



**INTERNATIONAL
URANIUM (USA)
CORPORATION**

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December 16, 1997

Via Overnight Mail

Mr. Joseph J. Holonich, Branch Chief
High Level Waste and Uranium Recovery Projects Branch
Division of Waste Management
Office of Nuclear Regulatory Commission
2 White Flint North, Mail Stop T-7J9
11545 Rockville Pike
Rockville, MD 20852

Re: NRC Letter dated August 19, 1997, and December 5, 1997, regarding NRC Acceptance
Review of the Reclamation Plan for the White Mesa Uranium Mill
Source Material License SUA-1358
Docket No. 40-8681

Dear Mr. Holonich:

Enclosed please find our responses to the August 19, 1997 and December 5, 1997 NRC comments to the White Mesa Reclamation Plan. This plan was submitted to the NRC by International Uranium (USA) Corporation on February 28, 1997.

These responses were developed following telephone conference with NRC staff and contractors and will hopefully address all of NRC's questions.

If you have any questions on the enclosed responses, or require additional clarification, please feel free to contact Michelle Rehmann or me at the letterhead phone or address.

Very truly yours,

Harold R. Roberts
Executive Vice President

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HRR/pl
Enclosures

cc: Earl E. Hoellen
David C. Frydenlund
William N. Deal
Michelle R. Rehmann



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Responses to NRC Letter Dated August 19, 1997 and December 5, 1997, Regarding NRC
Acceptance Review of the Reclamation Plan for the White Mesa Uranium Mill

Source Material License SUA-1358
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**Response to Nuclear Regulatory Commission / White Mesa Mill Reclamation Plan
Comments Dated August 19, 1997
Source Material License SUA-1358**

Comment:

- (1) Information to support a review of the acceptability of the radon barrier design.
Specifically:**

- (i) A description of the materials to be used for the radon barrier and as random fill**

Response:

The materials to be used for the radon barrier and as random fill are the clay and soil and rock derived from sandstone (random fill), respectively, that are available within the site boundaries. These materials were tested as part of the original tailings reclamation plan design. Based on empirical data collected to date on the cover material placed over portions of Cells 2 and 3, the random fill material alone provides an effective barrier to radon flux. Following are brief descriptions of the clay and random fill materials.

Clay Material

Laboratory testing of the clay material was performed following a field investigation of the Section 16 clay source. The laboratory results are presented in a letter report to Energy Fuels Nuclear, Inc., from D'Appolonia Consulting Engineers, Inc. entitled "Section 16 Clay Material Test Data, White Mesa Uranium Project, Blanding, Utah" dated March 8, 1982. This letter report is presented in Attachment 1 of this response.

Based on the above-mentioned report, the Section 16 clay source contains soils classified according to the Unified Soils Classification System (USCS) as CL, CH, SC, SM, and ML. The material is a fine grained soil with varying amounts of silt and clay particles. The plasticity of the soil samples ranged from non-plastic to highly plastic.

Random Fill

Random fill material consists of soil and rock derived from sandstone and obtained from excavations within the site boundaries. The soil and rock have been stockpiled on the site for future reclamation use. Soils in these stockpiles can be described generally as a granular soil with relatively low plasticity (Tailings Cover Design, Appendix A, Table 3.4-1, indicates Plasticity Index of 7, and 48 percent passing the No. 200 sieve). However, they cannot be representatively classified according to the USCS as the piles contain highly variable amounts of clay, silt, sand and gravel. Additionally, the piles contain varying amounts of sandstone cobbles and boulders which may be screened during reclamation and utilized as riprap.

Random fill material, obtained from the on-site stockpiles, currently covers portions of the tailings piles in Cells 2 and 3. The portions of these cells are covered with three to four feet of random fill. Radon flux measurements, presented in Section 1.1.2 of the Tailings Cover Design, indicate radon flux through the random fill material of less than 20 pCi/m²/second. These empirical data indicate that the random fill material alone is currently providing an effective barrier to radon flux.

(ii) *An analysis to show that each type of material is available in sufficient quantity*

Response:

Analysis of the quantity of the clay and random fill material demonstrates that sufficient material is available for constructing the cover.

Clay Layer

The reclamation construction estimate requires 259,100 cubic yards of clay material for construction of the clay layer. Based on analysis of the field investigation data, quantities in excess of 1.8 million cubic yards of clay are available. This quantity is more than six times the quantity of clay needed for the cover.

The field investigation data are presented in a letter report to Energy Fuels Nuclear, Inc., from D'Appolonia Consulting Engineers, Inc. entitled "Section 16 Clay Material Test Data, White Mesa Uranium Project, Blanding, Utah" dated March 8, 1982. This letter report is presented in Attachment 1 of this response.

The clay materials were identified in the Section 16 area encompassed by borings B-100, B-103, B-104 and B-106 as shown on Figure 1 contained within Attachment 1 of this response. The surface area between these borings is approximately 1,160,000 square feet. The average depth of CL and CH materials encountered in the seven borings is approximately 44 feet as indicated on Figure 2 contained within Attachment 1 of this response. Based on the surface area and average depth estimated from Figures 1 and 2, the quantity of clay material available is approximately 1.8 million cubic yards.

Random Fill

The reclamation construction estimate requires 1,547,500 cubic yards of random fill material to complete the cover. The material will be supplied from the random fill stockpiles generated from excavation of the cells for the tailings facility. The location of these stockpiles is shown on Figure 3.2.1 of the Reclamation Plan. According to planimetric measurements, these stockpiles contain in excess of 9 million cubic yards of random fill material, which is more than five times the quantity required for the cover.

(iii) An estimate of the potential for cover cracking due to shrinkage

Response:

The potential for cover cracking due to shrinkage is considered to be negligible for two reasons: First, the random fill material is a granular soil with low plasticity, which will inhibit cracking. Second, the clay layer will be overlain by a minimum of two feet of compacted random fill, which will prevent moisture loss from the clay.

Random Fill

Cover cracking due to shrinkage of the random fill material will not be a concern, as the random fill consists of soil and rock derived from sandstone and is generally a granular soil with very low plasticity (Tailings Cover Design, Appendix A, Table 3.4-1, indicates Plasticity Index of 7, and 48 percent passing the No. 200 sieve). These material properties will inhibit cracking.

During final reclamation, the stockpiles of random fill may contain isolated pockets of material classified as CL according to the Unified Soils Classification System. Clayey materials encountered will be mixed with the granular soils to minimize the risk of surface cracking.

Clay Layer

The clay layer will be overlain by a minimum of two feet of compacted random fill which will protect the clay layer from cracking.

In order to determine the potential of clay layer cracking, the performance of the soils cover was evaluated using the Hydrologic Evaluation of Landfill Performance (HELP) Model. Weather data were input using the default parameters from Grand Junction, Colorado. Grand Junction is located northeast of Blanding, Utah in a similar climate and elevation.

Based on these values the HELP model estimates an evaporative zone depth of 18-inches on barren soil. The surface of the soil cover will be considered barren as the riprap cover layer will inhibit vegetative growth. Therefore, the moisture content of the clay layer, and six inches of random fill overlying the clay layer, will be protected from moisture changes which may induce shrinkage.

This is a conservative estimate as the depth of the riprap layer, which will provide additional protection from evaporation of the underlying soils, was not considered in the HELP model evaluation.

(iv) *A delineation of measures that will be taken to prevent burrowing animals from penetrating the radon barrier*

Response:

No measures short of continual annihilation of target animals can prevent burrowing. However, reasonable measures will discourage burrowing including:

- Total cover thickness of at least six-feet;
- Compaction of the upper three feet of soil cover materials to a minimum of 95-percent, and the lower three feet to 80-90 percent, based on a standard Proctor (ASTM D-698); and
- Riprap placed over the compacted random fill material.

Comment:

(2) *An analysis of the total and differential settlements of the tailings surface and the effects of such settlements on soil cover integrity.*

Response:

Analysis of the total and differential settlement of the tailings surface was performed using the survey data compiled for the five settlement monuments installed within Cell 2. These monuments were installed on the surface of the tailings, between August 1989 and November 1992, prior to the placement of approximately three to four feet of random fill material. Survey results of each monument were plotted as time vs. settlement graphs and are presented in Attachment 2 of this response.

Review of the survey data and graphs shows that primary consolidation of the tailings has occurred, and the settlement rate has been very slow with minor total values. The maximum total settlement recorded is 0.97 feet, and the differential settlement is on the order of 0.50 feet. Approximately 81 percent of the total measured settlement occurred within the initial three years. Additionally, recent survey data indicates that the settlement has virtually stopped. From September 1995 to September 1997 the settlement averaged less than 0.03 feet per year, which is within the expectable survey error of 0.05 feet. This indicates that the settlement since 1995 is negligible.

Placement of the final reclamation cover will induce additional settlement. However, since primary consolidation of the tailings has occurred, under current loading conditions, additional settlement is expected to be less than the total settlement values to date. This minor amount of additional settlement, due to the added surcharge of the cap, will not effect the integrity of the six foot thick final reclamation cover.

Comment:

- (3) *An analysis of the liquefaction potential of subsurface materials and uranium mill tailings.*

Response:

Analysis of the liquifaction potential of the subsurface materials and mill tailings was performed utilizing the data from the Tailings Cover Design and the Reclamation Plan. The results of the analysis indicate that the subsurface materials have no liquifaction potential because they consist of unsaturated bedrock.

Liquifaction of the tailings would be a concern only if the cover material cracked to allow a path for additional moisture to enter the tailings and/or provide a potential escape path for liquefied tailings. However, the tailings placement methods have consolidated the sands to the maximum extent possible which will inhibit the potential for cover cracking.

Subsurface Materials

Information about the subsurface materials was obtained from boring logs compiled in Appendix G of the Tailings Cover Design, which contains Section 2 of the subsurface investigation report prepared by Chen and Associates, Inc.; entitled "Soil Property Study, Earth Lined Tailings Retention Cells, White Mesa Uranium Project, Blanding, Utah", dated July 18, 1978. The bore logs indicate that the bottom of the cells are within the unsaturated Dakota Sandstone bedrock. Therefore, liquefaction of the subsurface materials is not possible.

Mill Tailings

The liquifaction potential of the mill tailings was analyzed using the data on the tailings properties and operational data concerning placement of the tailings. Information about the properties of the mill tailings was obtained from Page 2-4, Section 2.2.3 of the Reclamation Plan. Operational procedures were obtained from Page 2-5, Section 2.2.3.1 of the Reclamation Plan.

As discussed in the Reclamation Plan, the tailings produced by the mill typically contain 30 percent moisture by weight, have an in-place density of 74.2 pounds per cubic foot and have a size distribution with a predominant -325 mesh size fraction. These tailings properties indicate that the material is potentially liquefiable.

Although the tailings properties indicate that the material is potentially liquefiable the method of tailings placement substantially reduces the potential for liquifaction.

Slurry disposal has taken place in both Cells 2 and 3. Tails placement in Cell 2 was accomplished by a perimeter discharge method with discharge points around the east, north, and west boundaries of the cell. The advantage of this method is that maximum beach stability is achieved by allowing tailings sands to interfinger with slimes during placement.

During slurry disposal, solutions from Cell 2 were decanted and pumped back to Cell 1 for evaporation, to minimize net water gains. Additionally, spray systems were utilized to enhance evaporation rates. These processes allowed maximum drainage from the sands.

Tailings placement in Cell 3 is accomplished with a final grade method. The discharge points are set up in the east end of the cell and the final grade surface is advanced to the slimes pool area. When the slimes pool is reached, the discharge points are moved to the west end of the cell and worked back to the middle of the cell. The advantage of this method is that maximum beach stability is achieved by allowing water to drain from the sands to the maximum extent, and by allowing coarse sand deposition to help provide stable beaches. Additionally, solution is recycled from the active cells to the maximum extent possible.

The tailings placement methods have consolidated the sands to the maximum extent possible during active operation. This consolidation will minimize the risk of liquifaction as well as minimize the effect of cover settlement which will in turn inhibit cover cracking. Cover cracking would be the only viable pathway for the release of liquefied tailings as the tailing cells are fully contained with non-liquefiable embankments.

Comment:

- (4) *The locations and depths of the samples used for estimating the properties of the tailings and cover materials in the laboratory, along with details of the laboratory results. Moreover, the standards and/or procedures used to collect the samples and to measure the specific properties should be identified.***

Response:

The soil cover design consists of two material types readily available within the site boundaries. The cover consists of a clay layer and a random fill layer.

Clay Layer

Locations, depths, and sampling methods of the samples used for estimating the properties of the clay layer are presented in a letter report to Energy Fuels Nuclear, Inc., from D'Appolonia Consulting Engineers, Inc. entitled "Section 16 Clay Material Test Data, White Mesa Uranium Project, Blanding, Utah" dated March 8, 1982. This letter report is presented in Attachment 1 of this response.

Random Fill

Random fill and tailings testing was performed by Chen and Associates (1987) and the results of the laboratory testing program are presented in Appendix A of the Tailings Cover Design.

Comment:

- (5) *A description of the soil sampling methodology and instrumentation, including the method for determining background radium concentration and a description of any other radionuclides (e.g. Th-230) for which samples will be tested.***

Response:

For details refer to the Reclamation Plan, Attachment A, Sections 3.3 through 3.5 which discusses the methodology for the determination of windblown contaminants. All methods utilized will be consistent with the guidance contained in NUREG-5849: "Manual for Conducting Radiological Surveys in Support of License Termination".

Soil sampling methodology and instrumentation is further discussed below in response to Comment 6 and Comment 7.

Comment:

- (6) *Rationale by which guideline values were selected in the scoping survey to determine whether an area requires remediation. The method used to determine the actual Ra-226 concentration in the soil should be described as well as the confidence levels used to establish the guideline values.*

Response:

Guideline Values

Reclamation Plan, Attachment A, Section 3.3.2, indicates that guideline values will be determined and will form the basis for the cleanup of the site. Specific guideline values have not been determined at this time.

Methods

The method used to determine the actual Ra-226 concentration in the soil will consist of collecting a series of soil samples and correlating the gamma reading of a Mount Sopris Model SC-132 scintillometer (or equivalent device) with the Ra-226 concentration determined by a multi-channel analyzer (or equivalent device) on each soil sample.

The actual number of samples used for correlation will depend on the correlation of the results between the gamma readings and the Ra-226 concentration. However, it is proposed that a minimum of 50 sample locations be tested in order to determine a statistically based correlation coefficient of not less than 0.7.

Comment:

- (7) *A description and rationale for the criteria used to define the extent of windblown tailings contamination beyond which further sampling is not necessary.***

Response:

The description and rationale for the criteria used to define the extent of windblown contamination is presented in the Reclamation Plan. The sections of the plan containing this information for the scoping survey and the final survey are discussed below.

Scoping Survey

Reclamation Plan, Attachment A, Sections 3.3.2 and 3.3.3, indicates that the scoping survey for windblown contamination will be initially conducted using a calibrated Mount Sopris Model SC-132 scintillometer (or equivalent device) on a 50 x 50 meter grid. Furthermore, Reclamation Plan, Attachment A, Section 3.3.3 states: "Grids where no readings exceed 75 percent of the guideline value will be classified as unaffected, and therefore will not require remediation."

Final Survey

Reclamation Plan, Attachment A, Section 3.3.5 states: "After remediation, the affected areas deemed to be in compliance with standards will then undergo a final survey, utilizing a 10 x 10 meter grid system with sample point locations as shown in Figure A-3.3.2." (Standard Sampling Pattern for Systematic Survey of Soil).

The final survey will be conducted with a calibrated Mount Sopris SC-132 scintillometer (or equivalent device) at each systematic survey point. When readings at two consecutive systematic sample locations do not exceed the cleanup standards the area extending outward from the first systematic point will be classified as unaffected and no further remediation will be required.

Comment:

- (8) *A description of the design to be adopted in the breach area of Cell 4A and an analysis to show adequacy of the design to prevent any potential erosion.***

Response:

Liquids in Cell 4A will be evaporated to dryness and the crystals, synthetic liner and any contaminated soils will be placed in Cell 2 or Cell 3. Non-contaminated soils will than be utilized to reduce the southern slopes of Cell 3 from the current 3:1 to the proposed 5:1.

Following reclamation, Cell 4A will contain no by-product materials. Therefore, the breach is designed only to prevent ponding within the area. The design does not consider potential erosion of the breach area.

It should be noted that no measures, limited to natural materials, will prevent any or all potential erosion.

Comment:

- (9) *A description of how the adequacy of the material properties used in the design of various components of tailing impoundments (e.g., field hydraulic conductivity of cover material, soil/tailings properties used for embankment design) is verified.***

Response:

Verification of the adequacy of the material properties used in the tailings impoundment design was provided in a study provided by Dames and Moore, the results of which are presented in the report entitled "Site Selection and Design Study - Tailings Retention and Mill Facilities, White Mesa Uranium Project" dated January 17, 1978.

A summary of the material properties from the Dames and Moore report is presented in Appendix A of the Tailings Cover Design.

Comment:

- (10) A description of the drainage catchment area(s) and diversion channel(s) design.**

Response:

Drainage catchment area(s) and diversion channel(s) are not included in the design.

A "discharge channel" is shown on Figure A-2.2.4-1, Sedimentation Basin Detail. The purpose of this channel is to prevent ponding within Cell 1 following reclamation. A detailed discussion of this discharge channel is presented in response to Comment 18.

Comment:

- (11) A replacement of seven pages of illegible data. These pages are in appendix A of Appendix D [Tailings Cover Design White Mesa Mill, October 1996 (pages marked p. 12 through p. 17)] and in appendix G of Appendix D [Tailings Cover Design White Mesa Mill, October 1996 (figure showing cross-section along Cell 4 dike)].**

Response:

The seven replacement pages are included in Attachment 3 of this response.

Comment:

- (12) It is not clear if the rock durability test results for the proposed sandstone source rock are based on a series of durability tests or on only one test; therefore, additional rock durability test information is needed. At a minimum, IUC should (1) provide durability test results for several representative rock samples; and (2) verify that the data represents average results for representative samples; and (3) provide separate test results if different rock types were used. Alternately, IUC should provide further justification that the information already provided is adequate to demonstrate rock durability.**

Further, IUC should provide information related to the location of the proposed source(s) and, in accordance with the criteria suggested in the NRC Staff Technical Position, should provide details of the petrographic examinations (mineralogy, cementation, fractures, clay content, etc) that were conducted on the rock.

Response:

One rock durability test has been performed to date. The sandstone sample was collected approximately one mile west of the site in the Westwater Canyon Area. Representative rock durability testing and petrographic examinations will be performed as an integral part of Construction Quality Assurance procedures, prior to reclamation, on each proposed source (i.e. from sandstone cobbles within the random fill stockpiles).

Comment:

- (13) *Based on a site visit conducted several years ago, the NRC staff is aware that high-quality alluvial rock exists in the site area. Based on the potentially questionable quality of the sandstone source, additional information should be provided regarding this alluvial source and the reasons for its rejection.***

Response:

The utilization of the alluvial source was not considered as the source is located within a riparian area. The potential impact during construction of haul roads and excavation of the alluvial material is not recommendable from an environmental standpoint. Conversely, the sandstone is readily available and the use of the material will not pose an environmental threat.

Comment:

- (14) *Additional information should be provided regarding the construction specifications (Appendix F) and construction testing program to be conducted on the riprap. The specifications should be revised to include specific criteria for rock placement. Specific tolerances for placement should be specified for the riprap and filter, depending on the size of the material being placed. Measures, such as depth checks on a specific grid, should be provided to verify the thickness of the riprap.***

Response:

Attachment 4 of this response contains "Specification for Construction of Rock Covers and Other Erosion Protection on the Tailings Cells".

Attachment 4 is intended as an outline of the formal construction specifications that will be provided prior to reclamation. Specific values have been precluded from the document as additional soil and rock properties (riprap durability for oversizing analysis / random fill grain size analysis for determining filter criteria) will be verified at a later date. (See also response to Comment 12).

Comment:

- (15) *The NRC staff notes that a filter layer is not proposed for the riprap layer to be placed on the 1V on 5H side slopes of the cells. In general, a filter layer is not required for top slopes (with relatively flat slopes of about one percent or less), but a filter is likely to be needed for the side slopes. The filter is needed because velocities through the larger rock voids may erode the underlying soil particles. IUC should provide a filter layer for the side slopes or provide justification that a filter layer is not needed.***

Response:

It is acknowledged that a filter layer will be required on the 5H:1V side slopes. The filter criteria will be designed in accordance with NUREG/CR-4620 "Methodologies for Evaluating Long-Term Stabilization Designs of Uranium Mill Tailings Impoundments".

According to NUREG/CR-4620, the D_{85} of the base (random fill) is required in order to establish the filter gradation. At this time, the proposed random fill materials are stockpiled on the site for future use and extensive laboratory testing has not been performed to determine a representative D_{85} . The filter layer gradation will be determined prior to reclamation, when additional laboratory analysis is available.

The filter layer will be included in the cost estimate. For costing purposes a filter thickness of six inches will be utilized.

Comment:

- (16) *Rock aprons/toes are likely to be needed at those locations where the steeper side slopes meet the natural ground. Based on site visits to the area, there is sufficient evidence of active gullying, and gully headcutting into the cells is possible. Several factors need to be taken into account, and the design of the apron/toe should be based on the following general concepts: (1) provide riprap of adequate size to be stable against the design storm (PMF); (2) provide uniform and/or gentle grades along the apron and the***

adjacent ground surface such that runoff is distributed uniformly at a relatively low velocity, minimizing the potential for flow concentration and erosion; and (3) provide an adequate apron thickness (depth) to prevent undercutting of the disposal cells by: (I) local scour that could result from the PMF; or (II) potential gully encroachment, that could occur due to gradual headcutting over a long period of time.

The key elements which IUC needs to consider in the design of riprap protection for the apron/toe are: (1) the downstream portion of the apron/toe which is assumed to have collapsed due to scour or long-term erosion; and (2) scour at the ground surface downstream of the apron/toe. To account for the potential uncertainties in toe design, the NRC staff suggests that it may be prudent to use several different analytical methods to design the riprap for these key elements.

As part of the analysis, IUC should assume that the natural ground downstream of the toe will be eroded due to cumulative local scour and/or erosion at its base, resulting in the collapse of the rock into the eroded area. To determine the depth to which the toe must be placed, it is necessary to estimate the depth of scour which will occur to the natural ground slope just downstream of the toe. The toe should then be placed at least to the estimated depth of scour.

To further document the acceptability of the design of the rock toe/apron, it may be very useful for IUC to provide a geomorphic report. The geomorphic bases for the design of the rock toe should be provided, including a geomorphic evaluation of the potential for formation of gullies. The geomorphic analysis may also document the depth of the gullies in the immediate area and help to justify the selection of a depth of scour.

It should also be noted that rock toes are considered to be critical areas, and the rock quality criteria for these rock toes are not likely to be met by the proposed sandstone source. Use of rock of higher quality appears necessary (see Comment 2).

Response:

Cell 2 South Slope

A toe apron will be included where the south slope of Cell 2 discharges to the surface of Cell 3. This toe apron will consist of the same riprap size and thickness as is on the south slope (see response to comment 17) and will extend outward from the toe of the slope for a distance of ten feet.

Cell 3 South Slope

A toe apron will be included for the south side slope of Cell 3 where the Dakota sandstone is not present at the surface. No other side slope has either the dimensions (height and length) or runoff to experience significant scour. The south slope toe apron will consist of the same riprap size and thickness as is on the side slope (see response to comment 17) and will extend outward from the toe of the slope a distance of ten feet.

The final grading plan will provide for distribution of runoff from the south slope of Cell 3, with surface gradients away from established drainage courses so that runoff is not concentrated along pathways where natural erosion has caused gulying. This measure will deprive the existing gullies of most of the runoff that they had previously carried, arresting or sharply reducing the potential for future erosion.

The toe apron will not be placed where the Dakota sandstone is present at the surface. The bedrock provides natural resistance to gulying that is better than, or equal to, any scour protection using natural materials that could be constructed. It is evident, however, that the sandstone does erode and erosion cannot be absolutely prevented using sandstone as riprap or erosion barriers. For that reason, diversion and distribution of runoff away from active gullies will be more effective than constructing riprap toes below final grade.

Comment:

- (17) *Review of the calculations/spreadsheets (Appendix F) for the design of the rock on the side slopes indicates that the flow lengths used for the design of the side slope rock (275 feet) does not include the length of the top slope that will contribute runoff to the side slopes. Beginning at the upper end of Cell 2 (near Cell 1), it appears that runoff from Cells 2 and 3 will flow southward for over 1000 feet and discharge down the side slopes of the cells. Accordingly, the riprap in these areas should be redesigned, as necessary, to account for the increased flow lengths.*

Response:

The calculations have been revised to include the total flow path length of the top slope which will contribute runoff to the south and east side slope of Cell 3, and the south side slope of Cell 2. The revised calculations are presented in Attachment 5 of this response.

The size of the riprap placed along the sides of the slopes was determined using the Stephenson method (NUREG/CR-4651). The side slopes are designed at 5H:1V. Assuming that the on-site sandstone would be utilized as riprap material, with a rock rating of 55.74%, the modified D_{50} size should be at least 8.0 inches.

According to the Stephenson model, the riprap thickness should be at least 2 times the D_{50} value. This indicates a minimum thickness of 16 inches, with a suggested thickness of 18 inches, on the sides of the slopes.

Comment:

- (18) *Additional information should be provided for the design of the sedimentation basin and the discharge channel. HEC-1 analysis should be provided, along with HEC-2 input and output data (or other water surface profile analysis), to document the acceptability of parameters used in the design of the riprap for the channel.*

In addition, sedimentation analyses should be provided to show that the capacity of the sedimentation basin is adequate and that the HEC-1 routings adequately account for decreases in storage capacity in the basin due to sediment accumulation over a long period of time. Further, each of the parameters used to design the riprap in the channel should be provided, including channel slope, width, flow rate, and water surface profiles, particularly if flow changes from subcritical to supercritical at some location in the channel.

Response:

Sedimentation Basin

Following reclamation, Cell 1 will trap sediment over long periods of time and the storage capacity of the basin will decrease. However, the cell is not designed to serve as a sedimentation basin, it will simply collect sediment due to the physical configuration of the reclaimed subsurface cell.

As Cell 1 is not designed to serve as a sedimentation basin, and the cell (following reclamation) will not be related to the reclamation of the by-product cells, sedimentation analyses to determine the long term capacity of the basin was not performed.

Discharge Channel

Surface runoff from the mill area, and immediately north of the mill area, will be routed through the sedimentation basin and ultimately discharged through the channel located in the southwest portion of Cell 1. The discharge channel and riprap were sized to accommodate the PMF flood using the HEC-1 model, as stated on Page 3-8, Section 3.2.2.2 of the Reclamation Plan.

Following reclamation Cell 1 will not contain by-product materials and will not be related to the reclamation of the by-product cells. Therefore, the purpose (and size) of the channel is only to prevent excessive ponding within the cell area.

**Response to Nuclear Regulatory Commission / White Mesa Mill Reclamation Plan
Comments Received December 8, 1997
Source Material License SUA-1358**

Comment:

- (1) *Technical justification that the frequencies of quality control (QC) tests proposed in the reclamation plan are adequate for controlling the quality of the construction of the final disposal cell.*

The NRC "Staff Technical Position (STP) on Testing and Inspection Plans During Construction of DOE's Remedial Action at Inactive Uranium Mill Tailings Sites" (NRC, 1989) provides recommended frequencies for QC tests for various parameters during the construction of tailings disposal cells. NRC has found the frequencies recommended in the STP acceptable for maintaining the quality of the construction activities for both Title I and Title II sites. In some cases, the staff recommends in the STP conducting the QC tests more frequently than IUSA has proposed in the reclamation plan.

The recommended test frequencies for specific QC tests are provided in the following table.

Test	Recommendations for Testing Frequency
Field Density and Moisture Tests	<i>Minimum of one test per 1,000 yd³ of contaminated material.</i> <i>Minimum of one test per 500 yd³ of other compacted material including seepage barrier and/or radon barrier earth cover.</i> <i>Minimum of two tests for each day that an appreciable amount of fill is placed (in excess of 150 yd³).</i> <i>Minimum of one test per lift and at least one test for every full shift of compacted operations.</i>
Compaction Tests	<i>One point Proctor test at a frequency of one test for every five field density tests.</i> <i>Approximately one laboratory compaction curve based on complete Proctor tests for every 10 or 15 field tests, depending on the variability of the materials.</i>

Gradation and Classification Tests	<i>Minimum of one test per 1,000 yd³ of radon/seepage barrier material, and one test per 2,000 yd³ of other engineered soil fill material.</i> <i>For all materials other than random fill and contaminated materials, at least one gradation test should be run for each day of significant material placement (in excess of 150 yd³).</i>
Atterberg Limit Tests	<i>At least one test for each day of significant cohesive cover or liner material placement (in excess of 150 yd³).</i>
Rock Durability Tests	<i>For any type of riprap where the volume is greater than 30,000 yd³, a test series should be performed for each additional 10,000 yd³ of riprap delivered.</i>

IUC should adopt the frequencies recommended in the STP, or alternately, present a technical justification that the frequencies of QC tests proposed in the reclamation plan are adequate for controlling the quality of construction of the final disposal cell.

REFERENCE NRC, 1989, "Staff Technical Position on Testing and Inspection Plans During Construction of DOE's Remedial Action at Inactive Uranium Mill Tailing Sites," Revision 2, January 1989.

Response:

At this time technical justification that the proposed QC testing frequencies are adequate for construction, should not be required. The testing frequencies, presented within Attachment A of the Reclamation Plan, are intended as a rough outline for the formal construction specifications that will be prepared prior to reclamation. The QC outline, although not detailed for construction, is adequate for determining the QC cost estimate.

It is clear that the NRC "Staff Technical Position (STP) on Testing and Inspection Plans During Construction of DOE's Remedial Action at Inactive Uranium Mill Tailings Sites" rigidly provides conservative QC testing frequencies adequate for controlling construction quality. However, the STP does not allow for site specific conditions which may exist.

In order to prepare the formal QC specifications a pre-construction laboratory testing program will be performed, immediately preceding the reclamation construction, on the proposed random fill and riprap materials. This additional testing, along with the current laboratory data from the Section 16

clay source, will provide the information required to establish the site specific QC testing frequencies adequate for controlling the quality of construction.

The following chart briefly summarizes the site specific conditions that will be utilized to determine the QC testing frequencies:

Test	Testing Frequency Determinations
Field Density and Moisture Testing	<p>Variability of the materials optimum moisture content and maximum dry density: Highly variable soils will require conservative field control. Conversely, consistent soils may require less testing than recommended in the STP as consistent compactor coverages will yield consistent results.</p> <p>Volume of fill materials: Large surface area fills, such as the top of the tailings cells, require less tests per cubic yard compared with small surface areas where rapid lift placement occurs.</p> <p>Topography of the fill area: Fill placement on slopes, and tight corners, require additional tests as compaction equipment movement is hindered.</p>
Compaction Tests	<p>Variability of the materials optimum moisture content and maximum dry density: Highly variable soils will require conservative laboratory control to properly identify the materials in the field. Conversely, consistent materials may require less testing than recommended in the STP as field control testing will be less conservative.</p> <p>If variable soils are encountered one point Proctors may be valid. However, if distinct visible variations are present within the various soils one point Proctors would not be necessary. Additionally, if consistent materials are encountered one point Proctors will not be valid.</p>
Gradation and Classification Tests	<p>Gradations will be necessary for soil classification purposes. However, gradation specifications for the random fill and clay layers are not included in the Reclamation Plan. Therefore gradation testing should require less testing than recommended in the STP.</p>

Atterberg Limit Tests	Atterberg Limit tests will be required for soil classification purposes. However, liquid and plastic limits are not specified in the Reclamation Plan. Therefore Atterberg Limit tests should be less than recommended in the STP.
Rock Durability Tests	Number of borrow areas: The testing frequency will be dependent upon the number of sources utilized. Each source should have at least one test.

Following additional laboratory analysis, the construction QC site specific specifications will be established to control the quality of the construction in accordance with standard industry procedures. However, at this time the QC outline is adequate for the QC cost estimate.

Comment:

(2) Additional information addressing details of disposal cell construction. This information should include:

- Methods, procedures, and requirements for excavating, hauling, stockpiling, and placing contaminated and non-contaminated materials and other disposal cell materials.**

Response:

The comment as stated indicates the reclamation plan includes the construction of a disposal cell. However, materials disposed during the reclamation will be placed in the existing tailings Cells 2 and 3.

Contaminated materials to be deposited within the tailings cells consists of raffinate crystals, synthetic liners, contaminated soils, and decommissioned mill equipment. The placement of these materials is discussed in Sections 2.0, 3.0 and 4.0, Attachment A, of the Reclamation Plan.

Non-contaminated materials to be placed within the tailings cells consists of the final cover materials. The placement of these materials is discussed in Sections 5.0 and 6.0, Attachment A, of the Reclamation Plan.

The excavation and hauling procedures for these materials will be the responsibility of the contractor. As per industry practice, the contractor may use any type of equipment he may desire, provided the equipment is in satisfactory condition and is of such capacity that the construction schedule can be maintained as planned. It is not expected that materials will be stockpiled during construction.

- ***Material placement and compaction procedures (e.g., lift height, compactive effort) to achieve the desired moisture content, placement density, and permeability.***

Response:

Material placement and compaction procedures will be developed by the earthwork contractor. The Quality Control testing (moisture content and dry density) will be used to verify that the specified moisture content and percent compaction is being achieved with the contractors chosen equipment and procedures.

The earthwork contractor will be required to place and compact the soil materials within the maximum lift thickness, percent compaction, and moisture content specifications presented in Table A-5.3.2.1-1, Attachment A, of the Reclamation Plan. The lift thickness and number of compactor passes required to conform to the soil placement and compaction specifications will be dependent upon the actual equipment utilized during construction. The contractor may use any type of earthmoving and watering equipment he may desire, or have at his disposal, provided the equipment is in satisfactory condition and is of such capacity that the construction schedule can be maintained as planned.

To determine that the moisture content and percent compaction requirements of the soil material is being met Quality Control field and laboratory tests will be conducted at specified intervals as presented in Section 7.4, Attachment A, of the Reclamation Plan. Furthermore, Quality Control observation will be performed to ensure that the maximum lift thickness is not exceeded during soil placement.

ATTACHMENT 1

**"Letter Report
Section 16 Clay Material Test Data
White Mesa Uranium Project
Blanding, Utah"**

**Prepared by D'Appolonia Consulting Engineers, Inc.
Dated March 8, 1982**

D'APPOLONIA

CONSULTING ENGINEERS, INC.

March 8, 1982

Project No. RM78-6823

Mr. H. R. Roberts
Energy Fuels Nuclear, Inc.
1515 Arapahoe Street
Three Park Central, Suite 900
Denver, Colorado 80202

Letter Report
Section 16 Clay Material Test Data
White Mesa Uranium Project
Blanding, Utah

Dear Harold:

This report presents the results of field investigations and laboratory tests performed on Section 16 clay material. The material tested was obtained from borings and test pits made in April 1979. The laboratory tests were performed and the data retained in our files until your recent request for the data.

Field Investigations

The area of investigation is a canyon located in Section 16, about three miles south of the mill site. Seven borings were drilled as part of the field investigations. These borings, 100 through 106, are located approximately as shown on Figure 1.

The borings were drilled with a rig provided by Energy Fuels using the rotary method with air pressure to flush out the cuttings. Samples were obtained by sampling the cuttings on five foot intervals. Only qualitative information on the subsurface materials is available because of the method of drilling and sampling utilized. However, the qualitative information and samples obtained are suitable to provide preliminary data on the character of the subsurface materials present.

Three test pits (1-3) were excavated to obtain bulk samples for laboratory testing. The location of the test pits is shown on Figure 1.

Samples from Boring 2-16 drilled by Energy Fuels in November 1978 were also provided to D'Appolonia for testing. The location of Boring 2-16 is shown on Figure 1.

Subsurface Conditions

The subsurface conditions in the canyon, based on the boring data, are shown on Cross Sections A-A' and B-B' presented on Figures 2 and 3, respectively. The plan locations of these cross sections is shown on Figure 1. As shown on the cross sections, the subsurface consists of a surficial layer of red clayey and silty sand about five feet thick. The underlying material is mostly a red or gray silty clay. The consistency of the silty clay layer varies from stiff to hard, based on observations of the drillers and rig during drilling. A lense or layer of very hard silt was noted in Boring 105. This layer appears to be a well cemented unit from the cutting samples obtained. In Boring 106, the surficial sand layer was about 20 feet thick and a clayey sand layer was also encountered at a depth of about 30 feet.

The laboratory soil classifications for the tested samples are also shown on Cross Sections A-A' and B-B'. The testing program is discussed in detail in the following section, however, the testing results indicate that the silty clay layer is mostly a CL or CH material with one sample being a SM and two a ML. These test results show the material is basically a fine grained soil with a varying amount of silt and clay size particles. The plasticity characteristics of the material vary from low to high. Further discussion of the test results and material characteristics is given below.

Water in the borings was not noted except for Boring 104 for which a depth of about 43 feet was measured. This depth is not considered completely reliable since it was measured only one day after drilling and the water level may not have had time to stabilize.

Laboratory Test Results

The laboratory testing program conducted on samples from the borings and test pits included the following types of tests:

- o Classification
 - Grain size, sieve and hydrometer
 - Atterberg limits
 - Specific gravity
- o X-Ray Diffraction
- o Cation Exchange Capacity
- o Exchangeable Cations
- o Modified Proctor Compaction Density
- o Permeability

The results of the classification tests are given on Table 1. The soil classifications given are shown on Cross Sections A-A' and B-B' (Figures 2 and 3) and were discussed above.

The cation exchange capacity (CEC) and exchangeable ions were conducted to evaluate the type of clays present and the chemical effects resulting from contact with the tailings liquid. Tests were run on samples from Test Pits 2 and 3 samples and Boring 103 (15-20 foot depth). Soil from each sample was treated by soaking in simulated tailings liquid for 48 hours before testing. Both treated and untreated (as received) samples were tested and the results are presented on Table 2. Results of the testing are summarized as follows:

- o The untreated samples indicate pH (1:1) values between 7.40 and 8.35 with CEC values in the 45-56 meq/100g range. The predominate exchangeable ions are calcium and sodium for Test Pits 2 and 3 and calcium and magnesium for Boring 103 (15-20 ft).
- o The treated samples indicate pH (1:1) values between 1.70 and 2.35 with CEC values in the 90-100 meq/100g range. The predominate exchangeable ions are hydrogen, calcium, and magnesium for all the samples.

These results indicate that exposure to the tailings water causes:

- the pH (1:1) of the material to decrease.
- the exchangeable hydrogen and magnesium to increase.
- the exchangeable calcium and sodium to decrease.
- the CEC to increase by a factor of about two due primarily to the large increase in exchangeable hydrogen.

The effects of these changes on clay material properties, particularly permeability, is discussed in the following paragraphs.

The X-ray diffraction tests were run on material from the same three samples as tested for CEC and exchangeable ions. The x-ray diffraction testing was conducted to evaluate the type of clay minerals occurring in the material. The results of the testing are given on Table 3. As shown, about 50 percent of the material is quartz, 25 percent montmorillonite, 25 percent illite, and minor percentages of other minerals. Montmorillonite is an active clay mineral which typically has a low coefficient of permeability. Illite is also a clay mineral, but it is typically relatively inactive with a somewhat higher coefficient of permeability.

Modified Proctor compaction tests were conducted on four different samples. Test Pits 1, 2 and 3 samples were tested and a composite sample from Boring 2-16 (85 to 210 feet depth). The results of the modified Proctor tests are given on Table 1. The average maximum dry density measured is 107 pounds per cubic foot and the average optimum water content is 17.5 percent.

Permeability tests were conducted on compacted samples of material from Boring 2-16 (composite 85-120 feet), Boring 101 (composite 0-25 feet), Boring 103 (composite 0-25 feet) and Test Pit 2. The tests were conducted in permeability cells with a confining pressure applied around the sample which is encased in a rubber membrane. A differential pressure was applied across the sample and flow of fluid through the sample measured. Both distilled water and simulated tailings liquid were used in the tests. The tests on Borings 101 and 103, and Test Pit 2 were conducted over a period of about five months to assess the effects of tailings liquid on the permeability of the material. The tests were conducted with distilled water for about two months to establish saturation and steady state flow. Tailings liquid was then introduced to the sample and the test continued for three more months. The results of the permeability tests are presented on Table 4 along with other pertinent sample data. The material has an average coefficient of permeability with water of 3.3×10^{-10} centimeters per second and 5.1×10^{-10} centimeters per second with simulated tailings liquid. The test results indicate that the permeability of the material was essentially the same with distilled water and tailings liquid and no degradation of the material was indicated.

Conclusions and Recommendations

Based on the field and laboratory investigations discussed above, conclusions which can be made regarding the materials in Section 16 are:

- o The material is mostly a silty clay (CL to CH) with slight variation in properties. The clay minerals are mostly montmorillonite with some illite.
- o The material varies laterally with some layers or lenses of sand and silt. The consistency of the material also varies from stiff to hard or very hard.
- o The permeability values of the material are very low and long-term permeability tests conducted with simulated tailings liquid indicate little change in permeability with time. This result is in good agreement with the results of the CEC, exchangeable ion tests and x-ray diffraction test results.
- o The clay material is suitable for use as borrow for use as a clay liner or in situ as a natural liner layer.

Recommendations for further assessment of the clay for use as a borrow area or in situ clay liner source are:

- o Geotechnical borings with split spoon samples to assess the material characteristics more specifically, including consistency, natural water content, and classification.

Mr. H. R. Roberts

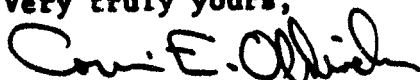
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March 8, 1982

- o Field permeability tests (falling or rising head) in the borings to measure the in situ permeability.
- o Installation of piezometers to determine the ground water level.

Additional discussion of the above recommendations can be provided as necessary depending on your needs.

Very truly yours,



Corwin E. Oldweiler
Project Engineer

CEO:par

D'APPOLONIA

TABLE 1
LABORATORY TEST RESULTS

BOILING/ TEST PIT	SAMPLE DEPTH (FEET)	GRAIN SIZE ANALYSIS			ATTENUATION LIMITS			UCCS CLASSIFICATION	OPTIMUM		
		SAND (PERCENT)	SILT (PERCENT)	CLAY (PERCENT)	LIQUID (PERCENT)	PLASTIC (PERCENT)	PLASTICITY (PERCENT)		SPECIFIC GRAVITY	DRY DENSITY (PCF)	WATER CONTENT (PERCENT)
101	0-5	61	22	17	24.0	18.5	5.5	SC-BH	-	-	-
	5-10	26	40	26	20.9	24.1	34.8	CH	-	-	-
	10-15	10	50	40	73.0	20.2	44.8	CH	-	-	-
	15-20	7	54	39	103.0	31.2	71.8	CH	2.59	-	-
	5-10	-	-	-	-	-	WP	ML	-	-	-
102	10-15	-	-	-	-	-	WP	ML	-	-	-
	15-20	-	-	-	20.3	10.2	10.1	CL	-	-	-
	0-5	70	10	12	17.0	14.9	2.1	SH	2.71	-	-
	5-10	15	38	47	73.8	24.9	40.9	CH	-	-	-
	10-15	13	49	38	59.8	26.6	33.2	CH	-	-	-
103	15-20	13	50	37	71.0	21.6	49.4	CH	-	-	-
	0-5	55	30	15	18.4	16.2	2.2	SH	-	-	-
	5-10	30	43	27	31.2	16.5	14.7	CL	-	-	-
	10-15	64	17	17	-	-	-	-	-	-	-
	15-20	37	31	32	35.7	11.0	23.9	CL	-	-	-
104	0-5	50	22	20	-	-	WP	SH	-	-	-
	5-10	65	17	18	-	-	WP	SH	-	-	-
	10-15	62	17	21	24.0	12.0	12.0	SC	-	-	-
	15-20	17	36	47	71.0	18.9	52.1	CH	-	-	-
	-	17	40	43	100.0	25.0	83.0	CH	-	99.9	19.9
105	-	17	50	33	141.2	18.4	122.8	CH	-	111.5	15.0
	-	3	42	55	115.0	23.0	92.0	CH	2.60	101.0	20.5
	65	-	-	-	32.0	15.8	16.2	CL	-	-	-
	125	7	43	50	57.5	25.9	31.6	CH	-	-	-
	100	-	-	-	148.5	25.3	123.0	CH	-	-	-
2-16	CONTROL 85-210	95	5	0	-	-	-	SH-SC	-	-	-
	CONTROL 85-210	18	47	35	-	-	-	cl-m	2.72	115.0	14.7

(1) These samples are Test Pits
(2) Sample tested before soaking.
(3) Sample tested after soaking 16 hours.

TABLE 2

CATION EXCHANGE CAPACITY AND EXCHANGEABLE CATION
TEST RESULTS

PARAMETER	UNITS	UNTREATED SAMPLES			TREATED SAMPLES ⁽¹⁾		
		TEST PIT 2	TEST PIT 3	BORING 103	TEST PIT 2 ⁽²⁾	TEST PIT 3	BORING 103
pH (1:1)	-	8.35	7.40	7.60	2.30	2.35	1.70
Buffer pH	-	NA	NA	NA	2.28	2.20	2.15
Exchangeable:							
H	meq/100g	0	0	0	56.6	57.6	58.2
Ca	meq/100g	19.5	21.1	25.8	12.3	13.5	18.7
Mg	meq/100g	4.3	4.9	15.4	17.0	20.3	17.8
Na	meq/100g	20.0	28.0	6.5	3.7	6.5	2.6
K	meq/100g	1.2	2.5	0.6	0.8	1.6	0.5
Cation Exchange Capacity (CEC)	meq/100g	45	56	48	90	100	98

(1) Samples soaked in simulated tailings liquid for 48 hours before testing.

(2) Represents triplicate results.

TABLE 3

X-RAY DIFFRACTION SEMI-QUANTITATIVE RESULTS

SAMPLE	QUARTZ	ANDESINE	MONTMORILLONITE	ILLITE	MIXED LAYER
Test Pit 2	50%+	-5%	10-25%	10-25%	5-10%
Test Pit 3	50%+	5-10%	10-25%	10-25%	5-10%
Boring 101 (15'-20' Depth)	50%+	5-10%	25-50%	Trace	-5%

TABLE 4
PERMEABILITY TEST RESULTS

BORING/ TEST PIT	SAMPLE DEPTH (FEET)	INITIAL CONDITIONS		COEFFICIENTS OF PERMEABILITY	
		DRY DENSITY (PCF)	WATER CONTENT (PERCENT)	WITH DISTILLED WATER (CM/SEC)	WITH TAILINGS LIQUID (CM/SEC)
103	0-25	116.7	13.3	1.2×10^{-9}	9.4×10^{-10}
101	0-25	117.5	14.6	5.2×10^{-10}	7.5×10^{-10}
2	-	110.7	14.7	4.7×10^{-10}	2.3×10^{-10}
2-16	85-210	101	15	-	1.0×10^{-10}
2-16	85-210	110	15	-	5.5×10^{-10}

DRAWING RM 78-682-B9
 NUMBER
 CHECKED BY
 APPROVED BY
 DRAWN BY



2-16

T 36 S

N 305,000

E 2,579,000

E 2,579,000

A

B-100

B-100

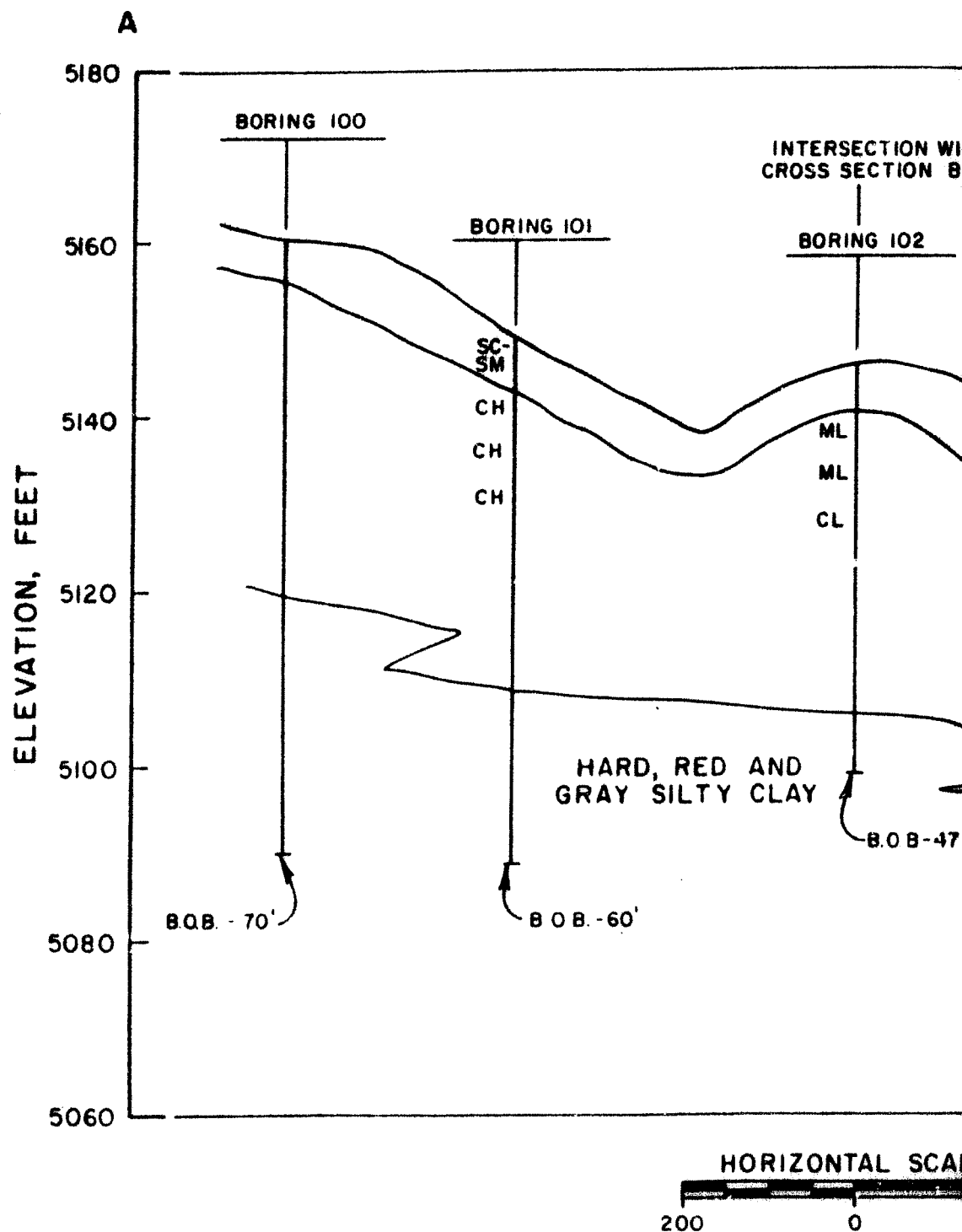
REFERENCE

TOPOGRAPHIC MAP OF BLANDING MILL
 SITE, SHEET 4, BY DELTA AERIAL
 SURVEYS, INC., 12-8-76

SECTION 16

R 22 E

DRAWN BY R Bricker
 CHECKED BY J Mar. 82
 APPROVED BY
 DRAWING RM 78-682-B7
 NUMBER



THE DEPTH AND THICKNESS OF THE SUBSURFACE STRATA INDICATED ON THE SECTIONS WERE GENERALIZED FROM AND INTERPOLATED BETWEEN THE TEST BORINGS. INFORMATION ON ACTUAL SUBSURFACE CONDITIONS EXISTS ONLY AT THE LOCATION OF THE TEST BORINGS AND IT IS POSSIBLE THAT SUBSURFACE CONDITIONS BETWEEN THE TEST BORINGS MAY VARY FROM THOSE INDICATED.

LEGEND

CH - LABORATORY SOIL CLASSIFICATION
 (UNIFIED SOIL CLASSIFICATION SYSTEM)

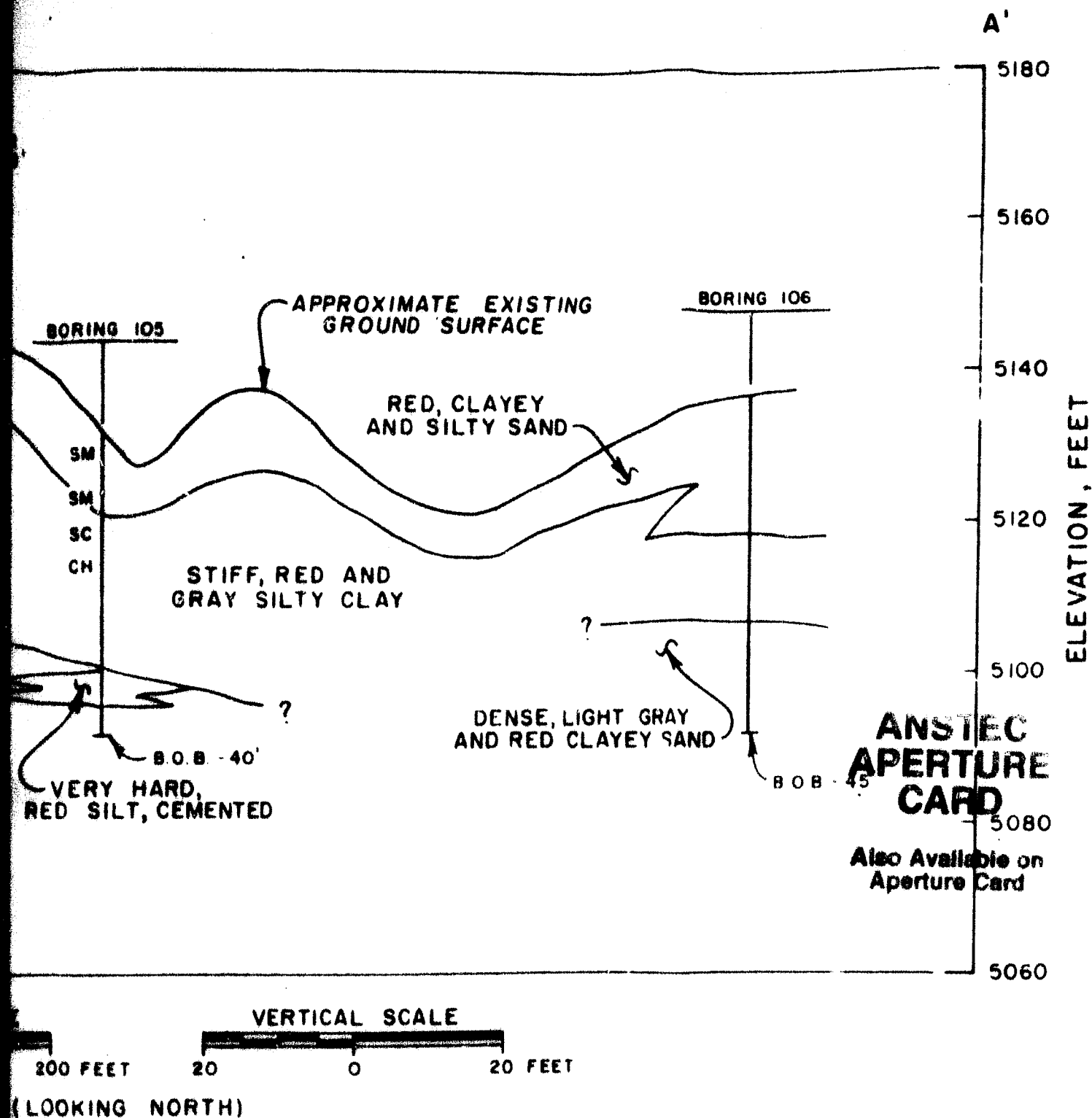


FIGURE 2

SUBSURFACE CROSS SECTION A-A'

NOTES:

1. FOR PLAN LOCATION OF CROSS SECTION, SEE FIGURE 1.
2. VERTICAL EXAGGERATION EQUALS 10X

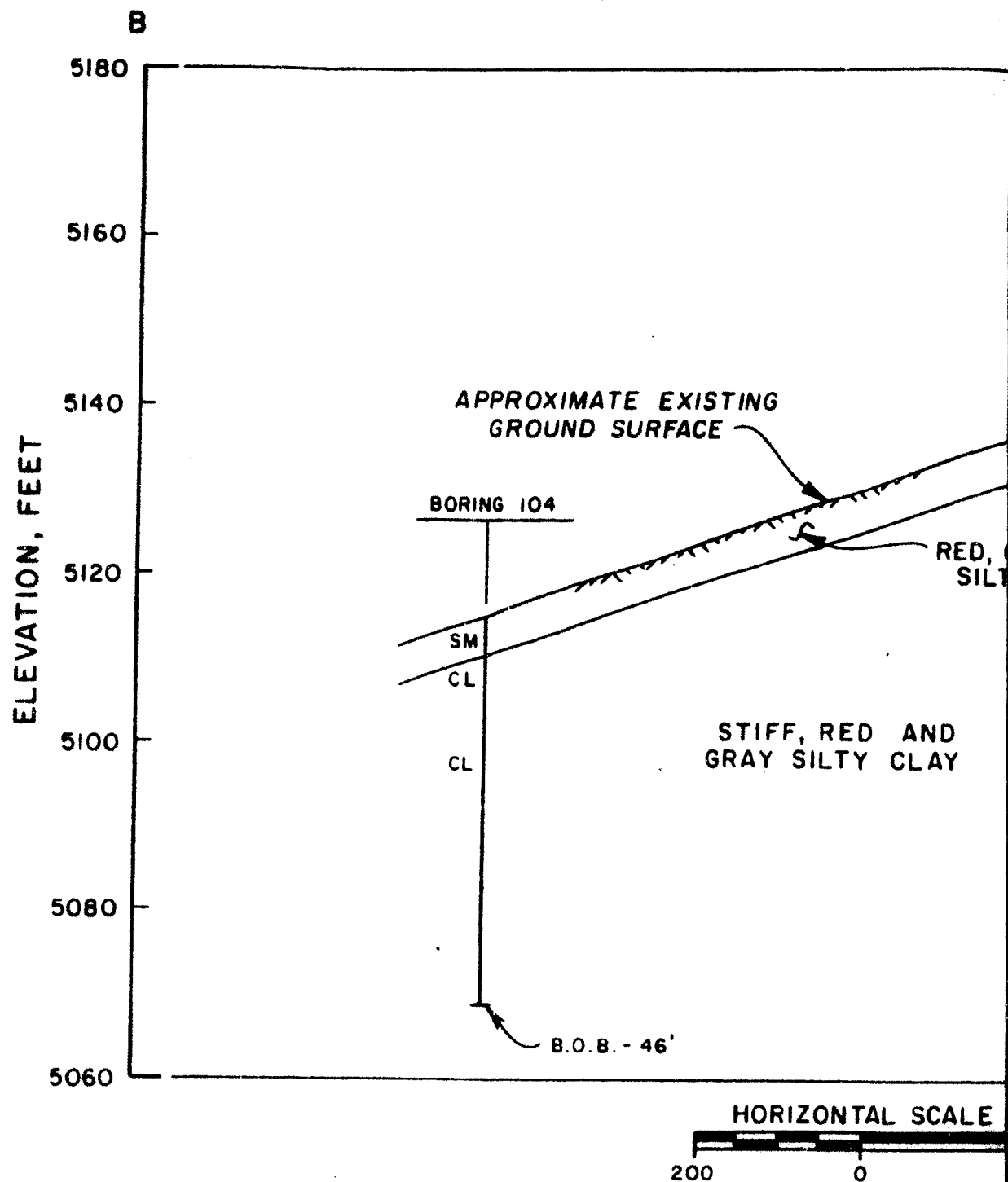
PREPARED FOR

ENERGY FUELS NUCLEAR, INC.
DENVER, COLORADO

AD:AM:PM:DI:ADN:LA

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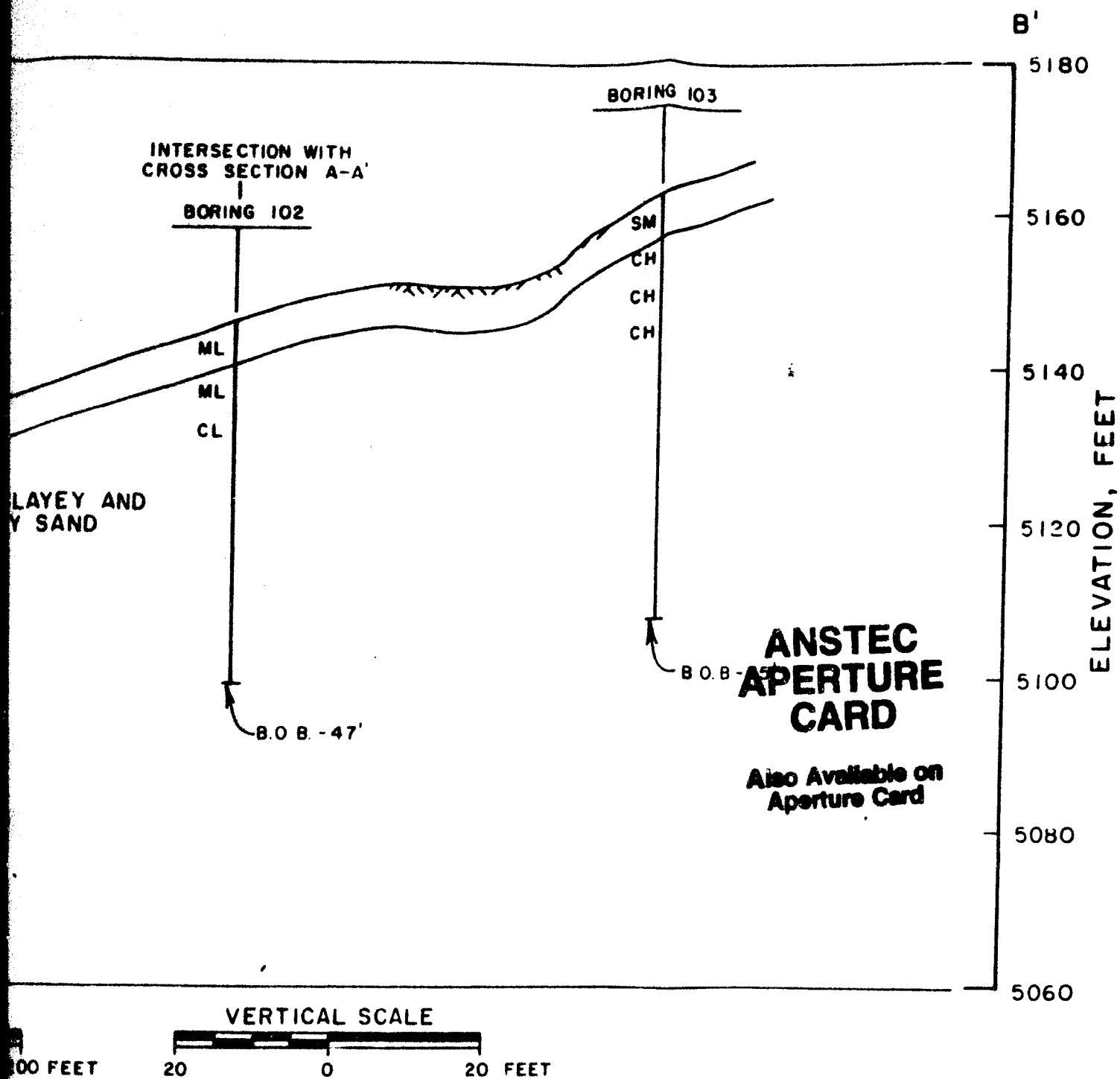
DRAWN BY: **D. Bricker**
 CHECKED BY: **4 Mar. 82**
 APPROVED BY: **TX**
 DRAWING RM78-682-B8
 NUMBER



THE DEPTH AND THICKNESS OF THE SUBSURFACE STRATA INDICATED ON THE SECTIONS WERE GENERALIZED FROM AND INTERPOLATED BETWEEN THE TEST BORINGS INFORMATION ON ACTUAL SUBSURFACE CONDITIONS EXISTS ONLY AT THE LOCATION OF THE TEST BORINGS AND IT IS POSSIBLE THAT SUBSURFACE CONDITIONS BETWEEN THE TEST BORINGS MAY VARY FROM THOSE INDICATED

LEGEND:

CH - LABORATORY SOIL CLASSIFICATION (UNIFIED SOIL CLASSIFICATION SYS)



**ANSTEC
APERTURE
CARD**

Also Available on
Aperture Card

FIGURE 3

SUBSURFACE CROSS SECTION B-B'

PREPARED FOR

ENERGY FUELS NUCLEAR, INC.
DENVER, COLORADO

IDENTIFICATION

NOTES:

1. FOR PLAN LOCATION OF CROSS SECTION, SEE FIGURE 1.
2. VERTICAL EXAGGERATION EQUALS 10 X.

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ATTACHMENT 2

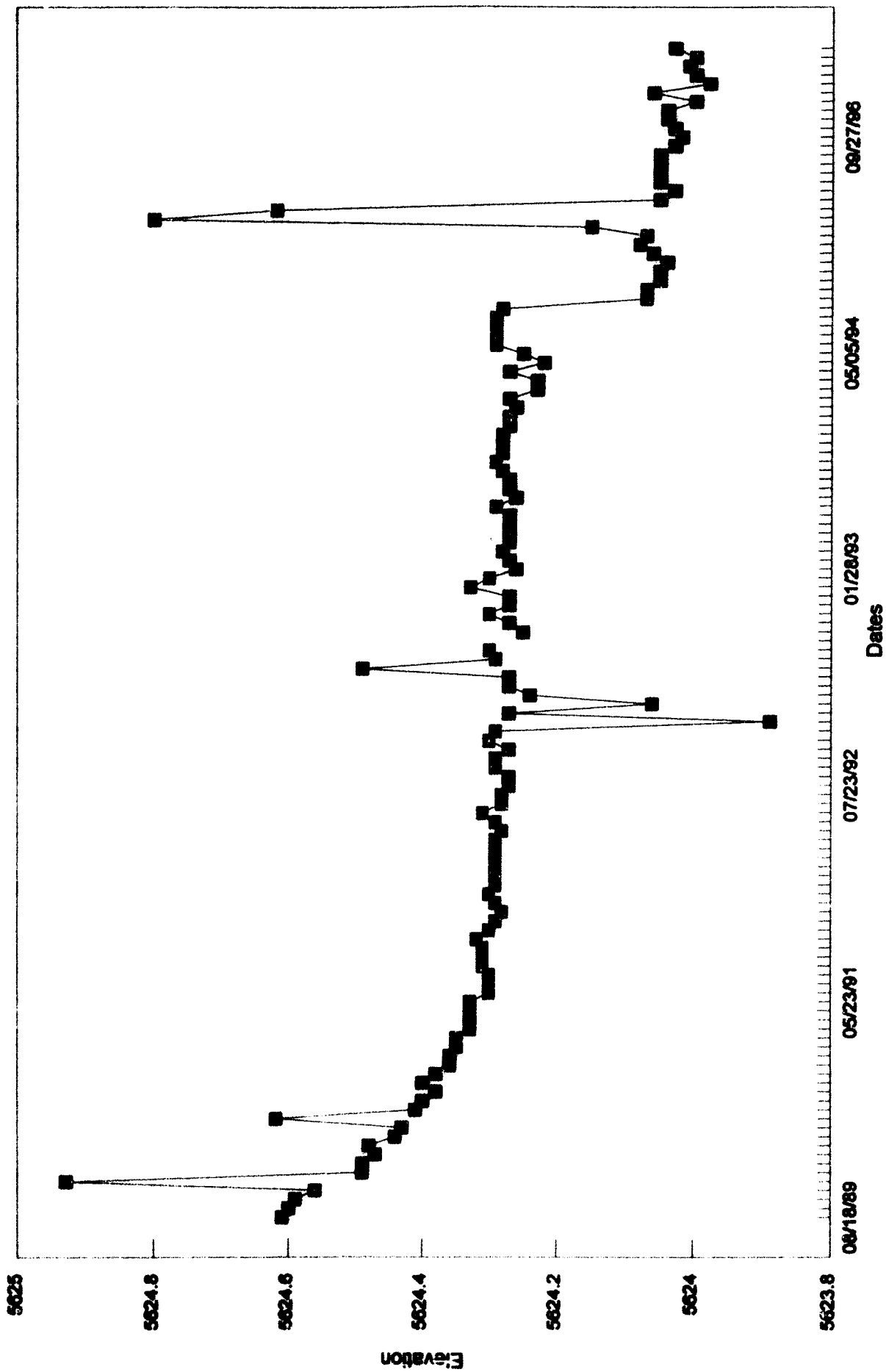
Time vs. Settlement Graphs

Cell 2

White Mesa Mill

Blanding, Utah

Cell 2 East Settlement Monitor

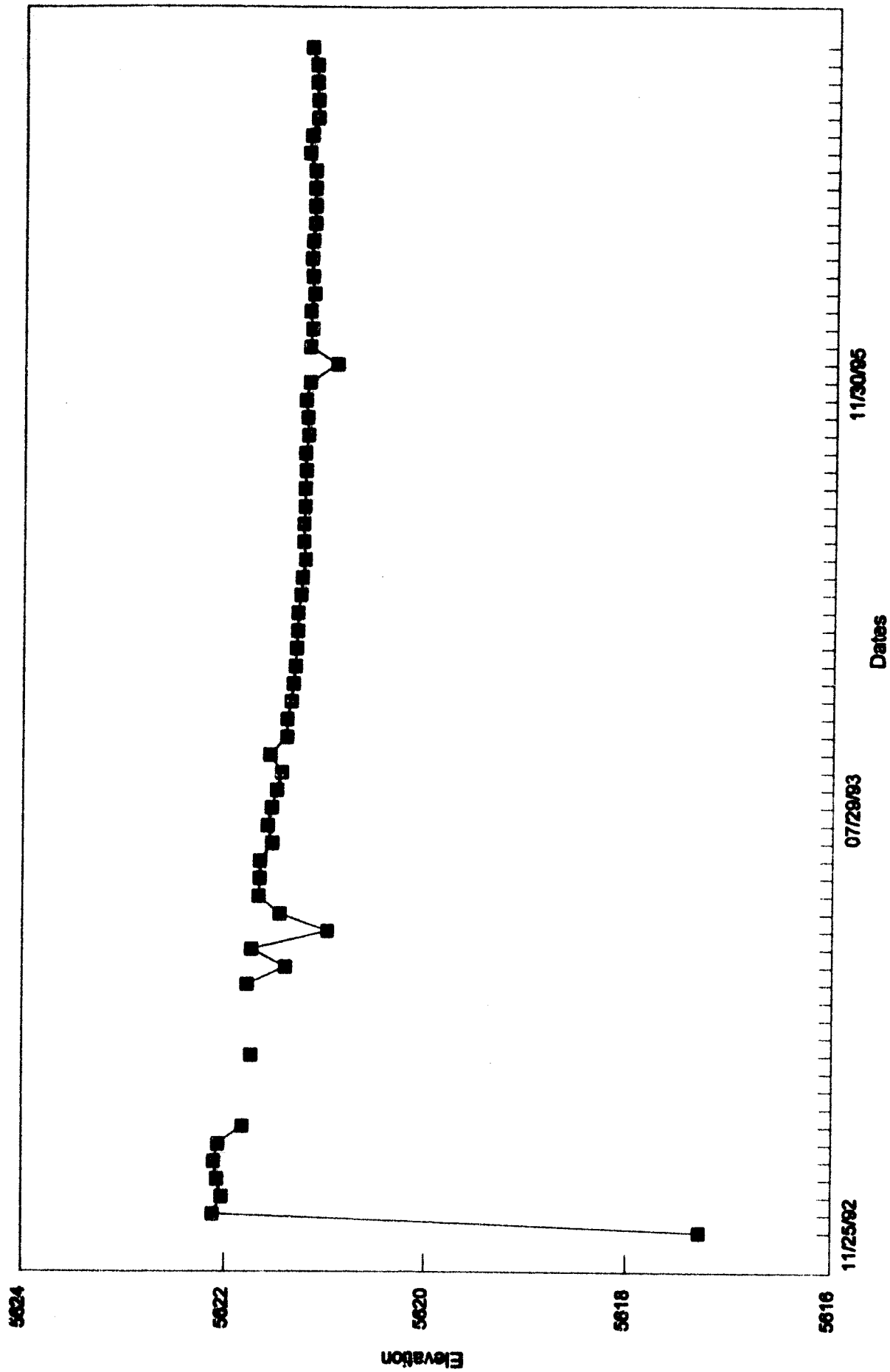


The graph illustrates the change in elevation over a five-year period. The Y-axis represents Elevation in feet, ranging from 5618.5 to 5620.5. The X-axis represents Dates, from 09/27/91 to 10/28/96. The data shows a general upward trend, starting at approximately 5618.8 feet in late 1991 and reaching about 5619.8 feet by late 1996. There are several notable fluctuations, including a sharp drop around 02/04/93 and a period of relative stability between 08/20/92 and 02/04/93.

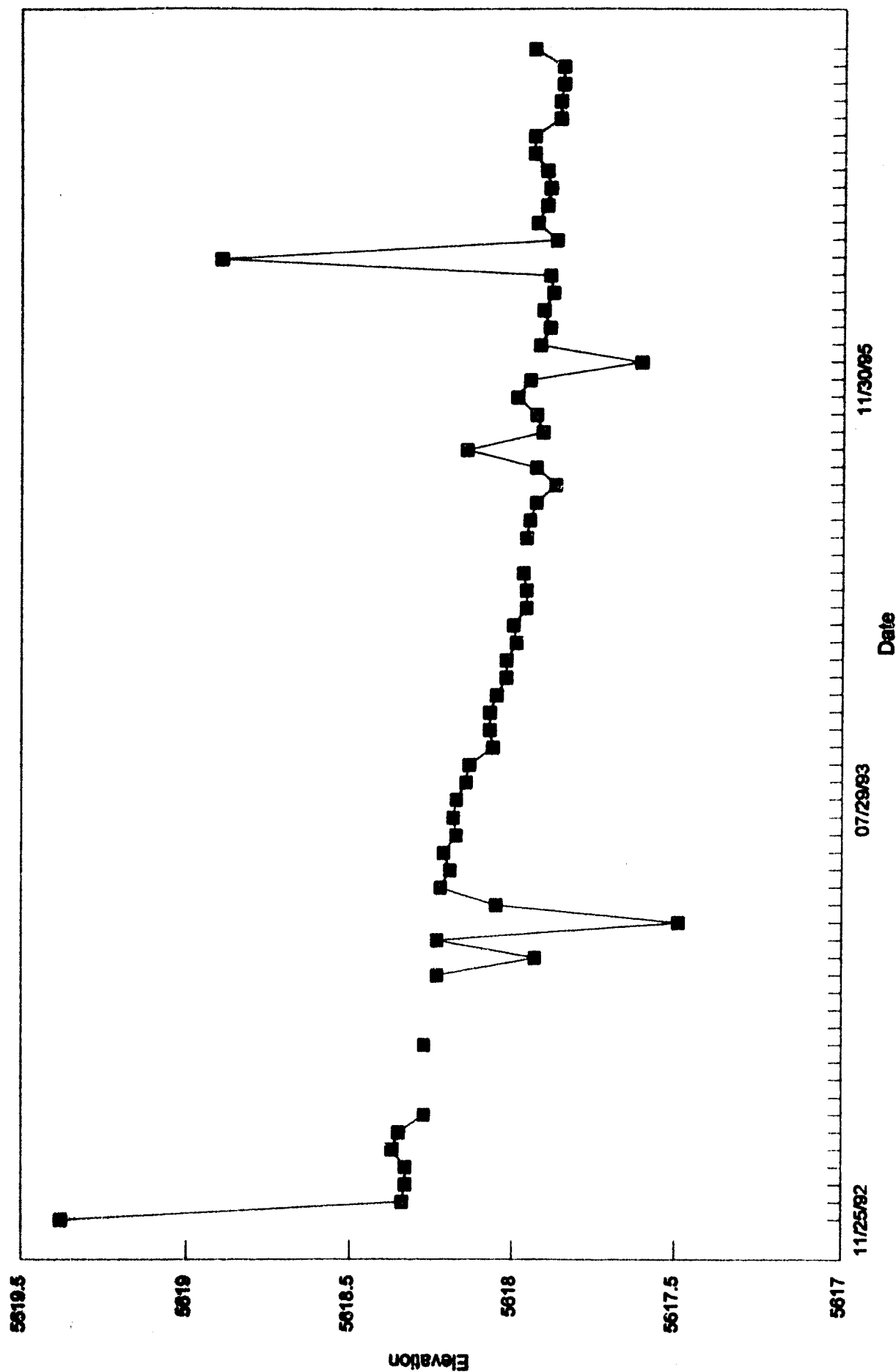
Date	Elevation (feet)
09/27/91	5618.8
10/01/91	5618.9
10/05/91	5619.0
10/09/91	5619.1
10/13/91	5619.2
10/17/91	5619.3
10/21/91	5619.4
10/25/91	5619.5
10/29/91	5619.6
11/02/91	5619.7
11/06/91	5619.8
11/10/91	5619.9
11/14/91	5620.0
11/18/91	5620.1
11/22/91	5620.2
11/26/91	5620.3
11/30/91	5620.4
12/04/91	5620.5
12/08/91	5620.6
12/12/91	5620.7
12/16/91	5620.8
12/20/91	5620.9
12/24/91	5621.0
12/28/91	5621.1
01/01/92	5621.2
01/05/92	5621.3
01/09/92	5621.4
01/13/92	5621.5
01/17/92	5621.6
01/21/92	5621.7
01/25/92	5621.8
01/29/92	5621.9
02/02/92	5622.0
02/06/92	5622.1
02/10/92	5622.2
02/14/92	5622.3
02/18/92	5622.4
02/22/92	5622.5
02/26/92	5622.6
03/01/92	5622.7
03/05/92	5622.8
03/09/92	5622.9
03/13/92	5623.0
03/17/92	5623.1
03/21/92	5623.2
03/25/92	5623.3
03/29/92	5623.4
04/02/92	5623.5
04/06/92	5623.6
04/10/92	5623.7
04/14/92	5623.8
04/18/92	5623.9
04/22/92	5624.0
04/26/92	5624.1
05/01/92	5624.2
05/05/92	5624.3
05/09/92	5624.4
05/13/92	5624.5
05/17/92	5624.6
05/21/92	5624.7
05/25/92	5624.8
05/29/92	5624.9
06/02/92	5625.0
06/06/92	5625.1
06/10/92	5625.2
06/14/92	5625.3
06/18/92	5625.4
06/22/92	5625.5
06/26/92	5625.6
07/01/92	5625.7
07/05/92	5625.8
07/09/92	5625.9
07/13/92	5626.0
07/17/92	5626.1
07/21/92	5626.2
07/25/92	5626.3
07/29/92	5626.4
08/02/92	5626.5
08/06/92	5626.6
08/10/92	5626.7
08/14/92	5626.8
08/18/92	5626.9
08/22/92	5627.0
08/26/92	5627.1
08/30/92	5627.2
09/03/92	5627.3
09/07/92	5627.4
09/11/92	5627.5
09/15/92	5627.6
09/19/92	5627.7
09/23/92	5627.8
09/27/92	5627.9
10/01/92	5628.0
10/05/92	5628.1
10/09/92	5628.2
10/13/92	5628.3
10/17/92	5628.4
10/21/92	5628.5
10/25/92	5628.6
10/29/92	5628.7
11/02/92	5628.8
11/06/92	5628.9
11/10/92	5629.0
11/14/92	5629.1
11/18/92	5629.2
11/22/92	5629.3
11/26/92	5629.4
11/30/92	5629.5
12/04/92	5629.6
12/08/92	5629.7
12/12/92	5629.8
12/16/92	5629.9
12/20/92	5630.0
12/24/92	5630.1
12/28/92	5630.2

16/27/91

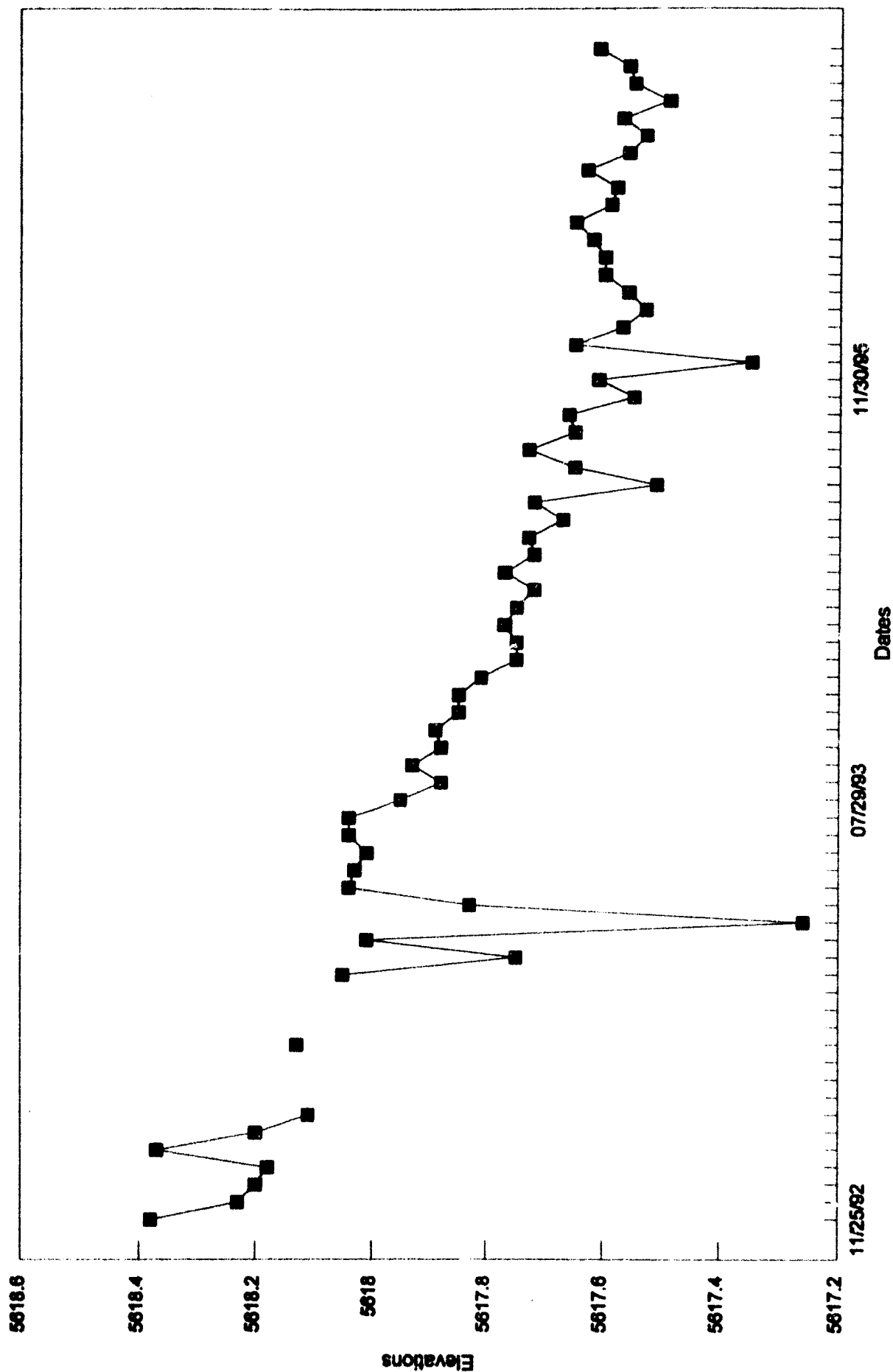
Cell 2 West 2 Settlement Monitor



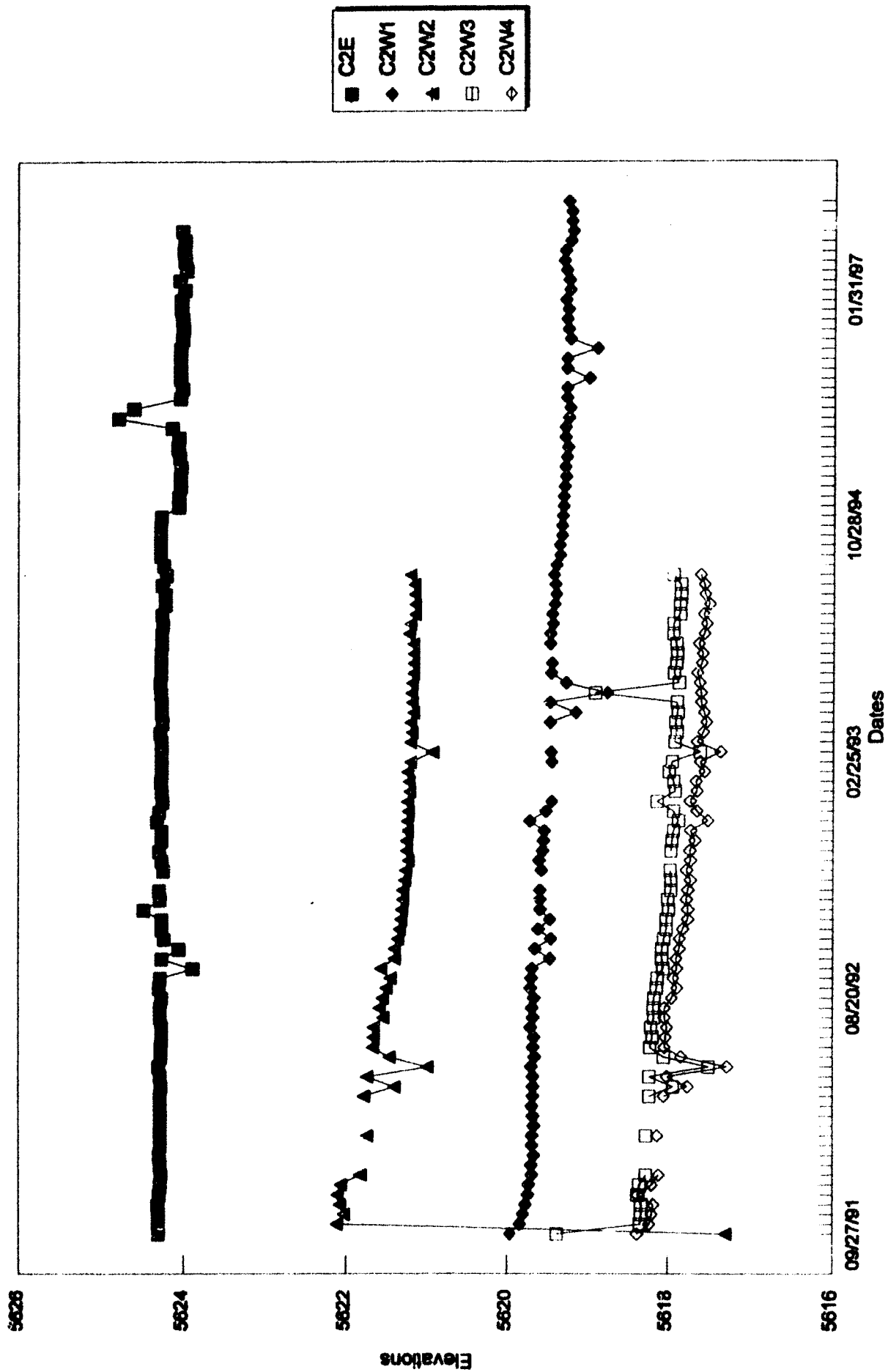
Cell 2 West 3 Settlement Monitor



Cell 2 West 4 Settlement Monitor



Cell 2 Settlement Monitors



ATTACHMENT 3

Replacement Pages

MAP - 71002307 - GEOTECHNICAL DATA BASE FOR MONTICELLO MILLSITE CHARACTERIZATION - R N MORRIS. CREATED 09/28/81. LAST UPDATED 11/22/81

AREA	DATA SOURCE	BOUND OR TEST PIT NUMBER	SAMPLE NUMBER & TYPE	TOP OF SAMPLE (FEET)	BASE OF SAMPLE (FEET)	SAMPLE DEPTH (FEET)	USCS SYMBOL	MATERIAL TYPE	WATER DRY DENSITY (PCF)	NATURAL MOISTURE CONTENT (PERCENT)	SPECIFIC GRAVITY	LIQUID PLASTICITY INDEX	FRACTION PASSING #200 SIEVE (PERCENT)	FRACTION PASSING #40 SIEVE (PERCENT)	ASTM D 698 MAXIMUM DRY DENSITY (PCF)	ASTM D 698 OPTIMUM MOISTURE CONTENT (PERCENT)	OTHER TESTS
CARBONATE PLE	SDHC	WAP-001	WIS-001ST	4.0	6.0	2.0	CL-MH	Tellurium	88.6	4.6	2.57	MP	100.0	97.8	NA	NA	Consistent with CUS @11 bar
CARBONATE PLE	SDHC	WAP-002	WIS-002ST	2.0	4.0	2.0	SM	Tellurium	107.5	4.0	2.60	MP	100.0	98.0	NA	NA	Consistent with CUS @11 bar
CARBONATE PLE	SDHC	WAP-003	WIS-003ST	1.0	1.0	0.0	SP	Over	NA	NA	NA	NA	100.0	100.0	14.7	14.3	
CARBONATE PLE	SDHC	WAP-004	WIS-004ST	3.0	3.0	0.0	SP	Tellurium	NA	NA	2.70	NA	NA	102.8	102.8	14.3	
CARBONATE PLE	SDHC	WAP-005	WIS-005ST	6.0	6.0	0.0	SP	Tellurium	NA	NA	2.87	NA	NA	94.8	94.8	28.3	
CARBONATE PLE	DAM	WAP-006	WIS-006ST	2.0	4.0	2.0	CL-MH	Tellurium	82.1	27.4	2.57	NA	100.0	94.0	NA	NA	Consistent with CUS @11 bar
CARBONATE PLE	DAM	WAP-007	WIS-007ST	2.0	4.0	2.0	CL	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-008	WIS-008ST	4.8	8.0	3.2	SP-SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-009	WIS-009ST	8.0	12.0	4.0	SP-SM	Tellurium	NA	NA	2.58	NA	100.0	11.4	NA	NA	
CARBONATE PLE	DAM	WAP-010	WIS-010ST	10.0	14.0	4.0	SP-SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-011	WIS-011ST	13.0	17.0	4.0	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-012	WIS-012ST	14.0	18.0	4.0	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-013	WIS-013ST	17.4	18.0	0.6	CL-MH	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-014	WIS-014ST	18.8	18.8	0.0	CL-MH	Tellurium	NA	NA	2.55	NA	100.0	NA	NA	NA	
CARBONATE PLE	DAM	WAP-015	WIS-015ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-016	WIS-016ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-017	WIS-017ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-018	WIS-018ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-019	WIS-019ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-020	WIS-020ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-021	WIS-021ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-022	WIS-022ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-023	WIS-023ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-024	WIS-024ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-025	WIS-025ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-026	WIS-026ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-027	WIS-027ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-028	WIS-028ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-029	WIS-029ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-030	WIS-030ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-031	WIS-031ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-032	WIS-032ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-033	WIS-033ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-034	WIS-034ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-035	WIS-035ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-036	WIS-036ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-037	WIS-037ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-038	WIS-038ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-039	WIS-039ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-040	WIS-040ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-041	WIS-041ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-042	WIS-042ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-043	WIS-043ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-044	WIS-044ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-045	WIS-045ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-046	WIS-046ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-047	WIS-047ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-048	WIS-048ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-049	WIS-049ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-050	WIS-050ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-051	WIS-051ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-052	WIS-052ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-053	WIS-053ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-054	WIS-054ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-055	WIS-055ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-056	WIS-056ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-057	WIS-057ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-058	WIS-058ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-059	WIS-059ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-060	WIS-060ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-061	WIS-061ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-062	WIS-062ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-063	WIS-063ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-064	WIS-064ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-065	WIS-065ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-066	WIS-066ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-067	WIS-067ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-068	WIS-068ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-069	WIS-069ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-070	WIS-070ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-071	WIS-071ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-072	WIS-072ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-073	WIS-073ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-074	WIS-074ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-075	WIS-075ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-076	WIS-076ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-077	WIS-077ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-078	WIS-078ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-079	WIS-079ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-080	WIS-080ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-081	WIS-081ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-082	WIS-082ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-083	WIS-083ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-084	WIS-084ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-085	WIS-085ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-086	WIS-086ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-087	WIS-087ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-088	WIS-088ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE	DAM	WAP-089	WIS-089ST	20.2	22.0	1.8	SM	Tellurium	NA	NA	NA	NA	NA	NA	NA	NA	
CARBONATE PLE																	

JFC
10/4/94

MAP - 11000007 - GEOTECHNICAL DATA - SE FOR MONTEBELLO MLLSITE CHARACTER - NON - R.N. MORRIS, CREATED 080801, LAST UPDATED 11/22/91

AREA	DATA SOURCE	BOXING OR TEST PIT NUMBER	SAMPLE NUMBER & TYPE	TOP OF SAMPLE (FEET)	BASE OF SAMPLE (FEET)	SAMPLE DEPTH (FEET)	USCS SYMBOL	MATERIAL TYPE	IN-PLACE DRY DENSITY (PCF)	NATURAL MOISTURE CONTENT (PERCENT)	FRACTION PASSING #100 sieve (PERCENT)	FRACTION PASSING #40 sieve (PERCENT)	LIQUID LIMIT (PERCENT)	PLASTICITY INDEX	ASTM D 1557 MAXIMUM DRY DENSITY (PCF)	ASTM D 1557 OPTIMUM MOISTURE CONTENT (PERCENT)	OTHER TESTS
CARBONATE PILE	DAM	TP-4	4BT	16.5	11.8	17.75	CL	Tabular	70.8	48.3	NA	NA	NA	NA	NA	NA	
CARBONATE PILE	DAM	TP-4	6BT	17.8	17.8	17.75	SM	Tabular	71.3	19.8	NA	NA	NA	NA	NA	NA	
CARBONATE PILE	DAM	TP-4	8BLLX	18.0	18.0	18.00	SM	Tabular(?)	71.3	19.8	NA	NA	NA	NA	NA	NA	
CARBONATE PILE	DAM	TP-11	8BLLX	18.0	18.0	18.00	CL	Tabular	NA	15.7	2.78	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-400BT	4.0	0.0	18.00	SC	Tabular	80.7	38.8	2.82	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-700BLLX	1.8	1.8	1.80	SP	Clay	NA	NA	2.87	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-700BLLX	2.8	2.8	2.80	SP	Clay	NA	NA	2.87	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-700BLLX	7.0	7.0	7.00	SP	Clay	NA	NA	2.87	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-700BLLX	8.0	8.0	8.00	CL	Tabular	70.1	41.7	2.80	NA	NA	NA	NA	NA	
VANADIUM PILE	BRIDGE	WAP-40-44	MIS-700BLLX	5.5	5.5	5.50	CL	Tabular	70.1	41.7	2.80	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-021	3BT	2.0	2.0	2.75	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	1000	3.5	3.5	4.25	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	2000	5.0	5.0	6.75	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	3000	8.5	8.5	10.25	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	6000	10.0	10.0	16.75	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	7000	11.5	11.5	18.25	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	9000	13.0	13.0	20.25	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	10000	18.0	18.0	18.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	11000	18.0	18.0	18.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	12000	18.0	18.0	18.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	13000	18.0	18.0	18.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	14000	21.5	21.5	23.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	14000	24.0	24.0	26.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	16000	27.5	27.5	29.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	16000	29.5	29.5	31.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	16000	31.5	31.5	33.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	16000	33.5	33.5	35.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	20000	38.0	38.0	39.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	21000	37.5	37.5	38.00	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	22000	38.5	38.5	39.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	23000	41.5	41.5	42.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	24000	43.5	43.5	44.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	24000	44.5	44.5	45.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	46.5	46.5	47.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	47.5	47.5	48.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	48.5	48.5	49.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	49.5	49.5	50.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	50.5	50.5	51.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	51.5	51.5	52.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	52.5	52.5	53.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	53.5	53.5	54.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	54.5	54.5	55.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	55.5	55.5	56.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	56.5	56.5	57.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	57.5	57.5	58.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	58.5	58.5	59.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	59.5	59.5	60.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	60.5	60.5	61.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	61.5	61.5	62.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	62.5	62.5	63.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	63.5	63.5	64.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	64.5	64.5	65.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	65.5	65.5	66.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	66.5	66.5	67.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	67.5	67.5	68.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	68.5	68.5	69.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	69.5	69.5	70.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	70.5	70.5	71.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	71.5	71.5	72.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	72.5	72.5	73.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	73.5	73.5	74.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	74.5	74.5	75.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	75.5	75.5	76.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	76.5	76.5	77.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	77.5	77.5	78.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	78.5	78.5	79.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	79.5	79.5	80.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	80.5	80.5	81.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	81.5	81.5	82.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	82.5	82.5	83.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	83.5	83.5	84.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	84.5	84.5	85.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	85.5	85.5	86.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	86.5	86.5	87.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	87.5	87.5	88.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	88.5	88.5	89.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	89.5	89.5	90.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	90.5	90.5	91.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	91.5	91.5	92.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	92.5	92.5	93.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	93.5	93.5	94.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	94.5	94.5	95.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	95.5	95.5	96.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	96.5	96.5	97.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	97.5	97.5	98.50	SM	Clay	NA	18.8	NA	NA	NA	NA	NA	NA	
VANADIUM PILE	DAM	318091-022	26000	98													

JPC
10/4/94

MAP - THESEPT - GEOTECHNICAL DATA BASE FOR MONTICELLO MILLSITE CHARACTERIZATION - R.N. MORRIS, CREATED ORIGIN: LACT UPDATED 11/28/81

AREA	DATA SOURCE	COMING OR TEST AT NUMBER	SAMPLE NUMBER & TYPE	TOP OF SAMPLE (FEET)	BASE OF SAMPLE (FEET)	SAMPLE DEPTH (FEET)	USCS SYMBOL	MATERIAL TYPE	WATER CONTENT (PERCENT)	MOISTURE SPECIFIC GRAVITY (PERCENT)	FRACTION PASSING #200 (PERCENT)	FRACTION PASSING #40 (PERCENT)	LIQUID LIMIT	PLASTICITY INDEX	ASTM D 698 MAXIMUM DRY DENSITY (PCF)	OPTIMUM MOISTURE CONTENT (PERCENT)	OTHER TESTS
EAST PILE	CDM	TP-1	18T	6.5	7.0	0.5	CL	SW	11.1	102.3	NA	NA	NA	NA	NA	NA	10.1 Penn. Mod. Pres.
EAST PILE	CDM	TP-1	20T	7.0	7.5	0.5	CL	SW	10.2	102.3	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-1	21T	8.5	9.0	0.5	CL	SW	10.2	102.3	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-1	22T	14.0	14.5	0.5	SM	SW	7.4	101.9	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-1	23T	14.0	14.5	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	18T	8.5	9.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	19T	10.0	10.5	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	20T	10.5	11.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	21T	13.5	14.0	0.5	SM	SW	11.7	102.3	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	22T	14.0	14.5	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-2	23T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	24T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	25T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	26T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	27T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	28T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	29T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	30T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	31T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	32T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	33T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	34T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	35T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	36T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	37T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	38T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	39T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	40T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	41T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	42T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	43T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	44T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	45T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	46T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	47T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	48T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	49T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	50T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	51T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	52T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	53T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	54T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	55T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	56T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	57T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	58T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	59T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	60T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	61T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	62T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	63T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	64T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	65T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	66T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	67T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	68T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	69T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	70T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	71T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	72T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	73T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	74T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	75T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	76T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	77T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	78T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	79T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	80T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	81T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	82T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	83T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	84T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	85T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	86T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	87T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	88T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	89T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	90T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	91T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	92T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	93T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	94T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	95T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	96T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	97T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	98T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	99T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	100T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	101T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	102T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA	NA	NA	NA
EAST PILE	CDM	TP-4	103T	14.5	15.0	0.5	SM	SW	10.0	102.0	NA	NA	NA	NA</			

COMP. NUMBER - DISTICTIONAL DATA BASE FOR MONITORING ALL SITE CHARACTERISTICS...ION - R.N. MORRIS: CREATED SOURCE: LAST UPDATED 11/22/91

AREA	SOIL SOURCE	TESTING NUMBER	SAMPLE NUMBER & TYPE	TOP OF SAMPLE (FEET)	DATE OF SAMPLE (MONTH)	SAMPLE WEIGHT (POUNDS)	USCS SYMBOL	MATERIAL TYPE	WATER CONTENT (%)	NATURAL DRY DENSITY (PCF)	SPECIFIC GRAVITY	FRACTION PASSING NO. 200 (PERCENT)	FRACTION RETAINED NO. 200 (PERCENT)	LIQUID LIMIT (%)	PLASTICITY INDEX	ASTM D-1557 CLASSIFICATION	ASTM D-1557 OPTIMUM MOISTURE CONTENT (%)	OTHER TESTS
ADP-1	SOIL	174	174	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-2	SOIL	175	175	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-3	SOIL	176	176	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-4	SOIL	177	177	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-5	SOIL	178	178	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-6	SOIL	179	179	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-7	SOIL	180	180	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-8	SOIL	181	181	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-9	SOIL	182	182	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-10	SOIL	183	183	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-11	SOIL	184	184	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-12	SOIL	185	185	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-13	SOIL	186	186	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-14	SOIL	187	187	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-15	SOIL	188	188	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-16	SOIL	189	189	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-17	SOIL	190	190	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-18	SOIL	191	191	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-19	SOIL	192	192	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-20	SOIL	193	193	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-21	SOIL	194	194	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-22	SOIL	195	195	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-23	SOIL	196	196	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-24	SOIL	197	197	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-25	SOIL	198	198	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-26	SOIL	199	199	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-27	SOIL	200	200	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-28	SOIL	201	201	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-29	SOIL	202	202	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-30	SOIL	203	203	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-31	SOIL	204	204	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-32	SOIL	205	205	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-33	SOIL	206	206	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-34	SOIL	207	207	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-35	SOIL	208	208	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-36	SOIL	209	209	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-37	SOIL	210	210	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-38	SOIL	211	211	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-39	SOIL	212	212	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-40	SOIL	213	213	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-41	SOIL	214	214	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-42	SOIL	215	215	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-43	SOIL	216	216	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-44	SOIL	217	217	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-45	SOIL	218	218	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-46	SOIL	219	219	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-47	SOIL	220	220	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-48	SOIL	221	221	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-49	SOIL	222	222	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-50	SOIL	223	223	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-51	SOIL	224	224	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-52	SOIL	225	225	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-53	SOIL	226	226	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-54	SOIL	227	227	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-55	SOIL	228	228	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-56	SOIL	229	229	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-57	SOIL	230	230	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-58	SOIL	231	231	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-59	SOIL	232	232	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-60	SOIL	233	233	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-61	SOIL	234	234	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-62	SOIL	235	235	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-63	SOIL	236	236	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-64	SOIL	237	237	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-65	SOIL	238	238	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-66	SOIL	239	239	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-67	SOIL	240	240	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-68	SOIL	241	241	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-69	SOIL	242	242	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-70	SOIL	243	243	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-71	SOIL	244	244	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-72	SOIL	245	245	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-73	SOIL	246	246	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-74	SOIL	247	247	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0.0	10.0	1.0	NA	14.0	Pen. Test, CLAY
ADP-75	SOIL	248	248	1.5	3.5	2.5	CL	CLAY	17.5	120.0	2.65	100.0	0					

SLOPE STABILITY ANALYSIS (SECTION BB BY UMETC)

5620'

5600'

5580'

(310, 5568)

(0, 5550.5)

(0, 5540)

CHEN #77

TOP OF
5520' LAYER
FOR

0'

100'

200'

300'

5500'

2.6, 5732.5

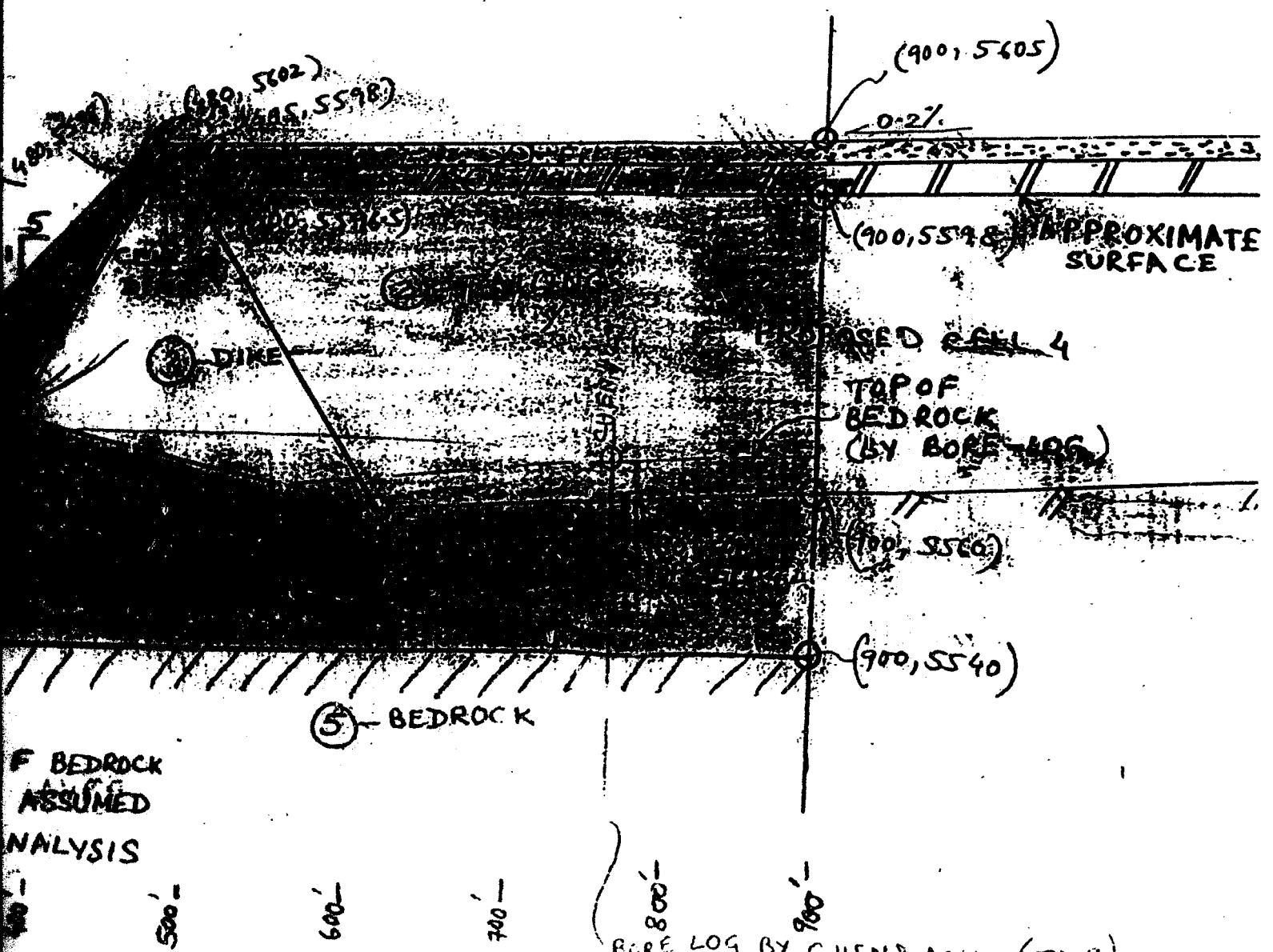
5732.5
- 5567.0

165.5

ANSTEC APERTURE CARD

Also Available on
Aperture Card

ANALYSIS ALONG CELL-4 DIKE



BORE LOG BY CHEN & ASSOC. (TYP)
① - DENOTES ELEVATION OF
BED-ROCK ENCOUNTERED

FIGURE 1

9801090276-04
FIGURE 1

ATTACHMENT 4

**Specification for
Construction of Rock Covers and Other Erosion Protection
on the Tailings Cells**

**SPECIFICATION
FOR
CONSTRUCTION OF ROCK COVERS AND OTHER EROSION PROTECTION
ON THE TAILING CELLS**

PART 1 - GENERAL

1.1 Project Description (to be added)

The rock covers consist of two different covers:

- a. Top cover - A layer of rock covering all portions of the tailing cell tops, a surface with gradients less than 0.10. This cover will be not less than _____ feet thick and will consist of rock with a d_{50} not less than _____ inches.
- b. Side slope cover - A two-part cover consisting of a lower _____ foot thick bedding layer of d_{15} not larger than _____ and d_{50} not larger than _____ and an upper _____ foot thick layer of rock with a d_{50} not less than _____ inches.

Other erosion protection to be constructed includes:

- c. Side slope toe apron - A 10-foot wide extension of the upper layer of the side slope rock cover along the _____ side slope toes of the tailing cells.

The rock to be used for the rock covers and other erosion protection is sandstone. This rock will be obtained from _____.

The work to be performed consists of loading and hauling the rock, placing the rock on the radon barrier surfaces and toe apron surfaces, finish-grading the rock cover surfaces.

1.2 Technical Definitions

d_{50} : The size, in mean diameter, of the rock material of which 50% by weight is finer.

Earthwork control grid: Orthogonal system of uniformly spaced lines (integer multiples of 100 feet), based on the coordinate system and survey control points to be established on the site, used to record locations, thicknesses, lateral extents, and types of earthwork performed each day.

Fines: Mineral particles passing the #200 U.S. Standard sieve; i.e. smaller than 0.075 mm grain size.

Foreign material: Any solid material that is not sandstone. Includes wood, iron and steel, plastic, rubber, glass, ceramic and concrete.

Job site: The location of the tailing cells as well as all access routes, borrow areas, equipment laydown locations and storage areas on Owner property used in the Included Work.

Native soil, natural soil: Naturally-occurring alluvial or residual soils existing below and at ground surface around the job site; consisting of gravel, sand, silt and clay materials.

Tailing cell: Cell # 2 or #3

Planarity: The degree to which a surface approaches a flat (but not necessarily horizontal) surface

Sand: Mineral particles with grain sizes between #200 and #4 sieve (0.075 mm to about 5 mm).

Tailings: Solid byproduct of uranium ore milling, consisting of particles of primarily silicate minerals and containing radioactive elements (mostly uranium and radium). Particle sizes range from clay (less than 0.002 mm) to medium sand (less than #40 sieve).

1.3 List of Construction Drawings

The following drawings are incorporated into this specification by reference:

(to be added)

1.4 Included Work

The activities required for rock cover and other erosion protection construction will be performed by the Contractor using its own or subcontracted labor and equipment. The Included Work, described in detail in Part 2, consists of:

- a) Preparation of haulage routes
- b) Rock placement: Loading, hauling and placement of rock for rock cover layers, riprap and toe aprons
- c) Scour protection trenches: Excavation and backfilling of soil; loading, hauling and placement of rock for construction of scour protection trenches, if required.
- d) Dust control: Operation of water pumping, distribution and spray systems to suppress fugitive wind-blown dust in all work areas.

1.5 Related Work Performed by Others

- a) Earthwork quality control: Sampling and testing to verify rock properties at the quarry site and gradations and thicknesses of placed rock
- b) Quality control surveying: Surveying for verifying line and grade and for pay-quantity determination.

1.6 Responsibilities

- a) International Uranium Corporation, or IUC, the "Owner", will provide controlled access to the work site, will make available construction water at locations on the mill property and will approve and make payment for work performed under this specification. The Owner will perform surveys to verify rock properties, to measure gradations and thicknesses of placed rock, and to verify finished lines and grades and placed-rock quantities.
- b) "Engineer" will review or inspect and advise the Owner on the acceptance of the Included Work. The Engineer will specify and review quality control measures.
- c) Contractor shall provide all equipment, materials, labor and supplies and perform all work necessary to accomplish the Included Work. Contractor shall be responsible for the knowledge of and compliance with all applicable federal, state, and local laws and regulations and for the safety of its job site and of all personnel and equipment which it employs and all others who are present on the job site. Contractor shall be responsible for limiting size segregation of rock materials during hauling and placement controlling thicknesses of rock layers, and achieving specified lines and grades of rock layers and finished rock cover surfaces.

PART 2 - EXECUTION

The Contractor shall perform the following work:

2.1 Haul Route Preparation and Maintenance

The Contractor shall select, prepare and maintain one or more haul roads from the rock stockpiles to the tailing cells. Preparation shall include:

- a. Clearing of vegetation and removal to an on-site disposal location approved by the Owner. Vegetation may be burned in lieu of removal for disposal.
- b. Preservation and protection of wells, water lines, and power lines needed for water supply or for the Owner's ground water restoration.
- c. Preservation and protection of power lines, telephone lines and other utilities along rights-of-way crossed by the haul route(s).
- d. Hauling and placement of soil or rock to construct the haul road surfaces. The Contractor may use any rock or soil it deems appropriate for this purpose. If the source of the rock or soil to be used is located on the Owner's property, the Contractor shall identify the location, types and volumes of material needed, submit a plan for regrading and revegetation of the borrow location, and obtain the Owner's approval before using that source.

- e. Maintaining the haul road(s), including dust control, for the entire period of use.
- f. Regrading and revegetation of both the haul-road construction material borrow site and the haul road(s) in accordance with a plan prepared by the Contractor and approved by the Owner.

Fences may be temporarily removed where they cross the haul route(s) provided that if any license-boundary (security) fence is breached, a guard shall be posted at each such location during working hours and all such openings shall be closed during non-working hours.

2.2 Loading, Hauling and Placement of Rock

The Contractor shall load and haul rock from the stockpile at _____, designated by the Owner, to placement locations on the tailing cells.

All rock used for rock covers and erosion protection shall be sandstone from _____.

2.2.1 Rock Covers

Rock covers shall be 90%-125% of the following thicknesses:

cell top	_____ feet
cell bedding (side slope)	_____ feet
cell side slope	_____ feet

A bedding layer will be placed on all side slope surfaces before placement of rock cover or riprap on those surfaces.

Rock for covers shall be loaded, hauled and placed by methods that maintain the gradation ranges in the stockpiled rock and prevent segregation of sizes during transport and placement.

The rock shall be placed and spread to create a uniform surface on the rock cover that is free of visible high or low spots. The planarity of the surface will be acceptable if irregularities of the surface do not exceed +/- 1.0 feet vertical difference from the design gradient surface over 100 feet and +/- 0.5 feet vertical difference within any 10-foot segment of a 100-foot survey line. On the rounded corners of the tailing cells this irregularity criterion shall apply along radial lines down the slope, perpendicular to the elevation contours.

2.2.2 Toe Apron

Along the south side slope toes of the tailing cells, the rock cover will be extended 10 feet beyond the toe of the side slope, as shown on Drawing _____. This toe apron will consist of the same rock sizes and gradations as the side slope rock cover and will be constructed so that the surface of the toe apron slopes away from the toe and the outer edge of the top surface is at the same elevation as the adjacent ground surface.

2.3 Dust Control

The Contractor shall employ those measures necessary to minimize dust from its operations. Unless otherwise approved by the Owner, acceptable measures are limited to spraying or other method of applying water to ground surfaces.

PART 3 - QUALITY CONTROL

The Contractor shall take all measures necessary to achieve all requirements of Part 2 of this specification. These measures shall include, as a minimum, the following:

3.1 Supervision

During all times that the Contractor's equipment or personnel are performing Included Work on the job site, the Contractor supervisor shall be present to direct the work. The supervisor shall have experience, satisfactory to Owner, in the type of work being executed. The supervisor shall have on-hand at all times a copy of the current revision of this specification and the drawings relevant to the work. The supervisor shall have the authority to make decisions for the Contractor in all matters related to parts 2 and 3 of this specification.

3.2 Line and Grade and Planarity Control

The Contractor shall perform land surveying to determine that the specified lines and grades and planarity have been achieved in accordance with the limits established in this specification. Ground control for surveys shall be based on established benchmarks and other control points on the mill property and tailing cells as shown on Drawing _____. Gradients shall be surveyed as often as necessary to control rock placement.

If any part of the rock layer surface appears by visual examination of the Owner to exceed the planarity limits, that part shall be surveyed to quantify the magnitude of irregularities. All final gradients and elevations shall be recorded on base drawings that include the site coordinate system, the earthwork control grid, and the topographic contours of the surfaces prior to fill placement. Base drawings will be provided in hard copy or Autocad (current version) plot file on 3.5-inch diskette.

When the Contractor reports to Owner that all Included Work has been completed, Owner will perform an acceptance survey to determine if line and grade requirements have been satisfied. Owner will survey the elevations and gradients at such locations as may be necessary. At its discretion, Owner may choose to have this survey done by aerial photogrammetry.

3.3 Field and Laboratory Testing of Rock and Rock Placement

Testing of rock for the necessary properties and gradations will be performed on rock in the stockpiles at the quarry by a qualified materials testing service contracted by Owner. The Contractor shall have no responsibility for the rock until it removes rock from the stockpiles. The testing service will perform measurements and tests to determine size gradations and layer thicknesses of the placed rock according to the following frequencies:

- a. Visual inspection of rock delivered to the site and rock placement will be performed at least once daily.
- b. Visual inspection of rock cover surfaces will be performed at least once in each control grid cell (100 feet x 100 feet) to evaluate surface uniformity and planarity. If the visual inspection results in uncertainty or dispute about adequacy of planarity at any location, the location shall be surveyed by rod and level, or other method of at least equal accuracy, to determine if allowable limits of surface irregularity are exceeded along 100-foot long horizontal and 20-foot slope-gradient lines of a 20-foot square grid covering the location in question. The allowable limits are +/- 1.0 feet vertical difference from the design gradient surface over 100 feet and +/- 0.5 feet vertical difference within any 10-foot segment of a 100-foot survey line. This requirement does not negate or substitute for rock thickness testing required below.
- c. One size and gradation test using a portable screen stack shall be performed for every 5000 cy of rock or bedding placed on the Tailing cell.
- d. Rock and bedding layer thicknesses shall be measured at least once per 2000 cy placed.

PART 4 - DOCUMENTATION

4.1 - Documentation by Contractor

The Contractor shall record and report, in a format acceptable to Owner, the following information:

Daily journal containing list of equipment used, hours worked, reimbursable materials consumed or used, and labor hours by wage category. The journal will also record Included Work tasks started, completed, and in progress and the units of work accomplished (e.g., volume of rock placed, area of final grading). Submit a copy to Owner by the start of the next working day.

Daily Work Summary listing all pay items and quantities. Submit by the start of the next working day.

Earthwork Control Plot, using the earthwork control grid at a scale of not less than 1 inch = 200 feet, showing the location, area, extent, and thickness of bedding or rock material placed accomplished each day.

Survey notes for line and grade and planarity control (verbally report results immediately, and submit copy to Owner within 24 hours)

Written notifications to Owner of unexpected conditions, conditions that prevent conformance with specifications, disputes over acceptance of Contractor's work. Verbally notify Owner immediately upon discovery or identification, submit in writing within 24 hours.

4.2 - Documentation by Owner

Owner will create and maintain the following documentation that relates to the Included Work:

Field inspection notes of Contractor's performance, work accomplished, and observed variances from the specification.

Records of all field and laboratory tests performed by Owner and its testing service.

Photographic and video tape records of the Included Work.

Chronological record of notifications to the Contractor of variances from specifications, unacceptable work performance, discrepancies in payment quantities claimed by the Contractor, and all related resolutions thereto.

Survey notes and calculations of the acceptance survey

As-built drawings of completed work

PART 5 - ACCEPTANCE

Owner shall have sole discretion to accept in part or in full, or to reject in part or in full, the Contractor's work. Acceptance or rejection will be based on Owner's visual inspections (including those of its Engineer and testing service), quality control data required under Part 3, and documentation required under Part 4.

Upon identification of unacceptable work, Owner will notify the Contractor of the deficiency. The notification will include the location, extent, and description of the unacceptable work. Before proceeding with additional work at that location the Contractor shall correct the deficiency by bringing the work into compliance with specifications and drawings to the satisfaction of Owner. All work and materials required for such corrective actions shall be at the expense of the Contractor.

PART 6 - SCHEDULE

Complete the Included Work by (to be added) days from Notice to Proceed.

ATTACHMENT 5

Revised Calculations

CALCULATION SHEET

PAGE 1 OF 5

PROJECT NO.

CLIENT TRC SUBJECT 5:1 (H.V)

Prepared By MMW Date 11-6-97

PROJECT WHITE MESA RIPRAP SIZING

Reviewed By JRM Date 11-7-97

Approved By Date

NRC Response to Comments

OBJECTIVE

DETERMINE SIZE OF RIPRAP ALONG 5:1 (H.V) SLOPES.

ASSUMPTIONS

- USE STEPHENSON'S METHOD FOR SIZING D_{50} OF RIPRAP
- 5 DIFFERENT FLOW PATHS ARE EVALUATED. EACH FLOW PATH IS DIVIDED INTO REACH 1 AND REACH 2. REACH 1 IS THE FLOW PATH OVER CELL 2, REACH 2 IS THE FLOW PATH ACROSS CELL 3. DESIGN FLOWS AND VELOCITIES ARE COMPUTED FOR REACH 1 AND COMBINED REACH 1/REACH 2.
- CALCULATIONS ARE SIMILAR TO CALCULATIONS PROVIDED BY TITAN. THE FLOWPATH LENGTHS ARE REVISED TO REFLECT THE ENTIRE FLOW PATH ACROSS THE COVER.
- MODIFY RIPRAP D_{50} BASED ON OVERSILING FOR ROCK QUALITY.
- THICKNESS OF RIPRAP LAYER IS $2 \times D_{50}$ PER NURTH/CR-4620
- TIME OF CONCENTRATION CALCULATED USING EQU 4.4 NURTH/CR-4620
- $PMP = 7.76''$
- $PEAK FLOW = Q = CIA$

CALCULATION SHEET

PAGE 2 OF 5

PROJECT NO. _____

CLIENT ENC SUBJECT S:1 (H.V)

Prepared By MW Date 11-6-97

PROJECT WHITE MASH RIPRAP 3.2 INCH

Reviewed By JEM Date 11-7-97

NRC RESPONSE TO COMMENTS

Approved By _____ Date _____

CALCULATIONS

① STEPHENSON'S METHOD

$$D_{50} = \left[\frac{q_c (\tan \theta)^{7/6} n_p^{1/6}}{C \sqrt{g} [(1-n_p) G_s - 1] (\cos \phi' (\tan \phi - \tan \theta))^{5/3}} \right]^{2/3} \times \frac{12 \text{ INCH}}{\text{FT}}$$

WHERE θ = SLOPE ANGLE = 11.31°

n_p = POROSITY = 0.30

C = STEPHENSON CONSTANT = 0.22 FOR GRAVEL
= 0.27 FOR CRUSHED GRANITE

g = 32.2 ft/s

G_s = RELATIVE DENSITY OF ROCK = 2.48

ϕ = FRICTION ANGLE OF ROCK = 40°

K = OLINGER'S CONSTANT = 1.2 FOR GRAVEL
FOR STABILITY = 1.0 FOR CRUSHED GRAVEL

q_c = PERM FLOW

② TIME OF CONSUMPTION

$$T_c = (0.00013 (L)^{0.77} / (L)^{0.385}) (60 \text{ min/hr})$$

T_c = TIME OF CONSUMPTION, MINUTES

L = SLOPE LENGTH, FT

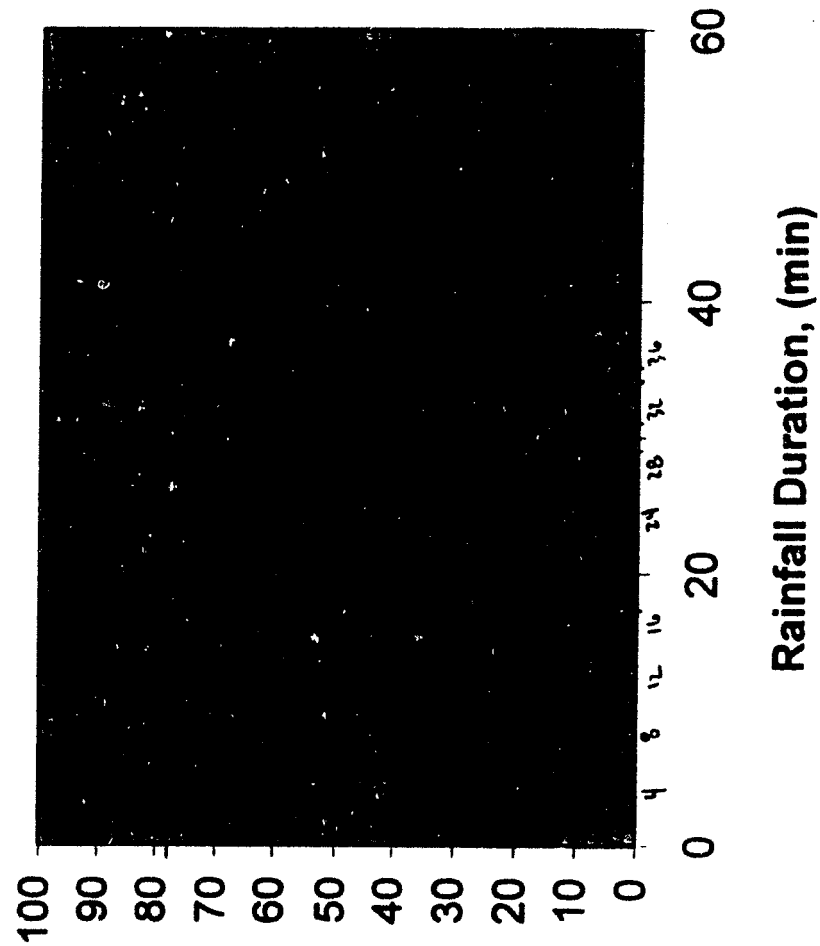
i = SLOPE, FT/FT

REACH 1													
	0.20% slope	5:1(h:v) slope		unit width								peak flow	
Flow Path	length ft.	length ft.	total length ft.	drainage area ac.	time of conc. min.	% pmp from table 2.1 NUREG	1 - hr precip. inches	rainfall depth inches	rainfall intensity in/hr	runoff coefficient	per unit width cfs	flow concent. factor	Design Flow cfs
1	0	0	0	0	0	0	7.78	0	0	0.8	0	2	0
2	720	15	735	0.0189	13.6	71	7.78	5.51	24.2	0.8	0.33	2	0.65
3	800	15	815	0.0187	14.8	73	7.78	5.66	23.0	0.8	0.34	2	0.69
4	1010	15	1025	0.0235	17.7	78	7.78	6.05	20.5	0.8	0.39	2	0.77
5	1130	20	1150	0.0284	19.3	81	7.78	6.29	19.5	0.8	0.41	2	0.83
6	1210	20	1230	0.0282	20.3	82	7.78	6.36	18.6	0.8	0.42	2	0.85
REACH 2													
	0.20% slope	5:1(h:v) slope		unit width									
Flow Path	length ft.	length ft.	total length ft.	drainage area ac.	time of conc. min.								
1	980	80	1060	0.0241	17.4								
2	925	150	1075	0.0247	17.1								
3	900	260	1160	0.0264	17.1								
4	880	220	1070	0.0246	16.3								
5	850	190	1040	0.0239	16.2								
6	790	40	830	0.0191	14.8								
COMBINED REACH 1 AND REACH 2													
	total length ft.	drainage area ac.	time of conc. min.	% pmp from table 2.1 NUREG	1 - hr precip. inches	rainfall depth inches	rainfall intensity in/hr	runoff coefficient	peak flow per unit width cfs	flow concent. factor	Design Flow cfs		
1	1060	0.0241	17.4	78	7.78	6.05	20.9	0.8	0.40	2	0.81		
2	1810	0.0418	30.7	89	7.78	6.91	13.5	0.8	0.45	2	0.90		
3	1885	0.0451	31.9	90	7.78	6.98	13.1	0.8	0.47	2	0.95		
4	2085	0.0481	34.0	91	7.78	7.06	12.5	0.8	0.48	2	0.98		
5	2180	0.0503	35.5	91	7.78	7.06	11.9	0.8	0.48	2	0.96		
6	2080	0.0473	35.1	91	7.78	7.06	12.1	0.8	0.46	2	0.91		
VELOCITY: REACH 1													
Flow Path	Design Flow cfs	manning's coefficient n	slope ft/ft	flow depth ft.	velocity ft/s								
1	0.00	0.03	0.20	0.00	0.0								
2	0.65	0.03	0.20	0.12	5.4								
3	0.69	0.03	0.20	0.12	5.5								
4	0.77	0.03	0.20	0.13	5.8								
5	0.83	0.03	0.20	0.14	5.9								
6	0.85	0.03	0.20	0.14	6.0								
VELOCITY: COMBINED REACH 1 AND REACH 2													
Flow Path	Design Flow cfs	manning's coefficient n	slope ft/ft	flow depth ft.	velocity ft/s								
1	0.81	0.03	0.20	0.06	6.0								
2	0.90	0.03	0.20	0.15	6.1								
3	0.95	0.03	0.20	0.15	6.3								
4	0.98	0.03	0.20	0.15	6.3								
5	0.96	0.03	0.20	0.15	6.3								
6	0.91	0.03	0.20	0.15	6.2								

$$VELOCITY = \frac{Q}{D}$$

0	0
2.5	27.5
5	45
10	62
15	74
20	82
30	89
45	95
60	100

Table 2.1 of NUREG 4620



CALCULATION SHEET

PAGE 4 OF 5

PROJECT NO. _____

CLIENT INCC SUBJECT 5:1 (W/V)

Prepared By MWJ Date 11-6-97

PROJECT WHITE MESA RIPRAP STABILIZATION

Reviewed By JRM Date 11-7-97

Approved By _____ Date _____

NBC RESPONSE TO COMMENTS

REMARKS (COMMENTS)

Flow Path	ρ_c CIS	C	D_{50} mm	Oversize K	Modified D_{50} mm
1	0	-	-	-	-
2	0.65	0.27	2.7	1.8	4.9
3	0.69	0.27	2.9	1.8	5.2
4	0.77	0.27	3.1	1.8	5.6
5	0.83	0.27	3.2	1.8	5.8
6	0.85	0.27	3.3	1.8	5.9

Flow Path	D_{50} BASED ON SPECIFICATIONS, mm	Oversize PERCENT	Revised D_{50} mm	$2 \times D_{50}$	Recommended Thickness
1	-	-	-	-	18"
2	4.9	1.25	6.1	12.2	18"
3	5.2	1.25	6.5	13.0	18"
4	5.6	1.25	7.0	14.0	18"
5	5.8	1.25	7.2	14.4	18"
6	5.9	1.25	7.4	14.8	18"

CALCULATION SHEET

PAGE 5 OF 5

PROJECT NO. _____

CLIENT IVC SUBJECT 5:1 (H.V)

Prepared By MWW Date 11-6-97

PROJECT WHITE MESA RIPRAP SIZING

Reviewed By JRM Date 11-7-97

NRL RESPONSE TO COMMENTS

Approved By _____ Date _____

CONCLUSIONS

- RIPRAP $D_{50} = 8"$

THICKNESS = 18"