, \quad b^2 (46.5 - b) = 1935.5$$

$$\text{For } b_2 = 14'' \quad b^2 = 196, \quad 46.5 - 14 = 32.5, \quad b^2 (46.5 - b) = 6370.0$$

$$y = (1935.5 + 6370) \times 1.01 \times 10^{-6} = 8389 \times 10^{-6}$$



3c. Natural Frequency in Transverse Horizontal Direction

The sum of side stringer and upright deflections, is:

$$y = (10,480 + 8,389) \times 10^{-6} = 18,869 \times 10^{-6} \text{ in}$$

$$\omega = \sqrt{g/(y \times M_f)} \quad \text{where } \omega \text{ is the frequency in radians per sec}$$

$$g = 386, \quad y = 18,869 \times 10^{-6}, \quad M_f \text{ is Transformation Factor}$$

$$\text{From Biggs Table } M_f = 0.41$$

$$\omega = \sqrt{386 / (18,869 \times 10^{-6} \times 0.41)} = \sqrt{49,895} = 223.37 \text{ radians/sec}$$

$$f = \omega / 2\pi = 223.37 / 6.28 = 35.6 \text{ cps} > 33$$

The rack system is rigid in transverse horiz. direction.

#### 4. Longitudinal Horizontal Direction

##### 4a. End Stringers

The end stringer is treated as a simply supported beam. Each unit of the rack carries 3.625 cells at 261 lbs.

Load:  $3.625 \times 261 = 946.13$

Own weight of 2 stringers:

$$2 \times 1.9 \times 24.25/12 = 7.67$$

Total 953.80 lbs.

per lin. inch  $w = 953.8/24.25 = 39.33$   
say 39.5 lbs/in

Deflection  $y = 5/384 \times w l^4 / EI$  where

$$w = 39.5 \text{ lbs/in}; l = 24.25 \text{ in}; E = 29 \times 10^6 \text{ psi}; I = 2 \times 0.186 = 0.372 \text{ in}^4$$

$$y = 5/384 \times (39.5 \times 24.25^4) / (29 \times 10^6 \times 0.372) =$$

$$1/76.8 \times (471,027) / (0.372 \times 10^6) = 16,487 \times 10^{-6}$$

##### 4b. Upright

Upright is treated as a cantilever with projection above point of support

$l = 15.5''$  and with concentrated loads

$P$  at distance  $b_1 = 7''$ ,  $b_2 = 14''$ .

The load  $P$  is the reaction from end

stringer  $2P = 1/2 \times (39.5 \times 24.25) = 478.94$

$P = 1/2 (478.94) = 239.47$  Say 239.5 lbs

$$y = Pb^2 (3l-b) / (6 EI) \text{ where } P = 239.5 \text{ lbs},$$

$$b_1 = 7'' \quad b_2 = 14'' \quad l = 15.5''$$

$$E = 29 \times 10^6 \text{ psi}, \quad I = 3.16 \text{ in}^4$$

$$y = 239.5 \times b^2 (3 \times 15.5 - b) / (6 \times 29 \times 10^6 \times 3.16)$$

$$y = 0.435 \times b^2 \times (46.5 - b)$$

$$\text{For } b_1 = 7'' \quad b^2 = 49, \quad 46.5 - 7 = 39.5 \quad b^2 (46.5 - b) = 1935.5$$

$$\text{For } b_2 = 14'' \quad b^2 = 196, \quad 46.5 - 14 = 32.5 \quad b^2 (46.5 - b) = 6370.0$$

$$y = (6370 + 1935.5) \times (0.435 \times 10^{-6}) = 3618 \times 10^{-6}$$

##### 4c. Bracing

The total weight of a rack unit is  $P = 1073 \text{ lbs}$   
(see item 1 of Section III.)

$$\tan \theta = 18/26 = 0.692; \quad \theta = 34.7^\circ$$

$$\sin \theta = 0.569 \quad \cos \theta = 0.822$$

$$2l = 26 / \cos \theta = 26 / 0.82 = 31.6''$$

Braces are connected at point of intersection

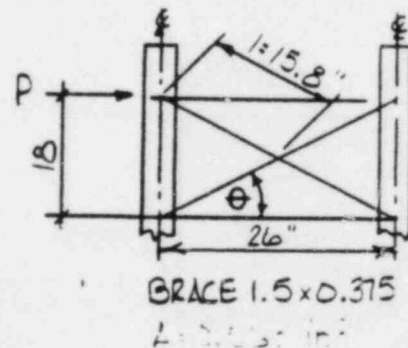
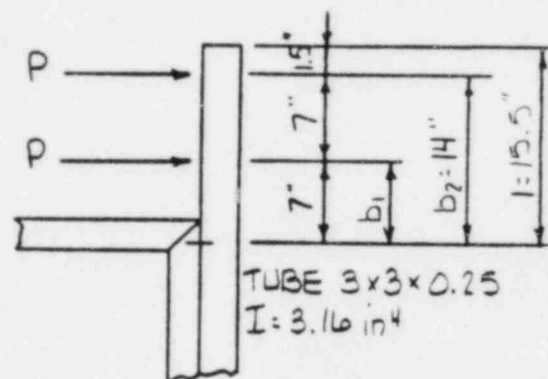
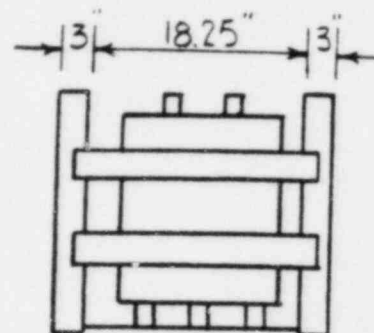
$$l = 31.6/2 = 15.8''$$

$$\text{Axial load in brace } N = P / \cos \theta = 1073 / 0.822 = 1305 \text{ lbs}$$

$$\text{Deflection } y = (N/A) l^3 / E \text{ where } N = 1305 \text{ lbs}, \quad A = 0.563 \text{ in}^2$$

$$l = 15.8'' \quad E = 29 \times 10^6$$

$$y = (1305 / 0.563) \times (15.8 / 29 \times 10^6) = 1263.0 \times 10^{-6}$$



#### 4d. Natural Frequency - Longitudinal Horizontal Direction

The total displacement in longitudinal horizontal direction is the sum of the deflections of end stringers, upright and bracing.

$$y = (16,487 + 3618 + 1263) \times 10^{-6} = 21,368 \times 10^{-6}$$

Circular frequency:

$$\omega = \sqrt{g/y M_f}, \text{ where } g = 386, y = 21,368 \times 10^{-6}$$

$M_f$  = Transformation Factor from Bigg's Table

$M_f$  = 0.41

$$\omega = \sqrt{386/21,368 \times 10^{-6} \times 0.41} = \sqrt{44,060} = 209.9$$

$$f = \omega / 2\pi = 209.9/6.28 = 33.4 \text{ c.p.s.} > 33$$

The rack is rigid in longitudinal horizontal direction.

#### 5. Summary of Deflections and Frequencies

The total deflections  $y$  (in) and the natural frequencies  $f$  (cps) in principal directions are as follows:

Vertical	$y = 7,885 \times 10^{-6}$	$f = 55.12 \text{ cps}$
Transverse Horizontal	$y = 18,869 \times 10^{-6}$	$f = 35.6 \text{ cps}$
Longitudinal Horizontal	$y = 21,368 \times 10^{-6}$	$f = 33.4 \text{ cps}$

Frequencies in all principal directions are above 33 cps. The rack is rigid, so that any amplification of load to the cells is negligible.

## SECTION IV. STRESS ANALYSIS

### 1. General

The stress analysis is done for O.B.E. and S.S.E. forces. The acceleration values are: (see Item 2 of Section II).

O.B.E.	Horizontal	1.75	Vertical	3.20
S.S.E.	Horizontal	2.40	Vertical	4.50

For determining the stresses multiply gravity loads

For O.B.E.	Horizontal by 1.75	Vertical by 4.20
S.S.E.	Horizontal by 2.40	Vertical by 5.50

All components of the rack are analyzed separately to demonstrate that the stress in each component is below the allowable stresses, as listed in item 3 of Section II.

### 2. Main Stringer

The stringer is a four-span continuous beam with the maximum moment (see Moment Distribution item 2a of Section III) is  $M = 1/3 \times 2651 = 883.67$  in-lbs. The Section Modulus of the Unistrut stringer is  $S_x = 0.203$  in<sup>3</sup>. For vertical acceleration  $s_v = M/S = 883.67/0.203 = 4353$  psi  
 $s_y = 0.294$  in<sup>3</sup> For horizontal acceleration  $s_H = \frac{883.67}{0.294} = 3006$  psi

The stresses under seismic loads are found by multiplying the stress so by acceleration factors. For vertical loads the multiplying factor is  $1 + a$  where  $a$  is the vertical seismic acceleration.

As per Unistrut Tables, strength for cold formed member for bending is 25 ksi. This is to be used for O.B.E. For S.S.E. stress increase of 33% = 33 ksi.

Stress Under Gravity Loads $s_v = 4353$ psi $s_H = 3006$ psi	Stress in Main Stringer Under Seismic Forces					
	O.B.E.			S.S.E.		
	Vertical	Horizontal	Combined	Vertical	Horizontal	Combined
Multiply by Acceleration Factor	4.20	1.75		5.50	2.40	
Stress $s$ psi	18,283	5,261	23,543	23,942	7,214	31,156
Allowable $F$ psi			25,000			33,000
Safety Factor $F/s$			1.06			1.05

The main stringers are safe under gravity and seismic loads.

### 3. Angle Frame

The top angle is treated as a rigidly supported beam with 3 equal concentrated loads symmetrically placed. The load is the maximum reaction from the continuous main stringers.

$P = 376 \text{ lbs}$   
(See item 2b of Section III)

The top angle is welded to the vertical members of the angle frame so that rigidity at support is provided.

From Roark Formulas:

Moment at center of span from load at distance  $a$  from support.

$$M = -P ab^2/l^2 + R_1 l/2 - P (l/2 - a)$$

$$R_1 = P b/l \text{ where } P = 376 \text{ lbs } b = 14.125" \quad l = 18.25"$$

$$R_1 = 376 \times 14.125 / 18.25 = 291 \text{ lbs}$$

$$M_1 = -376 \times 4.125 \times 14.125^2 / 18.25^2 + 291 \times 18.25 / 2 - 376 (18.25 / 2 - 4.125)$$

$$M_1 = -929.1 + 2655.5 - 1880 = -153.6 \text{ in-lbs}$$

Moment at midspan under center load

$$M_2 = Pl/8 = 376 \times 18.25 / 8 = 857.75 \text{ in-lbs}$$

Total moment at midspan

$$M = M_2 + M_1 + M_3 \text{ where } M_3 = M_1$$

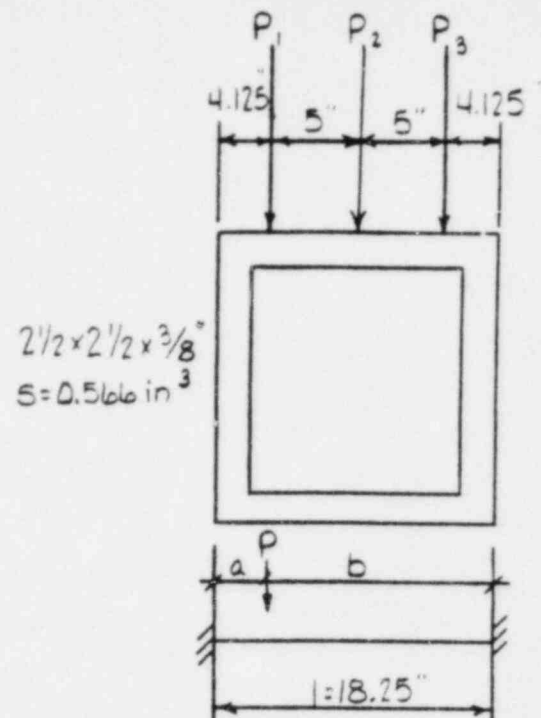
$$M = 857.75 - 153.6 - 153.6 = 550.55 \text{ in-lbs}$$

Moment at support

From $P_1$	$M_1 = P ab^2/l^2 = 376 \times 4.125 \times 14.125^2 / 18.25^2$	=	929.10 in-lbs
From $P_3$	$M_3 = P a^2/b/l^2 = 376 \times 4.125^2 \times 14.125 / 18.25^2$	=	271.30 in-lbs
From $P_2$	$M_2 = Pl/8 = 376 \times 18.25 / 8$	=	857.80 in-lbs
	Total		2058.20 in-lbs

Maximum Moment is at support

$$M = 2058.2 \text{ in-lbs} \quad S = 0.566 \text{ in}^3 \quad s = M/S = 2058.2 / 0.566 = 3636 \text{ psi}$$



### 3. Angle Frame (cont'd.)

Stress in Top Angle Under Gravity Loads so = 3636 psi	Stress in Top Angle Under Seismic Forces					
	O.B.E.			S.S.E.		
	Vertical	Horizontal	Combined	Vertical	Horizontal	Combined
Multiply by Acceleration Factor	✓ 4.20	1.75	-	✓ 5.50	✓ 2.40	-
Stress s psi	15,271	6,363	21,634	19,998	8,726	28,724
Allowable F psi			23,760			32,400
Safety Factor F/s			1.10			1.13

The angle frame is safe under gravity and seismic loads.

### 4. Side Stringer

There is no direct vertical load on the side stringers. The only force acting is the horizontal seismic force.

The design moment on the pair of side stringers is the same as on the set of three main stringers. Max. moment for the 4-span continuous beam is  $M = 2651$  ft lbs. The Section modulus of the two side stringers is  $S = 2 \times 0.203 = 0.406$  in<sup>3</sup>

$$so = M/S = 2651/0.406 = 6530 \text{ psi}$$

O.B.E. Multiply stress so by 1.75  
 $s = 1.75 \times 6530 = 11,428 \text{ psi}$  Safe Factor  $23,760/11,428 = 2.1$

S.S.E. Multiply stress so by 2.40  
 $s = 2.40 \times 6530 = 15,672 \text{ psi}$  Safe Factor  $32,400/15,672 = 2.1$

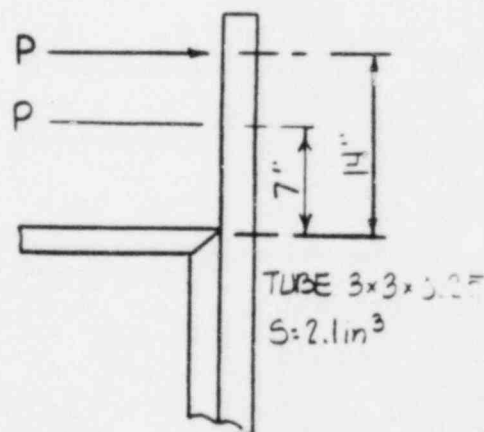
The side stringers are safe under O.B.E. and S.S.E. forces.

### 5. Intermediate Upright

There is no direct vertical load. The load consists only of horizontal seismic forces. The upright is treated as a cantilever with concentrated loads located at distance 7" and 14" above the point of support. The loads P are the reactions from the side stringers.  $P = 559.5$  lbs (see item 3b of Section II).

$$M = 559.5 \times (7" + 14") = 11,750 \text{ in-lbs}$$

$$so = M/S = 11,750/2.1 = 5,595 \text{ psi}$$





5. Intermediate Upright (cont'd.)

O.B.E. Multiply so by 1.75  $s = 5595 \times 1.75 = 9791$  psi  
Safety factor  $23,760/9791 = 2.42$

S.S.E. Multiply so by 2.40  $s = 2.4 \times 5595 = 13,428$  psi  
Safety factor  $32,400/13,428 = 2.41$

The uprights are safe under O.B.E. and S.S.E. forces.

6. End Stringers

There is no direct vertical load on the end stringers. The only force acting is the horizontal seismic load. The stringer is treated as a simply supported beam, see item 4a of Section III.

$l = 24.25''$  Section Modulus of two end stringers  $S = 2 \times 0.203 = 0.406 \text{ in}^3$

O.B.E.  $W = 39.5 \times 1.75 = 69.13 \text{ lbs/in}$   $M = (1/8) \times (69.13 \times 24.25^2) = 5081 \text{ in-lbs}$   
 $s = 5081/0.406 = 12,515 \text{ psi}$   
Safety Factor  $23,760/12,515 = 1.90$

S.S.E.  $W = 39.5 \times 2.4 = 94.8 \text{ lbs/in}$   $M = (1/8) \times (94.8 \times 24.25^2) = 6969 \text{ in-lbs}$   
 $s = 6969/0.406 = 17,164 \text{ psi}$   
Safety Factor  $32,400/17,164 = 1.88$

The end stringers are safe under O.B.E. and S.S.E. forces.

7. Corner Upright

There is no direct vertical load. The only force acting is the horizontal seismic load. The upright is treated as a cantilever with concentrated equal loads at distance 7" and 14" from point of support. See item 4b of Section III. The reaction from end stringer is  $P_o = 239.5 \text{ lbs}$ . The upright is a tube  $3 \times 3 \times 0.25$

Section Modulus  $S = 2.10 \text{ in}^3$

O.B.E.  $P = 239.5 \times 1.75 = 419.13 \text{ lbs}$   
 $M = 419.3 \times (7 + 14) = 8801.6 \text{ in-lbs}$   
 $s = 8801.6/2.1 = 4191 \text{ psi}$   
Safety Factor  $23,760/4191 = 5.66$

S.S.E.  $P = 2.4 \times 239.5 = 574.8$   
 $M = 574.8 \times (7 + 14) = 12,070.8 \text{ in-lbs}$   
 $s = 12,070.8/2.1 = 5748 \text{ psi}$   
Safety Factor  $32,400/5748 = 5.63$

Note: This upright at the corner of the rack supports also half a span of side stringers. The stress in the upright due to side stringers is half the stress computed in item 5 of this Section IV.

O.B.E.  $s = 0.5 \times 9791 = 4895.5 \text{ psi}$

S.S.E.  $s = 0.5 \times 13,428 = 6714 \text{ psi}$

The combined stress is the geometrical resultant of the stresses from end and side stringers.

O.B.E.  $s^2 = 4191^2 + 4895.5^2 = 17,564,481 + 23,965,920 = 41,530,401$   
 $s = 6444 \text{ psi}$  Safety Factor  $23,760/6444 = 3.68$

S.S.E.  $s^2 = 5748^2 + 6714^2 = 33,039,504 + 45,077,796 = 78,117,300$   
 $s = 8838 \text{ psi}$  Safety Factor  $32,400/8838 = 3.66$

The corner uprights are safe under O.B.E. and S.S.E. forces.

## 8. Brace

The load in the brace is the resultant of the horizontal component  $R_1$  and vertical component  $R_2$ .

$$\begin{aligned} N &= R_1 \cos \Theta + R_2 \sin \Theta \\ \tan \Theta &= 18/26 = 0.692, \quad \Theta = 34.7^\circ \\ \sin \Theta &= 0.569, \quad \cos \Theta = 0.822 \\ 21 &= 26/\cos \Theta = 26/0.822 = 31.6'' \\ l &= 15.8 \quad l_1 = l - c = 15.5 - \frac{1.5}{0.822} = 14'' \end{aligned}$$

The weight of the rack unit is 1073 lbs, see item 4c of Section III.

O.B.E.	Horizontal	$R_1 = 1.75 \times 1073 = 1877.75 \text{ lbs}$
	Vertical	$R_2 = 4.20 \times 1073 = 4506.6 \text{ lbs}$

$$\begin{aligned} N &= R_1 \cos \Theta + R_2 \sin \Theta = 1877.75 \times 0.822 + 4506.6 \times 0.569 = \\ &1543.5 + 2564.3 = 4107.8 \text{ lbs} \\ s &= N/A = 4107.8/0.563 = 7296 \text{ psi} \end{aligned}$$

Check for buckling. Radius of gyration  $r = 0.108$

Slenderness  $l/r = 14/0.108 = 130$

From A.I.S.C. Manual, allowable  $F_a = 8,840 \text{ psi}$

Safety Factor  $F_a/s = 8,840/7,296 = 1.21$

S.S.E.	Horizontal	$R_1 = 2.40 \times 1073 = 2757.2 \text{ lbs}$
	Vertical	$R_2 = 5.50 \times 1073 = 5901.5 \text{ lbs}$

$$\begin{aligned} N &= R_1 \cos \Theta + R_2 \sin \Theta = 2757.2 \times 0.822 + 5901.5 \times 0.569 = \\ &2266 + 3358 = 5624 \text{ lbs} \\ s &= N/A = 5624/0.563 = 9,989 \text{ psi} \end{aligned}$$

Slenderness  $l/r = 14/0.108 = 130$

From A.I.S.C. Manual,  $F_a = 8,840 \text{ psi}$  Multiply by ratio  $0.90/0.66 = 1.36$

$F = 8840 \times 1.36 = 12,022 \text{ psi}$

Safety Factor  $F/s = 12,022/9,989 = 1.20$

The braces are safe under gravity and seismic loads.

## 9. Connections

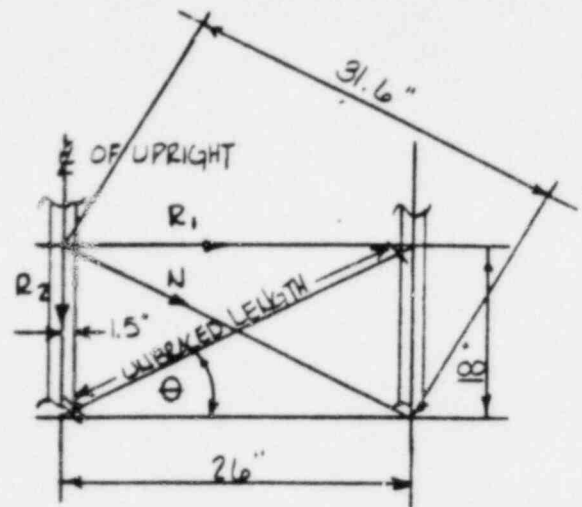
### 9a. Anchor Bolts

The bottom angle of the box-frame is anchored to the floor. Maximum load on the anchor bolts is caused by the horizontal acceleration force acting at top of frame.

Total weight of 8-unit rack with 29 cells, see Section II.

$$P = 29 \times 261 + 1015 = 8,584 \text{ lbs}$$

$$M = P \times h = 8584 \times 22 = 188,848 \text{ in-lbs}$$



$$\begin{aligned} B &= 1.5 \times 0.375 \\ A &= 0.563 \text{ in}^2 \end{aligned}$$

9a. Anchor Bolts (cont'd.)

There are 4 rows of 1/2" diameter anchor bolts, 5 bolts per row.

Resisting moment, considering exterior rows only is:

$$M = T \times l = T \times 33.5$$

$$T = M/L = 188,848/33.5 = 5637 \text{ lbs}$$

Cross sectional area of 5 bolts

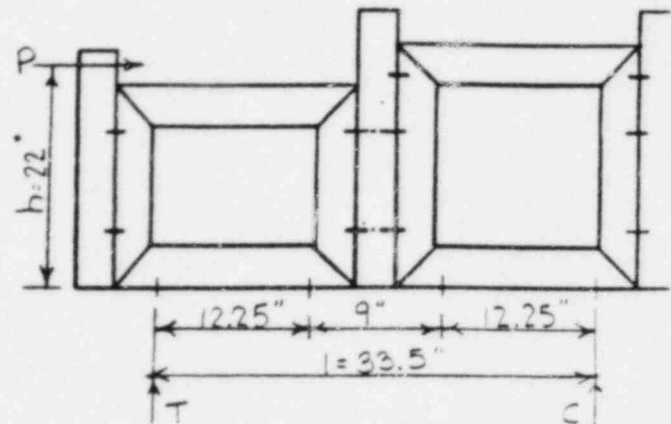
$$A = 5 \times 0.16 = 0.8 \text{ in}^2$$

$$s_o = T/A = 5637/0.8 = 7047 \text{ psi}$$

O.B.E. Horizontal Multiply stress  $s_o$  by 1.75  
 $s = 1.75 \times 7047 = 12,332 \text{ psi}$   
 Safety Factor  $20,000/12,332 = 1.62$

S.S.E. Horizontal Multiply stress  $s_o$  by 2.40  
 $s = 2.4 \times 7047 = 16,913 \text{ psi}$   
 Allowable:  $F_a = (0.90/0.66) \times 20,000 = 27,270 \text{ psi}$   
 Safety Factor  $27,270/16,913 = 1.61$

The anchor bolts are safe under O.B.E. and S.S.E. forces.



9b. Upright Connection

The reaction from side stringers is 1119 lbs, see item 5 of this Section.

O.B.E.  $P = 1119 \times 1.75 = 1958.3 \text{ lbs}$   $s = P/A = 1958.3/0.16 = 12,239 \text{ psi}$   
 Safety Factor  $20,000/12,239 = 1.63$

S.S.E.  $P = 2.4 \times 1119 = 2685.6 \text{ lbs}$   $s = 2685.6/0.16 = 16,785 \text{ psi}$   
 Safety Factor  $27,270/16,785 = 1.62$

The connection is safe under O.B.E. and S.S.E. forces.

9c. Stringer Connection

Maximum reaction from (3) main stringers is 1119 lbs. See item 2a of Section III. Cross section area of 3 bolts  $A = 3 \times 0.16 = 0.48 \text{ in}^2$

O.B.E. Horizontal  $P = 1119 \times 1.75 = 1958.3 \text{ lbs}$   
 $s = P/A = 1958.3/0.48 = 4079 \text{ psi}$   
 Safety Factor  $20,000/4079 = 4.9$

S.S.E. Horizontal  $P = 1119 \times 2.4 = 2685.6 \text{ lbs}$   
 $s = P/A = 2685.6/0.48 = 5,595 \text{ psi}$   
 Safety Factor  $27,270/5595 = 4.86$

The connection of stringer to angle frame is safe under O.B.E. and S.S.E. forces.

10. Table of Stresses

TABLE OF STRESSES		
	O.B.E.	S.S.E.
<u>Allowable Stress</u> psi		
a. Structural Members	23,760 ✓	32,400
b. Bolts	20,000 ✓	27,270
c. Unistrut Stringer (Main)	25,000 ✓	33,000
<u>Maximum Computed Stress</u> psi		
-Main Stringer	23,543	31,156
-Angle Frame	21,634	28,724
-Side Stringer	11,428	15,672
-End Stringer	5,081	6,969
-Intermediate Upright	9,791	13,428
-Corner Upright	6,430	8,838
-Brace	7,296	9,055
-Anchor Bolt	12,332	16,913
-Upright Connection Bolt	12,239	16,785
-Stringer Connection Bolt	4,079	5,595

### CONCLUSIONS

Based upon the analysis shown on the foregoing pages, the data adequately shows that the battery and rack will meet the requirements of the specification and will perform adequately during and after the seismic event.

The equipment should be considered rigid as the design frequency of the rack is higher than 33 c.p.s.

CHECKED BY:

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SECTION #7

TEST SERIES SUMMARY

WYLE TEST REPORT NO. 44681-2

1.0 SERIES 1 AND 2

The original testing procedure as defined in Gould Test Procedure GB-3454 up through Revision #10 dated November 14, 1980 was designed to attempt generic qualification of Gould planté and calcium type batteries for Class 1E application in nuclear power generating stations by using accelerated heat aging of the test specimens.

Test Series #1 and #2 disclosed that the accelerated heat aging resulted in degradation of cell components other than the positive and negative plates. The degree of degradation of certain components was in excess of that which would occur in actual service. Other components experienced degradation that would not have occurred during actual service.

This component degradation adversely affected the discharge capacity performance as well as the capability to satisfy the seismic requirements.

Accelerated heat aging was originally employed to evaluate the electrochemical performance and, specifically, the positive plate deterioration and its effect on discharge capacity delivery. No seismic requirement was involved.

Based on the test results, it was concluded that accelerated heat aging was not a viable method to properly evaluate the complete lead-acid cell system with respect to discharge capacity and seismic requirement capability.



## 1.0 SERIES 1 AND 2 (Cont'd.)

The decision was made to discontinue testing using accelerated heat aging, but to continue the test program using naturally aged cells from actual in service customer installations. (Reference IEEE 535-1979 Section 8 Item 8.2 Aging Procedure).

## 2.0 SERIES 3 (Wyle Test Report 44681-1 Appendix VIII)

This test series involves testing of Gould plante type cells which is not pertinent to the report on Gould calcium type cells and will not be addressed.

## 3.0 SERIES 4

### 3.1 Specimen #1

Specimen was comprised of 3 cells #'s 1, 2, 3 Type NCX-1680 naturally aged (10 years) from New Jersey Bell Telephone Co., Bordentown, N. J.

### 3.2 Specimen #2

Specimen was comprised of 3 cells #'s 3B1, 3B2, 3B3. Accelerated heat aged (10 years equivalency).

### 3.3 Specimen #3

Specimen was comprised of 3 cells #'s 12, 13, 14 Type MCX-510 naturally aged (8 years) from Enterprise Telephone Co., Intercourse, Penna.

NOTE: Specimens #1, 2, and 3 were run concurrently, resulting in a common test response spectra (TRS).

Specimen #1 pertains only to the qualification of N-line cells.

Specimen #2 was a repeat test of an additional group of cells for comparison purposes with a previously tested group and is not presented for qualification.

Specimen #3 pertains only to the qualification of M-line cells.

### 3.4 Pre-Seismic Capacity Discharge Test

All 3 specimen groups were given a pre-seismic capacity test per Gould Test Procedure GB-3454 Revision 11. Section 2.0 Pre-Seismic Capacity Test - IEEE 450-1980. - IEEE 535-1979. Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	1	107
1	NCX-1680	2	106
1	NCX-1680	3	96
2	NCX-1200	3B1	92
2	NCX-1200	3B2	93
2	NCX-1200	3B3	93
3	MCX-510	12	115
3	MCX-510	13	109
3	MCX-510	14	109

\*Corrected For Temperature

Documentation of above test results contained in Wyle Report #44681-2 Vol. #1 Section 11 Table II-I - Discharge Data Pages II-8, II-9 and Data Sheet II-69, II-72 and II-73 for N-line cells and Data Sheet II-70 for M-line cells.

#### 4.0 SEISMIC TESTING SERIES #4

##### 4.1 Seismic Qualification

See Section III Pages III-1 through III-27

NOTE: Test Series #4 is presented as the complete qualification of M-line cells. Proceeding Test Series #5 and #6 will complete the qualification of N-line cells.

##### 4.2 Test Run Descriptions

Page III - 16 Table III - III

##### 4.3 Required Response Spectrum

Appendix III

Page III - 20 Fig #1

Page III - 21 Fig #2

Page III - 22 Fig #3

Page III - 23 Fig #4

##### 4.4 Photographs

Refer to Pages III-41 through III-49. Photographs III-12 through III-20.

##### 4.5 Transmissibility Plots Appendix V

Test #1 Front-To-Back/Vertical Orientation

Pages III-115 through III-126

Test #9 Side-To-Side/Vertical Orientation

Pages III-127 through III-138

4.0 SEISMIC TESTING SERIES #4 (Cont'd.)

4.6 Test Response Spectrum Plots Appendix VI

Run #7 Front-To-Back/Vertical - OBE

Pages III-305 through III-314

Run #8 Front-To-Back/Vertical - SSE

Pages III-315 through III-314

Run #14 Side-To-Side/Vertical - OBE

Pages III-337 through III-346

Run #15 Side-To-Side/Vertical - SSE

Pages III-347 through III-368

4.7 Instrumentation Log Sheets and Instrumentation Equipment Sheets Appendix VII

Pages III-493 through III-500

5.0 TEST PROCEDURES AND RESULTS SECTION 3.0 3.3.1 RANDOM MULTIFREQUENCY TEST

RESULTS TEST SERIES #4 COMMENTS FOR N-LINE CELLS

5.1 Battery Rack Cross Braces - Specimen 1

See Anomaly 14 Page III-11.

Also Photographs III-13 Page III-42 and Photograph III-14 Page III-43.

Deformation of cross braces which resulted was due to use of improper 3/16" thick cross braces as shipped from factory. Proper thickness (3/8") braces were substituted and testing resumed.

5.2 Cell Container Cracking - Specimen 2

See Anomaly 14 Page III-11.

As addressed in 3.3 of this report, specimen #2 was a repeat test of an additional group of accelerated heat aged cells for comparison purposes with previously tested groups and is not part of this report.

5.3 Cell Container Cracking - Specimen 1

See Anomaly 14 Page III-11.

The crack in the bottom of one cell (#3) container resulted in minimal loss of electrolyte. Specimen was removed from test table and transported to another test site.

5.4 Post Seismic Capacity Discharge Test

Test cells were recharged to replace ampere hours withdrawn during seismic testing (110% return). Post seismic capacity discharge test per IEEE 450-1980 IEEE 535-1979 was conducted on Specimen 1. At start of test, 16 hours had elapsed from time container (#3 cell) cracked until start of capacity discharge test. Electrolyte level at start of test was down 3/4" (low level line). At end of 3 hour test, electrolyte level was down 1" (plates still covered).

#### 5.4 Post Seismic Capacity Discharge Test (Cont'd.)

Capacity test results were as follows:

<u>Specimen</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	1	101
1	NCX-1680	2	104
1	NCX-1680	3	92
*Corrected For Temperature			
2	NCX-1200	3B1	----**
2	NCX-1200	3B2	19
2	NCX-1200	3B3	28

For documentation of above data, see Wyle Report Vol. II Table IV-I Pages IV-6 and IV-7. Also, data sheets Pages IV-41, IV-42 and IV-43.

\*\*Note: Cell container cracked during seismic test releasing all electrolyte thus preventing capacity discharge evaluation.

Cell #3 with cracked container in Specimen 1 group delivered 92% capacity. Even though this capacity percentage satisfied IEEE 535 - 8.3.1.4 acceptance criteria, Gould did not classify this as acceptable.

#### 5.5 Cell Container Cracking Cause

Examination of cell #3 container disclosed a crack in the bottom across the cell width dimension about 1" in from the bottom-jar juncture. The cell bottom upon returning to contact with the rack rails following a vertical excursion during the test, did not return flush or perpendicular to the rack rails. Therefore, severe concentrated stresses resulted.



#### 5.5 Cell Container Cracking Cause (Cont'd.)

Examination of the 1/2" thick urethane (open cell) spacer material used between cells disclosed considerable permanent compression. The compression was wedge shape, starting about 1/3 the way down from the top and extending to the bottom where the thickness was about 1/8". Both spacers between the three cells were in similar condition.

This compression occurred during the side-to-side/vertical orientation operating basis earthquake tests.

Due to the spacer permanent compression, uniform spacing between cells was not maintained during testing thus permitting undue independent pendulous type movement.

#### 5.6 Conclusions

Evaluations of conditions outlined in 5.3 and 5.4 were made and a decision made to repeat the testing of another 3 cell group of naturally aged NCX-1680 cells. In addition, the cell spacer material was changed to a foamed polyethylene (closed cell) material.

#### RESULTS TEST SERIES #4 COMMENTS FOR M-LINE CELLS

#### 5.7 Seismic Testing

Specimen #3 possessed sufficient integrity to withstand, without compromise of structures, the prescribed simulated seismic environment. (See 3.3.1 - Random Multifrequency Test Results.)

#### 5.8 Electrical Monitoring Results

It was demonstrated that the specimen possessed sufficient integrity to withstand, without compromise of electrical function, the prescribed simulated seismic environment.

#### 5.9 Post Seismic Capacity Discharge Test

Test cells were recharged to replace the ampere hours withdrawn during seismic testing (110% return). Post seismic capacity discharge test was conducted per Gould Test Procedure GB-3454 Revision 11 Section 5.0 Post-Seismic Capacity Test - IEEE 450-1980 - IEEE 535-1979.

Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity</u>
3	MCX-510	12	111
3	MCX-510	13	107
3	MCX-510	14	107

Specimen #3 (MCX-510) cells #12, 13, and 14 satisfied IEEE 535 8.3.1.4.

#### 5.10 Conclusions

Refer to Sections 3.4 Pre-Seismic Capacity Discharge Test; Section 5.1 Seismic Testing and Section 5.3 Post Seismic Capacity Discharge Test.

This demonstrates that Specimen #3 (MCX-510) naturally aged cells 8 years old are capable of performing before, during, and after a seismic event.

## 6.0 TEST SERIES #5

### 6.1 Specimen #1

Specimen was comprised of 3 cells #'s 4, 5, 6 Type NCX-1680 naturally aged (10 years) from New Jersey Bell Telephone Co., Bordentown, N.J.

### 6.2 Specimen #2

Specimen was comprised of another 3 cell group of accelerated heat aged cells (10 years equivalency). Cell #'s 4B1, 4B2, 4B3

Note: 6.1 - Only Specimen #1 is pertinent to this report.

6.2 Specimen #2 was a repeat test for comparison purposes with previously tested groups and is not part of this report.

### 6.3 Pre-Seismic Capacity Discharge Test

Each specimen group was given a pre-seismic capacity test per Gould Test Procedure GB-3454 Revision 11-B dated January 28, 1982 Section 2.0 Pre-Seismic Capacity Test - IEEE 450-1980 - IEEE 535-1979. Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	4	96
1	NCX-1680	5	96
1	NCX-1680	6	98
2	NCX-1200	4B1	88
2	NCX-1200	4B2	86
2	NCX-1200	4B3	85

\*Corrected for Temperature

### 6.3 Pre-Seismic Capacity Discharge Test (Cont;d.)

Documentation of above test results contained in Wyle Report #44681-2 Vol. #1 Section II Table II-I - Discharge Data - Pages II-8, II-9 and Data Sheet II-69 II-72.

Note: The above specimens were tested concurrently with those specimens referenced Test Series #4.

## 7.0 SEISMIC TESTING SERIES #5

### 7.1 Photographs

Refer to Pages III-50, III-51, III-52. Photographs III-21, III-22, III-23.

### 7.2 Transmissibility Plots Appendix V

Test #1 Front-To-Back/Vertical Orientation

Pages III-140 through III-147

Test #8 Side-To-Side/Vertical Orientation

Pages III-148 through III-155

### 7.3 Test Response Spectrum Plots Appendix VI

Run #6 Side-To-Side/Vertical - OBE

Pages III-370 through III-379

Run #7 Side-To-Side/Vertical - SSE

Pages III-380 through III-397

Run #13 Front-To-Back/Vertical - OBE

Pages III-398 through III-407

Run #14 Front-To-Back/Vertical - SSE

Pages III-408 through III-425

7.4 Instrumentation Log Sheets and Instrumentation Equipment Sheets Appendix VII

Pages III-501 through III-508

7.5 Seismic Qualification

See Section III Pages III-1 through III-27

7.6 Test Procedure and Results Section 3.0

3.3.1 Random Multifrequency Test Results

7.7 Specimen Integrity

As indicated in 3.3.1, there were no exceptions with regard to demonstration of specimen integrity. (Test Series 5)

7.8 Post Seismic Capacity Discharge Test

Test cells were recharged to replace ampere hours withdrawn during seismic testing (110% return). Post seismic capacity discharge test per IEEE-450-1980 - IEEE-535-1979 was conducted on Specimen 1.

#### 7.8 Post Seismic Capacity Discharge Test (Cont'd.)

Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	4	92
1	NCX-1680	5	68
1	NCX-1680	6	69
*Corrected For Temperature			
2	NCX-1200	4B1	26
2	NCX-1200	4B2	45
2	NCX-1200	4B3	60

Cells #5 and #6 failed to deliver the minimum of 80% capacity, therefore, Specimen 1 group did not satisfy acceptance per IEEE 535-1979 Section 8.3.1.4.

For documentation of above data, see Wyle Report Vol. II Table IV-1 Pages IV-6 and IV-7. Also data sheets Page IV-45 and IV-46.

#### 7.9 Conclusions Test Series 5

Specimen 1 group (cells #4-5-6) failed qualification. Cells returned to Gould laboratory for evaluation of capacity discharge failure.

#### 7.10 Specimen 1 Failure Evaluation

Cells #4, 5, 6 of Specimen 1 were examined at the Gould laboratory and it was determined that cells #5 and #6 failed the post-seismic capacity test as a result of the excessive RRS, particularly at low frequencies (3 to 4 cycles) at 10g acceleration. It was decided to conduct another test at a reduced RRS which would still envelope the contract specifications.



## 8.0 TEST SERIES #6

### 8.1 Specimen 1

Specimen was comprised of 3 cells #7, 8, 9 Type NCX-1680 naturally aged (10 years) from New Jersey Bell Telephone Co., Bordertown, N.J.

### 8.2 Pre-Seismic Capacity Discharge Test

Specimen 1 was given a pre-seismic capacity discharge test per Gould Test Procedure GB-3454 Revision 11 Section 2.0 Pre-Seismic Capacity Test IEE-450-1980 - IEEE-535-1979.

Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	7	104
1	NCX-1680	8	116
1	NCX-1680	9	106

\*Corrected For Temperature

For documentation of above test results, see Wyle Report #44681-2 Vol. 1 Section II Table II-I Discharge Data - Pages II-8, II-9 and data sheets Pages II-75 and II-76.

## 9.0 SEISMIC TESTING SERIES #6

### 9.1 Photographs

See Pages III-53 and III-54. Photographs III-24 and III-25.

### 9.2 Transmissibility Plots Appendix V

Test #1 Front-To-Back/Vertical Orientation

Pages III-157 through III-160

Test #10 Side-To-Side/Vertical Orientation

Pages III-161 through III-164

9.3 Test Response Spectrum Plots Appendix VI

Run #8 Front-To-Back/Vertical - OBE

Pages III-427 through III-436

Run #9 Front-To-Back/Vertical - SSE

Pages III-437 through III-450

Run #15 Side-To-Side/Vertical - OBE

Pages III-451 through III-460

Run #16 Side-To-Side/Vertical - SSE

Pages III-461 through III-474

9.4 Instrumentation Log Sheets and Instrumentation Equipment Sheets Appendix VII

Pages III-509 through III-515

9.5 Seismic Qualification

See Section III Pages III-1 through III-27

9.6 Test Procedure and Results Section 3.0

3.3.1 Random Multifrequency Test Results

9.7 Specimen Integrity

As indicated in Section 3.0 Item 3.3.1, there were no exceptions with regards to demonstration of specimen integrity. (Test Series 6)

10.0 POST SEISMIC CAPACITY DISCHARGE TEST

Test cells were recharged to replace ampere hours withdrawn during seismic testing (110% return). Post seismic capacity discharge test per IEEE-450-1980 - IEEE-535-1979 was conducted on Specimen 1.

#### 10.0 POST SEISMIC CAPACITY DISCHARGE TEST (Cont'd.)

Capacity test results were as follows:

<u>Specimen #</u>	<u>Cell Type</u>	<u>Cell #</u>	<u>% Capacity*</u>
1	NCX-1680	7	102
1	NCX-1680	8	110
1	NCX-1680	9	103

\*Corrected For Temperature

For documentation of above test results, see Wyle Report #44681-2 Vol. I

Section II Table II-1 Discharge Data Pages II-8, II-9 and data sheet IV-48 and IV-49.

#### 11.0 CONCLUSIONS - TEST SERIES 6

Based on data presented in Sections Nos. 8.0, 9.0 and 10.0, it is demonstrated that the naturally aged (10 years) cells of NCX type are capable of performing before, during, and after a seismic event.

## SECTION #7

### ADDENDUM

#### Stress Levels

As indicated in comment on failure analysis of test series 5, the RRS was reduced approximately 40% at low frequencies (3-15 C.P.S.) along both horizontal and vertical axis during Test Series 6. Refer to Appendix III which is the original RRS used for Test Series 4 and 5. Appendix 4 portrays RRS for Test Series 6. The RRS for Test Series 6 are significantly above the RRS specified in the contract.

In conclusion, the stress levels were considerably reduced in Test Series 6 compared to Test Series 4 and 5.

#### Wyle Report Vol. I Table V Page-9

Cell No. 6 should have pre-seismic capacity of 98% as supported by test disclosure data sheet 11-69 dated 6-19-81. This was a typographical error by Wyle, and, secondly, this cell is not a part of qualification group.

#### How To Derive Capacity

The temperature correction was per I.E.E.E. 450-1975 (Table 1, Page 11) which corrects the capacity in terms of time and not current. This correction term was used to temperature correct capacity of each cell individually.

Example: Cell #6 of Page II-69

Starting temperature is 82°

The correction factor is 1.014

Time requirement at 82° is  $(1.014 \times 180 =) 182.5$  minutes

The capacity is  $(179 \div 182.5 =) 98.07\%$

Photograph Numbers (Refers to Page III-7, 44681-2)

This is a typographical error by Wyle. This again is not relevant to the qualification group. Photographs III-16 and III-17 apply, not VII-16 and VII-17.

Spacing Material

The material was changed to polyethelyne foam closed cell type Dow Chemical (Ethe Foam #220) on 3-12-82 E.C.O. #0017. This material has been standardized for all Class IE or non-IE applications. This material can be readily identified by compressing it with thumb and index finger. The original material (Urethane) when compressed, remains that way whereas the present ethefoam compresses and then returns to its original dimension after pressure release.

Temperature Margins

For every 15° rise in battery electrolyte temperature above 77°, (92°F), the qualified battery life would reduce by 50%. Any prevailing annual average battery electrolyte temperature either within or beyond will affect battery life in direct proportion.

Test No. M 6065

Per Cent Capacity to 1.75 VPC @ 77° F.

	Cell #1	Cell #2	Cell #3	Cell #4	Cell #5	Cell #6	Remarks
1	92.7	101.4	114.6	108.7	111.3	110.0	As received (12/9/65) 5-hour rate
2	<u>1 Mo. float</u> 111.1	113.4	123.9	117.4	121.2	119.5	(1/7/66) 5-hour rate
3	<u>2 Mo. float</u> 113.0	113.0	121.6	116.1	119.9	118.2	(3/11/66) 5-hour rate
4	<u>4 Mo. float</u> 110.7	111.4	118.0	114.5	116.1	115.4	(7/18/66) 5-hour rate
5	<u>8 Mo. float</u> 116.1	115.6	122.9	116.6	122.2	119.9	(3/17/67) 5-hour rate
6	<u>16 Mo. float</u>	Capacity test conducted to wrong cut-off voltage.					(7/17/68)
7	<u>32 Mo. float</u> 110.8	113.5	120.0	116.9	119.3	120.7	(4/1/71) 5-hour rate



TABLE V (Continued)

## TEST SUMMARY

CELL TYPE	NATURAL OR ARTIFICIAL AGING	CELL NO.	PRE-SEISMIC CAPACITY (PERCENT)	POST-SEISMIC CAPACITY (PERCENT)	ORIGIN OF CELLS
			1st DISCH./2nd DISCH.	1st DISCH./2nd DISCH.	
MCX-1680	10 Years - Nat.	1	107	101	New, June, 1971 from New Jersey Bell Telephone, Bordentown, New Jersey.
		2	106	104	
		3	96	92	
		4	96	92***	
		5	96	68	
		6	108 98	69	
MCX-510	8 Years - Nat.	12	115	111	New, June, 1973 from Enterprise Telephone, Intercourse, Pennsylvania.
		13	109	107	
		14	108	107	
MCX-1200	10 Years - Act. (1st Disch.)	3B1	84/92	*	New from Gould for test.
		3B2	86/93	19	
		3B3	86/93	28	
		4B1	88/99	26	
		4B2	86/92	45	
		4B3	85/92	60	
MCX-1680	10 Years - Nat.	7	104	102	New, June, 1971 from New Jersey Bell Telephone, Bordentown, New Jersey.
		8	116	110	
		9	106	103	

\*Damaged in Seismic - No discharge performed.

\*\*Stopped test when first cell did not reach 80% capacity.

\*\*\*Stopped test to prevent voltage reversal.

TABLE II-I. DISCHARGE DATA (POST-AGING) (Continued)

CELL TYPE	NATURAL OR ARTIFICIAL AGING	CELL NO.	INITIAL TEMPERATURE (°F)	INITIAL SPECIFIC GRAVITY	DISCHARGE RATE (AMPERES)	TIME TO REACH 1.75 VPC (MINUTES)	FINAL TEMPERATURE (°F)	FINAL SPECIFIC GRAVITY	CAPACITY (PERCENT)
FPS-25	10 yrs-Art.	17	78	1.222	256	179	79	1.139	108
		18	78	1.218		204*	80	1.135	122*
		21	78	1.226		204*	82	1.132	122*
FPR-23	16 yrs-Nat.	46	77	1.222	226	236	81	1.140	134
		47	77	1.220		226	81	1.137	128
		48	77	1.220		229	87	1.135	130
MCX-1680	10 yrs-Nat.	1	81	1.214	410	195	85	1.136	107
		2	81	1.211		192	85	1.133	106
		3	81	1.210		174	85	1.136	96
		4	80	1.210		175	87	1.134	96
		5	82	1.212		175	87	1.136	96
		6	82	1.214		179	87	1.136	108 98
MCX-510	8 yrs-Nat.	12	74	1.219	133	205	80	1.139	115
		13	74	1.215		194	81	1.134	109
		14	74	1.213		192	81	1.135	108
MCX-1200	10 yrs-Art. (1st Disch.)	3B1	82	1.210	312	154	83	1.124	84
		3B2	84	1.208		157	85	1.122	86
		3B3	82	1.210		157	84	1.125	86
		4B1	80	1.216		160	81	1.132	88
		4B2	82	1.210		157	86	1.130	86
		4B3	80	1.212		154	83	1.128	85
	(2nd Disch.)	3B1	80	1.208	312	167	83	1.107	92
		3B2	80	1.210		169	82	1.111	93
		3B3	80	1.214		168	83	1.112	93
		4B1	79	1.215	312	180	84	1.117	99
		4B2	79	1.214		167	83	1.116	92
		4B3	79	1.214		168	83	1.112	92
MCX-1680	10 yrs-Nat.	7	74	1.214	410	186	78	1.140	104
		8	74	1.213		206	80	1.132	116
		9	74	1.214		188	80	1.137	106

\*Test stopped to prevent voltage reversal.

## 3.0 TEST PROCEDURES AND RESULTS (Continued)

3.3.1 Random Multifrequency Test Results (Continued)Test Series 4 (Continued)

During Test Run 15, one (1) battery of Specimen 2 jammed against the horizontal restraint of the battery rack cracking the transparent battery jar, which allowed the electrolyte solution to leak out as documented in Notice of Anomaly 14 (Appendix I) and shown in Photograph III-15.

A post-test inspection (at the completion of Test Run 15) revealed a crack along the bottom of one of the batteries of Specimen 1, which allowed the electrolyte solution to leak out as documented in Notice of Anomaly 14 (Appendix I) and shown in Photographs VII-16- and-VII-17. III-16  
III-17

Descriptions of the test runs are contained in Appendix II.

TRS plots of the control accelerometers from a selected OBE test and the SSE test in each orientation for each test series at 0.5, 1, 2, 3, and 5% damping are contained in Appendix VI.

The Functional Test results are contained in Section IV.

3.4 Specimen Response Procedures

Four (4) uniaxial piezoelectric accelerometers were located on each specimen for each test series. The placement of the accelerometers was at the direction of the Gould Technical Representative. An FM tape recorder provided a record of each accelerometer response. The horizontal accelerometers were oriented in the front-to-back direction during the FB/V testing, and reoriented in the side-to-side direction during the SS/V testing. The number of specimen response accelerometers mounted on the specimens for each series are as follows:

<u>MOUNTED SERIES</u>	<u>NUMBER OF ACCELEROMETERS</u>	<u>PHOTOGRAPHS</u>
1	16	III-3 thru III-6
2	12	III-9 thru III-11
4	12	III-18 thru III-20
5	8	III-22 thru III-23
6	4	III-25

Revised 6/29/82

# DATA SHEET

Page No. IV-48  
Report No. 44681-2

Customer Gould  
Specimen Battery  
Part No. NCX-1680  
Spec. -  
Para. -  
S/N See Below  
GSI No

Amb. Temp. 72°F  
Photo No  
Test Med. Aia  
Specimen Temp. 46

## WYLE LABORATORIES

Job No. 44681-02  
Report No. 44681-2  
Start Date 8-14-81

Test Title Post-Seismic Discharge, 410 Amps (3hr. Rate)

TIME	CELL #7	CELL #8	CELL #9	TOTAL	REMARKS
	1214	1213	1214		Spec. G.A.P.
	76°F	76°F	76°F		Temp.
	34	35	36	37	Monitor Channel
8:35	2.067	2.064	2.063	6.103	O.C.V.
8:40	1.933	1.935	1.933	5.794	5 min. Elapsed Time
8:50	1.933	1.936	1.933	5.794	
9:05	1.924	1.927	1.925	5.770	30 min. Elapsed Time
9:20	1.916	1.920	1.917	5.746	
9:35	1.908	1.911	1.908	5.720	60 min. Elapsed Time
9:50	1.898	1.902	1.899	5.691	
10:05	1.887	1.891	1.888	5.660	90 min. Elapsed Time
10:20	1.875	1.879	1.876	5.623	
10:35	1.861	1.867	1.863	5.584	120 min. Elapsed Time
10:50	1.844	1.850	1.846	5.532	
11:05	1.824	1.833	1.827	5.476	150 min. Elapsed Time
11:20	1.797	1.812	1.800	5.401	
11:35	1.759	1.784	1.764	5.299	180 min. Elapsed Time
11:37	(1.749)	1.778	1.755	5.275	182 min. Elapsed Time
11:39	1.743	1.775	(1.749)	5.261	184 min. Elapsed Time
11:48	1.696	(1.749)	1.705	5.143	192 min. Elapsed Time

Specimen Failed —  
Specimen Passed —  
NOA Written —

Tested By James K. Pordue Date: 8-14-81  
Witness — Date: —  
Sheet No. 1 of 2  
Approved Robert W. Lincoln

WH-614A



## SECTION #8

### COMPARISON OF SEISMIC TEST TRS VS. RES

(NCX)

Seismic test was conducted on naturally aged cells as described in Gould Test Procedure GB-3454 Revision #11.

Gould response spectra at both O.B.E. and S.S.E. levels at 2% damping (Fig. 5, 6, 7, 8 - Page III-24-25-26-27 in Wyle Report) were enveloped during the seismic test at all frequencies.

Following comparison response spectra curves are provided at 2% damping:

- Fig. 8-1      Vertical operating basis earthquake front-to-back/vertical
- Fig. 8-2      Horizontal operating basis earthquake front-to-back/vertical
- Fig. 8-3      Vertical operating basis earthquake side-to-side/vertical
- Fig. 8-4      Horizontal operating basis earthquake side-to-side/vertical
- Fig. 8-5      Vertical safe shutdown earthquake front-to-back/vertical
- Fig. 8-6      Horizontal safe shutdown earthquake front-to-back/vertical
- Fig. 8-7      Vertical safe shutdown earthquake side-to-side/vertical
- Fig. 8-8      Horizontal safe shutdown earthquake side-to-side/vertical

These plots compare the Byron/Braidwood R.R.S. and Gould R.R.S. with T.R.S. T.R.S. envelopes Byron/Braidwood and Gould R.R.S. at all frequencies.

All the cells were capacity tested prior and after the seismic test as required by I.E.E.E. 535 documentation regarding the capacity test date as provided in Section SA of this report. All cells met the capacity requirement. The cells were also examined visually and no physical damage was observed.

The required transmissibility plots are provided in Wyle Test Report for Series 6 as follows:

Test #1            Front-to-back/vertical axis for both horizontal and vertical control accelerometers - Pages III-157 through III-160.

Test #10           Side-to-side/vertical axis for both horizontal and vertical control accelerometers - Pages III-161 through III-164.

These plots indicate that the fixture is a rigid one and amplification due its structure is insignificant.

Section 5 and Section 6 of this report analyze the test fixture and contract rack using static analysis approach to prove that both the test fixture and the contract racks are rigid structures.



Page No. III-434  
 Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

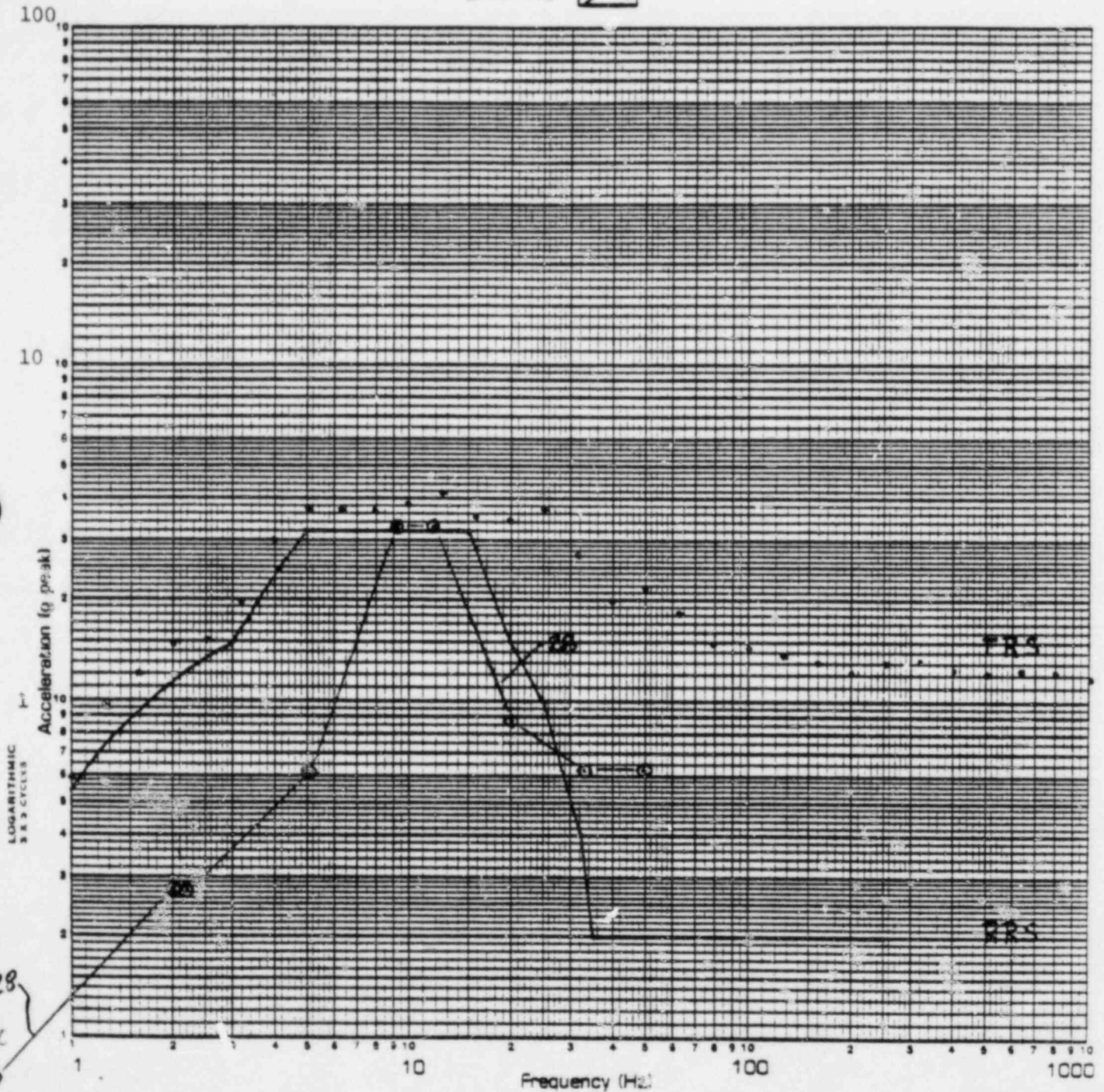
Legend

• TRS  
 © Byron/Braidwood RRS

Byron/Braidwood Station  
 Commonwealth Edison Co

1.0 □ 10 □ 100 2 1000 □

DAMPING 2%



SPECIMEN FR 1  
 AXIS FR 1

LOCATION NO. VCA  
 TEST RUN NO. 8

Fig. 8.1

Comparison of TRS vs RRS Vertical OBE  
 Front-to-Back/Vertical 2% Damping

Page No. III-429  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

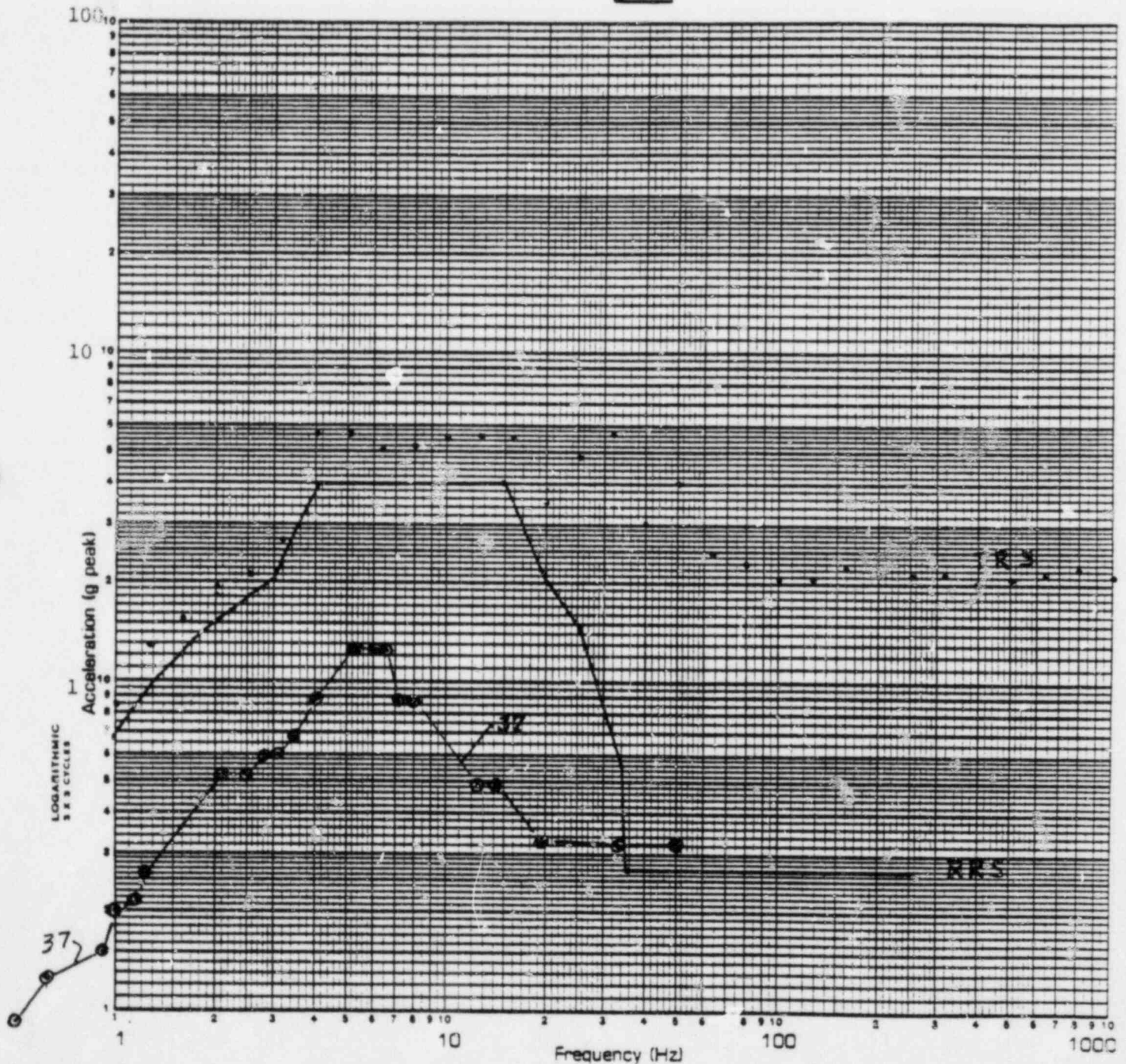
Legend

- TRS
- ⊙ Byron/Braidwood RRS

Byron/Braidwood Station  
Commonwealth Edison Co.

1.0 ☐ 10 ☐ 100 ☒ 1000 ☐

DAMPING ☒ 2%



SPECIMEN \_\_\_\_\_  
AXIS FB/V

LOCATION NO. HCA  
TEST RUN NO. 8

Fig. 8.2

Comparison of TRS vs RRS Horizontal OBE  
Front-to-Back/Vertical 2% Damping

Page No. III-458  
 Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

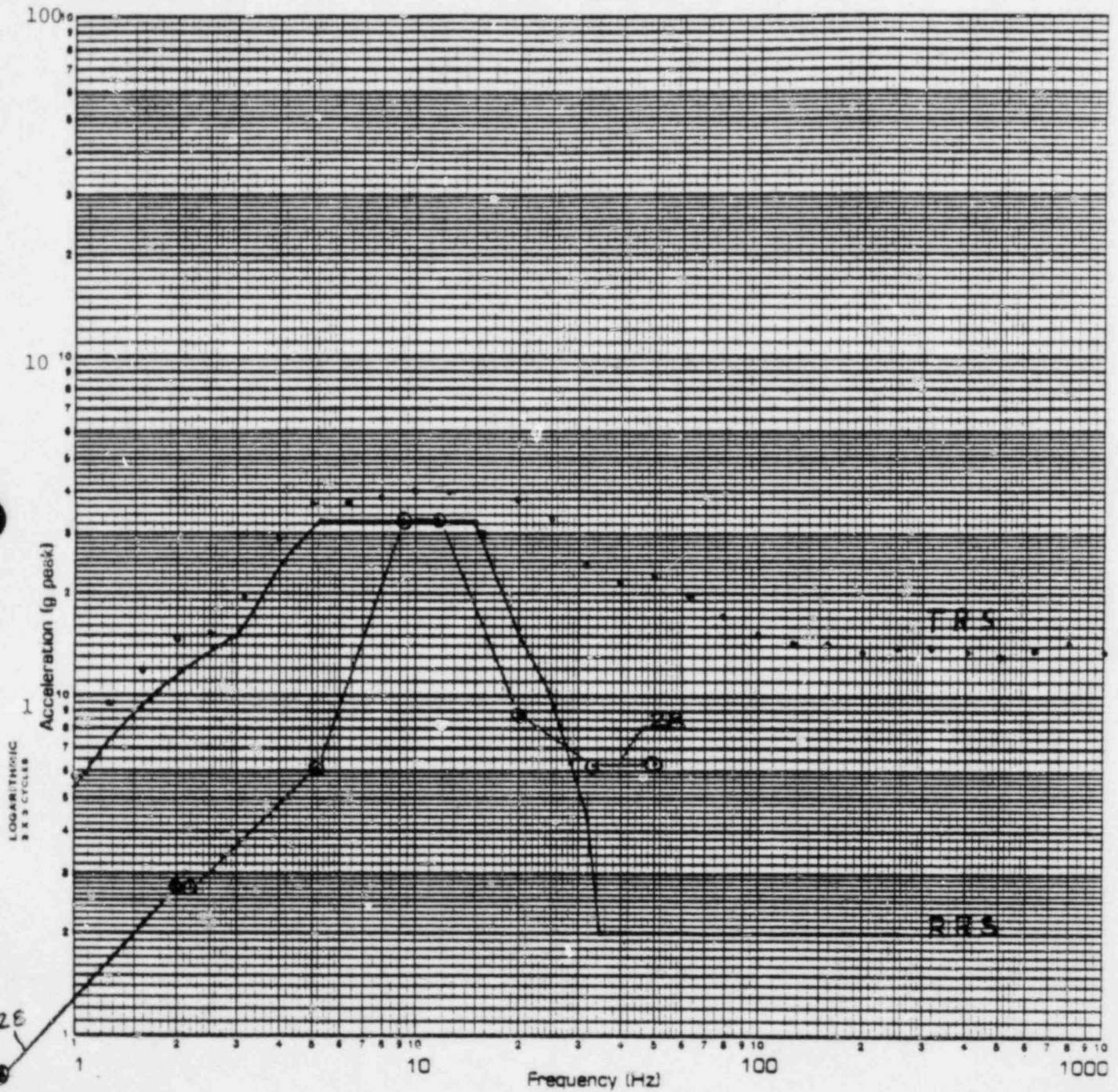
Legend

- TRS
- ⊙ Byron/Braidwood RRS

Byron/Braidwood Station  
 Commonwealth Edison Co.

1.0 □ 10 □ 100 □ 1000 □

DAMPING 2%



SPECIMEN                       
 AXIS SS/V

LOCATION NO. VCA  
 TEST RUN NO. 15

Fig. 8.3

Comparison of TRS vs RRS Vertical OBE  
 Side-to-Side/Vertical 2% Damping



Page No. III-453  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

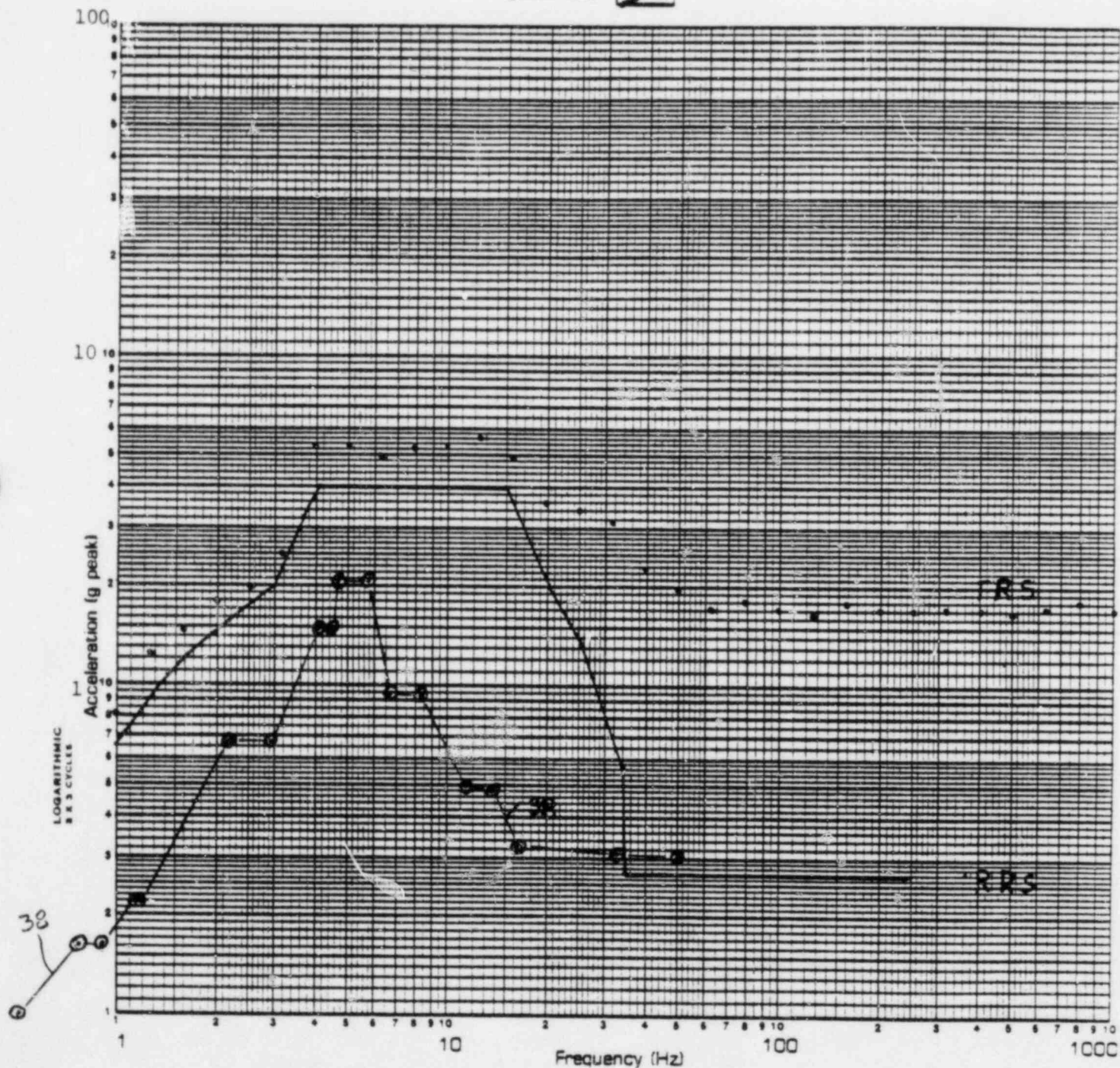
Legend

- TRS
- ⊙ Byron/Braidwood RRS

Byron/Braidwood Station  
Commonwealth Edison Co.

1.0 ☐ 10 ☐ 100 ☒ 1000 ☐

DAMPING ☒ 2%



SPECIMEN SS  
AXIS SS

LOCATION NO. HGA  
TEST RUN NO. 15

Fig. 8.4

Comparison TRS vs RRS Horizontal OBE  
Side-to-Side/Vertical 2% Damping

Page No. III-444  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

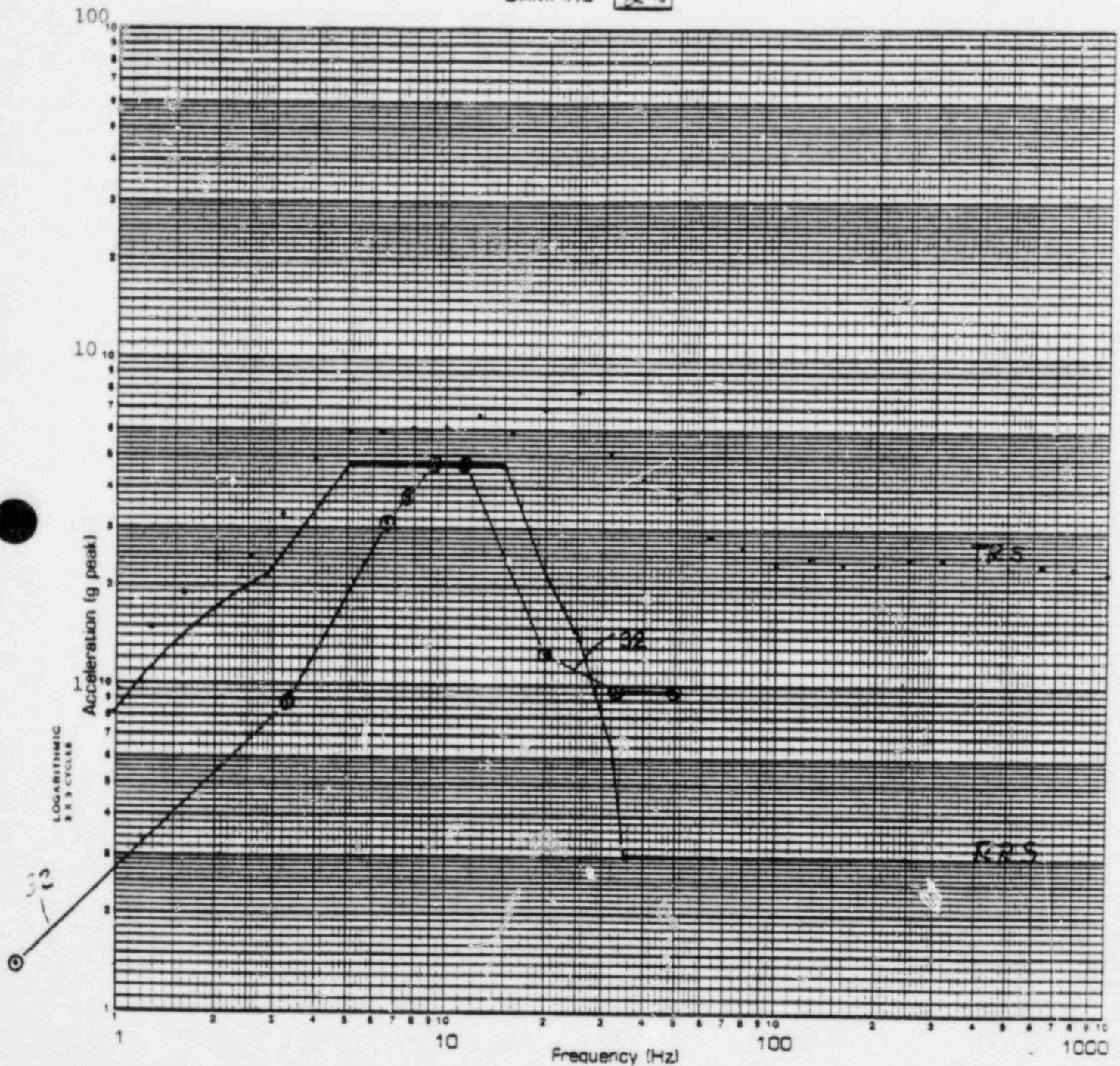
Legend

- TRS
- ⊙ Byron/Braidwood RRS

Byron/Braidwood Station  
Commonwealth Edison Co.

1.0 ☐ 10 ☐ 100 ☒ 1000 ☐

DAMPING ☒ 2%



SPECIMEN           

LOCATION NO. VCA

Fig 8.5

AXIS FB/V

TEST RUN NO 9

Comparison of TRS vs RRS Vertical DBE (SSE)  
Front-to Back/Vertical 2% Damping

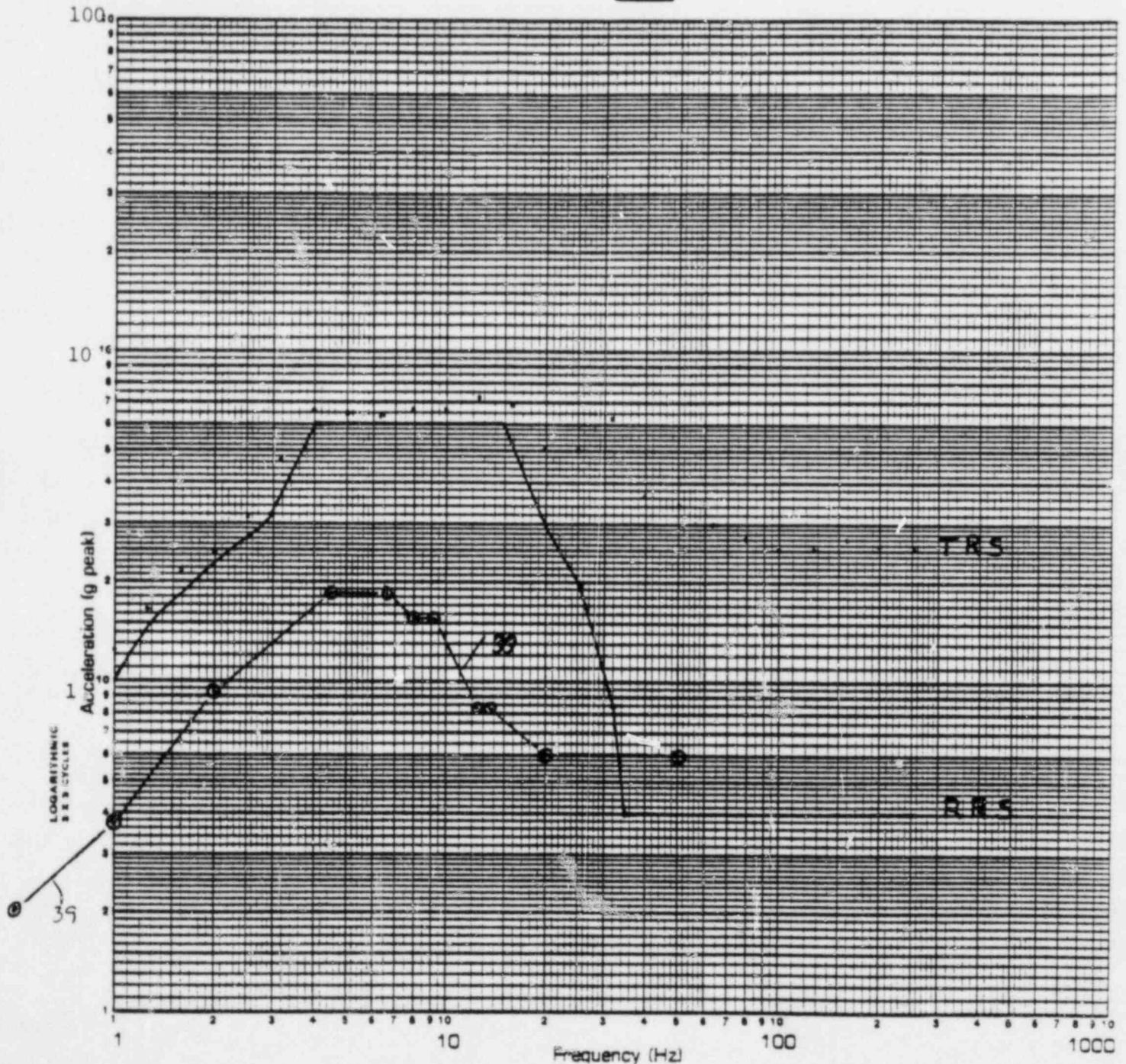
Page No. III-439  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

Legend

- TRS
  - ⊙ Byron/Braidwood RRS
- Byron/Braidwood Station  
Commonwealth Edison Co

1.0 ☐ 10 ☐ 100 ☒ 10000

DAMPING ☒ 2%



SPECIMEN \_\_\_\_\_  
AXIS FB/V

LOCATION NO. HCA  
TEST RUN NO. 9

Fig. 8.6

Comparison of TRS vs RRS Horizontal DBE (SSE)  
Front-toBack/Vertical 2% Damping



Page No. III-468  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

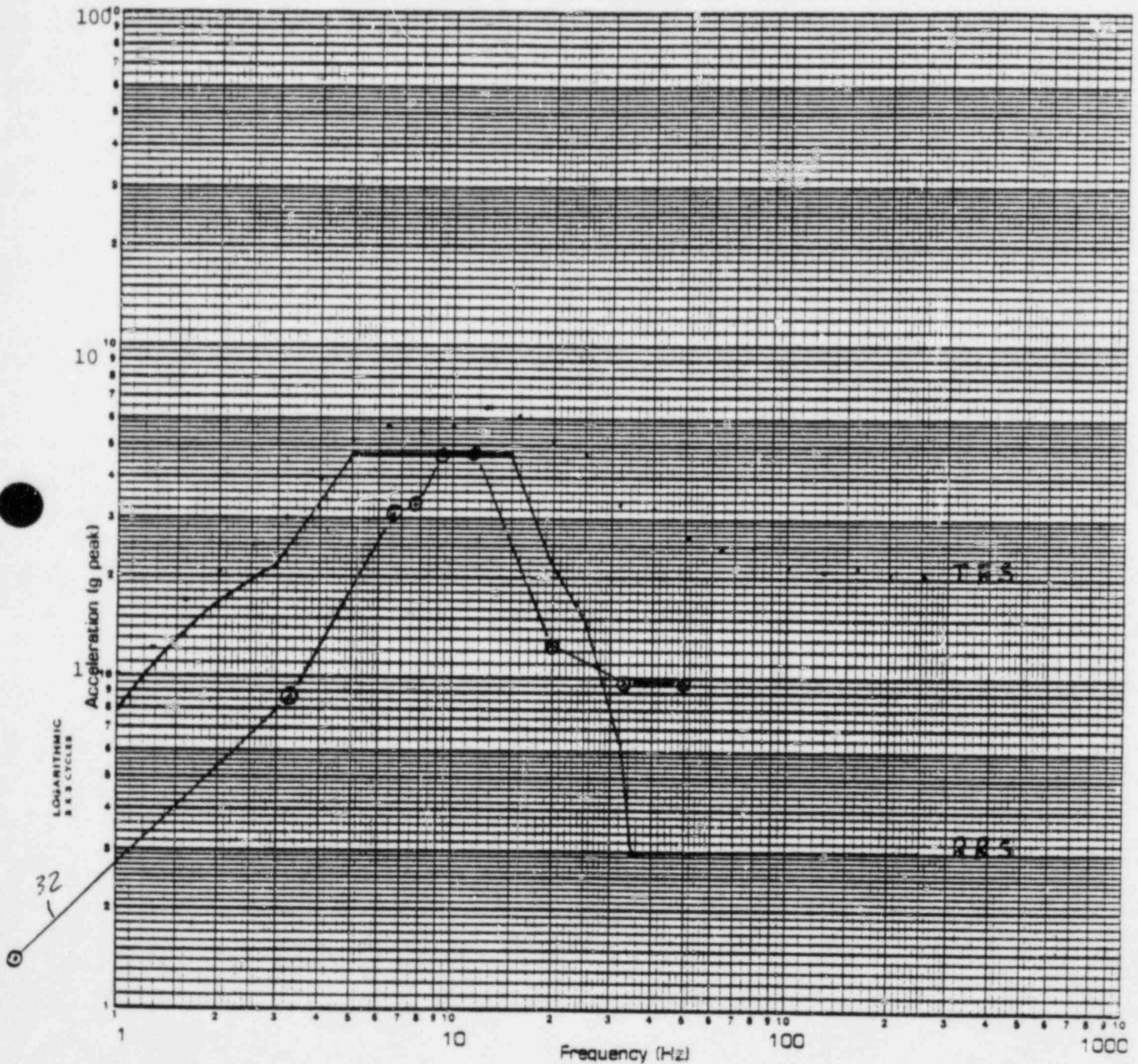
Legend

- TRS
- ⊙ Byron/Braidwood RRS

Byron/Braidwood Station  
Commonwealth Edison Co.

1.0 □ 10 □ 100 □ 1000 □

DAMPING 2%



SPECIMEN \_\_\_\_\_

LOCATION NO. VCA

Fig. 8.7

AXIS SS/V

TEST RUN NO. 16

Comparison of TRS vs RRS Vertical DBE (SSE)  
Side-to-Side/Vertical 2% Damping



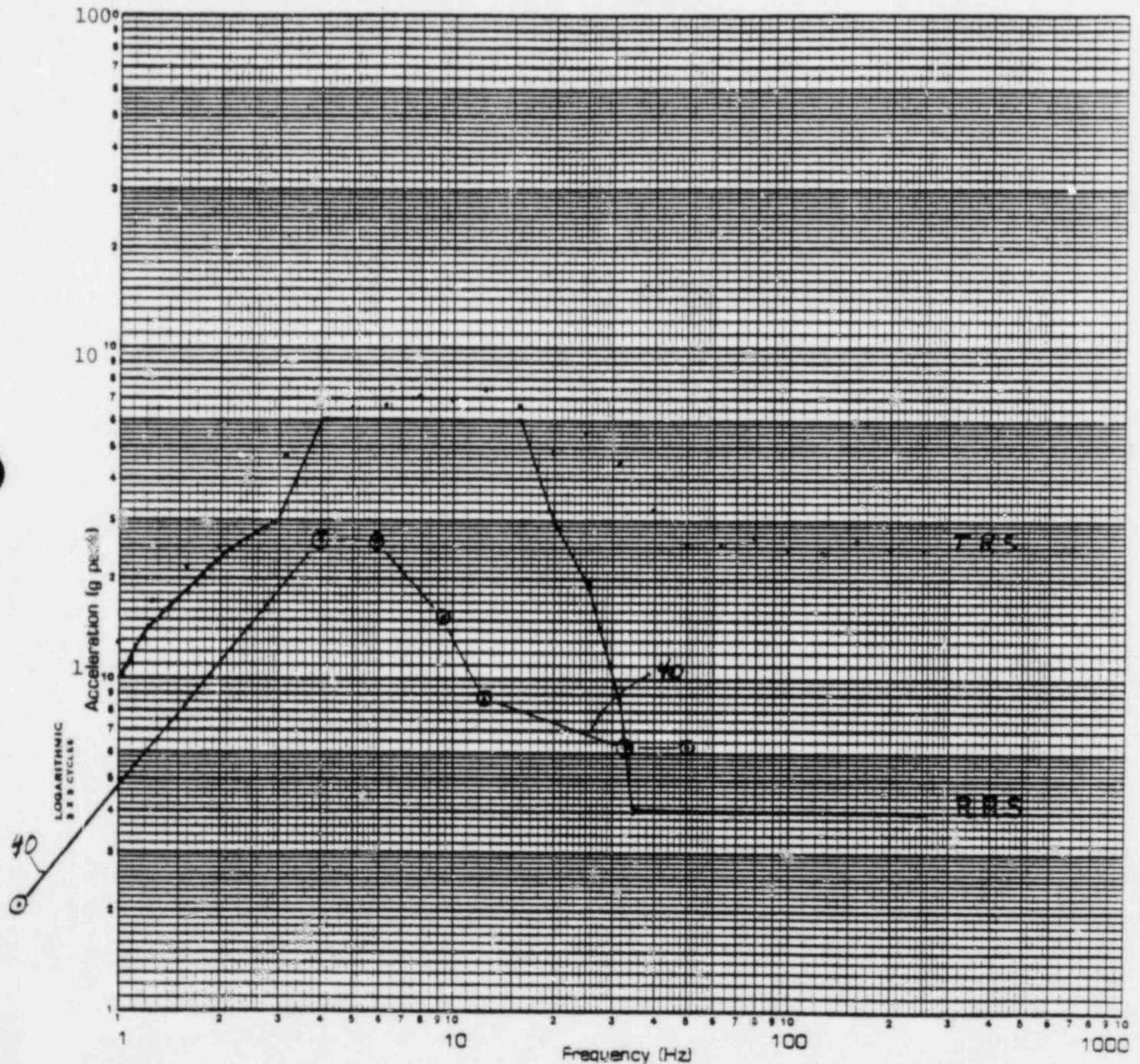
Page No. III-463  
Report No. 44681-2  
**FULL SCALE SHOCK SPECTRUM (g Peak)**

Legend

• TRS  
⊙ Byron/Braidwood RRS  
Byron/Braidwood Station  
Commonwealth Edison Co.

1.0 ☐ 10 ☐ 100 ☒ 1000

DAMPING ☒ 2%



SPECIMEN \_\_\_\_\_  
AXIS SS/V

LOCATION NO. HCA  
TEST RUN NO. 16

Fig 8.8

Comparison of TRS vs RRS Horizontal DBE(SSE)  
Side-to-Side/Vertical 2% Damping

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VOLUME I OF II

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Test #8                      Front-To-Back/Vertical Axis  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-73 through III-88

SECTION #9 (Cont'd.)

Series 2

Test #1 Front-To-Back/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-90 through III-101

Test #10 Side-To-Side/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-102 through III-113

Series 4

Test #1 Front-To-Back/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-115 through III-126

Test #9 Side-To-Side/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-127 through III-138

Series 5

Test #1 Front-To-Back/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-140 through III-147

Test #8 Side-To-Side/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-148 through III-155

Series 6

Test #1 Front-To-Back/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-157 through III-160

Test #10 Side-To-Side/Vertical Axis

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-161 through III-164

Appendix VI

Test Response Spectrum Plots

Test Series 1

Run #5 Side-To-Side/Vertical Axis - OBE

HCA = Horizontal Control Accelerometer.

VCA = Vertical Control Accelerometer.

Pages III-167 through III-176



SECTION #9 (Cont'd.)

Run #9                      Front-To-Back Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-177 through III-186

Run #14                     Front-To-Back/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-187 through III-210

Run #17                     Side-To-Side/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-213 through III-238

Test Series 2

Run #8                      Front-To-Back/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-240 through III-249

Run #9                      Front-To-Back/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-250 through III-271

Run #15                     Side-To-Side/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
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Run #16                     Side-To-Side/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
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Test Series 4

Run #7                      Front-To-Back/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-305 through III-314

Run #8                      Front-To-Back/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-315 through III-337

Run #14                     Side-To-Side/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-337 through III-346

Run #15                     Side-To-Side/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-347 through III-368



SECTION #9 (Cont'd.)

Test Series 5

Run #6                      Side-To-Side/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-370 through III-379

Run #7                      Side-To-Side/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-380 through III-397

Run #13                     Front-To-Back/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-398 through III-407

Run #14                     Front-To-Back/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-408 through III-425

Test Series 6

Run #8                      Front-To-Back/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-427 through III-436

Run #9                      Front-To-Back/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-437 through III-450

Run #15                     Side-To-Side/Vertical Axis - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages III-451 through III-460

Run #16                     Side-To-Side/Vertical Axis - SSE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
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Appendix VII

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Instrumentation Equipment Sheets

Series 1

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Seismic Simulation Test Program  
on  
One Battery Rack Containing 3FPR-23 Battery Cells

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Page 17	Photograph 2 Accelerometers 1H, 2V, 3H, and 4V (FPR-23).
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Pages 20 through 27

Test #8                      Side-To-Side/Vertical Axis  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages 28 through 35

Appendix II                      Test Response Spectrum Plots  
Run #6                      Front-To-Back/Vertical - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages 38 through 47

Run #7                      Front-To-Back/Vertical - DBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages 48 through 65

Run #14                      Side-To-Side/Vertical - OBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages 66 through 75

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Run #15                      Side-To-Side/Vertical - DBE  
HCA = Horizontal Control Accelerometer.  
VCA = Vertical Control Accelerometer.  
Pages 76 through 93

Appendix III                      Instrumentation Log Sheets And  
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Appendix VI                      History of the Naturally Aged Cells  
Pages 116 and 117

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Page IV-4                      Notice #14.

Appendix II                      Table  
Page IV-6 and IV-7              Table IV-I    Discharge Date - Post Seismic.

Appendix III                      Figure IV-1  
Page IV-1                      Capacity Discharge Test Setup.

Appendix IV                      Data Sheets - Capacity Discharge Data

Seismic Series 1

Cells:              1A1, 1A2, 1A3  
                         1B1, 1B2, 1B3  
                         1C1, 1C2, 1C3  
                         1D1, 1D2, 1D3

Pages IV-13 through IV-30

Seismic Series 2

Cells:              2A1, 2A2, 3A3  
                         3B1, 3B2, 3B3  
                         2C1, 2C2, 2C3

Pages IV-32 through IV-35

SECTION #9 (Cont'd.)

Seismic Series 3

Cells: 17, 18, 21  
46, 47, 48

Pages IV-37 through IV-39

Seismic Series 4

Cells: 1, 2, 3  
12, 13, 14  
3B2, 3B1

Pages IV-41 through IV-43

Seismic Series 5

Cells: 4, 5, 6  
4B1, 4B2, 4B3

Pages IV-45 and IV-46

Seismic Series 6

Cells: 7, 8, 9

Pages IV-48 and IV-49

Appendix V

Instrumentation Equipment Sheets

Pages IV-52 through IV-55

Section 5

Gould Test Procedure (Heat Aging)

Pages V-2 through V-18

SECTION #10

GOULD INSTALLATION AND OPERATING INSTRUCTIONS

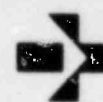
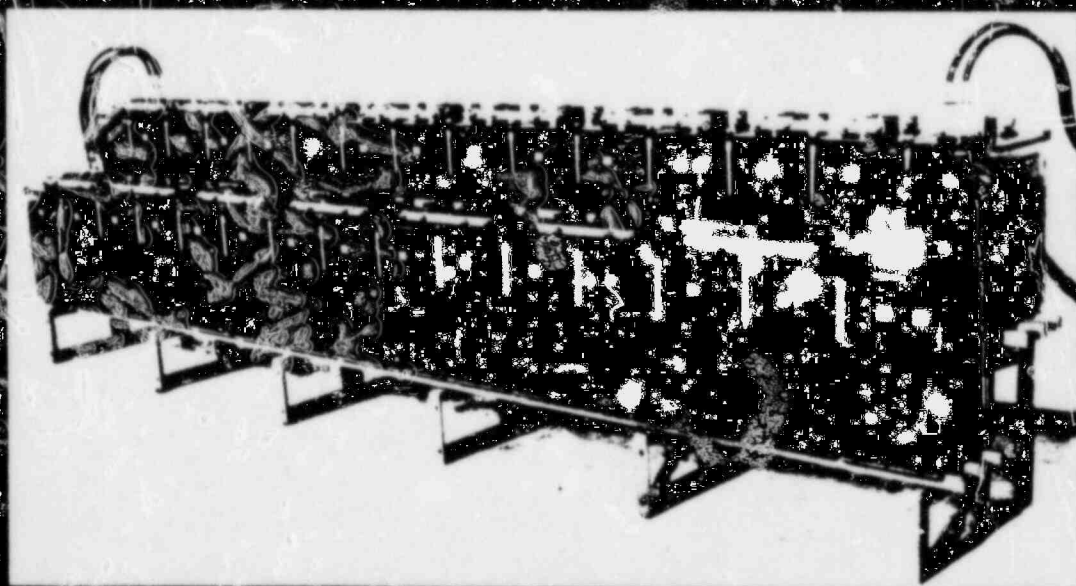
GB-3384B 6/80

These instructions are designed to cover a wide variety of battery applications and are, therefore, not finite for specific usage.

Additional guidance for Class 1E lead storage batteries is referenced in IEEE 484-1975 and IEEE 450-1980. These shall be used as the final determinant.

# Stationary Battery Installation and Operating Instructions

Lead-Acid Battery Type  
Lead-Acid Battery Type



**GOULD**

*Electronics & Electrical Products*



YOUR GOULD REPRESENTATIVE

SALESPERSON \_\_\_\_\_

TELEPHONE \_\_\_\_\_

LOCATION \_\_\_\_\_

# **STATIONARY BATTERY INSTALLATION AND OPERATING INSTRUCTIONS**

**LEAD-ANTIMONY TYPES**

**LEAD-CALCIUM TYPES**

Gould Inc., Industrial Battery Division  
2050 Cabot Boulevard West, Langhorne, Pa. 19047  
Telephone (215) 752-0555

 **GOULD**  
*Electronics & Electrical Products*

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## SECTION I

### 1.0 GENERAL INFORMATION

Caution! Before proceeding with the unpacking, handling, installation and operation of this lead-acid storage battery, the following general information should be reviewed together with the recommended safety precautions.

A lead-acid battery is an electro-chemical device containing an electrolyte which is a dilute solution of sulfuric acid and water. This electrolyte is corrosive and can cause injury.

Lead-acid batteries, when installed, are capable of high voltage which can cause electrical shocks to personnel.

All lead-acid batteries, in the course of normal operation, generate gases which can be explosive.

Stationary batteries (when installed) are usually on float charge continually, unless on discharge in the event of A.C. failure.

Failure to follow this precaution will result in excess heat and violent chemical reaction which may cause serious injury to personnel.

E. If electrolyte comes into contact with skin or clothing, immediately wash with water and neutralize with a solution of baking soda and water. Secure medical treatment. If electrolyte comes into contact with the eyes, wash or flush with plenty of clean water. Secure medical treatment immediately.

F. Exercise care when handling cells. When lifting straps and strap spreaders are provided, use them with appropriate mechanical equipment to safely handle cells and avoid injury to personnel.

G. Promptly neutralize and remove any electrolyte spilled when handling or installing cells. Use a baking soda/water solution (1 lb. per gallon of water) to prevent possible injury to personnel.

H. Make sure that all battery connections are properly prepared and tightened to prevent possible injury to personnel or failure of system.

I. Familiarize personnel with battery installation, charging and maintenance procedures. Restrict access to battery area, permitting trained personnel only, to reduce the possibility of injury.

J. Whenever possible, when making repairs to charging equipment and/or batteries, interrupt AC and DC circuits to reduce the possibility of injury to personnel and damage to system equipment.

## SECTION II

### 2.0 SAFETY PRECAUTIONS

A. Wear rubber apron, gloves and safety goggles (or face shield) when handling, installing or working with batteries. This will help prevent injury due to splashing or spillage of sulfuric acid.

B. Prohibit smoking. Keep flames and sparks of all kinds away from vicinity of storage batteries as liberated or entrapped hydrogen gas in the cells may be exploded, causing injury to personnel and damage to cells.

C. Never place metal tools on top of cells, since sparks due to shorting across cell terminals may result in an explosion of hydrogen gas in or near the cells. Insulate tool handles to protect against shorting.

D. When preparing electrolyte, always pour acid into water, NEVER water into acid.

### NOTE

If the foregoing precautions are not fully understood, clarification should be obtained from your nearest Gould representative. Local conditions may introduce situations not covered by Gould Safety Precautions. Here again, contact the nearest Gould representative for guidance with your particular safety problem; also refer to applicable federal, state and local regulations as well as industry standards.

## SECTION III

### 3.0 RECEIPT OF SHIPMENT

Immediately upon delivery by the carrier, examine for possible damage caused in transit. Damaged packing material or staining from leaking electrolyte would indicate rough handling. If such conditions are found, make descriptive notation on delivery receipt before signing. If cell damage is found, request an inspection by the carrier and file a damage claim.

#### 3.1 CONCEALED DAMAGE

Shortly after receipt (within 15 days), examine all cells for concealed damage. Pay particular attention to packing material exhibiting damage or electrolyte staining. Cells with electrolyte levels more than 1/2" below top of plates have suffered probable permanent damage due to plate exposure to air. If this condition or other cell damage is found, request an inspection by the carrier immediately and file a concealed damage claim.

#### 3.2 ELECTROLYTE LEVELS

Cells are shipped with electrolyte levels about 1/8" below the high level line. During shipment, the levels drop due to the loss of gases from internal cell components. The amount of drop in level will vary with each type of cell. Electrolyte levels, when received, may range from the high level line to slightly below the low level line. If this condition exists, make no addition of electrolyte or water at this time (*see Section 8.3*). If certain cells have low electrolyte levels, with less than 1/2" of plates exposed to air, add battery grade sulphuric acid of the same specific gravity as the remaining cells; thus bringing low level cells up to the average level of other cells.

## SECTION IV

### 4.0 STORAGE PRIOR TO INSTALLATION

#### 4.1 STORAGE LOCATION

If the battery is not to be installed at the time of receipt, it is recommended that it be stored indoors in a cool [60°F (15.6°C) to 90°F (32°C)], clean, dry location. Do not tier pallets or possible cell damage may occur.

#### 4.2 STORAGE INTERVAL

For batteries shipped wet, fully-charged, the following storage intervals from date of shipment to date of installation and initial charge should not be exceeded:

Lead-Antimony Types:  
Three (3) Months

Lead-Calcium Types:  
Six (6) Months

Storage beyond the above stated periods can result in sulphated plates which can be detrimental to battery life and performance.

The battery should be given its initial charge (*see Section 8.0*) before the end of the above stated storage intervals and repeated for each additional storage interval.

Failure to charge in accordance with the above can void the battery's warranty.

#### 4.3 DRY-CHARGED BATTERIES

For batteries shipped dry-charged, follow special handling and preparation instructions supplied as well as appropriate sections of this Manual.

## SECTION V

### 5.0 RACK ASSEMBLY

Assembly of the battery rack should be completed in accordance with the Gould drawing and/or instructions included with the rack, GB-3492 or GB-3493.

## SECTION VI

### 6.0 UNPACKING AND HANDLING

Most cells are packed in individual corrugated cartons. Some smaller size cells are packed in a master carton containing 2 (two) or 3 (three) cells. Cartons are shipped on wood pallets. Remove material holding cartons to pallets, exercising care when cutting banding material to prevent injury. If individual cells are to be moved to another location, do not remove carton at this time. Exercise caution if using a two-wheeled hand truck and, to prevent spillage of electrolyte, do not tilt cell more than 25 degrees from vertical. When cells have been brought to the installation site, remove carton sleeve and top corrugated spacers.

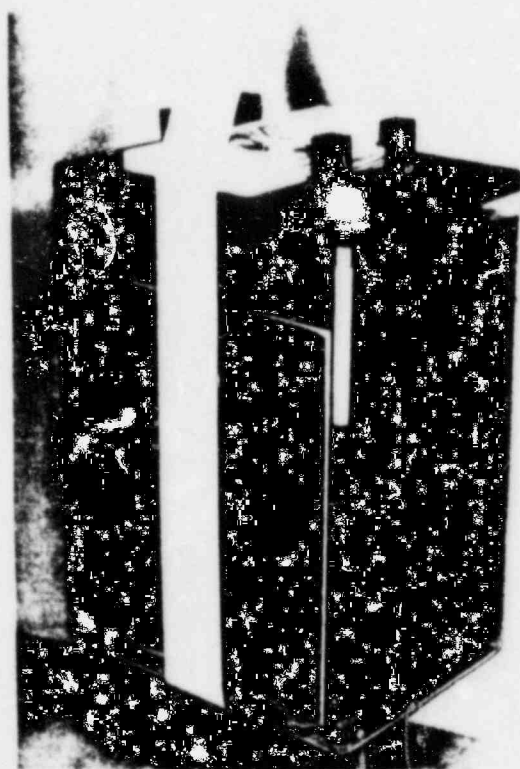


FIGURE 1

**DO NOT LIFT CELLS BY THEIR TERMINAL POSTS.** Support the cells from the bottom when handling and unpacking. In general, units weighing less than 75 pounds are handled manually, being supported from the bottom.

After removal of outer carton and top spacers, the cell should still be resting in the bottom corrugated tray. This tray is designed to be easily broken away to permit positioning of a lifting strap under the cell with a minimal amount of cell tilting.

A lifting strap and a strap spreader are furnished for use with mechanical lifting devices, when cells weigh 75 pounds or more. See Figure 1 which shows typical positioning of strap and spreader.

Always use lifting straps and spreaders, when provided, together with suitable mechanical lifting devices to prevent injury to personnel or damage to cells.

Never slide cells across rough surfaces as severe scratching of plastic container bottom may result in stressing and rupturing of the jar with subsequent loss of electrolyte. At all times, exercise care when handling cells to prevent scratching of plastic jars and covers.

## SECTION VII

### 7.0 INSTALLATION

#### 7.1 BATTERY LOCATION

It is recommended that the battery be installed in a clean, cool, dry location. Cells should not be exposed to heating units, strip heaters, radiators, steam pipes or sunshine through a window.

#### 7.2 TEMPERATURE

A battery location having an ambient temperature of 75°F (24°C) to 77°F (25°C) will result in optimum battery life. Batteries operated in high ambient temperatures will result in reduced life. Therefore, for longer life and ease of maintenance, locations having cooler ambient temperatures are recommended.

The normal battery operating temperatures are between 60°F (16°C) and 90°F (32°C).



### 7.3 TEMPERATURE VARIATION

The location or rack arrangement should result in no greater than 50°F (2.78°C) variation in cell temperatures of a series string at any given time. If a greater variation is found, steps should be taken to correct the condition. When uniform cell temperature is maintained, the need for equalizing charges may be eliminated or reduced in frequency.

### 7.4 VENTILATION

Ventilation should be provided in the battery room or area to prevent hydrogen, liberated from the cells in service, from exceeding a 1% concentration. Concentrations about this percentage can result in an explosive mixture, which could be ignited by sparks from adjacent electrical equipment as well as accidental sparks or open flames introduced by personnel. All air moved by ventilation in the battery room or area should be exhausted into the outside atmosphere and should not be allowed to recirculate into other confined areas.

### 7.5 PLACEMENT OF CELLS

It is assumed at this point that the battery rack has been assembled. Study the rack layout and wiring drawings to determine proper location of the positive and negative terminals of the battery; this will establish correct positioning of the initial cell on each rack row. Cells are normally installed with plate edges perpendicular to rack length. Measure and mark the center of the rack stringer length. Determine the number of cells to be placed in each row. When an odd number of cells are in the row, place the center of the initial cell at the center point of the rack stringer length.

When an even number of cells are in the row, locate the initial cells so that the center of the space between cells coincides with the center mark of the stringer length.

When installing cells on the rack, start at the lower step or tier for stability and safety reasons.

Place cells on the rack so that the positive terminal (marked "+") of each cell adjoins the negative terminal (marked "-") of the next cell. The standard spacing between cells is 1/2" at the top of the jars.

Adjacent cells should not touch; nor should any cell contact the metal rack supports or metal cable conduits. Check for proper alignment and 1/2" spacing between cells. Adjust cell position where necessary. This should be completed before installation of intercell connectors.

The cell post surfaces have a coating of No-Ox-Id "A"\* grease applied at the factory. Do not remove any grease from posts. Re-coat any surfaces that may have been exposed during handling of the cells.

### 7.6 CELL TERMINAL HARDWARE

On Gould "D", "E", and "F" type cells, two lead-covered brass nuts are used in conjunction with a brass stud on each post. These are pre-greased at the factory with No-Ox-Id "A" grease and are shipped installed on cell posts. During installation of the intercell connectors (*see Section 7.7*), exercise care to provide equal extension of the brass stud past each connector. Hold one end of the stud and install one of the lead-covered nuts finger tight. Install second nut while holding first nut. This will provide equal engagement of the nuts and stud.

On Gould "M" and "N" type cells, pre-greased stainless steel bolts, nuts and washers are supplied for cell terminals.

### 7.7 CONNECTING CELLS

Refer to the cell arrangement drawing to determine the quantity, size and correct positioning of the intercell connectors. On the "N" type cells using 1-1/4" wide connectors, the bolt holes are located off-center. Position connector so that the lesser dimension faces downward on the cell post.

\*Trademark of the Dearborn Chemical Co.

Gently clean contact surfaces only, of the lead plated intercell connectors, terminal plates and cable lugs using a brass suede brush or #00 grade sandpaper. Caution: Do not use powered wire brush or coarse abrasives, as lead plating may be removed exposing copper.

As contact surfaces are cleaned, apply a thin coating of No-Ox-Id "A" grease to these surfaces only.

Starting at center of the cell row, install connectors per wiring diagram and cell arrangement drawing.

On cells using stainless steel bolts, washers and nuts, make sure a washer is placed between the bolt head and connector as well as between the nut and connector.

#### CAUTION

When installing terminal hardware, do not permit any items to fall into cell. If such material remains in the cell, contamination will result; requiring replacement of the cell.

As intercell connectors are installed, adjust them to a level position and finger tighten hardware.

All terminal hardware installed on connectors should now be tightened as outlined in the following table:

- "D" type single cells —  
(2 lead-covered nuts with 1/4" stud.)  
Tighten to 75 inch pounds.
- "E" and "F" type cells —  
(2 lead-covered nuts with 5/16" stud.)  
Tighten to 100 inch pounds.
- "M" type cells —  
(Stainless steel hardware). Tighten to 100 inch pounds.
- "N" type cells —  
(Stainless steel hardware). See Figure 2.

#### NOTE

Torque both lead-covered nuts as well as the bolt head and nut of stainless steel hardware to their prescribed torque values. Torquing only one side of either combination will not provide the desired tightness.

#### QUANTITY AND THICKNESS OF INTERCELL CONNECTORS

#### TORQUE (INCH LBS.)

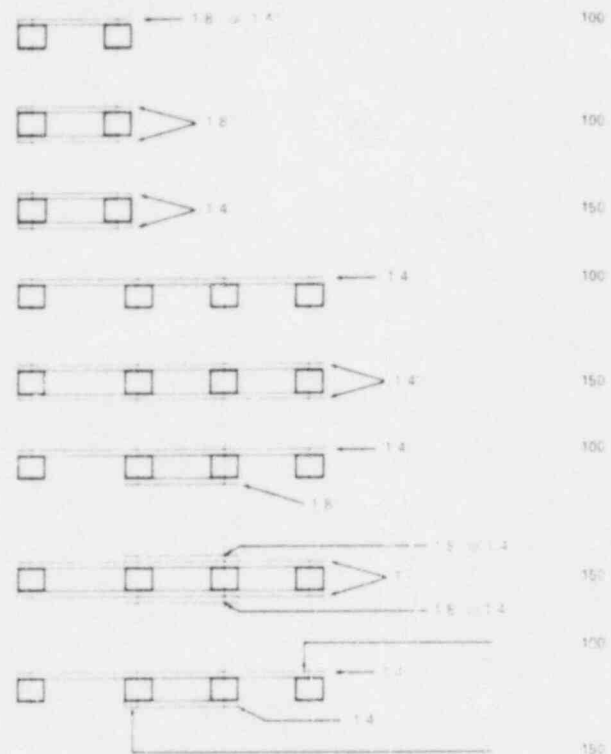


FIGURE 2

Following the torquing of stainless steel hardware, apply a thin coating of No-Ox-Id "A" grease to bolts, washers and nuts using a 1" paint brush.

Complete connecting of cells by installing necessary inter-row, inter-tier or inter-rack cable connectors. Do not connect battery to charger at this time.

Re-check to be certain that the cells are connected positive (+) to negative (-) throughout the battery string. Measure the total voltage at the battery terminals. The voltage should be equal to the number of cells times the voltage of one of the cells. Example: 60 cells times 2.05 volts = 123 volts.

## 7.8 COMPLETING INSTALLATION

Cells of 1200 ampere hours or less may have been shipped with Gould Pre-Vent™ vent/filling funnels in place. These vents have flexible plastic caps installed for shipping purposes. These caps may be removed and discarded, or they may be left in place if the battery environment is dusty. (See Figure 3).

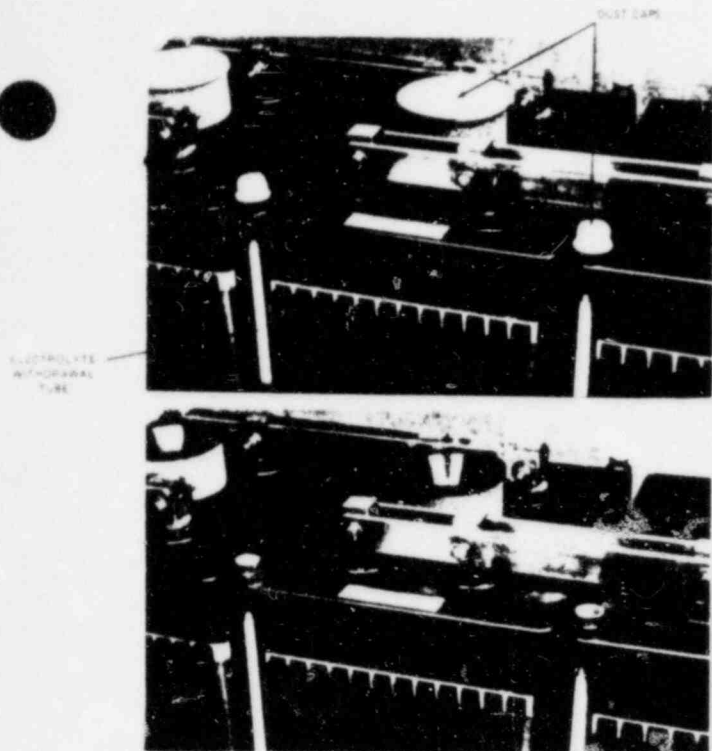


FIGURE 3



FIGURE 4

Cells from 1344 ampere hours up through 2550 ampere hours may have been supplied with a Gould Pre-Vent. For this size cell, the Pre-Vent units are not shipped in place. A standard screw-type vent is used for shipping purposes. If Pre-Vent units were specified, they would have been packed separately with other accessories. Remove the screw-type shipping vent one-at-a-time and install a Pre-Vent unit. Discard the shipping vent.

Other type cells may have separate explosion resistant vents installed at time of shipment. Separate plastic filling funnels are supplied along with this type vent. These funnels also have flexible plastic shipping caps. Here again, these may be removed and discarded or left in place if environment is dusty.

The Gould Pre-Vent assembly and other explosion resistant vents are designed to prevent external sparks or flames from igniting and exploding internal cell gases. (See Figure 4).

#### CAUTION

Before disposing of flexible plastic caps or screw-type shipping caps, neutralize any electrolyte on them in a baking soda - water solution to prevent injury to anyone handling these discarded items.

#### Electrolyte Withdrawal Tubes

Certain calcium cells are equipped with two electrolyte withdrawal tubes which are installed in the diagonal corners of the cell. These permit the taking of specific gravity readings at a point about one-third from the top of the plates. (See Section 11.1.) Refer to Figure 3.

A flexible shipping cap is installed on each withdrawal tube. These may be removed and discarded after neutralizing or left in place as dust covers.

### Plastic Numerals (See Page 15)

Plastic cell numerals and battery terminal polarity labels are provided for 12-cell batteries of 40 ampere hours and over. These should be installed per instructions included with the numerals. The positive terminal cell is usually designated as cell #1 in the series string

### Battery-to-Charger Connection

The positive (+) terminal of the battery should be connected to the positive (+) terminal of the charger and the negative (-) terminal of the battery to the negative (-) terminal of the charger.

## SECTION VIII

### 8.0 INITIAL CHARGE

Batteries lose some charge during shipment as well as during the period prior to installation. The battery should be installed and given its initial charge as soon after receipt as possible. (See Section 4.0).

### 8.1 CONSTANT VOLTAGE METHOD

Constant voltage is the principal method to give the initial charge, as most modern chargers are of the constant voltage design. In addition, some systems have equipment with voltage limitations making the use of constant current charging undesirable.

Determine the maximum voltage that may be applied to the system equipment. This voltage divided by the number of cells connected in series will establish the maximum voltage per cell that may be used.

Establish whether the battery is of lead-antimony, Planté or lead-calcium construction by referring to type on cell name plate and compare this with the cell type listings on pages 14 and 15.

For lead-antimony and Planté types, refer to Table A and for lead-calcium types refer to Table B to obtain various voltages and associated time periods recommended. Select the highest voltage the system will allow, to perform the initial charge in the shortest period of time.

## INITIAL CHARGE

### Recommended Voltages and Time Periods

TABLE A

#### Lead-Antimony and Planté Types

Cell Volts	Time-Hrs.
2.24	200
2.27	150
2.30	120
2.33	90
2.36	75
2.39	60

TABLE B

#### Lead-Calcium Types

Cell Volts	Time-Hrs. 1.215	Time-Hrs. 1.250	Time-Hrs. 1.300
	sp. gr.	sp. gr.	sp. gr.
2.24	444	-----	-----
2.27	333	-----	-----
2.30	210	-----	-----
2.33	148	333	-----
2.36	100	235	400
2.39	67	160	267
2.42	-----	108	182
2.45	-----	73	125
2.48	-----	-----	83

### NOTE

Time periods listed in Tables A and B are for cell temperatures from 70°F (21°C) to 90°F (32°C). For temperatures 55°F (13°C) to 69°F (20.5°C) double the number of hours. For temperatures 40°F (4°C) to 54°F (12°C) use four times the number of hours.

The above recommended time periods are considered minimum. Raise the voltage to the maximum value permitted by the system equipment. When charging current has tapered and stabilized (no further reduction for 3 hours), charge for the hours shown in the appropriate table and for the battery temperature, at the time of stabilization, until the lowest cell voltage ceases to rise. Monitoring of cell voltages should be started during the latter 10% of the applicable time period to determine lowest cell in battery.

## 8.2 CONSTANT CURRENT METHOD

If there is no limitation to the voltage that may be applied to the system equipment, constant current charging may be used for the initial charge. Charge the battery at its finish rate listed in the Tables on pages 14 and 15. Continue to charge at this value until the lowest cell specific gravity remains stable over a 5 hour period. If the ampere charge rate used is below the listed finish rate, increase the 5 hour stable period proportionately. For example, if the charge rate is 1/2 the finish rate, increase the stable period from 5 hours to 10 hours. Where high ambient temperatures prevail, cell temperatures should be monitored so that 110°F (43°C) is not exceeded. A reduction in the charge rate or temporary suspension of the charge should be made to permit cells to cool. Resume charging when cell temperatures are at 90°F (32°C) or below.

## 8.3 INITIAL CHARGE – ELECTROLYTE LEVELS

During the initial charge, there will be an increase in the electrolyte levels and they may go above the high level mark. (See Section 3.2). This is due to gases, that were lost during transportation or standing in storage, being restored to the cells. Do not remove any electrolyte even though levels may be above high level. When battery is placed on floating charge (See Section 9.2), the electrolyte levels should return close to the high level line.

# SECTION IX

## 9.0 OPERATION

### 9.1 FLOATING CHARGE METHOD

In this type of operation, the battery is connected in parallel with a constant voltage charger and the critical load circuits. The charger should be capable of maintaining the required constant voltage at battery terminals and also supply a normal connected load where applicable. This will then sustain the battery in a fully charged condition and also make it available to assume the emergency power requirements, in the event of an AC power interruption or charger failure.

## 9.2 FLOAT CHARGE – FLOAT VOLTAGES

The following are the float voltage ranges recommended for the various types of batteries. Select any "volts per cell" value within the range listed that will result in the series string having an average volts per cell equal to that value. Do not interchange voltage ranges from one type to another.

TABLE C  
Recommended Float Voltages

Lead-Antimony Types	2.15 to 2.17 volts per cell
Planté Types	2.17 to 2.19 volts per cell
Lead-Calcium Types:	
Nominal 1.215 sp. gr.	2.17 to 2.25 volts per cell
Nominal 1.250 sp. gr.	2.23 to 2.33 volts per cell
Nominal 1.300 sp. gr.	2.28 to 2.37 volts per cell

Modern constant voltage output charging equipment is recommended for the floating charger method of operation of Gould stationary type batteries. This type of charger, properly adjusted to the recommended float voltages, together with adherence to recommended maintenance procedures, will assist in obtaining consistent serviceability and optimum life.

After the battery has been given its initial charge (see Section 8.0), the charger should be adjusted to provide the recommended float voltage (see Table C) at the battery terminals. For example, a 60-cell lead-antimony battery should have 130 volts maintained at its terminals ... 60 cells x 2.17 volts per cell (V.P.C.) = 130 volts.

Do not use float voltages for lead-antimony or Planté types higher than shown in Table C, as excessive water consumption and reduced battery life will result.

Lead-calcium types may be floated at any of the voltage values (Table C) shown for a particular nominal specific gravity. Use the lower V.P.C. value in the appropriate nominal specific gravity group, where system equipment voltage limitations will not permit higher values. The use of higher V.P.C. values may make it unnecessary to give an equalizing charge. However, the use of higher float voltages where high ambient temperatures prevail, may result in reduced battery life.



### 9.3 VOLTMETER CALIBRATION

Panel and portable voltmeters used to indicate battery float voltages should be accurate at the operating voltage value. The same holds true for portable meters used to read individual cell voltages. These meters should be checked against a standard every six months and calibrated when necessary.

### 9.4 CYCLE METHOD OF OPERATION

This method is recommended for lead-antimony and Planté type cells only. Lead-calcium cells should not be cycle operated.

In cycle operation, the degree of discharge will vary for various applications. Therefore, the frequency of recharging will also vary. The recharge is conducted by manually starting the charge, generally using the normal finish rate listed on pages 14 and 15. The amount of charge necessary depends on the number of ampere hours discharge. If a shorter recharge period is desired, higher charge rates equal to the eight-hour rate of discharge may be used when the battery is more than 25% discharged and the cell voltage on charge is below 2.33 volts. When the cell voltage reaches 2.33, the charge rate should be reduced to the normal finish charge rate. The charge should be stopped when the specific gravity is ten (.010) points below the normal fully charged value.

The battery is now available for the next discharge requirement. The battery should be given an equalizing charge monthly by continuing the regular charge until there is no increase in specific gravity of the pilot cell for three hours, when using the finish charge rate.

### 9.5 RECHARGE

All batteries should be recharged as soon as possible following a discharge (within 8 hours). With constant voltage chargers, this will be accomplished automatically. However, to recharge in the shortest period of time, raise the charger output voltage to the highest value which the connected system will permit.

Do not exceed those voltage values listed in Table D or Table E on page 10.

## SECTION X

### 10.0 EQUALIZING CHARGE

An equalizing charge is a special charge given a battery when non-uniformity in voltage or specific gravity has developed between cells. It is given to restore all cells to a fully charged condition using a charging voltage higher than the normal float voltage and for a specified number of hours, as determined by the voltage used.

Non-uniformity of cells may result from low floating voltage due to improper adjustment of the charger or a panel voltmeter which reads incorrect (higher) output voltage. Also, variations in cell temperatures greater than 50°F (2.78°C) in the series string at a given time, due to environmental conditions or rack arrangement, can cause low cells.

### 10.1 EQUALIZING FREQUENCY

The following guidelines cover lead-antimony, Planté and lead-calcium types. Recommendations not applying to all types will be so designated.

A. An equalizing charge should be given quarterly or as required by conditions in the following paragraphs (Note: lead-calcium types at nominal 1.215 sp. gr. floated at 2.20 V.P.C. to 2.25 V.P.C., nominal 1.250 sp. gr. floated at 2.27 V.P.C. to 2.33 V.P.C. and nominal 1.300 sp. gr. floated at 2.31 V.P.C. to 2.37 V.P.C. may not require equalizing charges).

B. Equalize when the temperature corrected specific gravity of the pilot cell (or any cell for the quarterly reading) is more than 10 points below its full charge value. (See Section 11.2.)

C. Equalize when the floating voltage of the pilot cell (or any cell for the quarterly reading) is below 2.13 volts (nominal 1.215 sp. gr.), 2.18 volts (nominal 1.250 sp. gr.), and 2.23 volts (nominal 1.300 sp. gr.) or more than .04 volts below the average for the battery.

D. Equalize to complete a recharge of the battery in a minimum length of time following an emergency discharge.



E. If accurate quarterly records are maintained (*See Section 14.0*) and the individual cell voltages and temperature corrected specific gravities show no increase in spread from the previous quarterly readings, equalizing may be deferred. (*See Section 11.2*).

F. Equalize once a year even though preceding conditions did not require. (Lead-calcium types floated per paragraph A, may not require annual equalizing.)

## 10.2 EQUALIZING CHARGE METHOD

Constant voltage charging is the preferred method for giving an equalizing charge. Determine the maximum voltage that may be applied to the system equipment. This voltage, divided by the number of cells connected in series, will establish the maximum voltage per cell that may be used to perform the equalizing charge in the shortest period of time.

For lead-antimony and Planté types, refer to Table D and for lead-calcium types, refer to Table E to obtain various voltage and associated time periods recommended.

### EQUALIZING CHARGE

#### Recommended Voltages and Time Periods

TABLE D

#### Lead-Antimony & Planté Types

Cell Volts	Time-Hrs.
2.24	80
2.27	60
2.30	48
2.33	36
2.36	30
2.39	24

TABLE E

#### Lead-Calcium Types

Cell Volts	Time-Hrs. 1.215	Time-Hrs. 1.250	Time-Hrs. 1.300
	sp. gr.	sp. gr.	sp. gr.
2.24	222	-----	-----
2.27	166	-----	-----
2.30	105	-----	-----
2.33	74	166	-----
2.36	50	118	200
2.39	34	80	134
2.42	-----	54	91
2.45	-----	36	62
2.48	-----	-----	42

## NOTE

Time periods listed in Tables D and E are for cell temperatures from 70°F (21°C) to 90°F (32°C). For temperatures 55°F (13°C) to 69°F (20.5°C) double the number of hours. For temperatures 40°F (4°C) to 54°F (12°C) use four times the number of hours.

The above recommended time periods are considered minimum. Raise the voltage to the maximum value permitted by the system equipment. When charging current has tapered and stabilized (no further reduction for 3 hours), charge for the hours shown in the appropriate table and for the battery temperature at the time of stabilization, until the lowest cell voltage ceases to rise. Monitoring of cell voltages should be started during the latter 10% of the applicable time period to determine lowest cell in battery.

## SECTION XI

### 11.0 SPECIFIC GRAVITY

In a lead-acid cell, the electrolyte is a dilute solution of water and sulfuric acid. Specific gravity is a measure of the weight of acid in the electrolyte as compared to an equal volume of water. Therefore, electrolyte with a specific gravity of 1.215 means it is 1.215 times heavier than an equal volume of water which has a specific gravity of 1.000.

### 11.1 HYDROMETER READINGS

Specific gravity is used in determining a cell's state of charge. It decreases as the cell discharges and increases as the cell is charged; reaching its original value when the cell is fully charged. Specific gravity is expressed to the third decimal place (1.215) and is measured by a hydrometer float enclosed in a glass barrel/rubber bulb syringe. Draw sufficient electrolyte into the barrel, holding the syringe vertical and with no hand pressure on bulb, so that float is freely floating without touching sides or top of syringe.

The gravity is read on the hydrometer scale at the flat surface of the electrolyte. (See Figure 5).

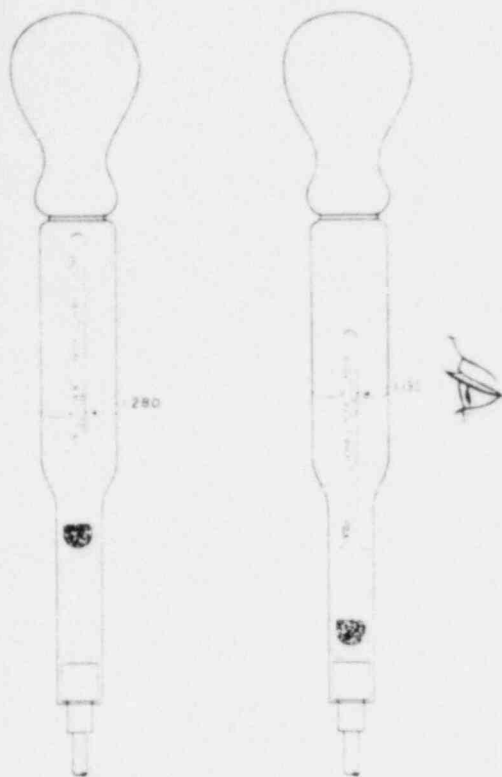


FIGURE 5

Clean the hydrometer glass barrel and float with soap and water as required for ease of reading and float accuracy.

When recharging a lead-calcium cell, the specific gravity reading lags behind the ampere hour input due mainly to the very low end of charge currents. Mixing of the electrolyte is slow due to the small amount of gas generated; so the gravity readings do not reflect the actual state of charge. A similar condition exists after water additions. Therefore, meaningful gravity readings can only be obtained at the top of the cell after an equalizing charge or after six weeks on float.

For this reason, most Gould lead-calcium cells have electrolyte withdrawal tubes to permit sampling of the electrolyte at a point one third down from the top of the plates. A long rubber tip on the hydrometer is inserted into the tube to provide an average value of cell specific gravity and a more accurate indication on the state of charge.

When taking a hydrometer reading, the base of the hydrometer syringe should be pressed firmly against the tube opening to prevent back splash of electrolyte. Fill and empty the hydrometer at least once in each cell before

reading. This will give a more accurate reading of the average electrolyte density.

## 11.2 CORRECTION FOR TEMPERATURE

When taking specific gravity readings, corrections must be made for variations in temperature of the electrolyte. For each 3°F (1.67°C) in temperature of the electrolyte above 77°F (25°C) add one point (.001) in specific gravity to the observed hydrometer readings; and for each 3°F (1.67°C) in temperature below 77°F (25°C) subtract one (.001) in specific gravity from the observed hydrometer reading.

### EXAMPLE:

Hydrometer Reading	Cell Temperature	Correction	Reading Corrected to 77°F (25°C)
1.213 sp. gr.	68°F (20°C)	-.003 points =	1.210 sp. gr.
1.207 sp. gr.	86°F (30°C)	+.003 points =	1.210 sp. gr.
1.204 sp. gr.	95°F (35°C)	+.006 points =	1.210 sp. gr.

## 11.3 CORRECTION FOR ELECTROLYTE LEVEL

The loss of water from the electrolyte due to evaporation as well as conversion of the water to hydrogen and oxygen by charging current; also affects the specific gravity value. For example: A fully charged cell with correct high level at 77°F (25°C) will have a nominal specific gravity of 1.215. When the electrolyte level has been reduced from evaporation and charging by 1/4", the specific gravity will be approximately 6 points (.006) higher or 1.221 @ 77°F (25°C). Therefore, when taking hydrometer readings, the electrolyte level referenced to the high level line should be recorded for proper evaluation of the specific gravity value. This applies when taking a pilot cell reading or for 10% of the cells when taking a quarterly set of readings.

## 11.4 SPECIFIC GRAVITY RANGE

Gould stationary batteries are furnished with a nominal fully charged specific gravity of 1.215 @ 77°F (25°C).

For special applications, nominal specific gravities such as 1.250 or 1.300 @ 77°F (25°C) may be used.

The specific gravity may range  $\pm 0.010$  points within a battery for any of the nominal values @ 77°F (25°C) with the electrolyte level at the high level line and still be considered satisfactory.

## SECTION XII

### 12.0 CELL VOLTAGE VARIATION

The tabulation below indicates the normal cell voltage variation that may exist with the battery on float and no greater than a 5°F (2.78°C) variation in cell temperature of a series string at any given time.

#### NORMAL VOLTAGE VARIATION

Type	Float Voltage	Variation
Lead-Antimony Platté	2.15 to 2.17 V.P.C.	±.04 V.P.C.
Lead-Calcium Nominal 1.215 sp. gr.	2.17 to 2.25 V.P.C.	±.05 V.P.C.
Nominal 1.250 sp. gr.	2.23 to 2.33 V.P.C.	±.05 V.P.C.
Nominal 1.300 sp. gr.	2.28 to 2.37 V.P.C.	±.05 V.P.C.

## SECTION XIII

### 13.0 PILOT CELL

A pilot cell is selected in the series string to reflect the general condition of all cells in the battery regarding specific gravities, float voltage and temperature. It serves as an indicator of battery condition between scheduled over-all individual cell readings.

A slight amount of electrolyte may be lost each time a specific gravity reading is taken, even though it is recommended that all electrolyte in the hydrometer be returned to the cell after reading. Therefore, it is suggested that the pilot cell be changed to another cell annually to provide a representative specific gravity indicator for the battery.

## SECTION XIV

### 14.0 RECORDS

A complete recorded history of the battery operation is most desirable and helpful in obtaining satisfactory performance. Good records will also show when corrective action may be required to eliminate possible charging, maintenance or environmental problems.

The following data should be read and permanently recorded for review by supervisory personnel:

A. Upon completion of the initial charge and with the battery floating at the desired float voltage for one week, read and record individual cell voltages, specific gravities (corrected to 77°F (25°C), ambient temperature plus cell temperatures and electrolyte levels for 10% of the cells. The cell temperature readings should be from each step or tier of the rack to reflect temperature range of the battery.

This first set of readings will be the basis for comparison with subsequent readings to reflect possible operating problems and the need for corrective action.

B. Weekly - Pilot cell voltage and also total battery float voltage at battery terminals.

C. Monthly - Pilot cell voltage, specific gravity, temperature and electrolyte level.

D. Quarterly - A complete set of individual cell readings as recommended in "A" above.

E. Any time the battery is given an equalizing charge (*see Section 10.1*), an additional set of individual cell readings should be taken after battery has been returned to normal float for one week. These will serve as an updated basis for comparison with future readings.

F. Record dates of any equalizing charges as well as total quantity of water when added. Also record any maintenance and/or testing performed.

The foregoing suggested frequency of record taking may have to be modified somewhat to suit local requirements.

*See Page 15 for Battery nameplate*

## SECTION XV

### 15.0 WATER ADDITIONS

There are two conditions in the operation of batteries which cause a reduction in the amount of water in the electrolyte, resulting in a lowering of the electrolyte level. These are normal evaporation and the conversion of water into hydrogen and oxygen gases by the charging current. These gases are liberated through the cell vents. Periodically, this water loss must

be replaced with approved or distilled water to maintain the electrolyte level between the high and low level lines.

If suitability of the local water supply for use in storage batteries is questionable, contact your nearest Gould representative for instructions regarding procedure for submitting a sample for analysis. A report will be rendered as to whether or not the water is suitable.

If water is to be stored in containers, they should be clean and of non-metallic material; such as: glass, hard rubber, porcelain or plastic.

Infrequently used water lines should be purged to remove accumulated impurities; thus preventing their introduction into the battery.

Water additions should be scheduled prior to an equalizing charge so that mixing with the electrolyte occurs. Also at unheated installations, arrange water additions when battery temperature is above 50°F (10°C).

Never introduce "battery additives" into a Gould battery.

## **SECTION XVI**

### **16.0 TAP CONNECTIONS**

It is not recommended that tap connections be used on a battery, as possible unbalance between the groups of cells may result. This can cause overcharging of the untapped group of cells and undercharging of the tapped cells supplying the load. This condition can cause unsatisfactory operation and reduced battery life.

## **SECTION XVII**

### **17.0 TEMPORARY NONUSE**

An installed battery that is permitted to stand idle for a period of time should be treated in the following manner. With the battery on normal float, add approved water to cells to bring electrolyte level to the high level line. Give the battery an equalizing charge per Section 10.2. Following completion of the equalizing charge, open connections at the battery terminals to separate charger and load circuit from battery.

Every three months, temporarily connect battery to charger and give it an equalizing charge.

To return to normal service, re-connect all open connections, give equalizing charge and then return battery to normal float voltage.

## **SECTION XVIII**

### **18.0 BATTERY CLEANING**

Periodically, clean cell jars and covers with a water dampened cloth to remove accumulated dust. Cell parts damp with electrolyte should be neutralized with a baking soda - water solution (1 lb. of soda per gallon of water). Apply with cloth dampened with the solution, making sure none is allowed to enter the cell. Continue to neutralize until fizzing action ceases, then wipe area with a water dampened cloth to remove soda solution. Wipe dry with a clean cloth.

#### **CAUTION:**

Do not clean plastic cell jars or covers with solvents, detergents, oils or spray type cleaners, as these materials may cause crazing and cracking of the plastic materials.

## **SECTION XIX**

### **19.0 CONNECTIONS**

Battery terminal and intercell connections should be corrosion free and tight to provide satisfactory operation while on float charging or when supplying emergency power. Periodically, these connections should be inspected. For proper removal of corrosion, disconnect the connections involved. Where circuit continuity must be maintained, use temporary flexible cables, of adequate current carrying capability, as parallel connections. Remove corrosion by neutralizing with a baking soda - water solution. Gently clean the affected area using a suede brush or #00 grade sandpaper. Apply a thin coating of No-Ox-Id "A" grease to the cleaned contact surfaces and re-establish connection. Reinstalled terminal hardware should be torqued to values in Section 7.7 (Connecting Cells). Annually, all terminal and intercell connections should be re-torqued per Section 7.7.



# BATTERY TYPES\*

LEAD-ANTIMONY CELL TYPE	8 HR. A. H.	ELECTRO-LYTE GALS. PER CELL	SPECIFIC GRAVITY RANGE**	LEAD-CALCIUM CELL TYPE	LEAD-ANTIMONY CELL TYPE	8 HR. A. H.	ELECTRO-LYTE GALS. PER CELL	SPECIFIC GRAVITY RANGE**	LEAD-CALCIUM CELL TYPE
AS 5	10	0.09	55	—	—	1200	6.6	80	FTC 21
2 AS 5	10	0.09	55	—	—	1320	6.5	90	FTC 23
3 AT 5	10	0.08	67	—	—	1440	6.4	95	FTC 25
2 or 3 BS 5	15	0.115	80	—	—	1560	8.3	85	FTC 27
2 BS 9	30	0.25	80	—	—	1680	8.2	90	FTC 29
3 BT 7	30	0.16	90	—	—	1800	8.2	95	FTCS 29
2 or 3 CS 7	50	0.32	85	—	2 MAX 170	170	1.3	75	2MCX 170
2 or 3 CSO 7	50	0.32	85	—	2 MAX 190	190	1.3	85	2MCX 190
2 or 3 CS 13	100	0.54	95	—	2 MAX 255	255	1.2	100	2MCX 255
2 or 3 CSO 13	100	0.54	95	—	MAX 285	285	1.7	80	MCX 285
2 or 3 DS 5	50	0.29	100	2 or 3 DSC 5	MAX 340	340	1.6	100	MCX 340
2 or 3 DSO 5	50	0.29	100	2 or 3 DSCO 5	MAX 380	380	2.1	90	MCX 380
2 or 3 DS 7	75	0.40	100	2 or 3 DSC 7	MAX 425	425	1.9	105	MCX 425
2 or 3 DSO 7	75	0.40	100	2 or 3 DSCO 7	MAX 475	475	2.8	110	MCX 475
2 or 3 DS 9	100	0.50	100	2 or 3 DSC 9	MAX 510	510	2.7	105	MCX 510
2 or 3 DSO 9	100	0.50	100	2 or 3 DSCO 9	MAX 595	595	2.7	120	MCX 595
DKR 5	50	0.34	85	DC 5	NAX 600	600	6.0	65	NCX 600
DKR 7	75	0.64	75	DC 7	NAX 672	672	6.0	60	NCX 672
DKR 9	100	0.59	95	DC 9	NAX 750	750	5.5	85	NCX 750
DKR 11	125	0.90	80	DC 11	NAX 840	840	5.6	80	NCX 840
DKR 13	150	0.86	105	DC 13	NAX 900	900	5.1	105	NCX 900
DKR 15	175	1.00	95	DC 15	NAX 1008	1008	5.1	100	NCX 1008
EKR 11	200	1.7	55	EC 11	NAX 1050	1050	4.9	125	NCX 1050
EKR 13	240	1.6	80	EC 13	NAX 1200	1200	5.0	150	NCX 1200
EKR 15	280	1.5	100	EC 15	NAX 1344	1344	6.8	100	NCX 1344
EKR 17	320	1.8	100	EC 17	NAX 1350	1350	6.3	100	NCX 1350
EKR 19	360	2.3	85	EC 19	NAX 1500	1500	6.0	120	NCX 1500
EKR 21	400	2.2	95	EC 21	NAX 1650	1650	8.0	90	NCX 1650
EKR 23	440	2.7	90	EC 23	NAX 1680	1680	8.3	100	NCX 1680
EKR 25	480	2.6	100	EC 25	NAX 1800	1800	7.6	115	NCX 1800
EKR 27	520	3.1	90	EC 27	NAX 1848	1848	12.6	80	NCX 1848
EKR 29	560	3.0	100	EC 29	NAX 1950	1950	7.3	150	NCX 1950
FKS 17	626	3.6	90	FCS 17	NAX 2016	2016	12.1	90	NCX 2016
FKS 19	704	3.4	105	FCS 19	NAX 2100	2100	11.5	105	NCX 2100
FKS 21	782	4.9	90	FCS 21	NAX 2184	2184	11.5	100	NCX 2184
FKS 23	860	4.8	105	FCS 23	NAX 2250	2250	10.9	115	NCX 2250
FKS 25	938	6.7	90	FCS 25	NAX 2400	2400	10.3	130	NCX 2400
FKS 27	1017	6.5	100	FCS 27	NAX 2550	2550	9.7	150	NCX 2550
FKS 29	1095	6.3	105	FCS 29	PLANTE TYPE CELLS				
FKS 31	1173	6.1	110	FCS 31	2 or 3 CPE 3	8	0.12	50	—
2 or 3 ETA 5	120	1.3	45	2 or 3 ETC 5	2 or 3 CPE 5	16	0.115	90	—
2 or 3 ETA 7	180	1.2	80	2 or 3 ETC 7	2 or 3 CPE 7	24	0.171	85	—
ETA 9	240	1.7	75	ETC 9	DPR 5	40	0.35	85	—
ETA 11	300	2.0	75	ETC 11	DPR 7	60	0.65	65	—
ETA 13	360	2.9	60	ETC 13	DPR 9	80	0.64	100	—
ETA 15	420	2.7	75	ETC 15	DPR 11	100	0.90	90	—
ETA 17	480	3.5	65	ETC 17	DPR 13	120	1.01	90	—
ETA 19	540	3.4	75	ETC 19	DPR 15	140	1.27	85	—
ETA 21	600	4.2	75	ETC 21	DPR 17	160	1.17	90	—
ETA 23	660	4.1	75	ETC 23	EPR 9	160	1.8	45	—
FTA 13	720	4.6	75	—	EPR 11	200	1.7	75	—
FTA 15	840	4.2	85	—	EPR 13	240	1.6	110	—
FTA 17	960	5.6	80	—	EPR 15	280	1.8	115	—
FTA 19	1080	5.3	90	—	EPR 17	320	2.5	95	—
FTA 21	1200	6.8	80	—	EPR 19	360	2.4	115	—
FTA 23	1320	6.5	90	—	FPS 11	415	5.2	75	—
FTA 27	1560	8.3	85	—	FPS 13	498	4.6	90	—
FTA 29	1680	8.2	90	—	FPS 15	581	4.0	105	—
FTAS 29	1800	8.2	95	—	FPS 17	664	3.7	120	—
—	720	4.0	75	FTC 13	FPS 19	747	5.5	90	—
—	840	3.9	85	FTC 15	FPS 21	830	4.9	100	—
—	960	5.5	80	FTC 17	FPS 23	913	6.4	100	—
—	1080	5.4	90	FTC 19	FPS 25	996	6.1	110	—

\* Refer to Paragraph 8.2 for use of these tables.

\*\* Specific gravity range applies to an 8 hour capacity discharge

## INSTRUCTIONS

### STATIONARY POWER BATTERY PLASTIC CELL NUMERAL APPLICATION

To insure proper adhesion of the pressure sensitive plastic cell numerals, and polarity markings supplied with your Gould Stationary Power Battery, the following procedure should be followed:

1. Numerals and polarity markings should not be applied until after the cells have been installed on the rack. It is recommended that they be applied to jar surfaces only, and not to cell covers or rack rails.

2. Clean the plastic jar surface, in the area where numeral is to be located, by using a cloth dampened with a washing soda solution. Immediately dry the area using a soft dry cloth to remove residual washing soda. **CAUTION:** Do not use any solvent type materials as they may cause damage to the plastic jar material.

3. It is a general practice to designate the positive terminal cell as #1 with succeeding cells in series in ascending order.

4. Numerals are shipped mounted on a plastic backing strip. They are easily removed by peeling back the plastic strip. Keep finger contact with adhesive backing on numeral to a minimum.

5. Locate and place numeral on side of jar, being careful that there is no conflict with electrolyte level lines or side rails of SEISMIC TYPE RACKS. For clean appearance, exercise care in numeral placement so that all numerals are in the same relative position on each cell.

Install polarity markings on the appropriate cells in the same manner.

6. Following application of cell numerals and polarity markings use a dry cloth to rub entire surface of each label to insure proper surface contact.

### TYPICAL BATTERY NAMEPLATE



NO. OF CELLS \_\_\_\_\_ TYPE \_\_\_\_\_ SERIAL NO. \_\_\_\_\_

CAPACITY \_\_\_\_\_ AMPERE HRS. AT \_\_\_\_\_ HR. RATE

SPECIFIC GRAVITY \_\_\_\_\_

GOULD INC., INDUSTRIAL BATTERY DIVISION, LANGHORNE, PA. 19047

ROI-041921



Gould Inc., Industrial Battery Division  
2050 Cabot Boulevard West, Langhorne, Pa. 19047  
Telephone (215) 752-0555

GB-3384B 5M 7/82 Printed in U.S.A. (Part No. Z99-003384)



*Electronics & Electrical Products*

## SECTION #11

### DYNAMIC CELL ANALYSIS

As described in this report, a battery three plate model was established using intermediate plates as mass points on the double cantilever beam. The busbar is treated as a center point of the cantilever. This model was analyzed using strudel software programming.

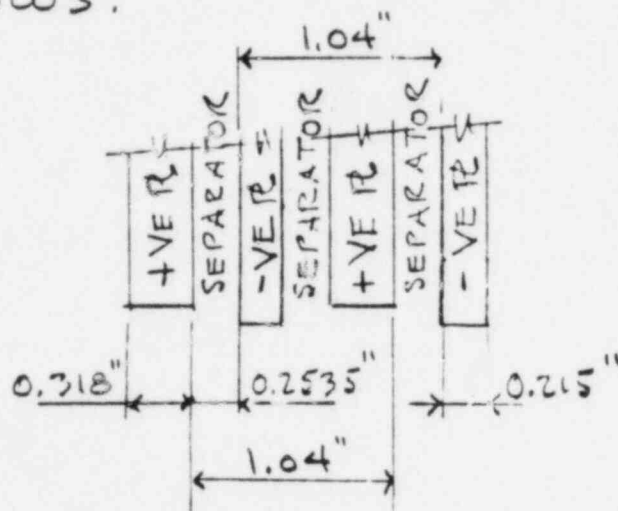
It was determined that the NCX-1680 cell has a first mode at 92.89 hz whereas the NCX-2550's first mode occurred at 105.11 hz. This analysis establishes that the NCX-1680 and NCX-2550 are similar in behavior as far as vibration is concerned.

As established in the other portion of this report, NCX-1680 and NCX-2550 are similar in design and also have the same materials for their component parts. The NCX-1680 weighs 332 lbs. whereas NCX-2550 weighs 496 lbs. The difference is due to the number of plates used in both designs. NCX-1680 uses 21 plates whereas NCX-2550 uses 35 plates.

# REPORT ON THE MATHEMATICAL DETERMINATION OF THE LOWEST NATURAL FREQUENCY OF THE INTERNAL ELEMENT OF GOULD BATTERIES NCX/NAX-1680 AND NCX/NAX-2550

## CONSTRUCTION OF THE NCX/NAX-1680 BATTERY:

THIS BATTERY CONSISTS OF 10 POSITIVE PLATES AND 11 NEGATIVE PLATES ARRANGED ALTERNATELY AS FOLLOWS:



FOR PLATE DETAILS REFER TO GOULD DRAWINGS:

105722-C-001, POSITIVE GRID  
105723-C NEGATIVE GRID

THE NEGATIVE PLATES REST ON THE BOTTOM OF THE BATTERY CASING AT TWO POINTS AND ARE INTERCONNECTED TO EACH OTHER AT THE TOP BY A COMMON TERMINAL BAR.

THE POSITIVE PLATES ARE SUPPORTED AT THE TOP AT TWO POINTS BY PLASTIC RODS AND ANGLES THAT IN TURN REST ON THE TOP OF THE NEGATIVE PLATES. EACH POSITIVE PLATE IS ALSO SUPPORTED AT THE TOP BY A COMMON TERMINAL BAR.

THE PLATES ARE CONSIDERED INFINITELY RIGID IN THE PERPENDICULAR PLANE BY VIRTUE OF THE TIGHTLY PACKED SEPARATORS. FOR DRAWINGS OF PARTS AND ASSEMBLY REFER TO GOULD DRAWINGS:

105792-B TERMINAL POST (POS)  
105853-A PLATE SUPPORT ROD  
106288-D CELL ASSEMBLY  
106303-C PLATE SUPPORT (ANGLE)

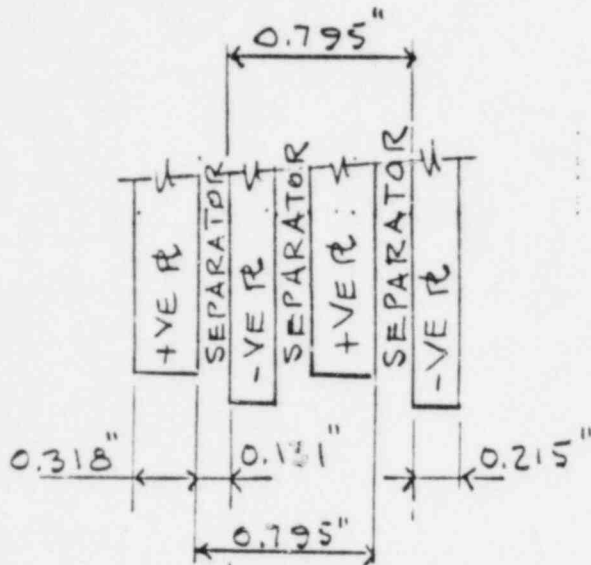
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SII #2

## CONSTRUCTION OF THE NCX/NAX-2550 BATTERY:

JOB 89

THIS BATTERY IS SIMILAR TO THE 1680 BATTERY EXCEPT THAT IT CONSISTS OF 17 POSITIVE PLATES AND 18 NEGATIVE PLATES ARRANGED ALTERNATELY AS FOLLOWS:



FOR PLATE DETAILS REFER  
TO GOULD DRAWINGS

105722-C-001, POSITIVE GR.  
105723-C NEGATIVE GR

FOR DRAWINGS OF PARTS AND ASSEMBLY REFER TO  
GOULD DRAWINGS:

105853-A PLATE SUPPORT ROD  
106302-D CELL SECTION  
106303-C PLATE SUPPORT (ANGLE)  
107123-D POSITIVE BUS BAR

## MATERIAL PROPERTIES COMMON TO BOTH BATTERIES:

### LEAD-CALCIUM

UNIT WGT. =  $691.2 \text{ #/FT}^3$ ; SUBMERGED DENSITY =  $615.4 \text{ #/FT}^3$   
 $E = 2 \times 10^6 \text{ P.S.I.}$ ; POISSON'S RATIO = 0.4

### PLASTIC ROD

UNIT WGT. =  $71.8 \text{ #/FT}^3$ ; SUBMERGED DENSITY = 0  
 $E = 0.5 \times 10^6 \text{ P.S.I.}$ ; POISSON'S RATIO = 0.42

### PLASTIC PLATE

UNIT WGT. =  $44 \text{ #/FT}^3$ ; SUBMERGED DENSITY = 0  
 $E = 0.32 \times 10^6 \text{ P.S.I.}$ ; POISSON'S RATIO = 0.42

### ELECTROLYTE

SPECIFIC GRAVITY = 1.215

## DEVELOPMENT OF STRUCTURAL MODELS REPRESENTING THE INTERNAL ELEMENTS OF EACH BATTERY.

NATURAL FREQUENCY IS DIRECTLY RELATED TO THE ELEMENT STIFFNESS. THE STIFFER THE ELEMENT IS, THE HIGHER THE NATURAL FREQUENCY WILL BE. COMPARING THE NEGATIVE PLATES WITH THE POSITIVE PLATES SUPPORT SYSTEM IT IS EVIDENT THAT THE NEGATIVE PLATES HAVE A STIFFER SUPPORT CONDITION. SINCE WE ARE INTERESTED IN THE ELEMENTS WITH THE LOWEST NATURAL FREQUENCY, ONLY THE POSITIVE PLATES WERE INVESTIGATED. ALL SOLUTIONS UTILIZED COMPUTER DYNAMIC ANALYSIS OFFERED BY GT STRUDL PROGRAMMING.

EACH POSITIVE PLATE IS SUPPORTED AT THE TOP AT THREE POINTS; AT EACH END BY THE PLASTIC ROD AND PLATE ANGLE REPRESENTING TWO SUPPORTS, THE THIRD SUPPORT BEING THE BUS BAR LOCATED NEAR ONE END SUPPORT POINT.

THE PRINCIPAL FEATURES OF THE STRUCTURAL MODEL ARE AS FOLLOWS:

1. EACH POSITIVE PLATE IS FABRICATED FROM LEAD IN A GRID PATTERN. TO SIMPLIFY OUR ANALYSIS WE CREATED AN IDEALIZED FRAME BY MAINTAINING THE GRID WEIGHT AS A CONSTANT. THE WET PASTE THAT IS ADDED TO THE GRID WAS CONSIDERED AS MASS ONLY AND NOT CONTRIBUTING TO RIGIDITY. ALL POSITIVE PLATES ARE IDENTICAL IN SIZE, SHAPE AND WEIGHT FOR BOTH BATTERIES.
2. ONLY TWO SUPPORTS WERE CONSIDERED ACTING FOR EACH POSITIVE PLATE AS A CONSERVATIVE ASSUMPTION ONE AT THE BUS BAR AND THE SECOND AT THE THE MOST DISTANT PLASTIC ROD LOCATION.
3. EACH BATTERY MODEL CONSISTED OF 3 FULL POSITIVE PLATE POSITIONED AT THE MOST FLEXIBLE LOCATIONS IN ORDER TO CONTROL COMPUTER DYNAMIC ANALYSIS WITH

R.C.D. 6-10-82

SH#4.

REASONABLE LIMITS. THE BUS BAR OF BOTH BATTERIES ARE SINGLE SPAN, DOUBLE CANTILEVER MEMBERS. FULL PLATES WERE PLACED AT THE ENDS OF EACH CANTILEVER AND THE THIRD FULL PLATE WAS POSITIONED AT MID-SPAN (BETWEEN TERMINALS) OF THE BUS BAR. SINCE THE 1680 BATTERY HAS AN EVEN NUMBER OF PLATE BETWEEN TERMINALS, THE POSITIVE PLATE NEAREST MID-SPAN POSITION WAS CONSIDERED. THE REMAINING POSITIVE PLATES FOR EACH BATTERY WAS ASSUMED AS MASS CONCENTRATIONS ALONG RESPECTIVE BUS BAR AND PLASTIC ROD LOCATIONS.

### RESULTS OF DYNAMIC ANALYSIS OF STADPL PROGRAMMING

LOWEST NATURAL FREQUENCY OF:

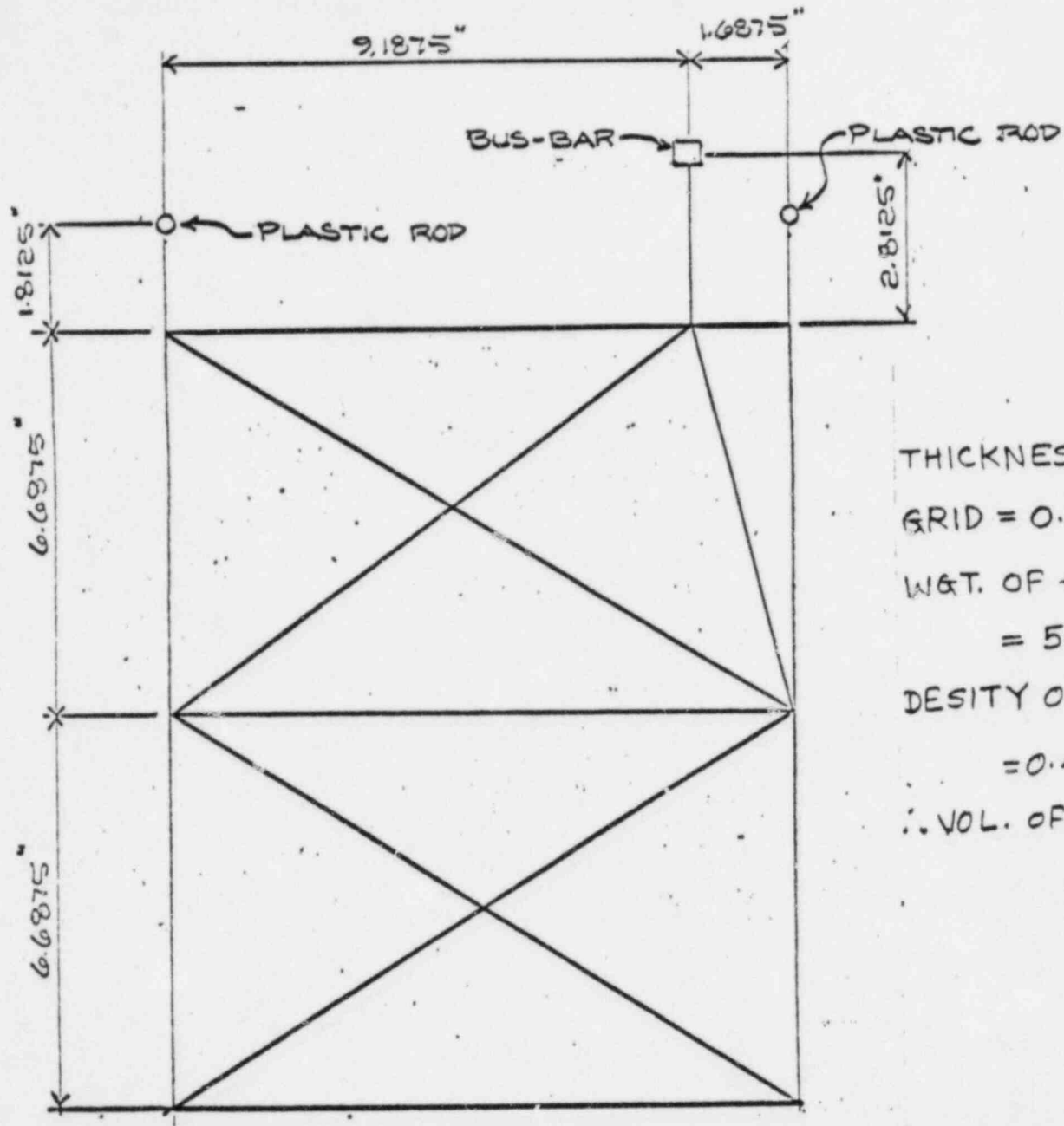
NCX-1680 BATTERY = 92.8 CYCLES/SEC.

NCX-2550 BATTERY = 105.1 CYCLES/SEC.

### CONCLUSION:

THE RIGIDITY OF THE NCX-2550 BATTERY IS GREATER THAN THE RIGIDITY OF THE NCX-1680 BATTERY.



MODEL OF POSITIVE PLATES

THICKNESS OF +VE  
GRID = 0.318"

W&T. OF +VE GRID  
= 5.9 #

DENSITY OF LEAD  
= 0.4 #/IN<sup>3</sup>

∴ VOL. OF GRID =  $\frac{5.9}{0.4}$   
= 14.75 IN<sup>3</sup>

PROPERTIES OF MEMBERS:

TOTAL LENGTH OF ALL MEMBERS =  $2 \times 15.1875 + 4 \times 12.77 + 3 \times 10.0$   
= 114"

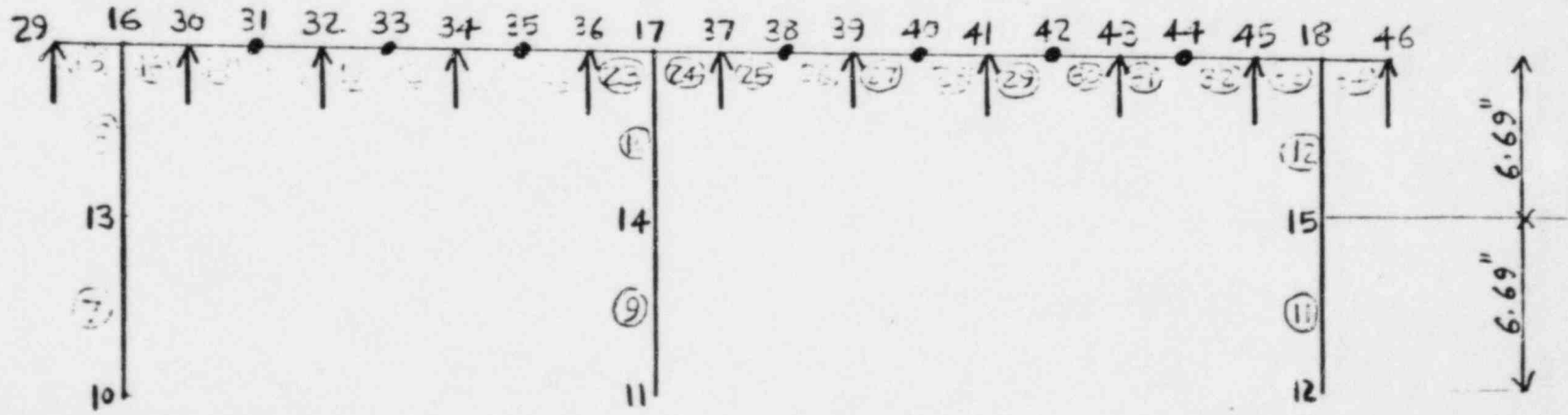
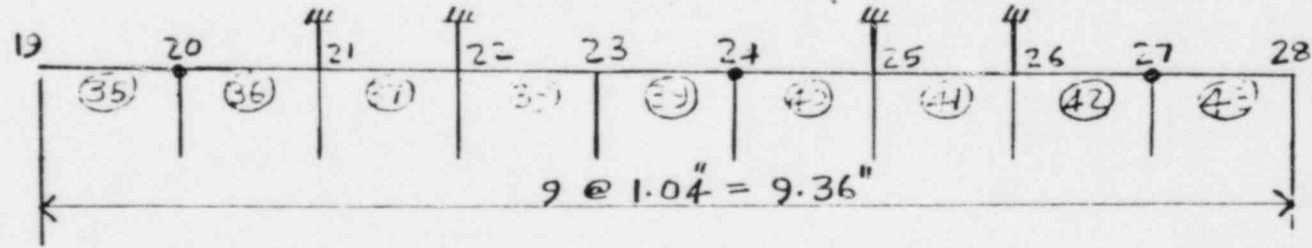
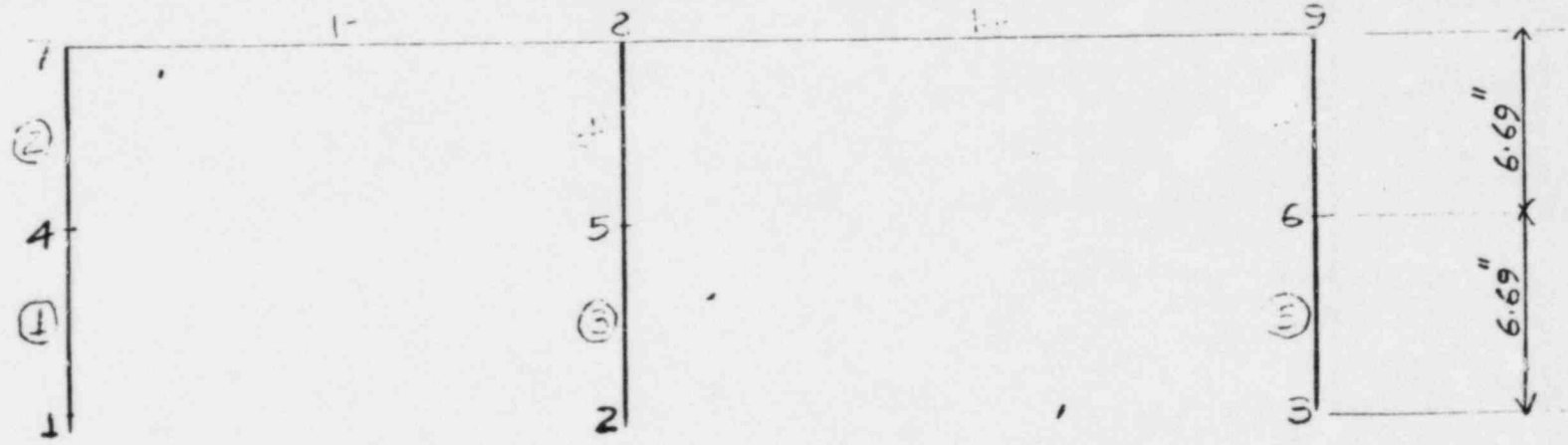
TOTAL VOL. OF ALL MEMBERS = 14.75 IN<sup>3</sup>

∴ CROSS SECTIONAL AREA OF MEMBERS =  $\frac{14.75}{114} = 0.13$  IN<sup>2</sup>

∴ EQUIVALENT WIDTH OF MEMBERS =  $0.13 / 0.318 = 0.41$  IN

SHEET # 10

JOB 890

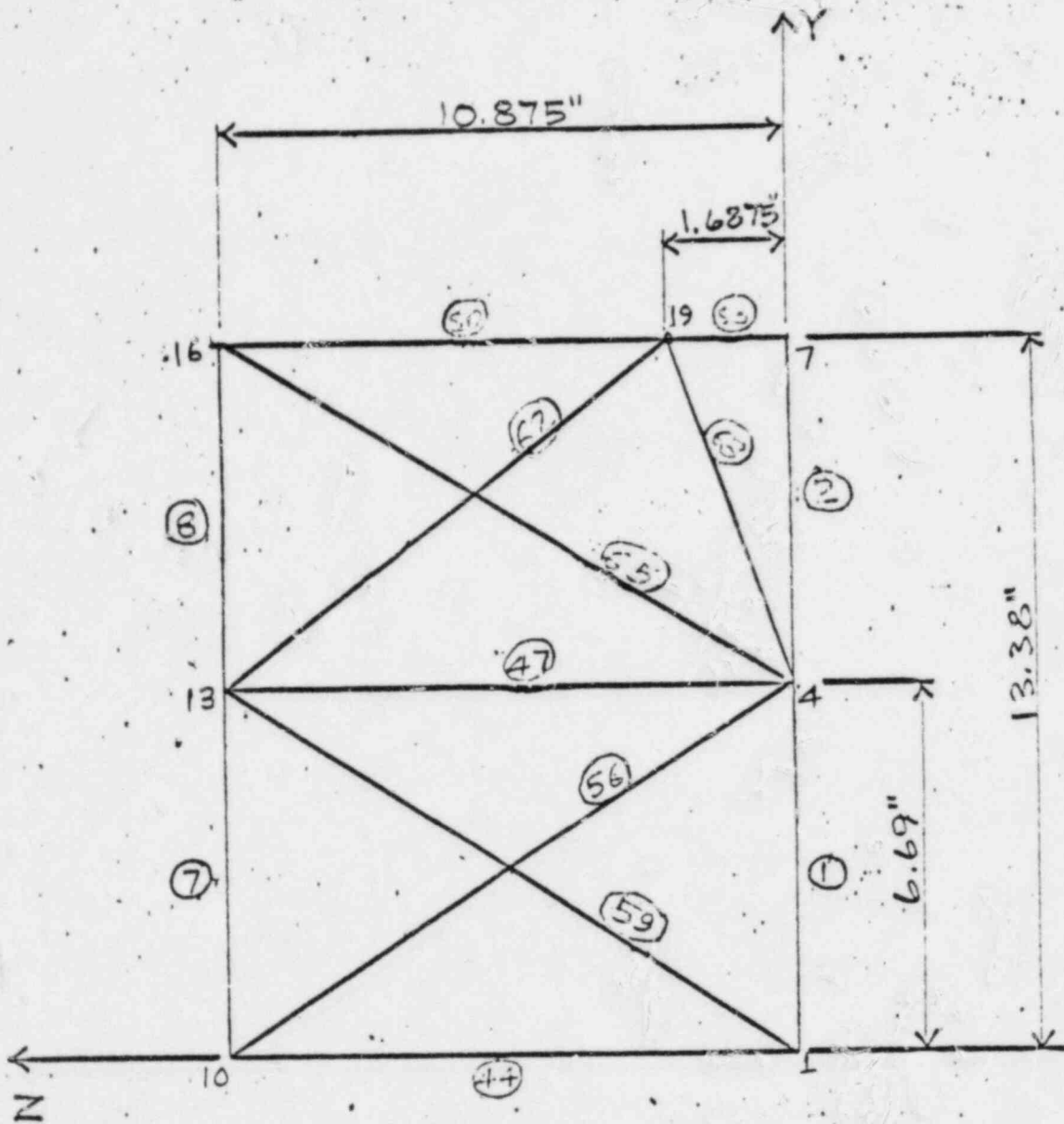


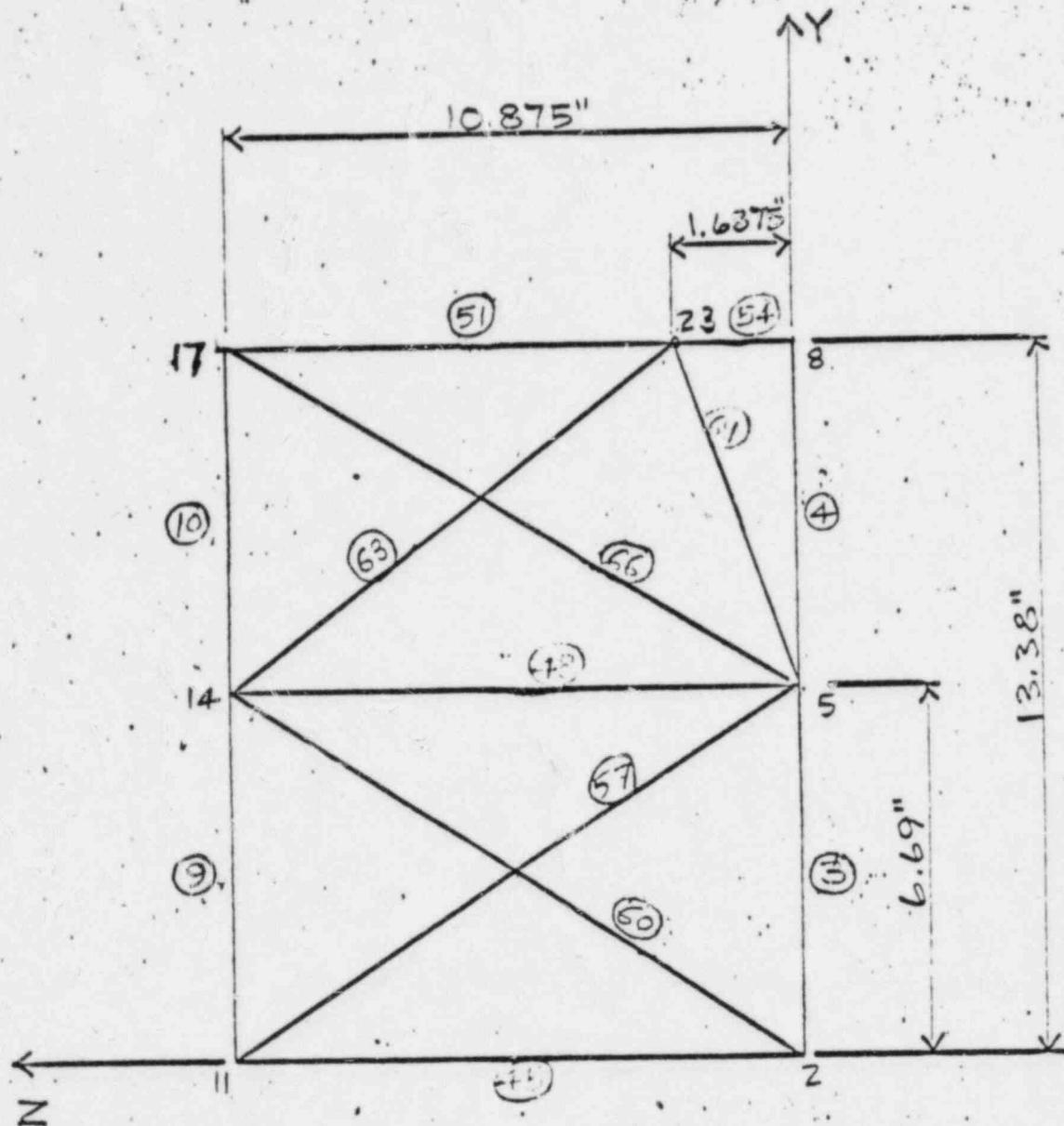
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JOB 890

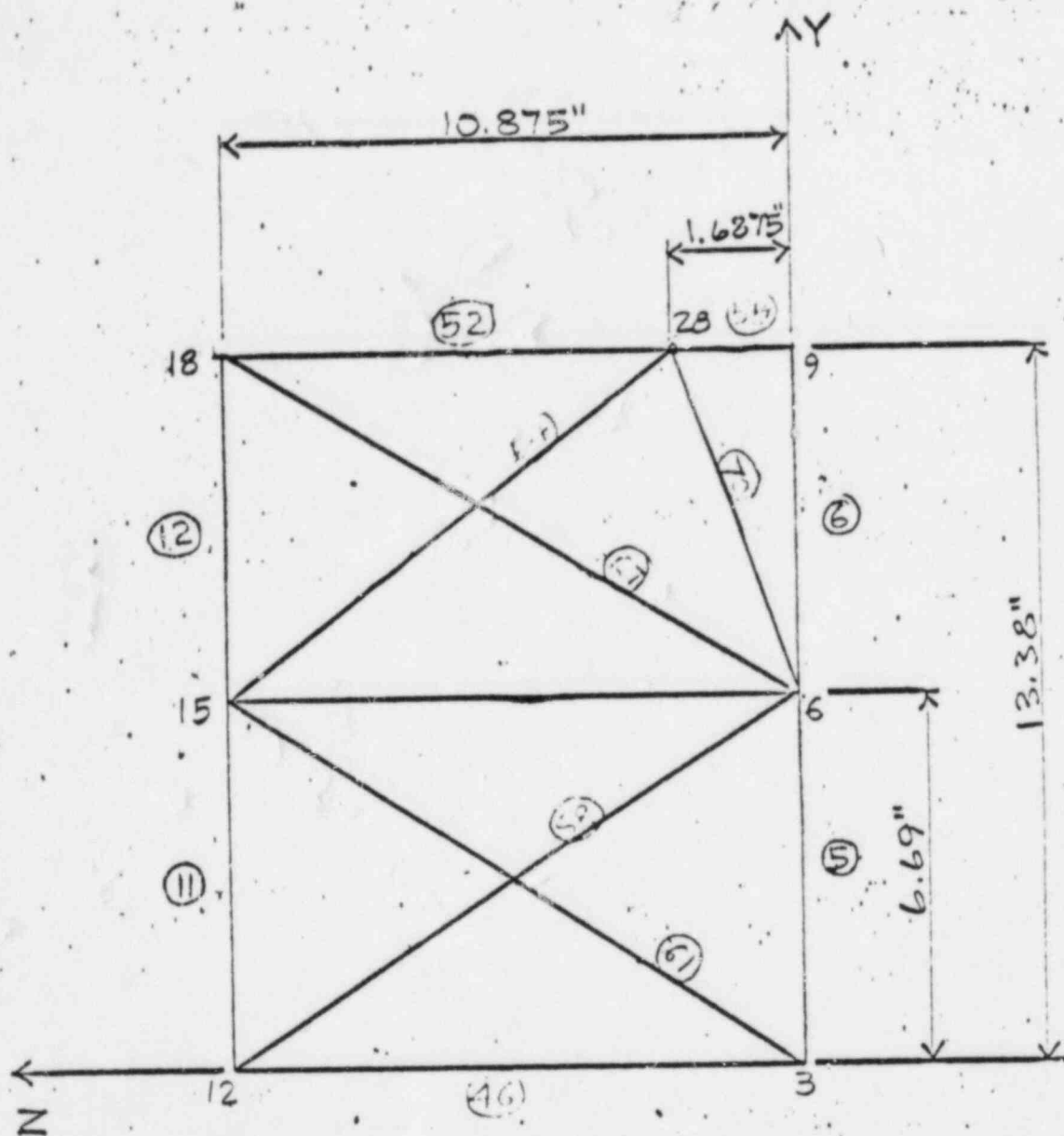


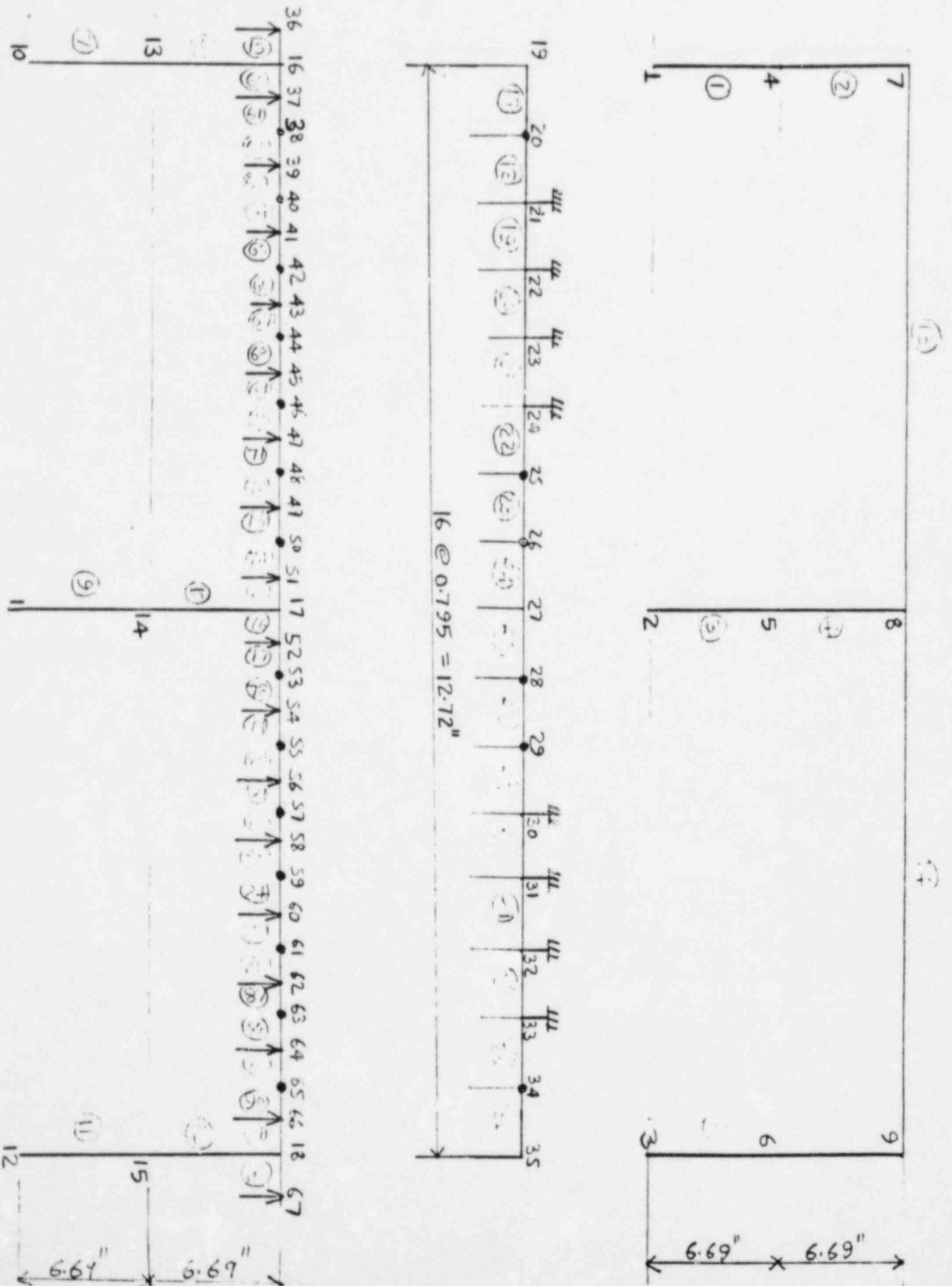


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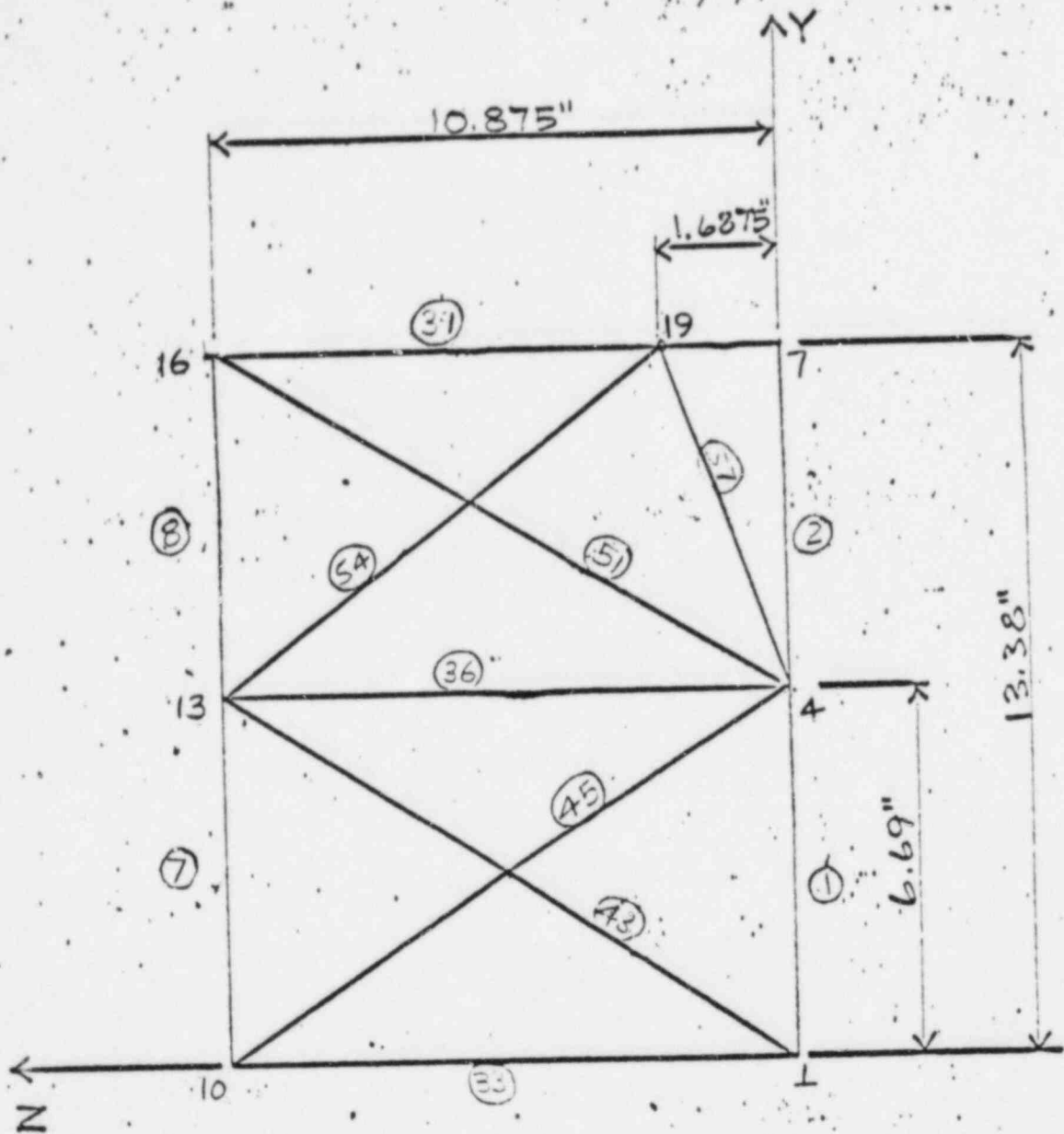




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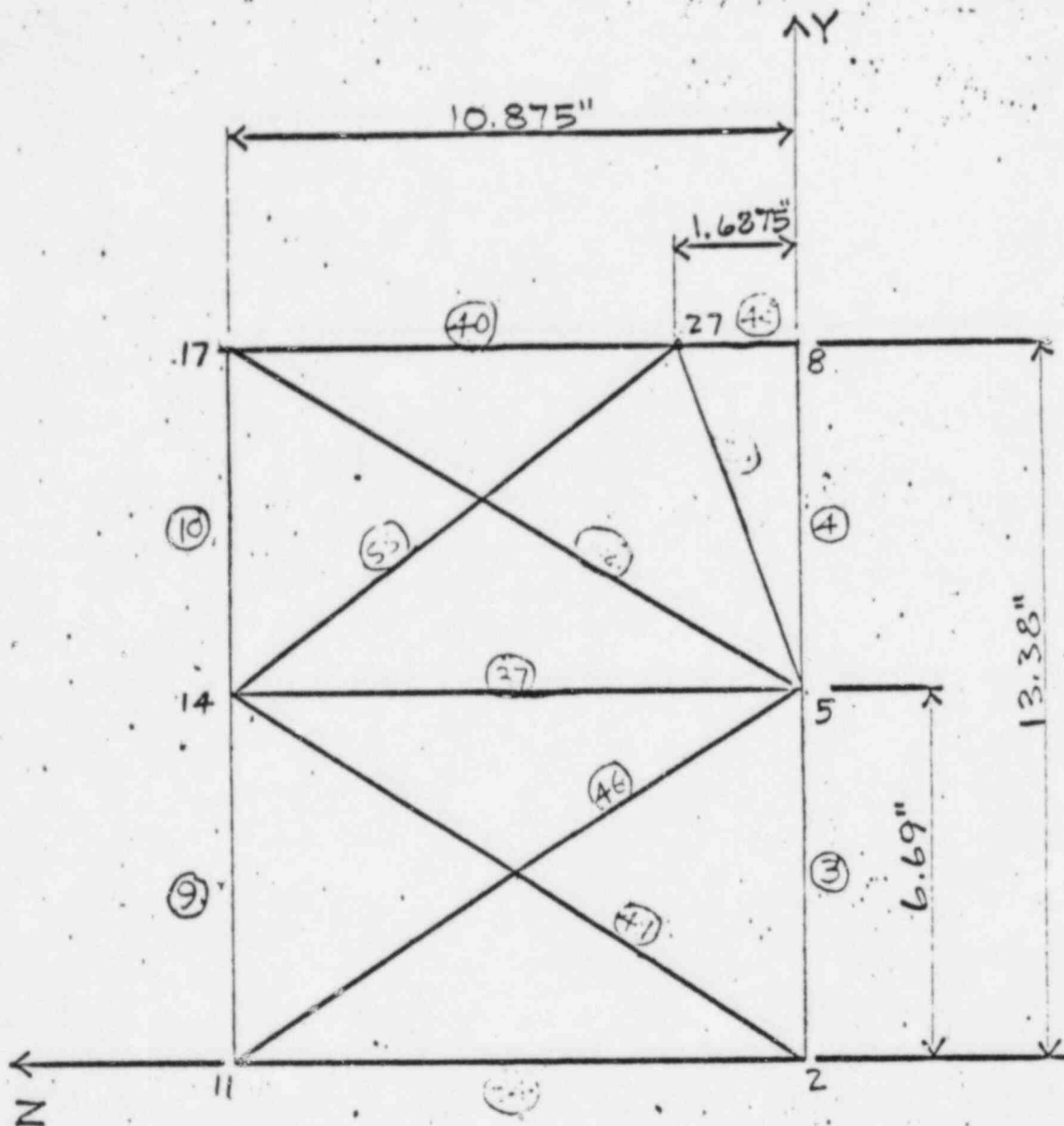
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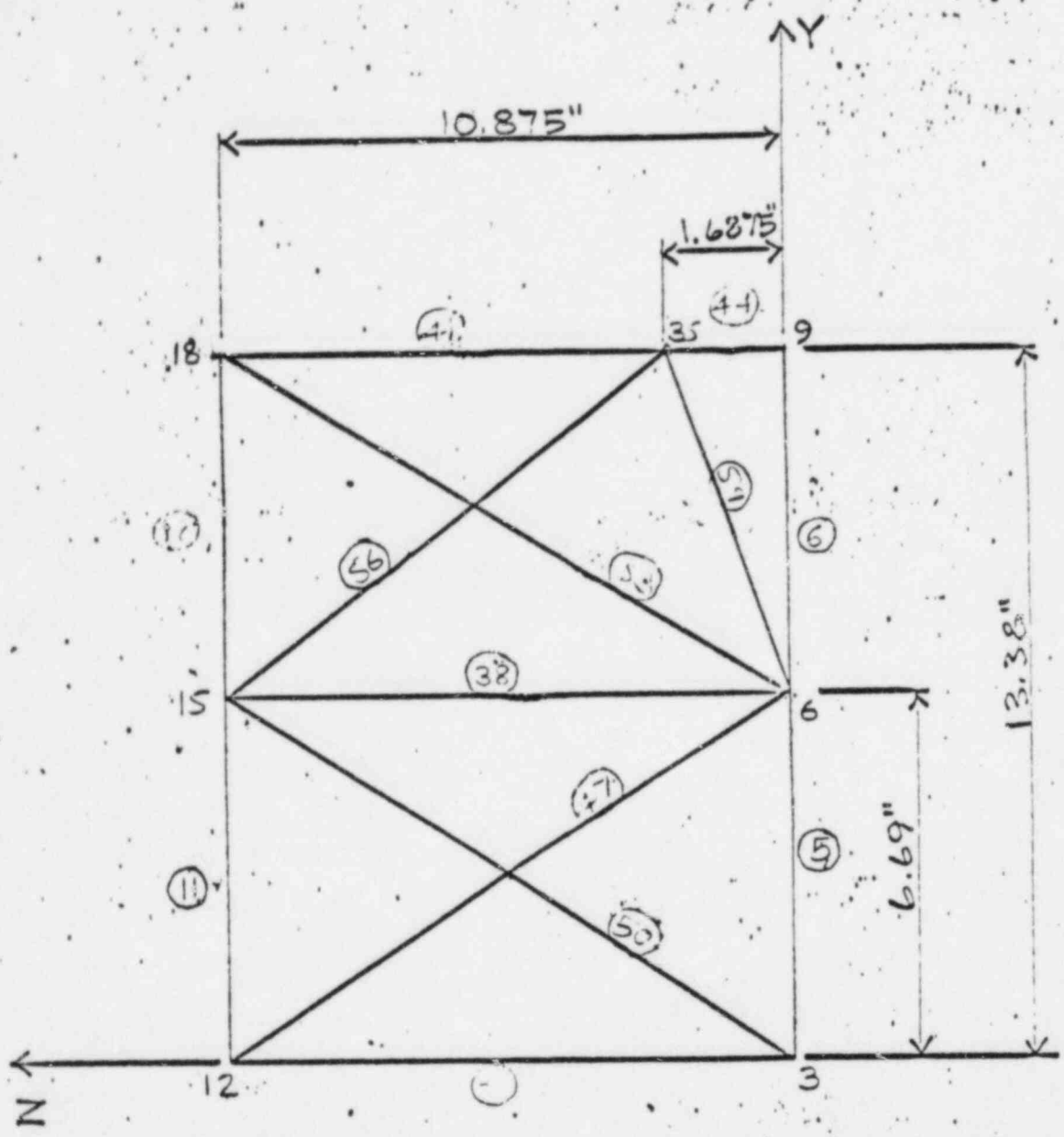
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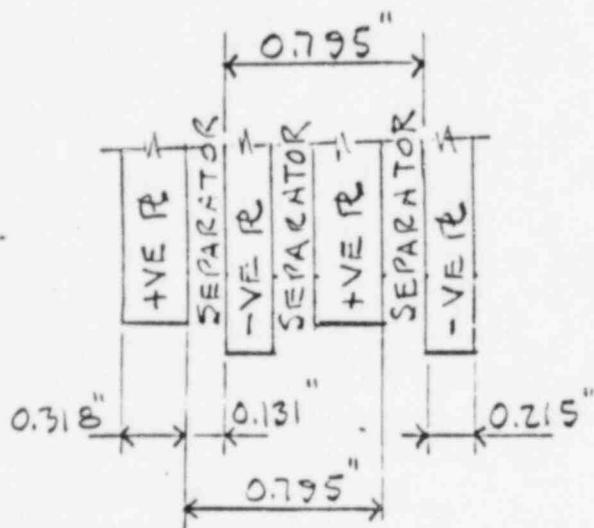
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SHEET #1 OF 1

JOB 890

REPORT ON THE MATHEMATICAL DETERMINATION OF THE  
LOWEST NATURAL FREQUENCY OF THE INTERNAL ELEMENTS  
OF GOULD BATTERY NCX/NAX-600.

CONSTRUCTION OF THE NCX/NAX-600 BATTERY:



FOR PLATE DETAILS REFER  
TO GOULD DRAWINGS:

105722-C-001, POSITIVE GK  
105723-C NEGATIVE G

THIS BATTERY CONSISTS OF 4 POSITIVE PLATES AND  
5 NEGATIVE PLATES ARRANGED ALTERNATELY AS SHOWN  
ABOVE.

THE NEGATIVE PLATES REST ON THE BOTTOM OF THE BATTERY  
CASING AT TWO POINTS AND ARE INTERCONNECTED TO  
EACH OTHER AT THE TOP BY A COMMON TERMINAL BAR.

THE POSITIVE PLATES ARE SUPPORTED AT THE TOP AT TWO  
POINTS BY PLASTIC RODS AND ANGLES THAT IN TURN REST  
ON THE TOP OF THE NEGATIVE PLATES. EACH POSITIVE  
PLATE IS ALSO SUPPORTED AT THE TOP BY A COMMON  
TERMINAL BAR.

THE PLATES ARE CONSIDERED INFINITELY RIGID IN THE  
PERPENDICULAR PLANE BY VIRTUE OF THE TIGHTLY PACKED  
SEPARATORS. FOR DRAWINGS OF PARTS AND ASSEMBLY  
REFER TO GOULD DRAWINGS:

105853-A PLATE SUPPORT ROD

107006-D SECTIONAL ASSEMBLY

106303-C PLATE SUPPORT (ANGLE)

107121-C POSITIVE BUS BAR

MATERIAL PROPERTIES:

<u>LEAD-CALCIUM</u>	UNIT WGT = $691.2 \frac{\#}{\text{FT}^3}$ ; SUBMERGED DENSITY = $615.4 \frac{\#}{\text{FT}^3}$ $E = 2 \times 10^6$ P.S.I.; POISSON'S RATIO = 0.4
<u>PLASTIC ROD</u>	UNIT WGT. = $71.8 \frac{\#}{\text{FT}^3}$ ; SUBMERGED DENSITY = 0 $E = 0.5 \times 10^6$ P.S.I.; POISSON'S RATIO = 0.42
<u>PLASTIC PLATE</u>	UNIT WGT = $44 \frac{\#}{\text{FT}^3}$ ; SUBMERGED DENSITY = 0 $E = 0.32 \times 10^6$ P.S.I.; POISSON'S RATIO = 0.42
<u>ELECTROLYTE</u>	SPECIFIC GRAVITY = 1.215

DEVELOPMENT OF STRUCTURAL MODEL REPRESENTING THE INTERNAL ELEMENTS OF THE BATTERY:

NATURAL FREQUENCY IS DIRECTLY RELATED TO THE ELEMENT STIFFNESS. THE STIFFER THE ELEMENT IS, THE HIGHER THE NATURAL FREQUENCY WILL BE. COMPARING THE NEGATIVE PLATES WITH THE POSITIVE PLATES SUPPORT SYSTEM IT IS EVIDENT THAT THE NEGATIVE PLATES HAVE A STIFFER SUPPORT CONDITION. SINCE WE ARE INTERESTED IN THE ELEMENTS WITH THE LOWEST NATURAL FREQUENCY, ONLY THE POSITIVE PLATES WERE INVESTIGATED. THE SOLUTION UTILIZES COMPUTER DYNAMIC ANALYSIS OFFERED BY GT STRUDL PROGRAMMING.

THE PRINCIPAL FEATURES OF THE STRUCTURAL MODEL ARE AS FOLLOWS:

1. EACH POSITIVE PLATE IS FABRICATED FROM LEAD IN A GRID PATTERN. TO SIMPLIFY OUR ANALYSIS WE CREATED AN IDEALIZED FRAME BY MAINTAINING THE GRID WEIGHT AS A CONSTANT. THE WET PASTE THAT IS ADDED TO THE GRID WAS CONSIDERED AS MASS ONLY AND NOT CONTRIBUTING TO RIGIDITY. THE POSITIVE PLATES FOR THE BATTERY ARE IDENTICAL IN SIZE, SHAPE AND WEIGHT TO

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ALL THE N-LINE BATTERY CELLS.

ONLY TWO SUPPORTS WERE CONSIDERED ACTING FOR EACH POSITIVE PLATE AS A CONSERVATIVE ASSUMPTION; ONE AT THE BUS BAR AND THE SECOND AT THE MOST DISTANT PLASTIC ROD LOCATION.

THE BATTERY MODEL CONSISTS OF 3 FULL POSITIVE PLATES. TWO ARE POSITIONED AT THE EXTREME ENDS OF THE BUS BAR AND ONE AT AN INTERIOR LOCATION. THE REMAINING POSITIVE PLATE WAS ASSUMED AS A MASS CONCENTRATION ALONG RESPECTIVE BUS BAR AND PLASTIC ROD LOCATIONS. THE PRIMARY REASON FOR MODELING 3 FULL POSITIVE PLATES FOR THIS BATTERY IS TO MATCH THE SELECTED MATHEMATICAL MODEL OF THE 1680 AND 2550 BATTERIES SO THAT A CREDITABLE COMPARISON COULD BE MADE.

### RESULTS OF DYNAMIC ANALYSIS OF STRUDL PROGRAMMING:

LOWEST NATURAL FREQUENCY FOR THE

NCX-600 BATTERY = 112.8 CYCLES PER SECOND

### COMPARISON WITH THE NCX-1680 BATTERY:

LOWEST NATURAL FREQUENCY FOR THE

NCX-1680 BATTERY = 92.8 CYCLES PER SECOND

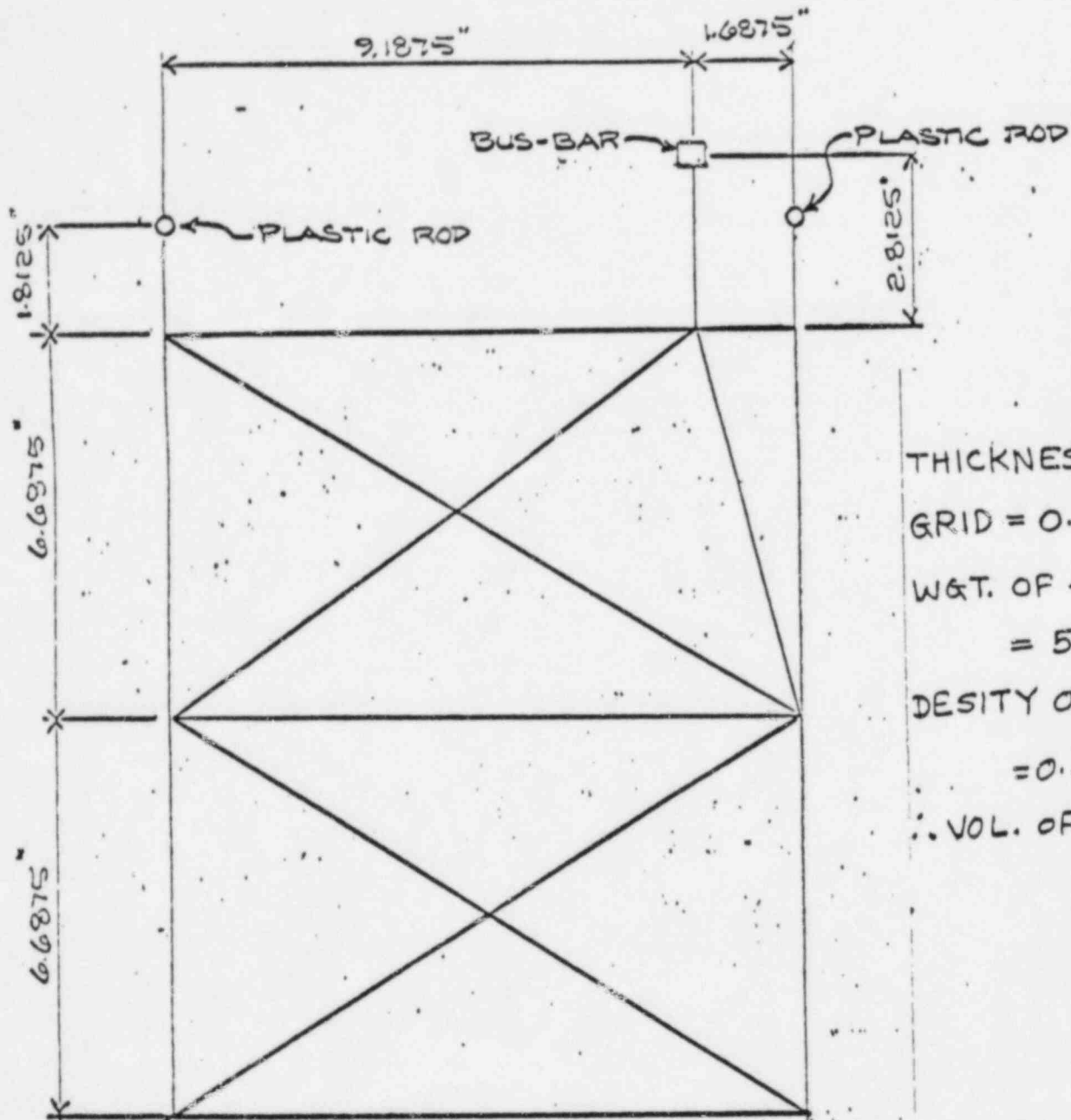
THEREFORE THE RIGIDITY OF THE NCX-600 BATTERY IS GREATER THAN THE NCX-1680 BATTERY.



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# MODEL OF POSITIVE PLATES



THICKNESS OF +VE  
GRID = 0.318"

WGT. OF +VE GRID  
= 5.9 #

DENSITY OF LEAD  
= 0.4 #/in<sup>3</sup>

∴ VOL. OF GRID =  $\frac{5.9}{0.4}$   
= 14.75 IN

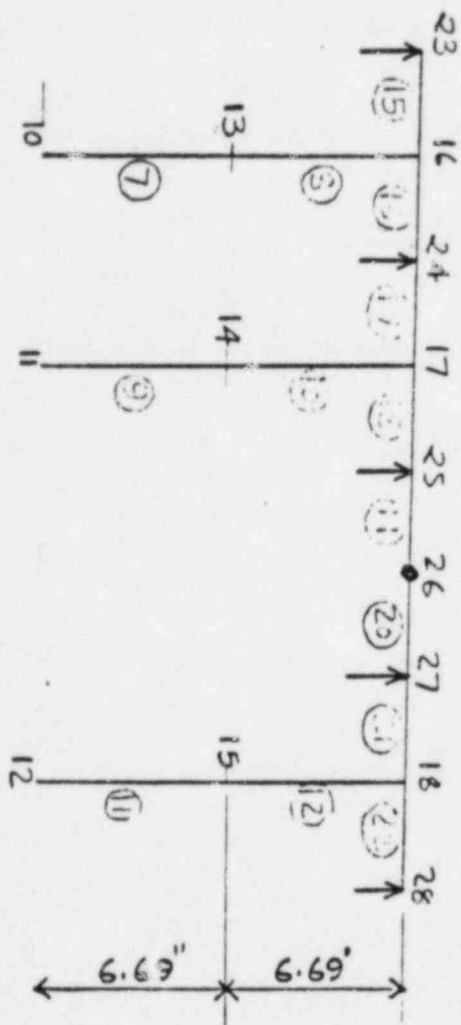
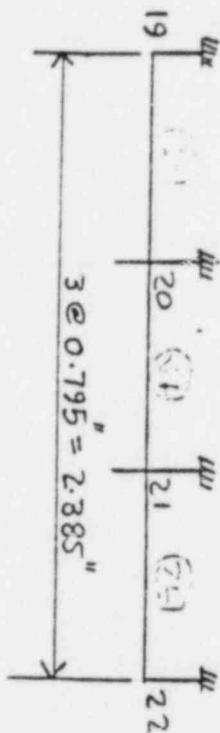
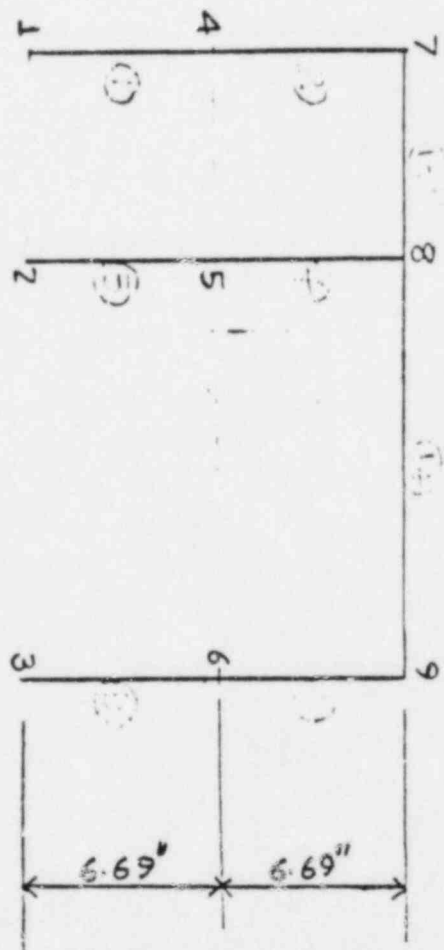
## PROPERTIES OF MEMBERS:

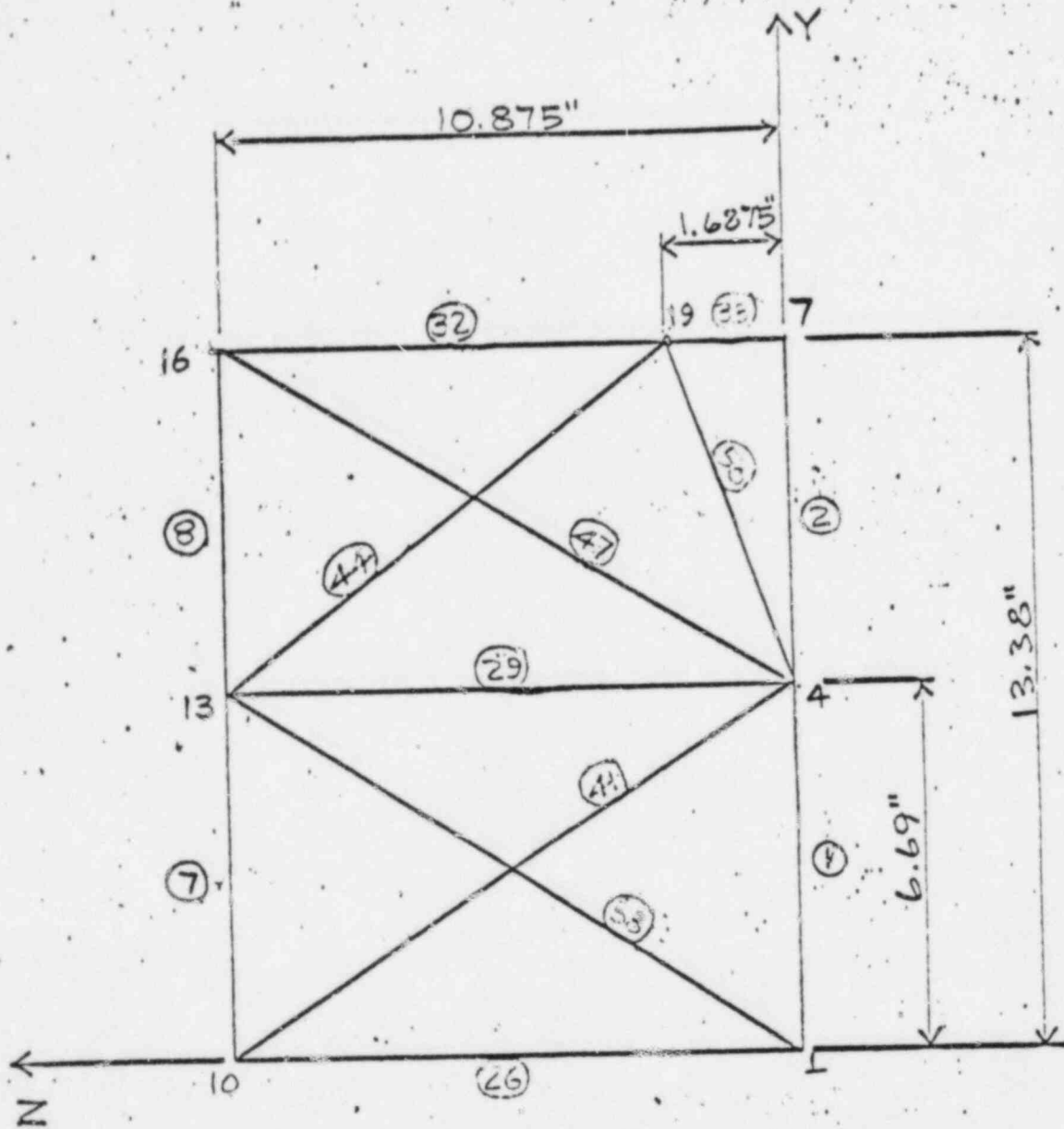
TOTAL LENGTH OF ALL MEMBERS =  $2 \times 15.1875 + 4 \times 12.77 + 3 \times 10$   
= 114"

TOTAL VOL. OF ALL MEMBERS = 14.75 IN<sup>3</sup>

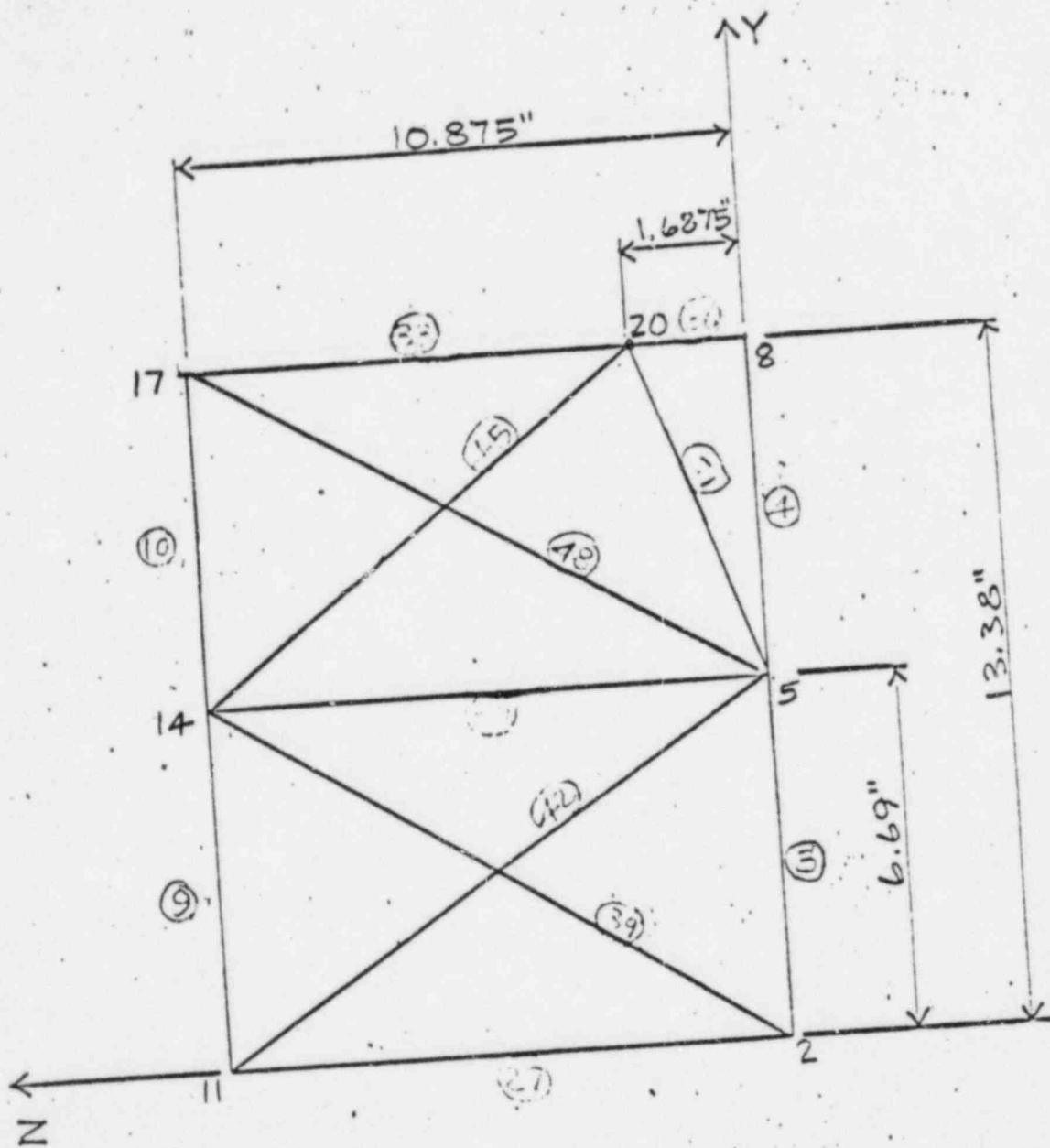
∴ CROSS SECTIONAL AREA OF MEMBERS =  $\frac{14.75}{114} = 0.13$  IN

∴ EQUIVALENT WIDTH OF MEMBERS =  $0.13 / 0.318 = 0.41$  IN

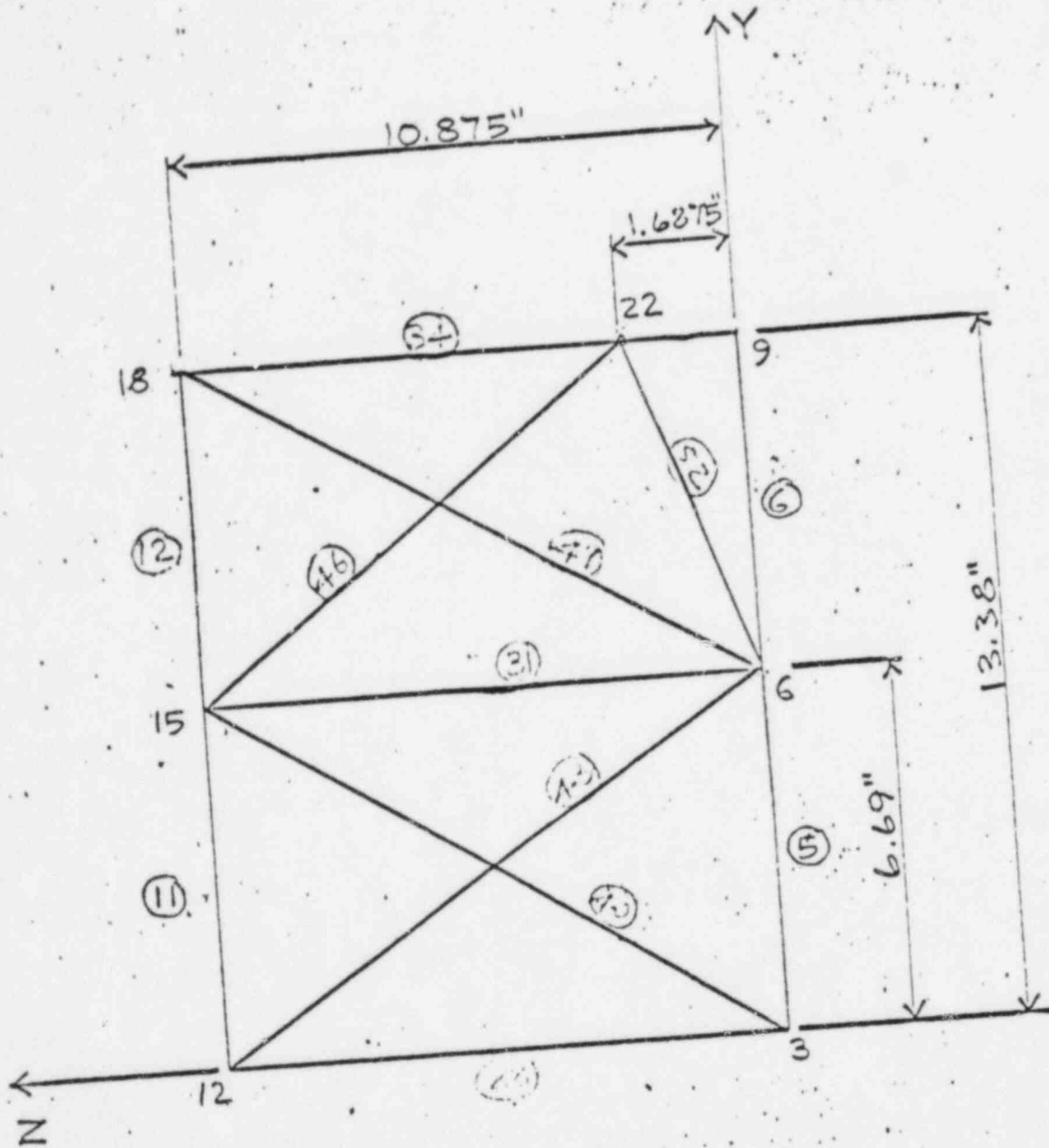




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## SECTION #12

### CONCLUSIONS

The primary objective of this test program was to demonstrate capability of our batteries and racks in accordance with the following documents in general and Sargent & Lundy Specification F/L 2819 for the Commonwealth Edison Company Byron/Braidwood Project in particular.

1. ANSI/IEEE Std 279-1971, Criteria for Protection Systems for Nuclear Power Generating Stations.
2. IEEE Std 308-1978, Criteria for Class 1E Power Systems for Nuclear Power Generating Stations.
3. IEEE Std 323-1974, Standard for Qualifying Class 1E Equipment for Nuclear Power Generating Stations.
4. ANSI/IEEE Std 450-1975, Recommended Practice for Maintenance, Testing, and Replacement of Large Lead Storage Batteries for Generating Stations and Substations.
5. ANSI/IEEE Std 484-1975, Recommended Practice for Installation Design and Installation of Large Lead Storage Batteries for Generating Stations and Substations.
6. ANSI/IEEE Std 344-1975, Recommended Practice for Seismic Qualification of Class 1E Equipment for Nuclear Power Generating Stations.
7. IEEE Std 380-1975, Definitions of Terms Used in IEEE Standards on Nuclear Power Generating Stations.
8. IEEE Std 485-1978, Recommended Practice for Sizing Large Lead Storage Batteries for Generating Stations and Substations.
9. American Institute of Steel Construction (AISC) Standard Specification for the Design, Fabrication and Erection of Structural Steel for Buildings, New York, 1965.
10. IEEE Std 535-1979 IEEE, Standard for Qualification of Class 1E Lead Storage Batteries for Nuclear Power Generating Stations.



Sections #1 and #7 of this test report describe the methodology used in carrying out this test program.

Sections #2 and 3 demonstrate the origin as well as the environmental and operating conditions to which the naturally aged test specimens were exposed.

Section #4 demonstrates that the naturally aged test specimens and those cells being supplied on the Commonwealth Edison Co./Sargent & Lundy contract for Byron/Braidwood Project are of the same design and materials.

Section #5 demonstrates that the test fixture used for this test program is adequately strong for the Gould required response spectra.

Section #6 demonstrates that the racks supplied under Commonwealth Edison Co./Sargent & Lundy Engineers contract for Byron/Braidwood Project are also adequately strong and are rigid structures having natural frequency of vibrations above 33 hz.

Section #7 summarizes the test series involving all naturally aged specimens. Each test series is comprised of pre and post seismic capacity discharge tests and random multi-frequency vibration test.

Sections #8 demonstrates that the test response spectra envelopes the required response spectra of Gould as well as Commonwealth Edison Co./Sargent & Lundy Engineers contract for the Byron/Braidwood -- all frequencies at 2% damping.

Section #9 contains two Wyle test reports -- No. 44681-1 and 44681-2. 44681-1 demonstrates the capability of Gould plante' type of batteries. This report is not part of this documentation. 44681-2 demonstrates the capability of Gould

calcium flat pasted plate type of batteries. These types of batteries are supplied under Commonwealth Edison Co./Sargent & Lundy Engineers contract for Byron/Braidwood Project.

The complete report, as described above, describes, documents and demonstrates that the batteries supplied by the Industrial Battery Division of Gould, Inc. under Commonwealth Edison Co. contract for Byron/Braidwood Project are capable of meeting with Sargent & Lundy Engineers Specification F/L 2819. The NCX type is qualified for a period of (10) ten years when operated in a yearly average temperature of 77°F and when installed and operated in accordance with IEEE 484-1981, IEEE 450-1980 and Gould Installation and Operating Instruction GB-3384B 6/80.

The Industrial Battery Division of Gould Inc. intends to pursue an ongoing qualification program to determine maximum qualified life beyond currently established 10 year qualified life for NCX type.