

Northeast Church Rock 95% Design Report

Appendix A: General Design Information

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LIST OF ACRONYMS / ABBREVIATIONS

| | |
|-----------|---|
| AOC | Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery |
| ARAR | Applicable or Relevant and Appropriate Requirement |
| BMP | best management practice |
| CC | Construction Contractor |
| CSF | construction support facility |
| CY | cubic yard(s) |
| GSR | Green and Sustainable Remediation |
| ICIAP | Institutional Control Implementation Assurance Plan |
| LED | light emitting diode |
| LEED | Leadership in Energy and Environmental Design |
| Mill Site | Church Rock Mill Site |
| Mine Site | Northeast Church Rock Mine Site |
| MSOC | Mine Site outlet channel |
| NECR | Northeast Church Rock |
| NRC | US Nuclear Regulatory Commission |
| OM&M | Operations, Monitoring and Maintenance |
| PDSP | Phased Development Site Plan |
| PTW | principal threat waste |
| RA | Removal Action |
| RAL | removal action limit |
| RAO | Remedial Action Objective or Removal Action Objective |
| ROD | Record of Decision |
| SOW | Scope of Work |
| TDA | Tailings Disposal Area |
| USEPA | US Environmental Protection Agency |

A.1 INTRODUCTION

This appendix to the Northeast Church Rock (NECR) 95% Design Report summarizes the general design information and includes:

- A description of the general section (Section 1) of the design drawings
- Reference to how the design addresses the Performance Standards contained in the Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Site (2011 Action Memo; USEPA, 2011), the Record of Decision, United Nuclear Corporation Site (ROD; USEPA, 2013), and the Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery (AOC; USEPA, 2015)
- A summary of the Removal Action (RA) elements and construction progression
- Green and Sustainable Remediation (GSR) considerations

A.2 PERFORMANCE STANDARDS

The Performance Standards presented here are defined 2011 Action Memo (USEPA, 2011), the ROD (USEPA, 2013), and the AOC (USEPA, 2015) including the Statement of Work attached as Appendix D to the AOC, and were developed to define attainment of the Removal Action and Remedial Action Objectives (RAOs) for the Selected Remedy. The Performance Standards include both general and specific standards applicable to the Selected Remedy work elements and associated work components. Table A.2-1 presents performance standards related to general design and explains how the design accomplishes these standards. In addition, each subsequent design appendix provides a similar performance standards table based on the table of performance standards included in the main text.

Table A.2-1: Performance Standards Applicable to General Design

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|---|--|--|
| 14 | 2015 AOC SOW, Paragraph 29 – Green Remediation Best Management Practices | Green Remediation Best Management Practices | Respondents shall incorporate applicable Best Management Practices for Green Remediation listed in ASTM-E2893-13 consistent with USEPA's policy Superfund Green Remediation Strategy (2010), found at http://www.epa.gov/superfund/greenremediation/sf-gr-strategy.pdf . | A general overview of GSR is discussed in Section A.5. The individual appendices include a description of GSR specific to the design components. |
| 16 | 2015 AOC SOW, Paragraph 32 – Data Gaps | Data Gaps | If USEPA notifies Respondents that there are data gaps that must be addressed by field investigations or by additional analyses not specifically identified in this SOW, or if the Respondents identify such data gaps, then the Respondents shall submit to USEPA for review and approval an addendum to the PDSP that will include work plans for additional investigations and/or reports necessary as determined by USEPA to support the Design. Respondents shall perform the data gap investigations identified in the addendum to the PDSP once each work plan has been approved by USEPA. Respondents shall also submit reports documenting the results of each of the supplemental pre-design investigations within 60 days of completion of the investigation and receipt of any associated laboratory and/or geotechnical data. | An additional investigation was conducted after the 30% Design, a geotechnical evaluation of the Jetty area. This evaluation was conducted in November 2016 following USEPA approval of the work plan. The geotechnical evaluation report was submitted to USEPA with the interim deliverable for Appendix I on April 24, 2017, and is included as Attachment I.8 to Appendix I of the 95% Design Report. The results of laboratory testing conducted prior to April 2017 on samples collected from this investigation are included in Attachment I.8 to Appendix I. Laboratory testing results for geotechnical and agronomic testing conducted after April 2017 are summarized in Appendix H and Attachment U.2 to Appendix U, respectively. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|--------------------|---|---|
| 17 | 2015 AOC SOW, Paragraph 37 – Preliminary Design | Preliminary Design | <p>Respondents shall submit to USEPA for review and approval a Preliminary Design based on the USEPA-approved preferred waste-Repository configuration when the design effort is approximately 30% complete but no later than 120 days from USEPA's approval of the Design Work Plan. Respondents shall include the following elements in their Preliminary Design:</p> <ul style="list-style-type: none"> a. Finalized design assumptions and parameters from the Basis of Design/Design Criteria Report; b. Preliminary plans, drawings, and sketches, including design calculations; c. An outline of required specifications; d. A plan for additional field sampling, if needed; e. A traffic safety plan, including upgrades to local roads; f. A storm-water management plan; g. An air monitoring plan; h. A site control and security plan; i. A project delivery strategy; j. A preliminary construction schedule, including a schedule for applicable permit requirements; k. A material management plan that shall describe how all NECR Site mine waste, borrow material, backfill material, and cover material will be managed and transported. The material management plan shall include a map and description with coordinates of routes that will be used by heavy equipment and a map and description with coordinates of staging and stockpiling areas. For staging and stockpiling areas, the material management plan shall include a description of activities that will be associated with these areas; l. A description of how the principal threat waste will be segregated and a description of the facility or facilities being considered for disposal of Principal Threat Waste from the NECR Site; m. Detailed plans and specifications for backfilling and re-grading excavated areas of the NECR Site, which address the impacts of re-grading of storm water runoff on and downstream of the NECR Site; n. A permitting requirements and compliance plan that shall ensure all on-site activities meet the substantive (but not the administrative) requirements of environmental permitting regulations; | <p>These items are addressed in the following parts of this report:</p> <ul style="list-style-type: none"> a. Appendices B through I b. Plans in Volume 2, calculations in Attachments to Appendices B through I c. Appendix J d. Appendix I e. Appendix M f. Appendices E, F, and I g. Appendix Q h. Appendix M i. Main text j. Appendix K k. Appendices B, C, D and H l. Appendices B and C m. Appendix C n. Appendix N o. Appendix U p. Appendix G q. Appendix G r. Appendix V s. Appendix M, Q t. Appendices O and P u. Tables throughout the main text and appendices v. Appendix T w. Appendix U |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|------------------|--|---|
| | | | <p>o. A revegetation plan which shall describe the approach that Respondents shall take to revegetate the borrow areas, and other disturbed areas on the UNC Site;</p> <p>p. A water-balance model report that provides the following: A description of how water would behave in the short-term and over an extended period of time within the enhanced design of the tailings impoundment; and, if the water-balance model indicates that, as a result of Repository construction, there may be increased water seepage from the impoundment, analysis explaining how increased seepage would not adversely impact the existing groundwater;</p> <p>q. An evapotranspiration analysis which shall describe the complete water-balance model assumptions and calculations for the Repository cover system. Respondents' evaluation shall show the percolation response to design parameters such as rooting depth, the type of flora, cover thickness, cover soil properties, initial moisture content of the cover soils, and hydraulic conductivity;</p> <p>r. An outline of the construction quality assurance plan;</p> <p>s. Mitigation measures to minimize traffic, noise, dust and any other impacts to the community and environment</p> <p>t. Biological and cultural resources surveys or reports;</p> <p>u. A description of how the design complies with all Performance Standards, including ARARs;</p> <p>v. A description of procedures for cleanup verification at the NECR Site (including the step out areas), including an updated QAPP for verification sampling; and,</p> <p>w. A description of procedures for revegetation of the NECR Site, including the approach that Respondents shall use to revegetate the NECR Site and to maintain revegetated areas until vegetation is established.</p> | |
| 19 | 2015 AOC SOW, Paragraph 42 – Preliminary Design | Pre-Final Design | Respondents shall submit to USEPA for review and approval a Pre-Final Design which represents 95% completion. The Pre-Final Design shall be based on the USEPA approved Preliminary Design, or if one has been requested, the USEPA approved Intermediate Design, and shall include the following additional items: | <p>These items are addressed in the following parts of this report:</p> <p>a. Appendix L</p> <p>b. Appendix X</p> <p>c. Appendix W</p> <p>d. Appendix R</p> |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|--|---|--|
| | | | a. A health and safety plan; b. An institutional control implementation and assurance plan ("ICIAP") that includes plans for implementing, maintaining, monitoring, and reporting on any institutional controls proposed in the design; c. A draft operation, monitoring, and maintenance plan; d. A release prevention/contingency plan; e. An emergency response plan; f. A construction quality assurance plan; | Incorporated into a. Appendix L b. Appendix V |
| 20 | 2015 AOC SOW, Paragraph 43 – Preliminary Design | Pre-Final NECR Mine Cleanup Verification and Revegetation Plan | Respondents shall submit a Pre-Final NECR Mine Cleanup Verification and Revegetation Plan for the NECR Site that shall be a continuation and expansion of the Preliminary NECR Mine Cleanup Verification and Revegetation Plan, and any Intermediate Design. | The Pre-Final Cleanup Verification Plan is included as Appendix T and the Pre-Final Revegetation Plans are included in Appendix U |
| 76, 79, 80 | 2011 Action Memo, Table A-1; 2013 ROD, Table 1 and Sections 2.9.2 and 2.9.5 | Repository Design | 40 CFR 192.02 (a) through (d). Control of residual radioactive materials. Refer to www.ecfr.gov . | The Repository cover and associated stormwater controls are designed to be effective for up to 1,000 years and at least 200 years, and to minimize reliance on active maintenance. The cover will attenuate radon flux to a rate of rate of 20 pCi/m ² -s, averaged over the surface and minimize infiltration into the existing Tailings Disposal Area (TDA). Also see appendices G and I. |

*Refers to identifying numbers listed in Summary of ARARs, Performance Standards and Applicable NRC Design Requirements Table (provided in Attachment 1 to main text of the 95% Design Report)

A.3 ENGINEERING DESIGN DRAWINGS

The general section (Section 1) of the design drawings contains information related to site location, access, general location of existing and proposed facilities, site boundaries, survey control points, and standard symbols and abbreviations used in subsequent Drawings. The Section 1 drawings also depict the principal remedial components as well as the design Site topography and layout upon completion of the RA. The NECR engineering design drawings are contained in Volume II – Design Drawings. Drawings related to the general project information and the site overview are listed in Table A.3-1.

Table A.3-1: Engineering Design Drawings

| Drawing No. | Drawing Title |
|-------------|---|
| 1-01 | Cover Sheet |
| 1-02 | Sheet Index |
| 1-03 | General Notes and Acronyms |
| 1-04 | Site Location Map |
| 1-05 | Principal Existing Condition, Site Features, and Survey Control |
| 1-06 | Existing Condition NECR Mine Site |
| 1-07 | Existing Condition UNC Site |
| 1-08 | Site Utilities |
| 1-09 | Principal Components and Limits of Disturbance |
| 1-10 | Final Reclaimed Topography |

A.4 REMOVAL ACTION CONSTRUCTION PROGRESSION

The RA will comprise an Early Works element, the RA, a final demobilization and revegetation element, and a schedule-independent Pipeline Arroyo stabilization element, as summarized in the following sections. The preliminary RA schedule is included in Appendix K – Removal Action Schedule.

A.4.1 Early Works

The Early Works element includes preparation of the construction support facilities (CSFs), construction of access and haul roads, preparation of borrow areas, implementation of environmental monitoring, and implementation of stormwater and traffic controls.

A.4.1.1 Construction Support Facilities

The CSFs will be prepared for use during all phases of RA construction, and include security, construction laydown areas, construction water and fuel storage, Decontamination Area facilities (including vehicle decontamination pad, drainage controls, and personnel facilities such as showers, lockers and laundry), and facilities required for handling principal threat waste (PTW). The CSFs are shown on the Section 2 Drawings. Additional details regarding the CSFs are included in Appendix B – Construction Support Facilities.

Selected locations for the CSFs include the former mill facilities area of the Church Rock Mill Site (Mill Site), an area at the east end of the NECR Mine Site (Mine Site), and two proposed laydown yards: one west of the Tailings Disposal Area (TDA) and one immediately north of the Mill Site TDA. These areas are identified on the Drawings as:

- Former Mill Site Yard
- Mine Site Staging Area
- Repository Yard
- Optional Repository Yard

Additionally, support facilities are organized in areas using the following terms and definitions for the various work areas:

- Support: Area(s) free of contamination.
- Controlled: Area(s) with potential contamination.
- Exclusion: Area(s) with contamination subject to the RA (Mine Site and Repository).
- Decontamination Area: The transition area between the Controlled Area and the Support Area. This is where personnel enter and exit the Controlled Area and where most decontamination activities take place.

CSF construction activities will include:

- Site grading
- Infrastructure construction followed by placement of temporary support facilities
- Post-construction radiological surveys of staging areas and support facilities

A.4.1.2 Access and Haul Roads

Several access and haul roads will be constructed to support the RA. These road details are summarized below and shown on the Section 4 Drawings. Additional details regarding haul and access roads are included in Appendix D – Haul Routes.

Temporary access roads will be constructed to provide access to the CSFs in the Former Mill Site Yard and the Repository Yard(s).

A mine waste haul road will be constructed to haul mine waste excavated at the Mine Site to the Repository located at the Mill Site. The haul road will begin at the Mine Site and will be located roughly parallel to New Mexico Highway 566 (NM 566), until it crosses the highway near the north end of the TDA.

Haul roads will be constructed to access each of the four proposed borrow areas. Plans and profiles for the north, east, and west borrow haul roads are shown on the Section 4 Drawings. Haul road construction would be conducted from each borrow area to the edge of the TDA. Once on the TDA, borrow haul trucks would operate directly on the existing cover surface. Upon completion of the RA, areas of the Repository cover and the TDA cover subjected to haul traffic would be reconstructed to mitigate over-compaction of cover soils, or other damage that may occur from haul traffic.

The anticipated sequence of road construction will be:

- Access road construction, including stormwater controls
- Mine waste haul road construction, including stormwater controls
- Borrow haul road construction, including stormwater controls
- Reclamation of haul roads
- Post-construction radiological surveys of impacted areas of the NM 566, and haul and access roads,

A.4.1.3 Borrow Areas

Approximately 510,000 cubic yards (CY) of soil and rock are required for Repository construction plus approximately 150,000 CY of rock and other materials for stormwater structures, including general fill to meet surface design grades, for the cover system, and long-term stormwater controls. Five proposed on-site borrow areas have been identified to meet volume and material property requirements. These borrow areas are identified as the East, West, North, and South Borrow Areas, and are shown on the Section 8 Drawings. An additional proposed borrow source, the jetty excavation, was identified during the 95% NECR design. At least one of these borrow areas will be developed as they are needed for Repository construction. Other borrow areas may be developed as needed during the RA.

Other borrow sources include on-site stockpiles and off-site commercial pits or quarries. Additional details regarding the borrow materials are included in Appendix H – Borrow Areas.

A.4.2 Removal Action

The RA element consists of preparation of the Repository to receive mine waste, mine site removals, Repository cover construction, and construction of permanent stormwater controls at the Mill and Mine sites.

A.4.2.1 Repository

Mine waste will be placed in a Repository constructed on the TDA as shown on the Section 7 Drawings and summarized below. Additional details regarding the Repository design are included in Appendix G – Mine Waste Repository Design.

The existing radon barrier above the tailings in the TDA will be modified in-place to serve as the foundation layer for the Repository. The sequence for radon barrier improvement is as follows:

- Baseline gamma radiation survey
- Rock mulch and riprap removal

- Visual inspection of radon barrier to verify tailings were not exposed during the removal process
- If tailings have been exposed, conduct a post-excavation (one-minute) static gamma survey and excavate radon barrier in the vicinity of one of the branch swales so that the material can be placed back below the radon barrier
- Filling swales
- Radon barrier compaction

Following preparation of the radon barrier, the initial lifts of the perimeter stormwater berms will be constructed using clean borrow soils at the edge of waste. These berms will allow for containment of contact water within the Repository during waste placement. Waste will be spread in lifts for compaction. The perimeter slopes of the waste surface will be built as the material is placed and the stormwater berms can be raised, as needed, to provide clean cover material over the outer slopes of the waste.

Once the berms are no longer needed for stormwater control, the berms will be graded over the waste surface during placement of the soil cover layer.

Cover placement will be conducted after areas of the Repository reach their design capacity. The cover design entails an erosion protection layer consisting of a rock soil admixture layer overlying a soil layer. The thicknesses of these layers and the sizes of the rock used for erosion protection vary based on locations on the Repository, with an overall cover thickness (including erosion protection layers) of 4 feet.

A.4.2.2 Mine Site

The Mine Site removals are divided into six phases of excavation, with a total estimated volume of about 761,000 CY. Mine site removals will include:

- Excavation of soils within the Mine Site above bedrock and with measured activity concentrations above the 2.24 pCi/g radium-226 removal action limit (RAL) or uranium concentrations above 230 mg/kg
- Disposal of mine site debris
- Transportation of excavated materials to the Mill Site Repository location or off-site as required for PTW
- Confirmation surveys to demonstrate that remaining materials within the Mine Site have measured activity concentrations below USEPA action levels
- Construction and maintenance of temporary stormwater controls during removal activity
- Mine Site excavation area final grading
- Mine Site restoration

Additional details regarding Mine Site removals are included in Appendix C – Mine Site Removals Excavations and Demolition. Additional details regarding verification surveys are included in Appendix T – Cleanup Verification Plan. Additional details regarding temporary stormwater controls are included in Appendix E – Stormwater Management Plan, as well as the other parts of the design documents.

A.4.2.3 Permanent Stormwater Controls

A.4.2.3.1 Mine Site Stormwater Controls

As part of the RA, the Mine Site outlet channel (MSOC) will be modified to convey stormwater from the Mine Site to minimize scouring of the existing engineered channel and unimproved sections of the downstream Unnamed Arroyo No. 1. These modifications will contain the predicted 100-year flood so that it does not impact homes located near the MSOC (Unnamed

Arroyo No. 1). The proposed channel improvements are shown on the Section 6 Drawings and will be constructed upon completion of the mine site removals. Additional detail is included in Appendix F – Mine Site Stormwater Controls.

A.4.2.3.2 Mill Site Stormwater Controls

Permanent stormwater controls for the Mill Site Repository use existing swales and channels constructed for the TDA with improvements and supplemental controls where necessary. These stormwater controls, shown on the Section 9 Drawings, include:

- North Diversion Channel
- East Repository Channel and related sediment controls
- Repository southwest and west side drainage

The East Repository Channel and related sediment controls will be constructed early in the RA to provide upstream stormwater control and to avoid construction constraints due to limited space if they are constructed after waste placement. The remaining Mill Site stormwater controls will be constructed after waste placement is completed and it is verified that any downstream contamination in these areas has been mitigated. Additional detail is included in Appendix I – Mill Site Stormwater Controls.

A.4.3 Demobilization and Revegetation

Upon completion of the RA, areas subject to mine waste removals will be graded as shown in the Section 3 Drawings, borrow areas will be graded as shown in the Section 8 Drawings, and the Repository cover surface will be graded as shown in the Section 7 Drawings.

Haul roads, access roads, and ground areas used for CSFs within the Exclusion Area will also be subject to final cleanup and verification in accordance with Appendix T. Trailers and equipment used within the Exclusion Area will be scanned and decontaminated (if required). Reclamation would consist of removal of imported gravel surfacing, removal of temporary culverts and stormwater controls, and grading according to the final approved post-reclamation grading plans.

Revegetation will be conducted in accordance with the plans in Appendix U – Revegetation Plans.

A.4.4 Pipeline Arroyo Stabilization

The Pipeline Arroyo is an existing ephemeral arroyo that runs along the northwest side of the TDA. Stability of the Pipeline Arroyo is important for long-term viability of the Repository and the TDA, as lateral southeastward migration of the arroyo could create embankment erosion, with significant erosion resulting in release of mine waste or tailings. The Section 9 Drawings show the design for the reconstructed rock jetty with a riprap chute designed for floods up to the Probable Maximum Flood. Additional detail on the design for the reconstructed rock jetty is included in Appendix I – Mill Site Stormwater Controls.

A.5 GREEN AND SUSTAINABLE REMEDIATION CONSIDERATIONS

Section 4.2.1 of the 95% Design Report includes an overview of the GSR evaluation and applicable best management practices (BMPs) that have been selected for implementation at the NECR RA. Specific GSR concepts to various parts of the design are included in subsequent design appendices. Steps 1 and 2 of the BMP Process involves identifying potential applicable BMPs to each phase of cleanup activities (ASTM, 2016). Table A.5-1 presents potentially applicable BMPs to the NECR RA, which BMPs have been determined to be appropriate for implementation at the site and why some have not been considered.

Table A.5-1: BMPs Selected during Step 1 and Step 2 of the BMP Process with Projected Impact and Decision to Carry BMP into Implementation

| BMP | Impact ¹ | Selected for Implementation | Location in RDR | Comment |
|---|---------------------|-----------------------------|-----------------|---|
| Use of energy star compliant equipment and premium-efficiency motors as available and prudent | Medium | Yes | B.7.1 | |
| Purchase of renewable energy for CSFs. This includes using renewable energy (photovoltaic cells and/or small wind turbines) for power. Use electric generators in place of diesel/gasoline generators if it is possible to extend local power lines to site or purchase of green energy from utility (e.g., Blue Sky Program) | Medium | Yes | B.7.1 | Use of alternative energy to supplement existing power will be suggested for contractors but will ultimately be at the discretion of the Construction Contractor (CC). |
| Transporting workers from centralized carpool and bus locations to the work site | Medium | Yes | B.7.2 | |
| Maintaining a single point of entry/exit to the remedial area helps prevent re-contamination of areas previously remediated while minimizing required support facilities. | High | Yes | B.7.3 | |
| Use of LEED-certified portable structures or if LEED-certified structures are not cost effective, utilize LEED principles when possible. This can include use of low energy light bulbs (i.e. LEDs), use of passive cooling instead of air conditioning when possible | Medium | Yes | B.7.1 | Will include in specifications but will ultimately be at the discretion of the CC. |
| Minimize site grading for construction facilities and associated roads to reduce required construction equipment operating time, greenhouse gas emissions and fill material | High | Yes | B.7.1 | Construction facility grading and site wide grading has been designed with the aim of minimizing required grading and cut/fill. |
| Use of 'Green' concrete with a percentage of fly ash if concrete is required | Medium | Yes | F.6.1, E.7.1 | CSFs will not utilize concrete |
| Consolidate CSFs in one area to minimize disturbance of remedial area and reduce emissions used to drive from facility to facility | Low | Yes | B.7.3 | |
| Plan for access roads to be used as permanent roads if needed, this reduces the requirement for construction of new roads at the end of the remedial activities | High | No | | Access roads are not planned to be used as permanent roads. |
| Use alternative vehicles such as electric vehicles, hybrid vehicles and compressed natural gas vehicles to reduce on-site emissions and fuel use | Low | No | | Such requirements would likely limit number of local contractors eligible for bidding. |
| Optimizing number of vehicles used for support activities | Medium | Yes | B.7.2 | Construction contractors will be instructed to optimize vehicle use and size. |
| Use products with recycled and bio-based contents instead of petroleum based | Low | Yes | B.7.1 | Construction contractors will be encouraged to use recycled products when possible. |
| Designate collection points for routine recycling of single-use items such as metal, plastic, and glass containers; paper and cardboard; and other items that may be recycled locally | Low | Yes | B.7.3 | |
| Utilize existing facilities for construction support facilities when possible | Medium | Yes | B.7.1 | |
| Plan excavation and placement to avoid moving material more than once, reducing the fuel and emissions created by multiple stock-piling and moving locations | Medium | Yes | D.6.3 | |
| Sizing equipment correctly with the task needs thereby minimizing use of heavy equipment for small tasks | High | Yes | D.6 | |
| Phasing of construction activities to avoid recontamination of remediated areas | High | Yes | C.4.6 | |
| If possible, use rails in place of trucks shipping of contaminated waste | Low | No | | No rail routes near site |
| Implementation of a no-idle policy and speed limit signs for all construction equipment and support vehicles | High | Yes | B.7.2 | Idle restrictions will be required of construction contractors through Technical Specifications (Appendix J) |
| Routine, on-time maintenance of equipment to improve efficiency and prevent unnecessary breakdown requiring additional resources and transport for repairs | Medium | Yes | B.7.2 | |
| Require new, energy efficient engines | Medium | Yes | B.7.2 | Require use of Tier 2 non-road diesel engines or better. Concerns that stricter requirements would make local contractors uncompetitive, which represents a major GSR goal. |
| Use of ultra-low sulfur diesel in construction equipment and support vehicles, including use of biodiesel where possible | Medium | Yes | B.7.2 | |

| BMP | Impact ¹ | Selected for Implementation | Location in RDR | Comment |
|--|---------------------|-----------------------------|--------------------|---|
| Requiring low-maintenance multistage filters for cleaner engine exhaust | Low | No | | Vehicle requirements, as described above, will be utilized to reduce emissions. |
| Use biodegradable fabric to cover excavated areas which act to control erosion and serve as substrate for regrowth | Medium | Yes | U Section 2.4 | |
| Reusing covers that secure and cover material in open trucks during off-site transport | Low | Yes | C.6.1 | |
| Limit speeds (10-15 MPH) in order to reduce production of dust and improve fuel efficiency | High | Yes | D.6.1 | |
| Where possible, use biodegradable tarps and mats for dust suppression rather than using water | Low | Yes | C.4.6/E.7.1 | |
| Use phosphate-free detergents in place of organic solvents or acids for decontamination of equipment | Low | Yes | Appendix J | Use of phosphate-free detergents will be required in specifications. |
| Evaluate potential of using excavated areas as retention basins in final stormwater control plans | High | Yes | H.4.1.1 | Soil excavated during the jetty improvement work will be utilized as clean fill. |
| Re-grade excavation areas to conform to pre-mining topography, rather than altering the site's natural setting, to improve the cover's long-term performance and protect local ecosystem services. | Medium | Yes | Appendix G | Where possible reclamation of disturbed areas will attempt to return disturbed areas to their natural state. |
| Where possible use products, packing material and disposable equipment that can be reused or recycled and where possible, are made of recycled materials | Low | Yes | B.7.1 | |
| Stockpile uncontaminated soil for use as fill or other purposes | High | Yes | Appendix G | Design strategy aims to maximize use of uncontaminated cut when possible. |
| Salvage uncontaminated objects with potential recycle, resale or donation. | Medium | Yes | G.13.1 | Erosion protection rock removed from TDA cover will be utilized for Repository cover. |
| Use local staff (including subcontractors) when possible to minimize transportation impacts | Medium | Yes | Appendix S | Local contractors will be given priority over non-local contractors |
| Maintaining a single point of entry/exit to the remedial area helps prevent re-contamination of areas previously remediated while minimizing required support facilities. | High | Yes | D.6.3 | |
| Use of ultra-low sulfur diesel in construction equipment and support vehicles | Medium | Yes | B.7.2 | |
| Minimize speed to reduce dust creation and minimize water use for dust suppression | Medium | Yes | B.7.2, D.6.3 | |
| Consider use of rumble grates with a closed-loop graywater washing system to minimize tracking of contaminated sediment and soil offsite | Low | No | | Decontamination procedure is more rigorous than use of rumble grates. |
| Optimize haul routes to minimize vehicle miles | Medium | Yes | B.6.3 | |
| Installation of silt fences and basins and other stormwater BMPs to capture sediment runoff along sloped areas | Medium | Yes | Attachment E.1 | |
| Construction of long-term structural controls such as earth dikes and swales to prevent up-gradient surface flow into excavated areas | Medium | Yes | Attachment E.1 | |
| Segregating contaminated water from clean water to minimize volume of stormwater requiring treatment | High | Yes | E.7.3 | |
| Diverting clean water away from pits and remediation activities to prevent potential contamination | High | Yes | E.7.3 | CC will be responsible for fulfilling this BMP in their CSWPPP. |
| Select drought-resistant plants for the upper vegetative layer, to reduce maintenance needs | High | Yes | App G.13 & App U | |
| Use nonsynthetic nutritional soil amendments such as compost or biochar instead of chemical fertilizers | Medium | Yes | App G.13.1 & App U | |
| Use recycled materials for capillary breaks instead of natural rock to minimize ecosystem disturbance | Medium | Yes | App G.13.1 | Site location would make importing recycled concrete not in line with emission reduction policies. Some rock for the Repository cover will be sourced from existing erosion protection rock on a previously used cover. When possible other rock and materials from the site will be utilized for temporary roads or laydown areas. |

| BMP | Impact ¹ | Selected for Implementation | Location in RDR | Comment |
|--|---------------------|-----------------------------|-----------------|---|
| Use geotextile fabric or drainage tubing composed of 100% recycled materials rather than virgin materials for lining, erosion control and drainage | Medium | Yes | App G.13.1 | |
| Use uncontaminated soil, sediment or sand rather than importing soils if possible. If importing soils is necessary, consider using industrial waste products as partial substitutes for cap soil | High | Yes | Appendix H | |
| Blend amendments into a single mixture that can be applied above the cover through a one-step process rather than a series of applications, to minimize operation of front loaders and other heavy machinery | High | Yes | App G.13.2 | |
| Operation, Monitoring and Maintenance (OM&M) Procedures: Use in-place soil depth indicators rather than manual depth probes to monitor cap thickness in order to minimize cover damage during inspection | Low | Yes | W.6.2.1 | |
| OM&M Procedures: Use remotely controlled or non-invasive techniques to avoid cover damage and minimize field visits | Low | Yes | Appendix U | Revegetation monitoring will incorporate 'laser point bars' to acquire high resolution coverage data while minimizing disturbance and time. |
| OM&M Procedures: Integrate onsite structures to capture rainfall as a source of water for rinsing and decontaminating field equipment | Low | No | | Decontamination procedures are minimal for site OM&M and utilizing on-site water well can provide more than enough water without requiring additional construction. |
| Use non-chemical solarizing techniques for soil preparation | Low | No | | Seeding contractor does not recommend use of solarizing for particular soil and environment. |
| Use minimum slope while maintaining proper drainage to reduce the volume of fill material | High | Yes | G.13.3 | |
| Use of nearest qualifying source for borrow material to reduce fuel emissions associated with increased transportation | High | Yes | H.4.1 | While on-sight borrow sources that are closer to the Mine Site may exist, it was determined to be more in line with GSR principles to utilize previously disturbed borrow sources located on-site; however, haul distances have been minimized so that the nearest borrow source is utilized for each activity. |
| Sizing equipment correctly with the task needs thereby minimizing use of heavy equipment for small tasks | High | Yes | Appendix H | |
| Throughout project re-evaluate volume of borrow needed in order to avoid over excavating and transportation of borrow source | Low-High | Yes | H.5 | |
| Restoration of land surface within a timely manner to minimize erosion and prevent growth of invasive species | High | Yes | H.5/G.13.3 | |
| Enhancements of habitat, in the form of trees and other native landscaping to be completed following construction | Medium | Yes | H.5/G.13 | |
| Minimizing soil and habitat disturbance of stormwater controls and effluent pipelines by aligning them with existing or proposed roadways | High | Yes | D.6.3 | |
| Prioritize prevention of noxious weeds via inspections rather than reactive application of herbicides. | Medium | Yes | Appendix U | |

1-Low, medium and high are relative designations that represent a qualitative judgment and is not meant to be a quantitative expression.

A.6 REFERENCES

- ASTM International, 2016. ASTM Standard E2893-16, "Standard Guide for Greener Cleanups," ASTM International, West Conshohocken, PA, 2016, DOI: 10.1520/E2893-16E01, www.astm.org.
- US Environmental Protection Agency (USEPA), 2011. Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Mine Site, McKinley County, New Mexico, Pinedale Chapter of the Navajo Nation. Prepared for U.S. EPA Regions 6 and 9. September 29.
- US Environmental Protection Agency (USEPA), 2013. Record of Decision, United Nuclear Corporation Site, McKinley County, New Mexico, EPA ID: NMD030443303. March 29.
- US Environmental Protection Agency (USEPA), 2015. Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery. April 27

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Appendix G: Mine Waste Repository Design

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LIST OF ACRONYMS / ABBREVIATIONS

| | |
|-----------|---|
| AOC | Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery |
| ARAR | Applicable or Relevant and Appropriate Requirement |
| ASTM | American Society for Testing and Materials |
| bgs | below ground surface |
| BMP | best management practice |
| CC | Construction Contractor |
| CPT | cone penetration test |
| CY | cubic yard(s) |
| DOE | US Department of Energy |
| DSHA | deterministic seismic hazard analysis |
| ET | evapotranspirative |
| FS | factor of safety |
| GE | General Electric |
| GSR | Green and Sustainable Remediation |
| HSA | hollow-stemmed auger |
| LAR | license amendment request |
| Mill Site | Church Rock Mill Site |
| Mine Site | Northeast Church Rock Mine Site |
| MWH | Montgomery Watson Harza |
| NAVFAC | Naval Facilities Engineering Command |
| NRC | US Nuclear Regulatory Commission |
| pcf | pounds per cubic foot |
| PDS | Pre-design studies |
| PGA | peak ground acceleration |
| PI | plasticity index |
| PSHA | probabilistic seismic hazard analysis |
| PTW | principal threat waste |
| RAO | Remedial Action Objective |
| ROD | Record of Decision |
| SHA | seismic hazard analysis |
| SOW | Statement of Work |
| SPT | standard penetration testing |
| TDA | Tailings Disposal Area |

UNC United Nuclear Corporation
USCS Unified Soil Classification System
USEPA US Environmental Protection Agency

G.1 INTRODUCTION

G.1.1 Repository Design Objectives

The design objectives for the Repository to be located on the Church Rock Mill Site (Mill Site) Tailings Disposal Area (TDA) were listed in Appendix A of the Design Work Plan (MWH, 2016). These design objectives are summarized below.

- Repository capacity designed for 1,000,000 (1M) cubic yards (CY) of mine waste. The design (with associated cover erosion control) will accommodate variations in the mine waste volume.
- Repository located on the north and central cells.
- Edge of Repository set back a minimum of 50 feet from the western embankment to limit traffic loading from haul traffic and stress from fill placement on the tailings impoundment.
- Minimize the rock size and volume required for the erosion protection layer on the Repository cover by limiting cover slope lengths and grades. Slope grades are to be designed to be between 2 and 5 percent.
- Fill thickness over the former borrow pits is to be limited to reduce the potential for differential settlement of the cover surface. The area of maximum fill thickness is to be located as far to the north as practical, with the western half of the central cell used for Repository capacity if necessary.
- Abrupt transition slopes, rock aprons at the toe of slopes, and additional diversion channels will be limited, with existing drainage channels and drainage swales on the existing cover used where possible. Flow path lengths for stormwater collection and conveyance from the TDA will be minimized.

G.1.2 Repository Design Summary

In addition to meeting the design objectives summarized above, the Repository design has been analyzed for long-term performance to maintain isolation of the mine waste within the Repository. This design appendix presents the supporting technical analyses for the Repository design. The technical analysis methods used are described in the Design Work Plan (MWH, 2016), and follow the analysis requirements of NUREG-1620 (NRC, 2003). The technical analyses and design objective discussions presented in this appendix are listed below.

- A description of preparation of the existing TDA radon barrier to be a foundation layer for the Repository
- A description of the mine waste placement sequence within the Repository
- Site specific seismic hazard analysis (SHA) for the Mill Site
- Slope stability analyses for the Repository
- Settlement analyses associated with mine waste placement, including discussion of immediate settlement, primary and secondary consolidation of underlying tailings, and seismic-induced settlement.
- Liquefaction analyses of the Repository and Repository foundation materials
- Cover cracking analyses of the existing radon barrier
- Stress influence evaluation from mine waste placement
- Evapotranspirative (ET) cover design including water-balance and infiltration modeling, erosion protection design, and radon modeling (by Dwyer Engineering, LLC)
- Evaluation of tailings pore water migration is provided under separate cover by Dwyer Engineering, LLC.

G.2 PERFORMANCE STANDARDS

The Performance Standards presented here are defined in the Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Site (2011 Action Memo; USEPA, 2011), the Record of Decision, United Nuclear Corporation Site, (ROD; USEPA, 2013), and the Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery (AOC; USEPA, 2015) including the Statement of Work (SOW) attached as Appendix D to the AOC, and were developed to define attainment of the Removal Action and Remedial Action Objectives (RAOs) for the Selected Remedy. The Performance Standards include both general and specific standards applicable to the Selected Remedy work elements and associated work components. Table G.2-1 presents Performance Standards related to the Repository design and construction and explains how the design accomplishes these standards.

Table G.2-1: Performance Standards Applicable to Waste Repository Design

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|----------------------------|--|---|
| 105 | 10 CFR 61.23(g) | Licensing | 10 CFR 61.23(e) Standards for Issuance of a License. Refer to www.ecfr.gov . | The Repository is designed for long-term stability and the design criteria used to eliminate, to the extent practicable, the need for ongoing maintenance. |
| 99 | 10 CFR 61.51(a)(2) | Site Controls and Security | 10 CFR 61.42. Refer to www.ecfr.gov . | See Dwyer Cover System Design Report, Attachment G.7 |
| 94 | 2011 Action Memo, V.A.1., Bullet 1 – Repository Design | Repository Design | Design a repository for the contaminated material excavated and removed from the NECR Mine Site. Design specifications will comply with CERCLA requirements, specifically all ARARs. The design, at a minimum, will include a low permeability layer (liner) and a cap structure that will mitigate direct contact, limit water infiltration, and perform as a radon barrier. | The radon barrier (low-permeability layer) is described in Section G.5. The Repository cover (cap structure) is discussed in Section G.12 (see also Dwyer Cover System Design Report, Attachment G.7) |
| 89 | 2011 Action Memo, V.A.1., Bullet 3 – Construction | Construction | Construct a repository that will contain the contaminated mine waste and soil excavated and removed from the NECR Mine Site in accordance with the approved design specifications. This action is contingent on the NRC approval of a license amendment for the UNC Mill Site disposal cells, and on EPA's decision document for the surface contamination at the UNC Mill Site. | The Repository design is being prepared with consideration for US Nuclear Regulatory Commission (NRC) license amendment requirements. The performance standards from the 2013 ROD (USEPA Region 6) are addressed later in this table. |
| 87 | 2011 Action Memo, V.A.1., Bullet 5 – Closure | Closure | Closure of the repository once all NECR Mine Site contaminated waste rock and soil is disposed. Once all contaminated mine waste and soil is excavated from the NECR Mine Site, transported to the repository and disposed in the repository, the repository will be closed, and the cap will be put in place. | Sequencing for waste and cover placement is described in Section G.5 and shown on Drawings 7-01 through 7-03 of the Drawings. After construction is complete, the final cover will be placed over the Repository. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|-------------------|--|---|
| 80 | 2011 Action Memo, Table A-1; 2013 ROD Table 1 and Sections 2.9.2 and 2.9.5 | Repository Design | 40 CFR 192.02(a) and 02(b) Standards for the Control of Residual Radioactive Materials from Inactive Uranium Processing Sites. Refer to www.ecfr.gov . | See Dwyer Cover System Design Report, Attachment G.7 |
| 155 | 2011 Action Memo Table A-1, 2013 ROD Table 1 | Closure | NMAC 20.9.6.9.A(3)(c) Closure and Post-Closure Requirements for Municipal and Special Waste Landfills and Monofills. See http://164.64.110.239/nmac/_titles.htm . | A description of the final cover and its placement is presented in Section G.12 (see also Dwyer Cover System Design Report, Attachment G.7) and the grading plan is shown on Drawings 7-01 through 7-03. The construction quality assurance plan is presented in Appendix V. |
| 76, 79 | 2011 Action Memo, Table A-1; 2013 ROD Table 1 and Sections 2.9.2 and 2.9.5 | Repository Design | 40 CFR 192.02(c) and (d) Standards for the Control of Residual Radioactive Materials from Inactive Uranium Processing Sites. Refer to www.ecfr.gov . | See Dwyer Cover System Design Report, Attachment G.7 |
| 24 | 2013 ROD, Section 1.4 – Repository Design | Repository Design | Design a repository at the UNC Site for the contaminated material excavated and removed from the NECR Site. Design specifications will comply with CERCLA requirements including all applicable or relevant and appropriate requirements (ARARs). The design will include a cap structure that will mitigate direct contact, limit water infiltration, and perform as a radon barrier. Final design will determine actual configurations of cap and liner structure and will be submitted as part of a license amendment request to the Nuclear Regulatory Commission (NRC). | The Repository has been designed to receive the anticipated volume of material to be removed from the Northeast Church Rock Mine Site (Mine Site). The Repository cover (cap structure) is discussed in Section G.12 (see also Dwyer Cover System Design Report, Attachment G.7) and the radon barrier (low-permeability layer) is described in Section G.5. Parts of this design including this appendix, once finalized, will be submitted to NRC for review and approval as part of the License Amendment Request (LAR). |
| 22 | 2013 ROD, Section 1.4 - Construction | Construction | Construct a repository at the UNC Site that will contain the contaminated mine waste and soil excavated and removed from the NECR Site in accordance with the approved design specifications. This action is contingent on the NRC approval of a license amendment for the UNC Site Tailings Disposal Area which comprises three covered tailing cells and two covered borrow pits. In addition, there are two open evaporation ponds located on the South Cell. That is, unless the NRC approves a | Parts of this design including this appendix, once finalized, will be submitted to NRC for review and approval as part of the LAR. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|-------------------|--|--|
| | | | license amendment for the UNC Site Tailings Disposal Area, the construction described in this ROD will not go forward. If NRC disapproves the request for a license amendment, EPA will stop its efforts to dispose of the NECR Site waste at the UNC Site Tailings Disposal Area, and EPA will evaluate other alternatives for disposal of the NECR Site waste. | |
| 26 | 2013 ROD, Section 2.9.1, Bullet 3 | Repository Design | Remediation Action Objectives Prevent the migration of concentrations of contaminants located in the soil, mine waste, and tailings contained within the Tailings Disposal Area to ground water where the migration of those contaminants would result in ground water concentrations that exceed remediation goals established in EPA's 1988 ROD for the Ground Water Operable Unit (including any amendment), and, through this action, prevent human and ecological receptors from being exposed to ground water with concentrations of contaminants that exceed remediation goals established in the 1988 ROD, including any amendment. | See Dwyer Consolidation and Groundwater Evaluation Report. |
| 27 | 2013 ROD, Section 2.9.2, Bullet 1 | Repository Design | Radionuclides and their daughter products in soil, mine waste, and tailings contained within the Tailings Disposal Area will not release radon-222 emissions from residual radioactive material to the atmosphere in exceedance of an average release rate of 20 picocuries per square meter per second (pCi/m ² s) 16 [40 CFR §§ 192.02(b)(1) and 192.32(b)(1)(ii)]. | See Dwyer Cover System Design Report, Attachment G.7 |
| 28 | 2013 ROD, Section 2.9.2, Bullet 2 | Repository Design | Radionuclides and their daughter products in soil, mine waste, and tailings contained within the Tailings Disposal Area will not release radon-222 emissions from residual radioactive material to the atmosphere that will increase the annual average concentration of radon-222 in air at or above any location outside the disposal site by more than one-half picocurie per liter [40 CFR § 192.02(b)(2)]. | See Dwyer Cover System Design Report, Attachment G.7 |
| 29 | 2013 ROD, Section 2.9.2, Bullet 3 | Repository Design | Remediation Goals Migration of contaminants from the Tailings Disposal Area shall not result in ground water concentrations that exceed remediation goals established in EPA's | See Dwyer Consolidation and Groundwater Evaluation Report. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|-------------------|---|--|
| | | | 1988 ROD for the Ground Water Operable Unit, including any amendment. | |
| 32 | 2013 ROD, Section 2.9.5 – Cap Design Criteria | Repository Design | Although the final design may vary, the major elements of the structure are not expected to be significantly different than those presented here. The cap design will be based on comprehensive planning, site-specific risk analysis, and ARARs. Cap design and cost estimates for Alternative 2 are based on the following elements: | See Dwyer Cover System Design Report, Attachment G.7 |
| 33 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 1 | Repository Design | Cap longevity designed for a minimum of 200 years with minimal maintenance and for effectiveness up to one thousand years, to the extent reasonably achievable [40 CFR §§ 192.02(a), 192.32(b)(1)(i), and 264.111(a)] | See Dwyer Cover System Design Report, Attachment G.7 |
| 34 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 2 | Repository Design | A sufficient clean (uncontaminated) soil layer to provide assurance that releases in the form of Radon-220 and -222 will not exceed an average release rate of 20 picocuries per meter squared per second [40 CFR §§ 192.02(b)(1) and 192.32(b)(1)(ii)], and will not increase the annual average concentration of radon-220 and -222 in air at or above any location outside the disposal site by more than one-half picocurie per liter [40 CFR § 192.02(b)(2)] | See Dwyer Cover System Design Report, Attachment G.7 |
| 35 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 3 | Repository Design | Cap construction to protect the mine waste, reduce the potential for leachate development, and prevent contaminated runoff by limiting infiltration of precipitation and by providing erosion protection and durability [40 CFR §§ 192.32(b)(1), 264.111(a), 264.111(b), 264.228(b)(1), 264.228(b)(3), and 264.228(b)(4)] | See Section G.12. Also see Dwyer Cover System Design Report, Attachment G.7 |
| 36 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 4 | Repository Design | Cap slope, shape and drainage construction to ensure stability and minimize the effects of erosion, root intrusion, and animal destruction [40 CFR §§ 192.32(b)(1), 264.111(a), 264.111(b), 264.228(b)(1), 264.228(b)(3), and 264.228(b)(4)]; | The shape of the Repository is designed with top slopes of 2 to 5% to minimize the effects of erosion. See Section G.12 for description of the cover design. See also Dwyer Cover System Design Report, Attachment G.7 |
| 37 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 5 | Repository Design | Use of biosolids or top soil to facilitate vegetation growth | The revegetation plan for the Repository (Appendix U, Attachment U.2) includes the use of amendments such as composted cow or green |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|-------------------|---|--|
| | | | | manure, or composted biosolids to promote vegetation growth. |
| 38 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 6 | Repository Design | ...the use of vegetation to emulate the structure, function, diversity, and dynamics of the native community to maximize resilience and sustainability | The Repository will be revegetated after cover soil placement is completed. See also Revegetation Plan for the Repository (Appendix U, Attachment U.2) See also Dwyer Cover System Design Report, Attachment G.7 |
| 39 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 7 | Repository Design | Erosion modeling to determine effectiveness of cap design | See Section G.12 and also see Dwyer Cover System Design Report, Attachment G.7 |
| 40 | 2013 ROD, Section 2.9.5 – Cap Design Criteria, Bullet 8 | Repository Design | A low permeability layer (liner) will be placed between the NECR mine waste and the tailings currently disposed within the Tailings Disposal Area. This layer will be constructed to eliminate the possibility that the layer will collect water and produce a “bathtub effect”. This layer will be constructed of natural materials, not synthetic, to eliminate the sudden failure risk associated with punctures and rips. This layer will be compacted to meet a hydraulic conductivity of no more than 1×10^{-7} centimeters per second (cm/s). The liner will serve the following purposes: 1 – The liner will help protect workers doing construction. 2 – The liner will be an added level of protection for groundwater 3 – The liner will provide a stable foundation on which to place the NECR Site waste. 4 – The liner will form an added barrier, preventing exposure to the higher level of radioactivity found in the mill tailings that are currently disposed in the UNC Site Tailings Disposal Area. | The existing clay radon barrier will be modified in-place and serve as the “low-permeability layer” located between the mine waste and the existing tailings. This layer is described in Section G.5.1. Only the upper 6 inches of material (erosion protection layer) will be removed from the cover, prior to re-compaction of the radon barrier. |
| 45 | 2013 ROD, Section 2.9.5, Waste Volume | Waste Volume | Approximately 871,000 cubic yards from the removal action described in the 2011 Non-Time-Critical Removal Action Memorandum for the NECR Site, 109,800 cubic yards from a removal action at the NECR Site that predates the 2011 Non-Time-Critical Removal Action Memorandum for the NECR Site, and an estimated 30,000 cubic yards to be excavated as part of a separate | The Repository design can be adjusted for a disposal volume of up to approximately 1.1M CY. Mine removal excavations are described in Appendix C. The estimated volume of material at the Mine Site, excluding the principal threat waste (PTW), will be moved to the Repository. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|-------------------|---|--|
| | | | time-critical removal action at the NECR Site will be interred at the Tailings Disposal Area and capped. Although the additional 109,800 and 30,000 cubic yards volume was not included in the EE/CA, the additional volume and associated cost are minimal compared to the overall volume and cost evaluated. In addition, the added expense is within the EE/CA's margin of error. Based on this, the additional volume and cost are considered included and addressed under this alternative. The waste acceptance criteria for mine waste that will be disposed at the UNC Site Tailings Disposal Area are 200 pCi/g or less of Ra-226 and/or 500 mg/kg or less of uranium. | |
| 58 | 2013 ROD Table 1 | Repository Design | 10 CFR 40 Appendix A, Criterion 1 Criteria Relating to the Operation of Uranium Mills and the Disposition of Tailings or Wastes Produced by the Extraction or Concentration of Source Material From Ores Processed Primarily for Their Source Material Content. Refer to www.ecfr.gov . | The siting of the Repository is based on the previously approved site of the licensed TDA. The Repository design minimizes erosion and maintains the current isolation of the tailings below from the mine waste to be placed. |
| 63 | 2013 ROD Table 1 | Repository Design | 10 CFR 40 Appendix A, Criterion 3 Criteria Relating to the Operation of Uranium Mills and the Disposition of Tailings or Wastes Produced by the Extraction or Concentration of Source Material From Ores Processed Primarily for Their Source Material Content. Refer to www.ecfr.gov . | See Dwyer Cover System Design Report, Attachment G.7 |
| 62 | 2013 ROD Table 1 | Repository Design | 10 CFR 40 Appendix A, Criterion 4 Criteria Relating to the Operation of Uranium Mills and the Disposition of Tailings or Wastes Produced by the Extraction or Concentration of Source Material From Ores Processed Primarily for Their Source Material Content. Refer to www.ecfr.gov . | <p>(a) See Appendix I – Mill Site Stormwater Controls. The upstream catchment area was fixed based on the location of the TDA.</p> <p>(b) Surrounding topography was also generally predetermined by the location of the TDA.</p> <p>(c) Repository slopes are no steeper than 5:1 and generally between 2 and 5 percent.</p> <p>(d) Cover erosion protection will be achieved with a rock admixture. See Section G.12 and also the Dwyer Cover System Design Report, Attachment G.7. The final cover surface will also be seeded for vegetation establishment according to the revegetation plan (see Appendix U, Attachment U.2)</p> |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|------------------------|---|---|
| | | | | <p>The surface grading design is contoured to prevent concentrated runoff and direct flows to existing rock-lined swales.</p> <p>(e) Site-specific SHA is included as Attachment G.1 and analyses relevant to seismic events are presented in Sections G.7, G.8, G.9.4, and G.11.</p> <p>(f) The Repository is designed to shed stormwater to the perimeter of the existing TDA. Run-on flows are controlled by upstream diversion channels and these flows will not come in contact with the Repository. Discussion of cover erosion protection is presented in Attachment G.7 – Dwyer Cover System Design Report.</p> |
| 64 | 2013 ROD Table 1 | Repository Design | 10 CFR 40 Appendix A, Criterion 5 Criteria Relating to the Operation of Uranium Mills and the Disposition of Tailings or Wastes Produced by the Extraction or Concentration of Source Material From Ores Processed Primarily for Their Source Material Content. Refer to www.ecfr.gov . | See Dwyer Cover System Design Report, Attachment G.7 |
| 57 | 2013 ROD Table 1 | Repository Design | 10 CFR 40 Appendix A, Criterion 6 Criteria Relating to the Operation of Uranium Mills and the Disposition of Tailings or Wastes Produced by the Extraction or Concentration of Source Material From Ores Processed Primarily for Their Source Material Content. Refer to www.ecfr.gov . | <p>See Dwyer Cover System Design Report, Attachment G.7</p> <p>Also, for</p> <p>6(1) see Section G.12.4</p> <p>6(2) see Section G.12.3</p> <p>6(3) The final cover will not be placed in phases. Radon testing will be conducted following completion of the cover.</p> <p>6(4) Results of the radon testing will be included with the As-Built Report for the Repository.</p> |
| 59 | 2013 ROD Table 1 | Repository Design | 10 CFR 61.41. Refer to www.ecfr.gov . | See Appendix T. |
| 55 | 2013 ROD Table 1 | Performance Objectives | 10 CFR 61.44. Refer to www.ecfr.gov . | <p>See Appendix I regarding long-term stability of the stormwater control features around the Repository.</p> <p>Regarding the Repository cover, See Dwyer Cover System Design Report, Attachment G.7</p> |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|--------------------------|---|--|
| 60 | 2013 ROD Table 1 | Repository Design | 10 CFR 61.51(a)(1), 51(a)(4), 51(a)(5) and 51(a)(6). Refer to www.ecfr.gov . | <p>(a)(1) The Repository is designed to provide long-term isolation of the mine waste, with avoidance of long-term active maintenance. This includes use of the probable maximum precipitation as the design storm event for erosion protection analyses.</p> <p>(a)(4) The ET cover is designed to minimize infiltration through to the mine spoils and the surface grading is designed to direct water away from the TDA.</p> <p>(a)(5) See Appendix I - Mill Site Storm Water Controls</p> <p>(a)(6) The mounded surface of the Repository promotes the shedding of surface runoff, with surface slopes and settlement analyses indicating no negative slope grades (depressions) from cover settlement.</p> <p>See also Dwyer Cover System Design Report, Attachment G.7</p> |
| 46 | 2013 ROD, Table 1 | Closure | 10 CFR 61.52(a)(9). Refer to www.ecfr.gov . | Closure and stabilization of the Repository will be completed with cover surface erosion protection rock and vegetation. See Section G.12 and the Dwyer Cover System Design Report, (Attachment G.7) for cover design details. See also the Revegetation Plan for the Repository (Appendix U, Attachment U.2). |
| 68 | 2013 ROD Table 1 | Waste Disposal | 10 CR 61.52(a)(10). Refer to www.ecfr.gov . | Areas of the existing TDA cover that will be trafficked during construction will be restored following completion of the Repository. Existing stormwater channels will be either maintained or improved (Appendix I). |
| 69 | 2013 ROD, Table 1 | Waste Disposal | 10 CFR 61.52(a)(11). Refer to www.ecfr.gov . | Materials designated for removal from the Mine Site, with the exception of materials classified as PTW, will be disposed of in the Repository. |
| 49 | 2013 ROD, Table 1 | Environmental Monitoring | 10 CFR 61.53(c) Environmental Monitoring During the land disposal facility site construction and operation, the licensee shall maintain a monitoring program. Measurements and observations must be made and recorded to provide data to | The pre-final design includes a monitoring program for the operation of the Repository and the associated stormwater control features. Observations will be made during periodic inspections to evaluate |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|---------------------------------|---|--|
| | | | evaluate the potential health and environmental impacts during both the construction and the operation of the facility and to enable the evaluation of long-term effects and the need for mitigative measures. The monitoring system must be capable of providing early warning of releases of radionuclides from the disposal site before they leave the site boundary. | performance of the cover, provide early warning of potential problems and identify issues that may require mitigative measures. |
| 72 | 2013 ROD Table 1 and Section 2.9.5 Cap Design, Bullets 1, 3, and 4. | Closure | 40 CFR 264.111(a). Refer to www.ecfr.gov . | See Appendix I regarding maintenance of the stormwater control features around the Repository. Regarding the Repository cover see Section G.12 and the Dwyer Cover System Design Report, Attachment G.7 |
| 74 | 2013 ROD, Table 1 and Section 2.9.5, Cap Design Criteria, Bullets 3 and 4 | Storm Water and Erosion Control | 40 CFR 264.228(b)(4). Refer to www.ecfr.gov . | Run-on to the Repository site is managed by the upstream diversion channel which diverts storm flows north and south of the TDA. Runoff from the cover will be collected in the existing stormwater channels and swales located around the Repository. Regarding erosion and damage to the Repository cover, see Dwyer Cover System Design Report, Attachment G.7 |
| 75 | 2013 ROD Table 1 and Sections 2.9.2 and 2.9.5 | Repository Design | 40 CFR 192.32(b)(1). Refer to www.ecfr.gov . (2) The requirements of § 192.32(b)(1) shall not apply to any portion of a licensed and/or disposal site which contains a concentration of radium-226 in land, averaged over areas of 100 square meters, which, as a result of uranium byproduct material, does not exceed the background level by more than: (i) 5 picocuries per gram (pCi/g), averaged over the first 15 centimeters (cm) below the surface, and (ii) 15 pCi/g, averaged over 15 cm thick layers more than 15 cm below the surface | (1)(i), See Dwyer Cover System Design Report (Attachment G.7) regarding design life of the Repository cover. (ii) See Dwyer Cover System Design Report, Attachment G.7 (2) See Dwyer Cover System Design Report, Attachment G.7 |
| 1 | 2015 AOC SOW, Paragraph 16 – Design of the | Repository Design | Respondents shall design a Repository at the Tailings Disposal Area for permanent disposal of approximately 1,000,000 cubic yards of contaminated soil and mine waste | The Repository design can be adjusted for a disposal volume of up to approximately 1.1 million CY. The Performance Standards are |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|---|---|---|
| | UNC Site Repository | | material with contamination that meets or exceeds the Performance Standards. | addressed throughout the design documents. |
| 5 | 2015 AOC SOW, Paragraph 20 – Site Preparation Activities | Site Preparation | <p>In the Design, Respondents shall include detailed plans and specifications for the following site preparation activities:</p> <ul style="list-style-type: none"> a. An underground utility survey for the identification and verification of the location of subsurface utilities in SA Site areas that will be used for consolidation or disposal; b. A land survey that will delineate the parts of the Tailings Disposal Area that will be used for NECR Site contaminated soil and mine waste disposal; c. A description of construction activities to be undertaken on the portion of the SA Site that is at the UNC Site in order to prepare for placement of the NECR Site contaminated soil and mine waste in the Tailings Disposal Area; d. A description of the methods that will be used to decontaminate existing structures such as culverts, catch basins, foundations, and vaults; and, where decontamination is not practicable, a description of methods that shall be used to disassemble these structures, demolish and remove these structures, or include these structures within the Tailings Disposal Area. | <ul style="list-style-type: none"> a. See Appendix B – Early Works and Construction Support Facilities b. A series of aerial and land surveys have been conducted over the last 10 years. The most recent set of survey data was collected during the pre-design studies (PDS). These data sets have been combined for use in this design. c. See Appendix B –Construction Support Facilities and Appendix D – Haul Roads. Additional detail on design and construction of the Repository is provided throughout this appendix. d. See Appendix C – Mine Waste Removal Excavations and Demolition |
| 11 | 2015 AOC SOW, Paragraph 26 – Acceptance Criteria | Administrative | For the part of the Tailings Disposal Area that is to contain the mine waste from the NECR Site and for the part of the current tailings cell that may be disturbed during implementation of the remedy, Respondents shall include, in their Design, detailed plans and specifications to meet and demonstrate compliance with Acceptance Criteria consistent with Section 5.1 of NUREG 1620. | Radon and gamma attenuation is addressed in the Dwyer Cover System Design Report (Attachment G.7) and Section G.12.4 of this appendix. |
| 14 | 2015 AOC SOW, Paragraph 29 – Green Remediation Best Management Practices | Green Remediation Best Management Practices | Respondents shall incorporate applicable Best Management Practices for Green Remediation listed in ASTM-E2893-13 consistent with EPA's policy <i>Superfund Green Remediation Strategy</i> {2010}, found at http://www.epa.gov/superfund/greenremediation/sf-gr-strategy.pdf . | See Section G.13. |
| 56 | 2013 ROD, Table 1 | Radiation Protection | 10 CFR 40, Appendix A, Criterion 6A. Refer to www.ecfr.gov . | The radon barrier over the byproduct materials will not be removed. It will |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|-------------|---|---|
| | | | | be compacted in- place. See Section G.5.1. |
| 18 | 2015 AOC SOW, Paragraph 38 – Potential Impacts on Ground Water Remediation | Groundwater | Respondent shall describe any potential impacts of the Design on the on-going ground water remediation infrastructure at the UNC Site, and a proposed approach to address such impacts. | The existing groundwater remediation infrastructure is shown on the utilities drawing in the Section 1 Drawings. The haul road from the North and South Borrow areas can be adjusted to prevent conflict with, or damage to, the existing wells in operation at the time of construction. |

*Refers to identifying numbers listed in Summary of ARARs, Performance Standards and Applicable NRC Design Requirements Table (provided in Attachment 1 to main text of the 95% Design Report)

G.3 ENGINEERING DESIGN DRAWINGS

The engineering design drawings for the Repository are contained in Volume II – Design Drawings (Section 7). The complete set of Drawings related to the Repository are listed in Table G.3-1 and referenced by sheet number in the text.

Table G.3-1: Engineering Design Drawings

| Drawing No. | Drawing Title |
|-------------|---|
| 7-01 | Repository Subgrade - Existing Radon Barrier |
| 7-02 | Repository Mine Waste Fill by Removal Phase and Temporary Stormwater Control Berms (Sheet 1 of 2) |
| 7-03 | Repository Mine Waste Fill by Removal Phase and Temporary Stormwater Control Berms (Sheet 2 of 2) |
| 7-04 | Repository Mine Waste Fill Profile by Removal Phase |
| 7-05 | Repository Top of Mine Waste and Cover Grading Plan |
| 7-06 | Repository Profiles |
| 7-07 | Repository Final Cover Grading Plan |
| 7-08 | Cover Surface Erosion Protection |
| 7-09 | Repository Cover Details |
| 7-10 | Repository Cover Details |

G.4 DESIGN BACKGROUND

G.4.1 Design Data

The section summarizes the analyses of the Repository design. Each analysis relies on data collected during the Mill Site Pre-Design studies (PDS; MWH, 2014a) as well as other sources of information specifically referenced. The Repository layout with a waste volume of 1.0M CY and shown on Drawing 7-07 was used for the analyses. As described in the introduction, the Repository design was developed based on the approved design objectives described in Appendix A of the Design Work Plan (MWH, 2016). The fill thicknesses over specific locations on the TDA are based on the design fill elevations for the proposed layout. The cross sections for the slope stability models are based on the layout shown on the Drawings.

The material properties used in the 95% Design analyses are based primarily on the geotechnical laboratory data from the PDS. Assumptions have been applied, specific locations selected, or parts of the data set used in some cases, for various types of materials for conservative input on specific analyses. Average values were used as the base-case scenario for all material properties in all analyses. A sensitivity analysis was conducted for each individual analysis, and the parameter(s) that most influenced the results was found and varied to assess the impact on the overall results. Details of the sensitivity analyses can be found in the Attachments G.2 through G.6.Repository

G.4.2 Design Basis

The design basis for the Repository is provided in Table G.4-1. The individual design basis items comply with regulatory requirements and/or generally accepted engineering practice and meet the overall project design criteria as provided in the Design Work Plan (MWH, 2016).

Table G.4-1: Repository Design Basis

| Design Category | Design Basis | Design Reference |
|-----------------|--|--|
| Seismic Hazard | Design life to be 1,000 years to the extent reasonably achievable, and at least 200 years | US Environmental Protection Agency (USEPA) (40 CFR 192) US Nuclear Regulatory Commission (NRC) (10 CFR Appendix A to Part 100 A) (NRC, 2013) |
| Slope Stability | FS (static) = 1.5. FS (pseudo-static) = 1.0. FS (Probable Maximum Flood) = 1.2 Design Horizontal Seismic Coefficient = 2/3 of the max peak ground acceleration, determined from the seismic hazard analysis Critical conditions determined by cross-sections including maximum slope steepness, maximum slope height, existing embankment, and global stability. | NUREG 1620, Section 2.2 (NRC, 2003) NRC Regulatory Guide 3.11, Section C (NRC, 2008) Technical Approach Document, Revision II, Section 6.2 (DOE, 1989) |
| Settlement | Maintain positive cover drainage long-term, prevent ponding or reverse grades One-dimensional settlement analyses performed at multiple locations Results of the calculations summed to determine maximum potential cover settlement Two-dimensional sections generated through the one-dimensional settlement profiles to identify long-term maximum and minimum slopes of the Repository cover and maximum differential settlement. | NUREG 1620, Section 2.3(1) (NRC, 2003) NAVFAC 7.01, Chapter 5, Sections 3 and 4 (Department of the Navy, 1986) NUREG 1620, Section 2.2(3i) (NRC, 2003) |

| Design Category | Design Basis | Design Reference |
|---------------------------------|--|---|
| | Seismically induced displacement is calculated and documented. | |
| Stress Influence/Cover Cracking | <p>Prevent ponding or settlement beyond the edge of the Repository cover; identify extent of necessary improvements to the existing TDA cover (if any)</p> <p>Minimize differential settlement of the underlying tailings which may affect the existing radon barrier that would be detrimental to Repository performance or stability</p> <p>Calculate horizontal strain of the existing radon barrier based on calculated cover settlements</p> <p>Calculated horizontal strain < maximum allowable strain of the radon barrier</p> | <p>NAVFAC 7.01, Chapter 4, (Department of the Navy, 1986)</p> <p>NUREG 1620, Section 2.1, 2.3. 2.5 (NRC, 2003)</p> <p>Technical Approach Document, Rev. II, Section 6.3.3 (DOE, 1989)</p> <p>Horizontal Movements Related to Subsidence (Lee and Shen, 1969)</p> |
| Liquefaction | <p>The factor of safety against liquefaction should be greater than 1.0 (NRC, 2008)</p> | <p>NUREG 1620, Section 2.4 (NRC, 2003)</p> <p>NRC Regulatory Guide 3.11, Section C (NRC, 2008)</p> <p>Liquefaction Susceptibility of Fine-Grained Soils (Bray et al., 2009)</p> <p>Soil Liquefaction During Earthquakes (Idriss and Boulanger, 2008)</p> <p>Liquefaction Resistance of Soils: Summary Report from the 1996 NCEER and 1998 NCEER/NSF Workshops of Evaluation of Liquefaction Resistance of Soils (Youd et al., 2001)</p> |
| Cover Design | See Attachments G.7 (Dwyer Cover Design Report, Dwyer Engineering, LLC) and G.8 (Rock Cover Design) | |

G.5 REPOSITORY DESIGN AND CONSTRUCTION

G.5.1 Repository Subgrade Preparation

The existing radon barrier above the tailings in the TDA will be prepared to serve as the foundation layer for the Repository. The erosion protection layer overlying the radon barrier (consisting of a nominal 6-inch-thick layer of soil and rock) will be removed and reused for Repository cover construction. The erosion protection layer and existing rock ($D_{50}=1.5$ inches) in the swales within the Repository footprint will also be removed. The rock from the swales will either be combined with the rock taken from the erosion protection layer and reused on the new cover or used for erosion protection on other areas of the site. The residual soil from the existing erosion protection layer will be reused for Repository cover construction, to fill in the swales located on the existing cover, or for general fill around the Repository.

As summarized in Table G.2-1, from the RAOs section of USEPA (2013), “a low-permeability layer (liner) will be placed between the NECR mine waste and the tailings currently disposed within the Tailings Disposal Area.” The existing radon barrier over the tailings in the TDA is planned to comprise this layer. The design criteria for this layer (from USEPA, 2013) include providing a stable foundation for mine waste placement in the Repository and a zone of separation from the underlying tailings. USEPA (2013) states that “This layer will be compacted to meet a hydraulic conductivity of no more than 1×10^{-7} centimeters per second” while also stating that “This layer will be constructed to eliminate the possibility that the layer will collect water and produce a ‘bathtub effect’.” These two statements indicate the desire for a low hydraulic conductivity within this layer, but not creating a hydraulic barrier that creates a zone of saturation at the top of the layer (the bathtub effect).

From review of the TDA reclamation plan, as-built reports, and tests results on radon barrier material in MWH (2014a), the radon barrier material cannot achieve a saturated hydraulic conductivity of less than 1×10^{-7} cm/sec, even under controlled laboratory conditions. However, a hydraulic conductivity of less than 1×10^{-7} cm/sec can be achieved under unsaturated conditions. One of the radon barrier samples (sample TI-CS01-04) was tested for unsaturated flow characteristics (MWH, 2014a). Analysis of the resulting soil water characteristic curve (see Figure G.5-1) and estimating the unsaturated hydraulic conductivity curve from relationships in van Genuchten (1980) show a decrease in hydraulic conductivity by several orders of magnitude from saturated to unsaturated conditions (see Figure G.5-2). From these calculations and using the geomean of the measured hydraulic conductivity of the radon barrier for samples remolded to 95 percent compaction (4.6×10^{-6} cm/sec; MWH, 2014a), a hydraulic conductivity of 1×10^{-7} cm/sec is achieved at a matric suction greater than 0.75 bars of negative pressure which correlates to a volumetric water content of equal to or less than 26 percent. The maximum measured optimum moisture content for the radon barrier is 15 percent gravimetric water content (sample TI-CS04-04A; MWH, 2014a) which is equivalent to a volumetric water content of 26 percent. The specifications will require the radon barrier to be compacted to 95 percent relative compaction per standard Proctor, at a water content less than the optimum water content to achieve an unsaturated hydraulic conductivity of less than 1×10^{-7} cm/sec. With the mine waste and Repository cover in place, the flux of meteoric water through the Repository would be sufficiently low that no zone of saturation would develop on the surface of the radon barrier, and the radon barrier would remain in an unsaturated condition (see Attachment G.7). The ET performance of the Repository cover would aid in maintaining unsaturated conditions in the radon barrier and achieve the hydraulic conductivity and “no bathtub” design criteria outlined in USEPA (2013).

To achieve a stable foundation for mine waste placement and maintain a zone of separation from the underlying tailings, the following preparation tasks for the existing radon barrier beneath the footprint of the Repository are outlined.

Rock mulch and riprap removal. The rock mulch on the surface of the radon barrier would be excavated and stockpiled for use on the Repository cover. Rock mulch excavation will be conducted carefully to minimize removal of radon barrier material. Riprap currently lining the swales would also be removed. The swales would be filled to grade with the soil portion of the existing rock mulch, which will be separated by screening. The excavated surface of the radon barrier would be regraded where necessary to smooth the surface for compaction.

Radon barrier compaction. The radon barrier surface would be compacted using a method specification (consisting of a minimum number of passes with specific compaction equipment). The method specification would be developed from a test fill, with the number of passes determined to achieve 95 percent of standard Proctor dry density for the material in the top 6 inches of the radon barrier. The method specification would be checked with additional test fills if there is a variation in the radon barrier material or if the Construction Contractor (CC) changes compaction equipment. Additional reworking or excavation or ripping into the radon barrier is not recommended due to the potential for contact with and exposure of underlying tailings. Material properties of the radon barrier are described in the As-Built Reclamation Reports (Canonie, 1991, 1992, 1994, and 1995; Smith, 1996a, 1996b, and 1997)) from construction of the radon barrier, as well as in the test pit sampling of the cover in the PDS report (MWH, 2014a). A discussion of the radon barrier material properties is included in Section G.6.

Water application. Water would be added, as necessary, for dust control during rock mulch removal and radon barrier compaction. Water would be applied to the compacted radon barrier surface for dust control during initial mine waste hauling and placement.

Prior to removal of the erosion protection material from the cover of the existing TDA within the Repository, a baseline gamma radiation survey will be conducted on the surface. The survey will provide a location-specific ambient background level for comparison with the completed Repository cover (see Section G.12.4). The gamma survey will consist of a GPS-based one-minute static survey at each node of a 150-foot square grid over the Repository area. The gamma radiation levels will be measured in exposure rate (uR/hr). The static gamma radiation survey will be conducted consistent with the static survey procedure described in Appendix T (Attachment T.2 Final Status Survey Plan) using a 2x2 NaI(Tl) scintillation detector interfaced with a scaler/rate meter. The scaler/rate meter will be integrated with a DGPS/controller to log the radiation levels and their corresponding position coordinates. Following removal of the erosion protection layer and prior to re-compaction of the radon barrier surface, a visual inspection of the cover will be completed to verify that tailings were not exposed during the removal process. The tailings are generally gray in color while the surficial materials are typically brown. This inspection will be completed prior to initiating compaction of the layer and prior to placement of mine waste in the Repository. If the visual inspection indicates tailings have been exposed, a post-excavation (one-minute) static gamma survey, similar to the baseline survey, can be performed to determine extent of the material exposed. If tailings are exposed in the process, the radon barrier would be excavated in the vicinity of one of the branch swales so that the material can be placed back below the radon barrier. The radon barrier extends down beneath the erosion protection in each of the branch swales. Removing the radon barrier within one of the swales would provide storage volume beneath the grade of the surrounding radon barrier.

G.5.2 Mine Waste Placement Sequence

Following preparation of the radon barrier, clean borrow soils will be used to construct perimeter stormwater berms at the edge of the mine waste placement area within the Repository. These berms will allow for containment of contact water within the Repository during waste placement (additional detail in G.5.3). As construction progresses, these berms will be incorporated into the construction of the soil cover layer. The stormwater containment berm design is described in Section G.5.3.1.

Excavated mine waste will be hauled from the Mine Site, and initially placed and compacted directly on the prepared radon barrier. The mine waste will be placed from the north to south across the Repository and will be spread in lifts for compaction. The perimeter slopes of the compacted mine waste surface will be extended as the mine waste surface is raised and the perimeter stormwater berms can be adjusted to maintain containment of contact water runoff from the outer slopes of the Repository. Once fill has been placed for the 2nd mine removal phase, subsequent fill phases should begin to mimic the final slope configuration so that the Repository can be completed with less volume, if necessary.

The Mine Site removal sequence is divided into six phases as shown on the Section 3 Drawings. Drawings 7-02 through 7-04 show, in concept, plan and section views of where each of the five phases (excluding the PTW phase) of removal from the Mine Site would be placed within the Repository. Three locations [Sandfill 1 (Area 7), Sediment Pad (Area 6), and Pond 1 (Area 9)] at the Mine Site have mine waste activity levels (for radium-226) greater than the weighted average (by volume) of the overall volume of mine waste. A summary of the activity levels for the mine removals is presented in Table G.5-1. The volume of these three areas combined account for less than 13 percent of the estimated total volume of mine waste to be placed in the

Repository. The activity levels were estimated from the analytical data in the PDS report, excluding the PTW. Of the 5 phases of removal and placement proposed, the majority of the volume is included in Phase 3. Areas 6, 7, and 9 (Sediment Pad, Sandfill 1, and Pond 1) would be removed in three different phases (Phases 2, 3, and 4). These three volumes would therefore will be dispersed within the other lower activity materials removed and then placed during that same phase. Further, the specifications for Mine Waste Excavation and Disposal limit the placement of the materials from these three Mine areas to greater than 200 feet from the outer slope of the Repository.

Table G.5-1: Mine Waste Weighted Average Activity Levels by Volume

| Mine Removal Phase | Area(s) | Average of 75 th Percentile Activity (pCi/g) | Estimated Removal Volume (CY) |
|--------------------|----------|--|-------------------------------|
| 2 | 1 | 2.4 | 14,763 |
| 2 | 2 | 26.7 | 37,005 |
| 2 | 3 | 11.4 | 67,138 |
| 2 | 4 | 5.2 | 13,771 |
| 2 | 7 | 50.5 | 40,917 |
| 3 | 6 | 86.3 | 23,086 |
| 3 | 8 | 24.2 | 422,473 |
| 4 | 5 and 11 | 9.8 | 6,399 |
| 4 | 9 | 85.2 | 29,302 |
| 4 | 10 | 10.2 | 7,758 |
| 5 | 12 | 7.1 | 34,272 |
| 6 | 13 | 6.5 | 28,356 |
| | | Weighted Average by Volume (75 th percentile vales) = 26.5 pCi/g | |

As described in Section G.12, the designed ET cover thickness is based on infiltration retention and evapotranspiration. This cover thickness has been analyzed for acceptable rates of radon-222 emanation at the cover surface, based on the weighted average (by volume) radium-226 activity levels of the mine waste. Refer to Attachment G.8 for specifics on the radon emanation calculations.

The Design Drawings (Section 7) show a Repository layout for storage of approximately 1.0M CY. The design capacity for the Repository is based on the removal estimate for soil and debris (waste) from the Mine Site described in Appendix C. The placement sequence allows for flexibility in the final volume by making adjustments to the top surface (at approximately 2 percent slope) and how this surface ties into the existing TDA cover on the south side of the Repository. The 1.0M CY capacity provides for approximately 30% contingency storage.

G.5.3 Stormwater Controls

The Repository design and layout has been incorporated into the existing site stormwater features of the previously reclaimed TDA. This has been accomplished by sloping the cover surface to existing Branch Swale C located on the east and south sides

of the Repository. On the north side, surface flow from the Repository will be directed into the North Cell Drainage Channel, which directs flow around the TDA from the east side to the north side and outlets to the Pipeline Arroyo. On the west side of the Repository, following construction of the Repository cover, clean fill will be added to the existing TDA cover, with similar erosion protection (rock admixture) to the Repository cover, to convey surface flow from the new cover to the crest of the TDA embankment and down into the existing Runoff Control Ditch. The Runoff Control Ditch will be modified, and the riprap will be enlarged.

On the southwest side of the Repository cover, clean fill will be required to fill in existing Branch Swale D, which currently drains to the northeast. Stormwater off the Repository will flow away from the new cover to the southwest. The surface of this fill area will include a similar erosion protection layer to the Repository cover. Flow from this small catchment area of the Repository will be directed onto the existing TDA cover and to existing Branch Swale H which flows to the south. Branch Swale H is planned to be reestablished once the evaporation ponds are decommissioned and removed. The Section 9 Drawings show the stormwater control designs for the Repository and they are described in Appendix I.

G.5.3.1 Stormwater Management during Construction

Stormwater control berms, constructed with clean borrow soil, will be used to contain contact water within the Repository footprint. These perimeter berms will be incorporated into the cover soils placed over the mine waste. The primary purpose for the stormwater controls will be to contain stormwater in contact with the mine waste within the Repository boundaries. The controls will include a clean soil berm placed along the perimeter of the Repository footprint that extends beyond the elevation of waste placement. The berm is sized to provide capacity for retention of the selected design storm event runoff volume, with an additional one foot of freeboard. Stantec selected the 10-year, 24-hour storm event as the design storm for the perimeter stormwater berms, based on the relatively short Repository construction period.

The perimeter berms will be constructed with clean fill in compacted lifts. The condition of the berms will be inspected and maintained by the CC during Repository construction for appropriate freeboard and discharge capacity and repaired or modified as necessary. The berms will be raised, as needed, as the waste surface is raised to maintain freeboard above the waste surface. Clean fill on the outside of the berms will also serve as interim cover over the mine waste so that stormwater in contact with the outer slopes is no longer considered contact water and can be allowed to be discharged as clean stormwater.

Temporary stormwater controls for the Repository and other areas of the Mill Site will include best management practices (BMPs) to prevent erosion from excavated areas and unprotected slopes. Proposed locations for the BMPs on the Repository are shown on the Section 5 Drawings. These measures will be required prior to establishment of vegetation on the cover. These BMPs are to be defined in the Stormwater Management Plan (Appendix E) which provides guidelines for the preparation of the Construction Stormwater Pollution Prevention Plan.

G.5.4 Potential Impacts to Ongoing Groundwater Remediation

Piping used by General Electric/United Nuclear Corporation (GE/UNC) site personnel at the pumping wells, the active sampling wells, and the bioventing area have been identified and are shown on the Site Utilities drawing in the Section 1 Drawings. Proposed haul roads and site improvements have been designed to account for the locations of the existing groundwater infrastructure and to minimize conflicts with existing operations during construction. Monitoring wells located near proposed areas of disturbance (particularly in the vicinity of the South Borrow Area) will be protected with visible barriers to prevent damage to the wells from construction traffic.

G.6 MATERIAL PROPERTIES

The material properties presented below represent the base case scenario used in the individual design analyses (i.e., stability, settlement, liquefaction, and cover cracking). Any variations to these material properties are discussed in the calculation briefs for each design analysis. Material strength parameters were used in the stability analysis, and the calculated total stress friction angles are discussed further in Appendix G.2, Slope Stability Analysis.

The Mill Site PDS investigation encountered the following materials within the TDA: general fill, erosion protection admixture, radon barrier (clay), tailings, alluvium, and various bedrock units (including coal, shale, and sandstone). Some of these units were further subdivided according to notation in the borehole logs, results of CPT logs, and results of laboratory analysis. The Mine Site PDS investigation included laboratory testing of mine spoils material.

The material identifications used in the analyses are discussed below and are as follows: erosion protection, cover soil, mine waste, radon barrier, general fill, coarse tailings, fine tailings, coarse/fine tailings, coarse alluvium, and fine alluvium. Table G.6-1 presents the material properties associated with each of these materials used in the analyses.

Erosion Protection

The erosion protection layer will be the uppermost layer of the proposed Repository. It will be constructed by placing a mixture of cover soil (67 percent by volume) and aggregate (33 percent by volume). Properties for this material were developed based on the material properties of the cover soil and testing results (specific gravity) from on-site aggregate stockpiles. Other assumptions regarding the erosion protection are as follows:

- It is assumed that this material will be placed at 90 percent relative compaction per standard Proctor.
- The aggregate in the erosion protection layer will have properties similar to those found in on-site stockpiles and tested as part of the PDS (MWH, 2014a).
- Due to the mixed nature of this material, it is assumed that the fines content of this material will be equal to 67 percent of that contained in the cover soil.
- This material will have a lower long-term moisture content than the cover soil due to the increased aggregate content.

Cover Soil

The cover soil consists of the material that will be placed over the mine waste as part of the Repository ET cover. It will be excavated from on-site borrow sources identified in the Mill Site PDS (MWH, 2014a) or from similar material to be excavated for the Jetty construction. These borrow sources were investigated during the Mill Site PDS and subsequent investigations conducted in the Jetty area. Samples obtained from the original borrow areas were classified as clayey and silty sands and low-plasticity clays. Fines contents range from 38 to 78 percent and plasticity index (PI) values range from 3 to 23 (MWH, 2014a). Samples from the Jetty area have fines contents (silt and clay-size particles by weight) ranging from 6 to 99 percent, with an average of 71 percent passing the no. 200 sieve. The Atterberg limits data from the Jetty area compared with the four original borrow areas indicates more high-plasticity clay (CH) materials and fewer silt samples (ML) (Stantec, 2019). The Dilco Hill Borrow Area is not a preferred borrow source and, at this time, there are no plans to use that material during the RA. Therefore, results of analyses performed on samples originating in the Dilco Hill Borrow Area were excluded when calculating the cover soil material properties. Other assumptions regarding the cover soil are as follows:

- The analyses related to Repository construction is based on the Jetty excavation being the primary source for cover borrow.
- The cover soil will be placed in a state of compaction similar to the in-situ conditions within the borrow areas.
- Specifications for the ET cover will require the soil be placed at 90 percent relative compaction per standard Proctor and have a water content no more than minus 3 percent from optimum water content.

Mine Spoils

Mine spoils consist of the material that will be removed from the Mine Site and placed into the Repository during the RA. Material properties of the mine spoils were estimated by averaging the results of laboratory tests performed on samples of mine spoils collected during the Mine Site PDS (MWH, 2014b). Mine spoils material was not tested for fines content and Atterberg limits. Therefore, due to the similarity between the geotechnical properties of these materials, it is assumed that the mine spoils material has the same fines content and Atterberg limits as the cover soils in the proposed borrow areas. The dry unit weights have a percent difference on average of 3.7 percent and the specific gravities have a percent difference of 1.1. In the absence of laboratory data, assuming other geotechnical properties are also similar is a reasonable approach. Other assumptions regarding the mine spoils are as follows:

- Mine spoils index properties are similar to the cover materials and are assumed to have the same fines content and PI as the cover soil.
- Mine spoils will be placed in horizontal lifts at 90 percent relative compaction per Standard Proctor.

Radon Barrier

The existing radon barrier over the TDA consists of low-plasticity clay with fines contents ranging from 51 to 69 percent (MWH, 2014a). Prior to construction of the Repository, the radon barrier within the footprint of the proposed Repository will be reconditioned and improved. Activities to improve the radon barrier will include: stripping the upper erosion protection material, separating unsuitable materials (organics and erosion protection rock), moisture conditioning the surface of the radon barrier material, and re-compaction of the radon barrier material. The radon barrier will be compacted to 95 percent relative compaction per Standard Proctor at a water content of less than a maximum of minus 1 percent of the optimum water content.

Strength Parameters of Cover, Mine Spoils, and Radon Barrier

Triaxial testing was not performed on the borrow material, mine spoils, or radon barrier material; therefore, strength parameters (cohesion, c , and friction angle, ϕ) for these materials were based on strength parameters obtained from triaxial testing on a sample of embankment material from borehole TI-B3 at 21 feet below ground surface (bgs). The average in-place dry densities were calculated to be 103.5 pcf, 106.5 pcf, and 104.7 pcf for the borrow material (Cover), mine spoils, and radon barrier materials, respectively. These values were compared to the Embankment material average in-place dry density of 105.3 pounds per cubic foot (pcf). The average fines content and PI of the embankment material are 50 percent and 13.5 percent, respectively. The average fines content and PI of the cover material (and assumed mine spoils) is 53 percent and 12 percent, respectively. The average fines content and PI of the radon barrier are 59 percent and 16 percent, respectively. Given the percent fines, PI, and liquid limit of these materials, each would classify as CL according to USCS (Unified Soil Classification System). Literature values (Lambe & Whitman, 1969, Figure 21.4) show that typical effective friction angles range from 28 degrees to 35 degrees for normally consolidated clays classified as CL with a similar plasticity index to the materials described above. Therefore, the triaxial shear strength test results of the embankment material (effective friction angle of 32 degrees) is considered representative of each of these three materials based on comparison of material index properties (i.e., dry density, fines content, and PI) and literature values.

General Fill

General fill was encountered in the TDA during the Mill Site PDS subsurface investigation. The existing fill material properties were calculated from the average of the laboratory test results from samples obtained from the existing impoundment material described as 'fill'. The fill samples were generally classified as CL, but some were classified as SM and SC. Laboratory testing indicates that this material is sandy and low-plasticity clays with fines contents ranging from 35 to 72 percent and PIs ranging from 17 to 20.

Triaxial testing was not performed on the existing fill material, so the strength parameters (c and ϕ) for the fill material were also based on triaxial testing on a sample of embankment material from borehole TI-B3 at 21 feet bgs. The embankment material has a slightly higher density and fines content than the existing fill material, and slightly lower PI value than the existing fill

material. Based on these relationships between the two materials, it was assumed that the strength properties of the fill material would also be slightly lower (Figure 21.4 from Lambe and Whitman, 1969). The cohesion value was assumed to be zero, and the phi values were assumed to be 10 percent less than the embankment material phi values, resulting in a phi value of 29 degrees. This correlates well to the lower end of phi values shown in Lambe and Whitman (Figure 21.4) for PI values of approximately 20 percent. Since some of the fill material samples were classified as SM and SC, typical phi values for fine sands were also considered. As shown in Lambe and Whitman (1969) Figure 11.11, the phi value for fine sands with an initial void ratio of 0.67, would also be approximately 30 degrees. Based on literature values, an assumed phi value of 29 degrees is a conservative assumption given the known material properties of the existing fill.

Tailings

Tailings produced by the UNC uranium mill were deposited in the TDA. They range from silty and clayey sands to sandy clays and high plasticity clays. The fines content of tailings samples analyzed during the Mill Site PDS ranged from 7 to 97 percent and the fine-grained particles ranged from non-plastic to a PI of 61. Due to this wide range of material properties, the tailings have been subdivided into three categories: coarse tailings, fine tailings, and coarse/fine tailings.

During the milling process, a significant amount of sulfate was introduced into the tailings (from the sulfuric acid used for acidification), causing gypsum to precipitate. The concentration of gypsum in the tailings varies with grain size, with the greatest amount of gypsum likely present in the fine-grained tailings.

The presence of gypsum is known to affect certain geotechnical laboratory test results, specifically particle-size distribution and water content. Results of water content tests performed on samples containing gypsum are often artificially elevated due to the loss of molecular water within the gypsum. For this reason, samples containing gypsum should be dried at a lower temperature to remove the soil (pore) moisture without removing the molecular water from the sample.

As presented in the Mill Site PDS (MWH, 2014a), some of the tailings samples were analyzed at the standard temperature (110°C). Other samples were analyzed at both the reduced temperature recommended by ASTM (60°C) and the standard temperature. The reason for the reduced temperature drying was the presence of gypsum in the samples. At the standard temperature (110°C), gypsum will decompose and release molecular water. This molecular water is fundamentally different than the pore water that defines water content. The lower temperature will evaporate the pore water without decomposing the gypsum. The results of analyses performed on samples at both temperatures were used to develop a correlation between the artificially elevated water contents measured during the tests performed at the standard temperature and the more appropriate results from samples dried at the lower temperature. The higher temperature drying yielded water contents 0.5 percent to 3 percent larger than the lower temperature drying (due to the loss of water contained in molecular bonds). Tests that were only conducted at 110°C were therefore decreased by the same ratio that was observed empirically in samples tested at both temperatures. This process and the correlation between these values are discussed in more detail in the Mill Site PDS (MWH, 2014a). The water content, specific gravity, and unit weight values used to represent the fine tailings and coarse/fine tailings in the design analyses were obtained from averaging the results of samples performed at 60°C, as well as the adjusted results of tests performed at 110°C.

Coarse Tailings

Tailings identified as coarse tailings were generally unsaturated and exhibited higher tip resistance and sleeve friction during cone penetration tests (CPTs) than the fine-grained tailings. Coarse tailings are sandy and have a fines content of less than 40 percent. Atterberg limits testing performed on coarse tailings indicated that the fines in all samples were non-plastic. The coarse-grained tailings material properties were calculated from the average of the laboratory test results on the samples labeled 'coarse tailings' from the impoundment samples. The friction angle was determined from triaxial testing from a sample at borehole location TI-B1 at a depth of 27 feet, where coarse grained tailings were identified, and is considered representative of the coarse tailings based on material index properties.

Fine Tailings

Fine tailings exhibited lower tip resistances during CPTs than those exhibited by coarser tailings. CPT sleeve frictions of the fine tailings were also lower than those exhibited by coarser tailings and were often near zero. Fine tailings are clayey, have fines contents ranging from 69 to 97 percent, and PIs ranging from 27 to 61. The fine-grained tailings material properties were calculated from the average of the laboratory test results on the samples labeled 'fine tailings' from the impoundment samples, using the results of water content, specific gravity, and unit weight performed at 60°C, as discussed previously. The friction angle for the fine tailings material was determined from triaxial testing from a sample at borehole location TI-B1 at a depth of 31 feet, where fine-grained tailings were identified, and is considered representative of the fine-grained tailings within the impoundment.

Coarse/Fine Tailings

Tailings samples containing between 40 and 65 percent fines were identified as coarse/fine tailings. Coarse/fine tailings are classified as clayey sands and sandy clays according to the USCS. Coarse/fine tailings samples analyzed during the Mill Site PDS had fines contents ranging from 48 to 57 percent and PIs ranging from 17 to 24. Tailings samples identified as coarse/fine tailings were generally near 100 percent saturation (83 to 99 percent). Triaxial testing was not performed on the coarse/fine tailings material and strength parameters were not applied to this material. The use of strength parameters (friction angle and cohesion) is only applicable to the stability analysis, and this material was not used in the stability analysis. As discussed further in Attachment G.2, layers of coarse/fine tailings were conservatively assumed to be fine tailings.

Alluvium

The TDA was constructed on top of unconsolidated alluvium which overlies bedrock. The alluvium consists of a mixture of sand, silt, clay, and to a lesser amount, gravel. The fines content of alluvium samples analyzed during the Mill Site PDS ranged from 17 to 91 percent and the fine-grained particles ranged from non-plastic to a PI of 31. Due to this wide range of material properties, the alluvium has been subdivided into two categories for applicable analyses: coarse alluvium and fine alluvium.

Coarse Alluvium

Alluvium samples with fines content less than 50 percent were identified as coarse alluvium. Generally, coarse alluvium is unsaturated silty or clayey sands with non-plastic fines. Coarse alluvium samples submitted for laboratory analysis contained 17 to 50 percent fines.

Fine Alluvium

Alluvium samples with fines content greater than 50 percent were identified as fine alluvium. Generally, Fine alluvium is sandy or lean clays with plastic fines. Fine alluvium samples submitted for laboratory analysis contained 61 to 91 percent fines with PIs ranging from 0 (non-plastic) to 31.

Tailings Impoundment Dam (Embankment)

The existing tailings impoundment dam (embankment) was classified as a sandy or silty clay to a clayey sand with a fines content ranging from 14 to 42 percent and a plasticity index ranging from 9 to 18 percent. The material properties were determined from the average of laboratory test results from samples collected in the embankment (labeled as 'dam') to a depth of 40 feet. The friction angle was based on triaxial testing from a sample taken from borehole location TI-B3 in the embankment material at a depth of 21 feet and is considered representative of the embankment material. The cohesion was conservatively assumed to be zero for the analyses. The friction angle was also compared to literature values in Lambe and Whitman (Figure 21.4, 1969). The average, 60th percentile, and maximum plasticity index values the embankment material (13 percent, 14 percent, and 18 percent, respectively) correlate to friction angles ranging from approximately 34 degrees to 31 degrees based on the trendline. The data used to generate the trendline fall above and below the line, but typically correlate to friction angles ranging from 37 degrees to 30 degrees for a plasticity index range of 13 percent to 18 percent, respectively.

Bedrock

Bedrock (sandstone) material properties were determined from information included in the boring logs as well as from the average of laboratory test results from samples labeled as 'sandstone' and/or 'claystone'. The cohesion value for the underlying bedrock was based on penetration resistance (blows/ft) data on the sandstone material, taken from the boring logs. A correlation between the penetration resistance and unconfined compressive strength was used to determine the cohesion value (Lambe and Whitman, 1969).

Table G.6-1: Summary of Material Properties

| Material | Specific Gravity, Gs | Void Ratio, e | Relative Compaction (%) | Dry Density (pcf) | Max Dry Density (pcf) | Optimum Water Content (%) | Moist Unit Weight (pcf) | Water Content (%) | Effective Friction Angle (°) | Total Friction Angle (°) | Fines Content (%) | Plasticity Index (PI) (%) | Coefficient of Consolidation, Cc | Comment/Justification |
|---------------------------|----------------------|---------------|-------------------------|-------------------|-----------------------|---------------------------|-------------------------|-------------------|------------------------------|--------------------------|-------------------|---------------------------|----------------------------------|--|
| Erosion Protection (rock) | 2.71 | 0.45 | 90 | 117.0 | 130.0 | - | 122.9 | 5.0 | - | - | - | - | - | Assumed maximum dry density and optimum water content. Assume 90 percent of maximum dry density and water content was calculated from optimum water content minus 3 percent. |
| Cover (soil) | 2.69 | 0.62 | 90 | 103.5 | 115.0 | 13.8 | 114.7 | 10.8 | 32 | - | 53 | 12 | 0.086 | Average maximum dry density and optimum water content from laboratory test results on borrow samples, excluding Dilco Hill. Calculated dry density from 90 percent maximum dry density and water content calculated from optimum water content minus 3 percent. Phi based on sample TI-B3 at 21 feet (median value of 3 tests), similar properties to borrow (i.e. fines and PI). Specification to be set at 90 percent of standard Proctor compaction. |
| Mine Spoils | 2.66 | 0.56 | 90 | 106.5 | 118.3 | 12.3 | 116.4 | 9.3 | 32 | - | 53 | 12 | 0.086 | Average maximum dry density and optimum water content from laboratory test results on mine samples. Calculated dry density as 90 percent of maximum dry density and water content calculated from optimum water content minus 3 percent. Strength based on embankment samples; density of recompacted mine spoils similar to the embankment fill. Specification to be set at a minimum of 90 percent of standard Proctor compaction. |
| Radon Barrier | 2.68 | 0.51 | 95 | 110.5 | 116.3 | 13.7 | 122.3 | 10.7 | 32 | - | 59 | 16 | - | Average maximum dry density and optimum water content from laboratory test results on radon barrier samples. Calculated dry density as 95 percent maximum dry density and water content as average of in-place water content of 'radon barrier' samples. Strength based on embankment CU samples, tested with similar properties to the radon barrier data (cover). Specification to be set at 95 percent of standard Proctor compaction and maximum of -1 percent to optimum water content. |
| Existing Fill | 2.69 | 0.67 | - | 100.7 | - | - | 113.8 | 13.0 | 29 | - | 48 | 19 | 0.086 | Average of results from laboratory test results for Impoundment - Samples labeled as "fill". Fill samples generally classified as CL, some SM, SC. Strength based on embankment fill, use lower value since density is lower than proposed fill and embankment samples |
| Coarse Tailings | 2.67 | 0.71 | - | 97.5 | - | - | 108.1 | 10.9 | 34 | - | 21 | 0 | 0.084 | Average of results from laboratory test results for Impoundment - Samples labeled as "coarse tailings", strength from B1 at 27 feet. |
| Coarse/Fine Tailings | 2.72 | 0.90 | - | 89.2 | - | - | 116.0 | 30.0 | - | - | 52 | 20 | | Average of results from laboratory test results for Impoundment - Samples labeled as "coarse/fine tailings". |
| Fine Tailings | 2.70 | 1.35 | - | 71.7 | - | - | 107.6 | 50.1 | 33 | 19 | 83 | 43 | 0.408 | Average of results from laboratory test results for Impoundment - Samples labeled as "fine tailings", strength from B1 at 31 feet. |
| Alluvium (All) | 2.72 | 0.73 | - | 97.9 | - | - | 114.8 | 17.3 | 22 | - | 57 | 22 | 0.090 | Average of results from laboratory test results for Impoundment - Samples labeled as "alluvium", strength based on clay alluvium from B3 at 56 feet. Used for stability analyses only. |
| Alluvium (Coarse) | 2.71 | 0.73 | - | 96.9 | - | - | 111.0 | 14.6 | - | - | - | - | - | Average of results from laboratory test results for Impoundment - Samples labeled as "alluvium". Coarse alluvium defined as alluvium samples with less than 50% fines. |
| Alluvium (Fine) | 2.74 | 0.72 | - | 99.4 | - | - | 120.7 | 21.4 | - | - | - | - | - | Average of results from laboratory test results for Impoundment - Samples labeled as "alluvium". Fine alluvium defined as alluvium samples with greater than 50% fines. |
| Dam | 2.66 | 0.55 | - | 107.0 | - | - | 119.1 | 11.3 | 32 | - | 45 | 13 | - | Average of results from laboratory test results for Impoundment - Samples labeled as "dam" to a depth of 40 feet at B3. |
| Bedrock | | | | 107.2 | | | 124.4 | 16.0 | | | | | | From laboratory testing on 3 samples of sandstone; Cohesion = 4000. |

G.7 SEISMIC HAZARD ANALYSIS

A site-specific probabilistic seismic hazard analysis (PSHA) and a deterministic seismic hazard analysis (DSHA) were conducted to develop seismic design criteria for the Repository. The complete seismic hazard analysis report is included as Attachment G.1. The probabilistic seismic hazard analysis is based on a seismotectonic model and source characterization of the Mill Site and surrounding area. The study evaluated an area within a 124-mile (200-km) radius surrounding the Mill Site. The SHA was performed to estimate the seismic hazard at the project site within a probabilistic and deterministic framework by characterizing potential seismic sources. The peak ground acceleration (PGA) calculated in this PSHA was used during design to evaluate liquefaction potential, seismic settlement, and slope stability for the Repository. A summary of the PSHA results are provided in Table G.7-1. A mean PGA value of 0.30 was used in the analyses for the 95% design.

Table G.7-1: Summary of PSHA Results

| Return Period (years) | V_{s30} (ft/s) | V_{s30} (m/s) | Mean PGA (g) |
|--------------------------|---------------------|--------------------|-----------------|
| 10,000 | 902 | 275 | 0.30 |
| | 1,348 | 420 | 0.28 |
| | 1,857 | 566 | 0.25 |

1. V_{s30} = shear wave velocity in the upper 30 meters.
2. g = acceleration due to gravity

G.8 SLOPE STABILITY ANALYSES

Static and pseudo-static slope stability analyses were conducted for the Repository. The slope stability analysis calculation brief, which includes a figure showing the cross-sections locations used for the analyses, is included as Attachment G.2. Three cross-sections were selected for stability analyses (shown on Figure G.8-1). The cross sections were selected as representative of the maximum loading conditions, critical slope geometry, and maximum fill height for the Repository. The cross-sections are located along the Repository slopes to represent loading conditions on the existing TDA and embankment, to evaluate design slopes of the final Repository cover slopes, and to evaluate the global stability of the final Repository and existing TDA embankment. Limit equilibrium slope stability analyses were performed using the GeoStudio software SLOPE/W (Geoslope International, 2016). Material properties and the geometry and stratigraphy of the selected cross-sections were based on the results of previous field investigations and laboratory analyses conducted during the PDS. The analysis evaluated both circular and block-type failure surfaces along the selected cross-sections.

The critical (lowest) calculated factors of safety for both static and pseudo-static loading conditions for each of the cross sections from the model outputs were evaluated against the required design factors of safety given by the US Nuclear Regulatory Commission (NRC) design guidance documents. A summary of the static and pseudo-static slope stability results are provided in Table G.8-1. The calculated factors of safety are greater than the recommended minimum factors of safety for each case evaluated. Additional slip surfaces and factors of safety are presented and summarized in Attachment G.2 for deep failures to bedrock, shallow failures, and failures that toe at the bottom of the cover and/or embankment for each analysis.

Table G.8-1: Summary of Slope Stability Results

| Cross Section | Failure Type | Loading Condition | Minimum Required Factor of Safety ⁽¹⁾ | Calculated Factor of Safety |
|--|--------------|-------------------|--|-----------------------------|
| Cross Section A – Southwest Slope | Circular | Static | 1.5 | 9.9 |
| | | Pseudo-Static | 1.0 | 1.8 |
| Cross Section A – Northeast Slope | Circular | Static | 1.5 | 2.7 |
| | | Pseudo-Static | 1.0 | 1.3 |
| | Block | Static | 1.5 | 2.7 |
| | | Pseudo-Static | 1.0 | 1.3 |
| Cross Section B – Repository Slope | Circular | Static | 1.5 | 7.9 |
| | | Pseudo-Static | 1.0 | 1.7 |
| Cross Section B – Existing Dam | Circular | Static | 1.5 | 2.4 |
| | | Pseudo-Static | 1.0 | 1.2 |
| Cross Section B – Arroyo Flood | Circular | Static | 1.2 | 2.6 |
| Cross Section C – North Slope | Circular | Static | 1.5 | 3.2 |
| | | Pseudo-Static | 1.0 | 1.7 |
| Cross Section C – North Slope (Entry/exit) | Circular | Pseudo-Static | 1.0 | 1.7 |
| Cross Section C – Arroyo Flood | Circular | Static | 1.2 | 2.6 |
| Cross Section C – South Slope | Circular | Static | 1.5 | 9.1 |
| | | Pseudo-Static | 1.0 | 1.8 |

1. NRC Regulatory Guide 3.11 (NRC, 2008)

G.9 SETTLEMENT ANALYSES

The settlement analyses conducted for the Repository includes immediate settlement, primary consolidation, secondary consolidation, and seismic induced settlement. The analyses were conducted to evaluate settlement due to placement of the mine waste and cover material on the existing TDA. The settlement analyses calculation briefs, except for seismically-induced settlement, are provided as Attachment G.3. The seismically-induced settlement calculation brief is provided as Attachment G.4. Figure G.8-1 shows the borehole and CPT locations, and the fill thicknesses used to conduct the analyses. The proposed surfaces for the top of mine waste and top of cover for the Repository were used to determine the proposed thickness of fill at each CPT and/or borehole location within the Repository. These fill thicknesses were used to determine the increase in stress within the Repository at each location.

Based on the filling plan progressing from north to south, the north slope of the Repository will be filled to design waste grade during early stages of filling. The north slope of the Repository would be completed several months before completion of fill placement in the southeast corner. During placement to the design waste grade and covering with a temporary cover of clean soil, settlement will be monitored, by survey while fill placement continues in other areas of the Repository. Settlement monitoring data will be collected during the construction period and compared to the predicted consolidation settlements to verify that no additional grading mitigation measures are necessary prior to completion of the final cover.

G.9.1 Immediate Settlement

Immediate settlement was calculated for one-dimensional settlement, following the guidelines presented in the NAVFAC 7.01 design manual (Department of the Navy, 1986), based on guidance in NUREG-1620 (NRC, 2003). The immediate settlement of the TDA surface near the perimeter of fill placement was evaluated to address potential impacts of cover cracking of the existing radon barrier as a result of differential settlement. Immediate settlement of the upper unsaturated materials (including the radon barrier) would occur rapidly and incrementally with each layer of mine waste and will therefore not impact the long-term performance of the Repository cover. Cover cracking of the existing radon barrier is discussed in Section G.10.

Immediate settlement was calculated at three locations (B15/CPT-15, CPT-26, and B1/CPT-01) on the southwest slope of the Repository. These locations were selected because they are closest to the area where the Repository fill will transition directly to an area where the existing radon barrier will remain in-place and unmodified. The primary focus for evaluating the immediate settlement is to determine the contribution to differential settlement near the perimeter. Within the interior of the Repository the immediate settlement will occur incrementally as fill is placed and will not affect the Repository. The results of the immediate settlement analysis were used to determine the extent of the impacts from Repository construction on the existing TDA cover. A summary of the results of the immediate settlement analysis are present in Table G.9-1.

Table G.9-1: Summary of Immediate Settlement Results

| Location | Immediate Settlement (ft) |
|----------|---------------------------|
| CPT-01 | 0.1 |
| CPT-15 | 1.0 |
| CPT-26 | 0.6 |

G.9.2 Primary Consolidation

Settlement during and following active construction of the Repository is anticipated to result from primary consolidation caused by fill placement and the resulting dissipation of porewater pressures in the fine-grained tailings. Primary consolidation was calculated using a one-dimensional consolidation settlement analysis following the guidelines presented in the NAVFAC design manual 7.01 (Department of the Navy, 1986) and based on guidance in NUREG-1620 (NRC, 2003). Each of the 25 CPT locations within the Repository footprint were used to estimate the primary consolidation. The soil profiles were created using

data from CPT and borehole testing locations conducted during the PDS (see Figure G.8-1). Consolidation was calculated using the fill thickness from the Repository design at the locations of the CPT hole locations. Seven of the 25 locations included boreholes paired with the CPT.

Because only the fine-grained tailings are near saturation, the primary consolidation calculations only include settlement results for fine-grained tailings. In locations where the estimated settlement is presented as 0.0, a near saturated layer or no fine-grained tailings layers were encountered. The primary consolidation was estimated for each location where fine-grained tailings were encountered in the subsurface profile, if the profile information along with the laboratory data, indicate the fine tailings are near saturation (85 percent degree of saturation or greater). Additionally, in locations where interlayered fine and coarse tailings were encountered, the overall thickness of this material was assumed to behave as fine tailings to present a more conservative estimate of primary consolidation settlement totals for the location. The calculated primary consolidation at each location is summarized in Table G.9-2, and the calculations are included in Attachment G.3.

Table G.9-2: Summary of Primary, Secondary, and Total Consolidation Results

| Location | Primary Consolidation (ft) | Secondary Consolidation (ft) | Total Primary and Secondary Consolidation Settlement (ft) |
|-----------------|----------------------------|------------------------------|---|
| CPT-01 (TI-B1) | 0.02 | 0.05 | 0.1 |
| CPT-02 (TI-B2) | 0.13 | 0.03 | 0.2 |
| CPT-04 | 0.17 | 0.04 | 0.2 |
| CPT-05 | 0.00 | 0.00 | 0.0 |
| CPT-06 | 1.03 | 0.22 | 1.2 |
| CPT-08 (TI-B8) | 0.51 | 0.21 | 0.7 |
| CPT-09 | 1.13 | 0.34 | 1.5 |
| CPT-10 (TI-B10) | 1.08 | 0.29 | 1.4 |
| CPT-11 (TI-B11) | 0.08 | 0.14 | 0.2 |
| CPT-12 | 0.02 | 0.03 | 0.1 |
| CPT-13 | 0.00 | 0.00 | 0.0 |
| CPT-14 | 0.20 | 0.05 | 0.3 |
| CPT-15 (TI-B15) | 0.00 | 0.00 | 0.0 |
| CPT-16 | 1.34 | 0.32 | 1.7 |
| CPT-17 | 0.40 | 0.09 | 0.5 |
| CPT-18 | 1.45 | 0.35 | 1.8 |
| CPT-19 | 0.59 | 0.21 | 0.8 |
| CPT-20 | 0.19 | 0.12 | 0.3 |
| CPT-23 (TI-23) | 0.00 | 0.00 | 0.0 |
| CPT-24 | 0.00 | 0.00 | 0.0 |
| CPT-25 | 0.00 | 0.00 | 0.0 |
| CPT-26 | 0.00 | 0.00 | 0.0 |
| CPT-27 | 0.00 | 0.00 | 0.0 |
| CPT-28 | 0.00 | 0.00 | 0.0 |
| CPT-29 | 0.00 | 0.00 | 0.0 |

G.9.3 Secondary Consolidation

Settlement estimates for the completed ET cover surface resulting from secondary consolidation (creep) are also included in the estimated overall total settlement. Secondary consolidation was calculated using a one-dimensional settlement analysis following the guidelines presented in the NAVFAC design manual 7.01 (Department of the Navy, 1986) and based on guidance in NUREG-1620 (NRC, 2003). The same soil profiles used for primary consolidation were used to estimate the secondary consolidation at 25 locations within the Repository footprint. The soil profiles were created using data from CPT and borehole testing locations conducted during the PDS. The secondary consolidation was calculated for each fine-grained tailings layer in each CPT vertical soil profile location. The summation of the secondary consolidation in each fine-grained tailings layer at one location resulted in the total estimated secondary consolidation at that location. The secondary consolidation calculations include estimates of secondary consolidation for locations where the fine-grained tailings are near saturation. The calculated secondary consolidation at each location is presented in Table G.9-2. In locations where the estimated settlement is presented as 0.0, a saturated or near saturated fine-tailings layer was not encountered. The overall total (primary plus secondary) consolidation estimates are also presented in Table G.9-2.

Figure G.9-1 shows the estimated total amounts of consolidation settlement expected to occur during the construction period as well as over the design life of the Repository. Immediate settlement is not included in the totals shown on Figure G.9-1 since immediate settlement will occur prior to the completion of the cover. In addition, a percentage of the consolidation settlement totals shown on the figure will occur prior to completion of the final surface of the cover as fill is placed. Therefore, the amounts shown represent upper limits for the post-construction settlement totals. The figure presents total estimated settlements based on profiles from the CPT and borehole locations as well as settled surface contours showing expected changes to the cover surface grading if the estimated total settlement were to occur. The contouring indicates changes to the design slopes will occur on the south facing slope and a portion of the east facing slope (over the east and south edge of the former borrow pit). However, based on the calculations, predicted settlements will not result in slope reversal or areas of ponding on the cover.

G.9.4 Seismic Settlement

Seismic settlement calculations were prepared to estimate the potential settlement that may occur within the footprint of the proposed Repository as a result of the design seismic event. Analysis of one-dimensional stratigraphic profiles was performed using the design seismic event which was characterized by the parameters presented in the PSHA (see Attachment G.1). The seismic settlement analysis used data collected during CPTs, hollow-stem auger (HSA) drilling, and laboratory testing from the PDS to estimate the magnitude of potential seismic settlement within the footprint of the proposed Repository.

Six one-dimensional stratigraphic profiles were developed and analyzed as part of the seismic settlement analysis. These six locations correspond with locations where shear wave velocities were measured during CPT. The stratigraphic profiles were developed based on conditions observed during the Mill Site PDS (MWH, 2014a) field investigation and modified to reflect proposed Repository construction (placement of mine waste and the Repository cover). During the Mill Site PDS field investigation, eight boreholes were “paired” with, and drilled adjacent to, CPT locations. Seven of these paired locations are within the footprint of the proposed Mill Site Repository, and shear wave velocity measurements were recorded during CPT at six of those locations. A summary of the seismic settlement analyses results is presented in Table G.9-3 and the calculation brief is included as Attachment G.4. These amounts of settlement are considered within the tolerable limits (6 to 12 inches) of seismic deformation for tailings impoundments described in NUREG-1620 (NRC, 2003).

Table G.9-3: Potential Seismic Settlement Resulting from the Design Seismic Event

| Borehole ID | Seismic Settlement (ft) |
|---------------|-------------------------|
| TI-B1/CPT-01 | 0.08 |
| TI-B2/CPT-02 | 0.12 |
| TI-B8/CPT-08 | 0.08 |
| TI-B10/CPT-10 | 0.12 |
| TI-B11/CPT-11 | 0.14 |
| TI-B15/CPT-15 | 0.09 |

G.10 COVER (RADON BARRIER) CRACKING ANALYSES AND STRESS INFLUENCE

G.10.1 Cover (Radon Barrier) Cracking

The differential settlement at the most likely location for cover cracking (along the tie-in between the proposed Repository and the existing TDA cover) was used to estimate the potential for cover cracking of the existing radon barrier. The cover cracking analysis calculation brief is included as Attachment G.5. The analysis was performed for the southwest edge of the proposed Repository, where the new cover ends on the existing TDA cover. Similar to the immediate settlement, these locations were selected because they are closest to the area where the Repository fill will transition directly to an area where the existing radon barrier will remain in-place and unmodified. Cover cracking of the existing radon barrier is calculated using the location with the maximum anticipated differential settlement between the new cover and the existing cover. Around the remainder of the Repository perimeter, the new cover extends to an existing swale or drainage channel on the east and south or to the west apron which extends to the top of the dam. On the north edge of the Repository, the new cover will extend beyond the limits of regraded tailings (Canonie, 1991) and to approximately the top of the North Drainage Channel. The purpose of the analysis is to determine if the stress increase from the fill placement results in detrimental differential settlement at the edge of the Repository that will negatively affect the radon barrier outside the Repository.

Using the overall total combined predicted settlements (immediate, primary consolidation, secondary consolidation, and seismic induced settlement) for subsurface profiles from TI-B15/CPT-15, CPT-26, and TI-B1/CPT-01, differential settlement was estimated between the southwest slope of the Repository and the radon barrier located immediately beyond the edge of the Repository. For CPT-26, where seismic settlement was not estimated, the estimate for the seismic settlement from TI-B15/CPT-15 was used in the total. These three locations are beyond (TI-B1/CPT1) or near the edge of waste placement (TI-B15/CPT-15, CPT-26) where the new cover material will transition directly to the existing radon barrier. The maximum differential settlement was estimated for each point over the distance to the edge of the proposed cover.

Table G.10-1: Estimated Differential Settlement and Cover Cracking Potential near the Edge of the Repository

| Borehole ID | Estimated Total Differential Settlement (ft) | Horizontal Distance to Edge of Cover (ft) | Resulting Slope Reduction (%) | Horizontal Strain (%) |
|---------------|--|---|-------------------------------|-----------------------|
| TI-B1/CPT-01 | 0.25 | 80 | 0.33 | 0.009 |
| TI-B15/CPT-15 | 1.10 | 180 | 0.61 | 0.008 |
| CPT-26 | 0.60 | 210 | 0.29 | 0.003 |

To evaluate the potential for cracking in the existing radon barrier a relationship between tensile strain and PI of the soil is used (Morrison-Knudson, 1993). The PI value for the existing radon attenuation layer was estimated to be 16 percent, calculated as the average of the measured PIs of ten radon barrier samples collected during the PDS (MWH, 2014a). Using this value for PI, the minimum estimated horizontal tensile strain that will induce cracking is 0.10 percent. The resulting slope reduction from the estimated maximum differential settlement predicted near the edge of the Repository was then used to calculate horizontal movement for the 21-inch-thick radon barrier in the central cell. The horizontal movement was then doubled based on Gourc et al. (2010) and Rajesh and Viswanadham (2010). The values of peak horizontal movement were then used to estimate peak horizontal strain, which was calculated to be less than 0.01 percent or one-tenth of the maximum allowable horizontal strain to prevent cracking for the three locations evaluated. These results indicate cover cracking will not occur for the proposed conditions.

G.10.2 Stress Influence Analysis

The shape of the Repository concentrates the greatest fill thicknesses in the middle of the layout, with thickness decreasing from the center to the perimeter. Due to the gently sloping surface of the Repository and only minimal fill thickness around the

perimeter, induced stresses beyond the edge of the Repository (due to fill placement within the Repository footprint) are expected to be minimal and would not influence the areas of the TDA outside of the Repository footprint. Due to the configuration of the Repository, there would be no compressive stresses that would extend outward from the edges of the Repository.

G.11 LIQUEFACTION

The potential for liquefaction of saturated tailings and alluvium beneath the proposed Repository during the design seismic event was evaluated for the 95% Design. The analysis was performed for the most critical condition (a design seismic event after completion of Repository construction) and based on the subsurface profile at the time the PDS field sampling was conducted. The liquefaction analysis calculation brief is included as Attachment G.6, with the method and results summarized below.

The liquefaction triggering analysis evaluated the potential for liquefaction of saturated tailings or underlying alluvium beneath the Repository, which may result in damage to the existing TDA radon barrier or compromising the effectiveness of the Repository in isolating mine waste. A liquefaction screening evaluation (Bray et al., 2009) was performed to identify zones of tailings or soil that may be susceptible to liquefaction. One-dimensional profiles were developed for analysis, based on conditions observed during the Mill Site PDS field investigation and modified to reflect proposed loading conditions. Identified zones of potentially susceptible materials within these profiles, as identified by the screening analysis, were evaluated for liquefaction potential using simplified liquefaction triggering analysis methods (Idriss and Boulanger, 2008; Youd et al., 2001).

The liquefaction triggering analysis used data collected during the CPT program, drilling and standard penetration testing (SPT), and laboratory testing to calculate the factor of safety (FS) against liquefaction for potentially susceptible zones of saturated material below the Repository. The primary liquefaction analysis used the results of CPTs. SPT results, where available, were used to provide secondary data against which the results of the CPT-based analyses were checked. The liquefaction triggering analyses incorporated supplemental data from laboratory testing and were performed according to the methods outlined in Idriss and Boulanger (2008) and Youd et al. (2001). The FS was calculated as the average of the FS values calculated by each of the analysis methods.

The liquefaction screening evaluation identified eight samples, out of 33 samples screened, that were moderately susceptible to liquefaction. The remaining 25 samples that were screened were not susceptible to liquefaction. Of the samples representing zones that were saturated or nearly saturated, two zones had minimum average calculated FS values below the acceptable FS criteria (per NRC, 2008) of 1.0 (at 0.9). These zones consisted of fine-grained tailings relatively deep in the tailings profile (33 to 45 feet depth) within Borrow Pit 1 (from borings B8 and B10).

The interlayered nature of the materials in Borrow Pit 1, and the proximity of the samples with low-FS values to each other, indicate that this zone of potential liquefaction represents a small percentage of the overall Repository foundation. The depth of these zones is also significantly below the depth of critical failure surfaces generated from pseudo-static slope stability analysis, along selected cross-sections, that include these zones.

The potential for liquefaction of the tailings or alluvium beneath the Repository footprint was evaluated using accepted screening and analysis methods. The potential for liquefaction only applies to materials that are saturated or nearly saturated and in a relatively loose condition. Due to the unsaturated condition of most of the underlying tailings and alluvium beneath the Repository footprint, the screening and analysis identified only two zones of materials at depth in Borrow Pit 1 that were moderately susceptible to liquefaction. The depth and localized nature of these two zones pose a risk for minor amounts of additional post-earthquake consolidation settlement, but no slope stability concerns are anticipated.

Liquefaction-induced settlement was estimated using the results of the liquefaction analysis and field data. Liquefaction-induced settlement occurs following a seismic event during which liquefaction occurs. The soil may experience a volume change as the excess pore water dissipates and the soil particles rearrange themselves. The method used for estimating this volume change is outlined in Idriss and Boulanger (2008), Section 4.4 "Post-liquefaction Reconsolidation Settlement". The liquefaction induced settlement calculations and results are included in Attachment G.6. Based on the field data and the results of the calculations, the potential for liquefaction-induced settlement at the site is contained in a localized area and occurs at a depth where surficial expression and damage to the radon barrier or ET cover is considered unlikely.

G.12 COVER DESIGN

The cover system design report prepared by Dwyer Engineering, LLC is included as Attachment G.7. The cover system design report includes analyses for erosional stability of the cover, ET cover design, water balance, infiltration, radon emanation, and bio-intrusion. The Design Report was updated in 2019 to include the use of Jetty soils as borrow material and is included as an addendum to Attachment G.7. The consolidation and groundwater evaluation report by Dwyer Engineering, LLC includes the tailings pore water migration calculations. The cover design consists of an erosion protection layer including rock overlying a soil layer. The thicknesses of these layers and the sizes of the rock used for erosion protection vary based on locations on the Repository. The layout for the different erosion protection layers and the cover design details for the three different Dwyer Engineering cover sections are shown on Drawing 7-09 in the Section 7 Drawings.

G.12.1 Cover Materials

Materials to be used for the cover will consist of: (1) soil from the onsite borrow areas, and (2) erosion protection rock both reused from the existing TDA cover and imported from an offsite rock quarry or quarries. Borrow materials are described in Appendix H. The ET cover has been modeled and designed based on the soil properties of the on-site borrow areas. Construction specifications (Appendix J) have been developed to provide quality assurance and material consistency for the materials from the borrow areas that are used for cover construction. Based on the relatively uniform geotechnical properties of the soils from the four borrow areas and the topsoil stockpile, soils from any of these sources may be used in any order, for cover construction. Mixing of the borrow soil materials is not required. A comparison of available borrow sources is included in Appendix H, Section H.4.1.9.3 and Figures H.4-1 and H.4-2. The Jetty soil was approved by Dwyer Engineering, LLC as use for borrow material for the repository cover. Rock to be used for the erosion protection layer on the cover will vary in size (1.5 to 3.5 inches), depending on location and slope length. The erosion protection layer will consist of a mixture of soil and 33 percent rock by volume. The rock will meet NRC requirements for durability or be appropriately upsized. Upsizing will depend on the rock source selected for construction. Previous geotechnical testing of the borrow soils and durability testing of candidate rock sources for the project are summarized in Appendix H.

G.12.2 Erosion Protection Design

The Dwyer Engineering, LLC report included as Attachment G.7 includes the erosion protection designs for the ET cover admixture and the perimeter fill material (transition areas) located on the west and southwest sides of the Repository. Erosion protection designs for the 20% slope located on the east side of the Repository are included in Attachment G.8. Due to the steepness, this slope was designed as a riprap slope rather than as an ET cover with an admixture layer. However, in order to match the vegetation to be incorporated on the other areas of the cover, the 1.5-inch gravel will be mixed with 15% soil by volume and seeded with the cover seed mix. The rock alone provides full erosional stability. The incorporation of vegetation in this area is intended to match the overall vegetated cover surface and provide additional transpiration on the slope. The rock size for the 20% slope is a minimum D_{50} of 1.5 inches and is similar to the design rock sizes for other areas of the Repository (D_{50} of 3.5 inches or less) due to the length of the slope and the small catchment area.

G.12.3 Radon Flux Measurements

The Repository cover is designed and constructed to limit the release of radon-222 (radon) to the atmosphere, not exceeding an average release rate of 20 picocuries per square meter per second (pCi/m²s). The ET cover design report (Attachment G.7) includes radon emanation calculations for the cover. Following completion of the erosion protection layer, radon emission testing will be conducted to verify that the design and construction of the cover is effective at limiting radon flux using the method described in 40 CFR part 61, Appendix B, Method 115. Section 2 of Method 115 describes the radon flux measurements.

Consistent with Method 115, a single set of radon flux measurements will be made over the entire Repository. The Repository will be considered a region within the existing TDA. Method 115 specifies a minimum of 100 measurements for a region. Radon flux measurements over the Repository will be made at 102 equally spaced locations (grid nodes of 150-foot square grid cast over the Repository). A radon measurement procedure, similar to the detailed measurement procedure provided in Appendix A of USEPA 520/5-85-0029 will be used to measure the radon flux on the Repository. This radon flux measurement procedure

involves adsorption of radon on activated charcoal in large-area canisters. The radon canisters will be placed on the surface of the pile and allowed to collect radon for 24-hour period. The radon flux measurements will not be initiated within 24 hours of a rainfall and will not be performed if the ambient temperature is below 35°F or if the ground is frozen. The radon collected on the charcoal will then be measured by gamma-ray spectroscopy.

The mean of the radon flux measurements will be calculated for verification of the 20 pCi/m²s radon emission performance standard for the Repository region. Results of the individual radon flux measurements with locations will be included in the as-built report for the Repository.

G.12.4 Repository Cover Gamma Exposure Rate Measurement

Criterion 6(1) of 10 CFR Part 40, Appendix A specifies that: "Direct gamma exposure from the tailings or wastes should be reduced to background levels." A direct gamma radiation survey will be performed following placement of the ET cover to verify that the direct gamma exposure attains the required ambient background levels. The results of this survey will be compared to the survey conducted prior to removal of the erosion protection layer on the existing TDA. The direct gamma radiation survey will be performed at the same 102 locations as the radon flux measurement locations described in the previous section. The direct gamma radiation survey will be conducted during radon canister placement for radon emission testing. The gamma survey will consist of a one-minute static gamma measurement at each location over the Repository area. The gamma radiation levels will be measured in exposure rate (uR/hr). The static gamma radiation survey will be conducted consistent with the static survey described in Appendix T.2 (Final Status Survey) using a 2x2 NaI(Tl) scintillation detector interfaced with a scaler/rate meter. The mean of the direct gamma exposure rates will be calculated for comparison to the background levels. Results of the individual direct gamma exposure rate measurements with locations will be included in the as-built report for the Repository.

G.13 GREEN AND SUSTAINABLE REMEDIATION CONSIDERATIONS

The areas where Green and Sustainable Remediation (GSR) has been evaluated for the mine waste Repository design relate to: (1) construction materials (characteristics, manufacturing and transportation considerations), (2) construction methods, and (3) low impact/sustainability measures during construction. The 'BMP Process', as outlined in the 'Standard for Greener Cleanups' (ASTM, 2016), has been followed to select and prioritize BMPs for implementation during remedial action. The BMPs relating to Mine Waste Repository Design are listed below, for a complete description of the BMP Process and list of all GSR BMPs see Section 4 of the Main RD document and Appendix A (Section A.5).

G.13.1 Construction Material Considerations

The borrow soils for the cover, as described in this appendix and Attachment G.7 will be sourced locally to the Repository. Based on similarities in the borrow properties by area, preference will be given to the shortest haul distances to limit emissions during cover construction. The smaller (1.5-inch) rock to be used in the Repository cover will come from recycling existing erosion protection rock on the TDA cover. This material will be stripped from the surface, within the footprint of the Repository, and screened from soil before being applied and mixed into the new cover. Other rock and materials stockpiled at the site will be used for construction of temporary roads or laydown areas, to the extent possible, to limit the need to import materials.

The ET cover has been designed with a seed mix of select drought-resistant plants for the upper vegetative layer, to reduce maintenance needs. The seed mix for the Repository is described in Appendix U. Preference will be given to using non-synthetic nutritional soil amendments such as compost instead of chemical fertilizers for the plant growth layer. If temporary erosion control measures (BMPs) are required around the Repository, preference will be given to 100 percent recycled materials rather than virgin materials for erosion controls.

G.13.2 Construction Methods

The upper, erosion protection layer of the cover (rock, soil, and growth amendment) can be mixed in place using specialized equipment, rather than through a series of steps to place thinner layers of premixed materials, which will require more haul and placement traffic to place the materials. This approach could minimize operations of front loaders and other heavy machinery during cover placement. As described in Appendix D, new roads to the Repository will be minimized to the extent possible to reduce the required construction equipment operating time, greenhouse gas emissions, fill material, and habitat disruption. Roads will be constructed from in-situ native soils to reduce material haul distances and use of imported materials. Construction equipment will be appropriately sized to reduce fuel consumption and greenhouse gas emissions. Dust suppression will be utilized in the Repository area and along the access roads to decrease visible dust related emissions. Section G.5.3.1 discusses temporary stormwater controls for the Repository area, and Appendix E identifies BMPs and specific sediment control measures that will be employed during construction for both sediment and stormwater control.

G.13.3 Low Impact Development/Sustainability

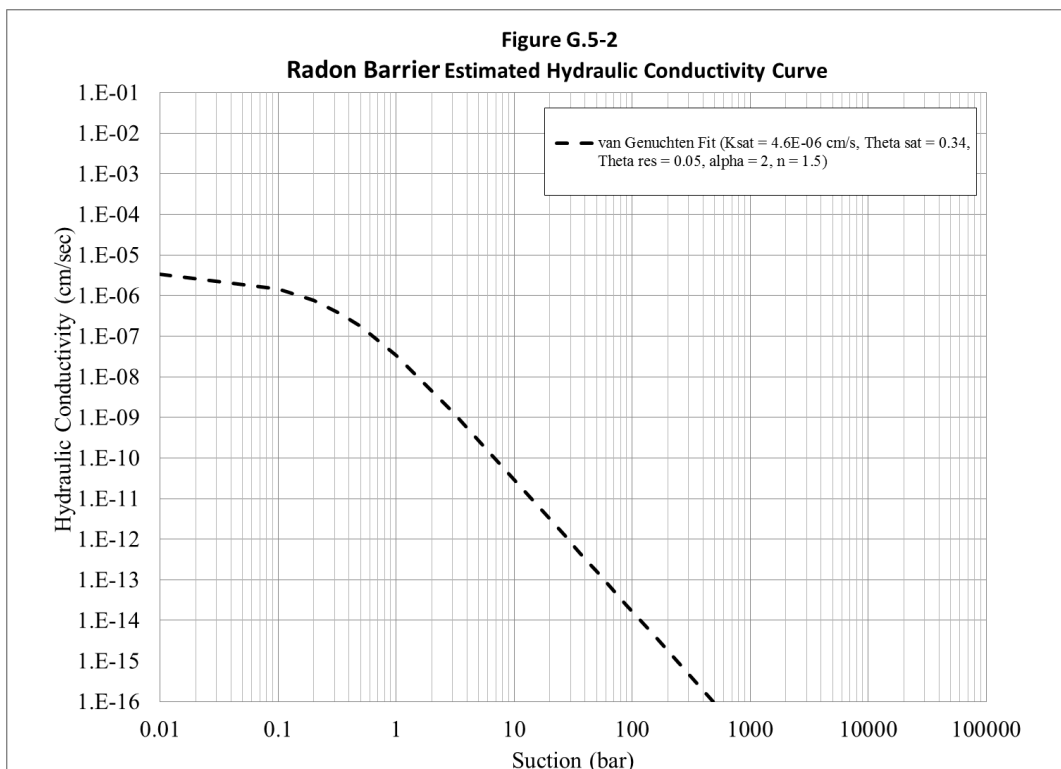
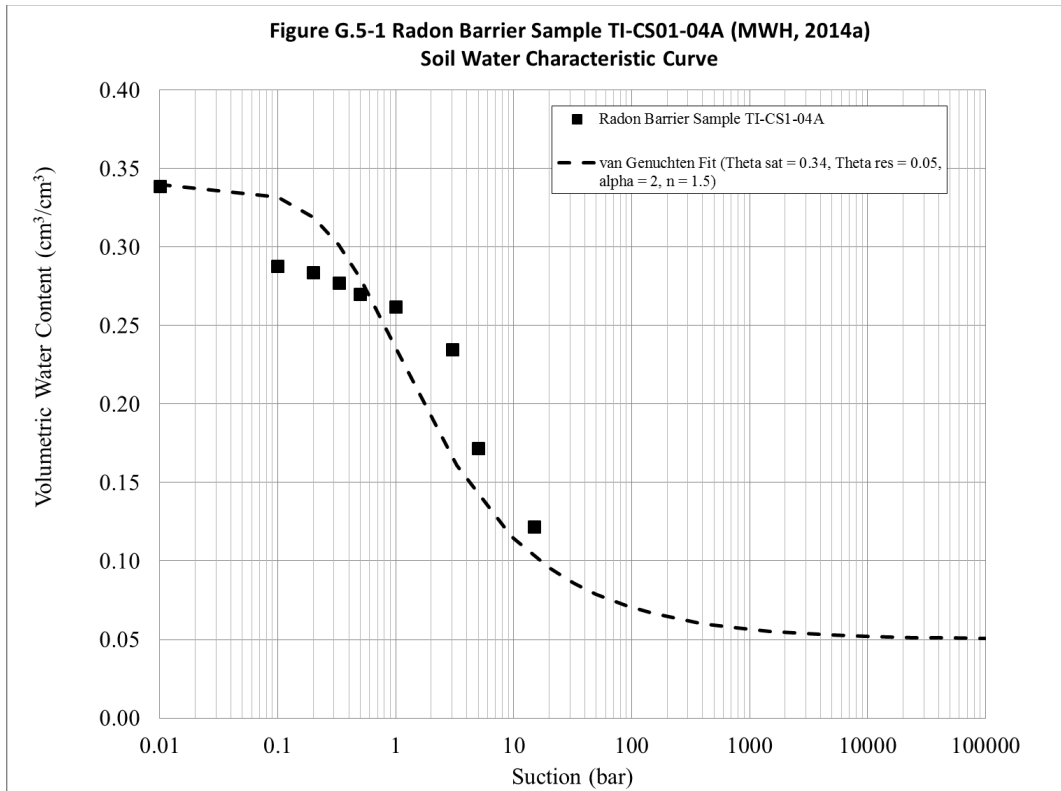
For an equally protective design, limiting the overall size (footprint) of the Repository is a key consideration of GSR, since the larger the overall footprint of the Repository is, the more materials will be required to construct it. However, construction of the Repository on the existing TDA, as well as the use of two previously disturbed borrow areas and primarily existing roads, limits the disturbance to undisturbed land. Additionally, the improvement and reuse (in-place) of the existing radon barrier that will become the foundation of the Repository limits the need for additional haulage to bring in a clean base layer of soil for the Repository. Access and haul routes are optimized to minimize site disruption, vehicle mileage, and to protect public health and safety. Minimizing vehicle mileage and limiting speeds is a high yield action as it limits fuel consumption, minimizes emissions of both greenhouse gasses and dust and increases site safety by reducing likelihood of both minor and serious crashes. Access and haul roads chosen utilize existing or historical roads to the extent practical to limit additional habitat degradation and reduce amount of cut/fill and grading required. Access and haul roads will be reclaimed and revegetated as quickly as possible upon completion of construction.

G.14 REFERENCES

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FIGURES



ATTACHMENT G.2
Repository Stability Analysis

Client: *General Electric (GE)*
Project: *NECR 95% Design*
Description: *Mill Site Repository Stability Analyses*

Sheet: 1 **of** 10
Date: *10/14/2019*
Job No: *233001369*

ATTACHMENT G.2: MINE WASTE RESPOSITORY STABILITY ANALYSIS

| Revisioning | | | | | |
|-------------|------------|-------------------------------|-----------|---------------|------------|
| Rev. | Date | Description | By | Checked | Date |
| 0 | 5/26/2016 | Repository Stability Analyses | S. Moore | M. Witler | 6/14/2016 |
| 1 | 07/19/2017 | 95% Design | S. Downey | S. Abbaszadeh | 7/25/2017 |
| 2 | 05/24/2019 | Response to RAIs | S. Downey | J. Cumbers | 05/31/2019 |
| 3 | 10/14/2019 | Revise Cover Thickness | S. Downey | M. Witler | 11/8/2019 |

| Location and Format |
|--|
| <p>Electronic copies of these calculations are located on the Stantec internal project teamsite.</p> <p>The following calculations were generated using the following software:</p> <p style="padding-left: 40px;">GeoStudio (Slope/w) 2016, Version 8.16.1.13452 Microsoft Excel 2013</p> |

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| Objective |
|---|
| <p>This calculation brief documents the input data, methods, assumptions, and results of the static and pseudo-static slope stability analyses on the Church Rock Mill Site (Mill Site) repository after placement of the fill material and final cover material on the existing impoundment.</p> |

| Background |
|------------|
| |

Client: *General Electric (GE)*
Project: *NECR 95% Design*
Description: *Mill Site Repository Stability Analyses*

Sheet: 2 **of** 10
Date: *10/14/2019*
Job No: *233001369*

This analysis was performed as part of the design of the Removal Action (RA) at the Northeast Church Rock Mine Site (Mine Site) and the related Remedial Action (RA) at the Mill Site. The Mine Site and Mill Site are located on adjacent Sections, approximately one-half mile apart, and approximately 16 miles northeast of Gallup, in McKinley County, New Mexico. The sites are temporarily being treated as one facility for purposes of the RA. The combined site is referred to as the "Settlement Agreement Site" (SA Site).

Site History

The NECR mine is a historical uranium mine operated by United Nuclear Corporation (UNC). Mining development began in 1967 and ended in 1982. While the mine operated, it served as the principal mineral source for the UNC uranium mill. The uranium mill and its adjacent disposal cells make up the UNC Superfund Site (the "UNC Mill Site"). Remedial activities addressing source control and on-site surface reclamation are being implemented by General Electric/United Nuclear Corporation (GE/UNC) under the direction of the U.S. Nuclear Regulatory Commission (NRC), pursuant to the UNC facility's NRC license, and integrated with the US Environmental Protection Agency's (USEPA's) selected remedy for the groundwater.

The tailings disposal area (TDA) is an unlined facility bounded by an embankment and subdivided by cross-dikes into three cells, which are identified as the South Cell, Central Cell, and North Cell. An estimated 3.5 million tons of tailings were pumped in slurry from the mill to the TDA.

Proposed Remedial Action

The proposed repository will be constructed on top of the existing TDA and will incorporate controlled placement of mine waste on top of the existing TDA cover/radon barrier, with a final evapo-transpirative (ET) cover placed over the mine waste. Improvements to the existing TDA cover/radon barrier within the footprint of the proposed repository will be completed prior to placement of mine waste. **Figure 1** shows the location and grading of the proposed repository.

The design for the selected repository alternative will be evaluated as part of a NRC license amendment request for the existing licensed facility. The repository features that affect the licensed facility will meet the performance standards outlined in NRC regulations and areas of the existing facility affected by the repository construction will be evaluated for compliance. However, existing conditions of the facility not affected by the proposed repository were not evaluated as part of this analysis, as they are managed by the existing NRC license.

Site Description

The natural stratigraphy at the Mill Site is divided into two main components: the surficial unconsolidated deposits (alluvium) and the underlying consolidated bedrock units. The alluvium consists of a mixture of sand, silt, and clay with minor portions of gravel. Alluvial thicknesses at the site are usually around 50 feet, but exceed 120 feet in some locations. Generally, the uppermost bedrock unit at the site is the Upper Gallup Sandstone, though in some locations it is overlain by coal or the Mancos shale (MWH, 2014a).

The TDA was constructed on top of the native alluvium and deposition of tailings, via slurry, within the TDA resulted in an interbedded accumulation of tailings. TDA closure construction began in 1989 and was completed in 1995. Closure construction included placement of an interim cover (general fill) from 1989 through 1991 followed by placement of the final cover (radon barrier and erosion protection layer) from 1993 through 1995.

Measurements taken in alluvial monitoring wells show an alluvial groundwater table in the vicinity of the TDA at approximately 6,867 feet above mean sea level (amsl), which indicates that the alluvium is unsaturated above this elevation. Additionally, subsurface investigations of the TDA indicate that there is not a consistent static water level within the tailings or the alluvium above approximately 6,867 feet amsl. However, localized perched zones of saturation exist within the low-permeability, fine-grained tailings. These zones of saturation do not appear to extend beyond the fine-grained tailings into the higher-permeability coarse-grained tailings.

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Site Investigation

In 2013, MWH performed pre-design studies (PDS) at the Mill Site and Mine Site to supplement previous site investigations and collect data necessary to perform the Remedial Design (RD). Activities performed as part of the Mill Site PDS included: surveying, cone penetration tests (CPTs), drilling, standard penetration tests (SPTs), excavation and soil sampling, and subsequent laboratory testing through the tailings material. Geotechnical data collected during the PDS are presented in the PDS reports (MWH, 2014a and MWH, 2014b) and summarized in **Attachment A**. A list of the materials encountered within the TDA during the PDS is presented in the Model Inputs section below. Geotechnical properties for these materials, the assumed phreatic surface, and discussion of one-dimensional stratigraphic profiles developed for the stability analysis are also presented in the Model Inputs section.

Applicable Codes and Standards

NUREG 1620, Section 2.2 (NRC, 2003)
 USNRC Regulatory Guide 3.11, Section C (NRC, 2008)
 Technical Approach Document, Revision II, Section 6.2 (DOE, 1989)

Methods

General

The slope stability analysis evaluated the potential for slope failure within the proposed repository. One-dimensional soil profiles were developed for the analysis, based on conditions observed during the Mill Site PDS investigation and were updated to reflect the proposed repository construction. The analysis used data collected during the PDS field investigation which included CPTs, Hollow Stem Auger drilling, SPTs, and laboratory testing to calculate the factor of safety (FS) against slope failure within the repository.

Slope stability analyses are typically conducted for scenarios that represent critical conditions. Critical cross sections were identified for: (1) the existing embankment (dam) on the west and north sides of the impoundment, (2) steepest repository final cover slopes, and (3) global stability of the final repository and existing embankment. Three cross sections were selected and evaluated for the slope stability analyses. A figure showing the cross section locations is included as **Figure 2**.

Cross section A was selected through the southwest slope of the repository and continuing through the center of the repository, where the height of fill will be highest compared to the existing ground surface. This location was selected based on where the height of fill will be the greatest and through the steep side slope (20 percent) on the northeast side of the impoundment.

Cross section B was selected through the existing embankment on the northwest side of the repository and along the northwest facing slope. This location was chosen to evaluate the existing embankment to the west of the repository as well as the western slope of the repository.

Cross section C was selected through the existing embankment on the north side of the repository, through the center of the repository and continuing beyond the southern boundary of the repository. This location was chosen to evaluate the global stability of the existing embankment and repository.

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Slope Stability Modeling

The limit equilibrium slope stability analyses were performed using the computer software Slope/W Version 8.16.1.13452 (Geo-Slope International, 2016). The Morgenstern-Price method (Morgenstern and Price, 1965) with a half-sine function for inter-slice forces was used for performing the calculations in Slope/W. This method uses both circular and non-circular shear surfaces and satisfies both moment and force equilibrium.

Grid and radius definitions were used to determine the failure surfaces. A block failure definition was used on the steeper slope (20 percent) in cross section A. These failure definitions were chosen to best model the actual field conditions and anticipated failure planes. The pore water pressures utilized as inputs to the stability analyses were defined using a piezometric line in the cross sections. The groundwater level was estimated from PDS borehole data (MWH, 2014a) and alluvial well data from January 2016 (water levels provided by Chester Engineers, included as **Attachment B**). The existing embankment depth in cross section B was estimated from historical data and figures (Hemphill, 1969). Each cross section was analyzed for long-term static and pseudo-static seismic loading conditions. The minimum slip surface depth was set to 3 feet for all scenarios. The static conditions of cross-sections B and C were also analyzed under the Probable Maximum Flood (PMF) loading condition. The material properties and other assumptions associated with these conditions are discussed below in the Model Inputs Section.

Design Criteria

The critical (lowest) calculated factors of safety for both static and pseudo-static loading conditions for each of the cross sections from the model outputs were evaluated against the required design factors of safety given by the NRC design guidance documents. The minimum acceptable factors of safety were adopted from the regulatory guides listed in the Applicable Codes and Standards section (NRC, 2003; NRC, 2008; and DOE, 1989). The following lists the applicable scenarios and their respective minimum acceptable factors of safety used in this analysis from the guidance documents:

| <u>Scenario</u> | <u>Minimum Factor of Safety</u> |
|---|---------------------------------|
| Long-term static stability | 1.5 |
| Long-term static stability with flood stability | 1.2 |
| Long-term seismic stability | 1.0 |

Model Inputs

Material Properties

Material strength parameters and other properties used for the slope stability analyses are based on results from laboratory testing of samples collected during the PDS. The laboratory test results are summarized in Table 3-4 of the Church Rock Mill Site PDS Report (MWH, 2014a) and are also included in **Attachment A**. The parameters used as a base-case scenario for each material are discussed in the main text of Appendix G, Section G.6 and summarized in Table 1.

The material properties used for the base case analysis were taken as the average of all samples for a given material and parameter, with the exception of the friction angle. The lowest effective stress friction angle from laboratory triaxial testing was used for each material and the cohesion was conservatively assumed to be zero. For the slope stability analysis, the coarse and fine alluvium layers were not differentiated because there was not enough information from the CPT or boreholes near the analysis cross sections to properly define the contact between the two units. Therefore, the alluvium layer was conservatively modeled as a uniform unit with lower strength properties of the fine alluvium. Material properties were varied to the 30th and 60th percentile values and used in a sensitivity analysis to determine if, or how much, the results change with the varying material properties.

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For the pseudo-static analyses, the total stress friction angle was used for the fine-grained materials. It was assumed that the other materials would behave as drained materials in the occurrence of a seismic event, and the effective stress friction angles were used for all other materials. The total stress friction angle for the fine-grained tailings materials was calculated using two different methods. Calculations for the total stress friction angle are included in **Attachment C**. The first method used the peak (failure) points obtained from the sample being placed under three different confining stresses in a triaxial consolidated undrained (CU) laboratory test, conducted during the PDS.. The friction angle was calculated for each confining stress peak point using the equation shown below, and then the three resulting friction angle values were averaged.

$$\sin \phi = \frac{q}{p} \text{ (Lambe and Whitman, 1969, pg. 141)}$$

where:

$$p = \frac{\sigma_1 + \sigma_3}{2}$$

$$q = \frac{\sigma_1 - \sigma_3}{2}$$

where:

ϕ is the friction angle, degrees

p and q are the coordinates of a stress point in a given state of stress, psf

σ_1 is the major principal stress, psf

σ_3 is the minor principal stress, psf

The second method plotted the peak points used the line of best fit between the three points. After plotting, the data showed a negative cohesion value (negative y-intercept). Since a negative cohesion value is meaningless, the intercept was set to zero, which is then conservative because it decreases the slope of the line and, therefore, decreases the calculated friction angle. The equation from the line of best fit was then used to determine the friction angle using the equation shown below.

$$\sin \phi = \tan \alpha \text{ (Lambe and Whitman, 1969, pg. 141)}$$

where $\tan (\alpha)$ is the slope of the straight line approximation through the peak points of the stress-strain curves.

The difference of total stress friction angles for the fine-grained tailings materials using the two methods used was found to be 0.4 degrees (18.9 and 19.3). As such value of 19 degrees was used in the pseudo-static analyses for this material.

The material properties used in the stability analysis are summarized in **Table 1**. A table summarizing the samples and properties used from the PDS to estimate the material properties of each material type are included in **Attachment A**.

Seismic Coefficient

Stability analyses under seismic conditions were performed as pseudo-static analyses, where a horizontal acceleration or seismic coefficient is applied to the entire model. This seismic coefficient, in a very simplified manner, simulates the horizontal accelerations that are applied to the structure during an earthquake. A site-specific probabilistic seismic hazard analysis was performed for this project, where the lower bound V_{s30} resulted in the highest estimated mean peak ground acceleration (PGA) of 0.30g (see Appendix G, Attachment G.1). The design seismic coefficient of 67 percent

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(2/3) of the PGA was used for the pseudo static stability analysis, resulting in a design seismic coefficient of 0.20g (NRC, 2003).

The cross sections were analyzed for long-term static and pseudo-static loading conditions. According to NUREG-1620, Rev. 1, a pseudo-static analysis is acceptable in lieu of a dynamic analysis if the strength parameters used in the analysis are conservative, the materials are not subject to significant loss of strength and development of high pore pressures under dynamic loads, the design seismic coefficient is 0.20 or less, and the resulting minimum FS suggests an adequate margin of safety, as provided in NRC Regulatory Guide 3.11 (NRC, 2008). A pseudo-static analysis was used due to a seismic coefficient of 0.20g and conservative strength parameters used in the analyses. As discussed in the material properties section above and the sensitivity analyses section below, strength parameters selected were conservative, and were also varied to determine the material property effects on the seismic analyses.

Stratigraphic Profiles

During the Mill Site PDS, eight CPTs were paired with boreholes to correlate CPT results with direct observation of the materials encountered. Seven of these “paired” CPT locations are within the footprint of the proposed repository, as shown on **Figure 1**. The CPT data combined with the profiles from the borehole logs were used to define the thickness and texture of the soil layers, as well as the location of the contact between the tailings and underlying alluvium. The relationships used to define the tailings-alluvium contact are described in the Mill Site PDS (MWH, 2014a) and also in Appendix G, Attachment G.3.

These subsurface profiles were modified using the proposed repository design to reflect proposed conditions after repository construction. The one-dimensional profiles from the CPT and borehole logs were used to create a three-dimensional surface in AutoCAD (CAD) for the top of bedrock surface, bottom of tailings surface, and existing grade surface (MWH, 2014a). Sections were cut through the surfaces in CAD to create the cross-sections used in the slope stability analysis. Locations and depths of additional material contacts (i.e., existing fill, coarse tailings, fine tailings, and alluvium) were interpreted from the CPT and borehole logs nearest to the cross section location. The depths were interpolated between CPT and boring locations along each cross section. The new cover was assumed to be 4.5 feet thick, and the existing radon barrier was assumed to be 1.5 feet thick for all sections. The existing fill varies in depth and underlies the cover and radon barrier materials. The locations and depths of the additional material contacts (i.e., coarse vs. fine tailings) are discussed below. A figure of each cross section showing the stratigraphic profiles is included in **Attachment D**, before the results for each respective cross section.

Cross Section A Cross section A is oriented from southwest to northeast through each end of the repository and is nearest to boring and CPT locations TI-B15/CPT-15, CPT-05, and CPT-25. A brief description of findings from these investigation is presented below.

TI-B15 and CPT-15

Silty sand (coarse) tailings were encountered approximately 3 feet bgs according to the boring log for TI-B15. At approximately 27 feet below ground surface (bgs), clayey (fine-grained) tailings were encountered. At 30 feet bgs, alluvium was encountered as a dark brown silty sand material. The CPT investigation reflects the boring log with coarse tailings present from approximately 3 feet bgs to 30 feet bgs, overlying alluvium at 30 ft bgs. A thin layer of fine tailings were included in the slope stability analysis to be conservative.

CPT-05

It was determined from the CPT data at location CPT-05 that coarse tailings were encountered from 6 feet bgs to approximately 10 feet bgs. Alluvium material was encountered directly below the coarse tailings.

CPT-25

Tailings were not encountered at CPT-25 location. The CPT penetrated 2.5 feet bgs, which was determined to be cover and fill material.

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Based on the CAD surfaces and CPT and boring log data, it was determined that tailings were present in the repository in the southwest portion of the cross section. At approximately mid-point of the cross section, tailings material is not present and the existing radon barrier and a thin layer of existing fill material was assumed to be directly overlying the alluvium.

Cross Section B

Cross section B is oriented from northwest to southeast through the existing embankment to approximately the middle of the repository. Boring and CPT locations TI-B1/CPT-01, CPT-28, CPT-04, CPT-13, and CPT-26 were considered to estimate tailings material contact details and depths for Cross Section B. A brief description of findings from these investigation is presented below.

TI-B1 and CPT-01

Sandy (coarse) tailings were encountered at approximately 18 feet bgs according to both the borehole log and CPT data. The tailings encountered at approximately 29 feet bgs were very fine grained according to the boring log. The CPT data also assumed fine tailings from 29 feet bgs to approximately 34 feet bgs, where alluvium was encountered.

CPT-28

At CPT-28, coarse tailings were encountered from approximately 13 feet bgs to 24 feet bgs. The thickness of the coarse tailings is consistent between CPT-01 and CPT-28, and was used as the approximate thickness in the cross section near the embankment.

CPT-04

Coarse tailings were encountered from approximately 9 feet bgs to 15 feet bgs. Fine tailings were encountered from approximately 15 feet bgs to 19 feet bgs. As a comparison between CPT-28 and CPT-04, the fine tailings material gets thicker further away from the existing embankment.

CPT-13 and CPT-26

These locations were analyzed together, since cross section B is located between the two locations. It was assumed that the stratigraphy could be interpolated between the two locations. CPT-13 data shows that only cover/fill material was encountered at this location, and no tailings were encountered. CPT-26 shows a significant layer of coarse tailings from 12 feet bgs to 25 feet bgs. The thickness of the tailings were assumed to be approximately 7 to 8 feet thick at this location.

Based on the CAD surfaces and CPT and boring log data, there are two separate deposits of tailings material within cross section B. One deposit is present on the northwest side of cross section B, inside of the existing embankment; the other is present from approximately the middle of the repository to the southeast edge of the repository. The deposits are separated by alluvium and bedrock that rises steeply in elevation, and then dips back down towards the southeast. Cross section B captures a portion of the tailings located on the southeast side of the repository.

Cross Section C

Cross section C traverses north to south across each end of the repository boundary and was analyzed for global stability of the repository. Locations TI-B23/CPT-23, CPT-25, CPT-16, CPT-09, and CPT-14 were considered for material contact details and depths for Cross Section C. A brief description of findings from this investigation is presented below.

TI-B23/CPT-23

A thin layer (less than 1 foot thick) of fine tailings with sand was encountered at 13.4 feet bgs. Silty sand (coarse) tailings were encountered from 14.2 feet bgs to 16 feet bgs. At 16 feet bgs, alluvium material was encountered to the bottom of the CPT at 44 feet, where refusal was met.

CPT-25

As discussed in Cross Section A, CPT-25 did not encounter tailings material.

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CPT-16

Coarse tailings were encountered from approximately 9 feet bgs to 14 feet bgs. A thick layer of fine tailings was encountered from 14 feet bgs to 43 feet bgs.

CPT-09

Coarse tailings were encountered from approximately 7 feet bgs to 15 feet bgs. A thick layer of fine tailings was again encountered from approximately 15 feet bgs to 44 feet bgs.

CPT-14

Coarse tailings were encountered from approximately 9 feet bgs to 15 feet bgs. Fine tailings were encountered from approximately 15 feet bgs to 21 feet bgs.

The coarse tailings across CPT-16, -09, and -14 are fairly consistent at 5 to 8 feet thick. The fine tailings are thickest at CPT-16 and CPT-09 at 29 feet thick, but taper dramatically from CPT-09 to CPT-14 from 29 feet to 6 feet thick. Combining the CAD data with the boring log and CPT data, there are two separate deposits of tailings material within cross section C. The thick deposit of tailings is located on the south end, whereas a thin deposit (approximately 3 feet thick) of fine tailings overlying coarse tailings is present on the north end.

Groundwater

Groundwater was encountered during drilling in two of the boreholes (TI-B10 and TI-B11) within the footprint of the proposed repository. In both boreholes, the groundwater elevation was approximately 6,885 feet amsl. Groundwater was also encountered at about 6,903 feet amsl while drilling in boring B3 (drilled through the dam). In addition, alluvial wells 509D and EPA 23 (measured on 1/4/2016) show an alluvial ground water elevation of approximately 6,867 feet amsl. In the stability analyses, the groundwater is assumed to be at or slightly above 6,900 ft amsl. The selected groundwater elevation is conservatively higher than those encountered during historical investigations.

The stability of the repository and embankment dam is also analyzed for the probable maximum flood (PMF) event as discussed in **Appendix I**. The PMF estimated that the floodplain extents will overtop the pipeline arroyo adjacent to the repository, and encroach the west and north edge of the TDA and base of the repository. The PMF extents affected cross sections B and C, and these cross sections were analyzed with a scenario including a phreatic surface at the floodplain extents to the PMF (approximately elevation 6,955 ft amsl and 6,958 ft amsl in cross sections B and C, respectively).

A plan view and cross section showing the PMF floodplain extents is included in **Attachment E**. The groundwater well levels provided to Stantec by Chester Engineers in 2016 is included in **Attachment B**.

Sensitivity Analyses

A sensitivity analysis is performed to evaluate the impact of the embankment material strength parameters in the pseudo-static analysis, cross section B. This analysis is selected as the worst-case scenario because it yielded to lowest FS and therefore is expected to be more sensitive to lowered material strengths and varying material properties. Material properties and strengths were varied for the embankment materials only, since the critical slip surface passes exclusively through the embankment materials.

The material index properties (dry density and water content) are varied from the 30th percentile values, median, and average (base case) for the embankment material, while keeping the effective stress friction angle at the base case. As mentioned before, the lowest friction angle from the PDS laboratory samples of embankment material was used in the base case analysis, instead of the average value. Also, the cohesion was conservatively assumed to be zero, despite triaxial testing on the embankment samples showing the material had cohesion values between 130 and 200 pounds per square foot (psf). The effective stress friction angle (strength parameter) of the embankment material was also varied in one degree increments from 28 degrees to 32 degrees. These strength values correlated to literature values

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for the embankment material based on PI's ranging from 60th percentile to 30th percentile values, respectively. The material properties used in the sensitivity analysis are included in Table 3.

Model Inputs Summary

The materials were modeled with the Mohr-Coulomb failure criteria, except for the bedrock material. The sandstone bedrock material was modeled as impenetrable based on the results from SPT blow counts. According to Lambe and Whitman (1969), a blow count of greater than 32 blows per foot correlates to a strength of greater than 4,000 psf. The blow count data for the sandstone material averaged 50 blows per 1.5 inches; therefore, the bedrock was assumed to have a high enough strength to be modeled as impenetrable in the slope stability analyses, and slope failures are not anticipated to extend through the sandstone bedrock. The material parameters used as inputs in the stability analyses models are summarized in **Table 2**. The stability analyses used the effective stress friction angles, with the exception of using the total stress friction angle for the fine-grained tailings material in the pseudo-static analyses.

As discussed in the previous section, a seismic coefficient of 0.20g was used as the seismic load horizontal coefficient for the pseudo static stability analysis.

Pore-water pressures were drawn in as a piezometric line in each model at the elevation discussed above in the Groundwater section of Model Inputs (approximately 6,900 ft amsl). In cases where the PMF condition was analyzed, the flood-level piezometric line was applied in the model at the elevations specified previously for each cross section.

Results

The calculated FS results of the base case stability analyses for cross sections A, B, and C are presented in **Table 4**. Output files from Slope/w showing slip surfaces and factors of safety for each scenario are included in **Attachment D**.

The sensitivity analysis showed that varying the dry density, water content, and friction angle had little to no effect on the calculated FS for the analyzed scenarios. The slip surface location changed slightly, and the resulting FS was reduced from 1.2 to 1.1 when the 60th percentile values were used for the dry density and water content as well as when the friction angle was reduced. The output files for the sensitivity analysis are included in **Attachment D.1**.

Conclusions

The cross sections selected represented three different locations and failure surfaces in the proposed repository. Cross-sections were evaluated for: (1) the existing embankment (dam) on the west and north sides of the impoundment, (2) steepest repository final cover slopes, and (3) global stability of the final repository and existing embankment. The geometries chosen represent critical conditions (i.e., highest embankment and steepest slope)

The average, median, 30th percentile and 60th percentile values for all material properties were evaluated. Average properties were used as the base case scenario, and a sensitivity analysis was conducted on a critical section where the resulting FS was low, but still above the minimum required values. In the sensitivity analysis, material properties were varied, as well as material strength parameters.

Based on the stability analyses using the methods and material parameters presented above, the representative cross sections meet or exceed the minimum FS requirements for static and pseudo-static loading conditions for all scenarios considered, including the PMF. The calculated FS for Cross Section B, pseudo static analysis was low (1.2), but still acceptable. The Slope/W output figures for static and pseudo-static loading conditions are included in **Attachment D**.

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Attachments

Figure 1 – Borehole and CPT Locations
 Figure 2 – Repository Layout and Cross-Sections

Attachment A – Impoundment Laboratory Results from Pre-Design Studies (MWH, 2014a and MWH, 2014b)
 Attachment B – Recorded Water Levels at the Church Rock Site (Chester Engineers, 2016)
 Attachment C – Total Stress Friction Angle Spreadsheet Calculations
 Attachment D – Slope/W Output Files
 Attachment E – PMF Extents

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TABLES

Table 1: Summary of Base Case Material Properties Used in Slope Stability Analysis

| Material | Specific Gravity, Gs | Relative Compaction (%)¹ | Dry Unit Wt (pcf) | Moist Unit Wt (pcf)² | Water Content (%) | Effective Friction Angle (°) | Total Friction Angle (°) | Fines Content (%) | PI (%) |
|-----------------|-----------------------------|--|--------------------------|--|--------------------------|-------------------------------------|---------------------------------|--------------------------|-----------------|
| Cover (soil) | 2.69 | 90 | 103.5 | 114.7 | 10.8 | 32 | 28 | 53 | 12 |
| Mine Spoils | 2.66 | 90 | 106.5 | 116.4 | 9.3 | 32 | 28 | 53 ⁴ | 12 ⁴ |
| Radon Barrier | 2.68 | 95 | 110.5 | 122.3 | 10.7 | 32 | 28 | 59 | 16 |
| Existing Fill | 2.69 | - | 100.7 | 113.8 | 13.0 | 29 | 25 | 48 | 19 |
| Coarse Tailings | 2.67 | - | 97.5 | 108.1 | 10.9 | 34 | 28 | 21 | 0 |
| Fine Tailings | 2.70 | - | 71.7 | 107.6 ³ | 50.1 | 33 | 19 | 83 | 43 |
| Alluvium (All) | 2.72 | - | 97.9 | 114.8 | 17.3 | 22 | 20 | 57 | 22 |
| Dam | 2.66 | - | 107.0 | 119.1 | 11.3 | 32 | 28 | 45 | 13 |
| Bedrock | | | 107.2 | 124.4 | 16.0 | | | | |

Notes:

pcf = pounds per cubic foot, PI = plasticity index

All values are the average of laboratory testing results from the PDS (MWH, 2014a, b), unless otherwise noted.

¹Assumed

²Calculated

³Assumes material is fully saturated

⁴Assumed to be the same as cover soil

Table 2: Material Parameters Used in Slope Stability Analysis

| Material Identification | Moist Unit Weight (pcf) | Cohesion (psf) | Effective Stress Friction Angle (°) | Total Stress Friction Angle (°) ¹ |
|--------------------------------------|-------------------------|----------------|-------------------------------------|--|
| Borrow - New Cover | 114.7 | 0 | 32 | - |
| Mine Spoils | 116.4 | 0 | 32 | - |
| Radon Barrier Recompacted (1.5-feet) | 122.3 | 0 | 32 | - |
| Existing fill | 113.8 | 0 | 29 | - |
| Coarse Tailings | 108.1 | 0 | 34 | - |
| Fine Tailings | 107.6 | 0 | 33 | 19 |
| Alluvium | 114.8 | 0 | 22 | - |
| Dam (Embankment) | 119.1 | 0 | 32 | - |
| Bedrock | Impenetrable | | | |

pcf = pounds per cubic foot, psf = pounds per square foot

¹Total stress friction angle is only applicable to the fine tailings material

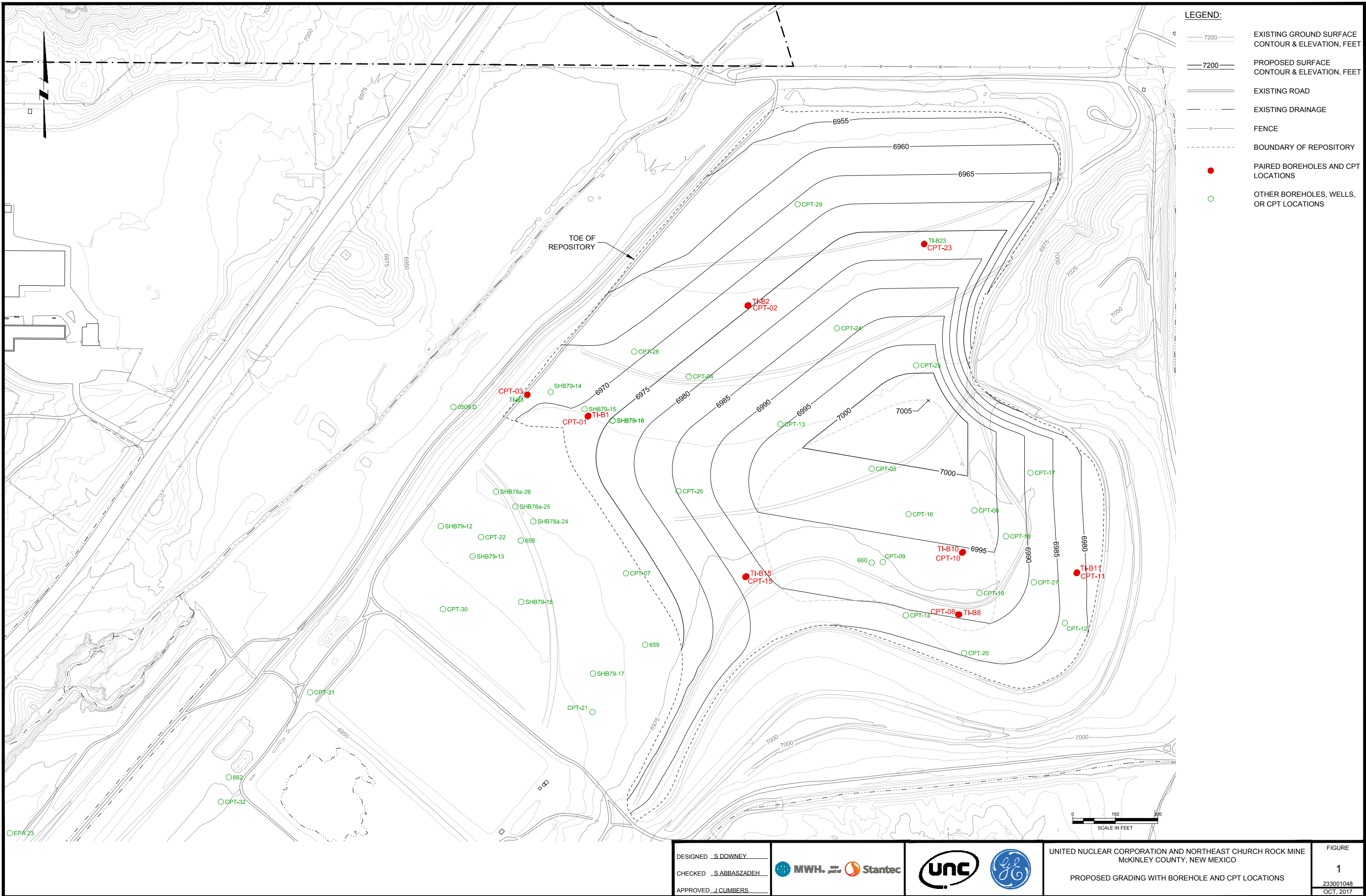
Table 3: Embankment Material Parameters Used in Sensitivity Analysis

| Sensitivity | Water content (by mass, %) | Dry density (pcf) | Calculated Moist Unit Wt (pcf) | PI (%) | Effective Friction Angle (°) |
|-----------------|----------------------------|-------------------|--------------------------------|--------|------------------------------|
| 30th percentile | 7.3 | 105.5 | 113.1 | 10 | 35 |
| Median | 12.0 | 107.6 | 120.5 | 12 | 34 |
| Average | 11.3 | 107.0 | 119.1 | 13 | 33 |
| 60th Percentile | 14.2 | 108.4 | 123.7 | 14 | 32 |

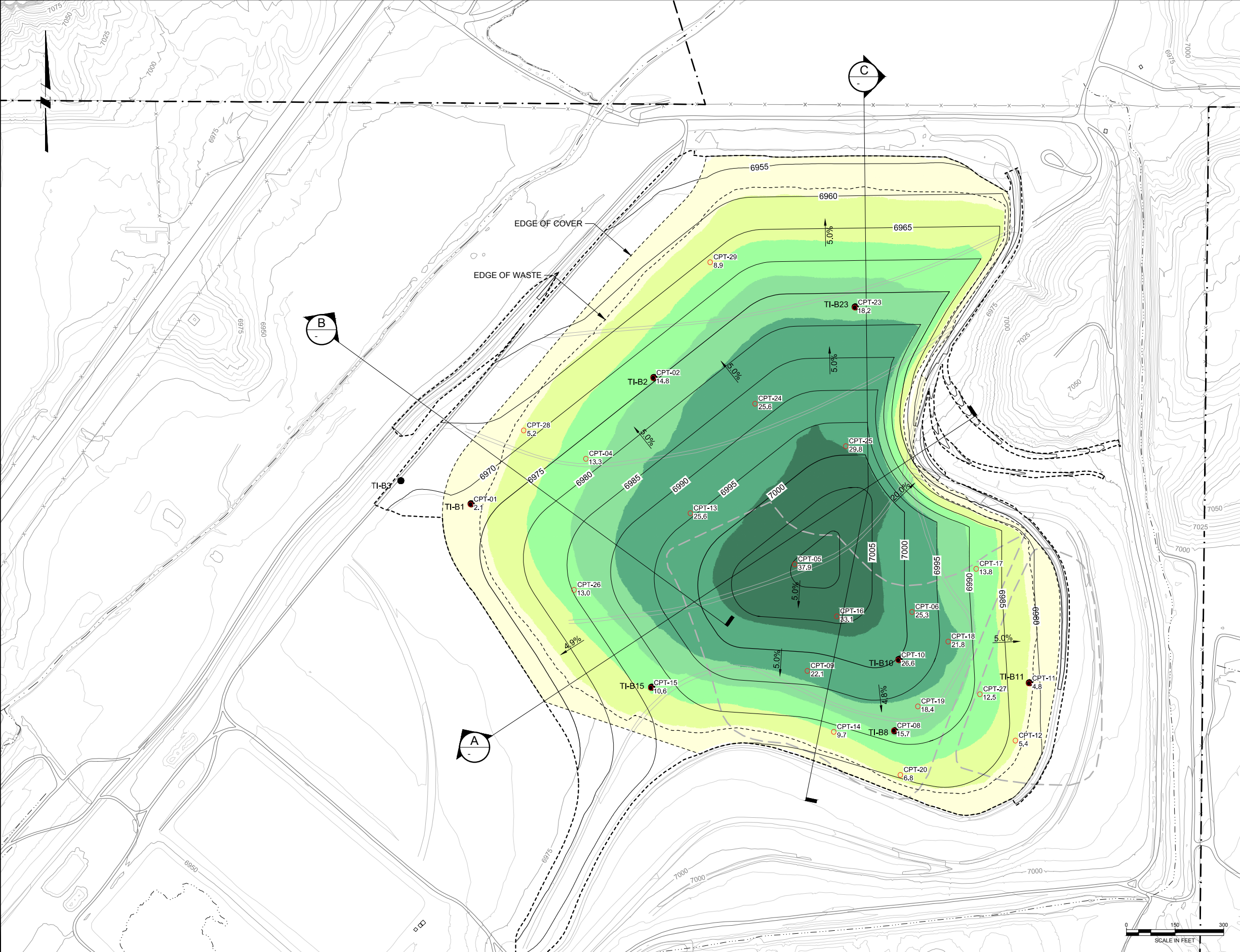
Table 4: Slope Stability Analyses Results

| Cross Section | Figure | Failure Type | Loading Condition | Required Factor of Safety | Calculated Factor of Safety |
|--|--------|--------------|-------------------|---------------------------|-----------------------------|
| Cross Section A – Southwest Slope | D.1 | Circular | Static | 1.5 | 9.9 |
| | D.2 | | Pseudo-Static | 1.0 | 1.8 |
| Cross Section A – Northeast Slope | D.3 | Circular | Static | 1.5 | 2.7 |
| | D.4 | | Pseudo-Static | 1.0 | 1.3 |
| | D.5 | Block | Static | 1.5 | 2.7 |
| | D.6 | | Pseudo-Static | 1.0 | 1.3 |
| Cross Section B – Repository Slope | D.7 | Circular | Static | 1.5 | 7.9 |
| | D.8 | | Pseudo-Static | 1.0 | 1.7 |
| Cross Section B – Existing Dam | D.9 | Circular | Static | 1.5 | 2.4 |
| | D.10 | | Pseudo-Static | 1.0 | 1.2 |
| Cross Section B – Probable Maximum Flood | D.11 | Circular | Static | 1.2 | 2.6 |
| Cross Section C – North Slope | D.12 | Circular | Static | 1.5 | 3.2 |
| | D.13 | | Pseudo-Static | 1.0 | 1.7 |
| Cross Section C – North Slope (Entry/exit) | D.14 | Circular | Pseudo-Static | 1.0 | 1.7 |
| Cross Section C – Probable Maximum Flood | D.15 | Circular | Static | 1.2 | 2.6 |
| Cross Section C – South Slope | D.16 | Circular | Static | 1.5 | 9.1 |
| | D.17 | | Pseudo-Static | 1.0 | 1.8 |

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LEGEND:

- EXISTING GROUND SURFACE CONTOUR & ELEVATION, FEET
- PROPOSED SURFACE CONTOUR & ELEVATION, FEET
- EXISTING ROAD
- EXISTING DRAINAGE
- FENCE
- BOUNDARY OF REPOSITORY
- FORMER BORROW PIT BOUNDARY
- CPT-17 CPT LOCATION
- TI-B15 BORING LOCATION
- SLOPE STABILITY CROSS SECTIONS

| REPOSITORY FILL THICKNESS | | | |
|---------------------------|----------|----------|-------|
| NUMBER | MIN (FT) | MAX (FT) | COLOR |
| 1 | 0.0 | 5.0 | |
| 2 | 5.0 | 10.0 | |
| 3 | 10.0 | 15.0 | |
| 4 | 15.0 | 20.0 | |
| 5 | 20.0 | 30.0 | |
| 6 | 30.0 | 42.0 | |

ATTACHMENT A

IMPOUNDMENT LABORATORY RESULTS FROM PRE-DESIGN STUDIES (MWH, 2014A AND B)

Table 3-1 Summary of Geotechnical Laboratory Data - Cover Samples

| Cover Layer | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (in) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass) (%) | Specific Gravity | Standard Proctor (max. dd@opt. w.c.) (pcf @ %) | Atterberg Limits (%) ⁽⁵⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | L.A. Abrasion ⁽⁶⁾ (%) loss | Sodium Soundness ⁽⁷⁾ (%) loss | Absorption ⁽⁸⁾ (%) | Pinhole Dispersion ⁽⁹⁾ | Remolded Saturated Hydraulic Conductivity ⁽¹⁰⁾ (cm/sec) | | | Confining Stress (psi) | SWCC: -5 bar Water Content (by mass) (%) ⁽¹⁰⁾ | SWCC: Saturated Water Content (by mass) (%) ⁽¹¹⁾ |
|-------------------------------|-----------------|----------------------------|----------------------------|----|-------------------------------------|---------------------|------------------------------------|-----------------------------|---------------------|--|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|---------------------------------------|--|-------------------------------|-----------------------------------|--|---------|---------|------------------------|--|---|
| | | | | | | | | | | | LL | PL | PI | | | | | | | | | | 90% | 95% | 100% | | | |
| Admix. (Gravel/ Soil Mixture) | TI - CS01 - 02A | Bulk | 0 | 11 | Clayey Gravel with Sand | | Loam | | | | | | | 33.3 | 23.4 | 43.3 | 28.0 | 15.3 | | | | | | | | | | |
| | TI - CS02 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Clay Loam | | 2.81 ⁽⁴⁾ | | | | | 36.9 | 17.0 | 46.1 | 28.8 | 17.3 | 3.8 | 0.37 | 1.06 | | | | | | | |
| | TI - CS03 - 02A | Bulk | 0 | 6 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.7 | 27.7 | 18.1 | 9.6 | | | | | | | | | | |
| | TI - CS04 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.2 | 28.2 | 18.0 | 10.2 | | | | | | | | | | |
| | TI - CS05 - 02A | Bulk | 0 | 9 | Sandy Lean Clay | | Loam | | | | | | | 13.9 | 34.4 | 51.7 | 31.2 | 20.5 | | | | | | | | | | |
| | TI - CS06 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | 2.77 ⁽⁴⁾ | | | | | 48.4 | 18.5 | 33.1 | 23.4 | 9.7 | 5.7 | 0.14 | 1.91 | | | | | | | |
| | TI - CS07 - 02A | Bulk | 0 | 20 | Sandy Lean Clay | CL | Loam | 7.8 | | | 28 | 13 | 15 | 1.1 | 41.0 | 60.9 | 42.4 | 18.5 | | | | | | | | | | |
| | TI - CS08 - 02A | Bulk | 0 | 8 | Clayey Gravel with Sand | | Loam | | | | | | | 56.7 | 18.5 | 24.8 | 17.2 | 7.6 | | | | | | | | | | |
| | TI - CS09 - 02A | Bulk | 0 | 9 | Clayey Gravel | | Loam | | 2.78 ⁽⁴⁾ | | | | | 53.6 | 14.2 | 32.2 | 21.2 | 11.0 | 5.1 | 1.17 | 1.55 | | | | | | | |
| | TI - CS10 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | | | | | | 41.4 | 19.7 | 38.9 | 26.1 | 12.8 | | | | | | | | | | |
| | TI - CS11 - 02A | Bulk | 0 | 9 | Clayey Gravel with Sand | | Sandy Loam | | | | | | | 30.7 | 30.1 | 39.2 | 26.1 | 13.1 | | | | | | | | | | |
| | TI - CS12 - 02A | Bulk | 0 | 14 | Sandy Lean Clay | CL | Loam | 9.1 | | | 33 | 13 | 20 | 1.3 | 28.8 | 69.9 | 43.5 | 26.4 | | | | | | | | | | |
| Radon barrier (clay layer) | TI - CS03 - 04A | Bulk | 6 | 24 | Sandy Lean Clay | CL | Loam | 6.0 | | | 28 | 14 | 14 | 6.3 | 38.7 | 55.0 | 36.1 | 18.9 | | | | | | | | | | |
| | TI - CS06 - 04A | Bulk | 7 | 24 | Sandy Lean Clay | CL | Loam | 11.0 | | | 30 | 13 | 17 | 6.7 | 34.2 | 59.1 | 40.2 | 18.9 | | | | | | | | | | |
| | TI - CS10 - 04A | Bulk | 7 | 25 | Sandy Lean Clay | CL | Loam | 7.7 | | | 29 | 14 | 15 | 2.3 | 39.5 | 58.2 | 36.9 | 21.3 | | | | | | | | | | |
| | TI - CS08 - 04A | Bulk | 8 | 28 | Sandy Lean Clay | CL | Loam | 8.1 | 2.67 | 119.4 @ 11.9 | 27 | 12 | 15 | 11.3 | 35.0 | 53.7 | 36.7 | 17.0 | | | | | 9.1E-06 | 1.1E-05 | 1.5E-06 | 24 | | |
| | TI - CS05 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Loam | 9.6 | | | 29 | 12 | 17 | 1.3 | 37.3 | 61.4 | 42.0 | 19.4 | | | | | | | | | | |
| | TI - CS09 - 04A | Bulk | 9 | 26 | Sandy Lean Clay | CL | Loam | 7.7 | | | 28 | 13 | 15 | 4.0 | 38.1 | 57.9 | 40.0 | 17.9 | | | | | | | | | | |
| | TI - CS11 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Clay Loam | 8.6 | 2.68 | 115.0 @ 14.9 | 32 | 13 | 19 | 5.1 | 28.4 | 66.5 | 40.7 | 25.8 | | | | | 7.6E-08 | 1.4E-07 | 1.0E-07 | 24 | | |
| | TI - CS02 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Sandy Clay Loam | 11.4 | | | 28 | 12 | 16 | 3.6 | 44.7 | 51.7 | 30.4 | 21.3 | | | | | | | | | | |
| | TI - CS04 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Clay Loam | 15.0 | 2.68 | 113.5 @ 15.0 | 35 | 15 | 20 | 0.9 | 35.0 | 68.2 | 37.2 | 26.9 | | | | | 4.6E-06 | 6.2E-06 | 2.3E-07 | 8 | | |
| | TI - CS01 - 04A | Bulk | 11 | 24 | Sandy Lean Clay | CL | Loam | 9.2 | 2.68 | 117.3 @ 13.0 | 29 | 15 | 14 | 2.0 | 39.8 | 58.2 | 39.0 | 19.2 | | | | ND3 | 3.0E-04 | 4.6E-05 | 7.8E-07 | 8 | 8.6 / 9.6 | 21.7 / 19.0 |

- Notes:** 1. Sample Types: Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. Bulk saturated surface dry (SSD) specific gravity of the gravel fraction, average of three results (ASTM C127).
5. LL = liquid limit, PL = plastic limit, PI = plasticity index
6. L.A. abrasion results are percent loss, by mass, for 100 revolutions.
7. Weighted percent loss for the 3/4-inch to 3/8-inch size range
8. Average of three results for the gravel fraction of the cover gravel/soil mixture samples
9. Pinhole dispersion test (ASTM method A) conducted on a specimen remolded to approximately 95% of the maximum standard Proctor density at optimum water content. ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head.
10. Flexible wall permeameter tests conducted on specimens remolded to approximately 90, 95 and 100% of the maximum standard Proctor density and tested at the confining stresses shown in the table.
11. SWCC test conducted on material passing the No. 10 sieve, remolded to approximately 95% of the maximum standard Proctor density and optimum water content. SWCC tests performed with pairs of specimens for each test.

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | saturation (%) | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | Atterberg limits (%) | | | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) |
|--------------|--------|----------------------------|-----------------------------|-------|-------------------------------------|----------------------------|---------------------------------|--------------------------------|----------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----------------------|----|----|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|
| CENTRAL | TI-B1 | CA | 36 | 36.5 | Alluvium Clayey Sand | coarse | 21.0 | 19.9 | 76% | 36.3 / 33.2 | 85.2 / 88.0 | 97.3 | | 2.73 | | LL | PL | PI | 0.0 | 62.5 | 37.5 | 32.8 | 4.7 | 1.7E-06 | 32 | 0.059 | | |
| CENTRAL | TI-B1 | ST | 45 | 46 | Alluvium Clayey Sand | coarse | 22 | 21.2 | | | | 106.0 | | | | | | | | | | | | | 0.058 | | | 34.4 |
| CENTRAL | TI-B10 | CA | 91 | 91.5 | Alluvium Clayey Sand | coarse | 18.6 | | | | | 105.6 | | 2.66 | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA | 41 | 41.5 | Alluvium Clayey Sand | coarse | 11.4 | 10.1 | | | | 87.1 | 88.1 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA | 66 | 66.5 | Alluvium Clayey Sand | coarse | 12.7 | 11.8 | | | | 100.7 | 101.5 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | CA | 81 | 81.5 | Alluvium Clayey Sand with Gravel | coarse | 11.0 | | | | | 107.6 | | 2.76 | | | | | 12.9 | 65.6 | 21.5 | 9.9 | 11.6 | | | | | |
| CENTRAL | TI-B10 | CA | 46 | 46.5 | Alluvium Silty Sand | coarse | 9.9 | | | | | 95.4 | | 2.74 | | | | | 0.0 | 65.8 | 34.2 | 23.4 | 10.8 | | | | | |
| CENTRAL | TI-B10 | ST | 55 | 56 | Alluvium Silty Sand | coarse | 14.1 | | | 25.7 / 24.8 | 98.0 / 99.9 | 100.8 | | | | | | | | | | | | 2.4E-05 | 72 | 0.139 | | |
| CENTRAL | TI-B10 | CA | 71 | 71.5 | Alluvium Silty Sand | coarse | 18.1 | | | | | 100.8 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | ST | 56 | 57 | Alluvium Silty Sand | coarse | 16.2 | | | 31.0 / 30.8 | 90.6 / 92.8 | 77.9 | | 2.64 | | NP | | | 0.0 | 60.4 | 39.6 | 31.9 | 7.7 | 5.6E-04 | 72 | 0.129 | | |
| CENTRAL | TI-B11 | CA | 66 | 66.5 | Alluvium Silty Sand | coarse | 14.2 | | | | | 96.2 | | | | | | | | | | | | | | | | |
| BORROW PIT 1 | TI-B8 | CA | 56 | 56.5 | Alluvium Silty Sand | coarse | 12.6 | | | | | 97.6 | | 2.70 | | NP | | | 0.0 | 57.0 | 43.0 | 30.9 | 12.1 | | | | | |
| CENTRAL | TI-B15 | CA (top) | 31 | 31.5 | Alluvium Silty Sand | coarse | 22.3 | 21.3 | | | | | | | | | | | 0.0 | 57.0 | 43.0 | 30.9 | 12.1 | | | | | |
| CENTRAL | TI-B15 | CA (bottom) | 31 | 31.5 | Alluvium Silty Sand | coarse | 17.1 | | | | | 101.8 | | 2.71 | | NP | | | 6.2 | 51.9 | 41.9 | 25.9 | 16.0 | | | | | |
| NORTH CELL | TI-B2 | CA | 15 | 15.5 | Alluvium Silty Sand | coarse | 6.9 | | | | | 90.4 | | 2.68 | | | | | | | | | | | | | | |
| NORTH CELL | TI-B2 | CA | 21 | 21.5 | Alluvium Silty Sand | coarse | 7.0 | | | | | 91.4 | | 2.74 | | | | | 0.0 | 82.9 | 17.1 | 11.5 | 5.6 | | | | | |
| CENTRAL | TI-B10 | CA | 66 | 66.5 | Alluvium Silty Sand / Sandy Silt | coarse | SM/ML | 13.8 | | | | 94.5 | | | | NP | | | 0.0 | 50.1 | 49.9 | 33.4 | 16.5 | | | | | |
| NORTH CELL | TI-B23 | ST | 26 | 27 | Alluvium Lean Clay | fine | CL | 21.6 | | | | 101.7 | | 2.73 | | 49 | 18 | 31 | 0.0 | 8.8 | 91.2 | 43.8 | 47.5 | | | 0.046 | | |
| DAM | TI-B3 | ST | 56 | 57 | Alluvium Lean Clay | fine | CL | 22.1 | 21.1 | | | 105.3 | 106.2 | 2.72 | | 43 | 14 | 29 | 0.0 | 11.7 | 88.3 | 48.4 | 39.9 | | | | -1.5 (7,204) | 22.2, 494 |
| CENTRAL | TI-B1 | CA | 41 | 41.5 | Alluvium Lean Clay with Sand | fine | CL | 26.7 | | | | 98.6 | | | | 31 | 15 | 16 | 0.0 | 18.2 | 81.8 | 54.7 | 27.1 | 1.2E-07 | 35 | | | |
| BORROW PIT 1 | TI-B8 | CA | 46 | 46.5 | Alluvium Lean Clay with Sand | fine | CL | 21.9 | | | | 95.2 | | 2.72 | | 30 | 16 | 14 | 0.0 | 27.9 | 72.1 | 55.6 | 16.5 | | | | | |
| NORTH CELL | TI-B2 | CA | 26 | 26.5 | Alluvium Lean Clay with Sand | fine | CL | 23.5 | | | | 93.2 | | | | 34 | 16 | 18 | 0.0 | 20.9 | 79.1 | 51.5 | 27.6 | | | | | |
| CENTRAL | TI-B11 | CA | 61 | 61.5 | Alluvium Sandy Clay | fine | | 16.0 | | | | 95.4 | | | | | | | 0.0 | 38.7 | 61.3 | 44.1 | 17.2 | | | | | |
| NORTH CELL | TI-B23 | ST | 17.25 | 17.5 | Alluvium Sandy Clay | fine | | 22.5 | | | | 101.9 | | 2.73 | | | | | 0.0 | 31.1 | 68.9 | 46.5 | 22.5 | | | | | |
| CENTRAL | TI-B15 | CA (top) | 46 | 46.5 | Alluvium Sandy Silt | fine | | 25.8 | 24.0 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA (bottom) | 46 | 46.5 | Alluvium Sandy Silt | fine | ML | 17.3 | | | | 99.3 | | 2.81 | | NP | | | 0.0 | 37.0 | 63.0 | 55.7 | 7.3 | | | | | |
| CENTRAL | TI-B15 | CA | 56 | 56.5 | Alluvium Silty Clay | fine | | 11.7 | 10.5 | | | 104.2 | 105.3 | | | | | | | | | | | | | | | |
| DAM | TI-B3 | CA | 61 | 61.5 | Alluvium Silty Clay | fine | | 25.8 | | | | 99.0 | | | | | | | 0.0 | 22.0 | 78.0 | 54.9 | 23.1 | | | | | |
| CENTRAL | TI-B10 | ST (top) | 10 | 11 | Coarse Tailings | | | 9.7 | 9.1 | | | 110 | 110.5 | 2.63 | 2.65 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | ST (bottom) | 10 | 11 | Coarse Tailings | Clayey Sand | | 9.0 | | | 20.7 / 21.5 | 102.6 / 101.2 | 96.8 | | | | | | 0.2 | 71.9 | 27.9 | 16.6 | 11.3 | 4.3E-04 | 34 | 0.094 | | |
| CENTRAL | TI-B10 | CC-AC ⁽⁴⁾ (top) | 12.5 | 14 | Coarse Tailings | | | 6.7 | 6.3 | | | | | 2.61 | 2.64 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CC-AC ⁽⁴⁾ (bot) | 12.5 | 14 | Coarse Tailings | Clayey Sand | | 7.5 | | | 31.3 / 31.4 | 85.0 / 85.0 | 99.1 | | | | | | 0.7 | 71.5 | 27.8 | 18.9 | 8.9 | 6.7E-05 | 36 | | | |
| CENTRAL | TI-B10 | CA | 15 | 15.5 | Coarse Tailings | | | 9.3 | | | | 103.0 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CA | 16 | 16.5 | Coarse Tailings | Silty Sand | SM | 6.5 | | | | 100.0 | | 2.65 | | NP | | | 2.4 | 82.3 | 15.3 | 10.2 | 5.1 | | | | | |
| CENTRAL | TI-B10 | ST | 32 | 32.5 | Coarse Tailings | | SM | 15.4 | | | | 100.1 | | 2.67 | | NP | | | 0.0 | 83.1 | 16.9 | 12.6 | 4.3 | | | | | |
| CENTRAL | TI-B1 | CA | 20.5 | 21 | Coarse Tailings | | | 6.1 | 5.7 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B1 | CA | 21 | 21.5 | Coarse Tailings | Poorly Graded Sand w/ Clay | | 7.5 | | | 21.9 / 19.8 | 96.5 / 99.6 | 105.5 | | | | | | 0.0 | 90.7 | 9.3 | 5.5 | 3.8 | 3.7E-04 | 18 | 0.024 | | |
| CENTRAL | TI-B1 | ST | 27 | 27.5 | Coarse Tailings | | SP | 4.0 | | | | 97.6 | | 2.67 | | NP | | | 0.0 | 92.7 | 7.3 | 5.2 | 2.1 | 2.9E-03 | 14 | | | 34.9 |
| CENTRAL | TI-B1 | CA | 30 | 30.5 | Coarse Tailings | | | 13.9 | 13.5 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B1 | CA | 30.5 | 31 | Coarse Tailings | | | 14.6 | | | 29.6 / 33.8 | 84.2 / 83.6 | 91.6 | | | | | | | | | | | 3.0E-07 | 25 | 0.092 | | |
| CENTRAL | TI-B1 | CA (top) | 31 | 31.5 | Coarse Tailings | | | 0.8 | 0.4 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CA | 25 | 25.5 | Coarse Tailings | | | 9.0 | 8.4 | | | 103.7 | 104.2 | 2.72 | 2.72 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CA ⁽⁵⁾ | 25.5 | 26 | Coarse Tailings | | | 6.2 | | | 25.7 | 94.6 | 99.6 | | | | | | 0.0 | 87.9 | 12.7 | 7.9 | 4.8 | 3.6E-04 | 46 | | | |
| CENTRAL | TI-B8 | CA ⁽⁵⁾ | 26 | 26.5 | Coarse Tailings | Silty Sand | SM | 16.8 | | | 27.0 | 94.8 | 91.7 | | | NP | | | 0.0 | 76.0 | 24.0 | 19.0 | 5.0 | | | | | |
| CENTRAL | TI-B8 | ST (top) | 35 | 36 | Coarse Tailings | | | 14.3 | 13.6 | | | 90.9 | 91.4 | 2.66 | 2.67 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST (bottom) | 35 | 36 | Coarse Tailings | | | 16.5 | | | 31.2 / 39.3 | 89.3 / 82.3 | 89.6 | | | | | | | | | | | 1.6E-05 | 43 | | | |
| CENTRAL | TI-B15 | CA | 6 | 6.5 | Coarse Tailings | | | 5.4 | | | | 101.1 | | | | | | | 0.0 | 87.5 | 12.5 | 9.8 | 2.7 | | | | | |
| CENTRAL | TI-B15 | CA | 11 | 11.5 | Coarse Tailings | | | 6.8 | | | | 93.8 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CC-AC | 13.5 | 14 | Coarse Tailings | | SM | 19.0 | 18.4 | | | | | 2.68 | | NP | | | 0.0 | 69.6 | 30.4 | 22.6 | 7.8 | | | | | |
| CENTRAL | TI-B15 | ST | 15.5 | 16 | Coarse Tailings | | SM | 14.2 | | | | 90.4 | | 2.66 | | NP | | | 0.0 | 54.9 | 15.1 | 10.1 | 5.0 | 8.3E-04 | 38 | 0.126 | | |
| CENTRAL | TI-B15 | CA | 21 | 21.5 | Coarse Tailings | | SM | 12.7 | | | | 99.8 | | 2.68 | | NP | | | 0.0 | 80.6 | 19.4 | 13.3 | 6.1 | | | | | |
| CENTRAL | TI-B15 | CC-AC | 28.5 | 29.5 | Coarse Tailings | | SM | 19.3 | | | | | | 2.66 | | NP | | | 0.0 | 65.4 | 34.6 | 24.4 | 10.2 | | | | | |
| NORTH CELL | TI-B23 | ST | 15.5 | 15.75 | Coarse Tailings | | | 20.7 | 19.6 | | | 87.7 | | 2.77 | | | | | 0.0 | 62.8 | 37.2 | 34.1 | 3.1 | | | | | |
| CENTRAL | TI-B10 | ST | 21.5 | 22.5 | Coarse/Fine Tailings | | CL | 28.1 | 26.7 | | | 91.9 | 92.9 | | | 43 | 19 | 24 | 0.0 | 43.0 | 57.0 | 51.4 | 5.6 | | | 0.111 | | |
| CENTRAL | TI-B1 | CC-AC | 32 | 33 | Coarse/Fine Tailings | | CL | 29.3 | 27.8 | | | | | | | 33 | 16 | 17 | 0.0 | 46.7 | 53.3 | 37.4 | 15.9 | | | | | |
| CENTRAL | TI-B10 | CA | 36 | 36.5 | Coarse/Fine Tailings | Clayey Sand / Sandy Clay | SC/CL | 33.9 | 32.2 | 94% | | 86.7 | 87.8 | 2.68 | 2.72 | 36 | 16 | 20 | 0.0 | 50.6 | 49.4 | 31.1 | 18.3 | | | | | |
| CENTRAL | TI-B8 | ST (bottom) | 41 | 42 | Coarse/Fine Tailings | Clayey Sand / Sandy Clay | SC/CL | 35.6 | 34.3 | | 33.1 / 31.6 | 88.7 / 90.7 | 82.8 | 83.6 | | 35 | 16 | 19 | 0.0 | 51.2 | 48.8 | 40.7 | 8.1 | 1.3E-07 | 53 | 0.262 | | |
| CENTRAL | TI-B8 | CC-AC (top) | 43.5 | 44.5 | Coarse/Fine Tailings | | | 31.2 | 29.3 | | | 91.0 | 92.3 | | | | | | | | | | | | | | | |
| DAM | TI-B3 | ST (top) | 35 | 36 | Dam Clayey Sand (dam) | | | 10.5 | 10.2 | | | | | | | | | | | | | | | | | | | |
| DAM | TI-B3 | ST (bottom) | 35 | 36 | Dam Clayey Sand (dam) | | SC | 14.7 | | | | 102.2 | | 2.67 | | 23 | 14 | 9 | 2.1 | 50.2 | 47.7 | 30.9 | 16.8 | | | | -0.7 (4,608) | 33.7, 135 |
| DAM | TI-B3 | ST | 21 | 22 | Dam Sandy Clay (dam) | | CL | 16.0 | | | | 111.1 | | | | 30 | 12 | 18 | 0.0 | 32.8 | 67.2 | 41.7 | 25.5 | | | | -0.03 (2,709) | 32.2, 195 |
| DAM | TI-B3 | CA | 26 | 26.5 | Dam Sandy Clay (dam) | | | 12.0 | | | | 106.8 | | | | 25 | 13 | 12 | | | | | | | | | | |
| DAM | TI-B3 | CA | 31 | 31.5 | Dam Sandy Clay (dam) | | | 16.1 | | | | 108.4 | | | | | | | | | | | | | | | | |
| DAM | TI-B3 | CA | 11 | 11.5 | Dam Silty Sand (dam) | | | 5.1 | | | | 108.4 | | 2.64 | | | | | 5.4 | 74.7 | 19.9 | 13.5 | 6.4 | | | | | |
| DAM | TI-B3 | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment (continued)

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | saturation (%) | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | Atterberg limits (%) | | | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) |
|--------------|--------|----------------------------|-----------------------------|-------|-------------------------------------|--------------------|---------------------|---------------------------------|--------------------------------|----------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----------------------|----|----|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|
| CENTRAL | TI-B10 | CA | 26 | 26.5 | Fine Tailings | Fat Clay | CH | 60.4 | 57.4 | | | | 63.1 | 64.3 | 2.71 | 2.80 | 74 | 27 | 47 | 0.0 | 10.0 | 90.0 | 82.6 | 7.4 | | | | | |
| CENTRAL | TI-B10 | ST | 30.3 | 30.7 | Fine Tailings | | CH | 47.7 | 45.3 | 92% | | | 72.2 | 73.4 | 2.71 | 2.78 | 57 | 22 | 35 | 0.0 | 24.3 | 75.7 | 68.4 | 7.3 | | | | | |
| CENTRAL | TI-B10 | ST (top) | 40 | 41 | Fine Tailings | Fat Clay with Sand | | 47.3 | 45.7 | | | | 70.5 | 73.7 | 2.54 | 2.56 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | ST (bottom) | 40 | 41 | Fine Tailings | Fat Clay with Sand | CH | 49.7 | 47.2 | | 47.7 / 55.7 | 75.3 / 67.9 | 73.3 | 74.5 | | | 61 | 21 | 40 | 0.0 | 20.7 | 79.3 | 46.5 | 32.9 | 2.9E-08 | 58 | 0.315 | | |
| CENTRAL | TI-B11 | ST | 51.5 | 52.5 | Fine Tailings | | CH | 63.0 | 59.9 | 95% | | | 62.5 | 63.7 | 2.75 | 2.84 | 91 | 30 | 61 | 0.0 | 2.7 | 97.3 | 90 | 7.3 | 3.1E-08 | 67 | 0.482 | | |
| CENTRAL | TI-B1 | CA (bottom) | 31 | 31.5 | Fine Tailings | | CL | | 41.6 | 94% | | | | 76.5 | 2.68 | 2.69 | 44 | 17 | 27 | 0.0 | 30.9 | 69.1 | 54.6 | 14.5 | | | | | 33.3 |
| CENTRAL | TI-B10 | CA | 35 | 35.5 | Fine Tailings | | | 50.2 | 47.7 | | | | 71.3 | 72.5 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CA | 35.5 | 36 | Fine Tailings | | | 54.2 | 51.4 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | CA | 45.5 | 46 | Fine Tailings | | | 117.2 | 88.7 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST | 30 | 31 | Fine Tailings | | CH | 65.1 | 61.8 | | | | 61.5 | 62.7 | | | 74 | 25 | 49 | 0.0 | 9.2 | 90.8 | 81.2 | 9.6 | | | 0.426 | | |
| CENTRAL | TI-B8 | ST | 31 | 31.5 | Fine Tailings | | | 44.3 | 41.4 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST (top) | 41 | 42 | Fine Tailings | | | 41.8 | 39.7 | 100% | | | 79.2 | 80.4 | 2.60 | 2.63 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CC-AC ⁽⁶⁾ (bot) | 43.5 | 44.5 | Fine Tailings | | | 45.6 | 43.3 | 96% | 47.9 / 49.0 | 74.4 / 73.6 | 73.6 | 74.8 | | | | | | 0.0 | 14.5 | 85.5 | 74.7 | 10.8 | 3.0E-08 | 61 | | | |
| BORROW PIT 1 | TI-B8 | CC-AC | 44.5 | 45 | Fine Tailings | | | | | | | | | | 2.59 | 2.60 | | | | | | | | | | | | | |
| NORTH CELL | TI-B2 | CC-AC | 13.5 | 14.5 | Fine Tailings | Sandy Clay | | 41.7 | 39.6 | | | | | | | | | | | 0.0 | 23.1 | 76.9 | 49.2 | 27.7 | | | | | |
| CENTRAL | TI-B10 | CC | 106.9 | 107.3 | Sandstone | | | 14.2 | | | | | 109.1 | | | | | | | | | | | 1.4E-07 | 115 | | | | |
| CENTRAL | TI-B11 | CA | 100 | 100.2 | Sandstone | | | 21.1 | | | | | 103.9 | | | | | | | | | | | 1.3E-05 | 112 | | | | |
| NORTH CELL | TI-B23 | CA | 45.2 | 45.7 | Sandstone | | | 13.8 | | | | | 108.7 | | | | | | | | | | | 2.4E-07 | 43 | | | | |
| NORTH CELL | TI-B2 | BULK | 38.4 | 38.7 | Sandstone | | | 13.5 | | | | | X | | | | | | | | | | | | | | | | |
| BORROW PIT 1 | TI-B8 | BULK | 63.5 | 64 | Shale | | | X | | | | | X | | | | | | | | | | | X | X | | | | |
| NORTH CELL | TI-B23 | CA ⁽⁸⁾ | 65.5 | 66 | Shale | | | 10.2 | | | | | 103.0 | | | | | | | | | | | 9.7E-08 | 62 | | | | |

Notes: 1. Material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available.
2. SWCC tests conducted with pairs of specimens for each test.
3. Flexible wall permeameter tests conducted at confining pressures representing confining stresses for the proposed design fill. Confining stresses were estimated as the existing overburden stress on the specimens (depth times total unit weight of material above) plus the maximum anticipated fill height for the location times the estimated unit weight of fill.
4. Specimen remolded to the in-situ water content and density of the Shelby tube sample from 10-12.5 for the SWCC.
5. Remolded SWCC and permeability tests conducted on a 50-50 mixture of the materials from these two specimens, remolded to the average measured density of the two CA samples.
6. SWCC specimen remolded to the in-situ water content and density of the Shelby tube sample from 41-42 feet.
7. Compression indices estimated using the maximum anticipated loading during fill placement and the range of loading during testing. Initial void ratios are calculated using the average specific gravity for all samples of 2.70.
8. Shale sample had multiple horizontal fractures and was likely disturbed during sampling.

9. Sample Types: CC = continuous core, CC-AC = continuous core in acrylic liner, top/bottom indicates the specimen was taken from the top or bottom of the sample interval
10. Values in italics were calculated based on the relationship $(WC60=0.951*(WC110)-.0611)$ between the water content results measured for 15 tailings samples at the two oven temperatures.
11. Shaded cells are alluvium.
12. Consolidated undrained (CU) triaxial shear, staged loading of one specimen with pore pressure measurements

ST = 3" diam. Shelby tube, CA = California sample
R = remolded, nc = Cc not calculated, because fill will not be placed in this location
X = testing not possible due to sample disturbance
LL = liquid limit, PL = plastic limit, PI = plasticity index

Table 3-5 Summary of Geotechnical Laboratory Data - Borrow Areas

| Area | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (ft) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass, %) | Dry Density (pcf) | Porosity | Specific Gravity | Standard Proctor (max. dd@opt. w.c.), (pcf @ %) | Atterberg Limits (%) ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | Pinhole Dispersion ^(5,6) | Remolded Saturated Hydraulic Conductivity ⁽⁷⁾ (cm/sec) | | | SWCC: -5 bar Water Content (by mass, %) ⁽⁸⁾ | | SWCC: Saturated Water Content (by mass, %) ⁽⁸⁾ | |
|--------------|------------|----------------------------|----------------------------|--------------------|-------------------------------------|---------------------|------------------------------------|----------------------------|-------------------|----------|------------------|---|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|-------------------------------------|---|---------|---------|--|-------------|---|--|
| | | | | | | | | | | | | | LL | PL | PI | | | | | | | 80% | 85% | 90% | | | | |
| West Borrow | WB-B1-01A | CA | 3.0 | 3.5 | Clayey Sand | | | 3.8 | 88.8 | 46.7 | 2.67 | | | | | | | | | | | | | | | | | |
| | WB-B1-03A | CA | 11.0 | 11.5 | Clayey Sand | SC | Sandy Loam | 6.4 | 111.0 | 33.3 | 2.67 | | 28 | 18 | 10 | 2.8 | 48.6 | 48.6 | 32.8 | 15.8 | | | | | | | | |
| | WB-B1-06 | Bulk | 5.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.64 | 112.5 @ 13.7 | 26 | 20 | 6 | 0.8 | 52.3 | 46.9 | 31.0 | 15.9 | ND3 | 7.2E-04 | 5.8E-04 | 2.1E-04 | 6.6 / 6.2 | 31.7 / 32.4 | | |
| | WB-B2-02A | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 5.6 | 87.1 | 47.8 | 2.67 | | | | | 8.6 | 53.5 | 37.9 | 23.8 | 14.1 | | | | | | | | |
| | WB-B2-05 | Bulk | 10.0 | 20.0 | Clayey Sand | SC | Sandy Loam | | | | | | 26 | 17 | 9 | 9.9 | 46.3 | 43.8 | 27.7 | 16.1 | ND3 | 8.5E-05 | 1.2E-04 | 6.4E-05 | 6.4 / 6.7 | 30.9 / 33.7 | | |
| | WB-B5-001B | CA | 3.0 | 3.5 | Clayey Sand | | | 3.7 | 92.5 | 44.3 | 2.66 | | | | | | | | | | | | | | | | | |
| | WB-B5-002A | CA | 6.0 | 6.5 | Silty, Clayey Sand | SC-SM | Sandy Loam | 5.1 | 86.9 | 47.7 | 2.66 | | 24 | 17 | 7 | 0.0 | 56.3 | 43.7 | 27.8 | 15.9 | | | | | | | | |
| WB-B5-005 | Bulk | 0.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | | 117.3 @ 12.7 | | | | | 0.0 | 61.6 | 38.4 | 22.8 | 15.6 | | | | | | | | |
| East Borrow | EB-B2-001A | CA | 3.0 | 3.5 | Weath. Sandstone | | | 5.8 | 107.1 | 35.8 | 2.67 | | | | | | | | | | | | | | | | | |
| | EB-B3-003B | CA | 10.5 | 11.0 | Sandy Lean Clay | CL | Sandy Loam | 6.0 | 83.1 | 50.7 | 2.70 | | 26 | 15 | 11 | 0.0 | 46.3 | 53.7 | 34.9 | 18.8 | | | | | | | | |
| | EB-B4-02A | CA | 6.0 | 6.5 | Sandy Lean Clay | CL | Sandy Loam | 5.4 | 80.7 | 51.2 | 2.65 | | | | | 0.0 | 48.5 | 51.5 | 33.9 | 17.6 | | | | | | | | |
| | EB-B4-06 | Bulk | 10.0 | 20.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.67 | 117.1 @ 12.9 | 23 | 17 | 6 | 0.0 | 50.5 | 49.5 | 32.0 | 17.5 | ND3 | 8.7E-04 | 9.0E-04 | 4.4E-04 | 4.6 / 4.2 | 30.8 / 29.8 | | |
| | EB-B5-02B | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 6.7 | 93.8 | 44.4 | 2.71 | | 27 | 15 | 12 | 8.8 | 45.7 | 45.5 | 28.8 | 16.7 | | | | | | | | |
| | EB-B6-01B | CA | 3.0 | 3.5 | Sandy Clay | | | 7.6 | 91.2 | 46.1 | 2.71 | | | | | | | | | | | | | | | | | |
| | EB-B6-03 | Bulk | 0.0 | 10.0 | Lean Clay with Sand | CL | Clay Loam | | | | | 114.8 @ 14.1 | | | | | 0.0 | 26.6 | 73.4 | 44.3 | 29.1 | ND3 | 2.3E-04 | 3.6E-05 | 2.9E-05 | 9.4 / 9.3 | 32.8 / 32.2 | |
| EB-B6-04A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 8.6 | 95.2 | 43.3 | 2.69 | | | 31 | 13 | 18 | 0.0 | 31.1 | 68.9 | 43.8 | 25.1 | | | | | | | | |
| South Borrow | SB-B1-01A | CA | 3.5 | 4.0 | Sandy Lean Clay | CL | Sandy Loam | 7.1 | 91.4 | 49.3 | 2.89 | | | | | 0.0 | 43.1 | 56.9 | 39.2 | 17.7 | | | | | | | | |
| | SB-B1-03A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 6.6 | 82.6 | 50.7 | 2.69 | | 31 | 15 | 16 | 0.0 | 46.7 | 53.3 | 32.9 | 20.4 | | | | | | | | |
| | SB-B1-04 | Bulk | 0.0 | 25.0 | Sandy Lean Clay | CL | Sandy Clay Loam | | | | 2.70 | 115.5 @ 14.2 | 33 | 14 | 19 | 0.0 | 42.6 | 57.4 | 30.7 | 26.7 | ND1 | 2.3E-04 | 5.7E-05 | 1.4E-04 | 6.4 / 5.9 | 31.9 / 30.3 | | |
| | SB-B2-02B | CA | 5.5 | 6.0 | Sandy Lean Clay | CL | Loam | 7.7 | 80.1 | 52.6 | 2.70 | | 36 | 15 | 21 | 0.0 | 29.8 | 70.2 | 45.4 | 24.8 | | | | | | | | |
| | SB-B3-02A | CA | 6.0 | 6.5 | Lean Clay with Sand | CL | Clay Loam | 10.2 | 84.3 | 49.7 | 2.69 | | 40 | 17 | 23 | 0.0 | 21.6 | 78.4 | 46.2 | 32.2 | | | | | | | | |
| SB-B4-01 | Bulk | 0.0 | 15.0 | Sandy Lean Clay | CL | Sandy Clay Loam | 7.1 | | | | 2.67 | 114.1 @ 14.4 | 33 | 15 | 18 | 0.8 | 39.6 | 59.6 | 35.7 | 23.9 | ND3 | 3.4E-04 | 2.0E-04 | 7.4E-05 | 9.1 / 8.6 | 29.6 / 33.5 | | |
| North Borrow | NB-B1-03B | CA | 10.5 | 11.0 | Silty Sand | SM | Sandy Loam | 5.4 | 84.4 | 49.5 | 2.68 | | 25 | 22 | 3 | 0.0 | 55.6 | 44.4 | 30.3 | 14.1 | | | | | | | | |
| | NB-B2-01B | CA | 3.0 | 3.5 | Silty Sand | SM | Sandy Loam | 4.9 | 81.9 | 50.3 | 2.64 | | 27 | 23 | 4 | 0.0 | 51.2 | 48.8 | 33.9 | 15.0 | | | | | | | | |
| | NB-B2-04 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | | | | | 113.9 @ 14.5 | 26 | 19 | 7 | 0.0 | 49.0 | 51.0 | 32.5 | 18.5 | ND3 | 4.0E-04 | 2.7E-04 | 7.5E-05 | 4.9 / 4.7 | 29.5 / 29.9 | | |
| Dilco Hill | DH-B1-01B | CA | 3.0 | 3.5 | Silty Sand | | | 3.5 | 88.8 | 46.6 | 2.66 | | | | | | | | | | | | | | | | | |
| | DH-B1-03 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | 5.4 | | | 2.67 | 117.5 @ 13.8 | 25 | 19 | 6 | 2.0 | 47.4 | 50.6 | 35.0 | 15.6 | ND4 | 6.3E-04 | 7.1E-04 | 2.5E-04 | 4.2 / 4.1 | 39.6 / 35.0 | | |
| | DH-B1-10 | Bulk | 35.0 | 45.0 | Lean Clay with Sand | CL | Loam | 10.3 | | | 2.38 | | | | | 1.5 | 20.9 | 77.6 | 60.9 | 16.7 | ND3 | 1.6E-04 | 2.5E-05 | 3.2E-06 | 5.8 / 6.0 | 25.7 / 24.5 | | |
| | DH-B2-03 | CA | 15.0 | 15.5 | Silty Clay with Sand | CL-ML | Sandy Loam | 10.5 | 96.7 | 39.2 | 2.55 | | 29 | 24 | 5 | 0.0 | 27.7 | 72.3 | 66.9 | 5.4 | | | | | | | | |
| | DH-B3-05 | Bulk | 20.0 | 30.0 | Sandy Lean Clay | CL | Loam | 7.3 | | | 2.66 | 116.3 @ 13.0 | 29 | 18 | 11 | 2.5 | 34.6 | 62.9 | 45.5 | 17.4 | | | | | | | | |

Notes: 1. Sample Types: CA = California sample, Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. LL = liquid limit, PL = plastic limit, PI = plasticity index
5. With the exception of DH-B1-03, which was tested at a density based on the natural in-situ density measured from the CA samples, specimens were remolded to approximately 85% of standard Proctor density and between the estimated natural and optimum water contents for the soil.
6. ND1 = nondispersive clay with very slight to no colloidal erosion under 15-inch or 40-inch head; ND4, ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head (ASTM test method A)
7. Specimens remolded to approximately 80%, 85%, and 90% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil.
8. Specimens remolded to approximately 85% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil. SWCC tests performed with pairs of speciments for each test.

Table 3-6 Summary of Geotechnical Laboratory Data - Site Stockpiles

| Area | Sample | Sample Type ⁽¹⁾ | Material Description | USCS ⁽²⁾ | Specific Gravity | Atterberg Limits (%) ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | L.A. Abrasion (% loss) ⁽⁵⁾ | Sodium Sulfate Soundness (% loss) ⁽⁶⁾ | Absorption (%) ⁽⁷⁾ | Unconfined Compressive Strength (psi) ⁽⁸⁾ | Splitting Tensile Strength (psi) ⁽⁸⁾ |
|------------|--------------|----------------------------|------------------------------|---------------------|---------------------|--|----|----|---------------|-------------|------------------------------------|--|---|----------------------------------|--|--|
| | | | | | | LL | PL | PI | | | | | | | | |
| Stockpiles | Topsoil-01 | Bulk | Sandy Clay | CL | 2.68 | 33 | 10 | 23 | 2.6 | 32.4 | 65.0 | | | | | |
| | Topsoil-02 | Bulk | Sandy Clay | CL | 2.71 | 39 | 12 | 27 | 0.5 | 26.8 | 72.7 | | | | | |
| | TI-SP1-01 | Bulk | Crusher Fines | | | | | | 1.9 | 80.8 | 17.3 | | | | | |
| | TI-SP2-01A | Bulk | Erosion Protection Gravel | | 2.78 ⁽³⁾ | | | | 93.0 | 6.3 | 0.7 | 5.7 | 8.26 | 1.868 | | |
| | TI-SP2-01C | Bulk | Erosion Protection Gravel | | | | | | 83.3 | 4.9 | 11.8 | | | | | |
| | TI-SP3-01A | Bulk | Road Base (gravel with sand) | | | | | | 67.4 | 24.6 | 8.0 | | | | | |
| | TI-SP4-01A | Bulk | Erosion Protection Gravel | | 2.75 ⁽³⁾ | | | | 98.0 | 1.2 | 0.8 | 6.1 | 10.47 | 2.091 | | |
| | TI-SP6 (56A) | Bulk | 9-inch riprap | | | | | | | | | | | | 20,780 and 23,630 | 1,320 and 1,400 |
| | TI-SP6 (56B) | Bulk | 9-inch riprap | | | | | | | | | | | | 19,100 and 14,440 | 1,530 and 1,720 |

Notes: 1. Bulk = bucket/grab sample

2. USCS = Unified Soil Classification System, material descriptions are based on field observations, and refined with laboratory data, if available.

USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay

3. Average of three bulk saturated surface dry (SSD) specific gravity results for the rock samples (ASTM C127)

4. LL = liquid limit, PL = plastic limit, PI = plasticity index

5. L.A. Abrasion results are percent loss, by mass, for 100 revolutions.

6. Weighted percentage loss for 0.75 to 1.5-inch size range

7. Average of three absorption results

8. Specimens were collected from the 9-inch stockpile and cored for strength testing.

Table 3-6 Geotechnical Test Results

| Sample ID ¹ | Sample Location | Sample Type | Sample Depth Interval | | Gravimetric Water content | Dry Density | Specific gravity | Standard Proctor | |
|------------------------|-----------------|-----------------|-----------------------|-----------------|---------------------------|----------------|----------------------|-------------------------------------|---------------------------|
| | | Units: | top (ft bgs) | bottom (ft bgs) | (% by mass) | (pcf) | (g/cm ³) | max. dry density (pcf) ³ | optimum water content (%) |
| NECR1-CC01 | NECR-1 | Bulk | 10 | 20 | | | 2.68 | 120.7 | 11.9 |
| NECR1-CC17 | | CA ² | 5.5 | 6 | 4.9 | 92.3 | | | |
| NECR1-CC17 | | CA | 10.5 | 11 | 6.2 | 96.5 | | | |
| NECR1-CC17 | | CA | 15.5 | 16 | 2 | 106.7 | | | |
| NECR1-CC17 | | CA | 20.5 | 21 | 19.1 | 95.8 | | | |
| NECR1-CC17 | | Bulk | 0 | 10 | | | | 120.3 | 11.3 |
| NECR1-CC17 | NECR-2 | Bulk | 10 | 20 | | | | 125.1 | 10 |
| NECR2-CC05 | | Bulk | 0 | 10 | | | | 118.8 | 11.9 |
| NECR2-CC07 | | Bulk | 0 | 10 | | | 2.71 | 117.8 | 11.6 |
| NECR2-CC05 | | CA | 2.5 | 3 | 8.1 | 93.7 | | | |
| NECR2-CC05 | | CA | 5 | 5.5 | 10 | D ³ | | | |
| NECR2-CC06 | | CA | 3.5 | 4 | 4.7 | 101.1 | | | |
| NECR2-CC07 | | CA | 6 | 6.5 | 2.7 | 101 | | | |
| NECR2-CC07 | | CA | 5.5 | 6 | 4.5 | 101.3 | | | |
| NECR2-CC07 | | CA | 10 | 10.5 | 4.1 | 97.1 | | | |
| NECR2-CC01 | | CA | 5.5 | 6 | 7.4 | 99.1 | | | |
| NECR2-CC06 | NECR-2 Drainage | CA | 3 | 3.5 | 5 | 103.4 | | | |
| N2D-CC01 | | Bulk | 0 | 10 | | | | 115.6 | 13.4 |
| N2D-CC01 | | CA | 3.5 | 4 | 8.6 | 91.2 | | | |
| N2D-CC01 | | CA | 6 | 6.5 | 4.7 | 87.2 | | | |
| N2D-CC01 | NEMSA | CA | 11 | 11.5 | 4 | 91.8 | | | |
| NMSA-CC02 | | CA | 3 | 3.5 | 8.1 | 110.6 | | | |
| NMSA-CC02 | | CA | 6 | 6.5 | 20 | 97.5 | | | |
| NMSA-CC02 | | CA | 10.5 | 11 | 15 | 86.6 | | | |
| NMSA-CC04 | Pond 2 | Bulk | 0 | 15 | | | 2.66 | 125.2 | 9.8 |
| P2-CC04 | | Bulk | 0 | 3 | | | 2.66 | 102.0 | 20.6 |
| P3-CC07 | Pond 3 | Bulk | 0 | 5 | | | 2.63 | 109.7 | 13.7 |
| SF2-CC01 | Sandfill 2 | Bulk | 0 | 10 | | | 2.65 | 121.5 | 10.5 |
| SF3-CC01 | Sandfill 3 | Bulk | 0 | 10 | | | 2.68 | 121.7 | 11.1 |
| SF3-CC01 | | CA | 3.5 | 4 | 17 | 99.3 | | | |
| SF3-CC01 | | CA | 6 | 6.5 | 10.5 | 96.4 | | | |
| SF3-CC01 | | CA | 11 | 11.5 | 8.2 | 83.5 | | | |
| SP-CC13 | Sediment Pad | CA | 5.5 | 6 | 10.2 | 101.4 | | | |
| SP-CC13 | | CA | 11 | 11.5 | 3.5 | 100.8 | | | |
| SP-CC13 | | CA | 15.5 | 16 | 6.9 | 97.5 | | | |
| SP-CC13 | | Bulk | 0 | 15 | | | 2.62 | 120.6 | 11.5 |

Notes:

pcf=pounds per cubic foot

1. Samples collected October-December 2013 during the Pre-Design Studies
2. CA = 2-inch diameter California sample, Bulk = 5-gallon bucket sample
3. Maximum dry density listed includes rock correction
4. D = Disturbed, moisture content only

ATTACHMENT B

RECORDED WATER LEVELS AT THE CHURCH ROCK SITE (CHESTER ENGINEERS, 2016)

| Well ID | Measurement Date | Measurement Time | Historical Reference Elev | Water Level Depth | Water Level Elev |
|---------|------------------|------------------|---------------------------|-------------------|------------------|
| 0509 D | 1/4/2016 | 8:37 | 6949.44 | 82.89 | 6866.55 |
| EPA 23 | 1/4/2016 | 9:30 | 6926.31 | 59.52 | 6866.79 |
| GW 1 | 1/4/2016 | 14:20 | 6916.46 | 65.01 | 6851.45 |
| GW 2 | 7/6/2015 | 14:25 | 6912.88 | 58.96 | 6853.92 |
| GW 3 | 7/7/2015 | 10:50 | 6910.04 | 56 | 6854.04 |
| 632 | 1/4/2016 | 12:35 | 6903.49 | 48.05 | 6855.44 |
| EPA 25 | 1/5/2016 | 10:35 | 6903.38 | 56.62 | 6846.76 |
| EPA 27 | 1/12/1999 | | 6910.95 | 55.45 | 6855.5 |
| EPA 28 | 1/4/2016 | 15:20 | 6917.86 | 65.83 | 6852.03 |
| 624 | 1/4/2016 | 16:35 | 6898.57 | 53.61 | 6844.96 |

Note: Water levels provided by email from Chester Engineers, on April 20, 2016.

ATTACHMENT C

TOTAL STRESS FRICTION ANGLE SPREADSHEET CALCULATIONS

Fine Tailings - Total Stress Internal Friction Angle Calculation

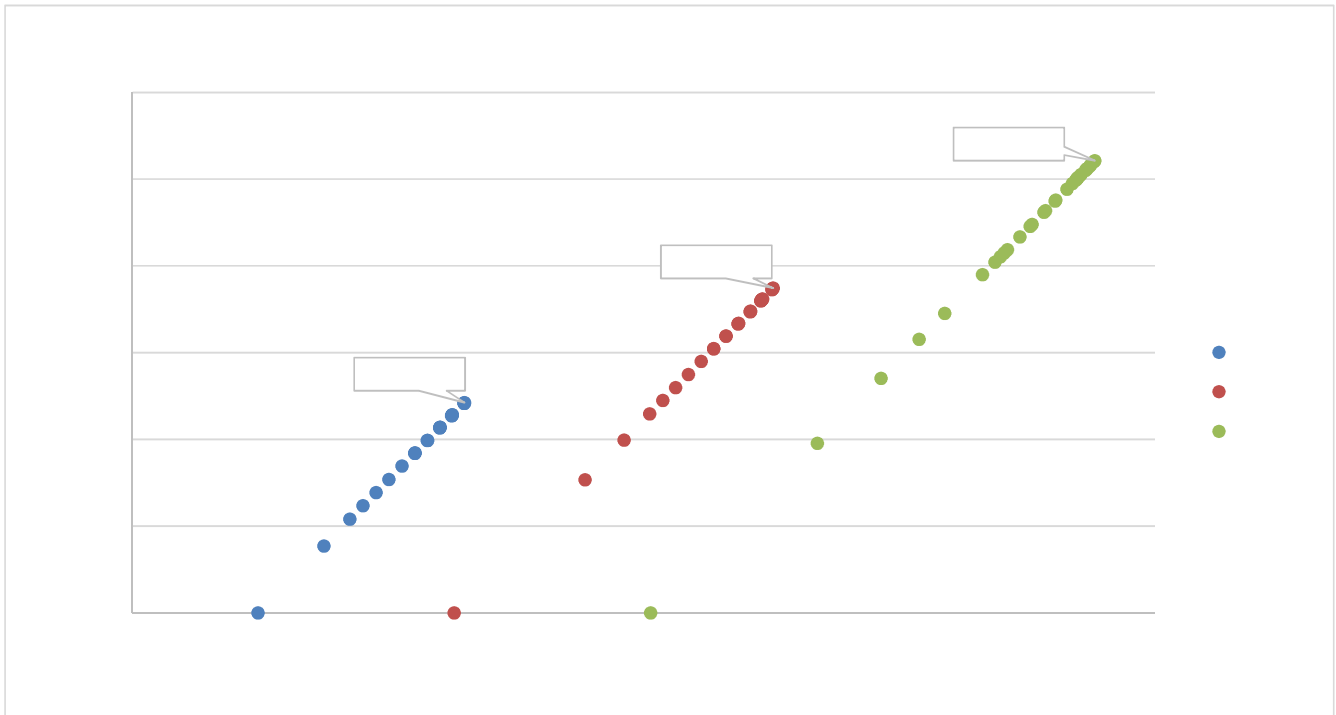
By: S. Downey
Date: 7/19/2017

Purpose:

Resolve the total stress internal friction angle based on laboratory results obtained from the PDS investigation (MWH, 2014). Two approaches were used to calculate the total stress friction angle. The first approach calculates the friction angle from each confining stress, and then averages the three friction angles. The second approach plots the peak values from each confining stress and finds the line of best fit with a y-intercept of 0 or greater. The equation from the line of best fit is then used to determine the total stress friction angle.

Approach 1:

- Found peak vertical stress values associated with peak confining stresses
- Used Lambe & Whitman equation with peak values of each sample
- Found the average of the three Lambe & Whitman values



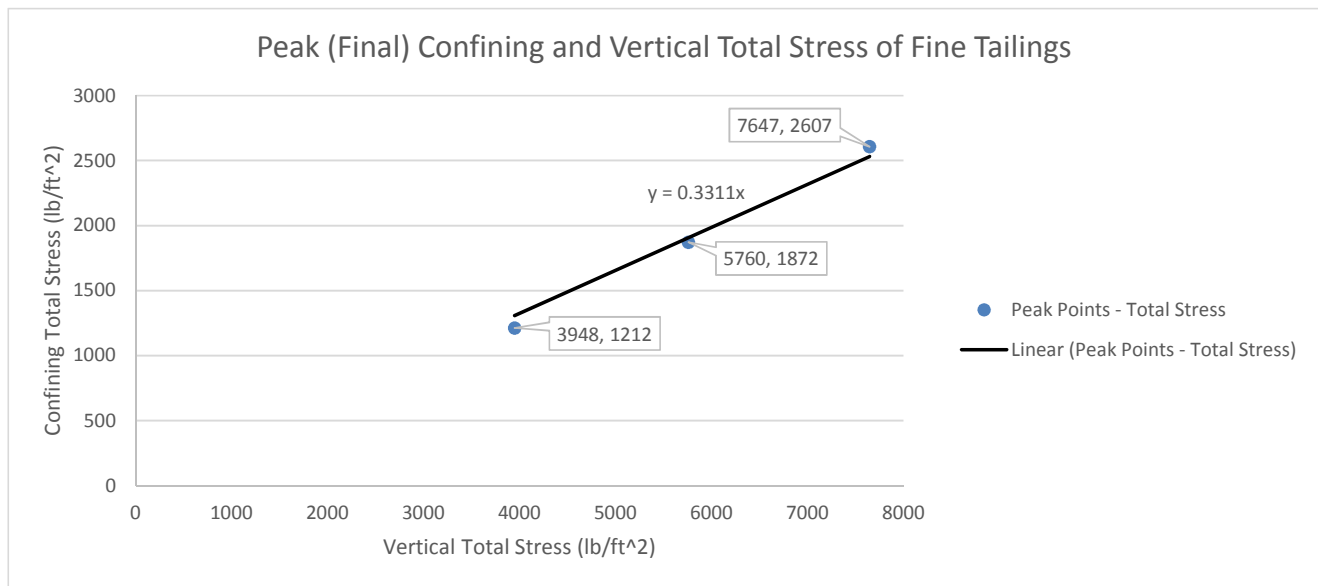
$$\sin \phi = \frac{q}{p} \quad (\text{Lambe and Whitman, 1969, pg. 141})$$

; q = peak (final) confining stress
p = peak (final) vertical stress
 ϕ = total stress internal friction angle

| Sample | p (lb/ft ²) | q (lb/ft ²) | ϕ (deg.) |
|----------------------|-------------------------|-------------------------|---------------|
| A | 3948 | 1212 | 18 |
| B | 5760 | 1872 | 19 |
| C | 7647 | 2607 | 20 |
| Avg. ϕ (deg.) = | | | 19 |

Approach 2:

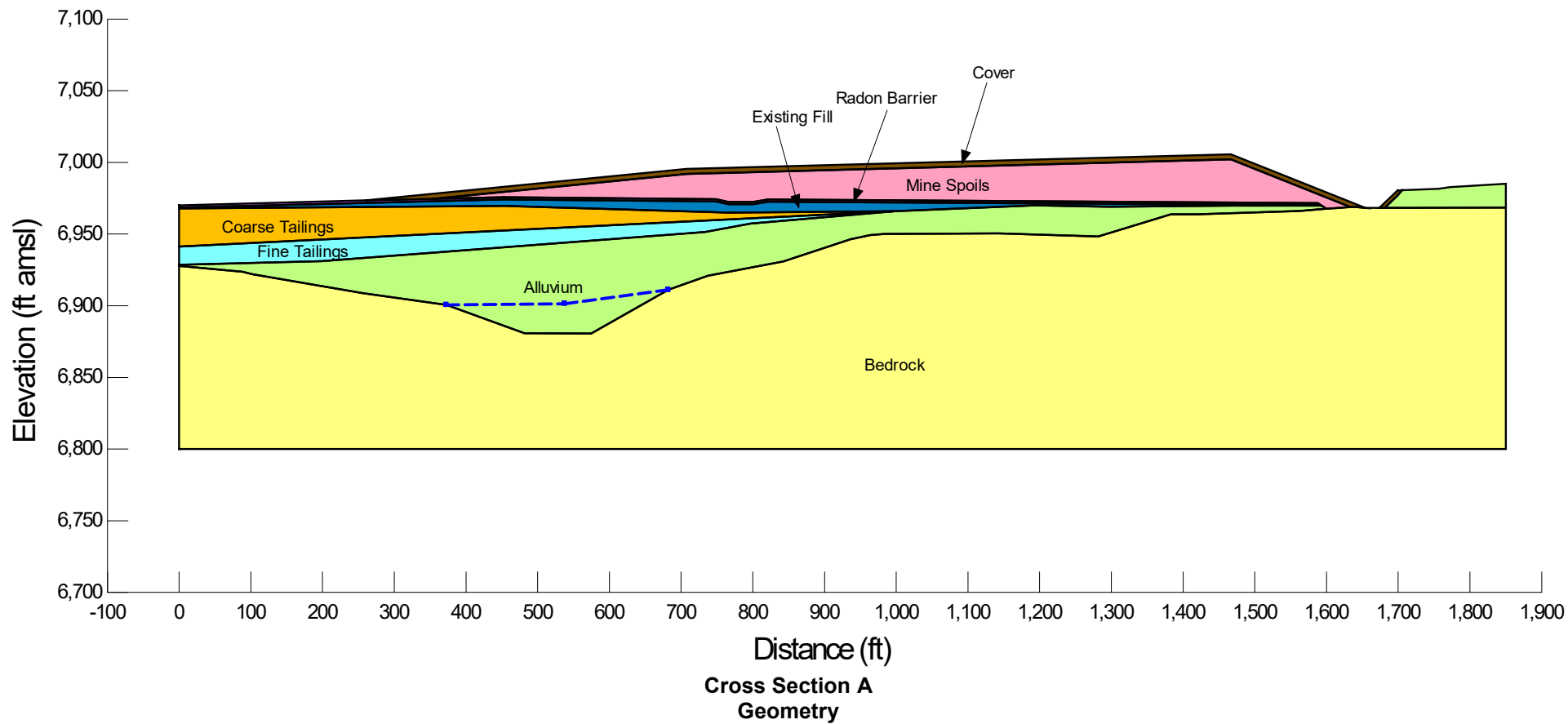
- Found peak vertical stress values associated with peak confining stresses
- Plotted peak values of each sample
- Established a best fit line with a conservative y-intercept of 0
- Used slope value from the best fit line equation in Lambe & Whitman equation



$\sin \phi = \tan \alpha$ (Lambe and Whitman, 1969, pg. 141) ; $\tan \alpha = \text{slope of best fit line}$

| Best Fit Line Equation | Best Fit Line Slope | ϕ (deg.) |
|------------------------|---------------------|---------------|
| 0.3311x | 0.3311 | 19 |

ATTACHMENT D
SLOPE/W OUTPUT FILES



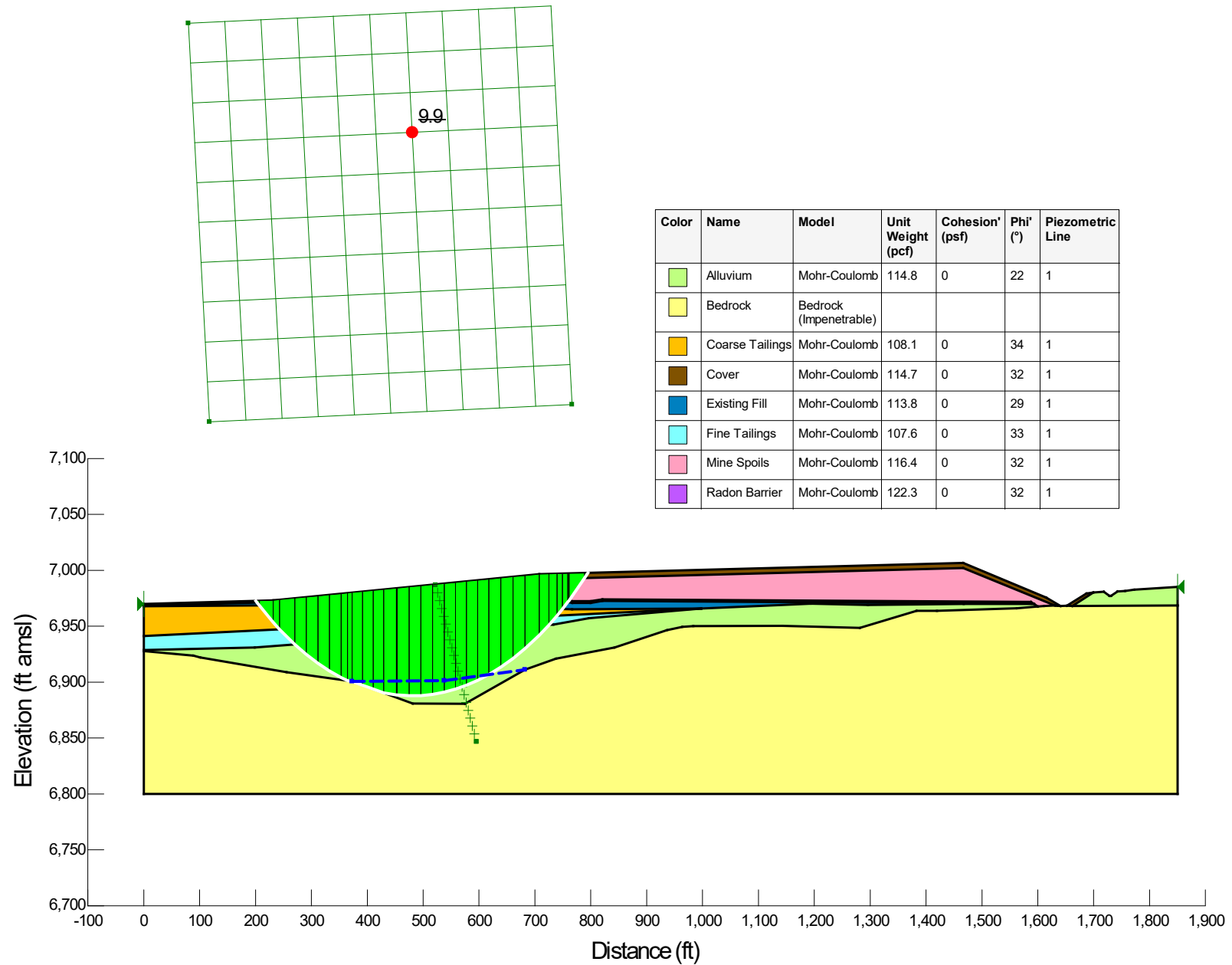
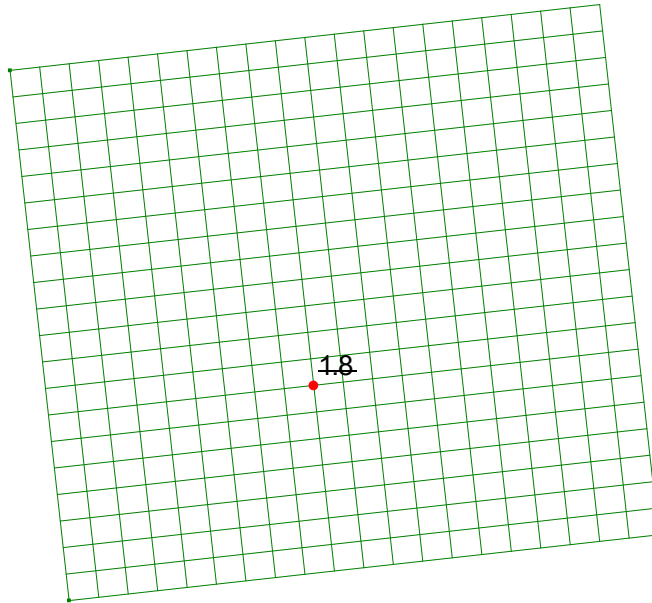
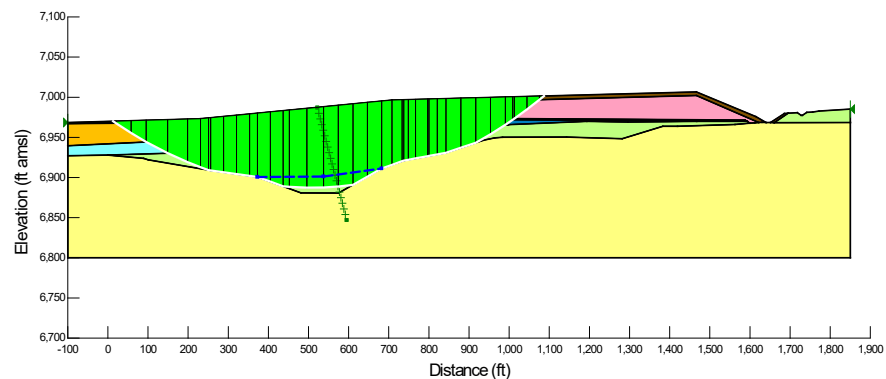










Figure D.1 - Cross Section A
Static Analysis – Southwest Slope



| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



**Figure D. 2 – Cross Section A
Pseudo-Static Analysis – Southwest Slope**

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|---|-----------------|------------------------|-------------------|-----------------|----------|------------------|
|  | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
|  | Bedrock | Bedrock (Impenetrable) | | | | |
|  | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
|  | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
|  | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
|  | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 1 |
|  | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
|  | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

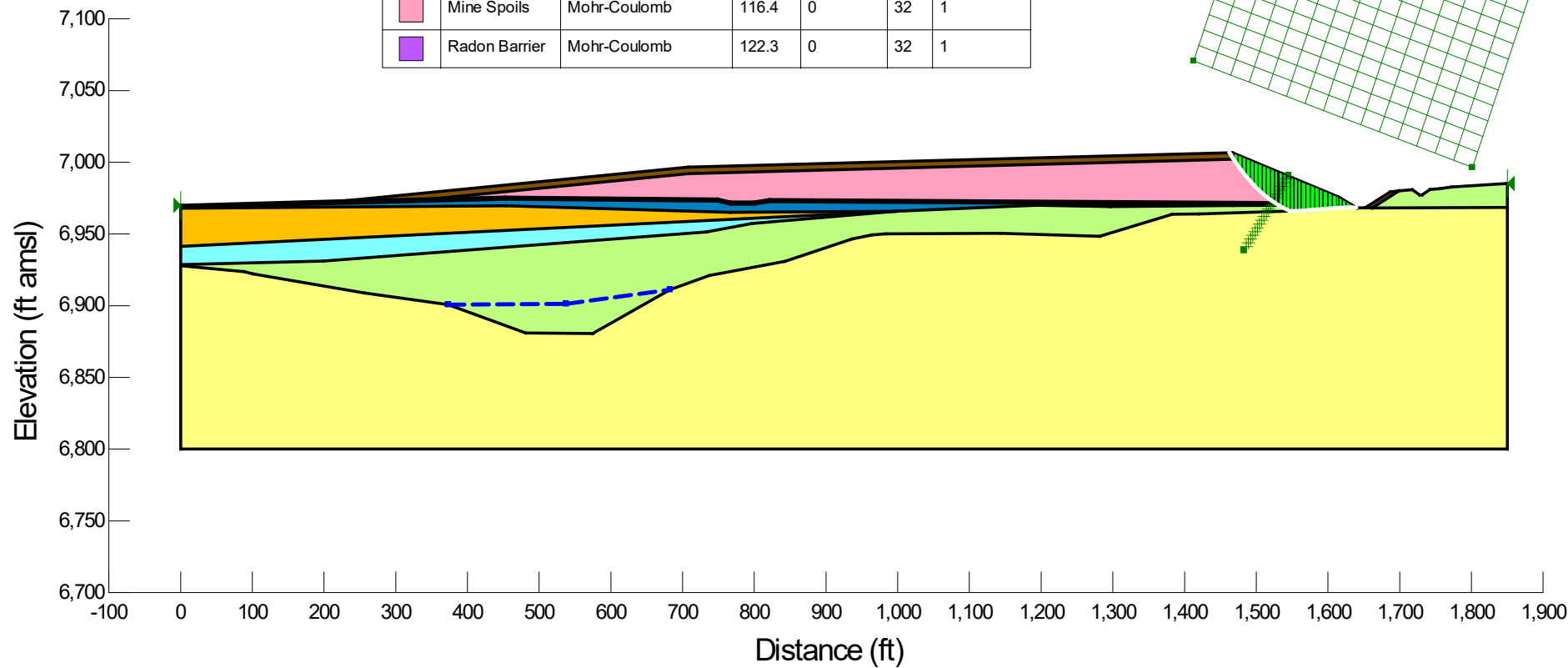










Figure D.3 – Cross Section A
Static Analysis – Northeast Slope

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|---|-----------------|------------------------|-------------------|-----------------|----------|------------------|
|  | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
|  | Bedrock | Bedrock (Impenetrable) | | | | |
|  | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
|  | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
|  | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
|  | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
|  | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
|  | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

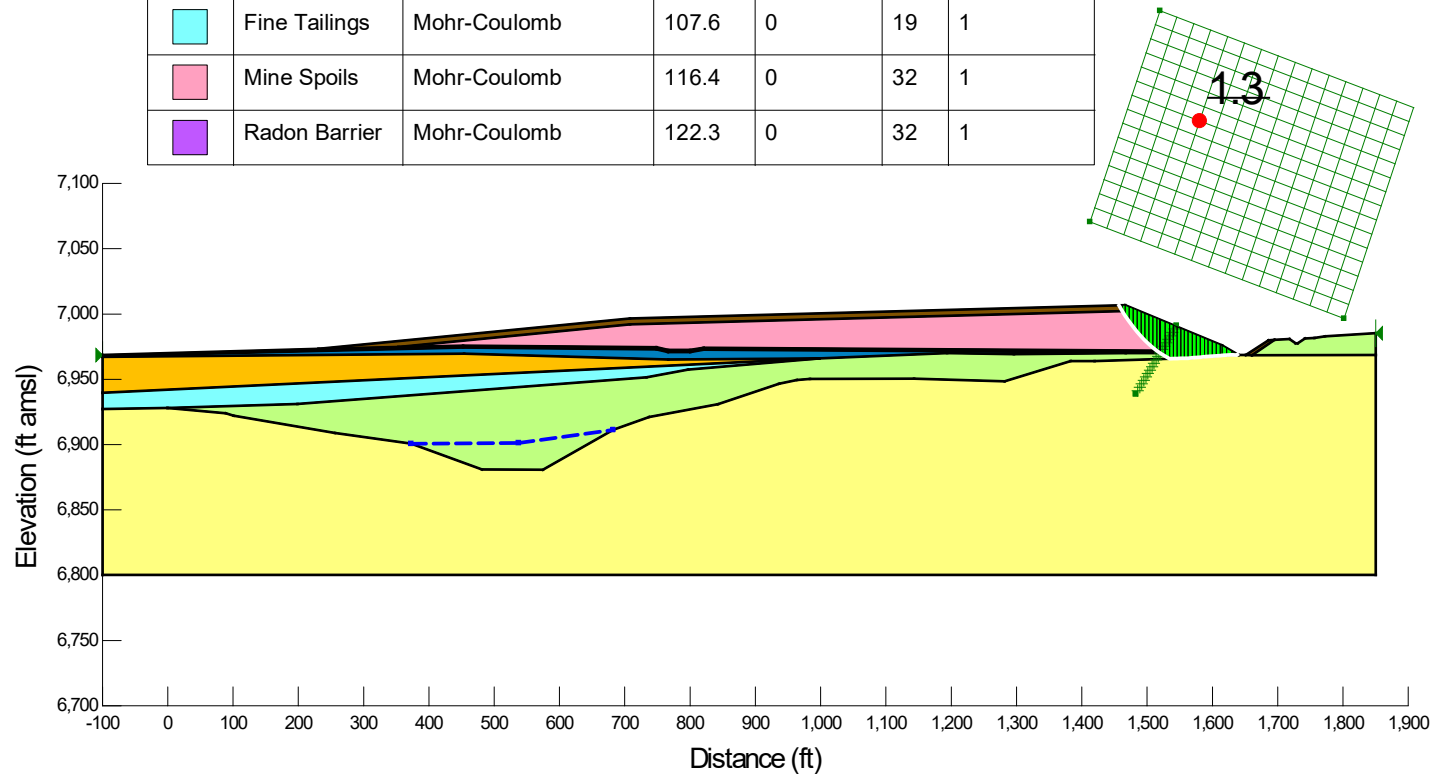


Figure D.4 – Cross Section A
Pseudo-Static Analysis – Northeast Slope

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

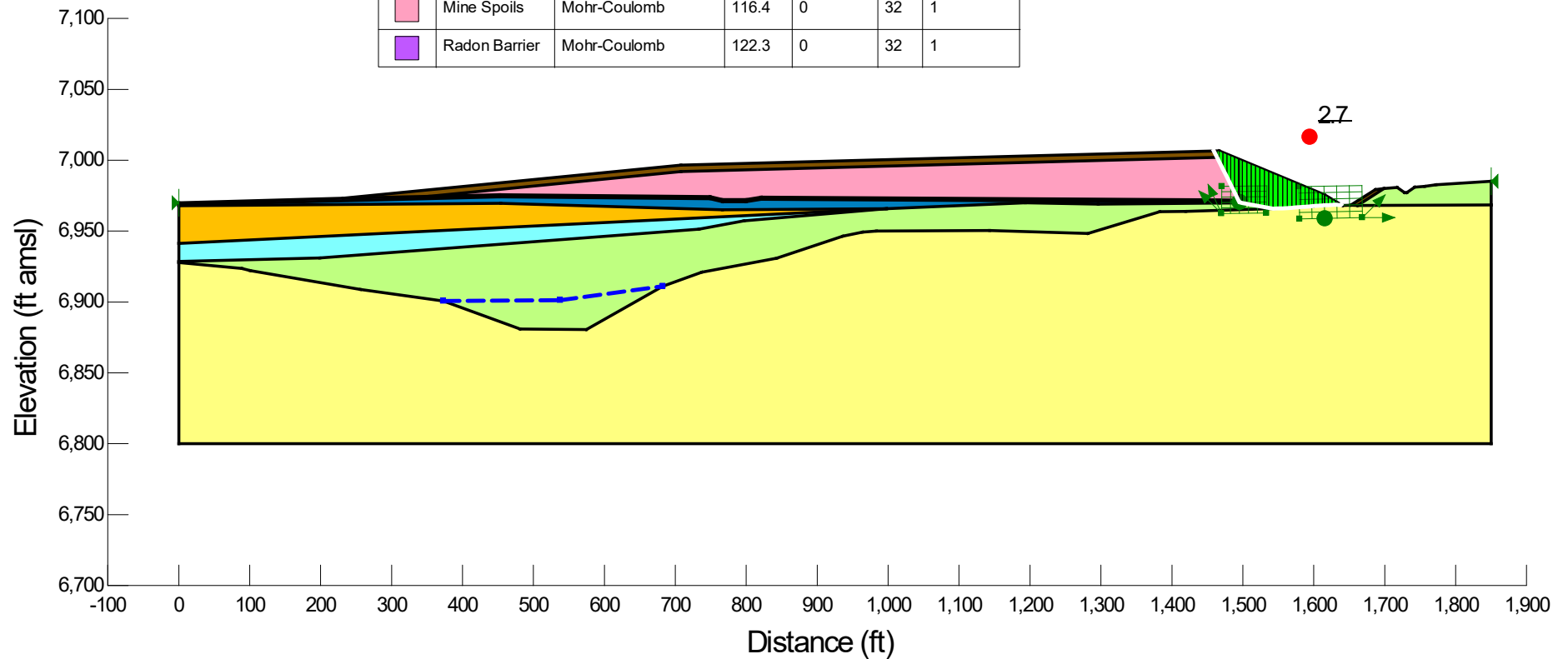


Figure D.5 – Cross Section A
Static Analysis – Northeast Slope – Block Failure

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

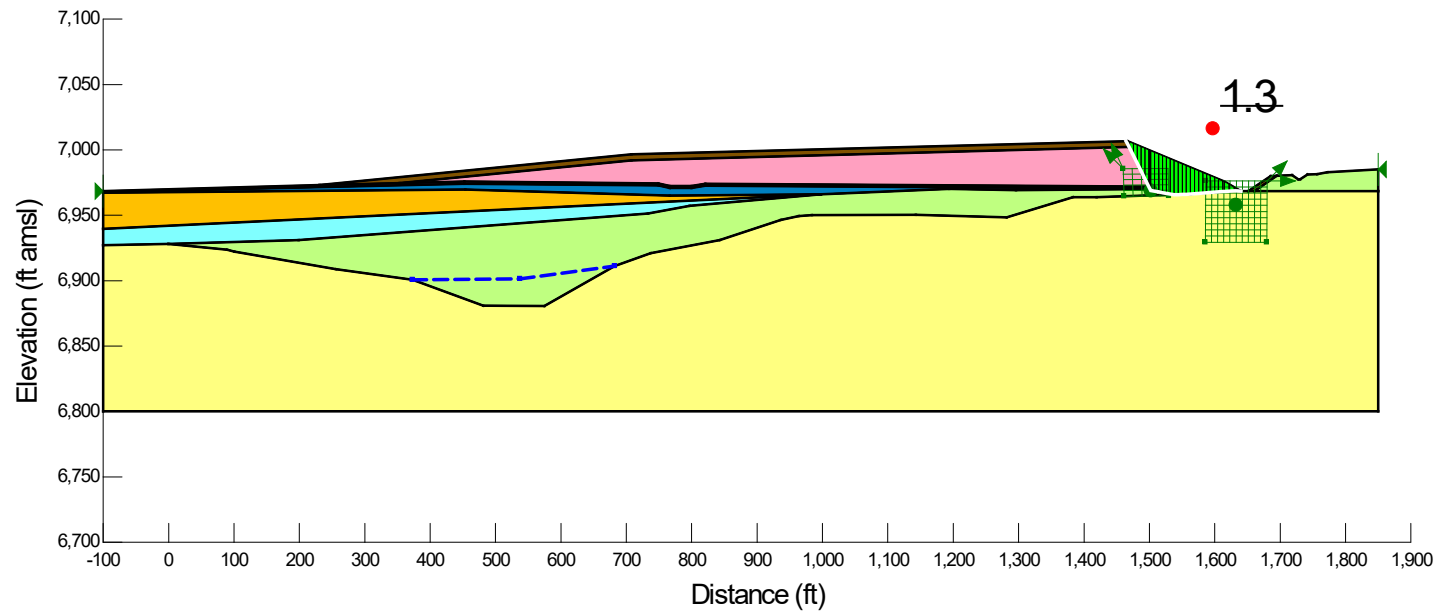
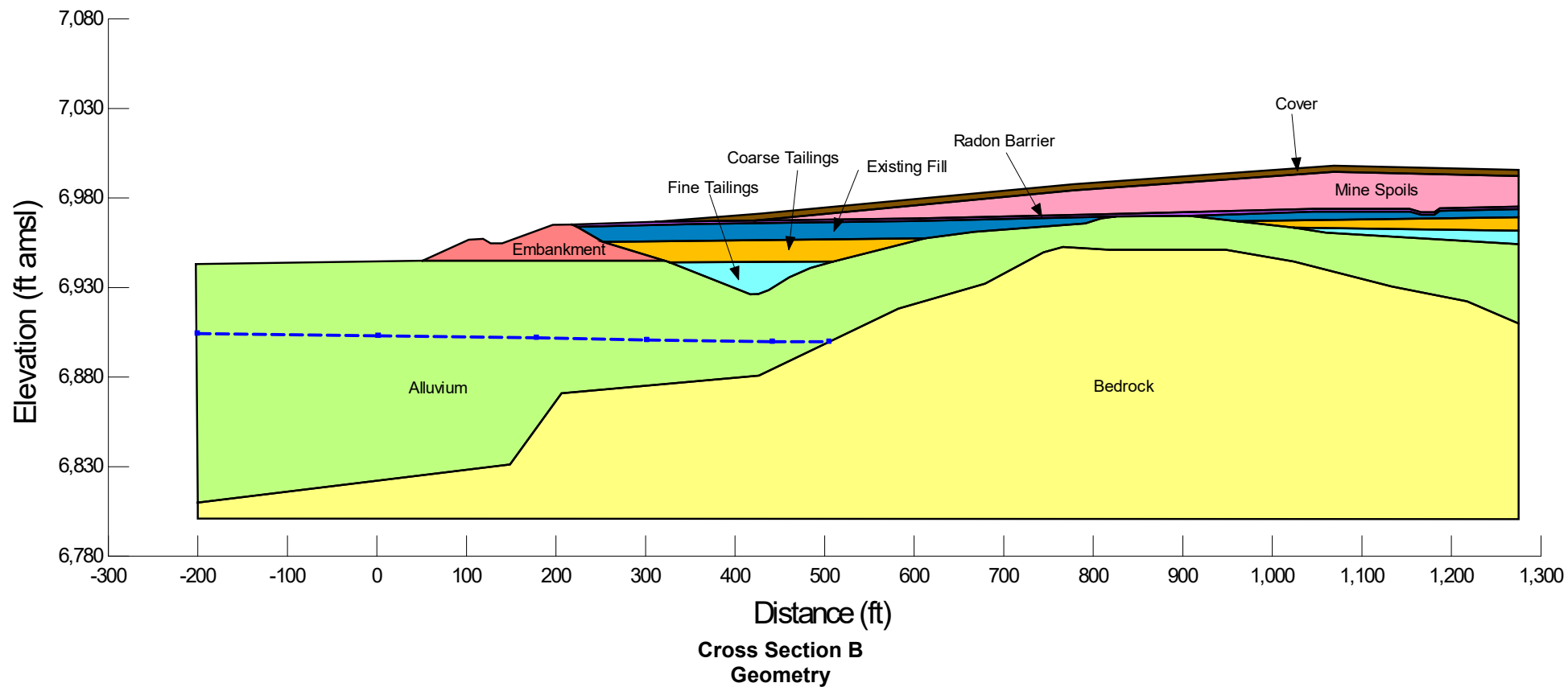
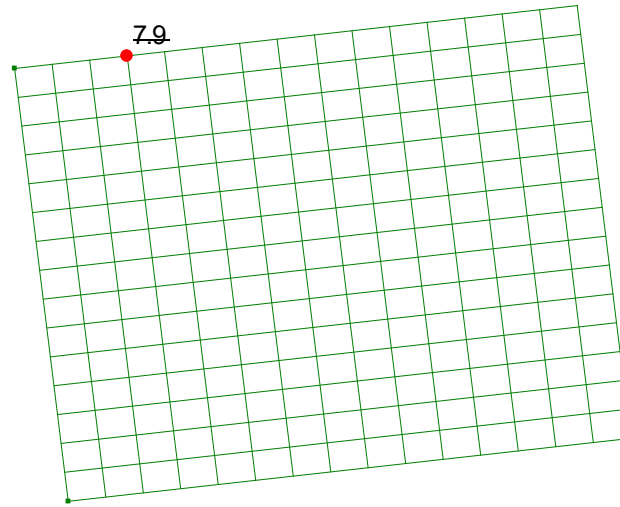


Figure D.6 – Cross Section A
Pseudo-Static Analysis – Northeast Slope – Block Failure





| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) | Mohr-Coulomb | 119.1 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

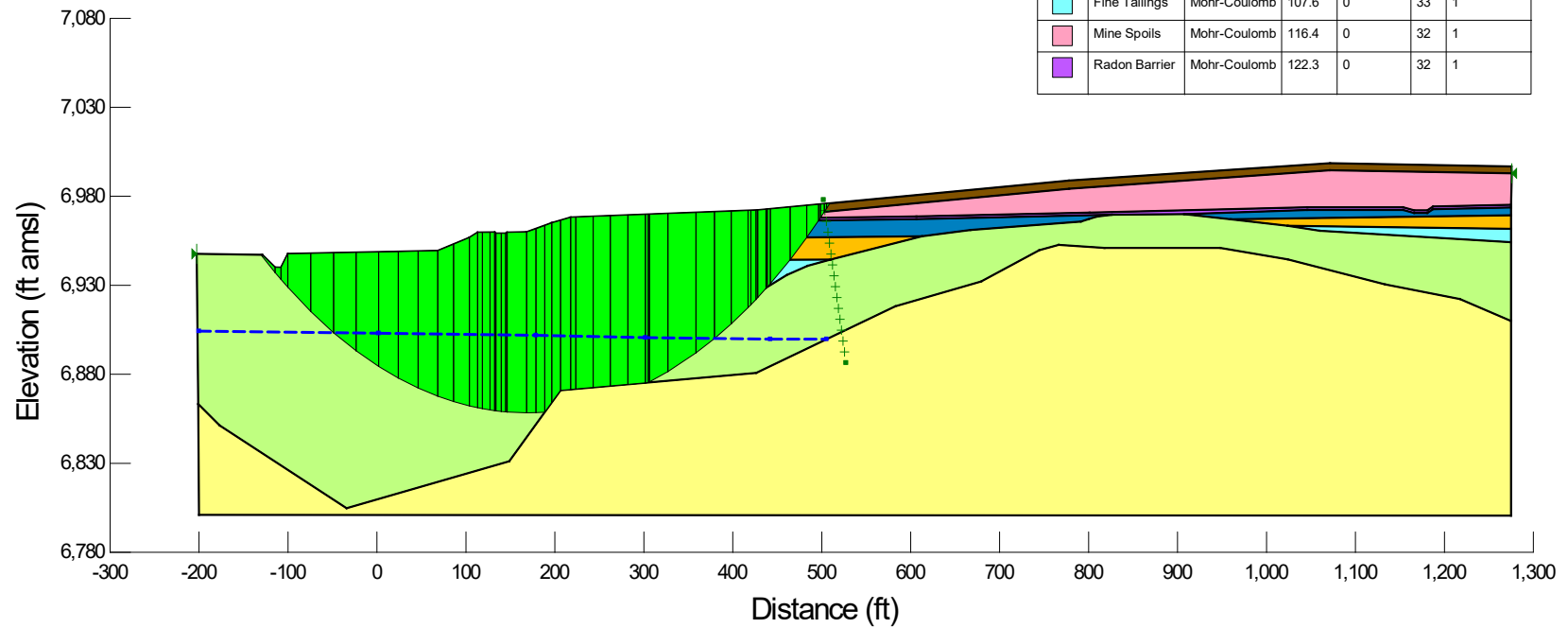
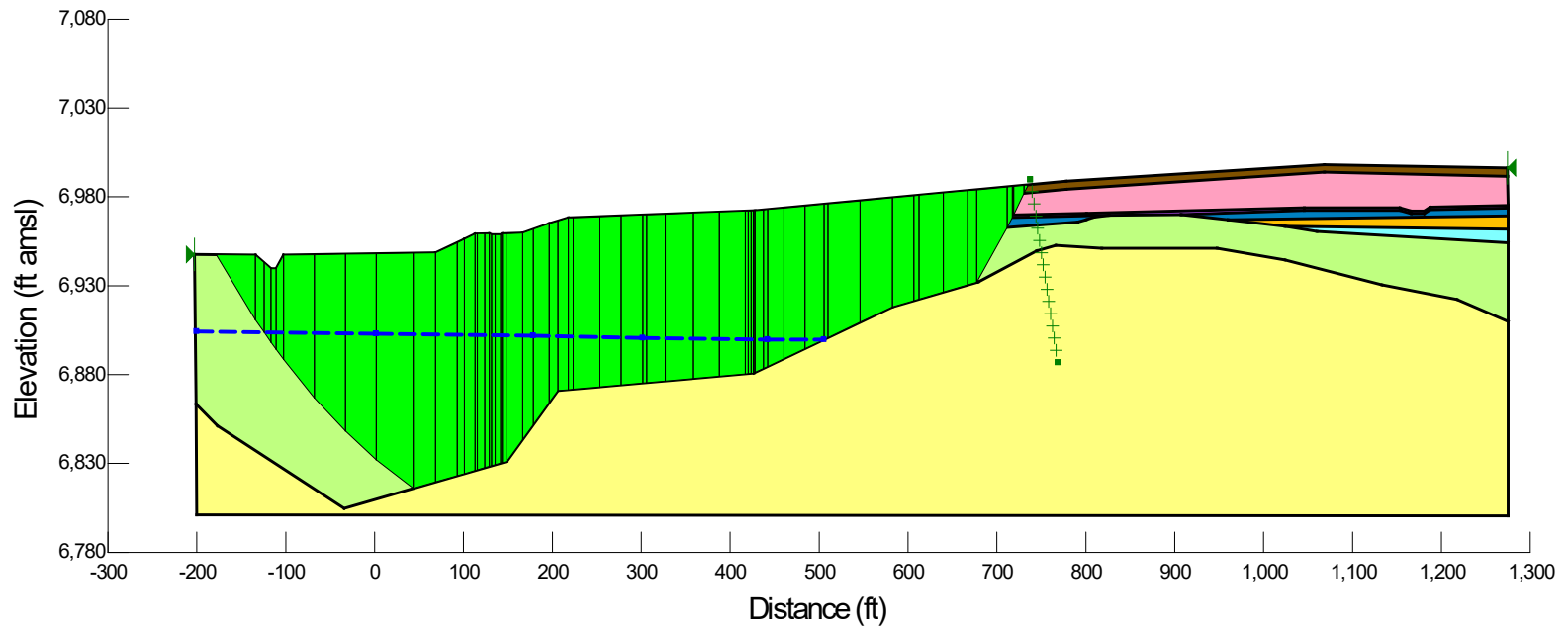
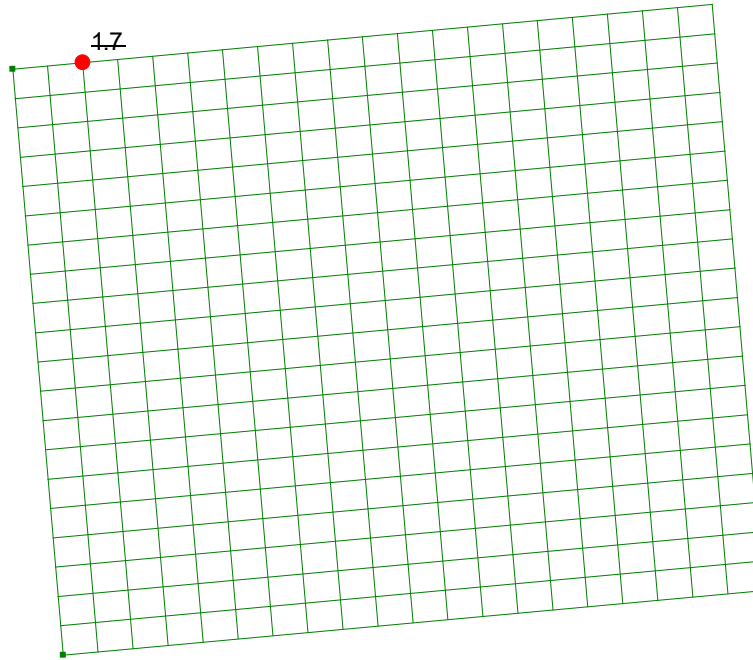


Figure D.7 – Cross Section B
Static Analysis – Repository Slope

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) | Mohr-Coulomb | 119.1 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



**Figure D.8 – Cross Section B
Pseudo-Static Analysis – Repository Slope**

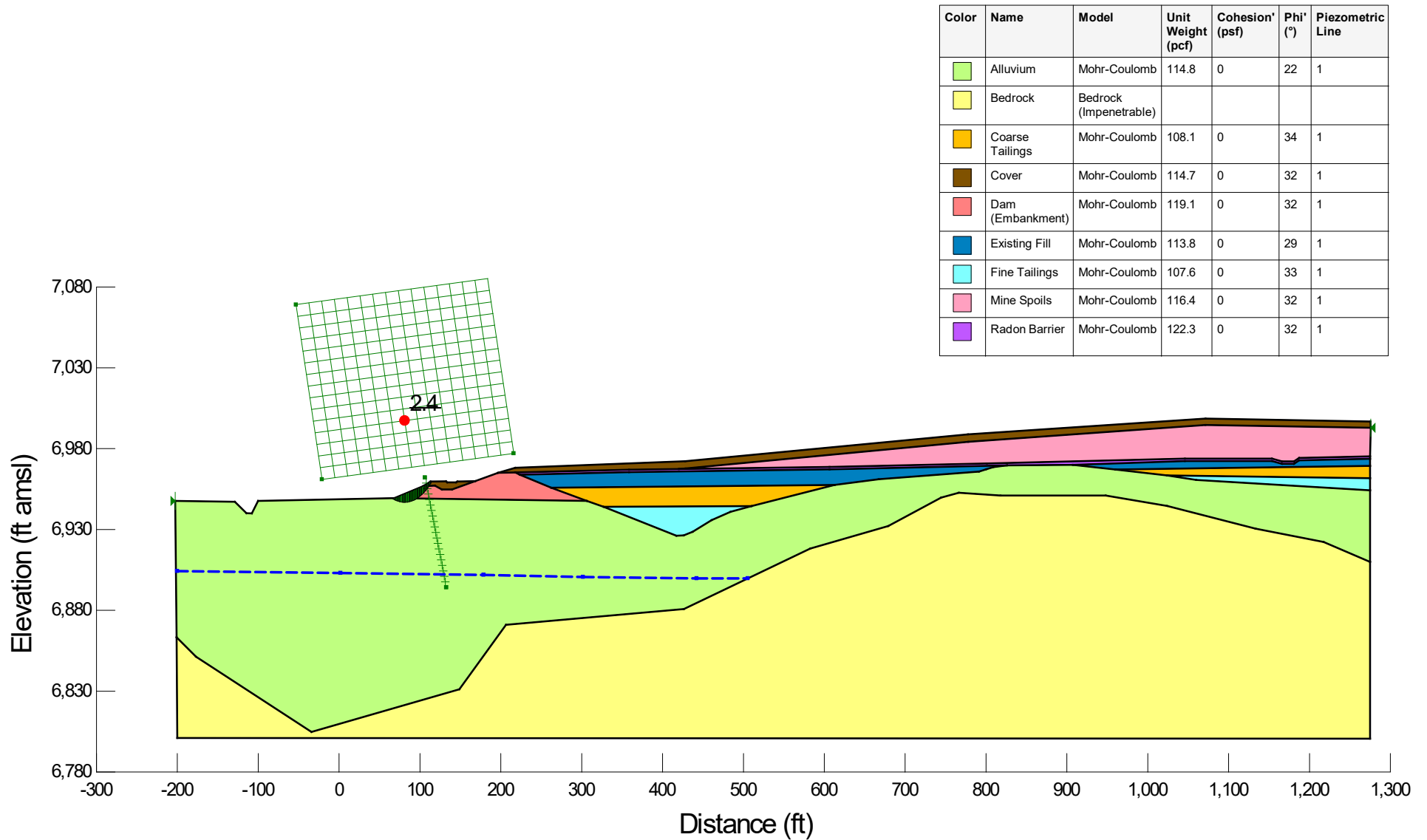


Figure D.9 – Cross Section B
Static Analysis – Existing Embankment (Dam)

| Color | Name | Model | Unit Weight (pcf) | Cohesion* (psf) | Phi' (°) | Piezometric Line |
|-------|------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) | Mohr-Coulomb | 119.1 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

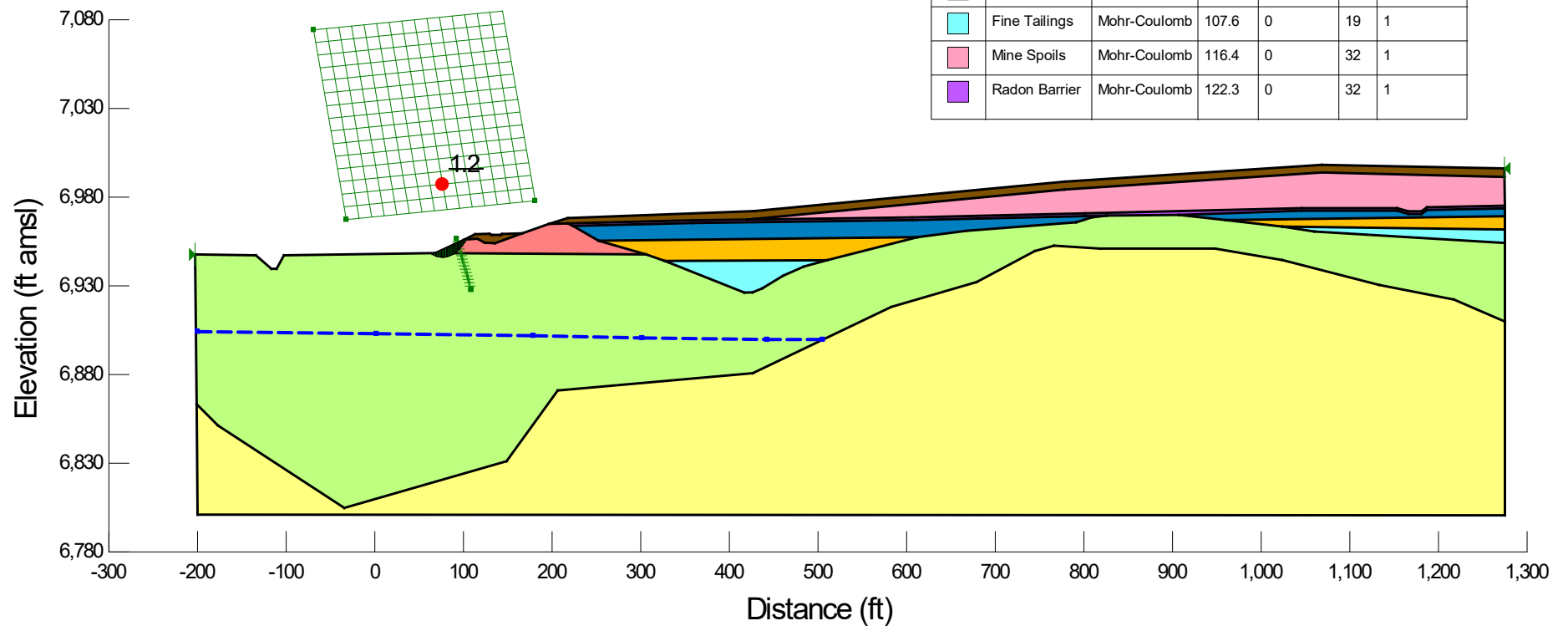


Figure D.10 – Cross Section B
Pseudo-Static Analysis – Existing Embankment (Dam)

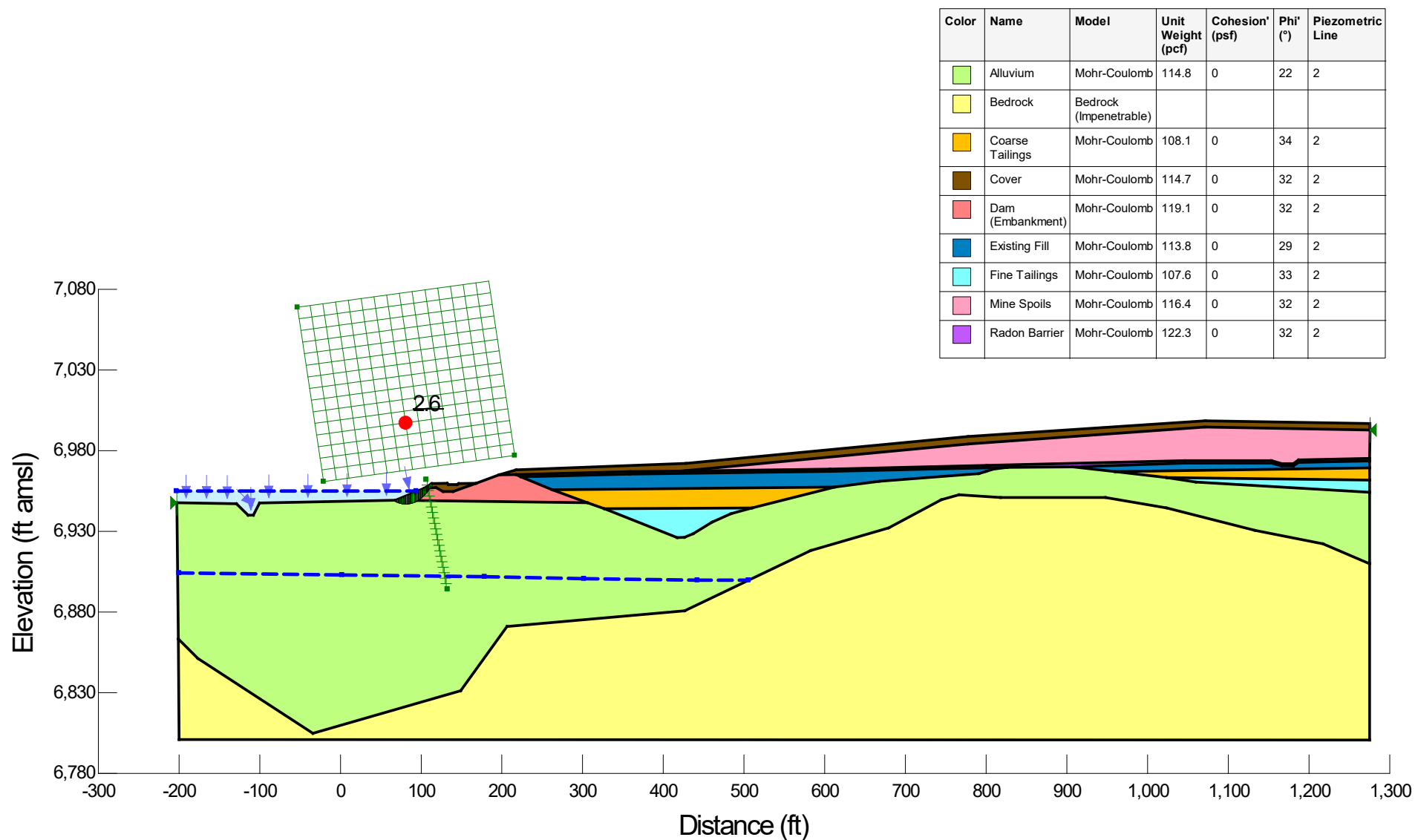
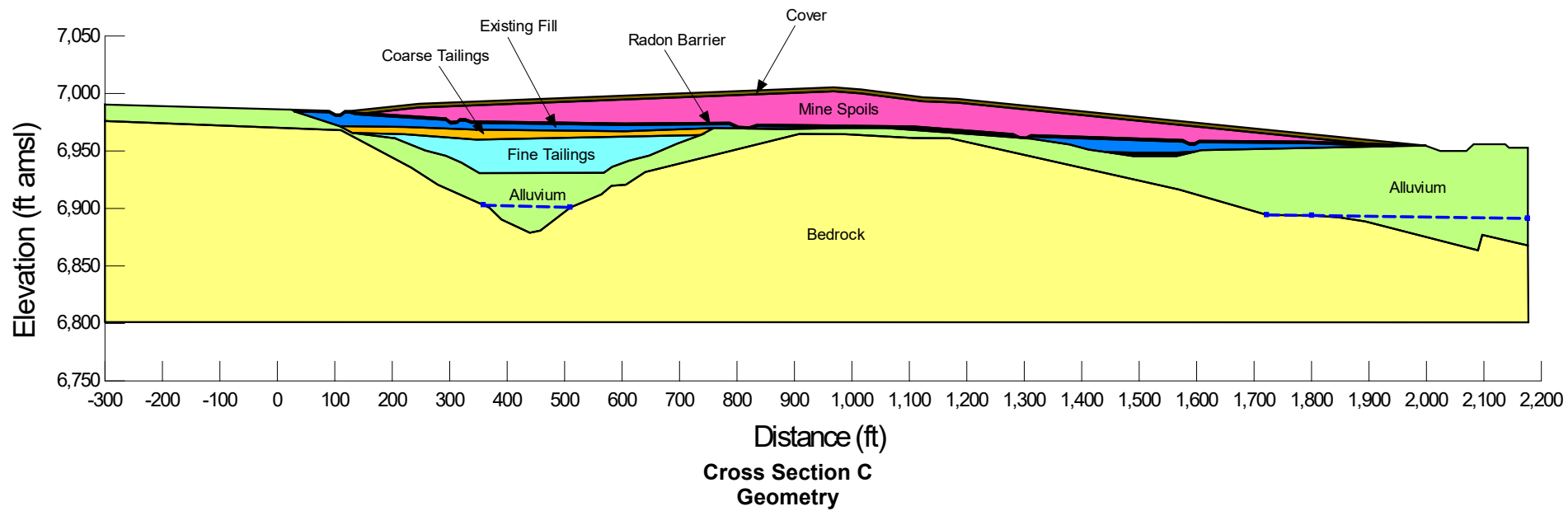


Figure D.11 – Cross Section B
Static Analysis – Probable Maximum Flood



| Color | Name | Model | Unit Weight (pcf) | Cohesion (psf) | Phi (°) | Piezometric Line |
|-------------|-----------------|------------------------|-------------------|----------------|---------|------------------|
| Light Green | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| Yellow | Bedrock | Bedrock (Impenetrable) | | | | 1 |
| Orange | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| Brown | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| Blue | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| Cyan | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 1 |
| Pink | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| Purple | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

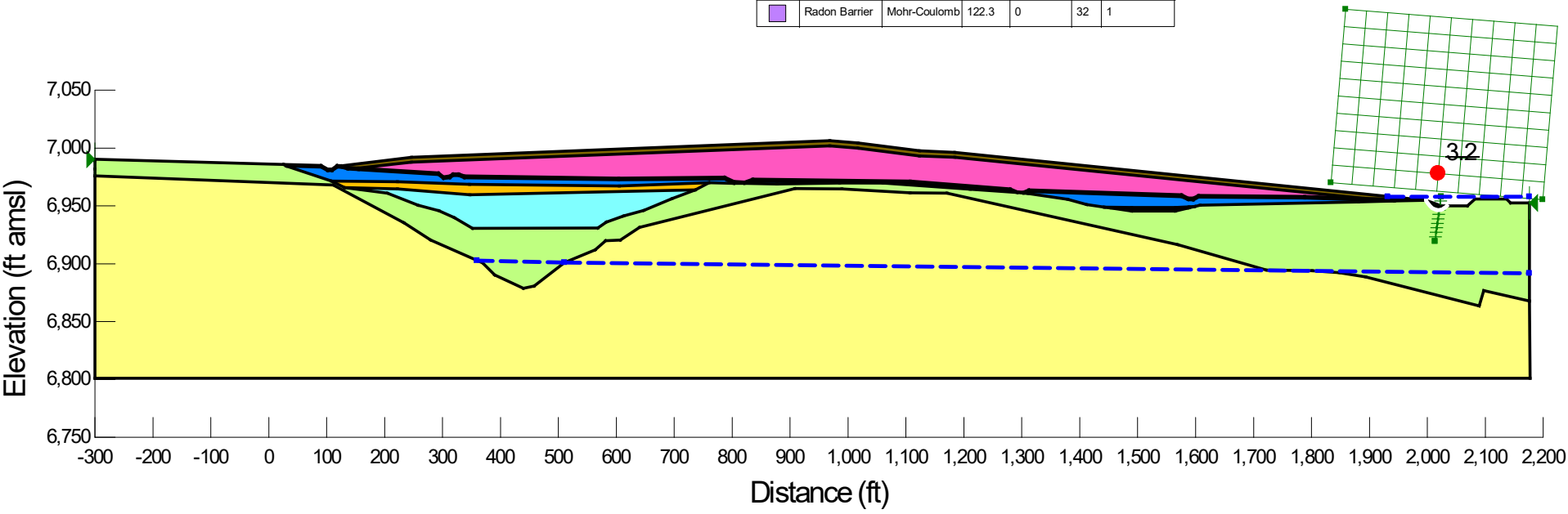


Figure D.12 – Cross Section C
Static Analysis – North Slope

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | 1 |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

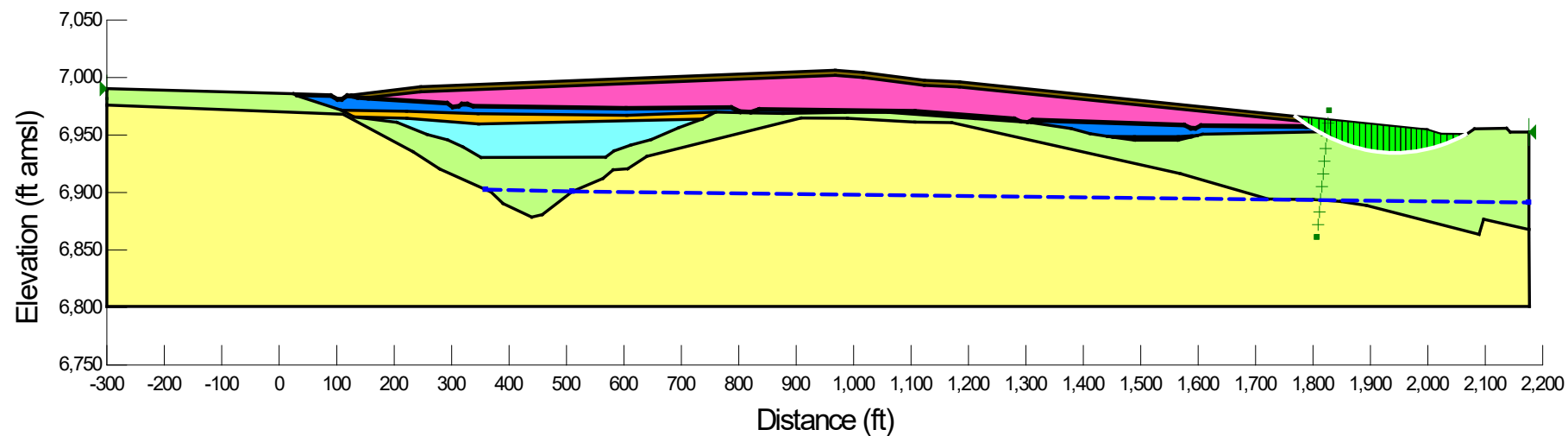
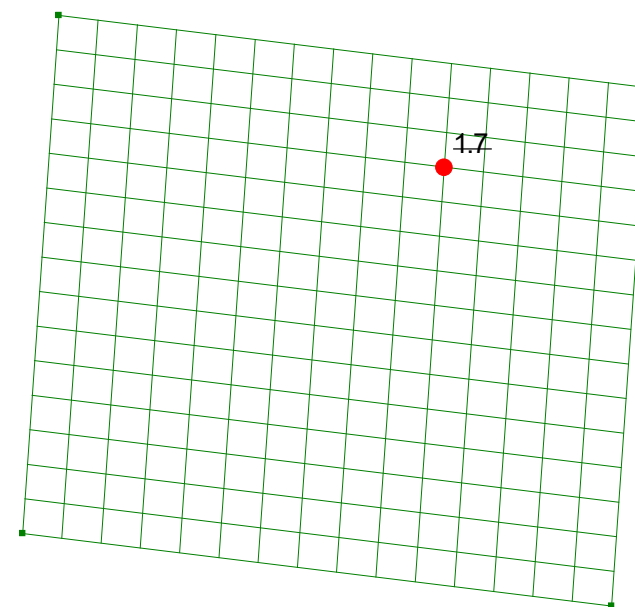










Figure D.13 – Cross Section C
Pseudo-Static Analysis – North Slope

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|---|-----------------|------------------------|-------------------|-----------------|----------|------------------|
|  | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
|  | Bedrock | Bedrock (Impenetrable) | | | | 1 |
|  | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
|  | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
|  | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
|  | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
|  | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
|  | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

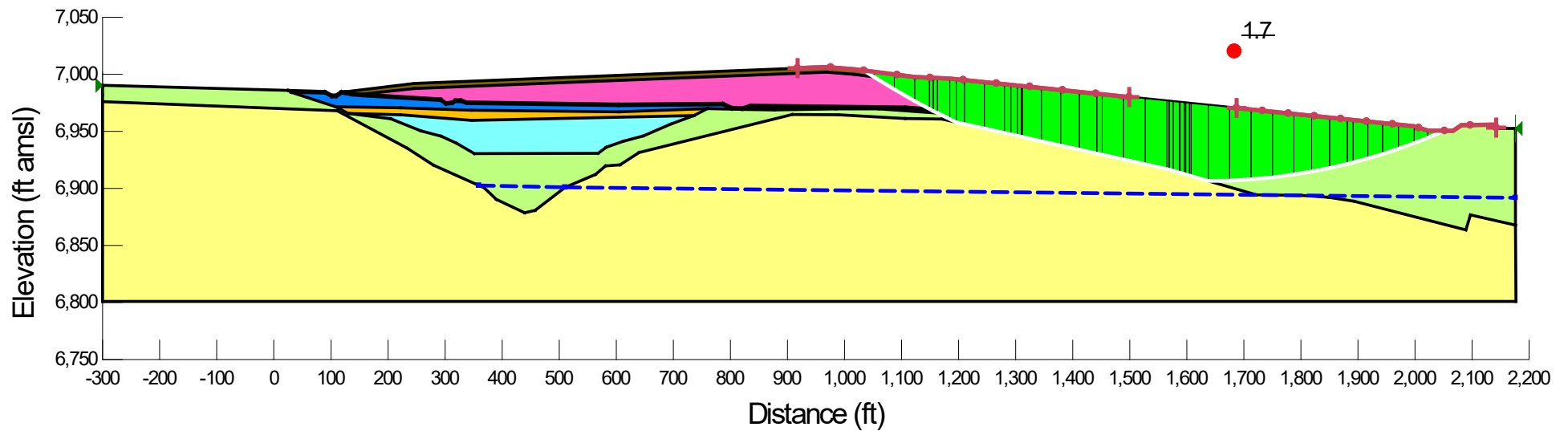


Figure D.14 – Cross Section C
Pseudo-Static Analysis – North Slope – Entry/Exit

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 2 |
| | Bedrock | Bedrock (Impenetrable) | | | | 2 |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 2 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 2 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 2 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 2 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 2 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 2 |

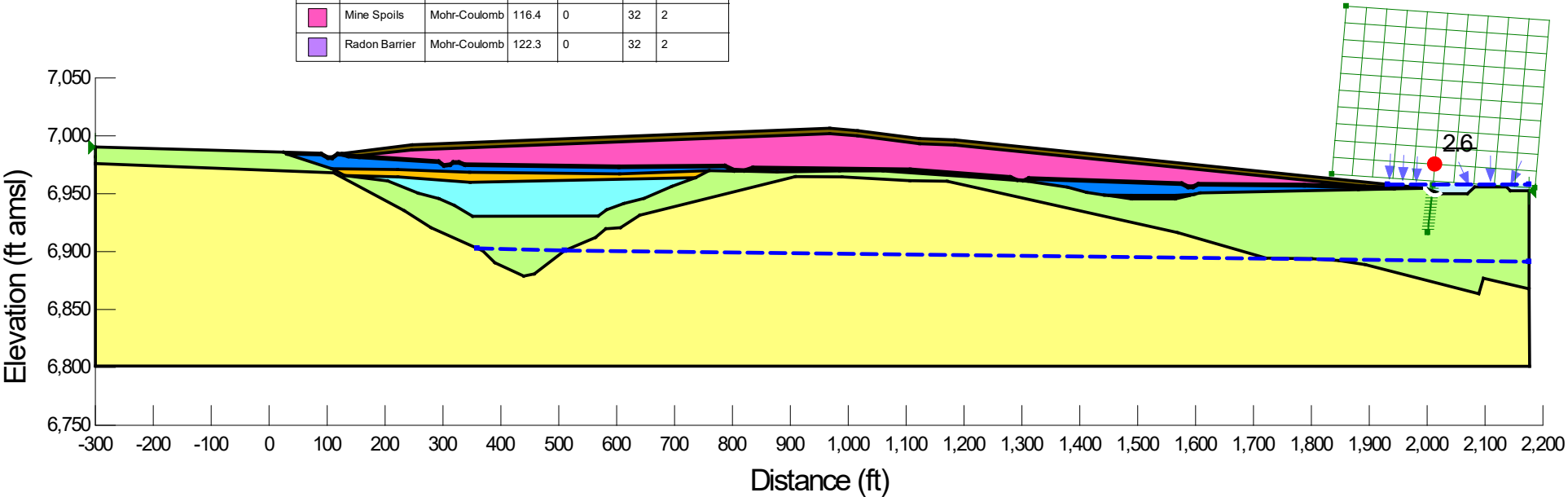
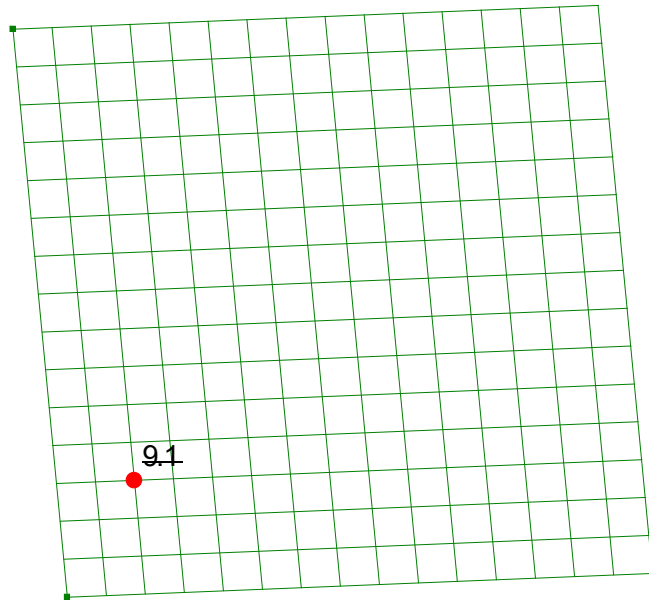
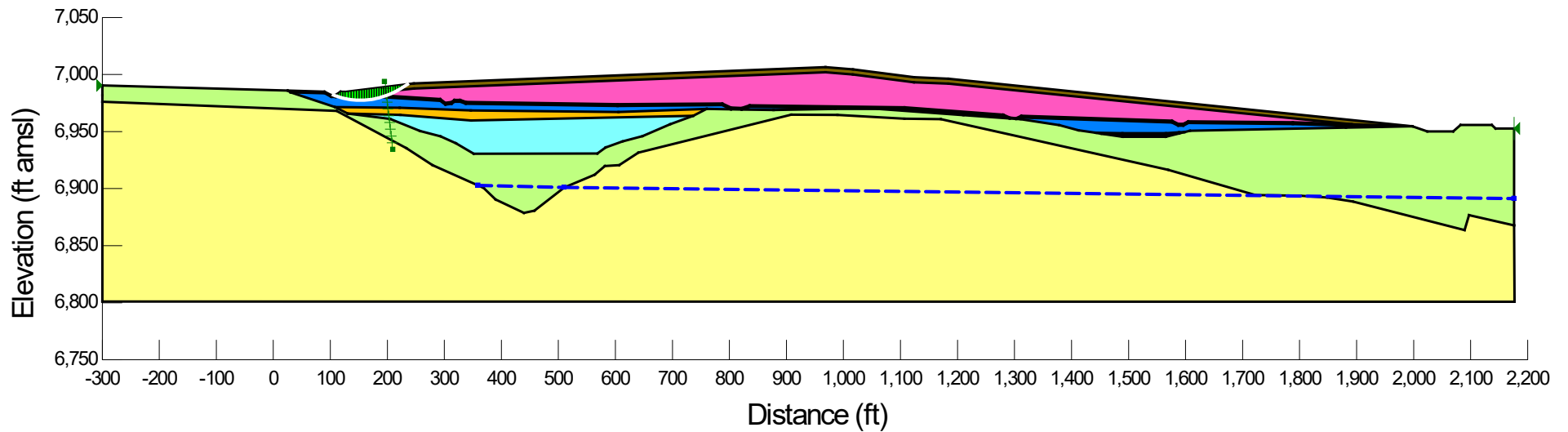


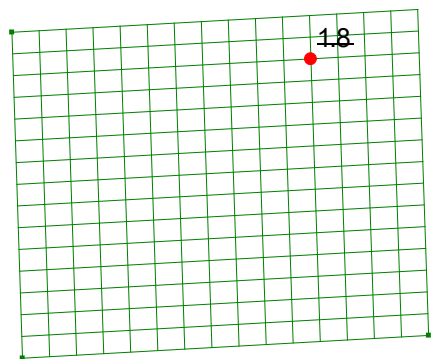
Figure D.15 – Cross Section C
Static Analysis – Probable Maximum Flood



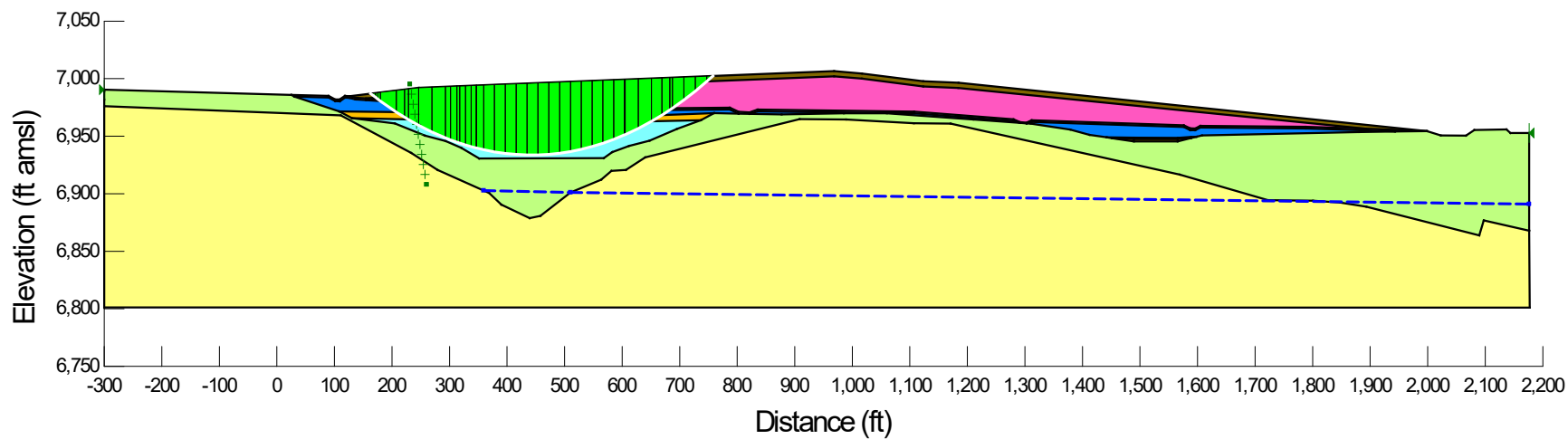
| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | 1 |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 33 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



**Figure D.16 – Cross Section C
Static Analysis – South Slope**



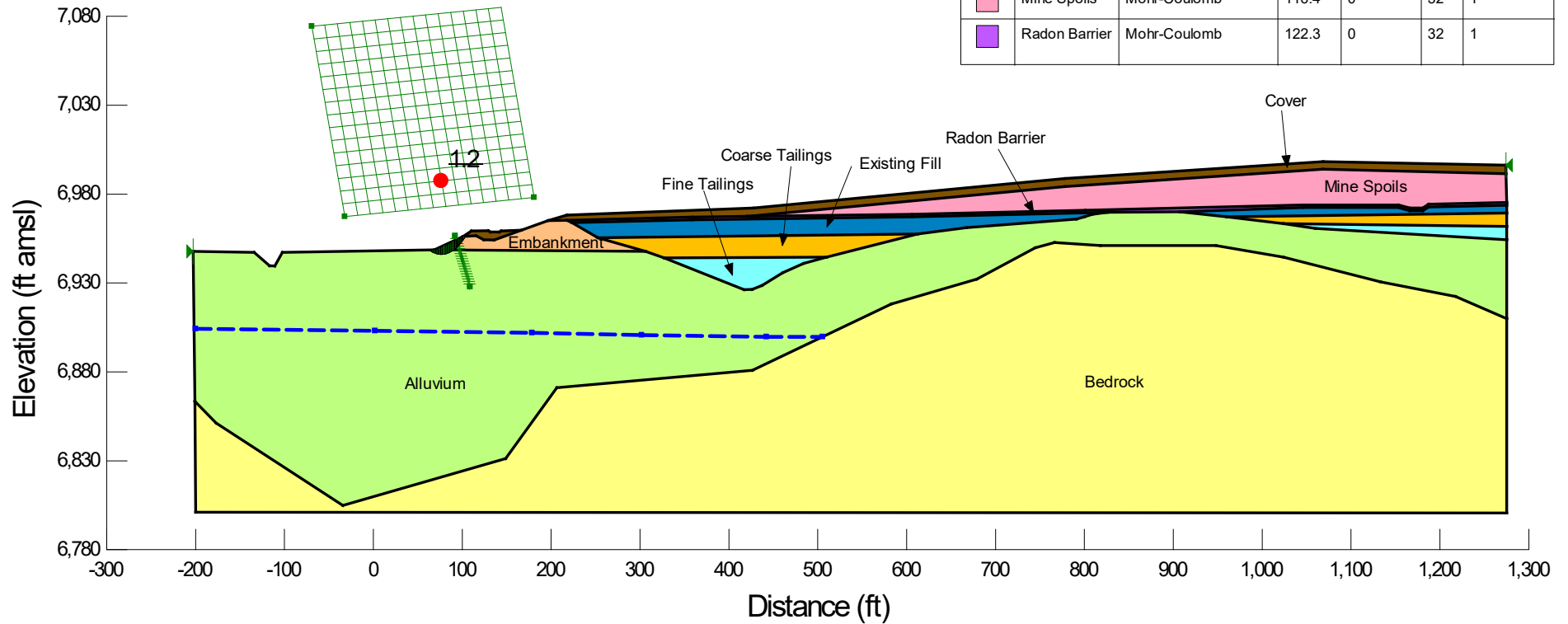
| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | 1 |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



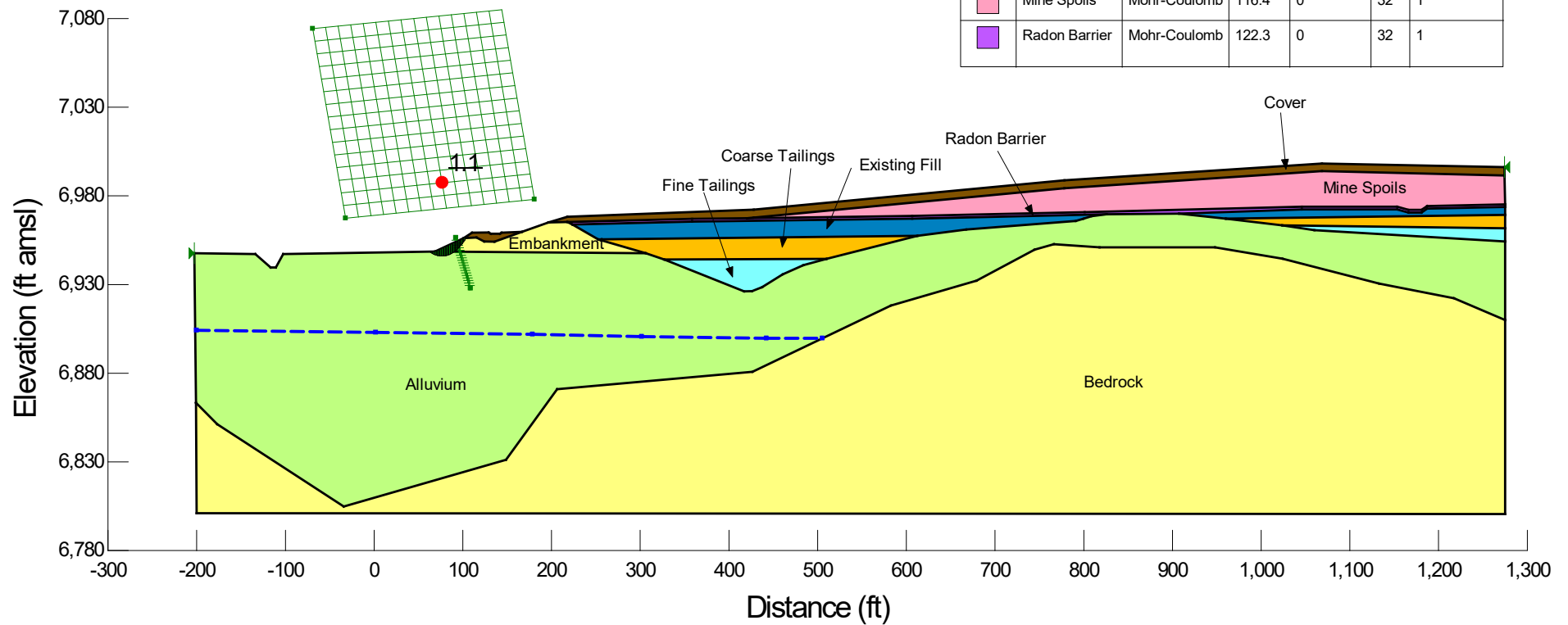
**Figure D.17 – Cross Section C
Pseudo-Static Analysis – South Slope**

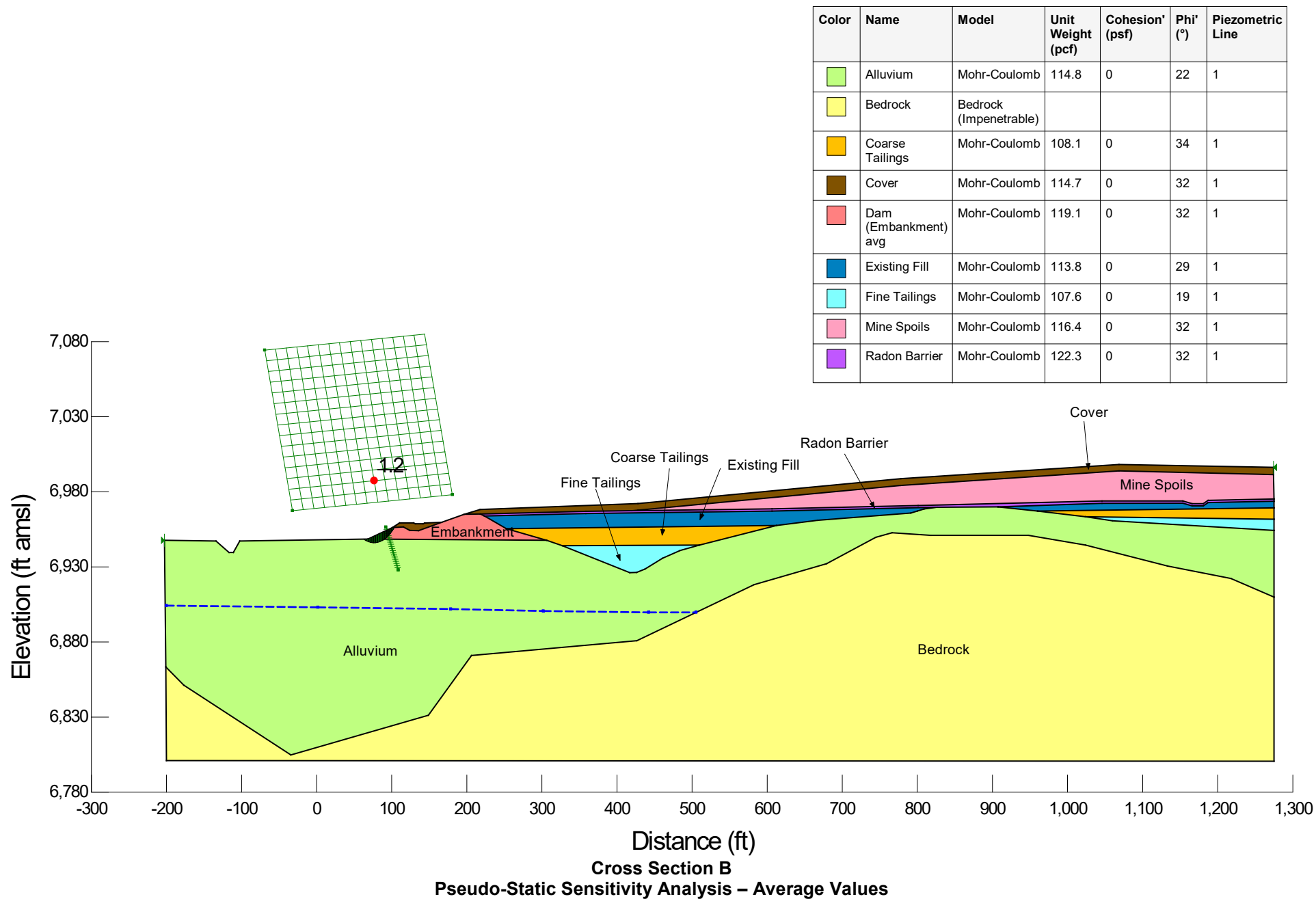
**Attachment D.1 Sensitivity Analysis
Slope/w Output Files**

| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) 30th | Mohr-Coulomb | 113.1 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

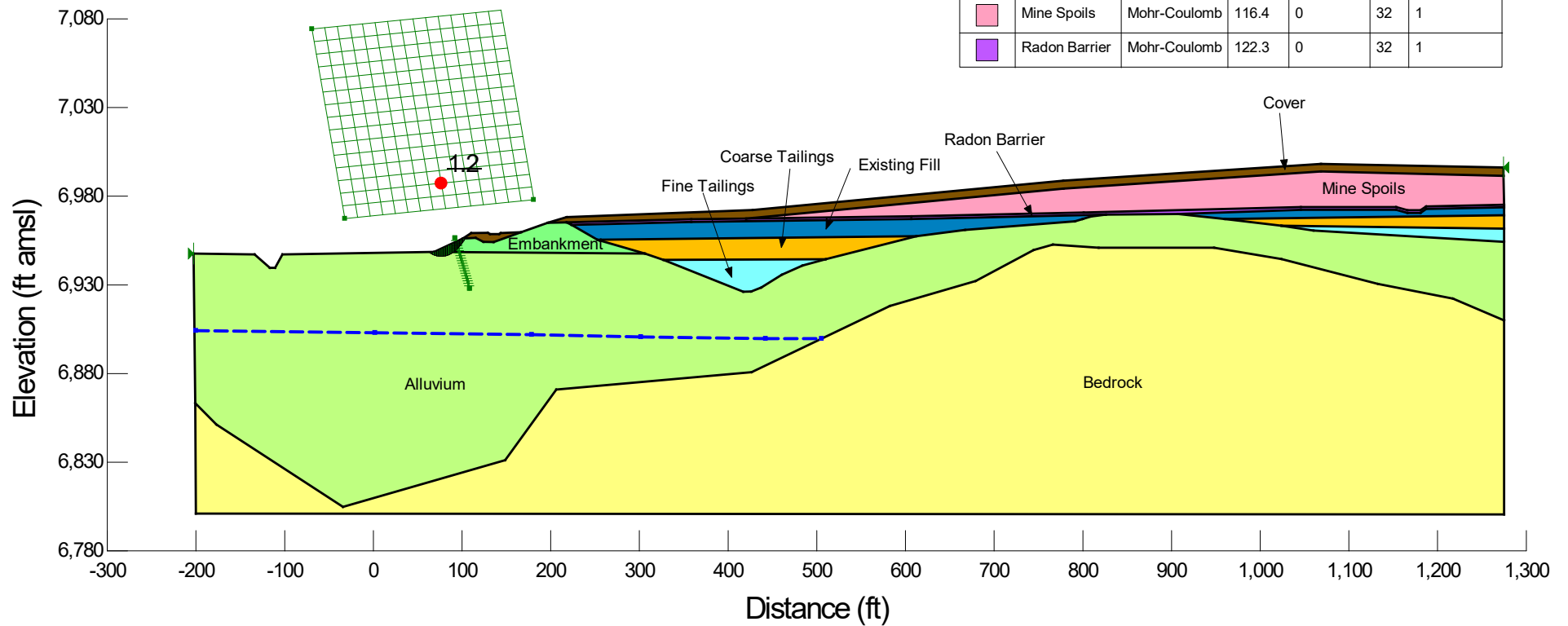


| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-----------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) 60th | Mohr-Coulomb | 123.7 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |

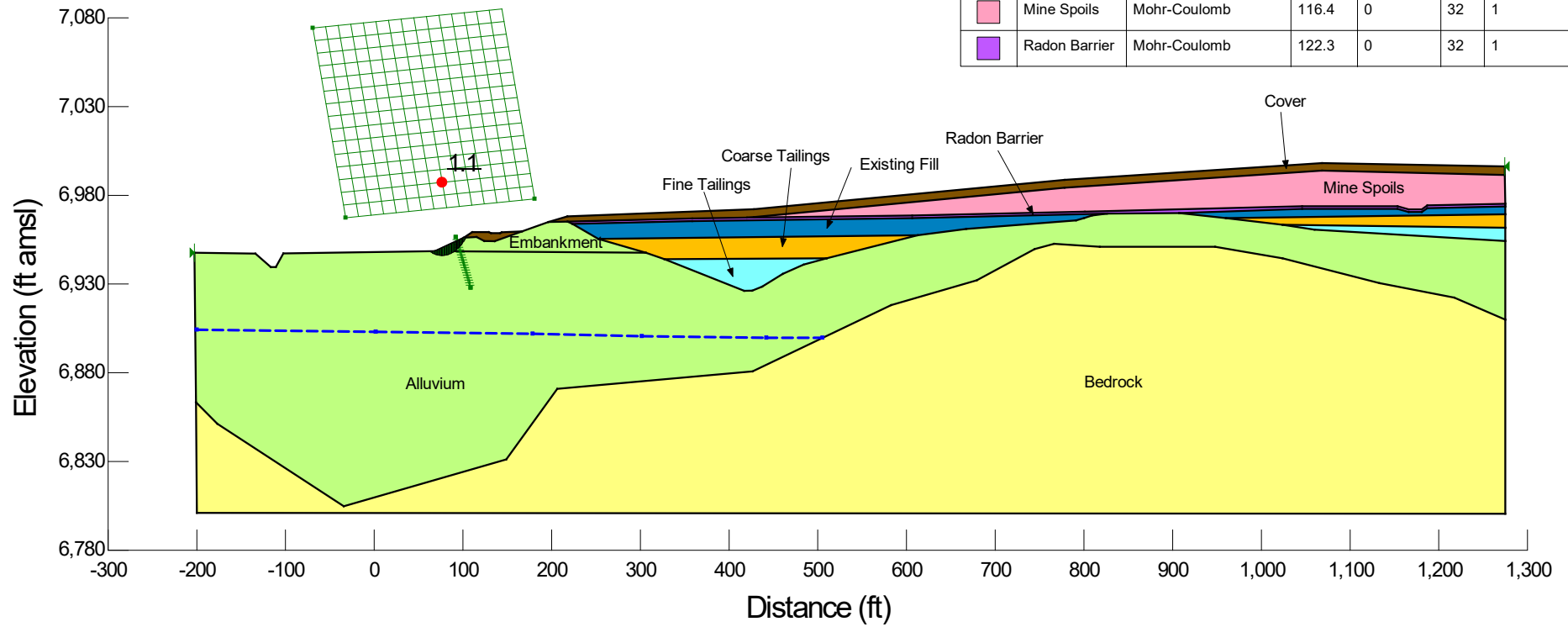




| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|-------------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) median | Mohr-Coulomb | 120.5 | 0 | 32 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



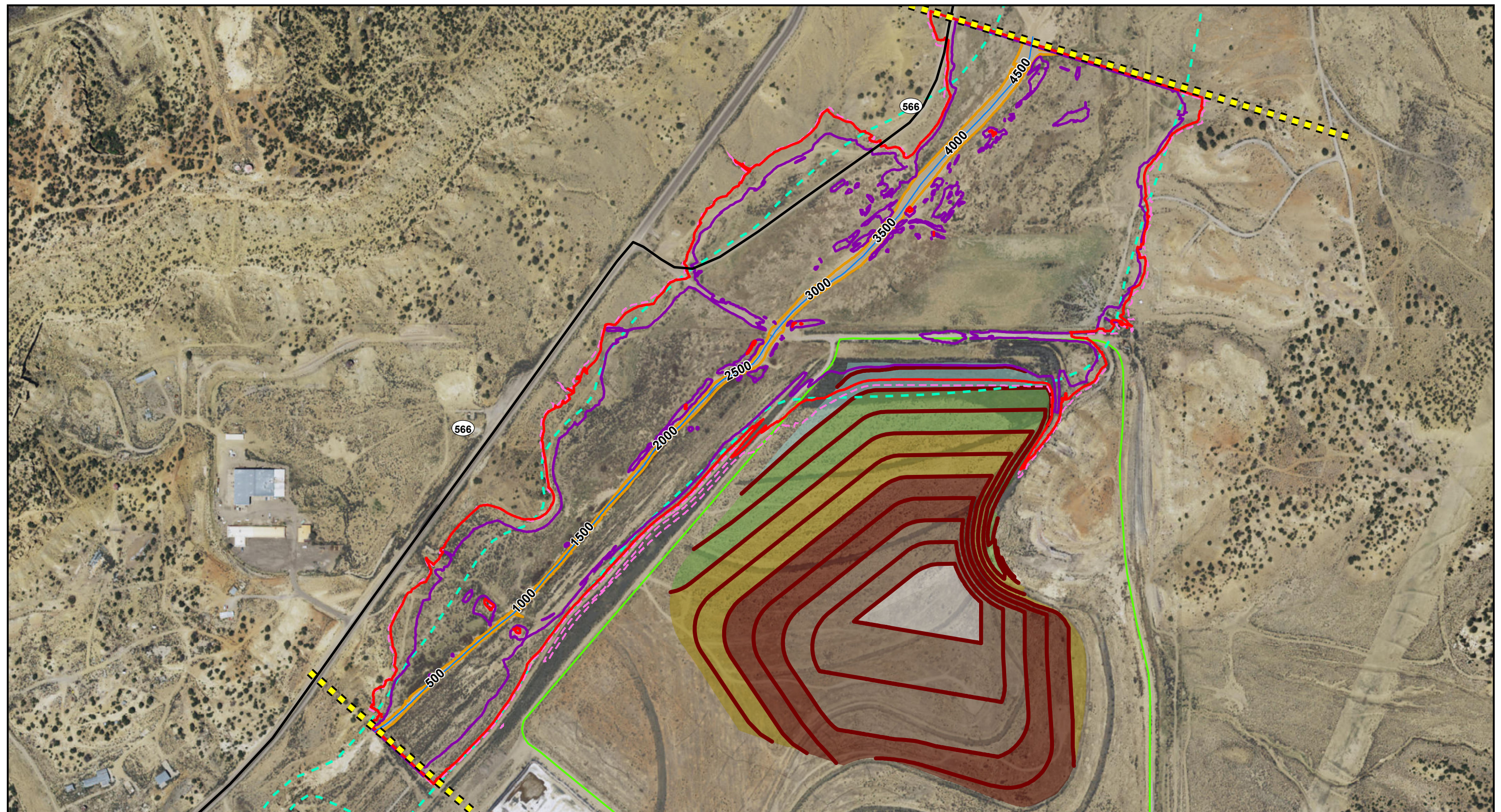
| Color | Name | Model | Unit Weight (pcf) | Cohesion' (psf) | Phi' (°) | Piezometric Line |
|-------|---------------------------|------------------------|-------------------|-----------------|----------|------------------|
| | Alluvium | Mohr-Coulomb | 114.8 | 0 | 22 | 1 |
| | Bedrock | Bedrock (Impenetrable) | | | | |
| | Coarse Tailings | Mohr-Coulomb | 108.1 | 0 | 34 | 1 |
| | Cover | Mohr-Coulomb | 114.7 | 0 | 32 | 1 |
| | Dam (Embankment) Vary Phi | Mohr-Coulomb | 119.1 | 0 | 28 | 1 |
| | Existing Fill | Mohr-Coulomb | 113.8 | 0 | 29 | 1 |
| | Fine Tailings | Mohr-Coulomb | 107.6 | 0 | 19 | 1 |
| | Mine Spoils | Mohr-Coulomb | 116.4 | 0 | 32 | 1 |
| | Radon Barrier | Mohr-Coulomb | 122.3 | 0 | 32 | 1 |



Cross Section B
Pseudo-Static Sensitivity Analysis – Reduce Dam Material Friction Angle


ATTACHMENT E

PMF EXTENTS



Legend

| | |
|--|---|
| — Proposed Repository Area (5' Contour Interval) | — TDA-North Cell |
| — Upper & Lower Extents of Study | — PMF Flood Extents |
| — Pipeline Road | — 100 Year Event Flood Extents |
| — Upper Pipeline Arroyo Thalweg | — 5 Year Event Flood Extents |
| — PMF Flood Extents from Canonie (1991) | — PMF Flood Extents (Maximum Roughness) |



0 250 500 1,000 1,500 Feet




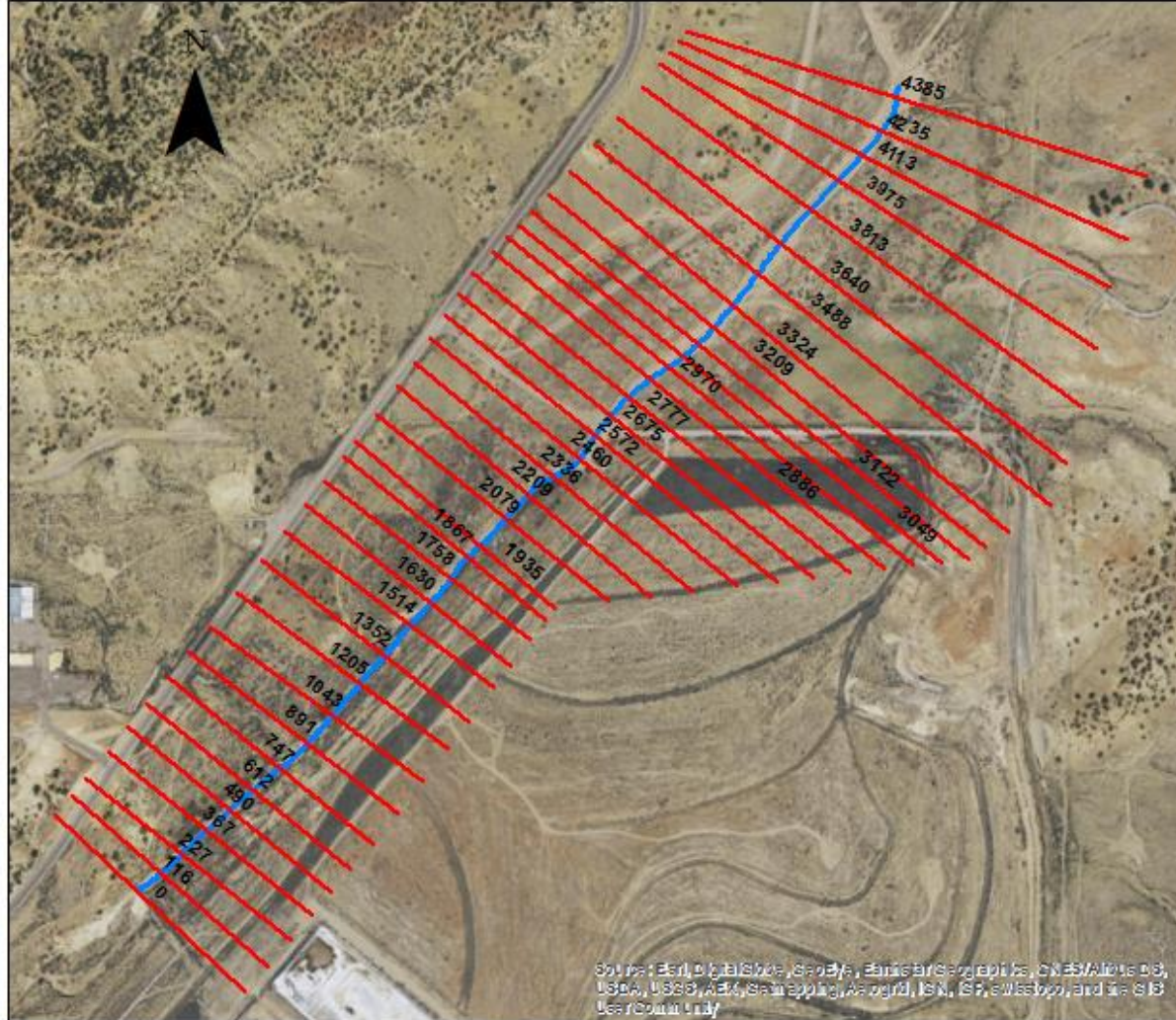



FIGURE I-6.3:
Estimated Pipeline Arroyo
Flood Extents
 Northeast Church Rock

, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS
 Date Revised: 5/18/2016

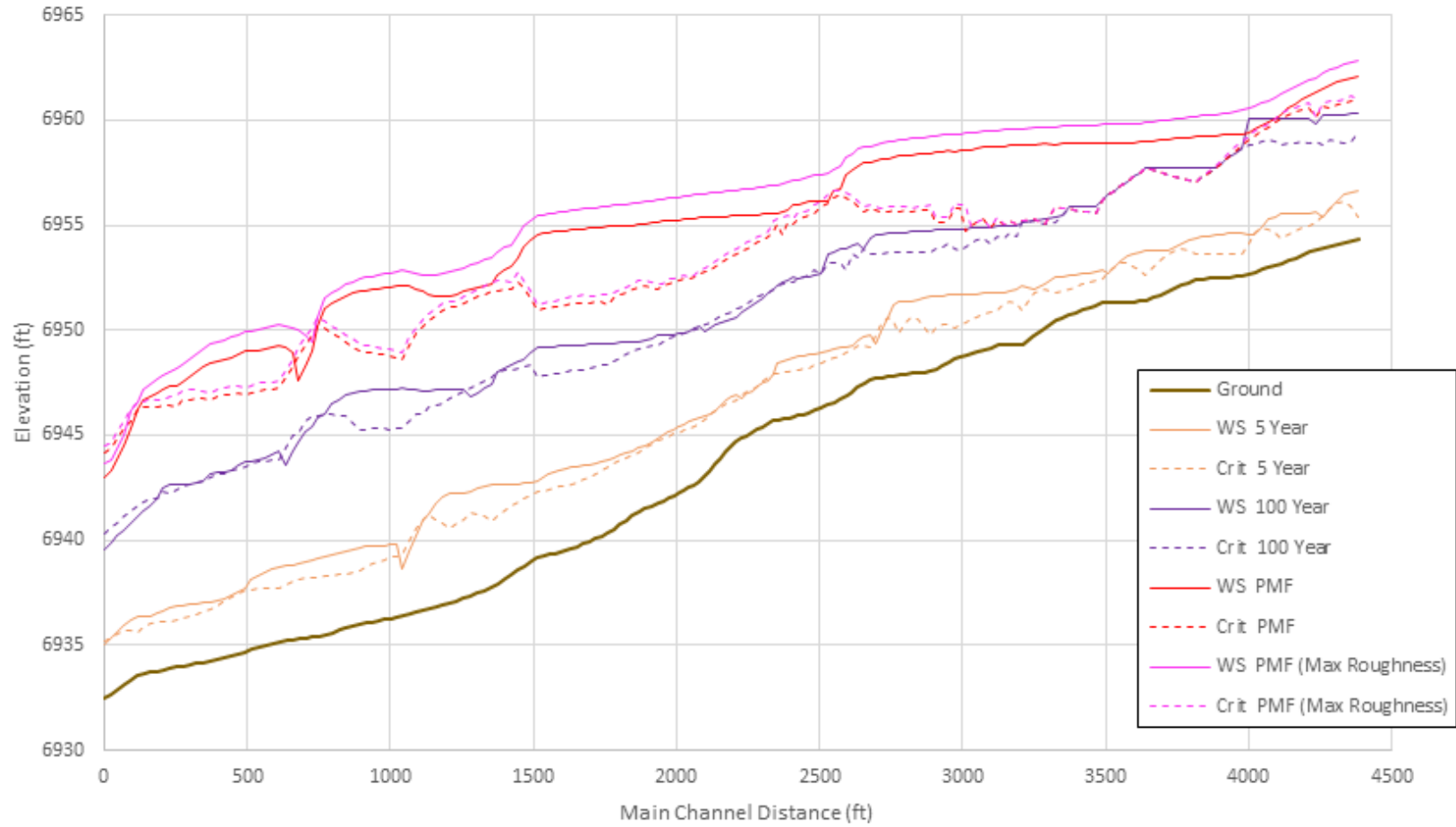




Legend

- Cross Sections
- Upper Pipeline Arroyo Thalweg

0 250 500 1,000 1,500
Feet



ATTACHMENT G.3
Repository Settlement Analysis

Client: *GE*
Project: *NECR 95% Design*
Description: *Repository Settlement*

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ATTACHMENT G.3: REPOSITORY SETTLEMENT ANALYSIS

| Revisioning | | | | | |
|-------------|-----------|------------------------|-----------|------------|------------|
| Rev. | Date | Description | By | Checked | Date |
| 0 | 5/31/2016 | 30% Design | S. Moore | J. Cumbers | 6/29/2016 |
| 1 | July 2017 | 95% Design | S. Downey | M. Davis | 10/11/2017 |
| 2 | Oct 2019 | Update Cover Thickness | S. Downey | M. Davis | 11/11/2019 |

| Location and Format |
|---|
| <p>Electronic copies of these calculations are located on the Stantec internal project teamsite.</p> <p>The following calculations were generated using the following software:</p> <p style="padding-left: 40px;">Microsoft Excel 2013</p> |

| Table of Contents | |
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| Location and Format | 1 |
| Table of Contents | 1 |
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| Objective |
|--|
| <p>The purpose of this calculation brief is to document the input data, assumptions, procedures, and results of the consolidation and immediate settlement analyses for the proposed Church Rock Mill Site (Mill Site) repository due to the waste and cover material that will be placed on the existing tailings impoundment. The analyses include immediate settlement and primary and secondary consolidation. Seismic settlement is addressed in a separate calculation brief (Attachment G.4 of Appendix G) and liquefaction-induced settlement is addressed in the liquefaction calculation brief (Attachment G.6 of Appendix G).</p> |

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Background

This analysis was performed as part of the design of the Removal Action (RA) at the Northeast Church Rock Mine Site (Mine Site) and the related Remedial Action (RA) at the Church Rock Mill Site (Mill Site). The Mine Site and Mill Site are located in close proximity to one another, approximately 16 miles northeast of Gallup, in McKinley County, New Mexico. They are located on adjacent Sections approximately one-half mile apart. The sites are temporarily being treated as one facility for purposes of the RA. The combined site is referred to as the "Settlement Agreement Site" (SA Site).

Site History

The NECR mine is a historical uranium mine operated by United Nuclear Corporation (UNC). Mining development began in 1967 and ended in 1982. While the mine operated, it served as the principal mineral source for the UNC uranium mill. The uranium mill and its adjacent disposal cells make up the UNC Superfund Site (the "UNC Mill Site"). Remedial activities addressing source control and on-site surface reclamation are being implemented by General Electric/United Nuclear Corporation (GE/UNC) under the direction of the U.S. Nuclear Regulatory Commission (NRC), pursuant to the UNC facility's NRC license, and integrated with the US Environmental Protection Agency's (USEPA's) selected remedy for the groundwater.

The Tailings Disposal Area (TDA) is an unlined facility bounded by an embankment and subdivided by cross-dikes into three cells, which are identified as the South Cell, Central Cell, and North Cell. An estimated 3.5 million tons of tailings were pumped in slurry from the mill to the TDA.

Proposed Remedial Action

The proposed repository will be constructed on top of the existing TDA and will incorporate controlled placement of mine waste on top of the existing TDA cover/radon barrier and a final evapotranspirative (ET) cover placed over the mine waste. Improvements to the existing TDA cover/radon barrier within the footprint of the proposed repository will be completed prior to placement of mine waste. **Figure 1** shows the location and grading of the proposed repository.

The design for the selected repository alternative will be evaluated as part of a NRC license amendment request for the existing licensed facility. The repository features that affect the licensed facility will meet the performance standards outlined in NRC regulations and areas of the existing facility affected by the repository construction will be evaluated for compliance. However, existing conditions of the facility not affected by the proposed repository were not evaluated as part of this analysis, as they are managed by the existing NRC license.

Site Description

The natural stratigraphy at the Mill Site is divided into two main components: the surficial unconsolidated deposits (alluvium) and the underlying consolidated bedrock units. The alluvium consists of a mixture of sand, silt, and clay with minor portions of gravel. Alluvial thicknesses at the site are usually around 50 feet, but exceed 120 feet in some locations. Generally, the uppermost bedrock unit at the site is the Upper Gallup Sandstone, though in some locations it is overlain by coal or the Mancos shale.

The TDA was constructed on top of the native alluvium and deposition of tailings via slurry within the TDA resulted in an interbedded accumulation of tailings. TDA closure construction began in 1989 and was completed in 1995. Closure construction included placement of an interim cover (general fill) from 1989 through 1991 followed by placement of the final cover (radon barrier and erosion protection layer) from 1993 through 1995.

Measurements taken in alluvial monitoring wells show an alluvial groundwater table in the vicinity of the TDA at approximately 6,867 feet above mean sea level (amsl), which indicates that the alluvium is unsaturated above this elevation. Additionally, subsurface investigations of the TDA indicate that there is not a consistent static water level within the tailings or the alluvium above approximately 6,867 feet amsl. However, localized perched zones of saturation exist within the low-permeability, fine-grained tailings. These zones of saturation do not appear to extend beyond the fine-grained tailings into the higher-permeability coarse-grained tailings.

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Site Investigation

In 2013, MWH performed pre-design studies (PDS) at the Mill Site and Mine Site to supplement previous site investigations and collect data necessary to perform the Remedial Design (RD). Activities performed as part of the Mill Site PDS included: surveying, cone penetration tests (CPTs), drilling, standard penetration tests (SPTs), excavation and soil sampling, and subsequent laboratory testing. Geotechnical data collected during the PDS are presented in the PDS reports (MWH, 2014a and MWH, 2014b) and summarized in **Attachment A**. A list of the materials encountered within the TDA during the PDS is presented in the Assumptions section below. Geotechnical properties for these materials and discussion of one-dimensional stratigraphic profiles developed for the settlement analysis are also presented in the Assumptions section.

Applicable Codes and Standards

NAVFAC Section 7.1 (Department of the Navy, 1986)
 NUREG-1620 (NRC, 2003)

Methods

One-dimensional (1-D) consolidation analyses were conducted for the tailings and existing fill/cover material in the Mill Site repository footprint to estimate total potential future settlement of the tailings and existing fill/cover material after placement of the waste material and final cover. Data from CPT and borehole testing locations from the PDS (MWH, 2014a and MWH, 2014b) were used for the settlement analysis.

The CPT data, in combination with the borehole logs, were used to determine the thickness of the layers and contact locations between different materials encountered. Eight boreholes were drilled adjacent to eight of the CPT locations to correlate and verify the CPT data with direct observation of the materials encountered from the borehole logs. Seven of these paired locations, located within the repository footprint, were used to analyze and verify the CPT data. The relationships observed in the paired CPT/borehole locations were then used to analyze the data from the CPT-only locations. Vertical soil profiles were created from the CPT/borehole data for each of the 25 CPT locations within the repository and were used in the 1-D consolidation analyses.

Primary and secondary consolidation analyses were conducted for the fine-grained tailings layers under the assumption that the fine-grained tailings layers are saturated, although in several locations the fine-grained tailings are unsaturated. The coarse-grained tailings were excluded from the calculations because the coarse-grained tailings are unsaturated. The alluvium is unsaturated like most of the tailings and the stresses from the mine spoils and cover placement will not induce consolidation settlement in the unsaturated alluvium that will impact the repository. Therefore, the alluvium was also excluded from the calculations. Immediate settlement was calculated for the cover and fill materials to the base of coarse-grained tailings in areas where the repository cover slope will transition directly to the existing radon barrier (CPT-15, CPT-26, and CPT-01). Immediate settlement was estimated using the NAVFAC method (Department of the Navy, 1986). Immediate elastic compression is not expected to influence the performance of the final cover system because it will occur in very small increments as the TDA is being filled incrementally with mine spoils and cover material. Any settlement that does occur from immediate elastic compression will be corrected for by the placement of mine waste in lifts, and will not be noticeable once the final cover is placed.

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Material Properties and Stratigraphic Profiles

Material Properties

Material strength parameters used for the settlement analysis were based on parameters from laboratory testing of materials at the site during the PDS. The laboratory test results are located in Table 3-4 of the Mill Site PDS Report (MWH, 2014a). The parameters used as a base-case scenario for each material are discussed in the main text of Appendix G, Section G.6 and are summarized in **Table 1** included in this appendix. The CPT and borehole locations are shown on **Figure 1**.

The material properties used for the base case analysis were taken as the average of all samples for a given material and parameter. The compression index values, C_c , were also taken as an average of all samples for a given material. As discussed previously, primary and secondary consolidation analyses were conducted for only the fine-grained tailings layers. Therefore, the C_c value of the fine-grained tailings is the only C_c value applied in the settlement analysis.

A sensitivity analysis was conducted to determine the critical material property and evaluate the effects of varying this property on the total calculated settlement at each CPT location. The material index properties (dry density and water content) were varied from the base case (average) to the 30th percentile values. The compression index, C_c , was also varied from the base case value to the 60th percentile value. The total settlement is directly proportional to the C_c value, so increasing the C_c value results in an increased settlement. The critical material property for the settlement analysis was found to be the C_c value for the fine-grained tailings material. This property was varied to the 60th percentile value of 0.448 for the sensitivity analysis.

The material properties were also varied to include material properties from the Jetty borrow area in the overall average of each property (dry density, moisture content, void ratio, and specific gravity). An additional sensitivity analysis was conducted to determine how the results change when the Jetty soil properties are used. The total settlement at CPT-02 decreased from 0.2 to 0.1 feet. Therefore, the material properties from the PDS borrow areas excluding the Jetty soils were used in the analyses and results presented in this calculation brief.

Stratigraphic Profiles

Soil profiles were used to estimate the settlement at various locations within the repository footprint. CPT data combined with profiles from borehole logs were used to determine the thickness of the material layers (cover/fill, coarse- or fine-grained tailings, and alluvium), as well as the contact between the different layers in each soil profile. The cone resistance and electrical resistivity measurements were used to identify contact locations between the cover/fill material, coarse tailings, fine tailings, and underlying alluvium. The cone sleeve resistance, f_s , and resistivity were plotted with depth to identify a trend or correlation between material contacts. The material contacts were verified with the borehole logs for seven paired locations, and the correlations were then used for the remaining CPT-only locations. The CPT profile plots used for interpretation are included as **Attachment A**. The borehole logs used are included as **Attachment B**. The CPT logs and complete CPT report are included in the PDS report but are not attached here.

The paired borehole and CPT locations were used to evaluate CPT properties for comparison with visual classification from the borehole logs. The trends for sleeve resistance and resistivity were semi-quantitatively used to identify the layers of fine tailings, coarse tailings and alluvium, using the paired borehole/CPT locations of B1, B2, B8, B10, B11, B15, and B23.

The cover/fill material exhibited the highest cone resistances and resistivity values. The coarse tailings had similar resistivity values to the cover/fill material, but exhibited lower overall average cone resistance values. The cone resistance values for the cover/fill material typically was greater than 2.0 tons per square foot (tsf), whereas the cone resistance values for the coarse tailings material typically ranged from about 0.5 to 2.0 tsf. While these values denoted the typical range, the limits varied depending on the CPT data, typically reducing the range (i.e., 0.5 to 1.0 tsf). These limits for the coarse tailings cone resistance values are shown on each CPT profile by a vertical green line. The

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resistivity values of the cover/fill material and coarse tailings were typically in the range of 630 to 700 ohm-m, however some coarse tailings resistivity measurements were as low as zero.

The fine tailings were identified based on the combination of the sleeve resistances, f_s , of less than 0.5 tsf and low or zero resistivity measurements. The lower limit of the coarse tailings sleeve resistance limit shown by the vertical green line on the CPT profiles also denotes the upper limit of the fine tailings sleeve resistances. At CPT-1, sleeve resistances in the fines ranged up to 1.8 tsf, however the fines content of the fine-grained tailings at this location was on the order of 53 to 69 percent, which was generally less than the fines content for fine-grained tailings at other locations.

Sixteen of the 25 CPTs encountered fine-grained tailings in contact with the alluvium. Typically, sleeve resistance values increased from approximately 0.2-0.3 tsf in the fine-grained tailings to 0.7-1.0 tsf or greater in the underlying alluvial material. Resistivity increased from zero ohm-m in the fine-grained tailings to approximately 450-650 ohm-m in the alluvium. In five of the 25 CPTs (CPT-05, CPT-15, CPT-23, CPT-26, CPT-28), coarse-grained tailings were in contact with the underlying alluvial material. The sleeve resistance provided the most distinct pattern at the transition from coarse-grained tailings to the alluvium. The resistance increased from about 0.5-1.5 tsf in the coarse-grained tailings to 1.5-2.5 tsf in the alluvium. The remaining four CPT locations (CPT-13, CPT-24, CPT-25, and CPT-27) did not contain tailings material and encountered either cover/fill material and alluvium, or just alluvium.

Layer thicknesses were limited to a minimum depth of 2 feet to minimize the number of layers in each soil layer profile. To be conservative, profiles that showed potential lenses of small, intermittent coarse- and fine-grained tailings were assumed to be one layer of fine tailings. Fine-grained tailings were assumed to be fully saturated, with the exception of two locations (CPT-23 and CPT-29), and the compression index (C_c) values measured in the lab were used to estimate total primary consolidation. Profiles of CPT holes were interpreted based on the profiles from the paired CPT/boreholes locations and the locations within the TDA. Based on the CPT interpretation and boring logs, it was determined that the fine-grained tailings at locations CPT-23 (TI-B23) and CPT-29 are unsaturated and, therefore, settlement will not occur at these two locations.

Stress increases for the settlement calculations are based on the repository cover thickness, and the proposed thickness of the mine spoils at each specific location, determined from the design grading plan.

Settlement totals include primary and secondary consolidation in the fine-grained tailings. These calculations excluded the coarse-grained tailings and alluvium. Immediate settlement included settlement totals for the radon barrier material and coarse-grained tailings (the unsaturated materials).

Calculations

Primary Consolidation

Settlement of the fine-grained tailings resulting from fill placement of the mine waste and cover is assumed to initiate as loading begins and progress to completion as the porewater pressures within the fine-grained tailings dissipate. Primary consolidation was calculated using the following equation:

$$S = \frac{C_c H}{1 + e_0} \log \frac{\sigma'_f}{\sigma'_i}$$

where:

S = settlement (ft)

C_c = compression index

H = thickness of tailings layer (ft)

e_0 = initial void ratio of tailings

σ'_i = initial average effective overburden pressure (psf)

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σ'_f = final effective vertical pressure (psf)

The total estimated primary consolidation was calculated for the fine-grained tailings layer in each CPT vertical soil profile location.

Secondary Consolidation

Additional settlement is expected to occur due to creep (secondary consolidation). For secondary consolidation, the secondary consolidation coefficient, C_α , was estimated from a time-compression curve. This curve was plotted using laboratory test data from consolidation tests performed on fine-grained tailings at borehole locations TI-B8, TI-B10, and TI-B11 (these plots will be included in the Pre-final Design). The elapsed time (log time) and dial reading data for each load step were plotted in excel to obtain the time-compression curve, and C_α was estimated by the slope of the final portion of the curve. The secondary consolidation was then calculated at each CPT location using the following equation:

$$S_s = C_\alpha * H * \log\left(\frac{t_2}{t_1}\right)$$

where:

- S_s = secondary settlement (ft)
- C_α = secondary consolidation coefficient
- H = layer thickness of the fine-grained tailings (ft)
- t_2 = time from t_1 (yrs)
- t_1 = time to complete primary consolidation (yrs)

The consolidation tests were performed at four load increments, and the C_α value was calculated for each of the four load increments in each consolidation test. The average C_α values were then calculated from each similar load increment from the consolidation tests at different locations. The C_α value used in the settlement calculations depended on the final effective stress at the mid-point of the fine-grained tailings layer at each CPT location. The average calculated C_α values and the range of stresses to which they apply are shown in **Table 2**.

The time to complete primary consolidation, t_1 , was estimated using data in the Central Cell Interim Stabilization As-Built Report (Canonie, 1992). Canonie recorded settlement data prior to construction activities and continued monitoring for several months after completing the interim soil cover construction, totaling 280 days of monitoring. Canonie reported that the data showed that 90 percent of primary consolidation had occurred at the end of 280 days of monitoring. The data suggests that monitoring was terminated too soon to determine when primary consolidation would have been complete. Stantec estimates from graphical interpretation using the Square-Root-of-Time Method that primary consolidation would have been completed by approximately 458 days. A second calculation was conducted using the time rate of consolidation theory, which estimates that consolidation would have been completed by 588 days. These calculations can be found in **Attachment E**. Since the data from the previous reclamation cover construction indicates that primary consolidation would have lasted more than 280 days, this settlement analysis assumed 280 days as a conservative estimate. It is anticipated that the tailings will complete the new primary consolidation in a shorter duration than previously since the material has been consolidating for nearly 30 years already and the new fill load will be significantly larger than the fill place previously for the current cover system. The secondary consolidation was calculated at a time (t_2) of 200 years from the completion of primary consolidation. The primary and secondary consolidation spreadsheet calculations are included in **Attachment C**.

Immediate Settlement

Immediate settlement of the unsaturated upper layers of the TDA beneath the repository will occur while the waste is placed and compacted. Therefore immediate settlement is not anticipated to contribute to the long-term performance of the new repository cover. However, immediate settlement was estimated in order to evaluate the potential for cover

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cracking of the existing radon barrier in locations where fill will be placed adjacent to areas of the radon barrier where fill will not be placed. Immediate settlement was estimated for the existing unsaturated cover, fill, and tailings, near areas where the repository cover slope will transition directly to the existing radon barrier. This was done for three locations near the southwest edge of the repository (B15/CPT-15, CPT-26, and B01/CPT-01). Around the perimeter of the repository, in the remainder of the areas, the cover will extend either to an existing swale or channel, or beyond the edge of the radon barrier. The estimate of immediate settlement was completed using a similar approach to that described in NAVFAC (Department of the Navy, 1986). The immediate settlement is estimated by the following equation:

$$\delta_v = \Delta q * B * \frac{(1 - \nu^2)}{E} * I$$

where:

- Δq = applied uniform pressure, kips per square foot (ksf)
- B = width of the loaded area (ft)
- I = combined shape and rigidity factor
- ν = Poisson's ratio
- E = modulus of elasticity (ksf)

The proposed thickness of the repository mine waste and cover materials were used to estimate Δq at each of the three selected CPT locations. The southwest slope of the repository where the three CPT holes are located was divided into three 250-foot sections (by width). The width of the loaded area, B , was assumed to be 250 feet for each of the calculations since the three borings were drilled at approximately equidistant spacing along the entire 750 feet of fill slope in contact with the existing cover. Therefore, the slope was divided into thirds and the profiles from each borehole or CPT were applied to a 250 foot section. The three borings used to complete the calculations were B15/CPT-15, CPT-26, and B01/CPT-01.

The vertical soil profiles were used to determine the thicknesses of each layer to the base of the coarse-grained tailings material. The CPT cone resistance (q_c) data with depth was used to estimate the modulus of elasticity, E , for the profiles. A correlation between E and q_c (McCarthy, 1998) was applied to each layer material, depending on the soil type. An average E value was then estimated for each material type based on the CPT. Published values of E by material type were also to calculate a weighted average of E for the profile. These weighted averages were compared with the values calculated from the cone resistances and the lower E values were selected for the settlement calculation, to present a more conservative total result.

The value of ν was estimated for each layer using a typical range of values from (McCarthy, 1998), and was based on both soil type and the estimated E value. The shape and rigidity factor, I , was taken from the NAVFAC table (Table 1, Section 7.1), assuming a circular (flexible) shape and rigidity, calculated on the edge of the shape. This is based on the locations of interest being on the edge of the repository. This approach and selection of the I value is assumed to be conservative since the shape of the fill is gently sloping and the shape factor in the references is for a circular vertical load (such as a tank) which would concentrate more load at the perimeter than the repository does.

The immediate settlement spreadsheet calculations are included in **Attachment D**.

Results

A settlement analysis was conducted to estimate future settlement after placement of the mine waste and final cover. The total settlement estimates ranged in value from 0.00 feet to 1.8 feet, by location. The primary and secondary consolidation at each CPT location is summarized in **Table 3**. The post-settlement surface is attached on **Figure 1**. The spreadsheet calculations of the settlement analysis are provided in **Attachment C**.

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The sensitivity analysis conducted on the settlement analysis resulted in a slight increase in settlement values when using the 60th percentile value for the fine-grained tailings C_c . The total settlement estimates ranged in value from 0.0 feet to 1.9 feet, by location. These results show that varying the critical property, C_c of the fine-grained tailings, had little effect on the calculated total settlement values. The primary and secondary consolidation at each location resulting from the sensitivity analysis are summarized in **Table 4**.

The immediate settlement calculations estimated 1.0 feet at CPT-15, 0.6 feet at CPT-26, and 0.1 feet at CPT-01.

Conclusions

Evaluation of total long-term settlement due to waste and final cover placement indicates potential future settlement of the cover ranging from 0 to 1.8 feet. The estimated settlement of the cover is small enough that slope reversal and ponding are not expected to occur on the final cover surface. **Figure 1** shows the post-settlement surface and **Figure 2** shows cross-sections confirming that slope reversal and ponding are not expected to occur.

Attachments

Figure 1 – Estimated Consolidation and Settled Isopach Contours
 Figure 2 – Settled Surface Cross-Sections

Attachment A – Paired borehole logs
 Attachment B – CPT profile interpretations
 Attachment C – Consolidation spreadsheet calculations
 Attachment D – Immediate settlement calculations
 Attachment E – Estimated time to complete primary consolidation

References

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TABLES

Table 1: Material Properties

| Material | Specific Gravity, G_s | Dry Unit Weight, γ_d (pcf) | Moist Unit Weight, γ_m (pcf) | Water Content, w (%) | Void Ratio, e | Compression Index, C_c |
|-------------------------|---|---|---|--|-----------------------------------|--|
| Coarse-grained tailings | 2.67 | 97.5 | 108.1 | 10.9 | 0.71 | 0.084 |
| Fine-grained tailings | 2.70 | 71.7 | 107.6 | 50.1 | 1.35 | 0.408 |
| Cover soil | 2.69 | 115.0 | 114.7 ¹ | 10.8 | 0.62 ² | 0.086 |
| Mine spoils | 2.66 | 118.3 | 116.4 ¹ | 9.3 | 0.56 ² | 0.086 |
| Erosion protection | 2.71 | 130.0 | 122.9 | 5.0 | 0.45 ² | NA ³ |

¹ Moist unit weights for cover soil, mine waste, and erosion protection materials were calculated at 90 percent relative compaction, $\gamma_{m90}=0.9(\gamma_d+\gamma_d*w)$

² Void ratio for cover soil, mine waste, and erosion protection materials were calculated at 90 percent relative compaction, $e_{90}=\{[1-(0.9*\gamma_d/G_s*\gamma_w)]/(0.9*\gamma_d/G_s*\gamma_w)\}$

³It was assumed the erosion protection layer will not settle, and assuming a compression index value for this material was unnecessary.

Table 2: Secondary Consolidation Coefficient Values for Load Increments

| Effective Stress | C_α |
|-------------------------|------------------------------|
| <1650 psf | 0.0017 |
| 1650 – 3250 psf | 0.0036 |
| 3250 – 6500 psf | 0.0047 |
| 6500 – 13000 psf | 0.0087 |

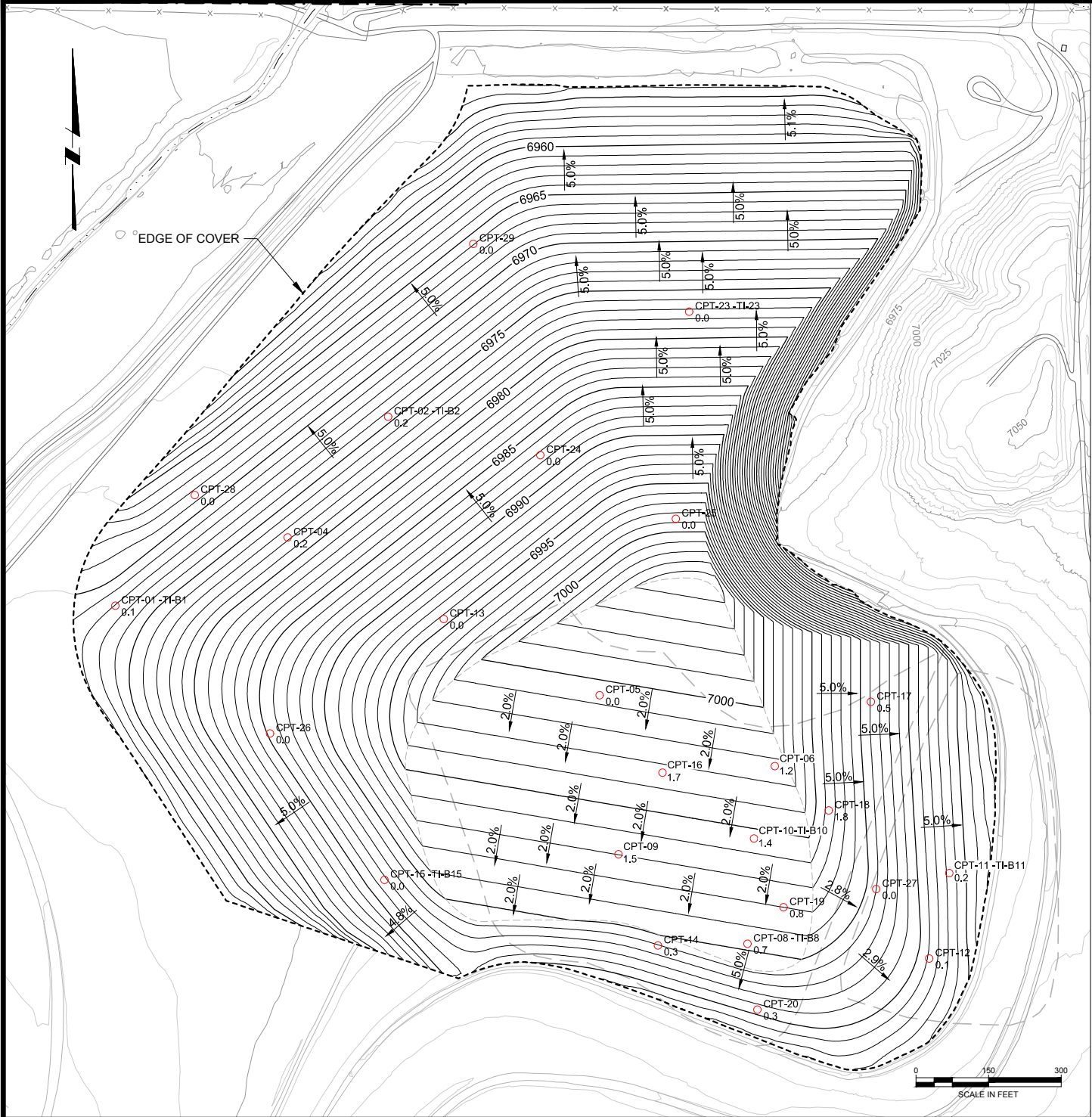
Table 3: Settlement Summary

| Location | Primary Consolidation (ft) | Secondary Consolidation (ft) | Total Settlement (ft) |
|-----------------|-----------------------------------|-------------------------------------|------------------------------|
| CPT-01 (TI-B1) | 0.02 | 0.05 | 0.1 |
| CPT-02 (TI-B2) | 0.13 | 0.03 | 0.2 |
| CPT-04 | 0.17 | 0.04 | 0.2 |
| CPT-05 | 0.00 | 0.00 | 0.0 |
| CPT-06 | 1.03 | 0.22 | 1.2 |
| CPT-08 (TI-B8) | 0.51 | 0.21 | 0.7 |
| CPT-09 | 1.13 | 0.34 | 1.5 |
| CPT-10 (TI-B10) | 1.08 | 0.29 | 1.4 |
| CPT-11 (TI-B11) | 0.08 | 0.14 | 0.2 |
| CPT-12 | 0.02 | 0.03 | 0.1 |
| CPT-13 | 0.00 | 0.00 | 0.0 |
| CPT-14 | 0.20 | 0.05 | 0.3 |
| CPT-15 (TI-B15) | 0.00 | 0.00 | 0.0 |
| CPT-16 | 1.34 | 0.32 | 1.7 |
| CPT-17 | 0.40 | 0.09 | 0.5 |
| CPT-18 | 1.45 | 0.35 | 1.8 |
| CPT-19 | 0.59 | 0.21 | 0.8 |
| CPT-20 | 0.19 | 0.12 | 0.3 |
| CPT-23 (TI-23) | 0.00 | 0.00 | 0.0 |
| CPT-24 | 0.00 | 0.00 | 0.0 |
| CPT-25 | 0.00 | 0.00 | 0.0 |
| CPT-26 | 0.00 | 0.00 | 0.0 |
| CPT-27 | 0.00 | 0.00 | 0.0 |
| CPT-28 | 0.00 | 0.00 | 0.0 |
| CPT-29 | 0.00 | 0.00 | 0.0 |

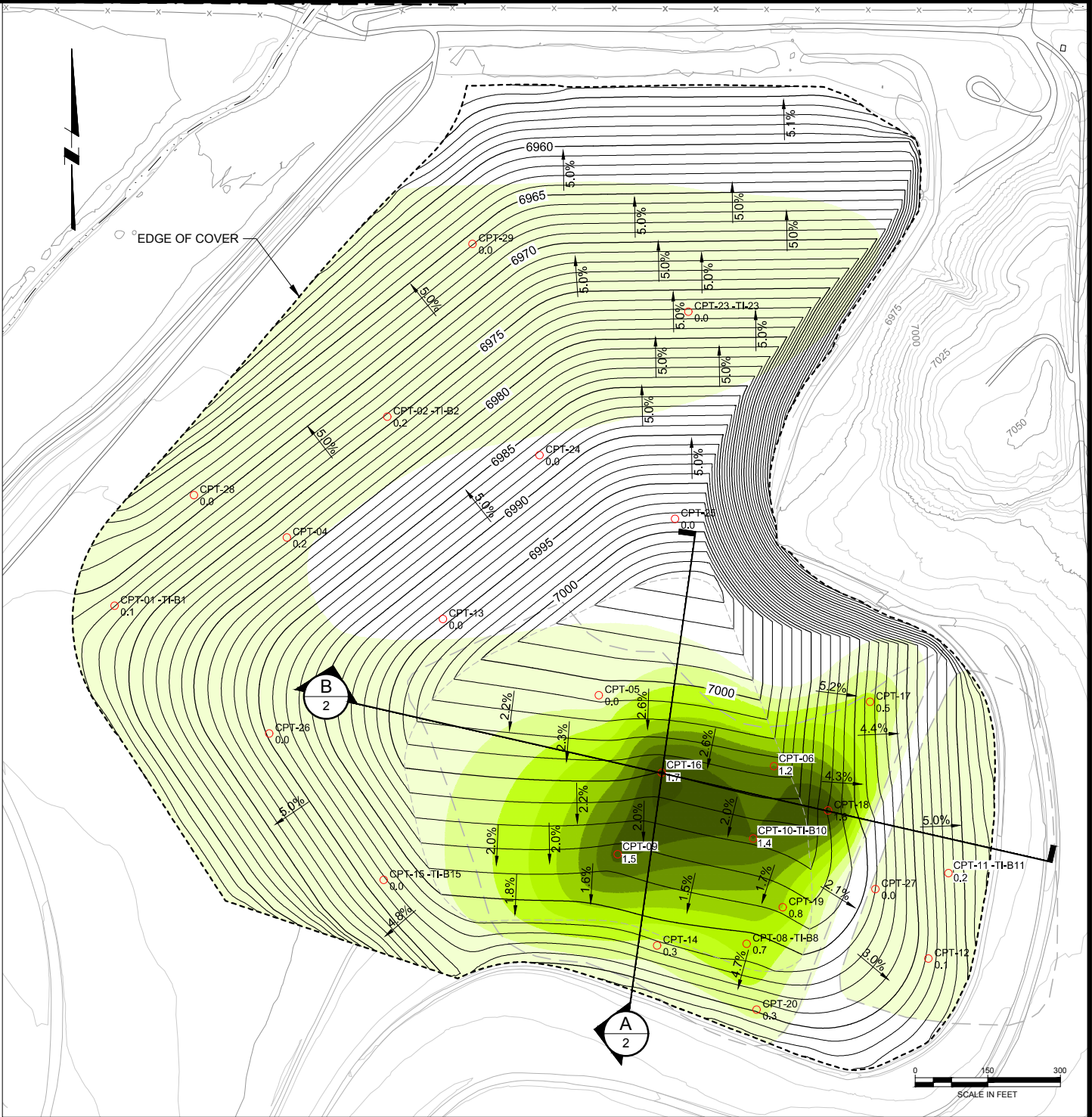
Table 4: Sensitivity Analysis Settlement Summary

| Location | Primary Consolidation (ft) | Secondary Consolidation (ft) | Total Settlement (ft) |
|-----------------|-----------------------------------|-------------------------------------|------------------------------|
| CPT-01 (TI-B1) | 0.03 | 0.05 | 0.1 |
| CPT-02 (TI-B2) | 0.14 | 0.03 | 0.2 |
| CPT-04 | 0.19 | 0.04 | 0.2 |
| CPT-05 | 0.00 | 0.00 | 0.0 |
| CPT-06 | 1.13 | 0.22 | 1.3 |
| CPT-08 (TI-B8) | 0.56 | 0.21 | 0.8 |
| CPT-09 | 1.24 | 0.34 | 1.6 |
| CPT-10 (TI-B10) | 1.18 | 0.29 | 1.5 |
| CPT-11 (TI-B11) | 0.09 | 0.14 | 0.2 |
| CPT-12 | 0.02 | 0.03 | 0.1 |
| CPT-13 | 0.00 | 0.00 | 0.0 |
| CPT-14 | 0.22 | 0.05 | 0.3 |
| CPT-15 (TI-B15) | 0.00 | 0.00 | 0.0 |
| CPT-16 | 1.47 | 0.32 | 1.8 |
| CPT-17 | 0.44 | 0.09 | 0.5 |
| CPT-18 | 1.59 | 0.35 | 1.9 |
| CPT-19 | 0.64 | 0.21 | 0.9 |
| CPT-20 | 0.21 | 0.12 | 0.3 |
| CPT-23 (TI-23) | 0.00 | 0.00 | 0.0 |
| CPT-24 | 0.00 | 0.00 | 0.0 |
| CPT-25 | 0.00 | 0.00 | 0.0 |
| CPT-26 | 0.00 | 0.00 | 0.0 |
| CPT-27 | 0.00 | 0.00 | 0.0 |
| CPT-28 | 0.00 | 0.00 | 0.0 |
| CPT-29 | 0.00 | 0.00 | 0.0 |

FIGURES



DESIGN CONTOURS AND TOTAL ESTIMATED CONSOLIDATION SETTLEMENT



CONSOLIDATION SETTLEMENT AND SETTLED SURFACE CONTOURS

LEGEND:

- 7200 EXISTING GROUND SURFACE CONTOUR & ELEVATION, FEET
- 7200 PROPOSED SURFACE CONTOUR & ELEVATION, FEET
- EXISTING ROAD
- EXISTING DRAINAGE
- FENCE
- BOUNDARY OF REPOSITORY
- BORROW PIT BOUNDARY
- CPT-26 1.8 ESTIMATED TOTAL SETTLEMENT (FT)

| SETTLEMENT DEPTHS | | | |
|-------------------|----------|----------|-------|
| Number | MIN (FT) | MAX (FT) | Color |
| 1 | 0.0 | 0.2 | |
| 2 | 0.2 | 0.4 | |
| 3 | 0.4 | 0.6 | |
| 4 | 0.6 | 0.8 | |
| 5 | 0.8 | 1.0 | |
| 6 | 1.0 | 1.2 | |
| 7 | 1.2 | 1.4 | |
| 8 | 1.4 | 1.6 | |
| 9 | 1.6 | 1.8 | |
| 10 | 1.8 | 2.0 | |

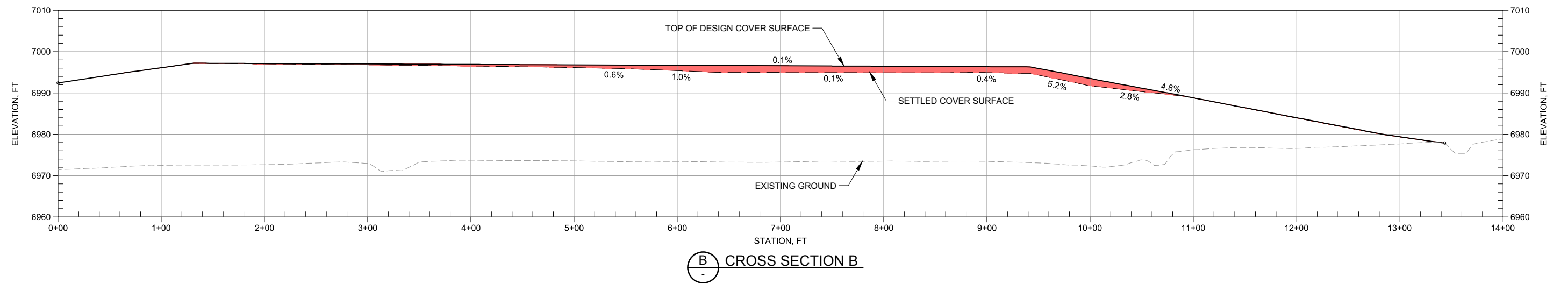
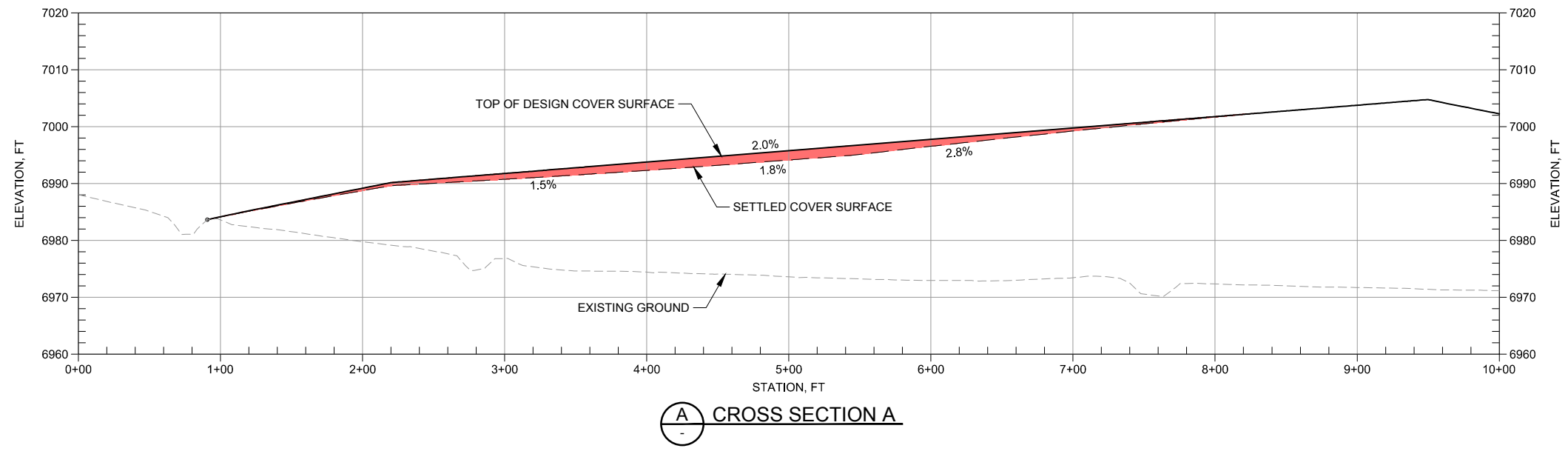
- NOTE(S):
- THE ESTIMATED SETTLEMENT TOTALS INCLUDE PRIMARY CONSOLIDATION AND SECONDARY CONSOLIDATION IN THE FINE-GRAINED TAILINGS.

DESIGNED S.DOWNEY
CHECKED C.FOWLER
APPROVED S.DOWNEY



UNITED NUCLEAR CORPORATION AND NORTHEAST CHURCH ROCK MINE
MCKINLEY COUNTY, NEW MEXICO
TOTAL ESTIMATED CONSOLIDATION SETTLEMENT

p:\omerve\win103\mwhglobal.com\AM_PROJECTS\202\Documents\General Electric\GE_NECR_Design\Civil\Figures\2016-3-30_REPO\REPOSITORY_COVER_NECR_REPO\ESTIMATED_SETTLEMENT_3.dwg LAYOUT: Xsect PLOT DATE: 7/7/2017 2:28 PM BY: KIREED



LEGEND:

AREAS OF SETTLEMENT





DESIGNED _S.DOWNEY
CHECKED _C.FOWLER
APPROVED _S.DOWNEY



UNITED NUCLEAR CORPORATION AND NORTHEAST CHURCH ROCK MINE
McKINLEY COUNTY, NEW MEXICO
TOTAL ESTIMATED CONSOLIDATION SETTLEMENT
CROSS SECTIONS




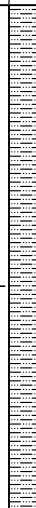

FIGURE
2
10508639
JULY, 2017

ATTACHMENT A
PAIRED BOREHOLE LOGS

| | | | | | | | |
|---|------------------|---|------------|-----------------------------|-----------------|--|--|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B1 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6969.7 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | FINISH: 11/21/2013 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | DEPTH TO BEDROCK (FT): N/A | |
| | | | | | | TOTAL DEPTH (FT): 70.0 | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS GRAPHIC |
| 14" | | | | | NA | (0' - 8") SILTY CLAY (FILL) - Light brown, soft, moist silty clay, trace to few very fine to fine sand. |  |
| 1 | | | | | | (8" - 12") ROCK - 1/2" to 3" crushed basalt. | |
| 2 | | | | | | (1' - 18.5') SILTY CLAY WITH SAND (FILL) - Dark brown, firm to hard, slightly moist silty clay, little to some very fine to fine sand, occasional coarse sand and gravel (upper ~5' may be compacted radon barrier). | |
| 3 | | | | | | [0 - 5' Core not retained.] | |
| 5 | 24" | CA 18" | 1C | 8 | | | |
| 6 | | | 1B | 9 | | | |
| 7 | | AC | 2 | 11 | | | |
| 8 | | | | | | | |
| 9 | | | | | | | |
| 10 | 30" | CA 18" | 3C | 10 | | [Below ~10', occasional elevated rad readings indicating possible sand tailings mixed with silty clay fill.] | |
| 11 | | | 3B | 12 | | | |
| 12 | | | 3A | 14 | | (~11' - ~11.5') 1/2" to 1" gravel observed. | |
| 13 | | AC | 4 | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY







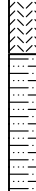
NOTES:
 Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|--|---|------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 30" | AC | 4 | | | NA | (~13' - ~14') - Sand Tailings - see description below, slightly elevated rad readings.] | CL |  | 16.2 | 104.7 | | 33/13/20 | 0.3 | 27.2 | 72.5 | | | |
| 14 | | | | | | | | | | | | | | | | | | |
| 15 | 32" | CA 18" | 5C | 5 | | | | | | | | | | | | | | |
| | | | 5B | 6 | | | | | | | | | | | | | | |
| 16 | | | 5A | 10 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| 17 | AC | 6 | | | | | | | | | | | | | | | | |
| 18 | | | | | | | | | | | | | | | | | | |
| 19 | | | | | | | | | | | | | | | | | | |
| 20 | 36" | CA 18" | 7C | 19 | | | | | | | | | | | | | | |
| | | | 7B | 22 | | | | | | | | | | | | | | |
| 21 | | | 7A | 24 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| 22 | AC | 8 | | | (18.5' - 34.3') SAND TAILINGS - Pale gray, dense, moist, fine to medium sand tailings, slightly elevated rad levels. |  | | 6.1 | | | | | | | | | | |
| 23 | | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | | |
| 25 | 30" | ST 28" | | 9 | | | | | | | | | | | | | | |
| 26 | | | | | | | | | | | | | | | | | | |
| 27 | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | AC | 10 | | | | | | | | | | | | | | | | |

LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:
Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.




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| | | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|--|--------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|-------|------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | | |
| 28 | 30" | AC | 10 | NA | (29.6' - 34.3') Very fine grained sand tailings, abundant clayey zones. | CL |  | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | | | |
| 30 | 36" | CA 18" | 11C | 5 | | | | 13.9 | | | | | | | | | | | |
| | | | 11B | 5 | | | | 14.6 | 91.6 | | | | | | | | 3.0E-7 | 0.092 | |
| 31 | | | 11A | 4 | | | | 41.6 | 76.5 | 2.69 | 44/17/27 | 0.0 | 30.9 | 69.1 | | | | | 33.3 |
| 32 | | AC | 12 | | (34.3' - ~41') CLAYEY SAND - Loose to medium dense, very moist to wet, very fine-grained clayey sand, increasing clay content with depth. | CL |  | 27.8 | | | 33/16/17 | 0.0 | 46.7 | 53.3 | | | | | |
| 33 | | | | | | | | | | | | | | | | | | | |
| 34 | | | | | | | | | | | | | | | | | | | |
| 35 | 40" | CA 18" | 13C | 7 | | | | | | | | | | | | | | | |
| | | | 13B | 20 | | | | | | | | | | | | | | | |
| 36 | | | 13A | 22 | (35' - 36.5') High blow counts due to rock in CA shoe. | |  | | | | | | | | | | | | |
| 37 | | AC | 14B | | | | | | | | | | | | | | | | |
| 38 | | AC | 14A | | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | | | |
| 40 | 42" | CA 18" | 15C | 3 | | | | | | | | | | | | | | | |
| | | | 15B | 5 | (41' - ~45') SANDY CLAY - Very moist to wet sandy clay, sand is very fine-grained. | CL |  | | | | | | | | | | | | |
| 41 | | | 15A | 5 | | | | | | | | | | | | | | | |
| 42 | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |




LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:
Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

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| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|---------|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42 | 42" | AC | 16 | NA | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 40" | ST 30" | 17 | | (~45' - 54') CLAYEY SAND - Loose to medium dense, very moist to wet, very fine-grained clayey sand, decreasing clay content with depth. | | | 22.0 | 106.0 | | | | | | | .058 | 34.4 |
| 46 | | | | | | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 42" | CA 18" | 18C | 4 | | | | | | | | | | | | | |
| | | | 18B | 5 | | | | | | | | | | | | | |
| 51 | | | 18A | 7 | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | (54' - 68.2') SILTY CLAY - Dark brown, firm, very moist to wet silty clay, trace to few very fine sand. | | | | | | | | | | | | |
| 55 | 60" | CA 18" | 19B | 7 | | | | | | | | | | | | | |
| 56 | | | 19A | 10 | | | | | | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | |
| LEGEND: | | | | | NOTES: | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | |
| Page 4 of 5 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|---|------------------|---|------------|-----------------|---|------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 60' | | | | NA | | | | | | | | | | | | | |
| 58' | | | | | | | | | | | | | | | | | |
| 59' | | | | | | | | | | | | | | | | | |
| 60' | 60' | CA 18' | 5 | | | | | | | | | | | | | | |
| | | 20B | 8 | | | | | | | | | | | | | | |
| 61' | | 20A | 12 | | | | | | | | | | | | | | |
| 62' | | | | | | | | | | | | | | | | | |
| 63' | | | | | | | | | | | | | | | | | |
| 64' | | | | | | | | | | | | | | | | | |
| 65' | 60' | CA 18' | 5 | | | | | | | | | | | | | | |
| | | 21B | 7 | | | | | | | | | | | | | | |
| 66' | | 21A | 11 | | | | | | | | | | | | | | |
| 67' | | | | | | | | | | | | | | | | | |
| 68' | | | | | | | | | | | | | | | | | |
| 69' | | | | | (68.2' - E.O.B.) SILTY SAND - Brown, silty, moist very fine to fine sand. | | | | | | | | | | | | |
| 70' | | | | | E.O.B. at 70.0' | | | | | | | | | | | | |
| 71' | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 5 of 5 | | | | | | | | | | | | | | | | | |

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|--|------------------|------------------|------------|--|-----------------|---|------------|-----------------------------|-----------------|--------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B2 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | | DRILL RIG INFORMATION | | | | BOREHOLE INFORMATION | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/20/2013 | | | | | | | | |
| DRILLER: M. CAIN | | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6959.9 | | FINISH: 11/21/2013 | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 33.5 | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 38.7 | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 8" | | | | | | (0" - 3") SILTY CLAY - Brown, soft, silty clay, dry, trace few very fine to fine sand. | | | | | | | | | | | | |
| 1 | CA 17" | | | 13 | | (3" - 6") ROCK - 1/2" to 3" crushed basalt. | | | | | | | | | | | | |
| | | 1B | | 11 | | (6" - ~2.5') SILTY CLAY - Dark brown, hard, slightly moist silty clay, few to little very fine sand, occasional coarse sand. | | | | | | | | | | | | |
| 2 | | 1A | | 18 | | | | | | | | | | | | | | |
| | | | | | | [0' - 5' Core not retained.] | | | | | | | | | | | | |
| 3 | | | | | | (~2.5' - 7.3') SILTY SAND WITH GRAVEL - Pale yellow, dense, dry to slightly moist silty very fine to medium sand with coarse sand and gravel. | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 42" | CA 15" | | 23 | | | | | | | | | | | | | | |
| | | 2B | | 22 | | | | | | | | | | | | | | |
| 6 | | 2A | | 18 | | | | | 7.7 | 100.4 | 2.68 | | 26.9 | 29.9 | 43.2 | | | |
| | | | | | | | | | | | | | | | | | | |
| 7 | | AC | | 3 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| 8 | | AC | | 4 | | (7.3' - 12.8') CLAYEY SAND - Predominantly brown, medium dense, slightly moist to moist clayey fine to medium sand, abundant clay "pockets" throughout. May be sand tailings mixed with site soils. | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 40" | CA 18" | | 6 | | | | | | | | | | | | | | |
| | | 5B | | 7 | | | | | | | | | | | | | | |
| 11 | | 5A | | 7 | | | | | 24.5 | 75.9 | 2.73 | | 0.0 | 65.4 | 34.6 | | | |
| | | | | | | | | | | | | | | | | | | |
| 12 | | AC | | 6 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| 13 | | AC | | 7 | | (12.8' - 15') SAND TAILINGS - Pale gray, moist sand | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




































NOTES:

Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.
At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation).

Page 1 of 3

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY




NOTES:
 Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.
 At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation).

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B2 | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 14-15 | 40" | AC 7 | | | tailings, very fine silty sand from 12.8' to 13.2', fine to medium sand from 13.2' to 15'. | |  | 39.6 | | | | 0.0 | 23.1 | 76.9 | | | |
| 15-16 | 42" | CA 18" 8C | 5 | | (15' - 25.7') SILTY SAND - Brown, medium dense, moist silty very fine to fine sand, occasional roots. Appears to be natural "alluvium." Occasional dark brown clay lenses. Rad levels ~ background. | |  | 6.9 | 90.4 | 2.68 | | | | | | | |
| | | 8B | 5 | | | |  | | | | | | | | | | |
| 16-17 | | 8A | 7 | | | |  | | | | | | | | | | |
| 17-18 | | AC 9 | | | | |  | | | | | | | | | | |
| 18-19 | | AC 10 | | | | |  | | | | | | | | | | |
| 20-21 | 42" | CA 18" 11C | 4 | | | |  | | | | | | | | | | |
| | | 11B | 4 | | | |  | | | | | | | | | | |
| 21-22 | | 11A | 6 | | | |  | | | | | | | | | | |
| 22-23 | | AC 12 | | | | |  | | | | | | | | | | |
| 23-24 | | AC 13 | | | | |  | | | | | | | | | | |
| 25-26 | 48" | CA 18" 14C | 5 | | (25.7' - 33.5') SILTY CLAY - Dark brown, moist, firm to hard, silty clay, trace to few very fine to fine sand, occasional coarse sand. | |  | | | | | | | | | | |
| | | 14B | 6 | | | |  | | | | | | | | | | |
| 26-27 | | 14A | 6 | | | CL |  | 23.5 | 93.2 | | 34/16/18 | 0.0 | 20.9 | 79.1 | | | |
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LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:
Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.
At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation).

Page 2 of 3

|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B2 | | | | | | | | | | | |
|---|------------------|---|------------|----------------------|---|------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28-48" | | | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 54" | CA 18" | 15C | 6 | | | | | | | | | | | | | |
| | | | 15B | 11 | | | | | | | | | | | | | |
| 31 | | | 15A | 12 | | | | | | | | | | | | | |
| 32 | | | | | (32' - 33.5') Softer (soft to firm). | | | | | | | | | | | | |
| 33 | | | | | | | | | | | | | | | | | |
| 34 | | | | | (33.5' - 38.7') WEATHERED SANDSTONE - Mottled pale yellow and reddish orange, moist, fissile, lightly cemented, very fine to fine sand. | | | | | | | | | | | | |
| 35 | 48" | NR | | 50/1" | | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | |
| 39 | | | | 16 | Bag sample of SS Core. | | | 13.5 | X | | | | | | | | |
| 40 | | | | | E.O.B. = 38.7' (Practical Auger Refusal) | | | | | | | | | | | | |
| 41 | | | | | | | | | | | | | | | | | |
| 42 | | | | | | | | | | | | | | | | | |




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
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 At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation).

| | | | | | | | |
|---|------------------|---|------------|-----------------------------|-----------------|--|------------------------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B3 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6968.6 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | FINISH: 11/19/2013 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | DEPTH TO BEDROCK (FT): N/A | |
| | | | | TOTAL DEPTH (FT): 70.0 | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS GRAPHIC |
| 31" | | | | | | (0' - 0.8') SANDY CLAY - Brown, hard, slightly moist sandy clay, silty, sand is fine-grained. | |
| 1 | | | | | | (0.8' - 10.8') GRAVELLY SAND - Pale yellow, dense, slightly moist gravelly very fine to medium sand. | |
| 2 | | | | | | | |
| 3 | | | | | | | |
| 4 | | | | | | | |
| 5 | 33" | CA 7" | | 18 | | [Sample loose - not retained.] | |
| 6 | | | | 50/6" | | | |
| 7 | | | | | | | |
| 8 | | | | | | | |
| 9 | | | | | | | |
| 10 | 50" | CA 18" | 1C | 30 | | | |
| | | | 1B | 34 | | | |
| 11 | | | 1A | 43 | | (10.8' - 16.8') SILTY SAND - Yellow/orange, dense, moist very fine to fine sand, silty, occasional gravel. | |
| 12 | | | | | | | |
| 13 | | | | | | | |
| | | | | | | | 5.1 108.4 2.64 5.4 74.7 19.9 |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|--|------------|---------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 50" | | | | | | | | | | | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | | |
| 15 | 38" | CA 18" | 2C | 18 | | | | | | | | | | | | | | |
| | | | 2B | 21 | | | | | | | | | | | | | | |
| 16 | | | 2A | 28 | | | | | 4.7 | 105.3 | | | | | | | | |
| 17 | | | | | | (16.8' - 46.5') SANDY CLAY - Dark brown, firm to hard, moist sandy clay, very fine to fine sand. | | | | | | | | | | | | |
| 18 | | | | | | | | | | | | | | | | | | |
| 19 | | | | | | | | | | | | | | | | | | |
| 20 | 50" | ST 28.5' | | 3 | | | | | | | | | | | | | | |
| 21 | | | | | | | CL | | 16.0 | 111.1 | | 30/12/18 | 0.0 | 32.8 | 67.2 | | | 32.2, 195 |
| 22 | | | | | | | | | | | | | | | | | | |
| 23 | | | | | | (22.6' - 26') More sand and gravel. | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | | |
| 25 | 52" | CA 18" | 4C | 10 | | | | | | | | | | | | | | |
| | | | 4B | 12 | | | | | | | | | | | | | | |
| 26 | | | 4A | 16 | | | CL | | 12.0 | 106.8 | | 25/13/12 | | | | | | |
| 27 | | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:




Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 2 of 5

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
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 CC = CONTINUOUS CORE
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NOTES:
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Page 2 of 5

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|--|--|---------------------------------------|--|--|--|----------------------|--|---------------------------|--|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B3 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| | | MATERIAL DESCRIPTION | | | | | | | |
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LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 3 of 5

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|--|------------------|---------------------------------------|------------|--|---|----------------------|---------|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 57" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 48" | CA 17" | 6 | | | | | | | | | | | | | | |
| | | 8B | 7 | | | | | 17.0 | 110.1 | | | | | | | | |
| 46 | | 8A | 12 | | | | | 18.0 | 104.8 | | 28/13/15 | | | | | | 29.3, 293 |
| 47 | | | | | (46.5' - ~55') SILTY/CLAYEY SAND - Brown, loose, very moist to wet, silty/clayey very fine sand. | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 27" | CA 17" | 9C | 2 | ["B" and "C" samples are best.] | | | | | | | | | | | | |
| | | 9B | 3 | | | | | | | | | | | | | | |
| 51 | | 9A | 6 | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | |
| 55 | 30" | ST 24" | 10 | | (~55' - 57.3') SILTY CLAY - Dark brown, firm to hard, wet silty clay, few to little very fine sand. | | | | | | | | | | | | |
| 56 | | | | | | CL | | 22.1 | 105.3 | 2.72 | 43/14/29 | 0.0 | 11.7 | 88.3 | | | 22.2, 494 |
| 57 | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




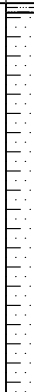



NOTES:

Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 4 of 5

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
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NOTES:
 Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

| | | | | | | | | | | | | | | | | | |
|---|------------------|---|------------|-----------------|--|------------------------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 30' | | | | | (57.3' - 61.9') SILTY SAND - Brown, loose, wet, silty very fine to fine sand. | |  | 25.8 | 99.0 | | | 0.0 | 22.0 | 78.0 | | | |
| 58 | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | |
| 60 | 33" | CA 18" 11C 11B 11A | 3 5 4 | | | | | | | | | | | | | | |
| 61 | | | | | (61.9' - E.O.B.) SILTY CLAY - Dark brown, firm to hard, wet silty clay, trace to few very fine to fine sand. | |  | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | |
| 65 | 32" | ST 28" | 12 | | | |  | | | | | | | | | | |
| 66 | | | | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
| 69 | | | | | | |  | | | | | | | | | | |
| 70 | | | | | | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | |
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| | | | | | E.O.B. at 70' | | | | | | | | | | | | |




LEGEND:




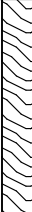





CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:

Hold backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 5 of 5

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---|-------------------------|------------|-------------------|--|------------------------------|---------|-------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | DRILL RIG INFORMATION | | | BOREHOLE INFORMATION | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 12/3/2013 | | | | | | | | | |
| DRILLER: M. CAIN | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6976.1 | | FINISH: 12/4/2013 | | | | | | | | | |
| DRILLER'S HELPER: L. ALDAZ | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 60.5 | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 65.5 | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Co) | TRIAxIAL (PHI, C (PSF)) |
| | | | | | | | | | | | | | | | | | | |
| 1 | | | | | | (0' - ~7') SANDY CLAY - Dark brown, slightly moist sandy clay, silty, sand is very fine to fine-grained, occasional coarse sand and fine gravel. | | | | | | | | | | | | |
| 2 | | | | | | (0' - 20' No sampling. Material descriptions based on cuttings and should be considered approximate.) | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | | | | | | | | | | | | | | | | | | |
| 6 | | | | | | | | | | | | | | | | | | |
| 7 | | | | | | | | | | | | | | | | | | |
| 8 | | | | | | (~7' - ~18') SAND TAILINGS - Predominantly pale yellowish brown, fine to medium grained, slightly moist, some clayey material. | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | | | | | | | | | | | | | | | | | | |
| 11 | | | | | | | | | | | | | | | | | | |
| 12 | | | | | | | | | | | | | | | | | | |
| 13 | | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |

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|---|------------------|---------------------------------------|------------|--|---|----------------------|---|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28 | 54" AC | 3A | | | (28.8' - 31') Pale gray, no sand. | CH |  | 61.8 | 62.7 | | 74/25/49 | 0.0 | 9.2 | 90.8 | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 24" ST 23" | 4 | | | (~31' - ~32.5') SAND TAILINGS - Pale yellowish brown, medium dense, moist, fine to medium sand, trace silt. | |  | 41.4 | | | | | | | | | 0.43 |
| 31 | | | | | | | | | | | | | | | | | |
| 32 | | | | | (~32.5' - 35') FINE TAILINGS WITH SAND - Pale gray, soft, moist, very fine to fine sand. | |  | | | | | | | | | | |
| 33 | AC | 5 | | | | | | | | | | | | | | | |
| 34 | | | | | (35' - 38.6') CLAYEY/SILTY SAND TAILINGS - Pale yellowish gray, soft, moist, very fine to fine sand. | |  | 14.3 | 90.9 | 2.66 | | | | | 1.6E-5 | | |
| 35 | 30" ST 28" | 6 | | | | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | |
| 38 | AC | 7 | | | (38.6' - 44.5') FINE TAILINGS - Pale gray, firm, moist, trace to few very fine sand. | |  | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | |
| 40 | 37" ST 27" | 8 | | | | | | | | | | | | | | | |
| 41 | | | | | | | | | | | | | | | | | |
| 42 | | | | | | SC / CL |  | 39.7 | 80.4 | 2.63 | | | | | | | |
| | | | | | | | | 34.3 | 83.6 | | 35/16/19 | 0.0 | 51.2 | 48.8 | 1.3E-7 | 0.262 | |

LEGEND:

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AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY





















NOTES:




Hole backfilled with cement/bentonite grout.

Page 3 of 5

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY




NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | |
|--|----------------------------|---------------------------------------|------------|--|---|----------------------|--|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|--------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| DEPTH (FT) | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSFI]) |
| 43 | 37" | AC | 9 | | (42.5' - 43.7') More sand (little to some). | |  | 29.3 | 92.3 | | | | | | | | |
| 44 | | | | | | |  | 43.3 | 74.8 | | | 0.0 | 14.5 | 85.5 | 3.0E-8 | | |
| 45 | 48" | CA 18" | 10C | 6 | (44.5' - 60.5') SILTY/CLAYEY SAND - Predominantly yellowish brown, medium dense, moist silty/clayey very fine to fine sand with abundant clay zones (as shown), occasional coarse sand throughout. (44.5' - 47.5') Silty clay with sand. | |  | | | 2.60 | | | | | | | |
| 46 | | | 10B | 9 | | |  | | | | | | | | | | |
| 46 | | | 10A | 10 | | CL |  | 21.9 | 95.2 | 2.72 | 30/16/14 | 0.0 | 27.9 | 72.1 | | | |
| 47 | | | | | | |  | | | | | | | | | | |
| 48 | | | | | | |  | | | | | | | | | | |
| 49 | | | | | (49' - 50') Reddish brown. | |  | | | | | | | | | | |
| 50 | 40" | CA 18" | 11B | 10 | | |  | | | | | | | | | | |
| 51 | | | 11A | 12 | | |  | | | | | | | | | | |
| 52 | | | | | | |  | | | | | | | | | | |
| 53 | | | | | | |  | | | | | | | | | | |
| 54 | | | | | (53.4' - 55') Silty clay with sand. | |  | | | | | | | | | | |
| 55 | 42" | CA 18" | 12C | 8 | | |  | | | | | | | | | | |
| 56 | | | 12B | 8 | | |  | | | | | | | | | | |
| 56 | | | 12A | 8 | | SM |  | 12.6 | 97.6 | 2.70 | NP | 0.0 | 57.0 | 43.0 | | | |
| 57 | | | | | | |  | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 4 of 5 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|---|------------------|---|------------|----------------------|---|------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 58 | | | | | (58.7' - 59.5') Silty clay with sand. | | | | | | | | | | | | |
| 59 | | | | | (59.5' - 60') Reddish brown, fine to medium sand. | | | | | | | | | | | | |
| 60 | 48" | CA 18" | 13C | 16 | | | | | | | | | | | | | |
| | | | 13B | 22 | (60.5' - 61') COAL - sandy. | | | | | | | | | | | | |
| 61 | | | 13A | 50/4" | (61' - E.O.B.) SHALE - Dark grayish brown, hard to very hard, moist, silty, trace very fine sand. | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| 64 | 12" | | | 14 | (bagged core) | | | | | | | | | | | | |
| | | | | | At 64' - becomes fissile, very hard, brittle, more sand (few to little). | | | | | | | | | | | | |
| 65 | | CA 2" | 15 | 50/2" | 65.2' E.O.B. (Practical Auger Refusal at 65.0') | | | | | | | | | | | | |
| 66 | | | | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | |
| 70 | | | | | | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | | |
|---|------------------|---|------------------------------|------------|-------------------|--|------------------------------|---------|--------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT:   | | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | DRILL RIG INFORMATION | | | BOREHOLE INFORMATION | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/26/2013 | | | | | | | | | |
| DRILLER: M. CAIN | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6973.3 | | FINISH: 11/27/2013 | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 105.0 | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 108.2 | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Co) | TRIAxIAL (PHI, C (PSF)) |
| 21" | | | | | | (0' - 0.6') SANDY CLAY - Light brown, soft, very moist sandy clay, very fine sand, some roots, silty. | | | | | | | | | | | | |
| 1 | | | | | | (0.6' - 0.9') ROCK - 1/2" to 3" crushed basalt. | | | | | | | | | | | | |
| 2 | | | | | | (0.9' - 6.8') SANDY CLAY - Dark brown, hard, slightly moist to moist sandy clay, very fine to fine sand, occasional coarse sand to fine gravel, silty. | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 45" | CA 17" | | 12 | | | | | | | | | | | | | | |
| 6 | | | 1B | 13 | | | | | | | | | | | | | | |
| 7 | | | 1A | 19 | | | | | | | | | | | | | | |
| 8 | | | | | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 33" | ST 27.5" | | 2 | | (6.8' - 18.9') SILTY SAND TAILINGS - Pale yellowish gray, loose to medium dense, moist, fine to medium, silty sand tailings, occasional more clayey/silty zones. | | | 9.7 | 110.0 | 2.63 | | | | | | | |
| 11 | | | | | | | | | 9.0 | 96.8 | | | 0.2 | 71.9 | 27.9 | 4.3E-4 | 0.094 | |
| 12 | | | | | | | | | | | | | | | | | | |
| 13 | | AC | | 3 | | | | | 6.7 | | 2.61 | | | | | | | |
| | | | | | | | | | 7.5 | 99.1 | | | 0.7 | 71.5 | 27.8 | 6.7E-5 | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)

ST = SHELBY TUBE (3-INCH OD)

AC = ACRYLIC LINER




HSA = HOLLOW-STEM AUGER








CC = CONTINUOUS CORE





NR = NO RECOVERY




NOTES:

Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|--|--|---|---------|----------------------|-------------------|----------------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 33' | AC | 3 | | | | | | | | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | |
| 15 | 42" | CA 18" | 4C | 5 | | | | 9.3 | 103.0 | | | | | | | | |
| | | | 4B | 5 | | | | | | | | | | | | | |
| 16 | | | 4A | 5 | | SM | | 6.5 | 100.0 | 2.65 | NP | 2.4 | 82.3 | 15.3 | | | |
| 17 | AC | 5B | | | | | | | | | | | | | | | |
| 18 | AC | 5A | | | | | | | | | | | | | | | |
| 19 | | | | | (18.9' - 24.4') FINE TAILINGS - Pale gray, soft, moist, trace to few fine to medium sand. | | | | | | | | | | | | |
| 20 | 36" | ST 30" | 6 | | | | | | | | | | | | | | |
| 21 | | | | | | | | | | | | | | | | | |
| 22 | | | | | | CL | | 26.7 | 92.9 | | 43/19/24 | 0.0 | 43.0 | 57.0 | | 0.111 | |
| 23 | AC | 7 | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | |
| 25 | 42" | CA 18" | 8C | 2 | (24.4' - 25.7') SAND TAILINGS - Pale yellowish gray, loose, moist to very moist, fine to medium sand. | | | | | | | | | | | | |
| | | | 8B | 2 | | | | | | | | | | | | | |
| 26 | | | 8A | 3 | (25.7' - ~31') FINE TAILINGS - Pale gray, soft, moist to very moist, trace to few fine to medium sand. | CH | | 41.0 | | | 74/27/47 | 0.0 | 10.0 | 90.0 | | | |
| | | | | | | | | 57.4 | 64.3 | 2.80 | | | | | | | |
| 27 | AC | 9B | | | | | | | | | | | | | | | |
| | AC | 9A | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> | | | | | | | | | | | | | | | | | |
| <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 2 of 8 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|--|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28 | 42" | AC 9A | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 42" | ST 27" | 10 | | | CH |  | 45.3 | 73.4 | 2.78 | 57/22/35 | 0.0 | 24.3 | 75.7 | | | |
| 31 | | | | | (~31' - 33.2') SAND TAILINGS - See description for 24.4' - 25.7'. | | | | | | | | | | | | |
| 32 | | | | | | SM |  | 15.4 | 100.1 | 2.67 | NP | 0.0 | 83.1 | 16.9 | | | |
| 33 | | AC 11 | | | | | | | | | | | | | | | |
| 34 | | | | | (33.2' - 44.6') FINE TAILINGS - Pale gray, soft, moist to very moist. | | | | | | | | | | | | |
| 35 | 48" | CA 18" | 12C | 2 | | | | 47.7 | 72.5 | | | | | | | | |
| | | | 12B | 2 | | | | 51.4 | | | | | | | | | |
| 36 | | | 12A | 2 | | SC / CL |  | 32.2 | 87.8 | 2.72 | 36/16/20 | 0.0 | 50.6 | 49.4 | | | |
| 37 | | AC 13B | | | (36.3' - 37.8') Some very fine to fine sand. | | | | | | | | | | | | |
| 38 | | AC 13A | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | |
| 40 | 30" | ST 30" | 14 | | | CH |  | 45.7 | 73.7 | 2.56 | | | | | | | |
| 41 | | | | | | | | 47.2 | 74.5 | | 61/21/40 | 0.0 | 20.7 | 79.3 | 2.9E-8 | 0.315 | |
| 42 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 3 of 8 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|------------|---|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 30 | 30" | ST 30" | 14 | 15 | (44.3' - 44.6') Appears finer grained (clayey), lighter gray, more moist. (44.6' - 62.5') SILTY SAND - Light brown, medium dense, moist, silty very fine to fine sand, occasional coarse sand and fine gravel. | |  | 9.9 | 95.4 | 2.74 | | 0.0 | 65.8 | 34.2 | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 42" | CA 17" | 12 | 12 | | | | | | | | | | | | | |
| 46 | | | 16B | 12 | | | | | | | | | | | | | |
| 47 | | | 16A | 14 | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 48" | CA 18" | 17B | 10 | | | | | | | | | | | | | |
| 51 | | | 17A | 11 | | | | | | | | | | | | | |
| 52 | | | | 14 | | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | |
| 55 | 42" | ST 17" | 18 | | | | | 14.1 | 100.8 | | | | | | 2.4E-5 | 0.139 | |
| 56 | | | | | (~56' - 57.5') Gravelly. (Shelby Tube refusal at 56.5') | | | | | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| Page 4 of 8 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|--|---|------------|----------------------|-------------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | | | | | | | | | | | | | |
| 60" | 39" | CA 18" | 11 | | | | | | | | | | | | | | |
| | | 19B | 11 | | | | | | | | | | | | | | |
| 61" | | 19A | 14 | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | (62.5' - 65.2') WEATHERED SANDSTONE (?) - Hard, moist, gravelly. | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 48" | CA 18" | 14 | | | | | | | | | | | | | | |
| | | 20B | 14 | | (65.2' - 82') SILTY SAND - See description above for 44.6' - 62.5'. | | | | | | | | | | | | |
| 66" | | 20A | 15 | | | SM / ML | | 13.8 | 94.5 | | NP | 0.0 | 50.1 | 49.9 | | | |
| 67" | | | | | | | | | | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | 30" | CA 18" | 4 | | | | | | | | | | | | | | |
| | | 21B | 6 | | | | | | | | | | | | | | |
| 71" | | 21A | 10 | | (70.5' - 71.5') Moist to very moist, increased clay. | | | 18.1 | 100.8 | | | | | | | | |
| | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:

Hole backfilled with cement/bentonite grout.

Page 5 of 8

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
|---|----------------------------|---|------------|-----------------|---|-------------------------------|---------|----------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| DEPTH (FT) | FIELD SAMPLE RECOVERY DATA | | | | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | LABORATORY TEST DATA | | | | | | | | | |
| | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | | | | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 72 | 30" | | | | | | | | | | | | | | | | |
| 73 | | | | | | | | | | | | | | | | | |
| 74 | | | | | | | | | | | | | | | | | |
| 75 | 42" | CA 18" | | 5 | | | | | | | | | | | | | |
| | | 22B | | 7 | | | | | | | | | | | | | |
| 76 | | 22A | | 11 | | | | | | | | | | | | | |
| 77 | | | | | | | | | | | | | | | | | |
| 78 | | | | | | | | | | | | | | | | | |
| 79 | | | | | | | | | | | | | | | | | |
| 80 | 36" | CA 18" | | 9 | (80' - 82') Gravelly (sandstone fragments) | | | | | | | | | | | | |
| | | 23B | | 14 | | | | | | | | | | | | | |
| 81 | | 23A | | 17 | | | | | | | | | | | | | |
| 82 | | | | | (82' - 85.5') WEATHERED SANDSTONE - Mottled red/gray/brown, moist, fine to medium weathered sandstone. | | | | | | | | | | | | |
| 83 | | | | | | | | | | | | | | | | | |
| 84 | NA | 3" | 24 | 50/3" | | | | | | | | | | | | | |
| 85 | 50" | | | | | | | | | | | | | | | | |
| 86 | | | | | (85.5' - 105') CLAYEY SAND - Dark brown, firm, very moist to wet, fine to medium clayey sand, occasional sandstone fragments. | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)

ST = SHELBY TUBE (3-INCH OD)

AC = ACRYLIC LINER




HSA = HOLLOW-STEM AUGER




CC = CONTINUOUS CORE

NR = NO RECOVERY

NOTES:

Hole backfilled with cement/bentonite grout.




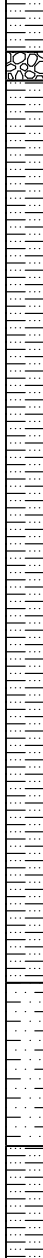
| | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|--|---|----------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|--------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B10 | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSFI]) |
| 50" | | | | | | | | | | | | | | | | | |
| 87- | | | | | | | | | | | | | | | | | |
| 88- | | | | | | | | | | | | | | | | | |
| 89- | | | | | | | | | | | | | | | | | |
| 90- | 40" | CA 18" | 7 | | [CA sampler wet 11/26/13.] [Water measured at approximately 90.2' bgs at 9:30 11/27/13.] | | | | | | | | | | | | |
| | | 25B | 12 | | | | | | | | | | | | | | |
| 91- | | 25A | 10 | | | | | 18.6 | 105.6 | 2.66 | | | | | | | |
| 92- | | | | | | | | | | | | | | | | | |
| 93- | | | | | [Core barrel wet 11/27/13.] | | | | | | | | | | | | |
| 94- | | | | | | | | | | | | | | | | | |
| 95- | 52" | NR | 1 | | | | | | | | | | | | | | |
| | | | 5 | | | | | | | | | | | | | | |
| 96- | | | 8 | | | | | | | | | | | | | | |
| 97- | | | | | | | | | | | | | | | | | |
| 98- | | | | | | | | | | | | | | | | | |
| 99- | | | | | | | | | | | | | | | | | |
| 100- | 44" | | | | | | | | | | | | | | | | |
| 101- | | | | | | | | | | | | | | | | | |
| LEGEND: | | | | | NOTES: | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | |
| Page 7 of 8 | | | | | | | | | | | | | | | | | |




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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 44" | | | | | | | | | | | | | | | | | |
| 102 | | | | | | | | | | | | | | | | | |
| 103 | | | | | [Core barrel wet.] | | | | | | | | | | | | |
| 104 | | | | | | | | | | | | | | | | | |
| 105 | 36" | | | | (105' - E.O.B.) WEATHERED SANDSTONE - Pale yellowish brown, very dense, very moist, very fine to fine sandstone, some cemented zones. | | | | | | | | | | | | |
| 106 | | | | | | | | | | | | | | | | | |
| 107 | | | | 26 | (106.9' - 107.3') Bagged core sample. | | | 14.2 | 109.1 | | | | | | 1.4E-7 | | |
| 108 | 1" | | 50/ 1.5" | 27 | (107.9' - 108') Bagged core sample. (108') CA sample not retained. E.O.B. = 108.2 ft at 9:00 on 11/27/13 (practical auger refusal) | | | | | | | | | | | | |
| 109 | | | | | | | | | | | | | | | | | |
| 110 | | | | | | | | | | | | | | | | | |
| 111 | | | | | | | | | | | | | | | | | |
| 112 | | | | | | | | | | | | | | | | | |
| 113 | | | | | | | | | | | | | | | | | |
| 114 | | | | | | | | | | | | | | | | | |
| 115 | | | | | | | | | | | | | | | | | |




LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:
Hole backfilled with cement/bentonite grout.

Page 8 of 8

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|--|------------------|---|------------|--|-----------------|--|---|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B11 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | START: 12/2/2013 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | SURFACE ELEV. (FT): 6977.3 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | FINISH: 12/3/2013 | |
| | | | | | | DEPTH TO BEDROCK (FT): 96.9 | |
| | | | | | | TOTAL DEPTH (FT): 103.0 | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS GRAPHIC |
| 19' | | | | | | (0' - 0.6') SANDY CLAY - Brown, soft, moist to very moist sandy clay, very fine to fine sand, roots, silty. |  |
| 1 | | | | | | (0.6' - 0.9') ROCK - 1/2" to 3" crushed basalt, silty clay in voids. | |
| 2 | | | | | | (0.9' - 10.3') SANDY CLAY - Dark brown, hard, slightly moist sandy clay, very fine to fine sand, occasional coarse sand and gravel up to 1.5". | |
| 3 | | | | | | | |
| 4 | | | | | | | |
| 5 | 30" | CA 17" | 1C | 12 | | | |
| 6 | | | 1B | 16 | | | |
| | | | 1A | 15 | | | |
| 7 | | | | | | | |
| 8 | | | | | | | |
| 10 | 42" | CA 18" | 2C | 7 | | (10.3' - 12') SILTY SAND - Light brown, medium dense, slightly moist, silty very fine to fine sand. | |
| 11 | | | 2B | 7 | | | |
| | | | 2A | 8 | | | |
| 12 | | | | | | (12' - 15') SANDY CLAY - Dark brown, firm to hard, slightly moist sandy clay, very fine to fine sand, occasional gravel up to 3" size. | |
| 13 | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | |

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|--|------------------|---------------------------------------|------------|--|-----------------|---|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | | |
| 15 | 45" | ST 13" | 3 | | | (15' - 18') CLAYEY SAND - Light yellowish brown, medium dense, slightly moist, fine to medium clayey sand, occasional gravel up to 1". | | | 8.2 | 110.4 | 2.67 | | 3.9 | 57.6 | 38.5 | 2.5E-5 | 0.09 | |
| 16 | | | | | | | | | | | | | | | | | | |
| 17 | | | | | | | | | | | | | | | | | | |
| 18 | | | | | | (18' - 32.9') SANDY CLAY - Predominantly dark brown, hard, slightly moist sandy clay, silty, very fine to medium sand, few to little coarse sand and gravel up to ~1" size. | | | | | | | | | | | | |
| 19 | | | | | | (19.2' - 19.4') Sand, very fine to fine. | | | | | | | | | | | | |
| 20 | 48" | CA 18" | 4C | 4 | | | | | | | | | | | | | | |
| | | | 4B | 7 | | | | | | | | | | | | | | |
| 21 | | | 4A | 10 | | | | | 12.3 | 107.6 | | | | | | | | |
| 22 | | | | | | | | | | | | | | | | | | |
| 23 | | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | | |
| 25 | 56" | CA 18" | 5C | 7 | | | | | | | | | | | | | | |
| | | | 5B | 8 | | | | | | | | | | | | | | |
| 26 | | | 5A | 13 | | | | | | | | | | | | | | |
| 27 | | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |
| Page 2 of 8 | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 51" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 60" | 7C | 7 | | (Photo 310 at 46'.) | | | | | | | | | | | | |
| | | 7B | 7 | | | | | | | | | | | | | | |
| 46 | | 7A | 8 | | (45.5' - 53.9') FINE TAILINGS - Mottled orange and dark greenish gray (to 50'), pale yellowish gray (50' - 53.9'), firm, moist tailings. | | | 88.7 | | | | | | | | | |
| | | | | | [Photo 311 at 46.5'.] | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 43" | ST 28" | 8 | | | | | | | | | | | | | | |
| 51 | | | | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | |
| | | AC | 9 | | [Photo 312 at 52.5'] | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | (53.9' - 55') SILTY CLAY - Dark brown, hard, moist silty clay, trace very fine sand. | | | | | | | | | | | | |
| 55 | 48" | ST 25" | 10 | | (55' - 77.5') SILTY SAND - Yellowish brown, medium dense, slightly moist to moist, silty, very fine to fine sand. | | | | | | | | | | | | |
| 56 | | | | | | SM | | 16.2 | 77.9 | 2.64 | NP | 0.0 | 60.4 | 39.6 | 5.6E-4 | 0.129 | |
| 57 | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:

Hole backfilled with cement/bentonite grout.




Page 4 of 8

LEGEND:

- CA = CALIFORNIA SAMPLE (2-INCH OD)
- ST = SHELBY TUBE (3-INCH OD)
- AC = ACRYLIC LINER
- HSA = HOLLOW-STEM AUGER
- CC = CONTINUOUS CORE
- NR = NO RECOVERY




NOTES:

- Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B11 | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | |
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LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:
Hole backfilled with cement/bentonite grout.

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|--|------------------|---------------------------------------|------------|--|---|---|---------|----------------------|-------------------|----------------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|
|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) | |
| 72-44" | | | | | (71.5' - 73.5') Abundant clayey sand zones. | | | | | | | | | | | | | |
| 73- | | | | | | | | | | | | | | | | | | |
| 74- | | | | | | | | | | | | | | | | | | |
| 75-38" | CA 18" | 15C | 7 | | | | | | | | | | | | | | | |
| | | 15B | 8 | | | | | | | | | | | | | | | |
| 76- | | 15A | 11 | | | | | | | | | | | | | | | |
| 77- | | | | | | | | | | | | | | | | | | |
| 78- | | | | 16 | (77.5' - 78') WEATHERED SANDSTONE - Rusty red, moist, fine to medium grained. (Sample #16 is bagged core.) | | | | | | | | | | | | | |
| 79- | | | | | (78' - 96.9') GRAVELLY SAND - Mottled rusty red/brown/yellow, dense, moist fine to medium sand, silty throughout, some clayey zones, abundant coarse material from coarse sand up to 3" gravel comprised of cemented sandstone. | | | | | | | | | | | | | |
| 80-42" | CA 18" | 17C | 16 | | | | | | | | | | | | | | | |
| | | 17B | 21 | | | | | | | | | | | | | | | |
| 81- | | 17A | 21 | | | | | | 11.0 | 107.6 | 2.76 | | 12.9 | 65.6 | 21.5 | | | |
| 82- | | | | | | | | | | | | | | | | | | |
| 83- | | | | | | | | | | | | | | | | | | |
| 84- | | | | | | | | | | | | | | | | | | |
| 85-36" | CA 17" | 18C | 18 | | | | | | | | | | | | | | | |
| | | 18B | 21 | | | | | | | | | | | | | | | |
| 86- | | 18A | 19 | | | | | | | | | | | | | | | |
| LEGEND: | | | | | NOTES: | | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | | |
| Page 6 of 8 | | | | | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY




NOTES:
 Hole backfilled with cement/bentonite grout.




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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 102 | | | | | | (102.5' - 103') Reddish brown, strongly cemented sandstone. | | | | | | | | | | | | |
| 103 | | | | | | E.O.B. at 103.0' at 10:00 (practical auger refusal) | | | | | | | | | | | | |
| 104 | | | | | | | | | | | | | | | | | | |
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| 115 | | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |

|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | | |
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| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | | | | | | | | | | | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6976.8 | | | | | | | | | | | | |
| DRILLER'S HELPER: L. ALDAZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | FINISH: 12/5/2013 | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | DEPTH TO BEDROCK (FT): N/A | | | | | | | | | | | | |
| TOTAL DEPTH (FT): 71.5 | | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Co) | TRIAxIAL (Phi, C (PSF)) |
| 18" | | | | | | (0' - 0.5') SANDY CLAY - Brown, soft, moist to very moist sandy clay, very fine sand, roots. | | | | | | | | | | | | |
| 1 | | | | | | (0.5' - 0.8') ROCK - Crushed basalt, up to 3" size, sandy clay in voids. | | | | | | | | | | | | |
| 2 | | | | | | (0.8' - ~3') SANDY CLAY - Dark yellowish brown, hard, moist sandy clay, very fine to fine sand. | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 30" | CA 18" | 1C | 10 | | | | | | | | | | | | | | |
| 6 | | | 1B | 11 | | | | | | | | | | | | | | |
| 7 | | | 1A | 12 | | | | | | | | | | | | | | |
| 8 | | | AC | 2 | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 30" | CA 18" | | 3 | | | | | | | | | | | | | | |
| 11 | | | 3B | 3 | | | | | | | | | | | | | | |
| 12 | | | 3A | 3 | | | | | | | | | | | | | | |
| 13 | | | AC | 4 | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | | | | | |
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| DEPTH (FT) | | | | | | | | | | MATERIAL DESCRIPTION | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| SAMPLES & RECOV. | | | | | | | | | | GRAPHIC | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| SAMPLE NO. | | | | | | | | | | WATER CONT. (%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| BLOW COUNT | | | | | | | | | | DRY DENSITY (PCF) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| BULK SAMPLE NO. | | | | | | | | | | SPECIFIC GRAVITY | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| | | | | | | | | | | CONSOLIDATION (Cc) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| 28 | 27" | AC | 10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| MWH | | CLIENT: | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | |
|----------------------------|------------------|---------------------------------------|------------|----------------------|---|---------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 48" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 26" | CA 18" | 15C 13 | | (45' - 50') SANDY SILT - Dark yellowish brown, hard, moist, very fine to fine sand, occasional clayey sand zones. | | | | | | | | | | | | |
| | | 15B 25 | | | | | | | | | | | | | | | |
| 46 | | 15A 26 | | | | | | 25.8 | 99.3 | 2.81 | NP | 0.0 | 37.0 | 63.0 | | | |
| 47 | | | | | | | | 17.3 | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 24" | CA 18" | 16C 6 | | (50' - 52') CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine sand, silty. | | | | | | | | | | | | |
| | | 16B 8 | | | | | | | | | | | | | | | |
| 51 | | 16A 11 | | | | | | | | | | | | | | | |
| 52 | | | | | (52' - 65') SILTY CLAY - Dark yellowish brown, firm to hard, moist silty clay, trace to few very fine to fine sand, occasional thin (1-6") clayey sand zones. | | | | | | | | | | | | |
| 53 | | | | | (~53' - 55') Very hard, very dense clay. | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | |
| 55 | 18" | CA 18" | 17C 10 | | | | | | | | | | | | | | |
| | | 17B 11 | | | | | | | | | | | | | | | |
| 56 | | 17A 12 | | | | | | 11.7 | 104.2 | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:




Hole backfilled with cement/bentonite grout.

Page 4 of 5

LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
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NOTES:
Hole backfilled with cement/bentonite grout.




|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | |
|--|------------------|---|------------|----------------------|---|-------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 18' | | | | | | | | | | | | | | | | | |
| 58 | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | |
| 60 | 60" | 18C | 8 | | | | | | | | | | | | | | |
| | | 18B | 11 | | | | | | | | | | | | | | |
| 61 | | 18A | 15 | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | |
| 65 | 40" | 18' | 6 | | (65' - E.O.B.) CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine clayey sand, silty, occasional 1-3" zones of sandy clay. | | | | | | | | | | | | |
| | | 19B | 8 | | | | | | | | | | | | | | |
| 66 | | 19A | 10 | | | | | 12.7 | 100.7 | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | |
| 70 | | CA 18' | 7 | | | | | | | | | | | | | | |
| | | 20B | 6 | | | | | | | | | | | | | | |
| 71 | | 20A | 9 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | |
| | | | | | E.O.B. 71.5' at 14:30 | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |




| | | | | | | | |
|---|------------------|---|------------|-----------------------------|-----------------|---|------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B23 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6959.3 | |
| DRILLER'S HELPER: L. ALDAZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 43.0 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 70.5 | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS |
| | | | | | | | GRAPHIC |
| 39" | | | | | | (0' - 0.6') SANDY CLAY - Light brown, soft, moist, very fine to fine sand, roots. | |
| 1 | | | | | | (0.6' - 0.9') ROCK - Crushed basalt, 1/2" - 3", sandy clay in voids. | |
| 2 | | | | | | (0.9' - 5') SANDY CLAY - Firm to hard, slightly moist to moist, sandy clay, very fine to fine sand, occasional coarse sand and very fine gravel. | |
| 3 | | | | | | | |
| 4 | | | | | | | |
| 5 | 44" | CA 18" | 1C | 12 | | (5' - 7') SILTY SAND WITH GRAVEL - Light brown, medium dense, slightly moist to moist, silty very fine to fine sand with little to some gravel to 2". | |
| 6 | | | 1B | 14 | | | |
| 7 | | | 1A | 10 | | | |
| 8 | | | | | | (7' - 13.4') SANDY CLAY - See 0.9' to 5' above. | |
| 9 | | | | | | | |
| 10 | 42" | CA 16" | 2B | 5 | | | |
| 11 | | | 2A | 6 | | | |
| 12 | | | | | | | |
| 13 | | | | | | | |




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|--|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|--------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSFI]) |
| 42" | | | | | | (13.4' - 14.2') FINE TAILINGS WITH SAND - Pale gray, soft to firm, moist with very fine to fine sand tailings. | | | | | | | | | | | | |
| 14 | | | | | | (14.2' - ~16') SILTY SAND TAILINGS - Pale yellowish gray, loose, moist, fine to medium sand. | | | | | | | | | | | | |
| 15 | 25" | ST 28" | 3 | | | | | | 20.7 | 87.7 | 2.77 | | 0.0 | 62.8 | 37.2 | | | |
| 16 | | | | | | | | | | | | | | | | | | |
| 17 | | | | | | (~16' - 20.3') SANDY CLAY - Dark yellowish brown, firm, moist, very fine to fine sand. | | | | | | | | | | | | |
| 18 | | AC | 4 | | | | | | | | | | | | | | | |
| 19 | | | | | | | | | | | | | | | | | | |
| 20 | 36" | CA 18" | 5C | 5 | | | | | 22.5 | 101.9 | 2.73 | | 0.0 | 31.1 | 68.9 | | | |
| 21 | | | 5B | 6 | | (20.3' - 23') SILTY SAND - Yellowish brown, medium dense, moist, silty very fine to fine sand. | | | | | | | | | | | | |
| 22 | | | 5A | 7 | | | | | | | | | | | | | | |
| 23 | | | | | | (23' - 38.6') SILTY CLAY - Predominantly dark yellowish brown, firm to hard, moist silty clay with varying amount of sand as shown, occasional coarse sand to very fine gravel throughout. | | | | | | | | | | | | |
| 24 | | | | | | (23' - 27.5') Trace to few sand. | | | | | | | | | | | | |
| 25 | 39" | ST 30" | 6 | | | | | | | | | | | | | | | |
| 26 | | | | | | | CL | | 21.6 | 101.7 | 2.73 | 49/18/31 | 0.0 | 8.8 | 91.2 | | 0.05 | |
| 27 | | | | | | (27.5' - 30') Little to some sand. | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |
| Page 2 of 5 | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|--|------------------|---|------------|-----------------|--|-------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| | | | | | | | | | | | | | | | | | |
| 28 | 39" | | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 44" | CA 18" | 5 | | (30' - 30.5') Gravelly. | | | | | | | | | | | | |
| | | 7B | 6 | | (30.5' - ~33.5') Little to some sand. | | | | | | | | | | | | |
| 31 | | 7A | 8 | | | | | | | | | | | | | | |
| 32 | | | | | | | | | | | | | | | | | |
| 33 | | | | | | | | | | | | | | | | | |
| 34 | | | | | (~33.5' - ~34.3') Clayey sand, very fine to fine. | | | | | | | | | | | | |
| | | | | | (~34.3' - 38.6') Few to little sand. | | | | | | | | | | | | |
| 35 | 42" | ST 28" | 8 | | | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | |
| 39 | | | | | (38.6' - 40.3') CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine sand, silty. | | | | | | | | | | | | |
| 40 | 25" | CA 18" | 6 | | | | | | | | | | | | | | |
| | | 9B | 9 | | (40.3' - 43') SILTY CLAY WITH SAND - Dark yellowish brown, firm to hard, moist, silty clay with little to some very fine to fine sand. | | | | | | | | | | | | |
| 41 | | 9A | 12 | | | | | | | | | | | | | | |
| 42 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|--|--|------------|----------------------|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 25" | | | | | | | | | | | | | | | | | |
| 43" | | | | | (43' - 65.5') SANDSTONE - Mostly very pale yellowish gray, moist, mostly non-or weakly cemented very fine to fine sand, some very hard, strongly cemented, fissile zones as shown, some clay zones as shown ("Zone 3"?). | | | | | | | | | | | | |
| 44" | | | | | (43' - 43.6') Strongly cemented, fissile. | | | | | | | | | | | | |
| 45" | | | | | (43.6' - 44') Clay - yellowish brown, firm to hard, moist, slightly silty. | | | | | | | | | | | | |
| 45" | 32" | CA 8" | 10A | 13 | (44' - 45.5') strongly cemented, fissile. | | | 13.8 | 108.7 | | | | | | 2.4E-7 | | |
| 46" | | | | 50/ 3" | (45.5' - 46.2') Clayey sand, yellowish brown. | | | | | | | | | | | | |
| 47" | | | | | | | | | | | | | | | | | |
| 48" | 16" | | | | (~47' - ~48') Very hard, strongly cemented, fissile. | | | | | | | | | | | | |
| 49" | | CA NR | | 50/ 4" | | | | | | | | | | | | | |
| 50" | 29" | | | | | | | | | | | | | | | | |
| 51" | | | | | | | | | | | | | | | | | |
| 52" | | | | | | | | | | | | | | | | | |
| 53" | | | | | | | | | | | | | | | | | |
| 54" | | | | | | | | | | | | | | | | | |
| 55" | 33" | CA 3" | 11A | 50/ 5" | (~55' - 63') Coarser (fine to medium). | | | | | | | | | | | | |
| 56" | | | | | (~56' - 56.8') Color is reddish yellow. | | | | | | | | | | | | |
| 57" | | | | | | | | | | | | | | | | | |
| LEGEND: | | | | | NOTES: | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | |
| Page 4 of 5 | | | | | | | | | | | | | | | | | |

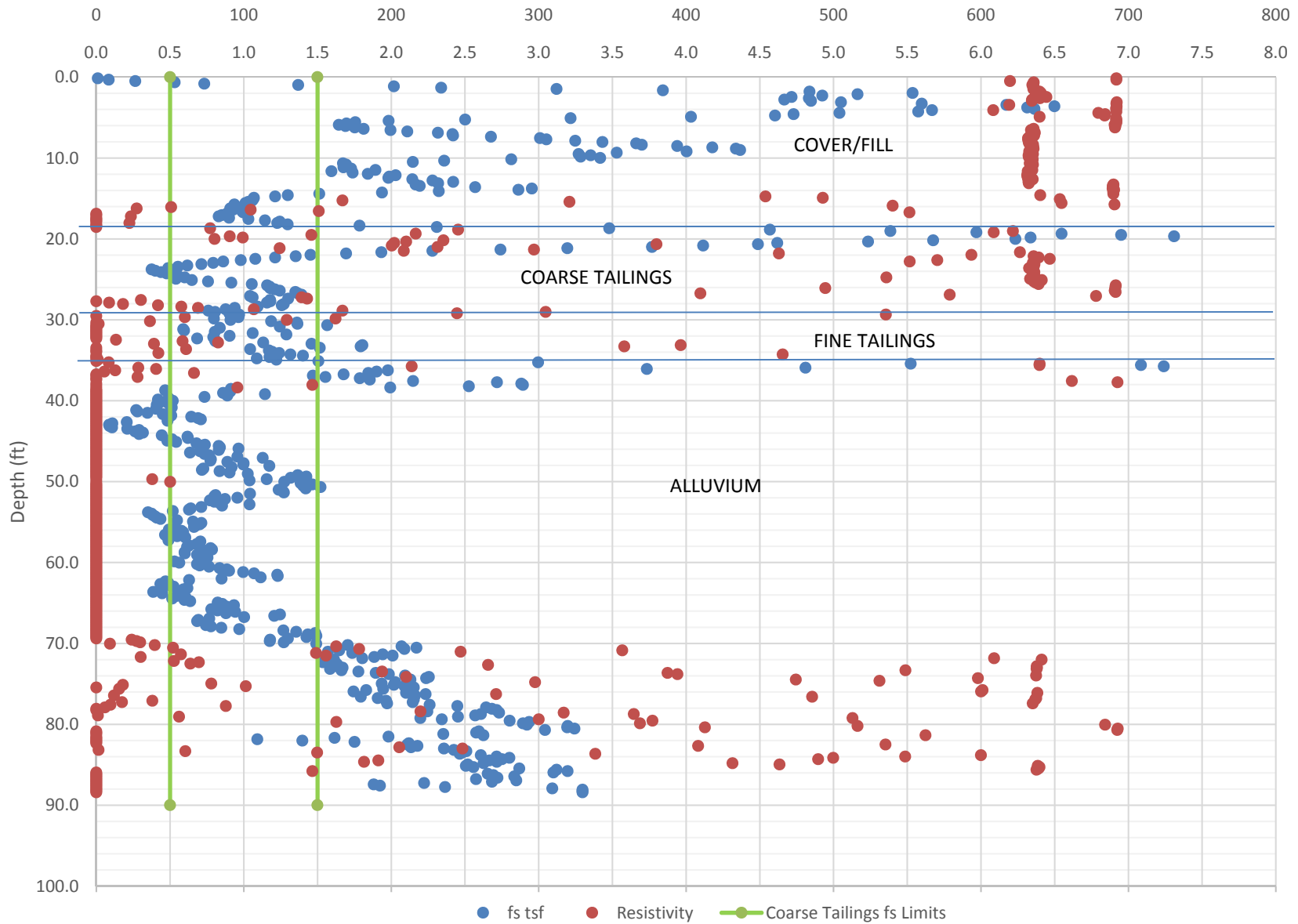
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|---|------------------|--|------------|-----------------|--|----------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | |   | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 33" | | | | | | | | | | | | | | | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | | | | | | | | | | | | | |
| 60" | 24" | CA 3" | 12A | 50/ 4" | | | | | | | | | | | | | |
| 61" | | | | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | (~63 - 65.5') COAL - Black, hard, dry to slightly moist, fissile. | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 30" | CA 13" | 13B | 24 | | | | | | | | | | | | | |
| 66" | | | 13A | 50/ 4.5" | (65.5' - E.O.B.) SHALE - Gray, very hard, slightly moist shale, trace silt, non- to weakly-cemented ("Zone 2"?). | | | 10.2 | 103.0 | | | | | | 9.7E-8 | | |
| 67" | | | | | | | | | | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | | CA 4" | 14 | 50/ 5" | | | | | | | | | | | | | |
| 71" | | | | | E.O.B. 70.5' @ 13:50 | | | | | | | | | | | | |
| LEGEND: | | | | | NOTES: | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | |
| Page 5 of 5 | | | | | | | | | | | | | | | | | |

ATTACHMENT B
CPT PROFILE INTERPRETATIONS

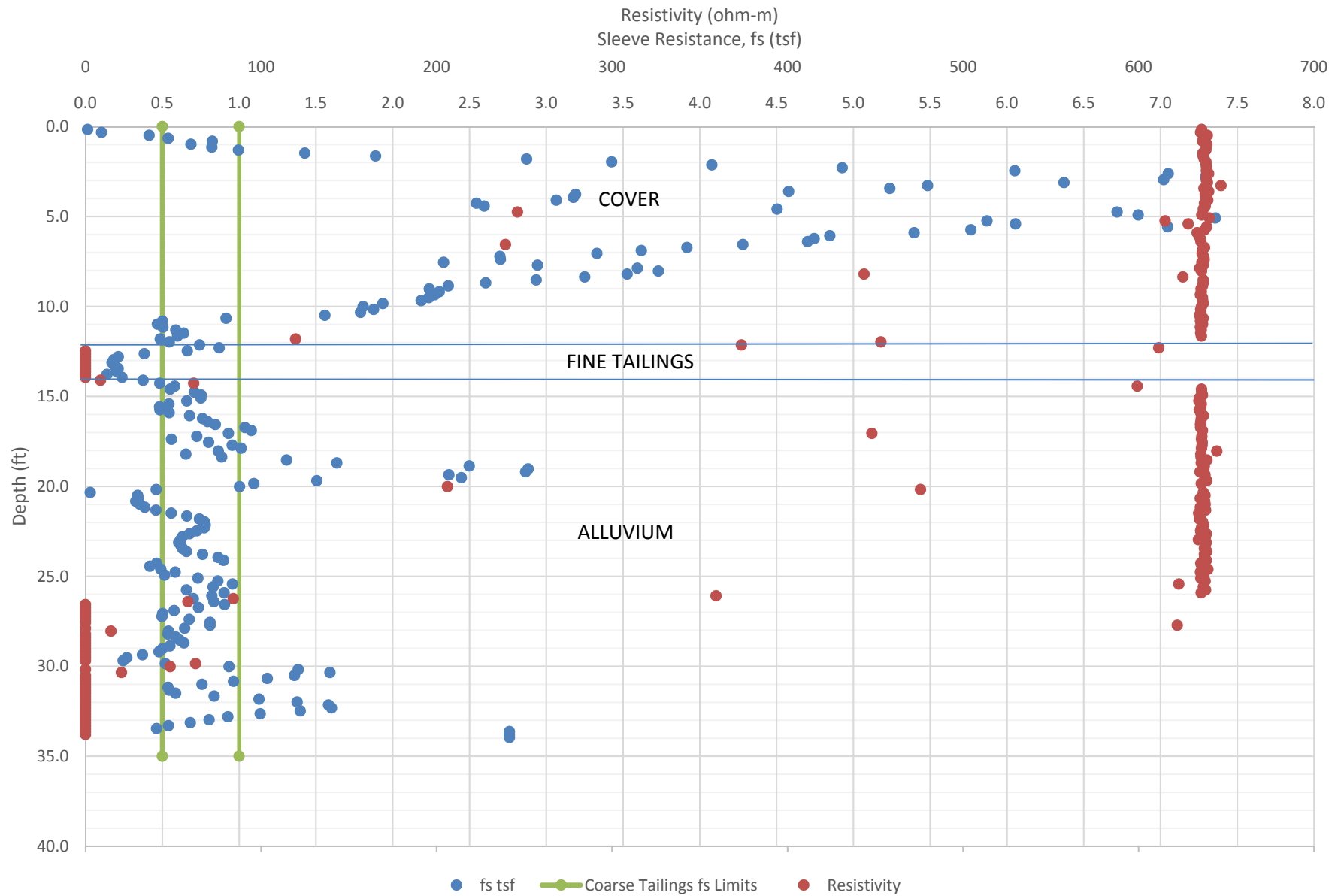
CPT-01

Resistivity (ohm-m)

Sleeve Resistance, fs (tsf)

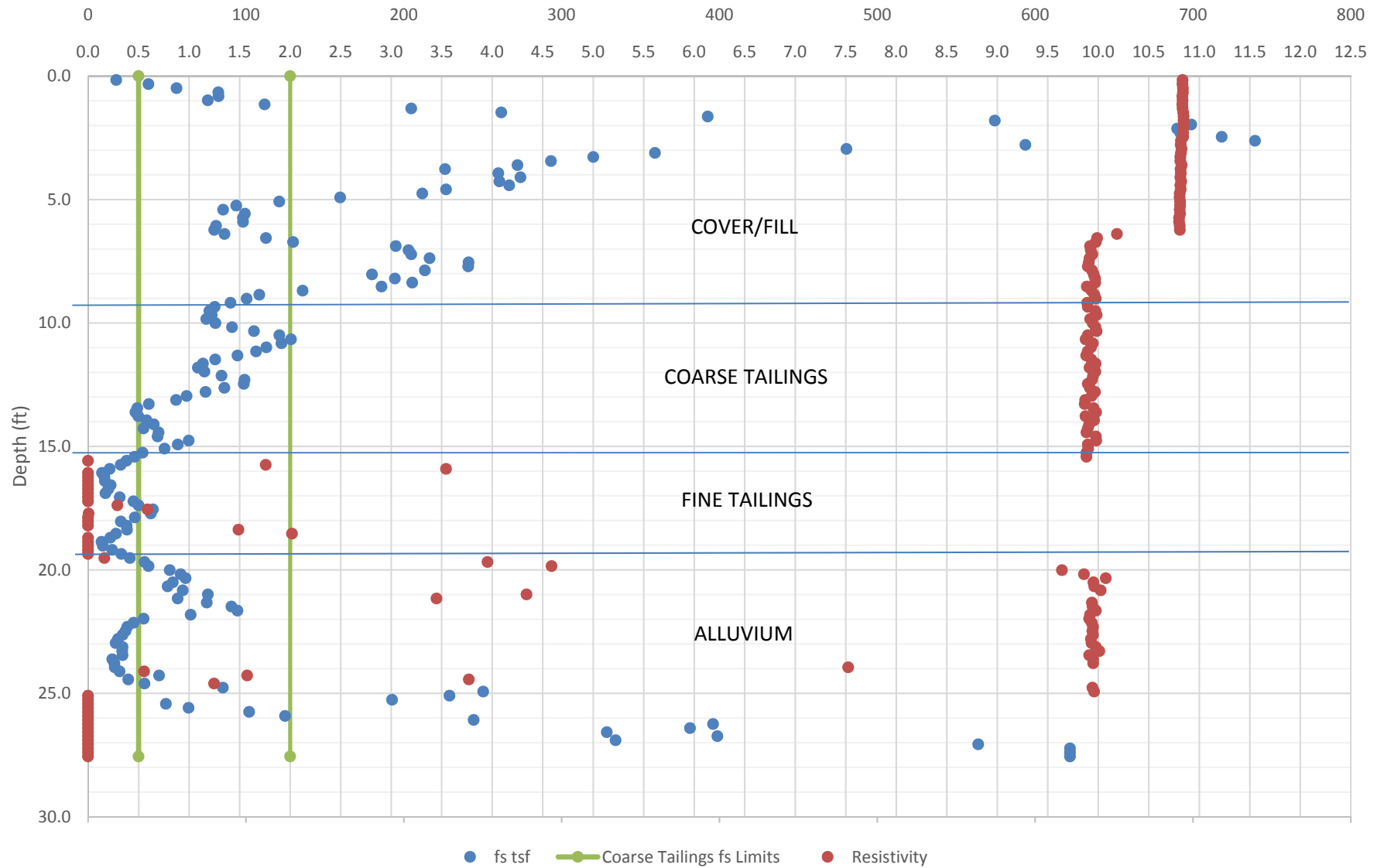


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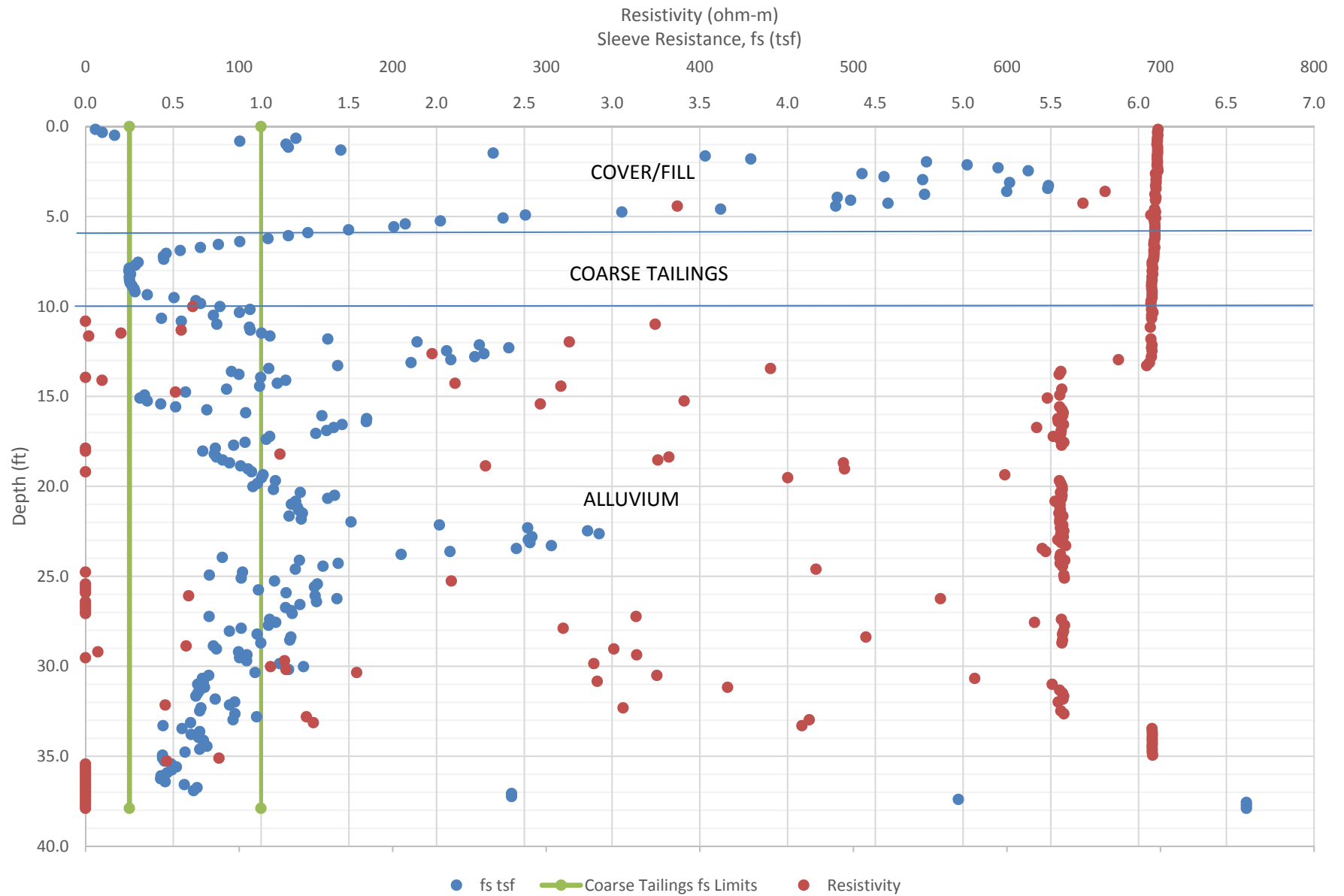


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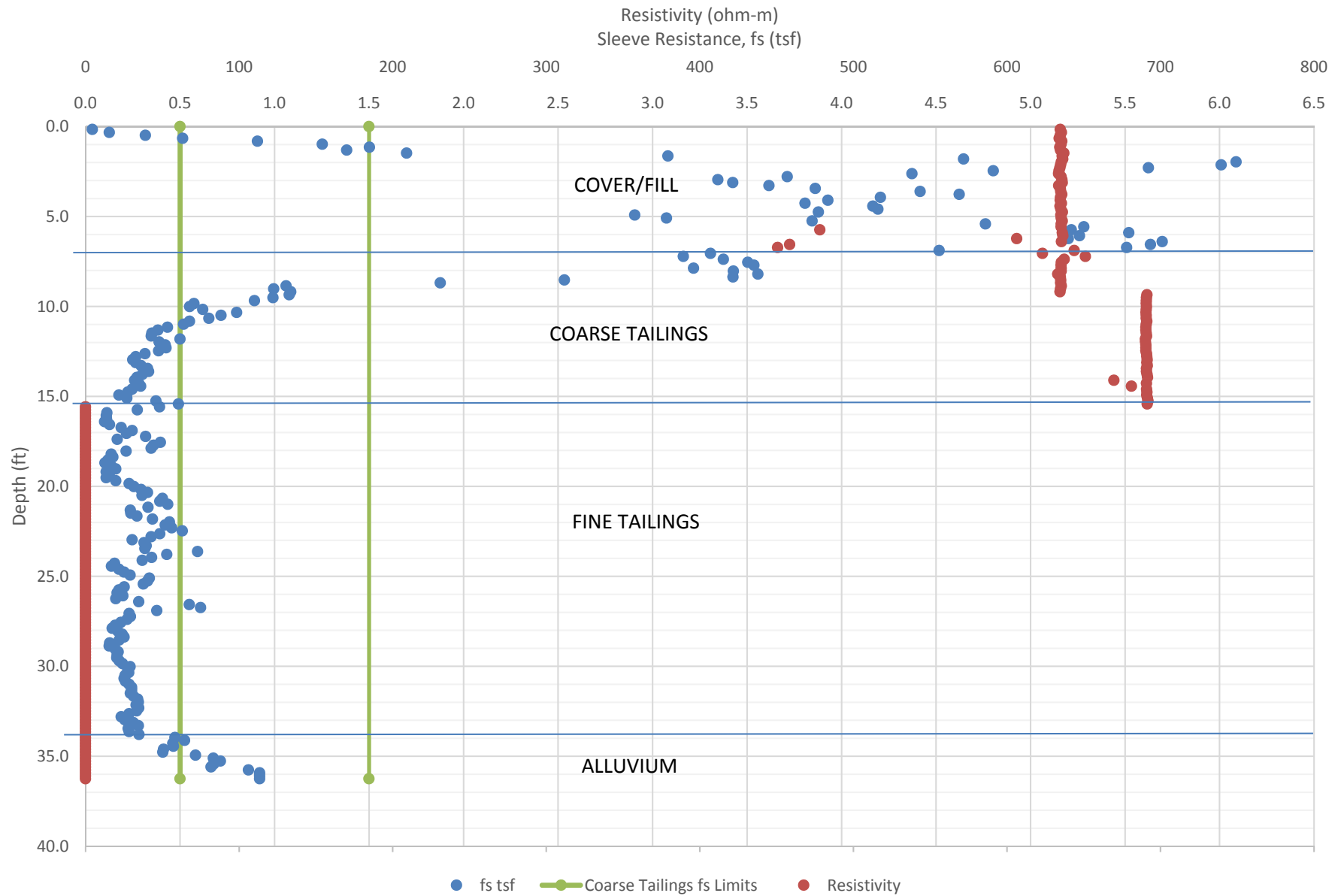
Resistivity (ohm-m)
Sleeve Resistance, fs (tsf)



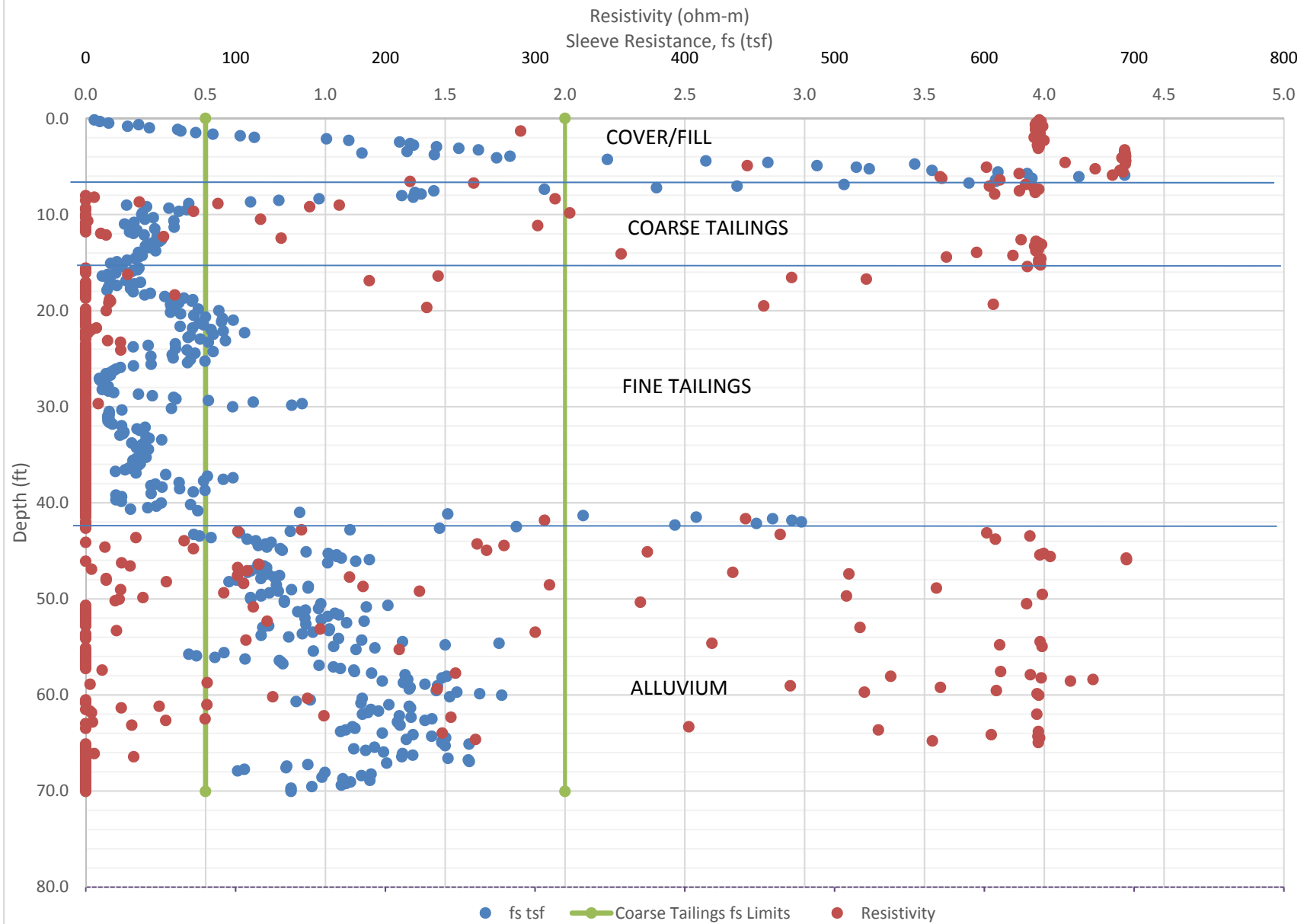
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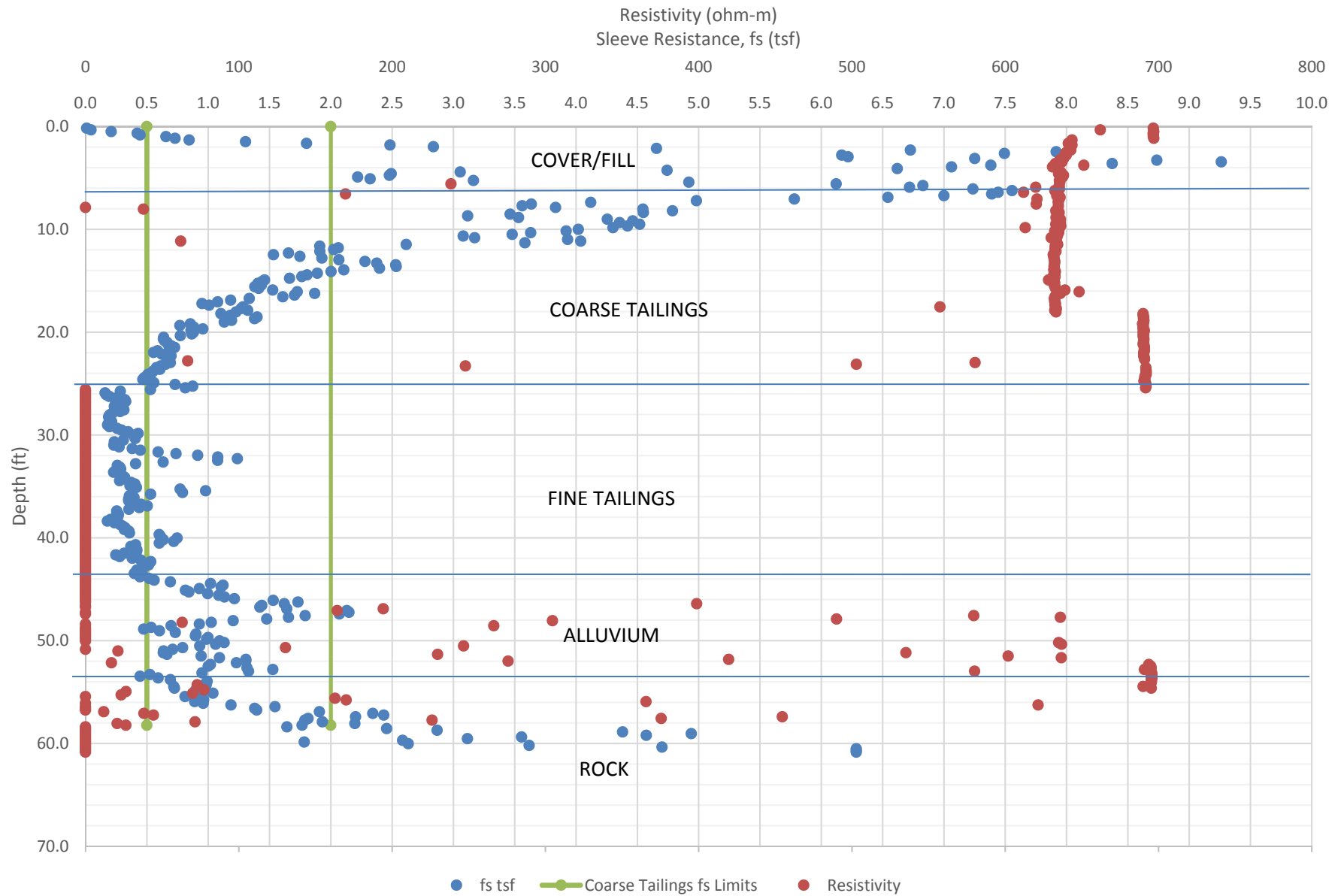
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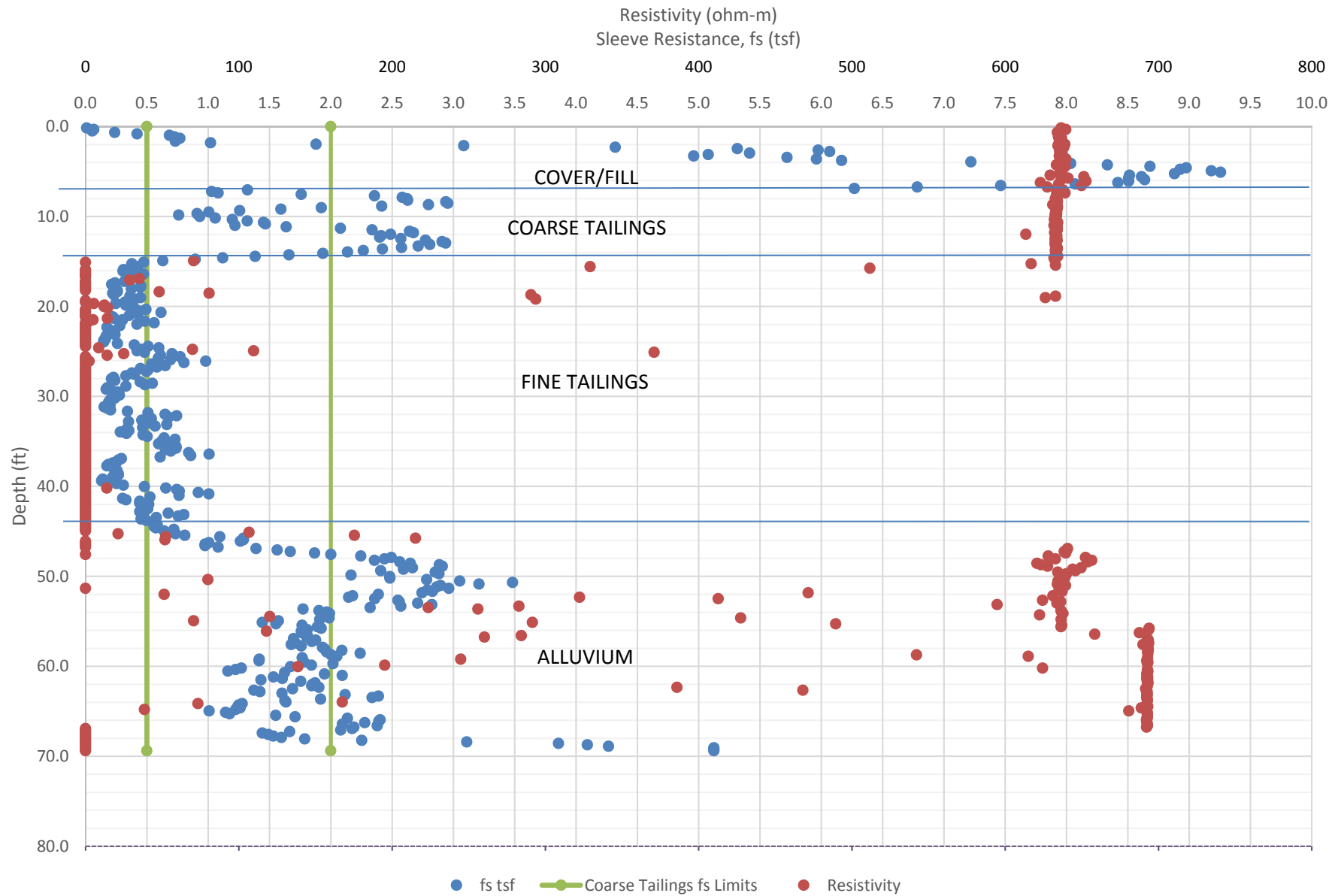
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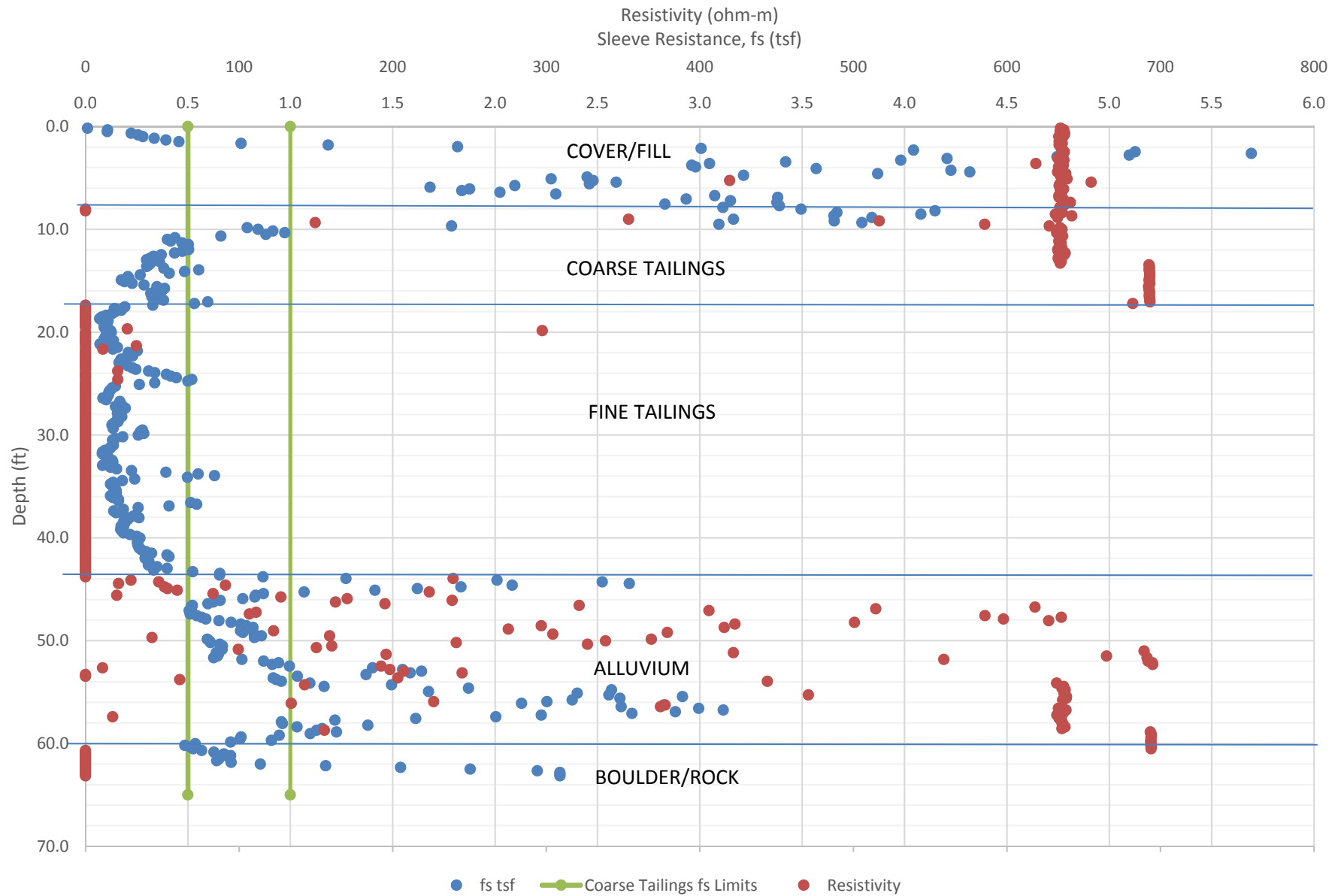
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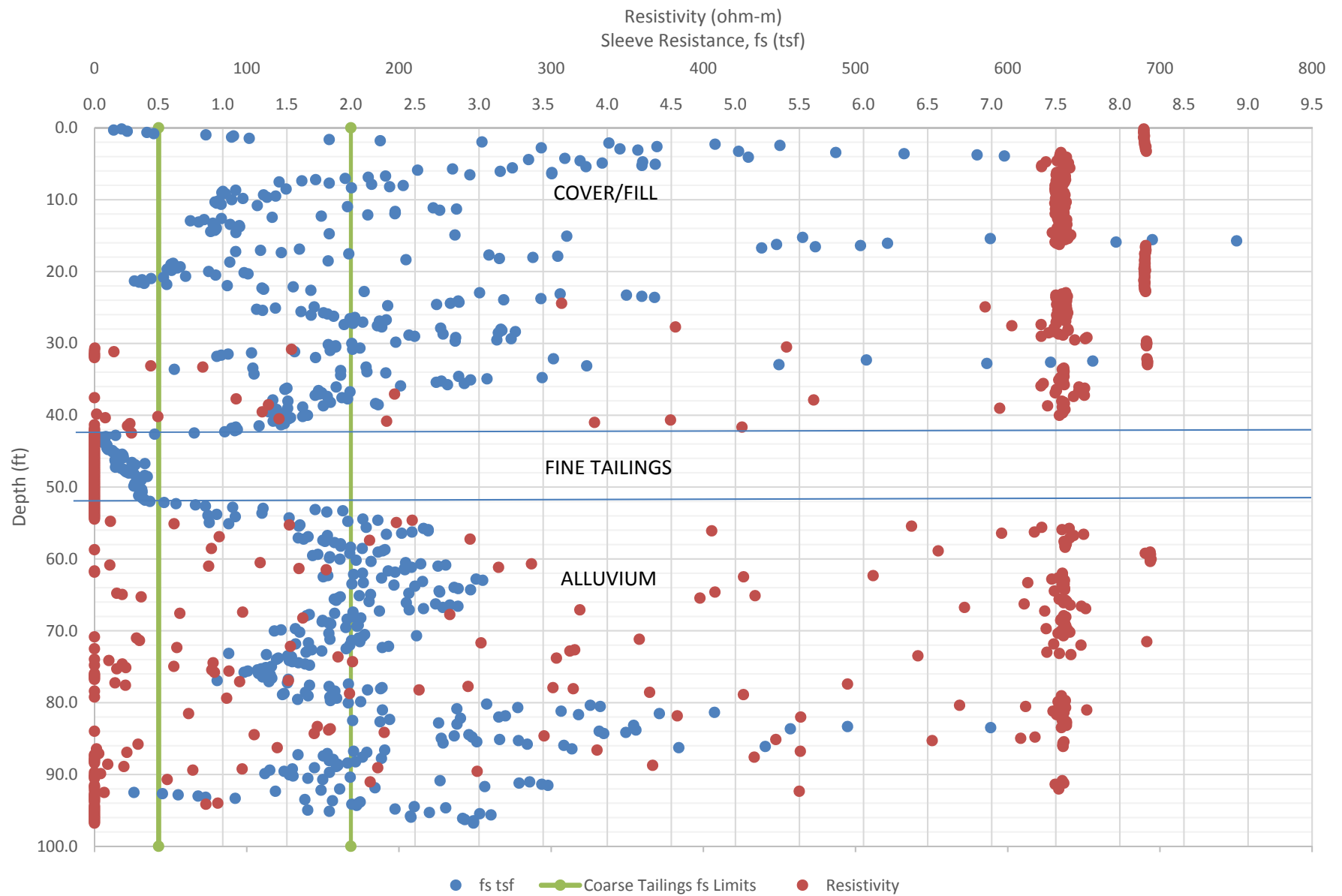
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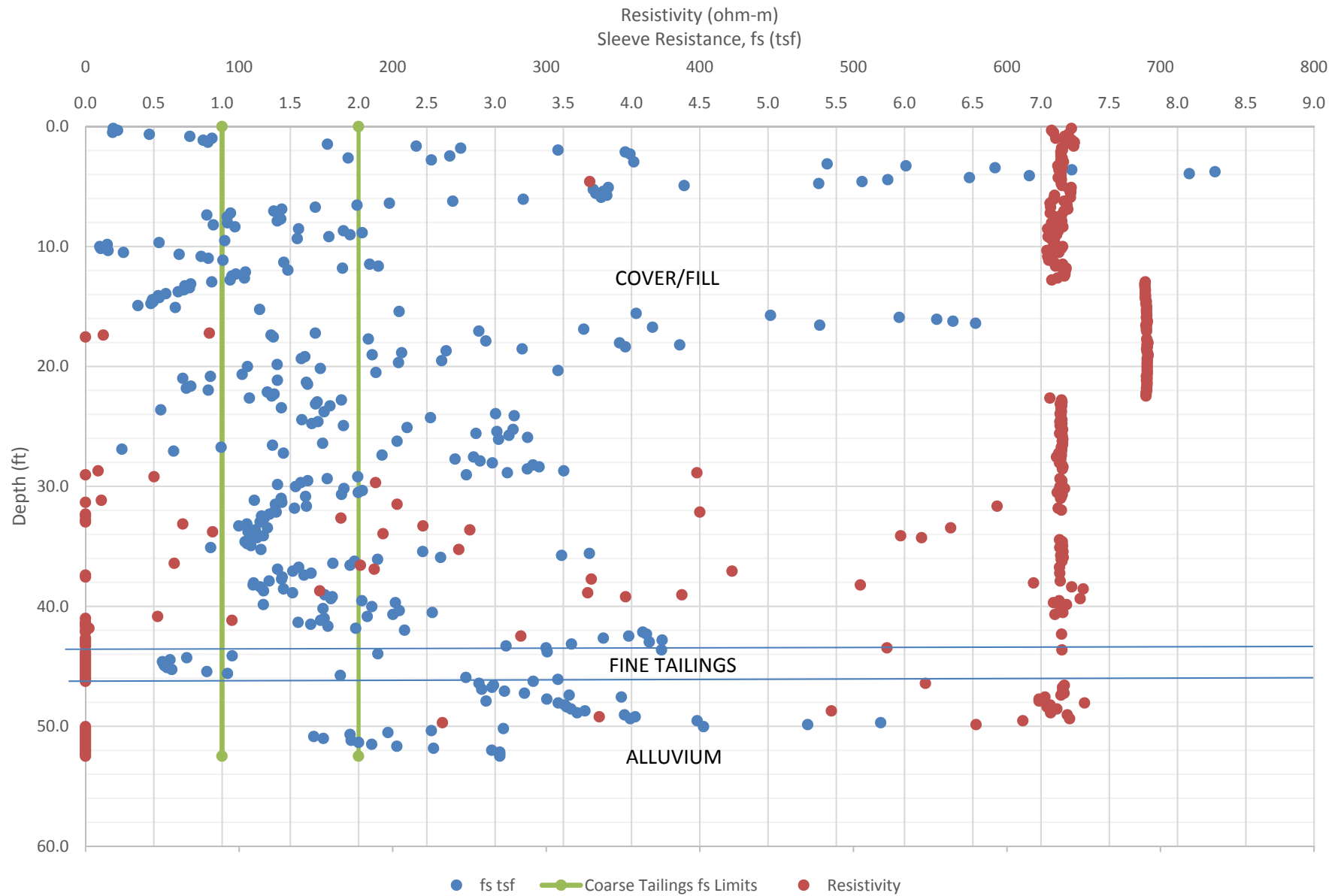
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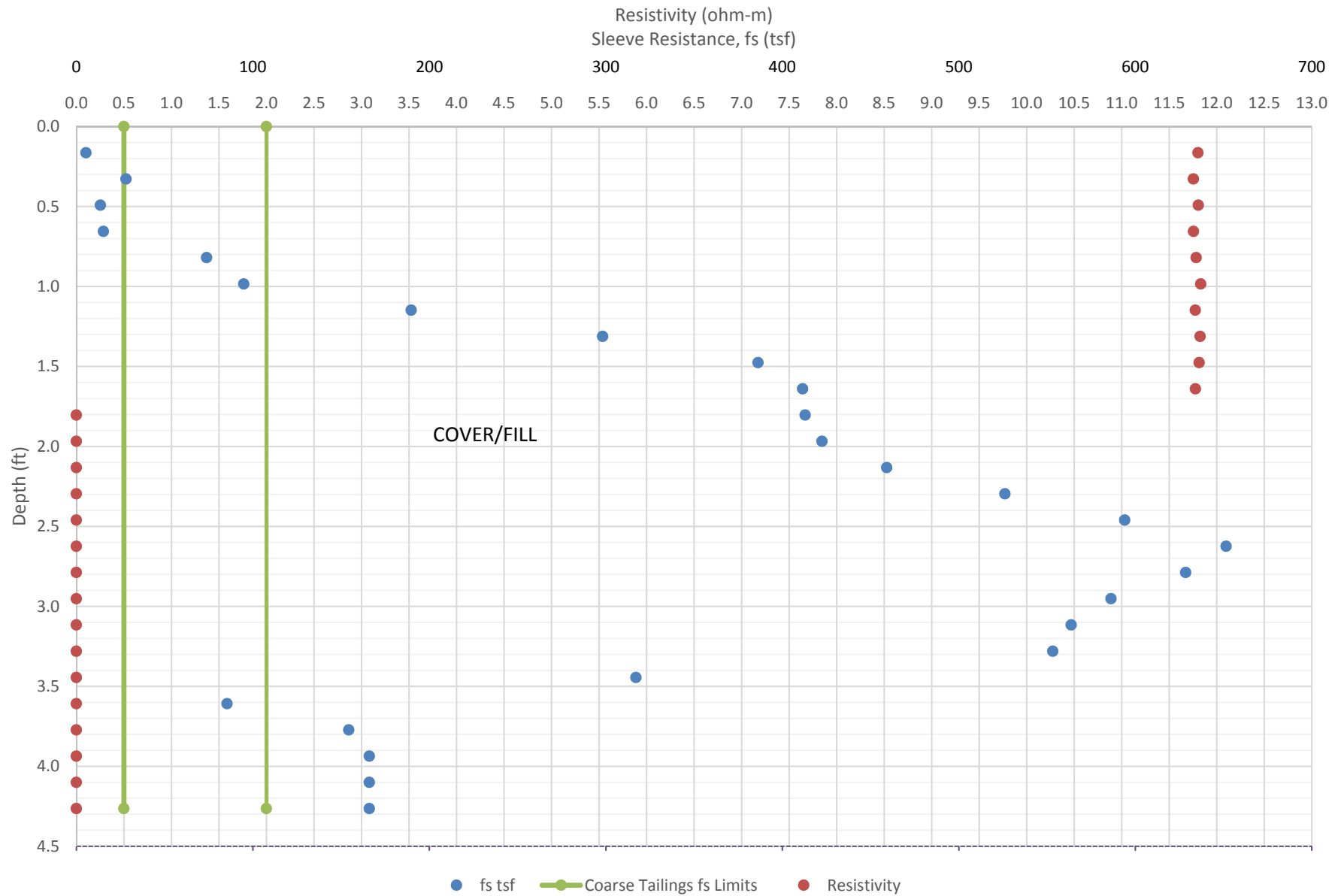
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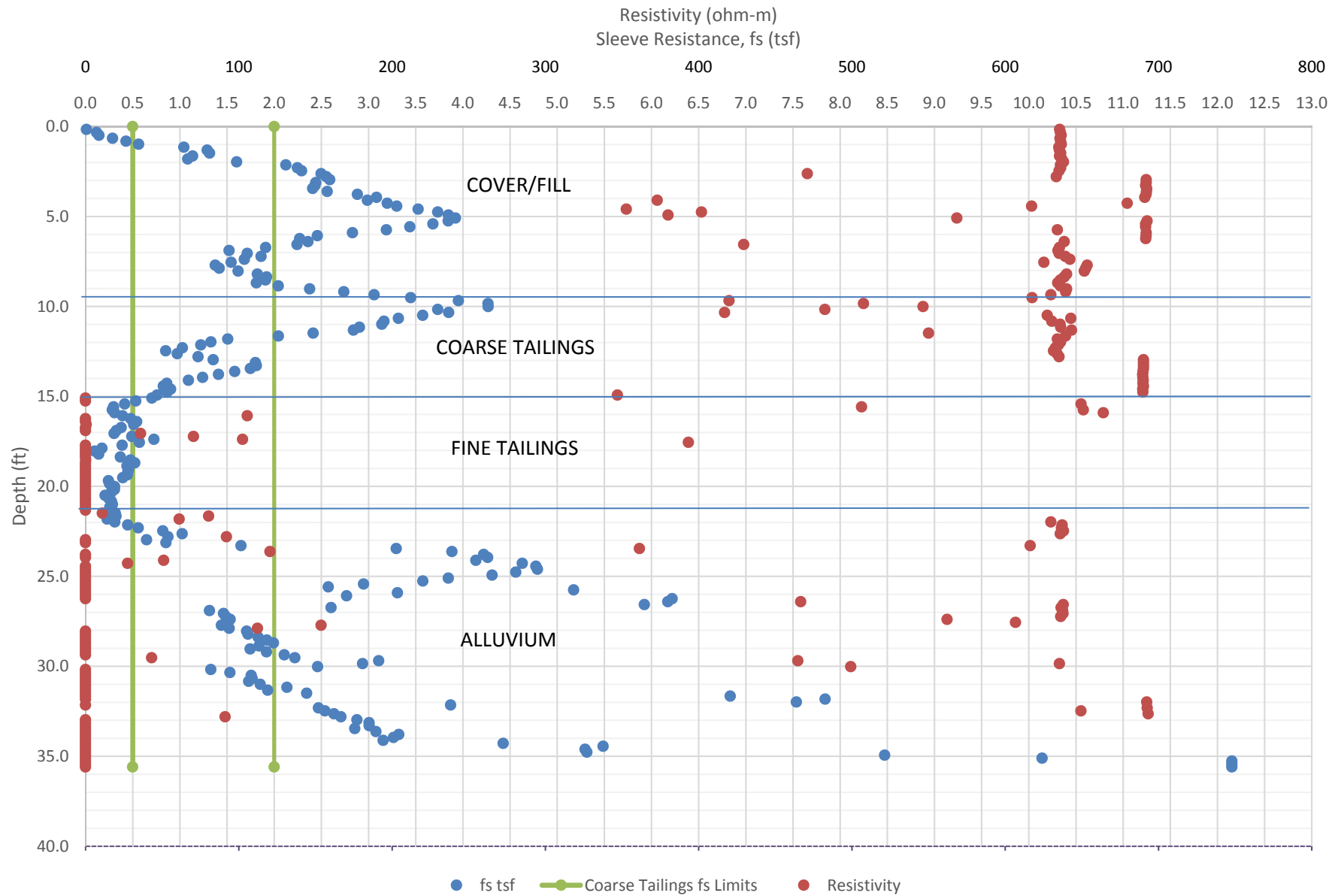
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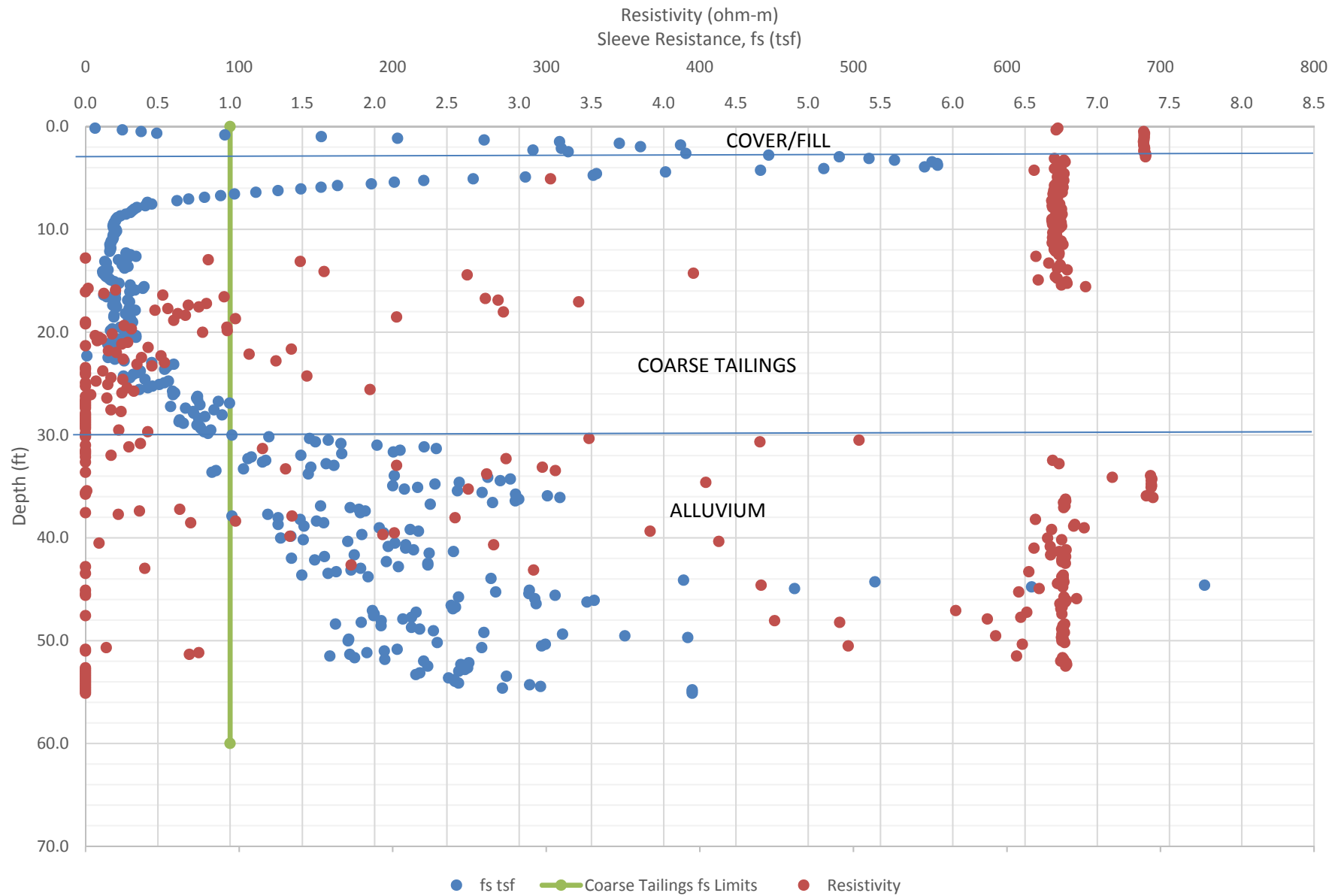
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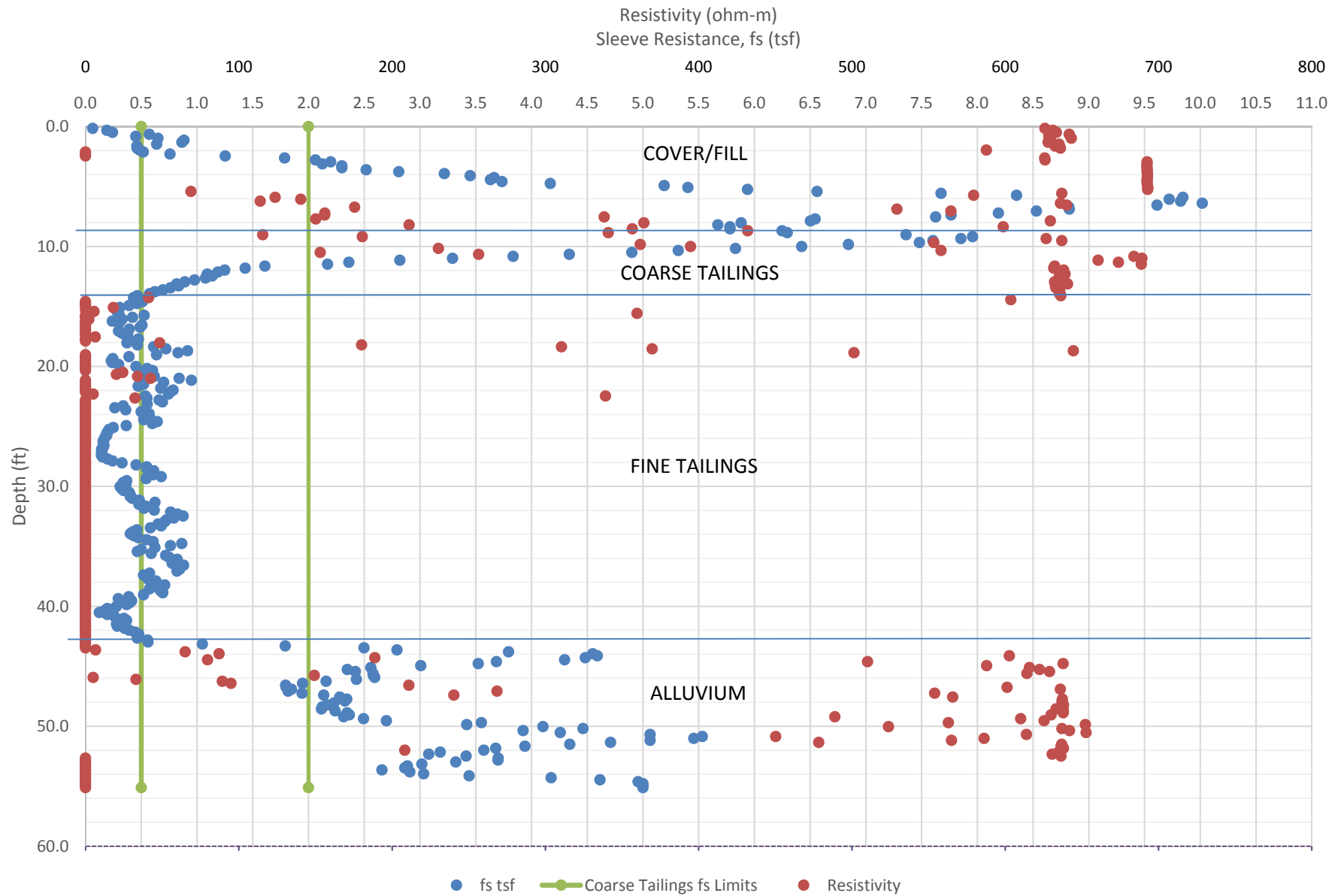
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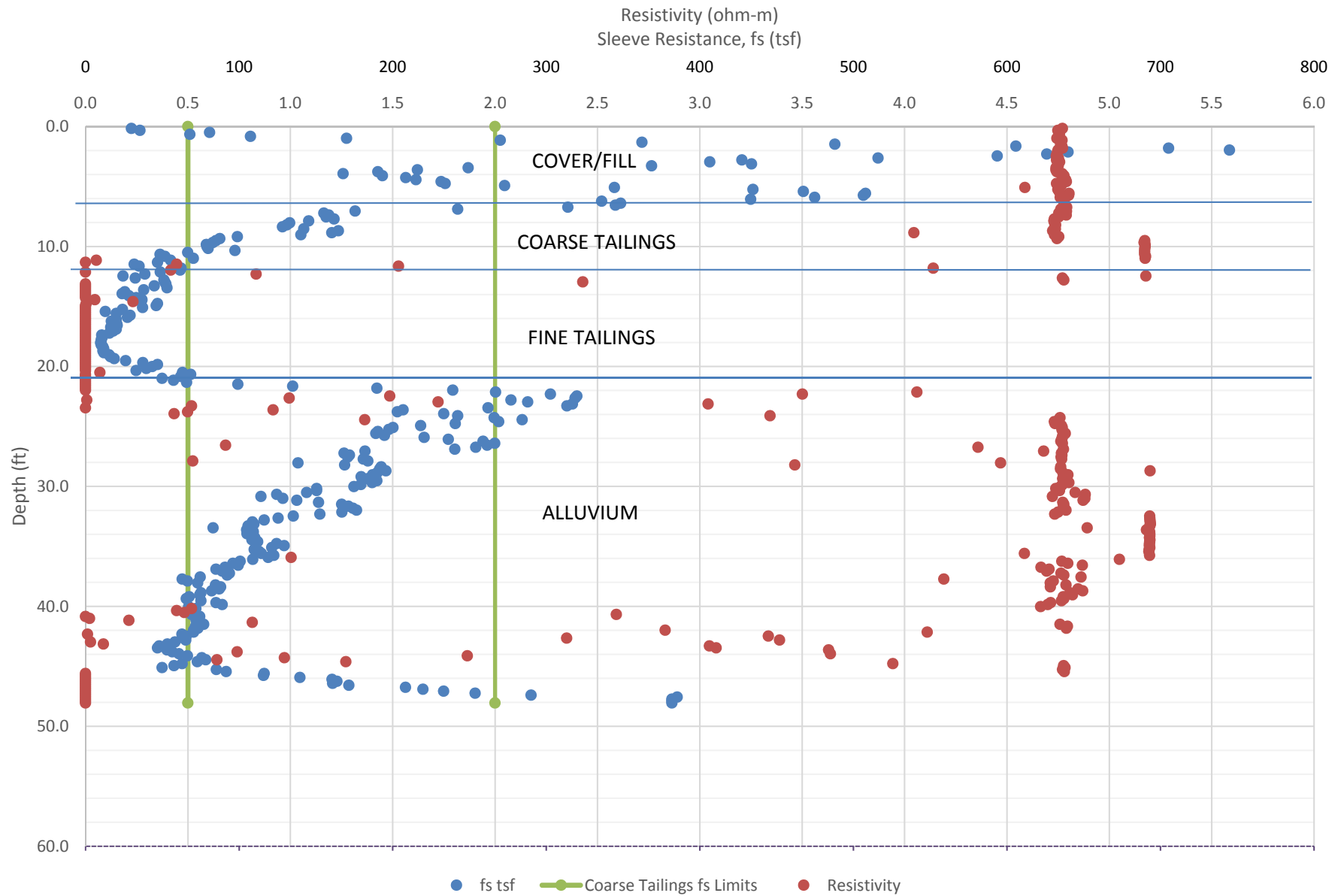
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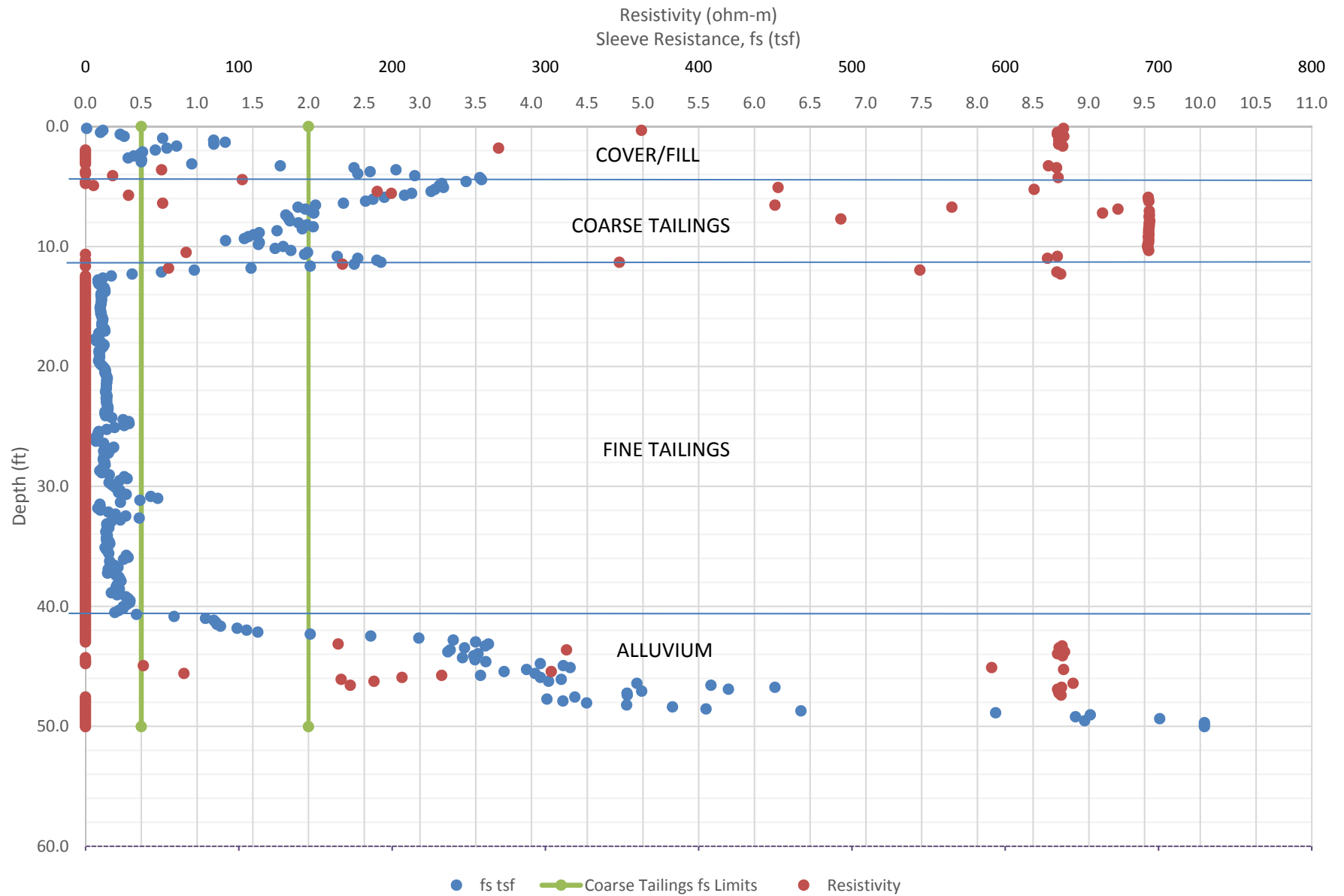
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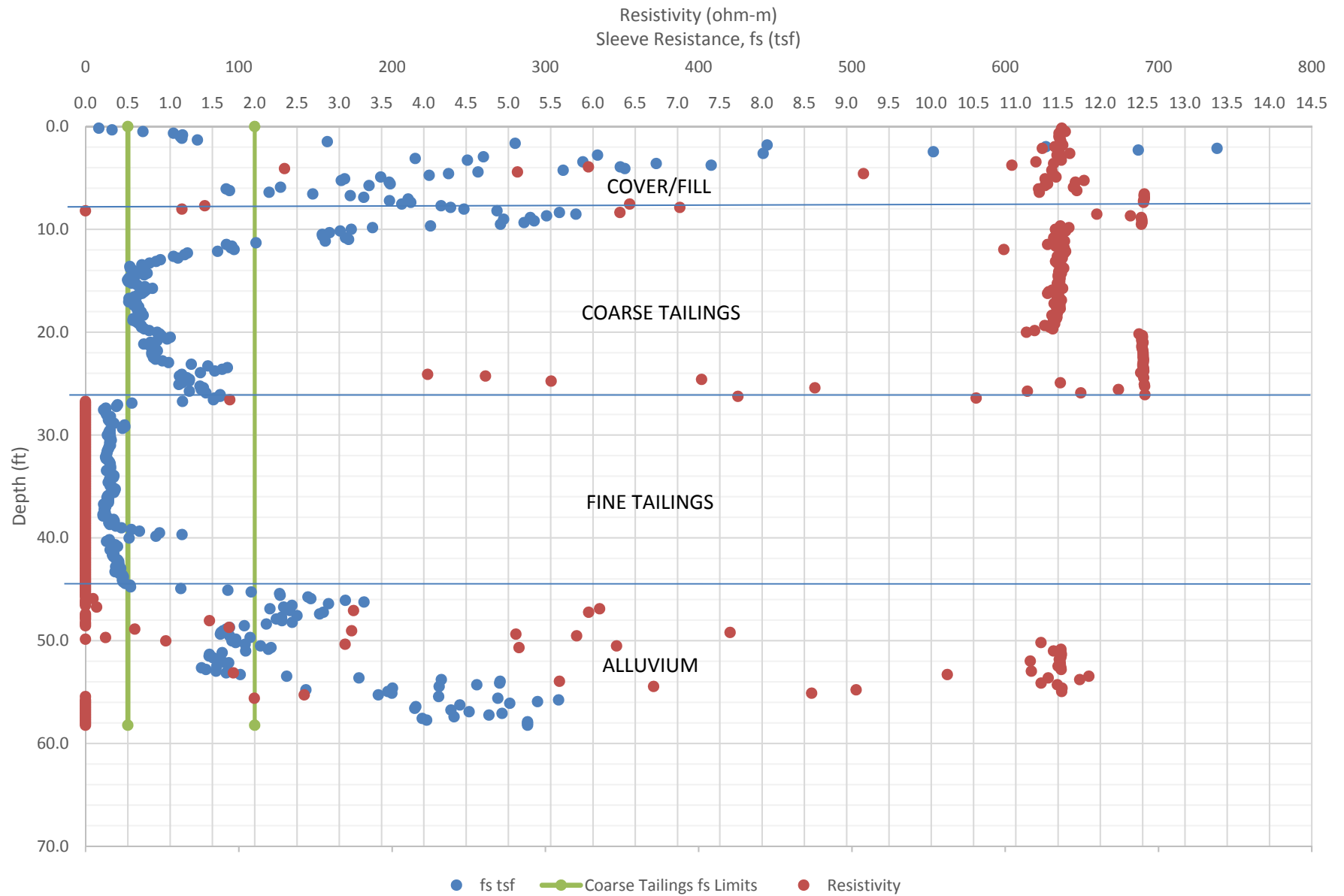
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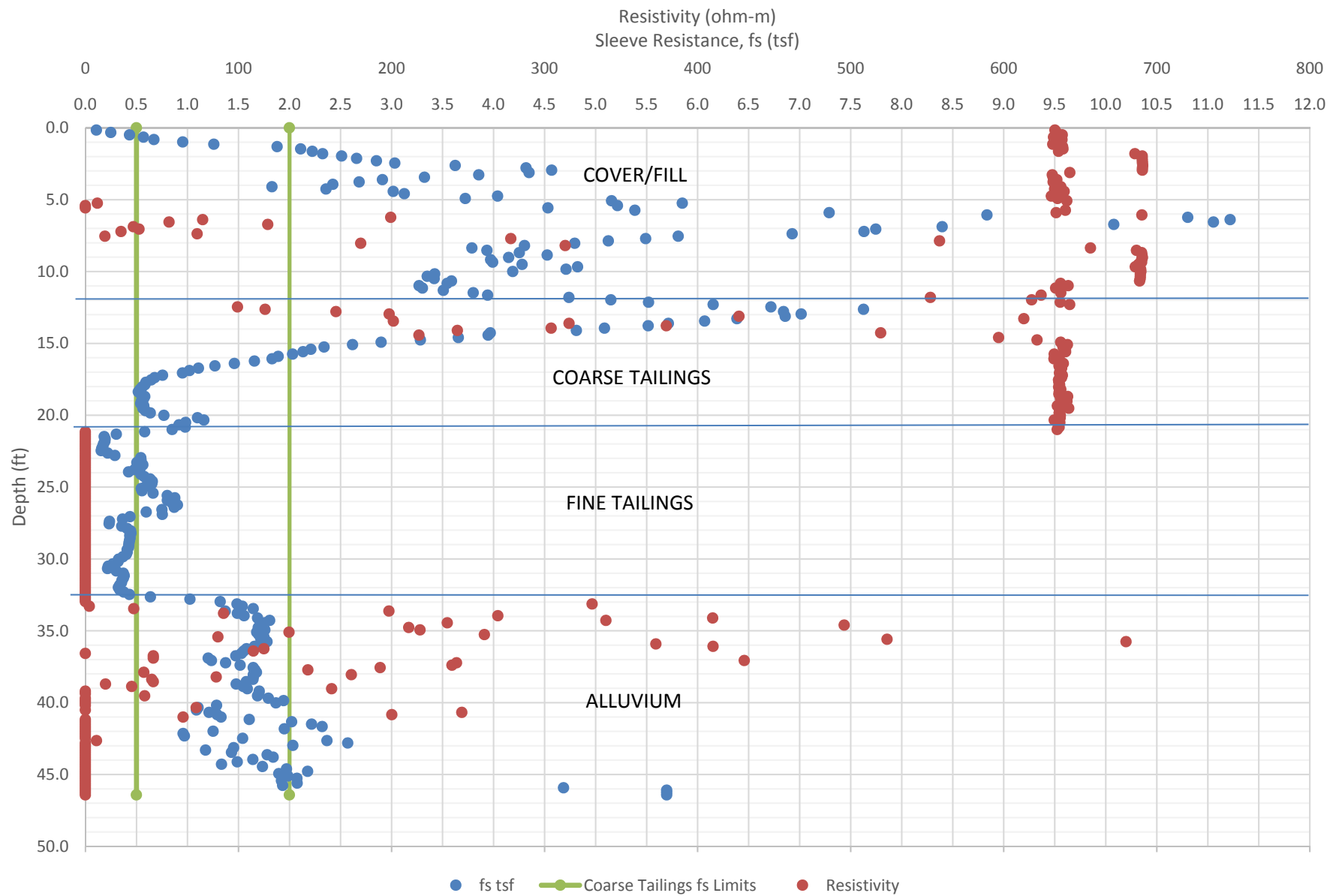
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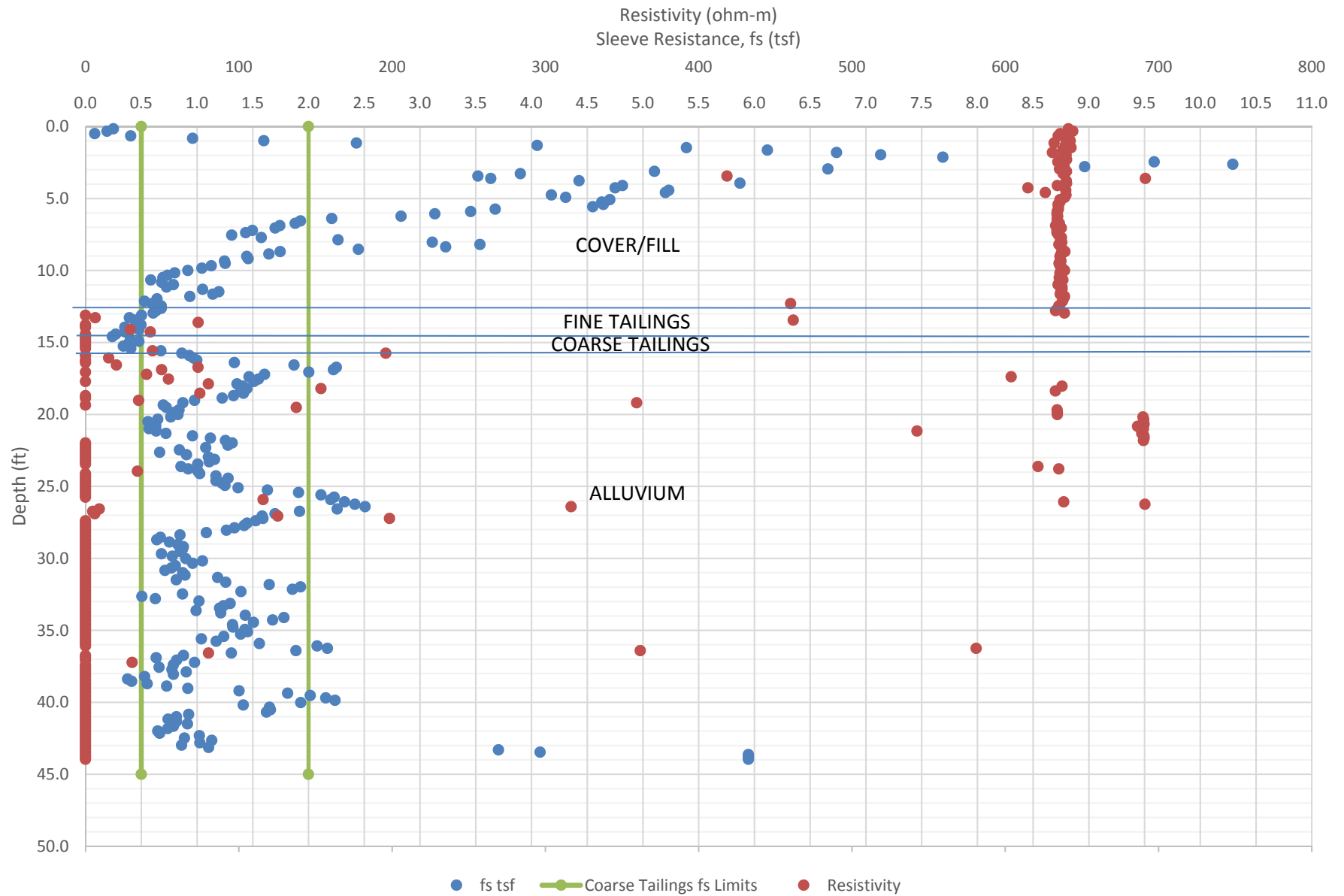
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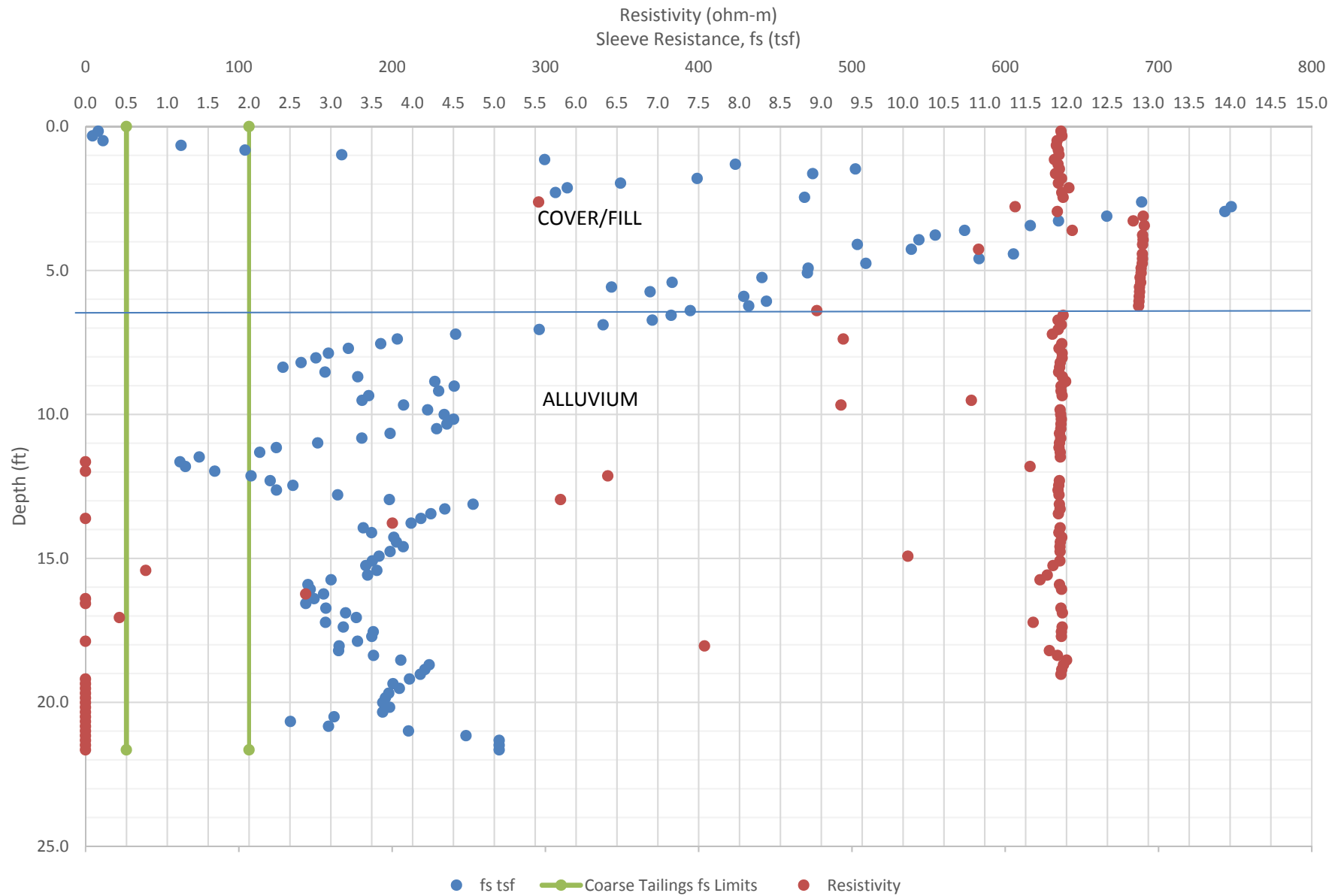
CPT-20



CPT-23

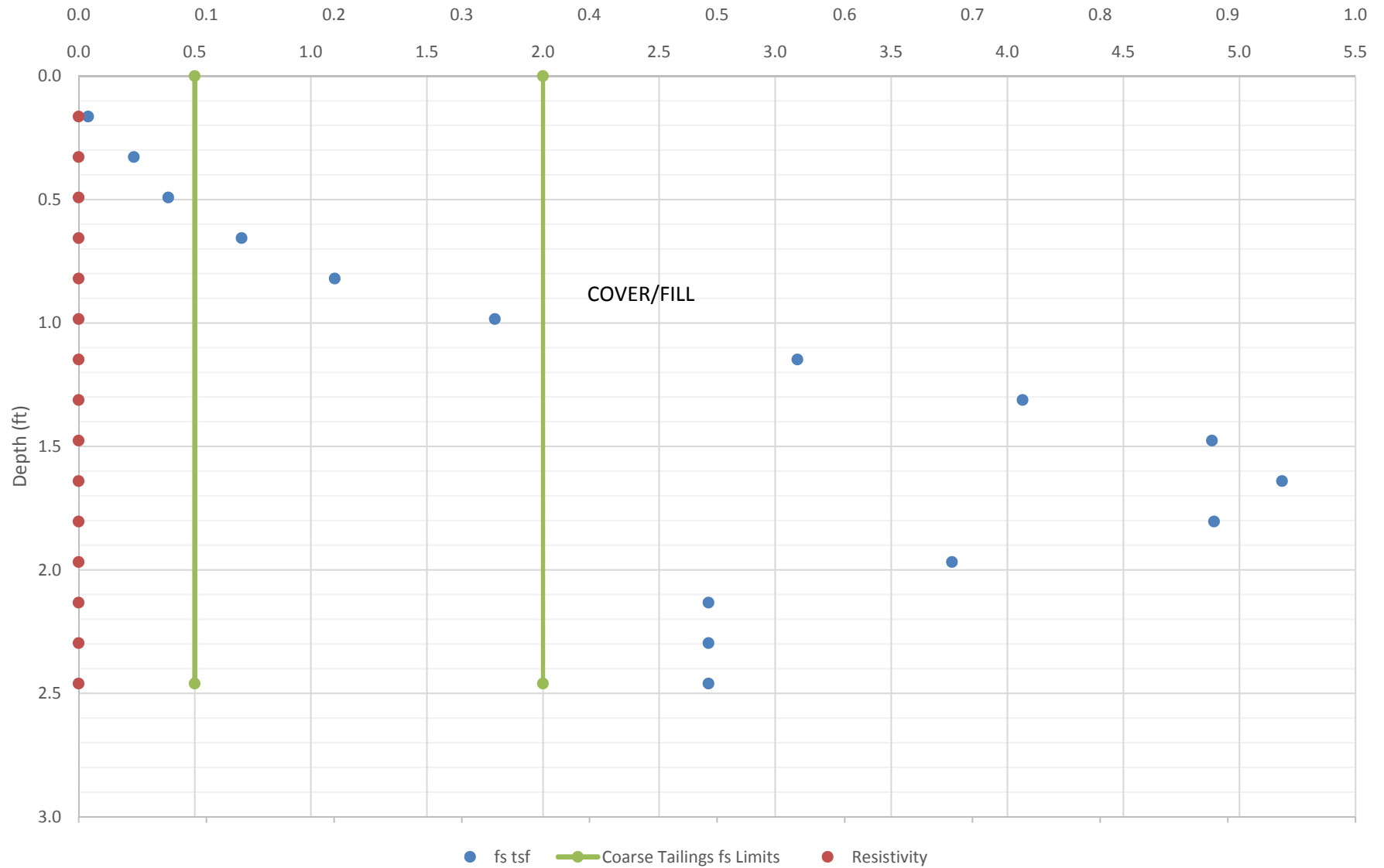


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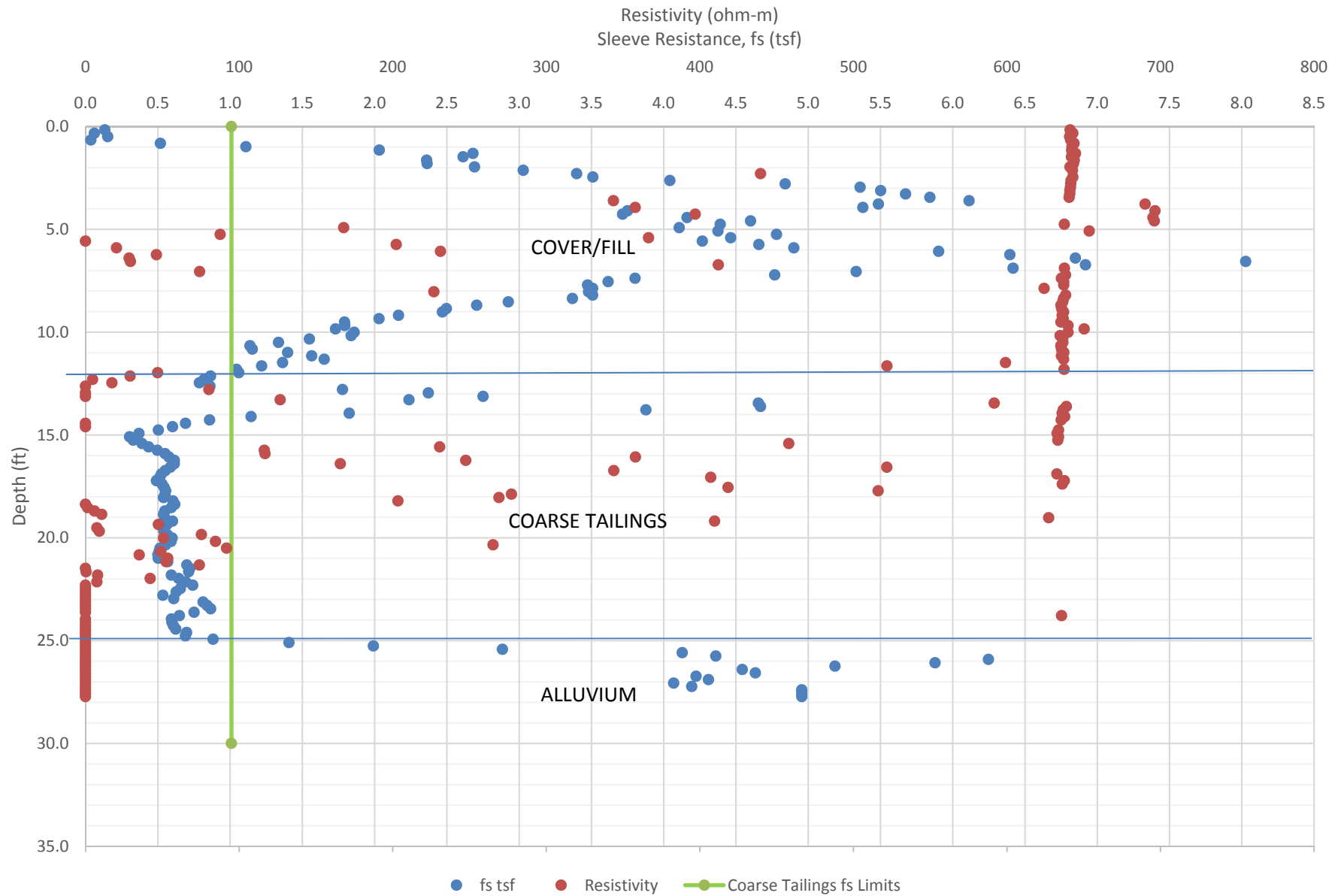


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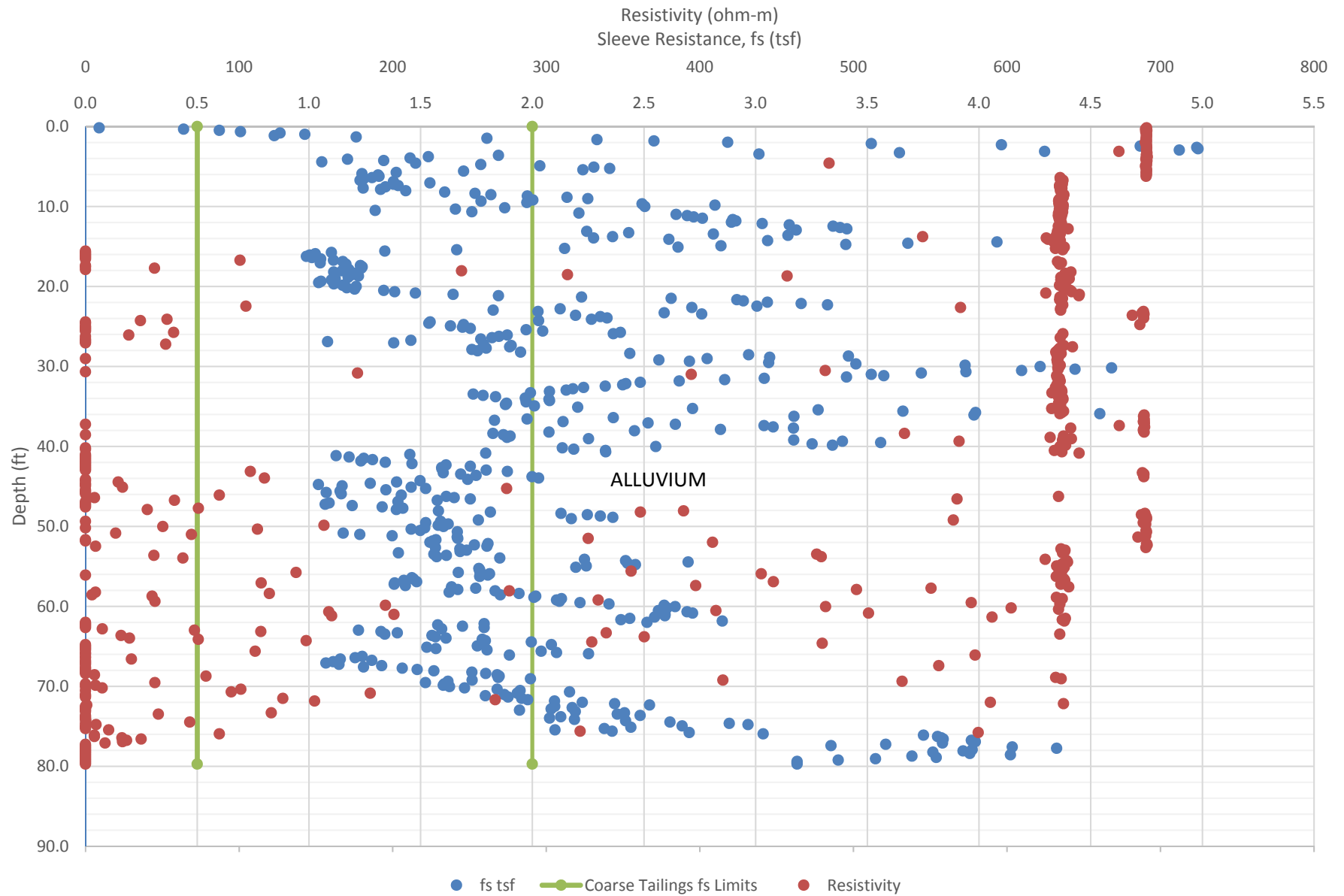
Resistivity (ohm-m)
Sleeve Resistance, fs (tsf)



CPT-26

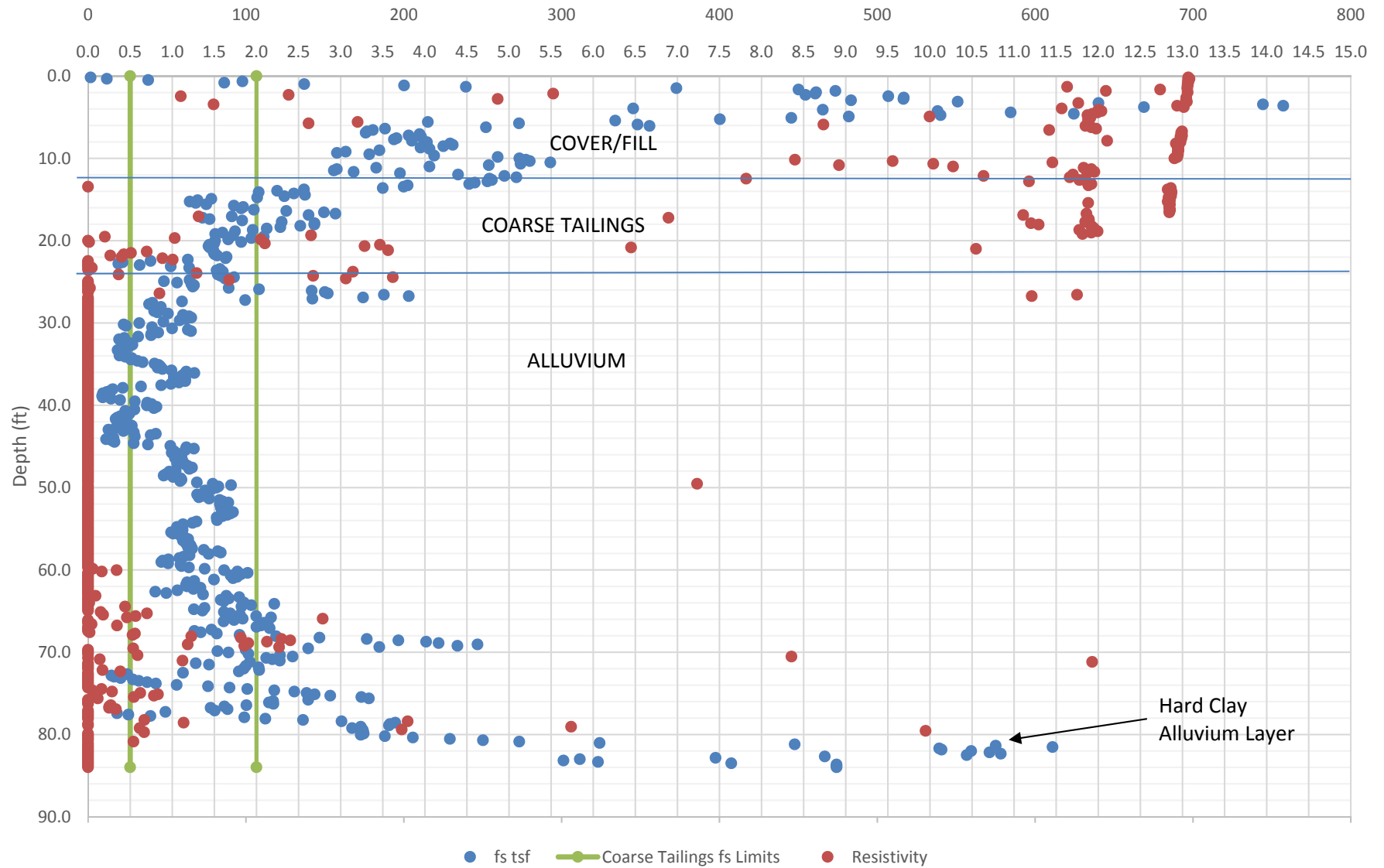


CPT-27

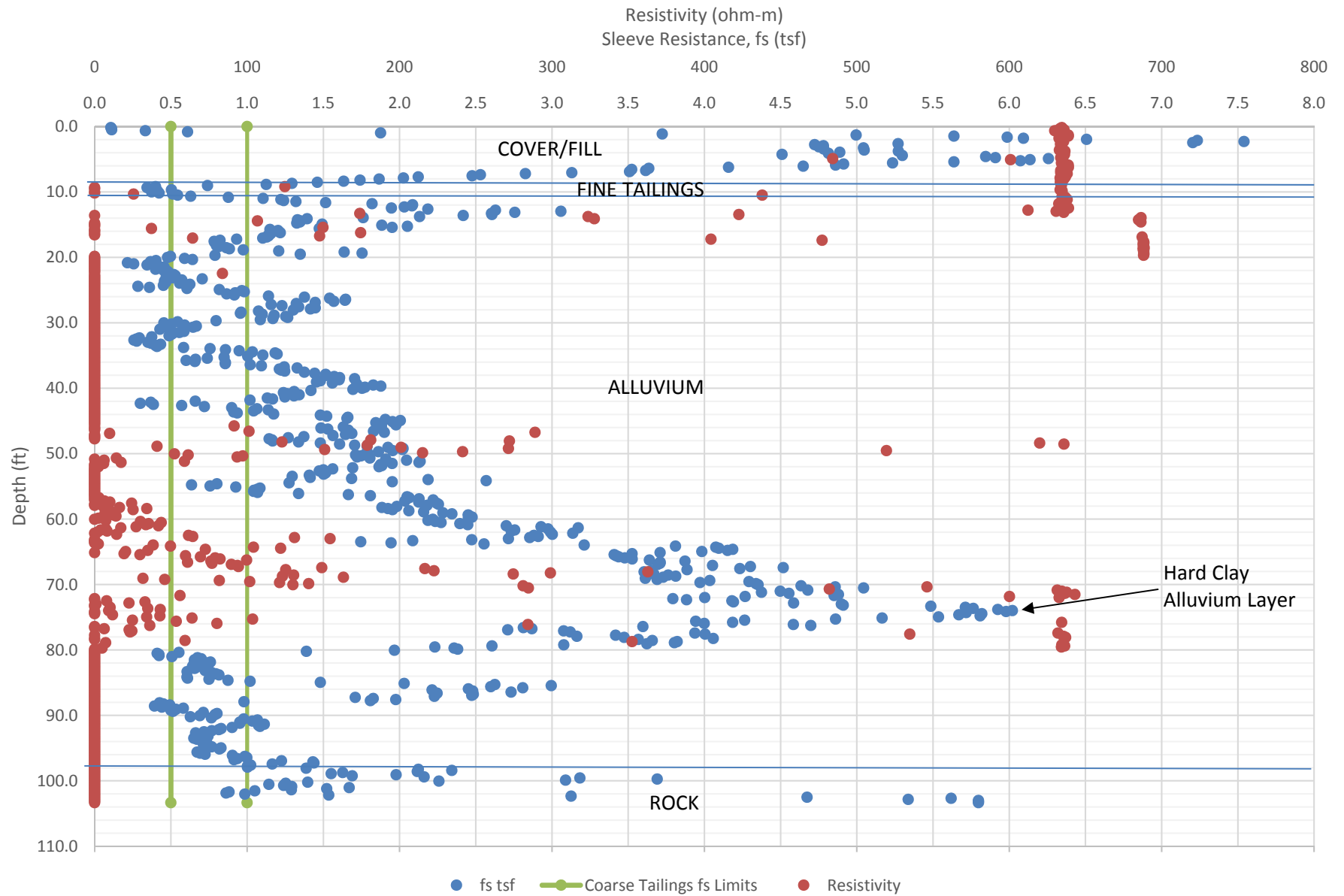


CPT-28

Resistivity (ohm-m)
Sleeve Resistance, fs (tsf)



CPT-29



ATTACHMENT C

CONSOLIDATION SPREADSHEET CALCULATIONS

Notes

t_0 corresponds to beginning of mine spoils and final cover placement
 t_1 corresponds to completion of settlement due to waste and cover placement.

Assumes 99% of consolidation due to existing stress conditions has taken place

SOIL PROPERTIES**TAILINGS****Specific Gravity, G_s**

- 2.67 Specific gravity of coarse tailings, G_{s-CT}
2.70 Specific gravity of fine tailings, G_{s-FT}

Average of results from laboratory testing of samples classified as "coarse tailings"
Average of results from laboratory testing of samples classified as "fine tailings"

Dry Unit Weight, γ_d

- 97.5 In-situ dry unit weight of coarse tailings at t_0 , γ_{d0-CT} (pcf)
71.7 In-situ dry unit weight of fine tailings at t_0 , γ_{d0-FT} (pcf)

Average of results from laboratory testing of samples classified as "coarse tailings"
Average of results from laboratory testing of samples classified as "fine tailings"

Saturated Unit Weight, γ_{sat}

- 123.4 In-situ saturated unit weight of coarse tailings at t_0 , $\gamma_{sat0-CT}$ (pcf)
107.5 In-situ saturated unit weight of fine tailings at t_0 , $\gamma_{sat0-FT}$ (pcf)

Calculated
Calculated

Moist Unit Weight, γ_m

- 108.1 Moist unit weight of coarse tailings, γ_{m-CT} (pcf)
107.6 Moist unit weight of fine tailings, γ_{m-FT} (pcf)

Calculated
Calculated, assuming 100% saturation

Void Ratio, e

- 0.71 Void ratio of coarse tailings at t_0 , e_{0-CT}
1.35 Void ratio of fine tailings at t_0 , e_{0-FT}

Calculated
Calculated

Saturated Water Content, w_{sat}

- 27.8% Saturated water content of coarse tailings at t_0 , $w_{sat0-CT}$ (%)
50.1% Saturated water content of fine tailings at t_0 , $w_{sat0-FT}$ (%)

Average of 12 specimens tested for SWCC
Average of 4 specimens tested for SWCC

Water Content of Moist Tailings, w_m

- 10.9% Water content of moist coarse tailings, w_{m-CT} (%)
50.1% Water content of moist fine tailings, w_{m-FT} (%)

Average of results from laboratory testing of samples classified as "coarse tailings"
Assumes 100% saturation of fine tailings

Compression Index, C_c

- 0.084 Compression index of coarse tailings, C_{c-CT}
0.408 Compression index of fine tailings, C_{c-FT}

Average of 4 results from laboratory testing of samples classified as "coarse tailings"
Average of 3 results from laboratory testing of samples classified as "fine tailings"

Other

- 62.4 Unit Weight of Water, γ_w

MINE SPOILS AND COVER MATERIALS**Specific Gravity, G_s**

- 2.71 Specific gravity of cover soil with rock admixture, $G_{s-rock-mix}$
2.69 Specific gravity of cover soil, $G_{s-cover}$
2.66 Specific gravity of mine spoils, $G_{s-spoils}$

Average of results of laboratory testing performed on samples from erosion protection gravel stockpiles and all borrow sources, weighted for 33% rock in the mixture
Average of results of laboratory testing performed on samples from all borrow sources, except the Dilco Hill borrow source
Average of results of laboratory testing performed on samples from mine spoils

Unit Weight, γ

- 130.0 Maximum dry unit weight of cover soil with rock admixture, $\gamma_{rock-mix-max}$ (pcf)
122.9 Moist unit weight of cover soil with rock admixture at 90% relative compaction, $\gamma_{rock-mix-90}$ (pcf)
115.0 Maximum dry unit weight of cover soil $\gamma_{cover-max}$ (pcf)
114.7 Moist unit weight of cover soil at 90% relative compaction, $\gamma_{cover90}$ (pcf)
118.3 Maximum dry unit weight of mine spoils, $\gamma_{spoils-max}$ (pcf)
116.4 Moist unit weight of mine spoils at 90% relative compaction, $\gamma_{spoils90}$ (pcf)

Assumed, based on the properties of materials and 33% rock by volume
Calculated
Average of results of laboratory Proctor testing performed on samples from all borrow sources, except the Dilco Hill borrow source
Calculated
Average of results of laboratory testing performed on samples from mine spoils
Calculated

Void Ratio, e

- 0.45 Void Ratio of cover soil with rock admixture at 90% relative compaction, $e_{rock-mix-90}$
0.62 Void Ratio of cover soil at 90% relative compaction, $e_{cover90}$
0.56 Void Ratio of mine spoils at 90% relative compaction, $e_{spoils90}$

Calculated
Calculated
Calculated

Long-Term Moisture Content, w

- 5.0% Long-term moisture content of cover soil with rock admixture, $w_{rock-mix}$ (%)
10.8% Long-term moisture content of cover soil, w_{cover} (%)
9.3% Long-term moisture content of mine spoils, w_{spoils} (%)

Assumed to be the same as the cover soil, average from the borrow samples
Average minus 3% of optimum, of results of laboratory Proctor testing performed on samples from all borrow sources, except the Dilco Hill borrow source
Average minus 3% of optimum water content based on results of laboratory testing performed on samples from mine spoils

| | | |
|--|---|---|
| Compression Index, C_c | | |
| | Compression index of cover soil with rock admixture, $C_{c-rock-mix}$ (%) | Assumed to be the same as coarse tailings |
| 0.086 | Compression index of cover soil, $C_{c-cover}$ (%) | Assumed to be the same as mine spoils |
| 0.086 | Compression index of mine spoils, $C_{c-spoils}$ (%) | Average of 5 results of laboratory testing performed on samples from alluvium located beneath the tailings impoundment. |

ALLUVIUM MATERIAL

| | | |
|--|--|---|
| Specific Gravity, G_s | | |
| 2.72 | | Average of results of laboratory testing performed on samples of alluvium |
| Unit Weight, γ | | |
| 97.9 | Dry unit weight of alluvium $\gamma_{alluvium}$ (pcf) | Average of results of laboratory testing performed on samples of alluvium |
| 114.8 | Moist unit weight of alluvium, $\gamma_{m-alluvium}$ (pcf) | Calculated |
| Void Ratio, e | | |
| 0.73 | | Calculated |
| In-place Moisture Content, w | | |
| 17.3 | | Average of results of laboratory testing performed on samples of alluvium |
| Compression Index, C_c | | |
| 0.09 | | Average of 5 results from laboratory testing performed on samples of alluvium |

SECONDARY COMPRESSION INDEX

| | | |
|---|---------------------------------------|--|
| Secondary Compression Index, C_α | | |
| 0.0017 | For loads < 1650 psf | Average of 3 consolidation test results from laboratory testing performed on fine tailings (TI-B8, TI-B10, and TI-B11) |
| 0.0036 | For loads < 3250 psf, but > 1650 psf | Average of 3 consolidation test results from laboratory testing performed on fine tailings (TI-B8, TI-B10, and TI-B11) |
| 0.0047 | For loads < 6500 psf, but > 3250 psf | Average of 3 consolidation test results from laboratory testing performed on fine tailings (TI-B8, TI-B10, and TI-B11) |
| 0.0087 | For loads < 13000 psf, but > 6500 psf | Average of 2 consolidation test results from laboratory testing performed on fine tailings (TI-B10, and TI-B11) |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-01(TI-B1)

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,692,256 | Northing coordinate of CPT sounding | |
| 2,524,535 | Easting coordinate of CPT sounding | |
| 6,972.1 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,970.0 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,935.7 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|----------------------------|
| 2.1 | Thickness of Erosion Protection Layer (ft) | B1 is beyond edge of waste |
| 0.0 | Thickness of General Fill Cover (ft) | B1 is beyond edge of waste |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6972.1 | 6971.0 | 6970.0 | 2.1 | N/A | N/A | 128.9 | 257.9 | 0.00 | 0.0% | 0.45 | 0.00 |
| 3 | Cover | 114.7 | 0.086 | 0.62 | 6970.0 | 6960.7 | 6951.5 | 18.5 | 1058.5 | 2117.1 | 1316.4 | 2374.9 | 0.00 | 0.0% | 0.62 | 0.00 |
| 4 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6951.5 | 6946.0 | 6940.4 | 11.1 | 2717.2 | 3317.3 | 2975.0 | 3575.1 | 0.00 | 0.0% | 0.62 | -0.09 |
| 5 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6940.4 | 6938.1 | 6935.7 | 4.7 | 3570.2 | 3823.1 | 3828.1 | 4081.0 | 0.02 | 0.5% | 0.61 | -0.74 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.02 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0047 | 4.7 | 0.05 |
| TOTAL SETTLEMENT: | | | | 0.05 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-02(TI-B2)

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,692,647 | Northing coordinate of CPT sounding | |
| 2,525,098 | Easting coordinate of CPT sounding | |
| 6,974.4 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,960.2 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,945.2 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.5 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.0 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6974.4 | 6973.7 | 6972.9 | 1.5 | N/A | N/A | 92.1 | 184.3 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6972.9 | 6971.4 | 6969.9 | 3.0 | N/A | N/A | 356.3 | 528.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6969.9 | 6964.8 | 6959.7 | 10.3 | N/A | N/A | 1126.1 | 1723.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6959.7 | 6953.5 | 6947.4 | 12.3 | 705.4 | 1410.9 | 2429.2 | 3134.7 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6947.4 | 6946.3 | 6945.2 | 2.2 | 1529.3 | 1647.7 | 3253.1 | 3371.4 | 0.13 | 5.7% | 0.53 | -0.82 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.13 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0047 | 2.2 | 0.03 |
| TOTAL SETTLEMENT: | | | | 0.03 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-04

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,692,397 | Northing coordinate of CPT sounding | | |
| 2,524,891 | Easting coordinate of CPT sounding | | |
| 6,977.8 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,965.0 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,945.2 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.5 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.0 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6977.8 | 6977.0 | 6976.3 | 1.5 | N/A | N/A | 92.1 | 184.3 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6976.3 | 6974.8 | 6973.3 | 3.0 | N/A | N/A | 356.3 | 528.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6973.3 | 6968.9 | 6964.5 | 8.8 | N/A | N/A | 1038.4 | 1548.4 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6964.5 | 6959.7 | 6955.0 | 9.5 | 543.9 | 1087.8 | 2092.3 | 2636.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6955.0 | 6952.0 | 6949.0 | 6.0 | 1412.2 | 1736.6 | 2960.6 | 3285.0 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6949.0 | 6947.1 | 6945.2 | 3.8 | 1942.9 | 2149.1 | 3491.3 | 3697.5 | 0.17 | 4.4% | 0.55 | -0.80 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.17 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 3.8 | 0.04 |
| TOTAL SETTLEMENT: | | | | 0.04 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-05

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,692,072 | Northing coordinate of CPT sounding | | |
| 2,525,535 | Easting coordinate of CPT sounding | | |
| 6,999.4 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,973.0 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,963.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , Z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , Z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $Z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6999.4 | 6998.8 | 6998.2 | 1.1 | N/A | N/A | 69.1 | 138.2 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6998.2 | 6996.5 | 6994.8 | 3.4 | N/A | N/A | 333.2 | 528.1 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6994.8 | 6983.7 | 6972.5 | 22.3 | N/A | N/A | 1826.2 | 3124.4 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6972.5 | 6969.8 | 6967.0 | 5.5 | 317.0 | 634.1 | 3441.4 | 3758.4 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6967.0 | 6965.0 | 6963.0 | 4.0 | 851.4 | 1068.8 | 3975.8 | 4193.2 | 0.00 | 0.0% | 0.62 | -0.09 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-06

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,925 | Northing coordinate of CPT sounding | |
| 2,525,897 | Easting coordinate of CPT sounding | |
| 6,997.7 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,974.2 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,940.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6997.7 | 6997.1 | 6996.6 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6996.6 | 6994.9 | 6993.2 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6993.2 | 6983.4 | 6973.7 | 19.5 | N/A | N/A | 1658.2 | 2791.3 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6973.7 | 6970.5 | 6967.3 | 6.4 | 367.0 | 733.9 | 3158.2 | 3525.2 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6967.3 | 6963.2 | 6959.0 | 8.3 | 1182.7 | 1631.4 | 3973.9 | 4422.7 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6959.0 | 6949.5 | 6940.0 | 19.0 | 2654.1 | 3676.7 | 5445.3 | 6468.0 | 1.03 | 5.4% | 0.53 | -0.82 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 1.03 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 19.0 | 0.22 |
| TOTAL SETTLEMENT: | | | | 0.22 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-08(TI-B8)

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,691,558 | Northing coordinate of CPT sounding | | |
| 2,525,841 | Easting coordinate of CPT sounding | | |
| 6,990.3 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,976.0 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,931.6 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6990.3 | 6989.7 | 6989.2 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6989.2 | 6987.5 | 6985.8 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6985.8 | 6980.7 | 6975.5 | 10.2 | N/A | N/A | 1119.4 | 1713.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6975.5 | 6972.3 | 6969.1 | 6.4 | 369.5 | 739.0 | 2083.3 | 2452.8 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6969.1 | 6959.5 | 6949.8 | 19.3 | 1782.4 | 2825.8 | 3496.2 | 4539.6 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6949.8 | 6940.7 | 6931.6 | 18.2 | 3805.2 | 4784.6 | 5519.0 | 6498.3 | 0.51 | 2.8% | 0.58 | -0.77 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.51 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 18.2 | 0.21 |
| TOTAL SETTLEMENT: | | | | 0.21 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-09

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,743 | Northing coordinate of CPT sounding | |
| 2,525,574 | Easting coordinate of CPT sounding | |
| 6,993.0 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,975.5 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,930.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6993.0 | 6992.5 | 6991.9 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6991.9 | 6990.2 | 6988.5 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6988.5 | 6981.8 | 6975.0 | 13.5 | N/A | N/A | 1310.4 | 2095.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6975.0 | 6971.8 | 6968.6 | 6.4 | 367.7 | 735.4 | 2463.5 | 2831.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6968.6 | 6964.4 | 6960.2 | 8.4 | 1189.6 | 1643.7 | 3285.4 | 3739.5 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6960.2 | 6945.1 | 6930.0 | 30.2 | 3268.8 | 4893.9 | 5364.6 | 6989.7 | 1.13 | 3.7% | 0.56 | -0.79 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | 1.13 | | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 30.2 | 0.34 |
| TOTAL SETTLEMENT: | | | 0.34 | |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-10(TI-B10)

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,691,776 | Northing coordinate of CPT sounding | | |
| 2,525,854 | Easting coordinate of CPT sounding | | |
| 6,994.6 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,973.6 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,930.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6994.6 | 6994.0 | 6993.5 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6993.5 | 6991.8 | 6990.1 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6990.1 | 6981.6 | 6973.1 | 17.0 | N/A | N/A | 1514.0 | 2503.0 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6973.1 | 6970.0 | 6967.0 | 6.1 | 349.7 | 699.3 | 2852.7 | 3202.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6967.0 | 6961.2 | 6955.4 | 11.6 | 1326.4 | 1953.6 | 3829.5 | 4456.6 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6955.4 | 6942.7 | 6930.0 | 25.4 | 3320.4 | 4687.2 | 5823.4 | 7190.2 | 1.08 | 4.2% | 0.55 | -0.80 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | 1.08 | | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 25.4 | 0.29 |
| TOTAL SETTLEMENT: | | | | 0.29 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-11(TI-B11)

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,691,704 | Northing coordinate of CPT sounding | | |
| 2,526,258 | Easting coordinate of CPT sounding | | |
| 6,981.7 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,977.4 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,920.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6981.7 | 6981.2 | 6980.6 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6980.6 | 6978.9 | 6977.2 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6977.2 | 6977.1 | 6976.9 | 0.3 | N/A | N/A | 541.0 | 557.0 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6976.9 | 6954.4 | 6931.9 | 45.0 | 2582.0 | 5164.1 | 3139.1 | 5721.1 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6931.9 | 6926.0 | 6920.0 | 11.9 | 5804.4 | 6444.8 | 6361.5 | 7001.8 | 0.08 | 0.7% | 0.61 | -0.74 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.08 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0047 | 11.9 | 0.14 |
| TOTAL SETTLEMENT: | | | | 0.14 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-12

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,527 | Northing coordinate of CPT sounding | |
| 2,526,216 | Easting coordinate of CPT sounding | |
| 6,983.8 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,978.9 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,932.4 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6983.8 | 6983.3 | 6982.7 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6982.7 | 6981.0 | 6979.3 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6979.3 | 6978.9 | 6978.4 | 0.9 | N/A | N/A | 576.8 | 628.5 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6978.4 | 6956.7 | 6934.9 | 43.5 | 2496.3 | 4992.6 | 3124.8 | 5621.1 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6934.9 | 6933.7 | 6932.4 | 2.5 | 5127.1 | 5261.7 | 5755.6 | 5890.2 | 0.02 | 0.9% | 0.61 | -0.74 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.02 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0047 | 2.5 | 0.03 |
| TOTAL SETTLEMENT: | | | | 0.03 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-13

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,692,229 | Northing coordinate of CPT sounding | |
| 2,525,213 | Easting coordinate of CPT sounding | |
| 6,994.4 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,969.2 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| - | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6994.4 | 6993.8 | 6993.3 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6993.3 | 6991.6 | 6989.9 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6989.9 | 6979.3 | 6968.7 | 21.1 | N/A | N/A | 1754.8 | 2984.6 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6968.7 | 6966.9 | 6965.1 | 3.6 | 208.3 | 416.5 | 3192.8 | 3401.1 | 0.00 | 0.0% | 0.62 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-14

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,691,554 | Northing coordinate of CPT sounding | | |
| 2,525,656 | Easting coordinate of CPT sounding | | |
| 6,988.9 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,979.7 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,957.3 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6988.9 | 6988.4 | 6987.8 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6987.8 | 6986.1 | 6984.4 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6984.4 | 6981.8 | 6979.2 | 5.2 | N/A | N/A | 829.4 | 1133.7 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6979.2 | 6974.6 | 6970.0 | 9.2 | 526.0 | 1052.1 | 1659.7 | 2185.7 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6970.0 | 6966.8 | 6963.5 | 6.5 | 1403.5 | 1754.9 | 2537.1 | 2888.5 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6963.5 | 6960.4 | 6957.3 | 6.3 | 2091.2 | 2427.5 | 3224.9 | 3561.2 | 0.20 | 3.3% | 0.57 | -0.78 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.20 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0036 | 6.3 | 0.05 |
| TOTAL SETTLEMENT: | | | | 0.05 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-15(TI-15)

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,690 | Northing coordinate of CPT sounding | |
| 2,525,091 | Easting coordinate of CPT sounding | |
| 6,986.7 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,976.6 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,946.7 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{cl-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6986.7 | 6986.2 | 6985.6 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6985.6 | 6983.9 | 6982.2 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6982.2 | 6979.2 | 6976.1 | 6.1 | N/A | N/A | 879.2 | 1233.4 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6976.1 | 6975.1 | 6974.0 | 2.1 | 123.0 | 246.0 | 1356.4 | 1479.4 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6974.0 | 6960.3 | 6946.7 | 27.4 | 1724.6 | 3203.3 | 2958.0 | 4436.7 | 0.00 | 0.0% | 0.62 | -0.09 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-16

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,911 | Northing coordinate of CPT sounding | |
| 2,525,665 | Easting coordinate of CPT sounding | |
| 6,996.6 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,973.3 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,930.2 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6996.6 | 6996.1 | 6995.5 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6995.5 | 6993.8 | 6992.1 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6992.1 | 6982.5 | 6972.8 | 19.4 | N/A | N/A | 1651.9 | 2778.7 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6972.8 | 6968.7 | 6964.7 | 8.1 | 463.3 | 926.6 | 3242.0 | 3705.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6964.7 | 6961.4 | 6958.2 | 6.5 | 1278.0 | 1629.4 | 4056.7 | 4408.1 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6958.2 | 6944.2 | 6930.2 | 28.0 | 3137.2 | 4645.0 | 5915.9 | 7423.7 | 1.34 | 4.8% | 0.54 | -0.81 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 1.34 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 28.0 | 0.32 |
| TOTAL SETTLEMENT: | | | | 0.32 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-17

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,692,058 | Northing coordinate of CPT sounding | | |
| 2,526,096 | Easting coordinate of CPT sounding | | |
| 6,988.9 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,975.6 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,955.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6988.9 | 6988.4 | 6987.8 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6987.8 | 6986.1 | 6984.4 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6984.4 | 6979.8 | 6975.1 | 9.3 | N/A | N/A | 1067.8 | 1610.6 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6975.1 | 6972.2 | 6969.2 | 5.9 | 338.2 | 676.4 | 1948.7 | 2286.9 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6969.2 | 6966.2 | 6963.1 | 6.1 | 1004.5 | 1332.6 | 2615.0 | 2943.2 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6963.1 | 6959.1 | 6955.0 | 8.2 | 1772.3 | 2212.0 | 3382.8 | 3822.5 | 0.40 | 4.9% | 0.54 | -0.81 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.40 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 8.2 | 0.09 |
| TOTAL SETTLEMENT: | | | | 0.09 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-18

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,833 | Northing coordinate of CPT sounding | |
| 2,526,009 | Easting coordinate of CPT sounding | |
| 6,993.6 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,972.4 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,930.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6993.6 | 6993.1 | 6992.5 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6992.5 | 6990.8 | 6989.1 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6989.1 | 6980.5 | 6971.9 | 17.3 | N/A | N/A | 1529.7 | 2534.3 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6971.9 | 6969.8 | 6967.8 | 4.1 | 234.7 | 469.4 | 2769.0 | 3003.7 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6967.8 | 6964.2 | 6960.6 | 7.2 | 859.6 | 1249.8 | 3393.9 | 3784.2 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6960.6 | 6945.3 | 6930.0 | 30.6 | 2894.7 | 4539.5 | 5429.0 | 7073.8 | 1.45 | 4.7% | 0.54 | -0.80 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 1.45 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 30.6 | 0.35 |
| TOTAL SETTLEMENT: | | | | 0.35 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-19

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,691,633 | Northing coordinate of CPT sounding | | |
| 2,525,915 | Easting coordinate of CPT sounding | | |
| 6,992.0 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,975.3 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,930.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6992.0 | 6991.4 | 6990.9 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6990.9 | 6989.2 | 6987.5 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6987.5 | 6981.2 | 6974.8 | 12.7 | N/A | N/A | 1262.5 | 1999.9 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6974.8 | 6970.6 | 6966.3 | 8.5 | 488.6 | 977.3 | 2488.6 | 2977.2 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6966.3 | 6957.5 | 6948.7 | 17.6 | 1926.2 | 2875.1 | 3926.1 | 4875.1 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6948.7 | 6939.4 | 6930.0 | 18.7 | 3883.8 | 4892.5 | 5883.8 | 6892.4 | 0.59 | 3.1% | 0.57 | -0.78 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.59 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 18.7 | 0.21 |
| TOTAL SETTLEMENT: | | | | 0.21 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-20

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,691,422 | Northing coordinate of CPT sounding | |
| 2,525,862 | Easting coordinate of CPT sounding | |
| 6,985.5 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,979.1 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6,947.7 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6985.5 | 6984.9 | 6984.4 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6984.4 | 6982.7 | 6981.0 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6981.0 | 6979.8 | 6978.6 | 2.3 | N/A | N/A | 661.4 | 797.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6978.6 | 6972.7 | 6966.8 | 11.8 | 676.8 | 1353.5 | 1474.6 | 2151.4 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6966.8 | 6962.3 | 6957.8 | 9.0 | 1841.7 | 2329.9 | 2639.6 | 3127.8 | 0.00 | 0.0% | 0.62 | -0.09 |
| 6 | Fine Tailings | 107.6 | 0.408 | 1.35 | 6957.8 | 6952.7 | 6947.7 | 10.1 | 2874.2 | 3418.5 | 3672.0 | 4216.3 | 0.19 | 1.8% | 0.59 | -0.76 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | 0.19 | | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 6 | Fine Tailings | 0.0047 | 10.1 | 0.12 |
| TOTAL SETTLEMENT: | | | | 0.12 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-23(TI-23)

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1,692,864 | Northing coordinate of CPT sounding | | |
| 2,525,720 | Easting coordinate of CPT sounding | | |
| 6,977.3 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6,959.5 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6,943.5 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.5 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.0 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6977.3 | 6976.5 | 6975.8 | 1.5 | N/A | N/A | 92.1 | 184.3 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6975.8 | 6974.3 | 6972.8 | 3.0 | N/A | N/A | 356.3 | 528.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6972.8 | 6965.9 | 6959.0 | 13.7 | N/A | N/A | 1328.2 | 2128.1 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6959.0 | 6952.6 | 6946.1 | 12.9 | 739.7 | 1479.3 | 2867.7 | 3607.4 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5* | Fine Tailings | 107.6 | 0.408 | 1.35 | 6946.1 | 6945.7 | 6945.3 | 0.8 | 43.0 | 86.1 | 3650.5 | 3693.5 | 0.00 | 0.0% | 1.35 | 0.00 |
| 6 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6945.3 | 6944.4 | 6943.5 | 1.8 | 183.4 | 280.7 | 3790.8 | 3888.1 | 0.00 | 0.0% | 0.71 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

Notes:

*Assumed that the fine tailings layer is unsaturated at this location and no settlement will occur

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0047 | 0.8 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-24

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1692567.3 | Northing coordinate of CPT sounding | |
| 2525413 | Easting coordinate of CPT sounding | |
| 6987.33 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6962.28 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| - | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.5 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.0 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6987.3 | 6986.6 | 6985.8 | 1.5 | N/A | N/A | 92.1 | 184.3 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6985.8 | 6984.3 | 6982.8 | 3.0 | N/A | N/A | 356.3 | 528.3 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6982.8 | 6972.3 | 6961.8 | 21.1 | N/A | N/A | 1753.4 | 2978.5 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6961.8 | 6958.6 | 6955.3 | 6.4 | 369.1 | 738.1 | 3347.5 | 3716.6 | 0.00 | 0.0% | 0.62 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | 0.00 | | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | 0.00 | |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-25

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1,692,436 | Northing coordinate of CPT sounding | |
| 2,525,693 | Easting coordinate of CPT sounding | |
| 6,998.6 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6,969.3 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| - | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidtion of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{cl-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|--|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6998.6 | 6998.1 | 6997.5 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6997.5 | 6995.8 | 6994.1 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6994.1 | 6981.6 | 6969.1 | 25.0 | N/A | N/A | 1981.2 | 3437.4 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6969.1 | 6969.1 | 6969.1 | 0.0 | 0.0 | 0.0 | 3437.4 | 3437.4 | 0.00 | 0.0% | 0.62 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-26

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1691992.5 | Northing coordinate of CPT sounding | | |
| 2524854.3 | Easting coordinate of CPT sounding | | |
| 6985.2 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6972.7 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6947.2 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6985.2 | 6984.7 | 6984.1 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6984.1 | 6982.4 | 6980.7 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6980.7 | 6976.5 | 6972.2 | 8.5 | N/A | N/A | 1017.9 | 1510.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6972.2 | 6966.2 | 6960.2 | 12.0 | 688.1 | 1376.1 | 2198.8 | 2886.9 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6960.2 | 6953.7 | 6947.2 | 13.0 | 2079.0 | 2781.8 | 3589.7 | 4292.5 | 0.00 | 0.0% | 0.62 | -0.09 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-27

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1691670.7 | Northing coordinate of CPT sounding | |
| 2526106.9 | Easting coordinate of CPT sounding | |
| 6989.0 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6977.0 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| - | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 1.1 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 3.4 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{cl-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6989.0 | 6988.5 | 6987.9 | 1.1 | N/A | N/A | 67.6 | 135.1 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6987.9 | 6986.2 | 6984.5 | 3.4 | N/A | N/A | 330.1 | 525.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6984.5 | 6980.5 | 6976.5 | 8.0 | N/A | N/A | 990.4 | 1455.8 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6976.5 | 6969.3 | 6962.0 | 14.5 | 831.4 | 1662.8 | 2287.2 | 3118.6 | 0.00 | 0.0% | 0.62 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | 0.00 | | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | 0.00 | |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-28

LOCATION INFORMATION

| | | |
|-----------|---|---|
| 1692484.7 | Northing coordinate of CPT sounding | |
| 2524698.6 | Easting coordinate of CPT sounding | |
| 6968.5 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file |
| 6963.7 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file |
| 6937.3 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file |

FINAL COVER

| | | |
|-----|--|---------|
| 2.3 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 2.2 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 , due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6968.5 | 6967.3 | 6966.2 | 2.3 | N/A | N/A | 138.2 | 276.4 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6966.2 | 6965.1 | 6964.0 | 2.2 | N/A | N/A | 402.6 | 528.7 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6964.0 | 6963.6 | 6963.2 | 0.8 | N/A | N/A | 573.2 | 617.7 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6963.2 | 6957.0 | 6950.8 | 12.5 | 714.4 | 1428.8 | 1332.1 | 2046.5 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5 | Coarse Tailings | 108.1 | 0.084 | 0.71 | 6950.8 | 6944.1 | 6937.3 | 13.5 | 2156.0 | 2883.3 | 2773.8 | 3501.0 | 0.00 | 0.0% | 0.62 | -0.09 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| NA | Fine Tailings | 0.0000 | 0.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-29

LOCATION INFORMATION

| | | | |
|-----------|---|---|--|
| 1693003.9 | Northing coordinate of CPT sounding | | |
| 2525274.5 | Easting coordinate of CPT sounding | | |
| 6965.9 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | From cover design grading plan AutoCAD file | |
| 6957.5 | Existing Ground Surface Elevation (ft amsl) | From AutoCAD file | |
| 6948.0 | Elevation of Base of Tailings (ft amsl) | From AutoCAD file | |

FINAL COVER

| | | |
|-----|--|---------|
| 2.6 | Thickness of Erosion Protection Layer (ft) | Assumed |
| 1.9 | Thickness of General Fill Cover (ft) | Assumed |

PRIMARY CONSOLIDATION

| Soil Layer | Material Type | Unit Weight, γ (pcf) | Compression Index, C_c | Void Ratio at t_0 , e_0 | Elevation at Top of Layer at t_0 , z_{i-top0} (ft amsl) | Elevation at Midpoint of Layer at t_0 , z_{i-mid0} (ft amsl) | Elevation at Bottom of Layer at t_0 , $z_{i-bott0}$ (ft amsl) ¹ | Thickness of Layer at t_0 , H (ft) | Effective Stress at Midpoint of Layer at t_0 , σ'_{i-mid0} (psf) | Effective Stress at Bottom of Layer at t_0 , $\sigma'_{i-bott0}$ (psf) | Effective Stress at Midpoint of Layer at t_1 , σ'_{i-mid1} (psf) | Effective Stress at Bottom of Layer at t_1 , $\sigma'_{i-bott1}$ (psf) | Consolidation of Layer from t_0 to t_1 due to Placement of Mine Spoils and Final Cover, δ_{ci-t1} (ft) | % Settlement | Void Ratio at t_1 , e_1 | Change in Void Ratio, Δe |
|-------------------|--------------------|-----------------------------|--------------------------|-----------------------------|---|--|--|--------------------------------------|---|--|---|--|---|--------------|-----------------------------|----------------------------------|
| 1 | Erosion Protection | 122.9 | 0.000 | 0.45 | 6965.9 | 6964.6 | 6963.3 | 2.6 | N/A | N/A | 161.2 | 322.5 | 0.00 | 0.0% | 0.45 | 0.00 |
| 2 | Cover | 114.7 | 0.086 | 0.62 | 6963.3 | 6962.4 | 6961.4 | 1.9 | N/A | N/A | 430.0 | 537.5 | 0.00 | 0.0% | 0.62 | 0.00 |
| 3 | Mine Spoils | 116.4 | 0.086 | 0.56 | 6961.4 | 6959.2 | 6957.0 | 4.4 | N/A | N/A | 796.0 | 1054.6 | 0.00 | 0.0% | 0.56 | 0.00 |
| 4 | Cover | 114.7 | 0.086 | 0.62 | 6957.0 | 6953.0 | 6949.0 | 8.0 | 458.7 | 917.4 | 1513.3 | 1972.0 | 0.00 | 0.0% | 0.62 | 0.00 |
| 5* | Fine Tailings | 107.6 | 0.408 | 1.35 | 6949.0 | 6948.5 | 6948.0 | 1.0 | 971.5 | 1025.6 | 2026.1 | 2080.2 | 0.00 | 0.0% | 1.35 | 0.00 |
| TOTAL SETTLEMENT: | | | | | | | | | | | | | 0.00 | | | |

Notes:

*Assumed that the fine tailings layer is unsaturated at this location and no settlement will occur

SECONDARY CONSOLIDATION

| | |
|-------|---|
| 0.8 | Time for completion of primary settlement, t_1 (years) |
| 200.0 | Time from completion of primary settlement, t_2 (years) |

| Soil Layer | Material Type | Secondary Compression Index, C_a | Thickness of Layer at t_0 , H (ft) | Secondary Consolidation, S_s (ft) |
|-------------------|---------------|------------------------------------|--------------------------------------|-------------------------------------|
| 5 | Fine Tailings | 0.0036 | 1.0 | 0.00 |
| TOTAL SETTLEMENT: | | | | 0.00 |

ATTACHMENT D
IMMEDIATE SETTLEMENT CALCULATIONS

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-01 Settlement

IMMEDIATE SETTLEMENT

CPT Profile Information

| | Thickness (ft) | E (ksf) | v | γ (psf) | |
|---------------------------------|----------------|-----------------|-----------------|---------|-------------------------|
| Erosion Protection (repository) | 2.1 | | | 122.9 | |
| Cover Fill (repository) | 0.0 | | | 114.7 | |
| Mine Spoils (repository) | 0.0 | | | 116.4 | |
| Radon Barrier (existing) | 2.5 | 450 | 0.3 | 122.3 | Hard, moist, sandy clay |
| Existing Fill (existing) | 16.0 | 680 | 0.35 | 113.8 | |
| Coarse Tailings (existing) | 11.1 | 150 | 0.3 | 108.1 | Silty sand tailings |
| | | McCarthy (1998) | McCarthy (1998) | | |

Total Fill Height2.1 above radon barrier

Soil Properties (McCarthy, 1998):

| Soil Type | E/N | E/qc |
|---|------|------|
| Silts, sand silts, slightly cohesive silt-sand mixtures | 4 | 1.5 |
| Clean, Fine to med, sands and slightly silty sands | 7 | 2 |
| Coarse sands and sands with little gravel | 10.0 | 3 |
| Sandy gravels and gravel | 12.0 | 4 |

Modulus of Elasticity, E (ksf)

Weighted average462by depth/thickness of layers

Poisson's Ratio, v

Weighted average0.3McCarthy (1998)

Influence Factor, I

0.64

$$\delta v = (\Delta q) \cdot B \cdot (1-v^2)/E \cdot I$$
NAVFAC (1989), section 7.1-211

$\Delta q = 258.1$ psf
 $\Delta q = 0.26$ ksf
 $B = 250$ ft

$I = 0.64$ From NAVFAC chart, assuming a circular (flexible) shape and rigidity, calculated on the edge

Solution: $\delta v = 0.1$ ft

Estimate E:

| Layer | Soil Description | z (ft) | Δz (ft) | E/qc | qc (ksf) | E (ksf) | Avg E (ksf) per layer |
|-------|------------------|---------|---------|------|----------|---------|-----------------------|
| 1 | Ex. Cover | 0.16404 | 0.16404 | 1.5 | 32.260 | 48.39 | 505.5 |
| 2 | Ex. Cover | 0.32808 | 0.16404 | 1.5 | 84.140 | 126.21 | |
| 3 | Ex. Cover | 0.49212 | 0.16404 | 1.5 | 138.060 | 207.09 | |
| 4 | Ex. Cover | 0.65616 | 0.16404 | 1.5 | 225.500 | 338.25 | |
| 5 | Ex. Cover | 0.8202 | 0.16404 | 1.5 | 312.820 | 469.23 | |
| 6 | Ex. Cover | 0.98424 | 0.16404 | 1.5 | 333.860 | 500.79 | |
| 7 | Ex. Cover | 1.14828 | 0.16404 | 1.5 | 381.540 | 572.31 | |
| 8 | Ex. Cover | 1.31232 | 0.16404 | 1.5 | 380.120 | 570.18 | |
| 9 | Ex. Cover | 1.47636 | 0.16404 | 1.5 | 424.100 | 636.15 | |
| 10 | Ex. Cover | 1.6404 | 0.16404 | 1.5 | 442.080 | 663.12 | |
| 11 | Ex. Cover | 1.80444 | 0.16404 | 1.5 | 449.340 | 674.01 | |
| 12 | Ex. Cover | 1.96848 | 0.16404 | 1.5 | 435.320 | 652.98 | |
| 13 | Ex. Cover | 2.13252 | 0.16404 | 1.5 | 402.300 | 603.45 | |
| 14 | Ex. Cover | 2.29656 | 0.16404 | 1.5 | 469.240 | 703.86 | |
| 15 | Ex. Cover | 2.4606 | 0.16404 | 1.5 | 543.940 | 815.91 | |
| 16 | Ex. Fill | 2.62464 | 0.16404 | 3.0 | 628.440 | 1885.32 | |
| 17 | Ex. Fill | 2.78868 | 0.16404 | 3.0 | 761.660 | 2284.98 | |
| 18 | Ex. Fill | 2.95272 | 0.16404 | 3.0 | 861.220 | 2583.66 | |
| 19 | Ex. Fill | 3.11676 | 0.16404 | 3.0 | 885.940 | 2657.82 | |

| | | | | | | |
|----|----------|----------|---------|-----|---------|---------|
| 20 | Ex. Fill | 3.2808 | 0.16404 | 3.0 | 892.700 | 2678.1 |
| 21 | Ex. Fill | 3.44484 | 0.16404 | 3.0 | 890.400 | 2671.2 |
| 22 | Ex. Fill | 3.60888 | 0.16404 | 3.0 | 836.740 | 2510.22 |
| 23 | Ex. Fill | 3.77292 | 0.16404 | 3.0 | 769.440 | 2308.32 |
| 24 | Ex. Fill | 3.93696 | 0.16404 | 3.0 | 767.520 | 2302.56 |
| 25 | Ex. Fill | 4.101 | 0.16404 | 3.0 | 764.600 | 2293.8 |
| 26 | Ex. Fill | 4.26504 | 0.16404 | 3.0 | 719.340 | 2158.02 |
| 27 | Ex. Fill | 4.42908 | 0.16404 | 3.0 | 674.720 | 2024.16 |
| 28 | Ex. Fill | 4.59312 | 0.16404 | 3.0 | 635.460 | 1906.38 |
| 29 | Ex. Fill | 4.75716 | 0.16404 | 3.0 | 568.660 | 1705.98 |
| 30 | Ex. Fill | 4.9212 | 0.16404 | 3.0 | 460.940 | 1382.82 |
| 31 | Ex. Fill | 5.08524 | 0.16404 | 3.0 | 359.220 | 1077.66 |
| 32 | Ex. Fill | 5.24928 | 0.16404 | 3.0 | 258.400 | 775.2 |
| 33 | Ex. Fill | 5.41332 | 0.16404 | 3.0 | 163.540 | 490.62 |
| 34 | Ex. Fill | 5.57736 | 0.16404 | 3.0 | 113.840 | 341.52 |
| 35 | Ex. Fill | 5.7414 | 0.16404 | 3.0 | 92.040 | 276.12 |
| 36 | Ex. Fill | 5.90544 | 0.16404 | 3.0 | 79.420 | 238.26 |
| 37 | Ex. Fill | 6.06948 | 0.16404 | 3.0 | 72.160 | 216.48 |
| 38 | Ex. Fill | 6.23352 | 0.16404 | 3.0 | 81.460 | 244.38 |
| 39 | Ex. Fill | 6.39756 | 0.16404 | 3.0 | 80.180 | 240.54 |
| 40 | Ex. Fill | 6.5616 | 0.16404 | 3.0 | 76.620 | 229.86 |
| 41 | Ex. Fill | 6.72564 | 0.16404 | 3.0 | 84.140 | 252.42 |
| 42 | Ex. Fill | 6.88968 | 0.16404 | 3.0 | 96.760 | 290.28 |
| 43 | Ex. Fill | 7.05372 | 0.16404 | 3.0 | 107.340 | 322.02 |
| 44 | Ex. Fill | 7.21776 | 0.16404 | 3.0 | 126.200 | 378.6 |
| 45 | Ex. Fill | 7.3818 | 0.16404 | 3.0 | 148.900 | 446.7 |
| 46 | Ex. Fill | 7.54584 | 0.16404 | 3.0 | 151.940 | 455.82 |
| 47 | Ex. Fill | 7.70988 | 0.16404 | 3.0 | 134.480 | 403.44 |
| 48 | Ex. Fill | 7.87392 | 0.16404 | 3.0 | 138.180 | 414.54 |
| 49 | Ex. Fill | 8.03796 | 0.16404 | 3.0 | 153.860 | 461.58 |
| 50 | Ex. Fill | 8.202 | 0.16404 | 3.0 | 153.740 | 461.22 |
| 51 | Ex. Fill | 8.36604 | 0.16404 | 3.0 | 151.180 | 453.54 |
| 52 | Ex. Fill | 8.53008 | 0.16404 | 3.0 | 183.440 | 550.32 |
| 53 | Ex. Fill | 8.69412 | 0.16404 | 3.0 | 186.240 | 558.72 |
| 54 | Ex. Fill | 8.85816 | 0.16404 | 3.0 | 178.980 | 536.94 |
| 55 | Ex. Fill | 9.0222 | 0.16404 | 3.0 | 156.800 | 470.4 |
| 56 | Ex. Fill | 9.18624 | 0.16404 | 3.0 | 157.940 | 473.82 |
| 57 | Ex. Fill | 9.35028 | 0.16404 | 3.0 | 144.180 | 432.54 |
| 58 | Ex. Fill | 9.51432 | 0.16404 | 3.0 | 136.020 | 408.06 |
| 59 | Ex. Fill | 9.67836 | 0.16404 | 3.0 | 141.240 | 423.72 |
| 60 | Ex. Fill | 9.8424 | 0.16404 | 3.0 | 142.900 | 428.7 |
| 61 | Ex. Fill | 10.00644 | 0.16404 | 3.0 | 124.800 | 374.4 |
| 62 | Ex. Fill | 10.17048 | 0.16404 | 3.0 | 111.280 | 333.84 |
| 63 | Ex. Fill | 10.33452 | 0.16404 | 3.0 | 98.420 | 295.26 |
| 64 | Ex. Fill | 10.49856 | 0.16404 | 3.0 | 90.000 | 270 |
| 65 | Ex. Fill | 10.6626 | 0.16404 | 3.0 | 84.780 | 254.34 |
| 66 | Ex. Fill | 10.82664 | 0.16404 | 3.0 | 75.200 | 225.6 |
| 67 | Ex. Fill | 10.99068 | 0.16404 | 3.0 | 70.500 | 211.5 |
| 68 | Ex. Fill | 11.15472 | 0.16404 | 3.0 | 70.740 | 212.22 |
| 69 | Ex. Fill | 11.31876 | 0.16404 | 3.0 | 73.420 | 220.26 |
| 70 | Ex. Fill | 11.4828 | 0.16404 | 3.0 | 74.060 | 222.18 |
| 71 | Ex. Fill | 11.64684 | 0.16404 | 3.0 | 68.080 | 204.24 |
| 72 | Ex. Fill | 11.81088 | 0.16404 | 3.0 | 67.300 | 201.9 |
| 73 | Ex. Fill | 11.97492 | 0.16404 | 3.0 | 148.760 | 446.28 |
| 74 | Ex. Fill | 12.13896 | 0.16404 | 3.0 | 228.560 | 685.68 |
| 75 | Ex. Fill | 12.303 | 0.16404 | 3.0 | 304.920 | 914.76 |
| 76 | Ex. Fill | 12.46704 | 0.16404 | 3.0 | 348.760 | 1046.28 |
| 77 | Ex. Fill | 12.63108 | 0.16404 | 3.0 | 377.580 | 1132.74 |
| 78 | Ex. Fill | 12.79512 | 0.16404 | 3.0 | 381.540 | 1144.62 |

661.3

| | | | | | | |
|-----|-----------------|----------|---------|-----|---------|---------|
| 79 | Ex. Fill | 12.95916 | 0.16404 | 3.0 | 371.580 | 1114.74 |
| 80 | Ex. Fill | 13.1232 | 0.16404 | 3.0 | 347.240 | 1041.72 |
| 81 | Ex. Fill | 13.28724 | 0.16404 | 3.0 | 283.760 | 851.28 |
| 82 | Ex. Fill | 13.45128 | 0.16404 | 3.0 | 208.800 | 626.4 |
| 83 | Ex. Fill | 13.61532 | 0.16404 | 3.0 | 148.260 | 444.78 |
| 84 | Ex. Fill | 13.77936 | 0.16404 | 3.0 | 113.700 | 341.1 |
| 85 | Ex. Fill | 13.9434 | 0.16404 | 3.0 | 96.380 | 289.14 |
| 86 | Ex. Fill | 14.10744 | 0.16404 | 3.0 | 80.700 | 242.1 |
| 87 | Ex. Fill | 14.27148 | 0.16404 | 3.0 | 72.020 | 216.06 |
| 88 | Ex. Fill | 14.43552 | 0.16404 | 3.0 | 58.380 | 175.14 |
| 89 | Ex. Fill | 14.59956 | 0.16404 | 3.0 | 55.700 | 167.1 |
| 90 | Ex. Fill | 14.7636 | 0.16404 | 3.0 | 54.180 | 162.54 |
| 91 | Ex. Fill | 14.92764 | 0.16404 | 3.0 | 50.980 | 152.94 |
| 92 | Ex. Fill | 15.09168 | 0.16404 | 3.0 | 49.080 | 147.24 |
| 93 | Ex. Fill | 15.25572 | 0.16404 | 3.0 | 46.920 | 140.76 |
| 94 | Ex. Fill | 15.41976 | 0.16404 | 3.0 | 42.840 | 128.52 |
| 95 | Ex. Fill | 15.5838 | 0.16404 | 3.0 | 45.260 | 135.78 |
| 96 | Ex. Fill | 15.74784 | 0.16404 | 3.0 | 42.840 | 128.52 |
| 97 | Ex. Fill | 15.91188 | 0.16404 | 3.0 | 41.560 | 124.68 |
| 98 | Ex. Fill | 16.07592 | 0.16404 | 3.0 | 36.720 | 110.16 |
| 99 | Ex. Fill | 16.23996 | 0.16404 | 3.0 | 38.360 | 115.08 |
| 100 | Ex. Fill | 16.404 | 0.16404 | 3.0 | 44.360 | 133.08 |
| 101 | Ex. Fill | 16.56804 | 0.16404 | 3.0 | 45.260 | 135.78 |
| 102 | Ex. Fill | 16.73208 | 0.16404 | 3.0 | 42.060 | 126.18 |
| 103 | Ex. Fill | 16.89612 | 0.16404 | 3.0 | 41.820 | 125.46 |
| 104 | Ex. Fill | 17.06016 | 0.16404 | 3.0 | 56.600 | 169.8 |
| 105 | Ex. Fill | 17.2242 | 0.16404 | 3.0 | 55.460 | 166.38 |
| 106 | Ex. Fill | 17.38824 | 0.16404 | 3.0 | 53.160 | 159.48 |
| 107 | Ex. Fill | 17.55228 | 0.16404 | 3.0 | 62.840 | 188.52 |
| 108 | Ex. Fill | 17.71632 | 0.16404 | 3.0 | 65.900 | 197.7 |
| 109 | Ex. Fill | 17.88036 | 0.16404 | 3.0 | 67.820 | 203.46 |
| 110 | Ex. Fill | 18.0444 | 0.16404 | 3.0 | 68.080 | 204.24 |
| 111 | Ex. Fill | 18.20844 | 0.16404 | 3.0 | 66.160 | 198.48 |
| 112 | Ex. Fill | 18.37248 | 0.16404 | 3.0 | 75.080 | 225.24 |
| 113 | Ex. Fill | 18.53652 | 0.16404 | 3.0 | 404.860 | 1214.58 |
| 114 | Coarse Tailings | 18.70056 | 0.16404 | 2.0 | 756.680 | 1513.36 |
| 115 | Coarse Tailings | 18.8646 | 0.16404 | 2.0 | 883.660 | 1767.32 |
| 116 | Coarse Tailings | 19.02864 | 0.16404 | 2.0 | 910.940 | 1821.88 |
| 117 | Coarse Tailings | 19.19268 | 0.16404 | 2.0 | 888.360 | 1776.72 |
| 118 | Coarse Tailings | 19.35672 | 0.16404 | 2.0 | 894.360 | 1788.72 |
| 119 | Coarse Tailings | 19.52076 | 0.16404 | 2.0 | 906.480 | 1812.96 |
| 120 | Coarse Tailings | 19.6848 | 0.16404 | 2.0 | 872.560 | 1745.12 |
| 121 | Coarse Tailings | 19.84884 | 0.16404 | 2.0 | 844.400 | 1688.8 |
| 122 | Coarse Tailings | 20.01288 | 0.16404 | 2.0 | 796.840 | 1593.68 |
| 123 | Coarse Tailings | 20.17692 | 0.16404 | 2.0 | 743.300 | 1486.6 |
| 124 | Coarse Tailings | 20.34096 | 0.16404 | 2.0 | 694.860 | 1389.72 |
| 125 | Coarse Tailings | 20.505 | 0.16404 | 2.0 | 654.460 | 1308.92 |
| 126 | Coarse Tailings | 20.66904 | 0.16404 | 2.0 | 614.800 | 1229.6 |
| 127 | Coarse Tailings | 20.83308 | 0.16404 | 2.0 | 553.120 | 1106.24 |
| 128 | Coarse Tailings | 20.99712 | 0.16404 | 2.0 | 501.620 | 1003.24 |
| 129 | Coarse Tailings | 21.16116 | 0.16404 | 2.0 | 441.580 | 883.16 |
| 130 | Coarse Tailings | 21.3252 | 0.16404 | 2.0 | 388.280 | 776.56 |
| 131 | Coarse Tailings | 21.48924 | 0.16404 | 2.0 | 331.560 | 663.12 |
| 132 | Coarse Tailings | 21.65328 | 0.16404 | 2.0 | 288.100 | 576.2 |
| 133 | Coarse Tailings | 21.81732 | 0.16404 | 2.0 | 252.140 | 504.28 |
| 134 | Coarse Tailings | 21.98136 | 0.16404 | 2.0 | 226.520 | 453.04 |
| 135 | Coarse Tailings | 22.1454 | 0.16404 | 2.0 | 204.600 | 409.2 |
| 136 | Coarse Tailings | 22.30944 | 0.16404 | 2.0 | 190.320 | 380.64 |
| 137 | Coarse Tailings | 22.47348 | 0.16404 | 2.0 | 157.180 | 314.36 |

| | | | | | | |
|-----|-----------------|----------|---------|-----|---------|--------|
| 138 | Coarse Tailings | 22.63752 | 0.16404 | 2.0 | 139.960 | 279.92 |
| 139 | Coarse Tailings | 22.80156 | 0.16404 | 2.0 | 130.020 | 260.04 |
| 140 | Coarse Tailings | 22.9656 | 0.16404 | 2.0 | 112.680 | 225.36 |
| 141 | Coarse Tailings | 23.12964 | 0.16404 | 2.0 | 102.620 | 205.24 |
| 142 | Coarse Tailings | 23.29368 | 0.16404 | 2.0 | 94.960 | 189.92 |
| 143 | Coarse Tailings | 23.45772 | 0.16404 | 2.0 | 88.980 | 177.96 |
| 144 | Coarse Tailings | 23.62176 | 0.16404 | 2.0 | 86.180 | 172.36 |
| 145 | Coarse Tailings | 23.7858 | 0.16404 | 2.0 | 82.340 | 164.68 |
| 146 | Coarse Tailings | 23.94984 | 0.16404 | 2.0 | 84.520 | 169.04 |
| 147 | Coarse Tailings | 24.11388 | 0.16404 | 2.0 | 78.660 | 157.32 |
| 148 | Coarse Tailings | 24.27792 | 0.16404 | 2.0 | 87.820 | 175.64 |
| 149 | Coarse Tailings | 24.44196 | 0.16404 | 2.0 | 90.380 | 180.76 |
| 150 | Coarse Tailings | 24.606 | 0.16404 | 2.0 | 91.900 | 183.8 |
| 151 | Coarse Tailings | 24.77004 | 0.16404 | 2.0 | 100.060 | 200.12 |
| 152 | Coarse Tailings | 24.93408 | 0.16404 | 2.0 | 109.120 | 218.24 |
| 153 | Coarse Tailings | 25.09812 | 0.16404 | 2.0 | 122.640 | 245.28 |
| 154 | Coarse Tailings | 25.26216 | 0.16404 | 2.0 | 148.640 | 297.28 |
| 155 | Coarse Tailings | 25.4262 | 0.16404 | 2.0 | 172.980 | 345.96 |
| 156 | Coarse Tailings | 25.59024 | 0.16404 | 2.0 | 206.900 | 413.8 |
| 157 | Coarse Tailings | 25.75428 | 0.16404 | 2.0 | 214.160 | 428.32 |
| 158 | Coarse Tailings | 25.91832 | 0.16404 | 2.0 | 209.320 | 418.64 |
| 159 | Coarse Tailings | 26.08236 | 0.16404 | 2.0 | 202.420 | 404.84 |
| 160 | Coarse Tailings | 26.2464 | 0.16404 | 2.0 | 196.820 | 393.64 |
| 161 | Coarse Tailings | 26.41044 | 0.16404 | 2.0 | 191.980 | 383.96 |
| 162 | Coarse Tailings | 26.57448 | 0.16404 | 2.0 | 181.660 | 363.32 |
| 163 | Coarse Tailings | 26.73852 | 0.16404 | 2.0 | 167.380 | 334.76 |
| 164 | Coarse Tailings | 26.90256 | 0.16404 | 2.0 | 142.780 | 285.56 |
| 165 | Coarse Tailings | 27.0666 | 0.16404 | 2.0 | 126.580 | 253.16 |
| 166 | Coarse Tailings | 27.23064 | 0.16404 | 2.0 | 133.980 | 267.96 |
| 167 | Coarse Tailings | 27.39468 | 0.16404 | 2.0 | 98.020 | 196.04 |
| 168 | Coarse Tailings | 27.55872 | 0.16404 | 2.0 | 73.940 | 147.88 |
| 169 | Coarse Tailings | 27.72276 | 0.16404 | 2.0 | 69.220 | 138.44 |
| 170 | Coarse Tailings | 27.8868 | 0.16404 | 2.0 | 129.520 | 259.04 |
| 171 | Coarse Tailings | 28.05084 | 0.16404 | 2.0 | 126.320 | 252.64 |
| 172 | Coarse Tailings | 28.21488 | 0.16404 | 2.0 | 99.680 | 199.36 |
| 173 | Coarse Tailings | 28.37892 | 0.16404 | 2.0 | 105.300 | 210.6 |
| 174 | Coarse Tailings | 28.54296 | 0.16404 | 2.0 | 136.520 | 273.04 |
| 175 | Coarse Tailings | 28.707 | 0.16404 | 2.0 | 137.540 | 275.08 |
| 176 | Coarse Tailings | 28.87104 | 0.16404 | 2.0 | 131.940 | 263.88 |
| 177 | Coarse Tailings | 29.03508 | 0.16404 | 2.0 | 130.780 | 261.56 |
| 178 | Coarse Tailings | 29.19912 | 0.16404 | 2.0 | 116.640 | 233.28 |
| 179 | Coarse Tailings | 29.36316 | 0.16404 | 2.0 | 100.580 | 201.16 |
| 180 | Coarse Tailings | 29.5272 | 0.16404 | 2.0 | 125.300 | 250.6 |
| 181 | Coarse Tailings | 29.69124 | 0.16404 | 2.0 | 120.080 | 240.16 |

576.4

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-15 Settlement

IMMEDIATE SETTLEMENT

CPT Profile Information

| | Thickness (ft) | E (ksf) | v | γ (psf) | |
|---------------------------------|----------------|-----------------|-----------------|---------|-------------------------|
| Erosion Protection (repository) | 1.1 | | | 122.9 | |
| Cover Fill (repository) | 3.4 | | | 114.7 | |
| Mine Spoils (repository) | 6.1 | | | 116.4 | |
| Radon Barrier (existing) | 2.1 | 450 | 0.3 | 122.3 | Hard, moist, sandy clay |
| Existing Fill (existing) | 0.0 | | | 113.8 | |
| Coarse Tailings (existing) | 27.4 | 150 | 0.3 | 108.1 | Silty sand tailings |
| | | McCarthy (1998) | McCarthy (1998) | | |

Total Fill Height10.6above radon barrier

Soil Properties (McCarthy, 1998):

| Soil Type | E/N | E/qc |
|---|------|------|
| Silts, sand silts, slightly cohesive silt-sand mixtures | 4 | 1.5 |
| Clean, Fine to med, sands and slightly silty sands | 7 | 2 |
| Coarse sands and sands with little gravel | 10.0 | 3 |
| Sandy gravels and gravel | 12.0 | 4 |

Modulus of Elasticity, E (ksf)

Weighted average172by depth/thickness of layers

Poisson's Ratio, v

Weighted average0.3McCarthy (1998)

Influence Factor, I

0.64

$$\delta v = (\Delta q) * B * (1-v^2)/E * I$$
NAVFAC (1989), section 7.1-211

$$\Delta q = 1235 \text{ psf}$$
$$\Delta q = 1.24 \text{ ksf}$$
$$B = 250 \text{ ft}$$

$$I = 0.64$$
From NAVFAC chart, assuming a circular (flexible) shape and rigidity, calculated on the edge

Solution:
$$\delta v = 1.0 \text{ ft}$$

Estimate E:

| Layer No. | Soil Description | z (ft) | Δz (ft) | E/qc | qc (ksf) | E (ksf) | Avg E (ksf) per layer |
|-----------|------------------|--------|---------|------|----------|---------|-----------------------|
| 1 | Ex. Cover | 0.16 | 0.16 | 1.5 | 77.9 | 116.8 | 449.1 |
| 2 | Ex. Cover | 0.33 | 0.16 | 1.5 | 136.9 | 205.4 | |
| 3 | Ex. Cover | 0.49 | 0.16 | 1.5 | 306.8 | 460.3 | |
| 4 | Ex. Cover | 0.66 | 0.16 | 1.5 | 397.6 | 596.4 | |
| 5 | Ex. Cover | 0.82 | 0.16 | 1.5 | 418.2 | 627.4 | |
| 6 | Ex. Cover | 0.98 | 0.16 | 1.5 | 403.7 | 605.6 | |
| 7 | Ex. Cover | 1.15 | 0.16 | 1.5 | 388.0 | 582.1 | |
| 8 | Ex. Cover | 1.31 | 0.16 | 1.5 | 352.7 | 529.1 | |
| 9 | Ex. Cover | 1.48 | 0.16 | 1.5 | 344.7 | 517.1 | |
| 10 | Ex. Cover | 1.64 | 0.16 | 1.5 | 351.3 | 527.0 | |
| 11 | Ex. Cover | 1.80 | 0.16 | 1.5 | 323.7 | 485.5 | |
| 12 | Ex. Cover | 1.97 | 0.16 | 1.5 | 268.1 | 402.1 | |
| 13 | Ex. Cover | 2.13 | 0.16 | 1.5 | 241.1 | 361.6 | |
| 14 | Ex. Cover | 2.30 | 0.16 | 1.5 | 229.2 | 343.8 | |
| 15 | Ex. Cover | 2.46 | 0.16 | 1.5 | 251.0 | 376.5 | |
| 16 | Coarse Tailings | 2.62 | 0.16 | 2.0 | 220.2 | 440.3 | |

| | | | | | | |
|----|-----------------|-------|------|-----|-------|-------|
| 17 | Coarse Tailings | 2.79 | 0.16 | 2.0 | 256.4 | 512.8 |
| 18 | Coarse Tailings | 2.95 | 0.16 | 2.0 | 305.1 | 610.1 |
| 19 | Coarse Tailings | 3.12 | 0.16 | 2.0 | 377.3 | 754.5 |
| 20 | Coarse Tailings | 3.28 | 0.16 | 2.0 | 421.6 | 843.1 |
| 21 | Coarse Tailings | 3.44 | 0.16 | 2.0 | 430.2 | 860.3 |
| 22 | Coarse Tailings | 3.61 | 0.16 | 2.0 | 431.4 | 862.9 |
| 23 | Coarse Tailings | 3.77 | 0.16 | 2.0 | 421.6 | 843.2 |
| 24 | Coarse Tailings | 3.94 | 0.16 | 2.0 | 400.9 | 801.8 |
| 25 | Coarse Tailings | 4.10 | 0.16 | 2.0 | 372.4 | 744.8 |
| 26 | Coarse Tailings | 4.27 | 0.16 | 2.0 | 342.8 | 685.6 |
| 27 | Coarse Tailings | 4.43 | 0.16 | 2.0 | 307.9 | 615.7 |
| 28 | Coarse Tailings | 4.59 | 0.16 | 2.0 | 291.3 | 582.7 |
| 29 | Coarse Tailings | 4.76 | 0.16 | 2.0 | 272.4 | 544.8 |
| 30 | Coarse Tailings | 4.92 | 0.16 | 2.0 | 249.3 | 498.6 |
| 31 | Coarse Tailings | 5.09 | 0.16 | 2.0 | 231.8 | 463.6 |
| 32 | Coarse Tailings | 5.25 | 0.16 | 2.0 | 201.6 | 403.2 |
| 33 | Coarse Tailings | 5.41 | 0.16 | 2.0 | 185.0 | 370.1 |
| 34 | Coarse Tailings | 5.58 | 0.16 | 2.0 | 170.7 | 341.4 |
| 35 | Coarse Tailings | 5.74 | 0.16 | 2.0 | 159.5 | 318.9 |
| 36 | Coarse Tailings | 5.91 | 0.16 | 2.0 | 146.7 | 293.4 |
| 37 | Coarse Tailings | 6.07 | 0.16 | 2.0 | 132.1 | 264.3 |
| 38 | Coarse Tailings | 6.23 | 0.16 | 2.0 | 121.9 | 243.9 |
| 39 | Coarse Tailings | 6.40 | 0.16 | 2.0 | 113.3 | 226.5 |
| 40 | Coarse Tailings | 6.56 | 0.16 | 2.0 | 104.0 | 207.9 |
| 41 | Coarse Tailings | 6.73 | 0.16 | 2.0 | 96.0 | 192.0 |
| 42 | Coarse Tailings | 6.89 | 0.16 | 2.0 | 85.2 | 170.3 |
| 43 | Coarse Tailings | 7.05 | 0.16 | 2.0 | 76.4 | 152.8 |
| 44 | Coarse Tailings | 7.22 | 0.16 | 2.0 | 71.0 | 142.0 |
| 45 | Coarse Tailings | 7.38 | 0.16 | 2.0 | 66.5 | 133.1 |
| 46 | Coarse Tailings | 7.55 | 0.16 | 2.0 | 59.5 | 118.9 |
| 47 | Coarse Tailings | 7.71 | 0.16 | 2.0 | 54.6 | 109.1 |
| 48 | Coarse Tailings | 7.87 | 0.16 | 2.0 | 49.6 | 99.2 |
| 49 | Coarse Tailings | 8.04 | 0.16 | 2.0 | 48.4 | 96.9 |
| 50 | Coarse Tailings | 8.20 | 0.16 | 2.0 | 46.7 | 93.3 |
| 51 | Coarse Tailings | 8.37 | 0.16 | 2.0 | 45.0 | 90.0 |
| 52 | Coarse Tailings | 8.53 | 0.16 | 2.0 | 43.4 | 86.8 |
| 53 | Coarse Tailings | 8.69 | 0.16 | 2.0 | 40.9 | 81.7 |
| 54 | Coarse Tailings | 8.86 | 0.16 | 2.0 | 40.2 | 80.4 |
| 55 | Coarse Tailings | 9.02 | 0.16 | 2.0 | 40.4 | 80.7 |
| 56 | Coarse Tailings | 9.19 | 0.16 | 2.0 | 39.9 | 79.8 |
| 57 | Coarse Tailings | 9.35 | 0.16 | 2.0 | 39.5 | 78.9 |
| 58 | Coarse Tailings | 9.51 | 0.16 | 2.0 | 39.1 | 78.1 |
| 59 | Coarse Tailings | 9.68 | 0.16 | 2.0 | 37.4 | 74.7 |
| 60 | Coarse Tailings | 9.84 | 0.16 | 2.0 | 37.0 | 74.1 |
| 61 | Coarse Tailings | 10.01 | 0.16 | 2.0 | 39.5 | 78.9 |
| 62 | Coarse Tailings | 10.17 | 0.16 | 2.0 | 37.7 | 75.5 |
| 63 | Coarse Tailings | 10.33 | 0.16 | 2.0 | 35.4 | 70.7 |
| 64 | Coarse Tailings | 10.50 | 0.16 | 2.0 | 33.7 | 67.3 |
| 65 | Coarse Tailings | 10.66 | 0.16 | 2.0 | 33.1 | 66.2 |
| 66 | Coarse Tailings | 10.83 | 0.16 | 2.0 | 32.8 | 65.5 |
| 67 | Coarse Tailings | 10.99 | 0.16 | 2.0 | 32.4 | 64.9 |
| 68 | Coarse Tailings | 11.15 | 0.16 | 2.0 | 31.7 | 63.5 |
| 69 | Coarse Tailings | 11.32 | 0.16 | 2.0 | 31.0 | 62.0 |
| 70 | Coarse Tailings | 11.48 | 0.16 | 2.0 | 30.8 | 61.6 |
| 71 | Coarse Tailings | 11.65 | 0.16 | 2.0 | 30.2 | 60.4 |
| 72 | Coarse Tailings | 11.81 | 0.16 | 2.0 | 30.5 | 60.9 |
| 73 | Coarse Tailings | 11.97 | 0.16 | 2.0 | 29.2 | 58.4 |
| 74 | Coarse Tailings | 12.14 | 0.16 | 2.0 | 27.0 | 53.9 |
| 75 | Coarse Tailings | 12.30 | 0.16 | 2.0 | 23.4 | 46.8 |

| | | | | | | |
|-----|-----------------|-------|------|-----|------|------|
| 76 | Coarse Tailings | 12.47 | 0.16 | 2.0 | 16.3 | 32.5 |
| 77 | Coarse Tailings | 12.63 | 0.16 | 2.0 | 12.4 | 24.9 |
| 78 | Coarse Tailings | 12.80 | 0.16 | 2.0 | 15.2 | 30.5 |
| 79 | Coarse Tailings | 12.96 | 0.16 | 2.0 | 20.6 | 41.2 |
| 80 | Coarse Tailings | 13.12 | 0.16 | 2.0 | 24.8 | 49.6 |
| 81 | Coarse Tailings | 13.29 | 0.16 | 2.0 | 26.3 | 52.5 |
| 82 | Coarse Tailings | 13.45 | 0.16 | 2.0 | 22.3 | 44.6 |
| 83 | Coarse Tailings | 13.62 | 0.16 | 2.0 | 19.6 | 39.3 |
| 84 | Coarse Tailings | 13.78 | 0.16 | 2.0 | 25.8 | 51.6 |
| 85 | Coarse Tailings | 13.94 | 0.16 | 2.0 | 27.2 | 54.4 |
| 86 | Coarse Tailings | 14.11 | 0.16 | 2.0 | 28.2 | 56.3 |
| 87 | Coarse Tailings | 14.27 | 0.16 | 2.0 | 28.9 | 57.9 |
| 88 | Coarse Tailings | 14.44 | 0.16 | 2.0 | 30.1 | 60.2 |
| 89 | Coarse Tailings | 14.60 | 0.16 | 2.0 | 30.0 | 59.9 |
| 90 | Coarse Tailings | 14.76 | 0.16 | 2.0 | 29.6 | 59.3 |
| 91 | Coarse Tailings | 14.93 | 0.16 | 2.0 | 27.3 | 54.7 |
| 92 | Coarse Tailings | 15.09 | 0.16 | 2.0 | 22.9 | 45.8 |
| 93 | Coarse Tailings | 15.26 | 0.16 | 2.0 | 23.2 | 46.4 |
| 94 | Coarse Tailings | 15.42 | 0.16 | 2.0 | 22.4 | 44.9 |
| 95 | Coarse Tailings | 15.58 | 0.16 | 2.0 | 23.3 | 46.7 |
| 96 | Coarse Tailings | 15.75 | 0.16 | 2.0 | 15.4 | 30.7 |
| 97 | Coarse Tailings | 15.91 | 0.16 | 2.0 | 13.8 | 27.5 |
| 98 | Coarse Tailings | 16.08 | 0.16 | 2.0 | 24.9 | 49.8 |
| 99 | Coarse Tailings | 16.24 | 0.16 | 2.0 | 27.6 | 55.2 |
| 100 | Coarse Tailings | 16.40 | 0.16 | 2.0 | 25.9 | 51.8 |
| 101 | Coarse Tailings | 16.57 | 0.16 | 2.0 | 25.8 | 51.5 |
| 102 | Coarse Tailings | 16.73 | 0.16 | 2.0 | 25.9 | 51.9 |
| 103 | Coarse Tailings | 16.90 | 0.16 | 2.0 | 26.6 | 53.2 |
| 104 | Coarse Tailings | 17.06 | 0.16 | 2.0 | 26.6 | 53.2 |
| 105 | Coarse Tailings | 17.22 | 0.16 | 2.0 | 30.6 | 61.2 |
| 106 | Coarse Tailings | 17.39 | 0.16 | 2.0 | 34.6 | 69.2 |
| 107 | Coarse Tailings | 17.55 | 0.16 | 2.0 | 29.3 | 58.5 |
| 108 | Coarse Tailings | 17.72 | 0.16 | 2.0 | 28.8 | 57.6 |
| 109 | Coarse Tailings | 17.88 | 0.16 | 2.0 | 23.4 | 46.8 |
| 110 | Coarse Tailings | 18.04 | 0.16 | 2.0 | 30.2 | 60.3 |
| 111 | Coarse Tailings | 18.21 | 0.16 | 2.0 | 32.4 | 64.9 |
| 112 | Coarse Tailings | 18.37 | 0.16 | 2.0 | 38.3 | 76.6 |
| 113 | Coarse Tailings | 18.54 | 0.16 | 2.0 | 36.6 | 73.2 |
| 114 | Coarse Tailings | 18.70 | 0.16 | 2.0 | 28.6 | 57.1 |
| 115 | Coarse Tailings | 18.86 | 0.16 | 2.0 | 18.4 | 36.8 |
| 116 | Coarse Tailings | 19.03 | 0.16 | 2.0 | 14.9 | 29.7 |
| 117 | Coarse Tailings | 19.19 | 0.16 | 2.0 | 22.6 | 45.3 |
| 118 | Coarse Tailings | 19.36 | 0.16 | 2.0 | 33.4 | 66.8 |
| 119 | Coarse Tailings | 19.52 | 0.16 | 2.0 | 33.8 | 67.6 |
| 120 | Coarse Tailings | 19.68 | 0.16 | 2.0 | 32.5 | 65.0 |
| 121 | Coarse Tailings | 19.85 | 0.16 | 2.0 | 30.9 | 61.7 |
| 122 | Coarse Tailings | 20.01 | 0.16 | 2.0 | 28.7 | 57.4 |
| 123 | Coarse Tailings | 20.18 | 0.16 | 2.0 | 22.4 | 44.7 |
| 124 | Coarse Tailings | 20.34 | 0.16 | 2.0 | 22.1 | 44.1 |
| 125 | Coarse Tailings | 20.51 | 0.16 | 2.0 | 20.1 | 40.2 |
| 126 | Coarse Tailings | 20.67 | 0.16 | 2.0 | 22.8 | 45.5 |
| 127 | Coarse Tailings | 20.83 | 0.16 | 2.0 | 28.8 | 57.6 |
| 128 | Coarse Tailings | 21.00 | 0.16 | 2.0 | 27.9 | 55.7 |
| 129 | Coarse Tailings | 21.16 | 0.16 | 2.0 | 29.6 | 59.1 |
| 130 | Coarse Tailings | 21.33 | 0.16 | 2.0 | 31.3 | 62.6 |
| 131 | Coarse Tailings | 21.49 | 0.16 | 2.0 | 32.4 | 64.8 |
| 132 | Coarse Tailings | 21.65 | 0.16 | 2.0 | 32.8 | 65.5 |
| 133 | Coarse Tailings | 21.82 | 0.16 | 2.0 | 34.4 | 68.8 |
| 134 | Coarse Tailings | 21.98 | 0.16 | 2.0 | 34.7 | 69.5 |

145.0

| | | | | | | |
|-----|-----------------|-------|------|-----|------|------|
| 135 | Coarse Tailings | 22.15 | 0.16 | 2.0 | 35.6 | 71.3 |
| 136 | Coarse Tailings | 22.31 | 0.16 | 2.0 | 37.2 | 74.4 |
| 137 | Coarse Tailings | 22.47 | 0.16 | 2.0 | 40.2 | 80.4 |
| 138 | Coarse Tailings | 22.64 | 0.16 | 2.0 | 6.0 | 12.0 |
| 139 | Coarse Tailings | 22.80 | 0.16 | 2.0 | 46.2 | 92.3 |
| 140 | Coarse Tailings | 22.97 | 0.16 | 2.0 | 40.5 | 80.9 |
| 141 | Coarse Tailings | 23.13 | 0.16 | 2.0 | 26.1 | 52.3 |
| 142 | Coarse Tailings | 23.29 | 0.16 | 2.0 | 19.6 | 39.1 |
| 143 | Coarse Tailings | 23.46 | 0.16 | 2.0 | 22.0 | 44.0 |
| 144 | Coarse Tailings | 23.62 | 0.16 | 2.0 | 21.9 | 43.7 |
| 145 | Coarse Tailings | 23.79 | 0.16 | 2.0 | 17.2 | 34.3 |
| 146 | Coarse Tailings | 23.95 | 0.16 | 2.0 | 20.5 | 41.0 |
| 147 | Coarse Tailings | 24.11 | 0.16 | 2.0 | 28.2 | 56.5 |
| 148 | Coarse Tailings | 24.28 | 0.16 | 2.0 | 32.1 | 64.2 |
| 149 | Coarse Tailings | 24.44 | 0.16 | 2.0 | 34.6 | 69.2 |
| 150 | Coarse Tailings | 24.61 | 0.16 | 2.0 | 31.4 | 62.7 |
| 151 | Coarse Tailings | 24.77 | 0.16 | 2.0 | 19.6 | 39.3 |
| 152 | Coarse Tailings | 24.93 | 0.16 | 2.0 | 28.9 | 57.7 |
| 153 | Coarse Tailings | 25.10 | 0.16 | 2.0 | 21.7 | 43.5 |
| 154 | Coarse Tailings | 25.26 | 0.16 | 2.0 | 29.9 | 59.8 |
| 155 | Coarse Tailings | 25.43 | 0.16 | 2.0 | 41.6 | 83.1 |
| 156 | Coarse Tailings | 25.59 | 0.16 | 2.0 | 37.9 | 75.8 |
| 157 | Coarse Tailings | 25.75 | 0.16 | 2.0 | 35.0 | 70.0 |
| 158 | Coarse Tailings | 25.92 | 0.16 | 2.0 | 28.9 | 57.7 |
| 159 | Coarse Tailings | 26.08 | 0.16 | 2.0 | 17.6 | 35.2 |
| 160 | Coarse Tailings | 26.25 | 0.16 | 2.0 | 32.7 | 65.4 |

NECR TAILINGS IMPOUNDMENT SETTLEMENT ANALYSIS - CPT-26 Settlement

IMMEDIATE SETTLEMENT

CPT Profile Information

| | Thickness (ft) | E (ksf) | v | γ (psf) |
|---------------------------------|----------------|-----------------|-----------------|--|
| Erosion Protection (repository) | 1.1 | | | 122.9 |
| Cover Fill (repository) | 3.4 | | | 114.7 |
| Mine Spoils (repository) | 8.5 | | | 116.4 |
| Radon Barrier (existing) | 2.5 | 450 | 0.3 | 122.3 Hard, moist, sandy clay |
| Existing Fill (existing) | 9.5 | 680 | 0.35 | 113.8 Silty sand, sandy clay w/ gravel |
| Coarse Tailings (existing) | 13.0 | 150 | 0.3 | 108.1 Silty sand tailings |
| | | McCarthy (1998) | McCarthy (1998) | |

Total Fill Height13.0 above radon barrier

Soil Properties (McCarthy, 1998):

| Soil Type | E/N | E/qc |
|---|------|------|
| Silts, sand silts, slightly cohesive silt-sand mixtures | 4 | 1.5 |
| Clean, Fine to med, sands and slightly silty sands | 7 | 2 |
| Coarse sands and sands with little gravel | 10.0 | 3 |
| Sandy gravels and gravel | 12.0 | 4 |

Modulus of Elasticity, E (ksf)

Weighted average381by depth/thickness of layers

Poisson's Ratio, v

Weighted average0.3McCarthy (1998)

Influence Factor, I

0.64

δv = (Δq) * B * (1-v^2)/Eu * I

NAVFAC (1989), section 7.1-211

Δq =1515psf
Δq =1.51ksf
B =250ft

I =0.64

From NAVFAC chart, assuming a circular (flexible) shape and rigidity, calculated on the edge

Solution:

δv =0.6ft

Estimate E:

| Layer | Soil Description | z (ft) | Δz (ft) | E/qc | qc (ksf) | E (ksf) | Avg E (ksf) per layer |
|-------|------------------|---------|---------|------|----------|---------|-----------------------|
| 1 | Ex. Cover | 0.16404 | 0.16404 | 1.5 | 40.160 | 60.24 | 363.9 |
| 2 | Ex. Cover | 0.32808 | 0.16404 | 1.5 | 56.980 | 85.47 | |
| 3 | Ex. Cover | 0.49212 | 0.16404 | 1.5 | 127.480 | 191.22 | |
| 4 | Ex. Cover | 0.65616 | 0.16404 | 1.5 | 129.520 | 194.28 | |
| 5 | Ex. Cover | 0.8202 | 0.16404 | 1.5 | 200.640 | 300.96 | |
| 6 | Ex. Cover | 0.98424 | 0.16404 | 1.5 | 414.920 | 622.38 | |
| 7 | Ex. Cover | 1.14828 | 0.16404 | 1.5 | 398.480 | 597.72 | |
| 8 | Ex. Cover | 1.31232 | 0.16404 | 1.5 | 333.220 | 499.83 | |
| 9 | Ex. Cover | 1.47636 | 0.16404 | 1.5 | 273.820 | 410.73 | |
| 10 | Ex. Cover | 1.6404 | 0.16404 | 1.5 | 251.120 | 376.68 | |
| 11 | Ex. Cover | 1.80444 | 0.16404 | 1.5 | 270.620 | 405.93 | |
| 12 | Ex. Cover | 1.96848 | 0.16404 | 1.5 | 305.680 | 458.52 | |
| 13 | Ex. Cover | 2.13252 | 0.16404 | 1.5 | 318.040 | 477.06 | |
| 14 | Ex. Cover | 2.29656 | 0.16404 | 1.5 | 272.920 | 409.38 | |
| 15 | Ex. Cover | 2.4606 | 0.16404 | 1.5 | 245.140 | 367.71 | |
| 16 | Ex. Fill | 2.62464 | 0.16404 | 3.0 | 290.000 | 870 | |
| 17 | Ex. Fill | 2.78868 | 0.16404 | 3.0 | 365.980 | 1097.94 | |
| 18 | Ex. Fill | 2.95272 | 0.16404 | 3.0 | 375.280 | 1125.84 | |
| 19 | Ex. Fill | 3.11676 | 0.16404 | 3.0 | 383.700 | 1151.1 | |
| 20 | Ex. Fill | 3.2808 | 0.16404 | 3.0 | 386.240 | 1158.72 | |

| | | | | | | | |
|----|-----------------|----------|---------|-----|---------|---------|-------|
| 21 | Ex. Fill | 3.44484 | 0.16404 | 3.0 | 336.140 | 1008.42 | 679.7 |
| 22 | Ex. Fill | 3.60888 | 0.16404 | 3.0 | 330.920 | 992.76 | |
| 23 | Ex. Fill | 3.77292 | 0.16404 | 3.0 | 272.280 | 816.84 | |
| 24 | Ex. Fill | 3.93696 | 0.16404 | 3.0 | 225.760 | 677.28 | |
| 25 | Ex. Fill | 4.101 | 0.16404 | 3.0 | 216.060 | 648.18 | |
| 26 | Ex. Fill | 4.26504 | 0.16404 | 3.0 | 275.980 | 827.94 | |
| 27 | Ex. Fill | 4.42908 | 0.16404 | 3.0 | 454.200 | 1362.6 | |
| 28 | Ex. Fill | 4.59312 | 0.16404 | 3.0 | 589.960 | 1769.88 | |
| 29 | Ex. Fill | 4.75716 | 0.16404 | 3.0 | 559.740 | 1679.22 | |
| 30 | Ex. Fill | 4.9212 | 0.16404 | 3.0 | 470.260 | 1410.78 | |
| 31 | Ex. Fill | 5.08524 | 0.16404 | 3.0 | 361.140 | 1083.42 | |
| 32 | Ex. Fill | 5.24928 | 0.16404 | 3.0 | 252.520 | 757.56 | |
| 33 | Ex. Fill | 5.41332 | 0.16404 | 3.0 | 191.460 | 574.38 | |
| 34 | Ex. Fill | 5.57736 | 0.16404 | 3.0 | 225.240 | 675.72 | |
| 35 | Ex. Fill | 5.7414 | 0.16404 | 3.0 | 272.040 | 816.12 | |
| 36 | Ex. Fill | 5.90544 | 0.16404 | 3.0 | 346.740 | 1040.22 | |
| 37 | Ex. Fill | 6.06948 | 0.16404 | 3.0 | 353.620 | 1060.86 | |
| 38 | Ex. Fill | 6.23352 | 0.16404 | 3.0 | 427.300 | 1281.9 | |
| 39 | Ex. Fill | 6.39756 | 0.16404 | 3.0 | 464.140 | 1392.42 | |
| 40 | Ex. Fill | 6.5616 | 0.16404 | 3.0 | 440.160 | 1320.48 | |
| 41 | Ex. Fill | 6.72564 | 0.16404 | 3.0 | 371.200 | 1113.6 | |
| 42 | Ex. Fill | 6.88968 | 0.16404 | 3.0 | 319.960 | 959.88 | |
| 43 | Ex. Fill | 7.05372 | 0.16404 | 3.0 | 245.000 | 735 | |
| 44 | Ex. Fill | 7.21776 | 0.16404 | 3.0 | 195.040 | 585.12 | |
| 45 | Ex. Fill | 7.3818 | 0.16404 | 3.0 | 187.900 | 563.7 | |
| 46 | Ex. Fill | 7.54584 | 0.16404 | 3.0 | 168.900 | 506.7 | |
| 47 | Ex. Fill | 7.70988 | 0.16404 | 3.0 | 186.360 | 559.08 | |
| 48 | Ex. Fill | 7.87392 | 0.16404 | 3.0 | 183.440 | 550.32 | |
| 49 | Ex. Fill | 8.03796 | 0.16404 | 3.0 | 163.540 | 490.62 | |
| 50 | Ex. Fill | 8.202 | 0.16404 | 3.0 | 164.560 | 493.68 | |
| 51 | Ex. Fill | 8.36604 | 0.16404 | 3.0 | 162.660 | 487.98 | |
| 52 | Ex. Fill | 8.53008 | 0.16404 | 3.0 | 149.400 | 448.2 | |
| 53 | Ex. Fill | 8.69412 | 0.16404 | 3.0 | 132.320 | 396.96 | |
| 54 | Ex. Fill | 8.85816 | 0.16404 | 3.0 | 115.240 | 345.72 | |
| 55 | Ex. Fill | 9.0222 | 0.16404 | 3.0 | 115.100 | 345.3 | |
| 56 | Ex. Fill | 9.18624 | 0.16404 | 3.0 | 106.820 | 320.46 | |
| 57 | Ex. Fill | 9.35028 | 0.16404 | 3.0 | 100.060 | 300.18 | |
| 58 | Ex. Fill | 9.51432 | 0.16404 | 3.0 | 100.700 | 302.1 | |
| 59 | Ex. Fill | 9.67836 | 0.16404 | 3.0 | 96.880 | 290.64 | |
| 60 | Ex. Fill | 9.8424 | 0.16404 | 3.0 | 90.640 | 271.92 | |
| 61 | Ex. Fill | 10.00644 | 0.16404 | 3.0 | 86.300 | 258.9 | |
| 62 | Ex. Fill | 10.17048 | 0.16404 | 3.0 | 95.980 | 287.94 | |
| 63 | Ex. Fill | 10.33452 | 0.16404 | 3.0 | 78.140 | 234.42 | |
| 64 | Ex. Fill | 10.49856 | 0.16404 | 3.0 | 69.980 | 209.94 | |
| 65 | Ex. Fill | 10.6626 | 0.16404 | 3.0 | 83.500 | 250.5 | |
| 66 | Ex. Fill | 10.82664 | 0.16404 | 3.0 | 88.600 | 265.8 | |
| 67 | Ex. Fill | 10.99068 | 0.16404 | 3.0 | 87.580 | 262.74 | |
| 68 | Ex. Fill | 11.15472 | 0.16404 | 3.0 | 74.580 | 223.74 | |
| 69 | Ex. Fill | 11.31876 | 0.16404 | 3.0 | 62.200 | 186.6 | |
| 70 | Ex. Fill | 11.4828 | 0.16404 | 3.0 | 54.680 | 164.04 | |
| 71 | Ex. Fill | 11.64684 | 0.16404 | 3.0 | 52.520 | 157.56 | |
| 72 | Ex. Fill | 11.81088 | 0.16404 | 3.0 | 47.420 | 142.26 | |
| 73 | Ex. Fill | 11.97492 | 0.16404 | 3.0 | 37.740 | 113.22 | |
| 74 | Coarse Tailings | 12.13896 | 0.16404 | 2.0 | 37.480 | 74.96 | |
| 75 | Coarse Tailings | 12.303 | 0.16404 | 2.0 | 38.620 | 77.24 | |
| 76 | Coarse Tailings | 12.46704 | 0.16404 | 2.0 | 43.600 | 87.2 | |
| 77 | Coarse Tailings | 12.63108 | 0.16404 | 2.0 | 42.840 | 85.68 | |
| 78 | Coarse Tailings | 12.79512 | 0.16404 | 2.0 | 37.860 | 75.72 | |
| 79 | Coarse Tailings | 12.95916 | 0.16404 | 2.0 | 52.900 | 105.8 | |

| | | | | | | |
|-----|-----------------|----------|---------|-----|---------|--------|
| 80 | Coarse Tailings | 13.1232 | 0.16404 | 2.0 | 174.260 | 348.52 |
| 81 | Coarse Tailings | 13.28724 | 0.16404 | 2.0 | 170.440 | 340.88 |
| 82 | Coarse Tailings | 13.45128 | 0.16404 | 2.0 | 230.720 | 461.44 |
| 83 | Coarse Tailings | 13.61532 | 0.16404 | 2.0 | 187.760 | 375.52 |
| 84 | Coarse Tailings | 13.77936 | 0.16404 | 2.0 | 94.340 | 188.68 |
| 85 | Coarse Tailings | 13.9434 | 0.16404 | 2.0 | 99.440 | 198.88 |
| 86 | Coarse Tailings | 14.10744 | 0.16404 | 2.0 | 62.980 | 125.96 |
| 87 | Coarse Tailings | 14.27148 | 0.16404 | 2.0 | 32.640 | 65.28 |
| 88 | Coarse Tailings | 14.43552 | 0.16404 | 2.0 | 25.120 | 50.24 |
| 89 | Coarse Tailings | 14.59956 | 0.16404 | 2.0 | 27.660 | 55.32 |
| 90 | Coarse Tailings | 14.7636 | 0.16404 | 2.0 | 52.400 | 104.8 |
| 91 | Coarse Tailings | 14.92764 | 0.16404 | 2.0 | 64.620 | 129.24 |
| 92 | Coarse Tailings | 15.09168 | 0.16404 | 2.0 | 70.500 | 141 |
| 93 | Coarse Tailings | 15.25572 | 0.16404 | 2.0 | 76.860 | 153.72 |
| 94 | Coarse Tailings | 15.41976 | 0.16404 | 2.0 | 81.200 | 162.4 |
| 95 | Coarse Tailings | 15.5838 | 0.16404 | 2.0 | 88.340 | 176.68 |
| 96 | Coarse Tailings | 15.74784 | 0.16404 | 2.0 | 97.140 | 194.28 |
| 97 | Coarse Tailings | 15.91188 | 0.16404 | 2.0 | 103.900 | 207.8 |
| 98 | Coarse Tailings | 16.07592 | 0.16404 | 2.0 | 106.320 | 212.64 |
| 99 | Coarse Tailings | 16.23996 | 0.16404 | 2.0 | 104.780 | 209.56 |
| 100 | Coarse Tailings | 16.404 | 0.16404 | 2.0 | 99.040 | 198.08 |
| 101 | Coarse Tailings | 16.56804 | 0.16404 | 2.0 | 92.680 | 185.36 |
| 102 | Coarse Tailings | 16.73208 | 0.16404 | 2.0 | 89.240 | 178.48 |
| 103 | Coarse Tailings | 16.89612 | 0.16404 | 2.0 | 87.320 | 174.64 |
| 104 | Coarse Tailings | 17.06016 | 0.16404 | 2.0 | 85.280 | 170.56 |
| 105 | Coarse Tailings | 17.2242 | 0.16404 | 2.0 | 84.000 | 168 |
| 106 | Coarse Tailings | 17.38824 | 0.16404 | 2.0 | 86.040 | 172.08 |
| 107 | Coarse Tailings | 17.55228 | 0.16404 | 2.0 | 85.020 | 170.04 |
| 108 | Coarse Tailings | 17.71632 | 0.16404 | 2.0 | 88.080 | 176.16 |
| 109 | Coarse Tailings | 17.88036 | 0.16404 | 2.0 | 88.080 | 176.16 |
| 110 | Coarse Tailings | 18.0444 | 0.16404 | 2.0 | 85.660 | 171.32 |
| 111 | Coarse Tailings | 18.20844 | 0.16404 | 2.0 | 89.240 | 178.48 |
| 112 | Coarse Tailings | 18.37248 | 0.16404 | 2.0 | 94.840 | 189.68 |
| 113 | Coarse Tailings | 18.53652 | 0.16404 | 2.0 | 90.260 | 180.52 |
| 114 | Coarse Tailings | 18.70056 | 0.16404 | 2.0 | 91.140 | 182.28 |
| 115 | Coarse Tailings | 18.8646 | 0.16404 | 2.0 | 90.640 | 181.28 |
| 116 | Coarse Tailings | 19.02864 | 0.16404 | 2.0 | 89.860 | 179.72 |
| 117 | Coarse Tailings | 19.19268 | 0.16404 | 2.0 | 87.320 | 174.64 |
| 118 | Coarse Tailings | 19.35672 | 0.16404 | 2.0 | 86.560 | 173.12 |
| 119 | Coarse Tailings | 19.52076 | 0.16404 | 2.0 | 82.980 | 165.96 |
| 120 | Coarse Tailings | 19.6848 | 0.16404 | 2.0 | 83.360 | 166.72 |
| 121 | Coarse Tailings | 19.84884 | 0.16404 | 2.0 | 79.680 | 159.36 |
| 122 | Coarse Tailings | 20.01288 | 0.16404 | 2.0 | 78.780 | 157.56 |
| 123 | Coarse Tailings | 20.17692 | 0.16404 | 2.0 | 79.420 | 158.84 |
| 124 | Coarse Tailings | 20.34096 | 0.16404 | 2.0 | 81.960 | 163.92 |
| 125 | Coarse Tailings | 20.505 | 0.16404 | 2.0 | 81.960 | 163.92 |
| 126 | Coarse Tailings | 20.66904 | 0.16404 | 2.0 | 84.900 | 169.8 |
| 127 | Coarse Tailings | 20.83308 | 0.16404 | 2.0 | 85.540 | 171.08 |
| 128 | Coarse Tailings | 20.99712 | 0.16404 | 2.0 | 83.760 | 167.52 |
| 129 | Coarse Tailings | 21.16116 | 0.16404 | 2.0 | 84.780 | 169.56 |
| 130 | Coarse Tailings | 21.3252 | 0.16404 | 2.0 | 82.600 | 165.2 |
| 131 | Coarse Tailings | 21.48924 | 0.16404 | 2.0 | 78.140 | 156.28 |
| 132 | Coarse Tailings | 21.65328 | 0.16404 | 2.0 | 72.920 | 145.84 |
| 133 | Coarse Tailings | 21.81732 | 0.16404 | 2.0 | 77.120 | 154.24 |
| 134 | Coarse Tailings | 21.98136 | 0.16404 | 2.0 | 77.240 | 154.48 |
| 135 | Coarse Tailings | 22.1454 | 0.16404 | 2.0 | 68.840 | 137.68 |
| 136 | Coarse Tailings | 22.30944 | 0.16404 | 2.0 | 57.100 | 114.2 |
| 137 | Coarse Tailings | 22.47348 | 0.16404 | 2.0 | 68.080 | 136.16 |
| 138 | Coarse Tailings | 22.63752 | 0.16404 | 2.0 | 82.600 | 165.2 |

158.4

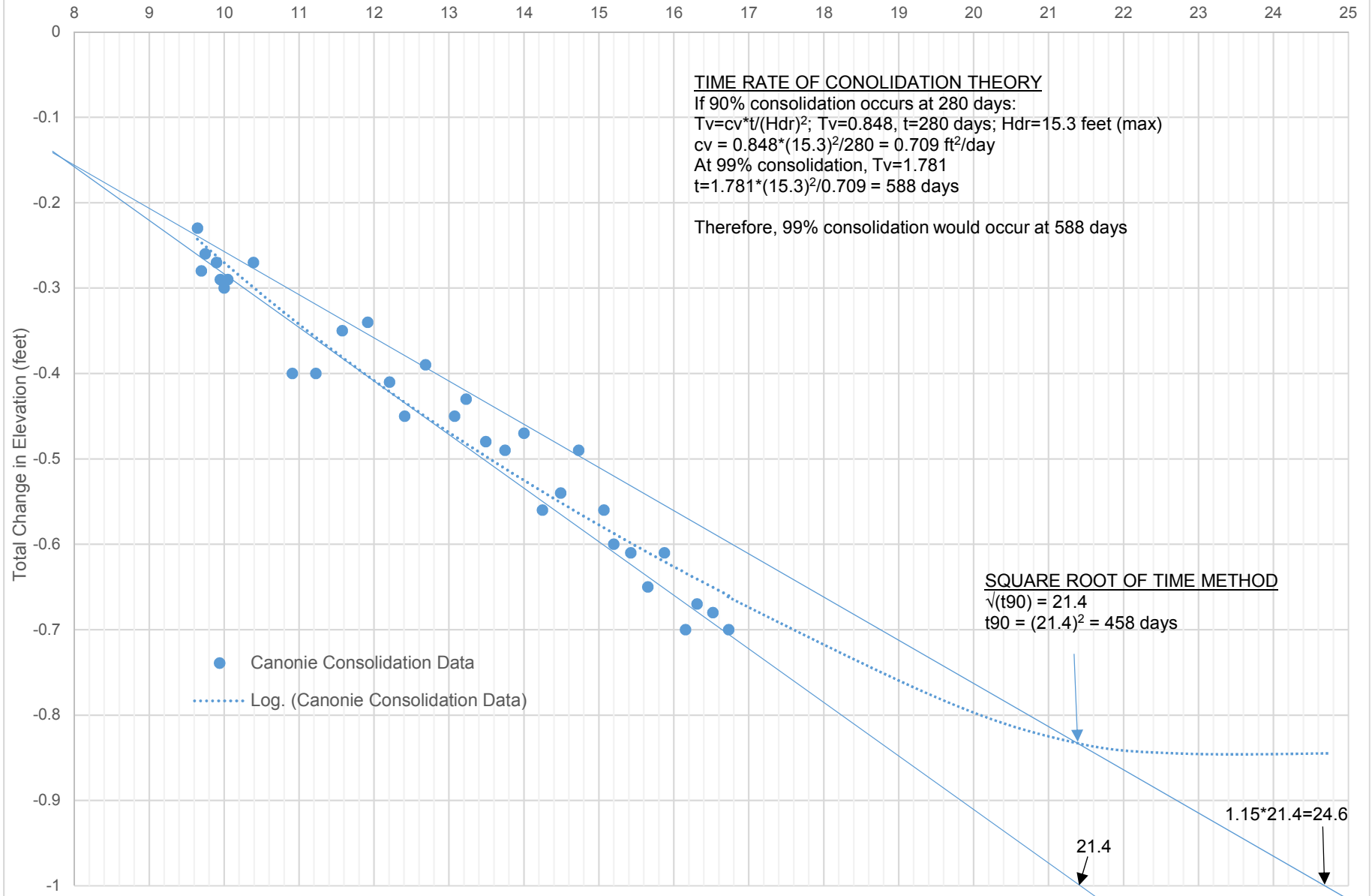
| | | | | | | |
|-----|-----------------|----------|---------|-----|--------|--------|
| 139 | Coarse Tailings | 22.80156 | 0.16404 | 2.0 | 90.380 | 180.76 |
| 140 | Coarse Tailings | 22.9656 | 0.16404 | 2.0 | 87.440 | 174.88 |
| 141 | Coarse Tailings | 23.12964 | 0.16404 | 2.0 | 65.780 | 131.56 |
| 142 | Coarse Tailings | 23.29368 | 0.16404 | 2.0 | 53.800 | 107.6 |
| 143 | Coarse Tailings | 23.45772 | 0.16404 | 2.0 | 45.120 | 90.24 |
| 144 | Coarse Tailings | 23.62176 | 0.16404 | 2.0 | 49.720 | 99.44 |
| 145 | Coarse Tailings | 23.7858 | 0.16404 | 2.0 | 48.960 | 97.92 |
| 146 | Coarse Tailings | 23.94984 | 0.16404 | 2.0 | 35.820 | 71.64 |
| 147 | Coarse Tailings | 24.11388 | 0.16404 | 2.0 | 32.380 | 64.76 |
| 148 | Coarse Tailings | 24.27792 | 0.16404 | 2.0 | 43.600 | 87.2 |
| 149 | Coarse Tailings | 24.44196 | 0.16404 | 2.0 | 41.040 | 82.08 |
| 150 | Coarse Tailings | 24.606 | 0.16404 | 2.0 | 48.320 | 96.64 |
| 151 | Coarse Tailings | 24.77004 | 0.16404 | 2.0 | 58.120 | 116.24 |
| 152 | Coarse Tailings | 24.93408 | 0.16404 | 2.0 | 72.920 | 145.84 |
| 153 | Coarse Tailings | 25.09812 | 0.16404 | 2.0 | 95.100 | 190.2 |

ATTACHMENT E

ESTIMATED TIME TO COMPLETE PRIMARY CONSOLIDATION

Time to Complete Primary Consolidation

Square Root of Time (days)



ATTACHMENT G.4
Repository Seismic Settlement Analysis

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Seismic Settlement Analysis for the Mill Site Repository*

Sheet: *1* of *13*
Date: *11/12/2019*
Job No: *233001048*

ATTACHMENT G.4: REPOSITORY SEISMIC SETTLEMENT ANALYSIS

| Revisioning | | | | | |
|-------------|-----------|--------------------------|------------|----------|-------------|
| Rev. | Date | Description | By | Checked | Date |
| 0 | June 2016 | Preliminary (30%) Design | S. McManus | N. Brink | 27 May 2016 |
| 1 | July 2017 | 95% Design | S. Downey | C. Weber | 15 Sep 2017 |
| 2 | Oct 2019 | Update Cover Thickness | S. Downey | C. Weber | 12 Nov 2019 |

| Location and Format |
|--|
| <p>Electronic copies of these calculations are located on the Stantec internal project teamsite.</p> <p>The following calculations were generated using the following software:</p> <p>Microsoft Excel</p> |

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| References | 13 |

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Seismic Settlement Analysis for the Mill Site Repository*

Sheet: 2 of 13
Date: 11/12/2019
Job No: 233001048

Objective

This calculation brief documents the inputs, assumptions, methods, results and conclusions of the seismic settlement analysis for the proposed Church Rock Mill Site (Mill Site) repository. This brief discusses settlement that may occur within the footprint of the proposed repository as a result of the design seismic event. Other components of settlement (immediate, primary, and secondary settlement) are discussed in Attachment G.3. Liquefaction induced settlement is discussed in Attachment G.6.

Background

This analysis has been performed as part of the design of the Removal Action (RA) at the Northeast Church Rock Mine Site (Mine Site) and the related Remedial Action (RA) at the Mill Site. The Mine Site and Mill Site are located in close proximity to one another, approximately 16 miles northeast of Gallup, in McKinley County, New Mexico. They are located on adjacent Sections approximately one-half mile apart. The sites are temporarily being treated as one facility for purposes of the RA. The combined site is referred to as the "Settlement Agreement Site" (SA Site).

Site History

The NECR mine is a historical uranium mine operated by United Nuclear Corporation (UNC). Mining development began in 1967 and ended in 1982. While the mine operated, it served as the principal mineral source for the UNC uranium mill. The uranium mill and its adjacent disposal cells make up the UNC Superfund Site (the "UNC Mill Site"). Remedial activities addressing source control and on-site surface reclamation are being implemented by UNC under the direction of the US Nuclear Regulatory Commission (NRC), pursuant to the UNC facility's NRC license, and integrated with the USEPA's selected remedy for the groundwater.

The tailings disposal area (TDA) is an unlined facility bounded by an embankment and subdivided by cross-dikes into three cells, which are identified as the South Cell, Central Cell, and North Cell. An estimated 3.5 million tons of tailings were pumped as slurry from the UNC mill to the TDA.

Proposed Remedial Action

The proposed repository will be constructed on top of the existing TDA and will incorporate controlled placement of mine waste on top of the existing TDA cover/radon barrier and a final evapotranspirative (ET) cover placed over the mine waste. Improvements to the existing TDA cover/radon barrier within the footprint of the proposed repository will be completed prior to placement of mine waste. **Figure 1** shows the location and grading of the proposed repository.

The design for the selected repository alternative will be evaluated as part of a NRC license amendment request for the existing licensed facility. The repository features that affect the licensed facility will meet performance standards outlined in NRC regulations and areas of the existing facility affected by the repository construction will be evaluated for compliance. However, existing conditions of the facility not affected by the proposed repository were not evaluated as part of this analysis, as they are managed by the existing NRC license.

Site Description

The natural stratigraphy at the Mill Site is divided into two main components: the surficial unconsolidated deposits (alluvium) and the underlying consolidated bedrock units. The alluvium consists of a mixture of sand, silt, and clay with minor portions of gravel. Alluvial thicknesses at the site are usually around 50 feet, but exceed 120 feet in some locations. Generally, the uppermost bedrock unit at the site is the Upper Gallup Sandstone, though in some locations it is overlain by coal or the Mancos shale.

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The TDA was constructed on top of the native alluvium and deposition of tailings via slurry within the TDA resulted in an interbedded accumulation of tailings. TDA closure construction began in 1989 and was completed in 1995. Closure construction included placement of an interim cover (general fill) from 1989 through 1991 followed by placement of the final cover (radon barrier and erosion protection layer) from 1993 through 1995.

Measurements taken in alluvial monitoring wells (see **Attachment E**) show an alluvial groundwater table in the vicinity of the TDA at approximately 6,867 feet above mean sea level (amsl), which indicates that the alluvium is unsaturated above this elevation. Additionally, subsurface investigations of the TDA indicate that there is not a consistent static water level within the tailings or the alluvium above approximately 6,867 feet amsl. However, localized perched zones of saturation exist within the low-permeability, fine-grained tailings. These zones of saturation do not appear to extend beyond the fine-grained tailings into the higher-permeability coarse-grained tailings.

Site Investigation

In 2013, MWH performed pre-design studies (PDS) at the Mill Site and Mine Site to supplement previous site investigations and collect pre-design data necessary to perform the Remedial Design (RD). Activities performed as part of the Mill Site PDS included: surveying, cone penetration tests (CPTs), drilling, standard penetration tests (SPTs), excavation and soil sampling, and subsequent laboratory testing. Geotechnical data collected during the PDS are presented in the PDS reports (MWH, 2014a and MWH, 2014b) and summarized in **Attachment A**. A list of the materials encountered within the TDA during the PDS is presented in the Assumptions section below. Geotechnical properties for these materials and discussion of one-dimensional stratigraphic profiles used in the seismic settlement analysis are also presented in the Assumptions section below.

Applicable Codes and Standards

Applicable regulatory guidance documents include the following:

- NUREG 1620, Section 2.3 (NRC, 2003)
- Naval Facilities Engineering Command (NAVFAC) Design Manual 7.01, Chapter 5, Sections 3 and 4 (NAVFAC, 1986)
- Technical Approach Document, Revision II, Section 6.3 (DOE, 1989).

Methods

General

The seismic settlement analysis evaluated the magnitude of potential settlement that may occur within the footprint of the proposed Mill Site repository as a result of the design seismic event. Analysis of one-dimensional stratigraphic profiles was performed according to the methods outlined in *Seismic Compression Analysis of As-Compacted Fill Soils with Variable Levels of Fines Content and Fines Plasticity* (Stewart et al., 2004). The design seismic event was characterized by the parameters presented in the UNC-NECR Seismic Hazard Analysis (see Appendix G to the Northeast Church Rock Mine Site Removal Action Design Report). The seismic settlement analysis used data collected during CPTs, hollow-stem auger (HSA) drilling, and laboratory testing to estimate the magnitude of potential seismic settlement within the footprint of the proposed Mill Site repository.

Six one-dimensional stratigraphic profiles were developed and analyzed as part of the seismic settlement analysis (see **Figure 2**). These stratigraphic profiles were developed based on conditions observed during the Mill Site PDS (MWH, 2014a) field investigation and modified to reflect proposed repository construction (placement of mine waste and the repository cover). During the Mill Site PDS field investigation, eight HSA boreholes were “paired” with, and drilled adjacent to, CPTs. Seven of these paired locations are located within the footprint of the proposed Mill Site repository and shear wave velocity measurements were recorded during CPTs at six of those locations. The information acquired

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at these six locations was used to develop the aforementioned profiles. **Figure 1** presents the boreholes and CPTs drilled during the Mill Site PDS.

Subsurface materials considered in the seismic settlement analysis are identified in the Assumptions section below. Subsurface material properties relevant to the seismic settlement analysis and one-dimensional stratigraphic profiles evaluated in this analysis are also presented in the Assumptions section.

Analysis of One-Dimensional Profiles

The following equations were used to analyze the one-dimensional stratigraphic profiles for potential seismic settlement.

The stress reduction factor (r_d) and associated parameters are defined by the following equations (Stewart et al., 2004):

$$\text{For } z < 20\text{m: } r_d = \frac{\left[1 + \frac{a_1}{a_2(z)}\right]}{\left[1 + \frac{a_1}{a_3}\right]}$$

$$\text{For } z > 20\text{m: } r_d = \frac{\left[1 + \frac{a_1}{a_2(z=20)}\right]}{\left[1 + \frac{a_1}{a_3}\right]} - 0.0046 \cdot (z - 20)$$

$$a_1 = -23.013 - 2.949 \cdot \frac{PHA}{g} + 0.999 \cdot m + 0.0053 \cdot V_{s-12}$$

$$a_2(z) = 16.258 + 0.201 \cdot e^{0.341(-z + 0.0785 \cdot V_{s-12} + 7.586)}$$

$$a_3 = 16.258 + 0.201 \cdot e^{0.341(0.0785 \cdot V_{s-12} + 7.586)}$$

Where:

r_d : stress reduction factor, ratio of actual shear stress at depth vs. theoretical "rigid body" shear stress

PHA: peak horizontal acceleration

g : acceleration due to gravity

z : depth below ground surface (meters)

m : earthquake magnitude

V_{s-12} : average shear wave velocity in upper 12m of the site (meters/sec)

$a_2(z=20)$: $a_2(z)$ for $z=20$

The equivalent number of uniform strain cycles for the design seismic event is calculated as follows (Stewart et al., 2004):

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$$N = \frac{\left[\frac{\exp(b_1 + b_2(m - m^*))}{10^{1.5m+16.05}} \right]^{-1/3}}{4.9 \cdot 10^6 \beta} + Sc_1 + rc_2$$

Where:

N: equivalent number of uniform strain cycles

*b*₁: 1.53 (Stewart et al., 2004)

*b*₂: 1.51 (Stewart et al., 2004)

*c*₁: 0.75 (Stewart et al., 2004)

*c*₂: 0.095 (Stewart et al., 2004)

β: 3.2 (Stewart et al., 2004)

m^{*}: 5.5 (Stewart et al., 2004)

m: design earthquake magnitude

r: site-source distance (km)

S: 1.0 [equal to 0 if rock or shallow soil (<20m) underlies the fill and 1 if >20m soil underlies the fill (Stewart et al., 2004)]

Shear strain and related equations are as follows (Stewart et al., 2004):

$$\gamma = \frac{1 + g_1 \cdot e^{g_2 \cdot P}}{1 + g_1} P \cdot 100 \text{ (units of \%)}$$

$$PI \approx 0: \quad g_1 = 0.199 \cdot (\sigma' / p_a)^{0.231} \quad g_2 = 10850 \cdot (\sigma' / p_a)^{-0.410}$$

$$PI \approx 15: \quad g_1 = 0.194 \cdot (\sigma' / p_a)^{0.265} \quad g_2 = 7490 \cdot (\sigma' / p_a)^{-0.418}$$

$$PI \approx 30: \quad g_1 = 4.0 \quad g_2 = 1400$$

$$\gamma_{eff} \frac{G_{eff}}{G_{max}} = \frac{0.65 \cdot PHA \cdot \sigma_0 \cdot r_d}{g \cdot G_{max}} \equiv P$$

Where:

γ: shear strain

*γ*_{eff}: effective shear strain

PI: plasticity index

σ[']: effective stress

*σ*₀: total overburden pressure

*p*_a: atmospheric pressure (calculated for an average elevation of 5,600 feet for the site)

*G*_{eff}: effective shear modulus

*G*_{max}: small strain shear modulus

*G*_{max} is defined by the following equation (Robertson and Cabal, 2012):

$$G_{max} = \rho \cdot V_s^2$$

Where:

ρ: mass density of soil (γ/g)

*V*_s: shear wave velocity of the soil

The volumetric strain and associated parameters were calculated using the following equations (Stewart et al., 2004):

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$$\varepsilon_v = \varepsilon_{v,N=15} \cdot C_N \cdot 2$$

$$\varepsilon_{v,N=15} = a(\gamma_c - \gamma_{tv})^b$$

$$C_N = R \ln(N) + c$$

$$c = 1 - \ln(15) \times R$$

Where:

ε_v : volumetric strain for design seismic event

C_N : normalized vertical strain

$\varepsilon_{v,N=15}$: volumetric strain at 15 cycles

a : material-specific constant (estimated based on relative compaction, soil type, fines content, and plasticity using Figures 6.5 – 6.7 in Stewart et. al, 2004. See Table 1.)

b : material-specific constant (estimated based on relative compaction, soil type, fines content, and plasticity using Figures 6.5 – 6.7 in Stewart et. al, 2004 See Table 1.)

γ_{tv} : threshold shear strain (estimated based on relative compaction, soil type, fines content, and plasticity using Figures 6.5 – 6.7 in Stewart et. al, 2004 See Table 1.)

γ_c : shear strain (same as shear strain, γ , listed above)

R : slope parameter (estimated as 0.36, 0.32, and 0.34 for soils with non-plastic fines, soils with low-plasticity fines, and soils with medium plasticity fines, respectively, as presented in Stewart et al., 2004 pages 86 through 89 See Table 1.)

c : slope parameter estimated from equation listed above

The vertical seismic settlement of a given layer of soil is the product of the volumetric strain for the design seismic event (ε_v) and the thickness of the soil layer (h):

$$\Delta_i = \varepsilon_v \cdot h$$

Material Properties

Subsurface Materials

Material strength parameters used for the settlement analysis were based on data from laboratory testing of materials at the site during the PDS. The laboratory test results are located in Table 3-4 of the Mill Site PDS Report (MWH, 2014a). The parameters used as a base-case scenario for each material are discussed in the main text of Appendix G, Section G.6 and are also summarized in **Table 1** included in this document. Properties for each material were estimated by averaging the results of laboratory tests performed on samples collected during the Mine Site and Mill Site PDS.

A sensitivity analysis was conducted to identify which material property most influences the results and evaluate the effects of varying this property on the total calculated settlement at each location analyzed. The material index properties (dry density and water content) were varied from the base case (average) to the 30th percentile values on individual materials and combined materials. When evaluating the 30th percentile index properties, the results of the seismic settlement analysis were found to be more sensitive to variation in the properties of the tailings materials (coarse, fine, and coarse/fine) than other materials. The 30th percentile material properties for the tailings used in the sensitivity analysis are summarized in **Table 2**.

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The seismic settlement coefficients used in the settlement calculations were obtained from equations and figures in Stewart et al. (2004), as discussed previously in the Methods section. The coefficients (“a”, “b”, and γ_{tv}) for each material were based on the plasticity index of the material (low, moderate, or high plasticity), and varying the index properties to the 30th percentile values did not alter the plasticity of the material or the coefficients, as defined in Stewart et al. (2004). The coefficients were also based on the relative compaction (RC) and saturation (S) for each material. The coefficients and assumptions are discussed below.

Cover and Fill Materials

The cover soil, erosion protection material, and mine spoils are assumed to have similar fines content and plasticity based on the material index properties. The borrow sources that will be used for the cover soil were investigated during the PDS and samples obtained from the investigation were classified as clayey and silty sands and low plasticity clays. Fines content ranged from 38 to 78 percent and plasticity index ranged from 3 to 23. For the purpose of estimating the coefficients, it was assumed that the erosion protection and cover soil materials have moderate plasticity with a relative compaction of approximately 90 percent and an unknown saturation. This results in coefficients a and b of 2.0 and 0.65, respectively and a strain threshold value of 0.03 percent. It was assumed that the mine spoils would also have moderate plasticity with a RC of approximately 90 percent and a S of 90 percent, resulting in coefficients a and b of 1.0 and 0.75, respectively, and a strain threshold value of 0.02 percent.

It has been assumed that the long-term moisture content of the Radon Barrier will be equal to the average of the results of laboratory testing on the Radon Barrier samples (9.3 percent). The saturated moisture content of the Radon Barrier material is 16.1 percent (calculated from laboratory testing results). For the purpose of estimating the coefficients, it was conservatively assumed that the Radon Barrier will have a long-term degree of saturation of 70 percent. Assuming the radon barrier is a moderate plasticity soil with a saturation of 70 percent and a RC of approximately 90 percent, the coefficients a and b were assumed to be 0.65 and 0.75, respectively, with a strain threshold value of 0.02 percent.

The existing fill was encountered during the PDS investigation and was found to be a sandy and low-plasticity clay with fines contents ranging from 35 to 72 percent and PIs ranging from 17 to 20. As defined in Stewart et al. (2004), this material is classified as a moderate plasticity soil, and it was assumed to have a RC of approximately 90 percent with a S of 60 percent, resulting in coefficients a and b of 1.7 and 0.75, respectively, and a threshold strain value of 0.02 percent.

Tailings

Tailings produced by the UNC uranium mill were deposited in the TDA. They range from silty and clayey sands to sandy clays and high plasticity clays. The fines content of tailings samples analyzed during the Mill Site PDS ranged from 7 to 97 percent and the fine-grained particles ranged from non-plastic to a PI of 61. Due to this wide range of material properties, the tailings have been subdivided into three categories for the seismic settlement analysis: coarse tailings, fine tailings, and coarse/fine tailings. The coefficients selected for each material are discussed below.

The coefficients a and b for the coarse tailings were interpolated from values presented in Figures 6.4 and 6.5 of Stewart et al (2004) for soils with varying contents of non-plastic fines. This interpolation assumed that the fines content is equal to the average fines content from laboratory testing of coarse tailings (21 percent), a relative density of 60 percent, a relative compaction of 87 percent, and a saturation of 30 percent. This resulted in coefficients for a and b of 1.79 and 1.00, respectively, and a threshold strain value of 0.01 percent.

For the purpose of estimating coefficients for the fine tailings, it has been conservatively assumed that the fine tailings are high plasticity, 90 percent saturated (or more) and have a low relative compaction (87 percent). The coefficients were assumed to be 0.90 and 0.75 for a and b, respectively, and a threshold strain value of 0.06 percent.

Tailings samples identified as coarse/fine tailings were generally near 100 percent saturation (83 to 99 percent) and were assumed to be moderately plastic. For the purpose of estimating coefficients and the threshold strain value, it has been conservatively assumed that the coarse/fine tailings have a low relative compaction (84 percent) which is applicable to

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any saturation value. This resulted in coefficients a and b of 2.00 and 0.65, respectively, and a threshold strain value of 0.03 percent.

Alluvium

The fines content of alluvium samples analyzed during the Mill Site PDS ranged from 17 to 91 percent and the fine-grained particles ranged from non-plastic to a PI of 31. Due to this wide range of material properties, the alluvium has been subdivided into two categories for the seismic settlement analysis: coarse alluvium and fine alluvium.

The coefficients for the coarse alluvium were interpolated from values presented in Tables 6.4 and 6.5 of Stewart et al. (2004) for soils with varying contents of non-plastic fines. This interpolation assumed that the fines content is equal to the average fines content from laboratory testing of coarse alluvium (36 percent) and a relative density of 60 percent. This resulted in coefficients a and b of 2.00 and 1.00, respectively, and a threshold strain value of 0.01 percent.

For the purpose of estimating coefficients of the fine alluvium, it has been assumed that the fine alluvium is highly plastic, has a degree of saturation of 90 percent (or more), and has a lower relative compaction (87 percent). This resulted in coefficients a and b of 0.90 and 0.75, respectively, and a threshold strain value of 0.06 percent.

Shear Wave Velocities

Shear wave velocities used in this analysis were estimated using the following assumptions:

- Shear wave velocities within existing soils are equal to those measured during the Mill Site PDS (MWH, 2014a) subsurface investigation (see **Attachment B**).
- The shear wave velocity for proposed repository fill is equal to 866 ft/sec. This is the average of the shear wave velocities measured in soil layers logged as general fill during the Mill Site PDS (MWH, 2014a) subsurface investigation (see **Table 3**).
- The average shear wave velocity in upper 12m of the site, V_{s-12} , is equal to 237 m/s. This is the average of the shear wave velocities measured in the upper 12m during the Mill Site PDS (MWH, 2014a) subsurface investigation.

General Assumptions

- The ground surface elevations for paired CPT and HSA boreholes have been estimated from the topographic survey using AutoCAD Civil3D.
- The design seismic event is the 10,000-year return period earthquake, which has a maximum peak ground acceleration (PGA) of 0.30g, a magnitude of 5.5, and a site-source distance of 20 km, as identified in the UNC-NECR Seismic Hazard Analysis (SHA; see Appendix G, Attachment G.1).
Consolidation and the corresponding increase in saturation was not considered in the seismic settlement analysis. The majority of materials are tailings, and the coarse tailings material are not subject to consolidation since they are unsaturated (see Appendix G, Attachment G.3 for discussion on coarse tailings material). The effect of the increase in saturation in the fine tailings and fine/coarse tailings would be negligible due to the assumption in the analyses that these materials are near saturation (fine tailings are at least 90 percent saturated and coarse/fine tailings are at least 83 percent saturated). The saturation in Stewart et al. (2004) ranged from 60 to 90 percent for both high and moderate plasticity soils (fine tailings and coarse/fine tailings, respectively). As discussed previously, the fine tailings were assumed to have a 90 percent saturation value for selecting the coefficients, and the coarse/fine tailings were assumed to have a low relative compaction that applied to any saturation value for selecting the coefficients. This resulted in using the highest "a" value and lowest "b" value for each material. An increase in saturation (due to consolidation or otherwise) would not affect the selected a and b values for the tailings, thus no change in the results presented herein would be anticipated.

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Stratigraphic Profiles

During the Mill Site PDS, eight CPTs were paired with boreholes to correlate CPT results with direct observation of the materials encountered. The borehole logs are presented in **Attachment C** and plots of the CPT measurements are presented in **Attachment D**. Seven of these “paired” CPT locations are within the footprint of the proposed Mill Site repository and shear wave velocity measurements were recorded during CPTs at six of those locations. The CPT data combined with the profiles from the borehole logs were used to define the thickness and texture of the soil layers, as well as the location of the contact between the tailings and underlying alluvium. The relationships used to define the tailings-alluvium contact are described in the Mill Site PDS (MWH, 2014a). These subsurface profiles were modified using the proposed repository design to reflect proposed repository construction (placement of mine waste and the repository cover). The final one-dimensional profiles (**Figure 2**) and associated shear wave velocity and CPT results (**Attachments B and D**) were used in the seismic settlement analysis.

Groundwater

Groundwater was encountered during drilling in two of the boreholes (TI-B10 and TI-B11) within the footprint of the proposed repository. In both of these boreholes the groundwater elevation was approximately 6,885 feet amsl. Groundwater was also encountered at about 6,903 feet amsl while drilling in boring B3 (drilled through the dam). In addition, alluvial wells 509D and EPA 23 (measured on 1/4/2016) show an alluvial ground water elevation of approximately 6,867 feet amsl. These elevations are below the bottom of the tailings and exceed the depth at which seismic settlement is likely to occur. Therefore, the seismic settlement analysis did not assume the presence of a consistent static water level within the TDA or the underlying alluvium. Water level measurements taken in the vicinity of the TDA are presented in **Attachment E**.

For the purpose of this analysis, it is assumed that localized perched zones of saturated tailings are present above the water levels in the alluvial wells and encountered while drilling. Saturated tailings are comprised mostly of the fine-grained tailings that exhibit a low hydraulic conductivity. It is assumed that hydrostatic conditions do not exist in the alluvium, above the static water levels observed while drilling the HSA boreholes or the nearby alluvial monitoring wells.

In some cases, the laboratory data does not support the assumption of saturation within the fine tailings. However, for the purposes of this analysis, it has been conservatively assumed that hydrostatic conditions are present in tailings at or above 85 percent saturation.

It is also assumed that pore pressure dissipation tests performed in the tailings were unable to reach equilibrium due to the low hydraulic conductivity of the tailings deposited in the TDA. Therefore, results of the pore pressure dissipation tests are likely to be artificially elevated and are poor indicators of saturation. Assumptions regarding perched zones of saturated tailings at each of the paired CPT/boreholes are based on the results of CPTs, observations during HSA drilling and sampling, and subsequent laboratory testing. These assumptions are presented below.

CPT-01 and TI-B1

- It is assumed that saturated tailings do not exist at this location.
- Dynamic pore pressures recorded by the CPT in the tailings at CPT-01 were not elevated, indicating that these tailings are unsaturated.
- Eight tailings samples from TI-B1 were analyzed for water contents. All but one of these samples had degrees of saturation below 85 percent (1 percent to 63 percent saturation).
- One tailings sample (from approximately 31.25 ft to 31.5 ft below ground surface [bgs]) was nearly saturated (94 percent saturation). However, it is assumed that this sample is not indicative of the soils in this area and was collected from a discontinuous layer of interbedded fine tailings for the following reasons:
 - Three samples from the approximately 1.25 ft of immediately overlying soil (30 ft to 31.25 ft bgs) did not exhibit this level of saturation (1 percent to 47 percent saturation).
 - Analysis of a soil sample from approximately one foot below this sample (32 ft to 33 ft bgs) did not exhibit this level of saturation (63 percent saturation).

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- The boring log for TI-B1 indicated that the soils from 18.5 ft to 34.3 ft bgs are coarse tailings, an observation that is supported by laboratory analysis (other samples in this layer had fines contents of 7 percent, 9 percent and 53 percent). However, the sample in question had a fines content of 69 percent and was identified as fine tailings.

CPT-02 and TI-B2

- It has been assumed that saturated tailings do not exist at this location.
- Dynamic pore pressures recorded by the CPT in this zone were not elevated, indicating that this zone is unsaturated.
- This assumption is supported by the degree of saturation calculated from laboratory testing results (77 percent saturation).

CPT-08 and TI-B8

- It has been assumed for this analysis that there are three localized perched zones of saturation (hydrostatic conditions) within the tailings at this location.
- These zones of saturation correspond to the layers of fine tailings observed during HSA drilling:
 - 26.3 ft to 31.0 ft bgs
 - 32.5 ft to 35.0 ft bgs
 - 38.6 ft to 44.5 ft bgs
- The uppermost zone that is assumed to be saturated (26.3 ft to 31.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 26.9 ft to 31.5 ft bgs during the CPT.
 - A sample from this layer was submitted for laboratory analysis, the results of which indicate that the material is fully saturated (100 percent saturation).
- The middle zone of assumed saturation (32.5 ft to 35.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 32.8 ft to 35.1 ft bgs during the CPT.
- The lowermost zone that is assumed to be saturated (38.6 ft to 44.5 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured in this layer during the CPT.
 - Samples from this layer were submitted for laboratory analysis, the results of which indicate that the material is 93 percent to 99 percent saturated.
- Laboratory analysis of samples of the coarse tailings and coarse/fine tailings between the layers of fine tailings at TI-B8 indicated that these soils are unsaturated (44 percent to 67 percent saturation).
- The dynamic pore pressure measurements taken during CPT-08 also support this pattern of nearly-saturated layers of fine tailings, interbedded with layers of coarser unsaturated tailings.
- The pore pressure dissipation test performed at 31.7 ft bgs did not reach zero, which may indicate saturated conditions; however, the material is classified as a CH (USCS) and has a fines content of 91 percent. It is likely that dynamic pore pressures generated by the CPT probe shearing the soils were very slow to dissipate, resulting in misleading measurements during the pore pressure dissipation test.

CPT-10 and TI-B10

- At this location, it has been assumed for this analysis that free-draining layers of coarse tailings separate three perched zones of saturated tailings:
 - 18.9 ft to 24.4 ft bgs
 - 25.7 ft to 31.0 ft bgs

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- 33.2 ft to 44.6 ft bgs
- The uppermost zone that is assumed to be saturated (18.9 ft to 24.4 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B10. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 17.7 ft to 24.3 ft bgs during the CPT.
 - A sample from this layer was submitted for laboratory analysis, the results of which indicate that the material is 89 percent saturated.
- The uppermost and middle zones of assumed saturation are separated by a layer of unsaturated coarse tailings (24.4 ft to 25.7 ft bgs) identified during HSA drilling.
- The middle zone of assumed saturation (25.7 ft to 31.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B10. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured in this layer during the CPT.
 - Samples from this layer were submitted for laboratory analysis, the results of which indicate that the material is 95 percent to 96 percent saturated.
- The middle and lowermost zones of assumed saturation are separated by a layer of unsaturated coarse tailings (31.0 ft to 33.2 ft bgs) identified during HSA drilling. A sample of this layer was submitted for laboratory analysis, the results of which indicate that the material is unsaturated (62 percent saturation).
- The lowermost zone of assumed saturation (33.2 ft to 44.6 ft bgs) spans two layers of fine tailings (33.2 ft to 36.3 ft bgs and 37.8 ft to 44.6 ft bgs) and the layer of coarse/fine tailings by which they are separated. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured during the CPT at this location to a depth of 43.8 ft bgs.
 - Laboratory analysis of samples of these tailings indicated that these soils are 95 percent to 100 percent saturated.

CPT-11 and TI-B11

- It has been assumed that there is a localized perched zone of saturation (hydrostatic conditions) within the tailings at this location (44.5 ft to 53.9 ft bgs).
- This assumption corresponds to a layer of fine tailings observed during HSA drilling at TI-B11 and is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 43.5 ft to 54.8 ft bgs during the CPT.
 - Laboratory analysis of samples of the fine tailings at TI-B11 indicated that these soils are 95 percent to 100 percent saturated.

CPT-15 and TI-B15

- It has been assumed that there are no saturated tailings at this location. This assumption is based on the following:
 - The tailings at this location are of a coarse nature and therefore have a higher hydraulic conductivity than fine-grained tailings.
 - Multiple samples of tailings from this location were submitted for laboratory analysis. Laboratory analysis indicated that the samples were unsaturated (22 percent to 58 percent saturation).

Other Stratigraphic Assumptions

- The 4.5-foot-thick ET cover will consist of two layers: (1) an erosion protection layer (14-31.5 inches thick) on top of (2) a layer of cover soil (22.5-34 inches thick).
- The top of the ET cover will be the same as the finished grade of the proposed repository grading plan (as shown in **Figure 1**).

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Description: *Seismic Settlement Analysis for the Mill Site Repository*

Sheet: *12* of *13*
Date: *11/12/2019*
Job No: *233001048*

- Improvement and reconditioning of the existing TDA cover and radon barrier within the footprint of the proposed repository will result in a minimum 18-inch-thick radon barrier. The existing erosion protection material will be removed from within the proposed repository footprint during this process. The finished grade of the improved radon barrier will be equal to existing grade.
- Mine waste will be placed from the top of the radon barrier (existing grade) to the bottom of the ET cover (4.5 ft below finished grade).

Calculations

Attachment F presents the seismic settlement analysis calculations.

Results

The seismic settlement analysis evaluated the magnitude of potential settlement that could occur within the footprint of the proposed Mill Site repository as a result of the design seismic event. Analysis of one-dimensional stratigraphic profiles was performed according to the methods outlined in *Seismic Compression Analysis of As-Compacted Fill Soils with Variable Levels of Fines Content and Fines Plasticity* (Stewart et al., 2004). The settlement ranged from 0.08 to 0.14 feet, by location. The results of the seismic settlement analysis are presented in **Table 4** and also included on **Figure 1**.

The sensitivity analysis conducted on the seismic settlement resulted in a slight increase in settlement values when using the 30th percentile values for dry density and water content for the evaluated materials, as compared to using the base case average values. The settlement ranged from 0.09 to 0.15 feet, by location. The results show that varying the material properties to the 30th percentile values had little effect on the calculated seismic settlement values. The settlement values at each location resulting from the sensitivity analysis are summarized in **Table 5**.

Conclusions

The results presented here are based on the current conditions of existing soils, anticipated conditions of repository fill placement, and the design seismic event. The estimated seismic settlements resulting from the design earthquake are small and future densification of existing soils caused by the repository construction may mitigate these effects, reducing the potential seismic settlement that could occur during an earthquake. The potential seismic settlement results are considered within the tolerable limits (6 to 12 inches) of seismic deformation for tailings impoundments described in NUREG-1620 (NRC, 2003).

Client: *GE/UNC*
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Sheet: 13 **of** 13
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Attachments

Figures

- Figure 1 – Borehole and CPT Locations for Seismic Settlement Analysis
- Figure 2 – One-Dimensional Stratigraphic Profiles

Attachments

- Attachment A – Laboratory Results from Pre-Design Studies (MWH, 2014a and MWH, 2014b)
- Attachment B – Measured Shear Wave Velocities in TDA and Underlying Alluvium (MWH, 2014a)
- Attachment C – Tailings Disposal Area Borehole Logs (MWH, 2014a)
- Attachment D – Tailings Disposal Area Cone Penetration Test Results (MWH, 2014a)
- Attachment E – Recorded Water Levels at the Church Rock Site (Chester Engineers, 2016)
- Attachment F – Seismic Settlement Analysis Calculations

References

- Chester Engineers, 2016. Personal Correspondence – Email Table of Measured Water Levels. April 20.
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- US Department of Energy (DOE), 1989. *Technical Approach Document, Revision II*. DOE/UMTRA 050425-002, December.

TABLES

Table 1: Material Properties Used in Seismic Settlement Analysis

| Material Identification | Moist Unit Weight, γ_m (pcf) | Relative Compaction, C_r (% of Standard Proctor) | Fines Content, FC (%) | Plasticity Index, PI | a^2 | b^2 | γ_{tv}^2 | R^2 |
|-------------------------|-------------------------------------|--|-----------------------|----------------------|-------|-------|-----------------|-------|
| Erosion Protection | 122.9 ¹ | 90 ¹ | 37 ¹ | 12 ⁴ | 2.00 | 0.65 | 0.03% | 0.34 |
| Cover Soil | 114.7 | 90 | 53 | 12 | 2.00 | 0.65 | 0.03% | 0.34 |
| Mine Spoils | 116.4 | 90 ¹ | 53 ⁴ | 12 ⁴ | 1.18 | 0.75 | 0.02% | 0.34 |
| Radon Barrier | 122.3 | 95 ¹ | 59 | 16 | 0.65 | 0.75 | 0.02% | 0.34 |
| Existing Fill | 113.8 | 90 | 48 | 19 | 1.70 | 0.75 | 0.02% | 0.34 |
| Coarse Tailings | 108.1 | N/A | 21 | 0 | 1.79 | 1.00 | 0.01% | 0.36 |
| Coarse/Fine Tailings | 116.0 | N/A | 52 | 20 | 2.00 | 0.65 | 0.03% | 0.34 |
| Fine Tailings | 107.6 ³ | N/A | 83 | 43 | 0.90 | 0.75 | 0.06% | 0.25 |
| Coarse Alluvium | 111.0 | N/A | 36 | 0 | 2.00 | 1.00 | 0.01% | 0.36 |
| Fine Alluvium | 120.7 | N/A | 76 | 22 | 0.90 | 0.75 | 0.06% | 0.25 |

Notes:

All values are the average of laboratory testing results, unless otherwise noted.

¹ Assumed

² From Stewart et al., 2004 (pages 84 – 90).

³ Assumes material is fully saturated

⁴ Assumed to be the same as cover soil from the proposed borrow areas

Table 2: Material Properties Used in the Seismic Settlement Sensitivity Analysis

| Material Identification | Water content (by mass, %) | Dry density (pcf) | Calculated Moist Unit Wt (pcf) |
|-------------------------|----------------------------|-------------------|--------------------------------|
| Coarse Tailings | 6.8 | 91.7 | 97.9 |
| Coarse/Fine Tailings | 27.6 | 85.7 | 109.4 |
| Fine Tailings | 41.6 | 66.8 | 94.6 |

Table 3: Shear Wave Velocities Measured in General Fill at the Tailings Disposal Area

| Borehole | Depth below Finished Ground Surface at Time of CPT (ft) | Soil Layer Thickness (ft) | Average Shear Wave Velocity, V_s (ft/sec) | Wave Travel Time Through Soil Layer (ms) |
|--|---|---------------------------|---|--|
| TI-B1/CPT-01 | 2.0 - 13.0 | 11.0 | 777.0 | 14.2 |
| TI-B1/CPT-01 | 14.0 - 18.5 | 4.5 | 731.6 | 6.2 |
| TI-B2/CPT-02 | 2.0 - 12.8 | 10.8 | 849.9 | 12.7 |
| TI-B8/CPT-08 | 2.0 - 7.0 | 5.0 | 1177.0 | 4.2 |
| TI-B8/CPT-08 | 18.0 - 20.7 | 2.7 | 786.4 | 3.4 |
| TI-B10/CPT-10 | 2.0 - 6.8 | 4.8 | 926.0 | 5.2 |
| TI-B11/CPT-11 | 2.0 - 44.5 | 42.5 | 890.5 | 47.7 |
| TI-B15/CPT-15 | 2.0 - 3.0 | 1.0 | 680.0 | 1.5 |
| Total Travel Time (ms): | | | | 95.1 |
| Overall Average Shear Wave Velocity, V_s (ft/sec): | | | | 866 |

Table 4: Potential Seismic Settlement Caused by the Design Seismic Event

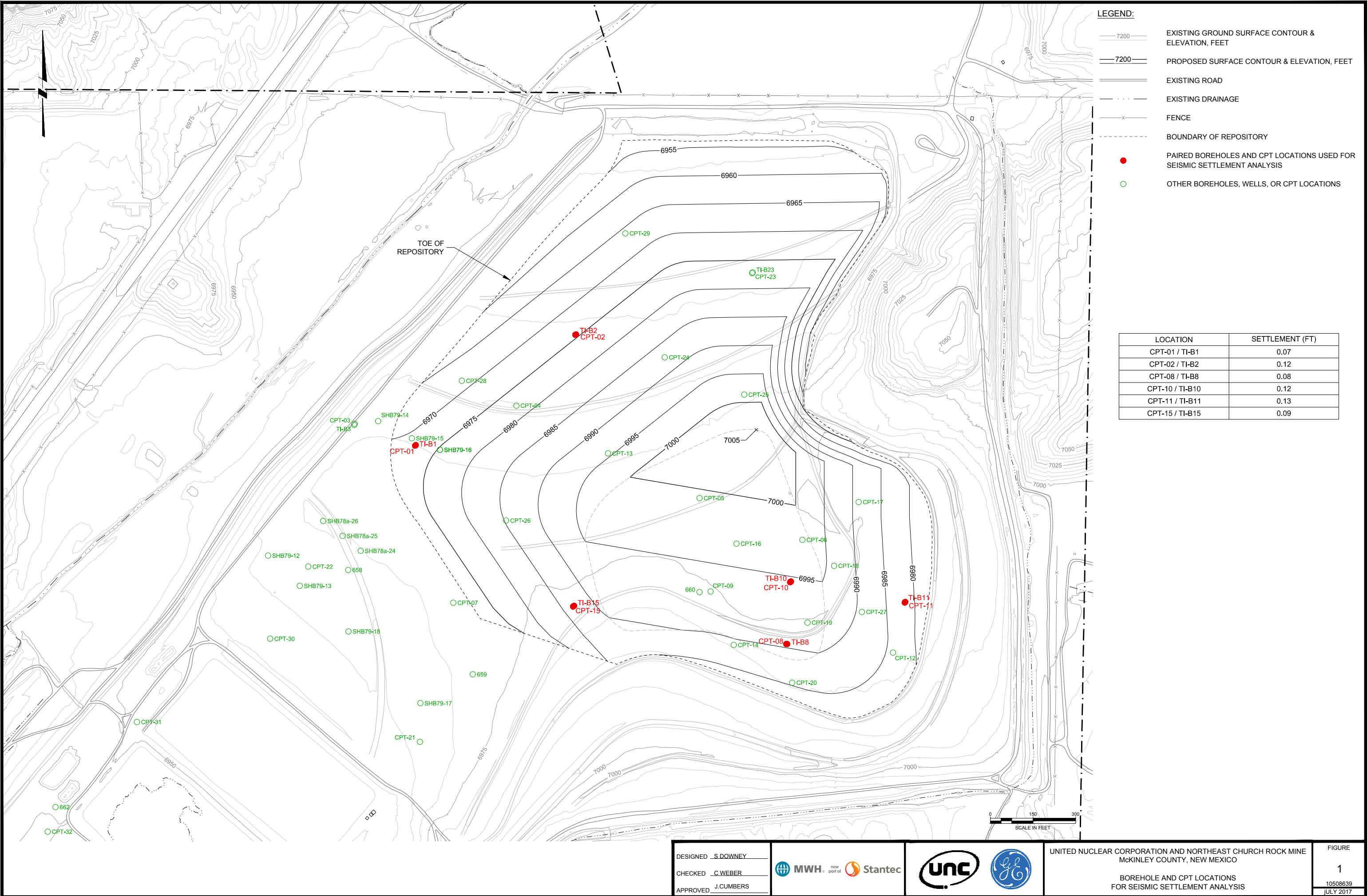
| Borehole ID | Depth of Seismic Settlement (ft) |
|--------------------|---|
| TI-B1/CPT-01 | 0.08 |
| TI-B2/CPT-02 | 0.12 |
| TI-B8/CPT-08 | 0.08 |
| TI-B10/CPT-10 | 0.12 |
| TI-B11/CPT-11 | 0.14 |
| TI-B15/CPT-15 | 0.09 |

Table 5: Potential Seismic Settlement Sensitivity Results

| Borehole ID | Depth of Seismic Settlement (ft) |
|--------------------|---|
| TI-B1/CPT-01 | 0.09 |
| TI-B2/CPT-02 | 0.13 |
| TI-B8/CPT-08 | 0.09 |
| TI-B10/CPT-10 | 0.15 |
| TI-B11/CPT-11 | 0.13 |
| TI-B15/CPT-15 | 0.12 |

FIGURES

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| TI-B1/CPT-01 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 2.1 | Cover Soil |
| 0.0 - 2.0 | 2.1 - 4.1 | Radon Barrier |
| 2.0 - 13.0 | 4.1 - 15.1 | General Fill |
| 13.0 - 15.0 | 15.1 - 16.1 | Coarse Tailings |
| 15.0 - 18.5 | 16.1 - 20.6 | General Fill |
| 18.5 - 34.3 | 20.6 - 36.4 | Coarse Tailings |
| 34.3 - 41.1 | 36.4 - 43.2 | Coarse Alluvium |
| 41.1 - 45.0 | 43.2 - 47.1 | Fine Alluvium |
| 45.0 - 54.0 | 47.1 - 56.1 | Coarse Alluvium |
| 54.0 - 68.2 | 56.1 - 70.3 | Fine Alluvium |
| 68.2 - 70.0 | 70.3 - 72.1 | Coarse Alluvium |

| TI-B2/CPT-02 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 14.3 | Mine Spoils |
| 0.0 - 2.0 | 14.3 - 16.3 | Radon Barrier |
| 2.0 - 12.8 | 16.3 - 27.1 | General Fill |
| 12.8 - 15.0 | 27.1 - 29.3 | Fine Tailings |
| 15.0 - 25.7 | 29.3 - 40.0 | Coarse Alluvium |
| 25.7 - 33.5 | 40.0 - 47.8 | Fine Alluvium |

| TI-B8/CPT-08 | | |
|---|--|----------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 14.2 | Mine Spoils |
| 0.0 - 2.0 | 14.2 - 16.2 | Radon Barrier |
| 2.0 - 7.0 | 16.2 - 21.2 | General Fill |
| 7.0 - 18.0 | 21.2 - 32.2 | Coarse Tailings |
| 18.0 - 20.7 | 32.2 - 34.9 | General Fill |
| 20.7 - 26.3 | 34.9 - 40.5 | Coarse Tailings |
| 26.3 - 31.1 | 40.5 - 45.3 | Fine Tailings |
| 31.1 - 32.5 | 45.3 - 46.7 | Coarse Tailings |
| 32.5 - 35.0 | 46.7 - 49.2 | Fine Tailings |
| 35.0 - 38.6 | 49.2 - 52.8 | Coarse/Fine Tailings |
| 38.6 - 44.5 | 52.8 - 58.7 | Fine Tailings |
| 44.5+ | 58.7+ | Coarse Alluvium |

| TI-B10/CPT-10 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 21.0 | Mine Spoils |
| 0.0 - 2.0 | 21.0 - 23.0 | Radon Barrier |
| 2.0 - 6.8 | 23.0 - 27.8 | General Fill |
| 6.8 - 18.9 | 27.8 - 39.9 | Coarse Tailings |
| 18.9 - 24.4 | 39.9 - 45.4 | Fine Tailings |
| 24.4 - 25.7 | 45.4 - 46.7 | Coarse Tailings |
| 25.7 - 31.0 | 46.7 - 52.0 | Fine Tailings |
| 31.0 - 33.2 | 52.0 - 54.2 | Coarse Tailings |
| 33.2 - 44.6 | 54.2 - 65.6 | Fine Tailings |
| 44.6+ | 65.6+ | Coarse Alluvium |

| TI-B11/CPT-11 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 4.3 | Mine Spoils |
| 0.0 - 2.0 | 4.3 - 6.3 | Radon Barrier |
| 2.0 - 44.5 | 6.3 - 48.8 | General Fill |
| 44.5 - 53.9 | 48.8 - 58.2 | Fine Tailings |
| 53.9 - 55.0 | 58.2 - 59.3 | Fine Alluvium |
| 55.0 - 77.5 | 59.3 - 81.8 | Coarse Alluvium |

| TI-B15/CPT-15 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 5.1 | Mine Spoils |
| 0.0 - 2.0 | 5.1 - 7.1 | Radon Barrier |
| 2.0 - 3.0 | 7.1 - 8.1 | General Fill |
| 3.0 - 30.0 | 8.1 - 35.1 | Coarse Tailings |
| 30.0 - 38.0 | 35.1 - 43.1 | Fine Alluvium |
| 38.0 - 45.0 | 43.1 - 50.1 | Coarse Alluvium |
| 45.0 - 50.0 | 50.1 - 55.1 | Fine Alluvium |
| 50.0 - 52.0 | 55.1 - 57.1 | Coarse Alluvium |
| 52.0 - 65.0 | 57.1 - 70.1 | Fine Alluvium |



PROJECT UNITED NUCLEAR CORPORATION AND
NORTHEAST CHURCH ROCK MINE

TITLE ONE-DIMENSIONAL STRATIGRAPHIC PROFILES



DATE May 2016

FIGURE 2

FILENAME Figure_2

ATTACHMENT A

LABORATORY RESULTS FROM PRE-DESIGN STUDIES (MWH, 2014A AND MWH, 2014B)

Table 3-1 Summary of Geotechnical Laboratory Data - Cover Samples

| Cover Layer | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (in) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass) (%) | Specific Gravity | Standard Proctor (max. dd@opt. w.c.) (pcf @ %) | Atterberg Limits (%) ⁽⁵⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | L.A. Abrasion ⁽⁶⁾ (%) loss | Sodium Soundness ⁽⁷⁾ (%) loss | Absorption ⁽⁸⁾ (%) | Pinhole Dispersion ⁽⁹⁾ | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽¹⁰⁾ | | | Confining Stress (psi) | SWCC: -5 bar Water Content (by mass) (%) ⁽¹⁰⁾ | SWCC: Saturated Water Content (by mass) (%) ⁽¹¹⁾ |
|-------------------------------|-----------------|----------------------------|----------------------------|----|-------------------------------------|---------------------|------------------------------------|-----------------------------|---------------------|--|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|---------------------------------------|--|-------------------------------|-----------------------------------|--|---------|---------|------------------------|--|---|
| | | | | | | | | | | | LL | PL | PI | | | | | | | | | | 90% | 95% | 100% | | | |
| Admix. (Gravel/ Soil Mixture) | TI - CS01 - 02A | Bulk | 0 | 11 | Clayey Gravel with Sand | | Loam | | | | | | | 33.3 | 23.4 | 43.3 | 28.0 | 15.3 | | | | | | | | | | |
| | TI - CS02 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Clay Loam | | 2.81 ⁽⁴⁾ | | | | | 36.9 | 17.0 | 46.1 | 28.8 | 17.3 | 3.8 | 0.37 | 1.06 | | | | | | | |
| | TI - CS03 - 02A | Bulk | 0 | 6 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.7 | 27.7 | 18.1 | 9.6 | | | | | | | | | | |
| | TI - CS04 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.2 | 28.2 | 18.0 | 10.2 | | | | | | | | | | |
| | TI - CS05 - 02A | Bulk | 0 | 9 | Sandy Lean Clay | | Loam | | | | | | | 13.9 | 34.4 | 51.7 | 31.2 | 20.5 | | | | | | | | | | |
| | TI - CS06 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | 2.77 ⁽⁴⁾ | | | | | 48.4 | 18.5 | 33.1 | 23.4 | 9.7 | 5.7 | 0.14 | 1.91 | | | | | | | |
| | TI - CS07 - 02A | Bulk | 0 | 20 | Sandy Lean Clay | CL | Loam | 7.8 | | | 28 | 13 | 15 | 1.1 | 41.0 | 60.9 | 42.4 | 18.5 | | | | | | | | | | |
| | TI - CS08 - 02A | Bulk | 0 | 8 | Clayey Gravel with Sand | | Loam | | | | | | | 56.7 | 18.5 | 24.8 | 17.2 | 7.6 | | | | | | | | | | |
| | TI - CS09 - 02A | Bulk | 0 | 9 | Clayey Gravel | | Loam | | 2.78 ⁽⁴⁾ | | | | | 53.6 | 14.2 | 32.2 | 21.2 | 11.0 | 5.1 | 1.17 | 1.55 | | | | | | | |
| | TI - CS10 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | | | | | | 41.4 | 19.7 | 38.9 | 26.1 | 12.8 | | | | | | | | | | |
| | TI - CS11 - 02A | Bulk | 0 | 9 | Clayey Gravel with Sand | | Sandy Loam | | | | | | | 30.7 | 30.1 | 39.2 | 26.1 | 13.1 | | | | | | | | | | |
| | TI - CS12 - 02A | Bulk | 0 | 14 | Sandy Lean Clay | CL | Loam | 9.1 | | | 33 | 13 | 20 | 1.3 | 28.8 | 69.9 | 43.5 | 26.4 | | | | | | | | | | |
| Radon barrier (clay layer) | TI - CS03 - 04A | Bulk | 6 | 24 | Sandy Lean Clay | CL | Loam | 6.0 | | | 28 | 14 | 14 | 6.3 | 38.7 | 55.0 | 36.1 | 18.9 | | | | | | | | | | |
| | TI - CS06 - 04A | Bulk | 7 | 24 | Sandy Lean Clay | CL | Loam | 11.0 | | | 30 | 13 | 17 | 6.7 | 34.2 | 59.1 | 40.2 | 18.9 | | | | | | | | | | |
| | TI - CS10 - 04A | Bulk | 7 | 25 | Sandy Lean Clay | CL | Loam | 7.7 | | | 29 | 14 | 15 | 2.3 | 39.5 | 58.2 | 36.9 | 21.3 | | | | | | | | | | |
| | TI - CS08 - 04A | Bulk | 8 | 28 | Sandy Lean Clay | CL | Loam | 8.1 | 2.67 | 119.4 @ 11.9 | 27 | 12 | 15 | 11.3 | 35.0 | 53.7 | 36.7 | 17.0 | | | | | 9.1E-06 | 1.1E-05 | 1.5E-06 | 24 | | |
| | TI - CS05 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Loam | 9.6 | | | 29 | 12 | 17 | 1.3 | 37.3 | 61.4 | 42.0 | 19.4 | | | | | | | | | | |
| | TI - CS09 - 04A | Bulk | 9 | 26 | Sandy Lean Clay | CL | Loam | 7.7 | | | 28 | 13 | 15 | 4.0 | 38.1 | 57.9 | 40.0 | 17.9 | | | | | | | | | | |
| | TI - CS11 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Clay Loam | 8.6 | 2.68 | 115.0 @ 14.9 | 32 | 13 | 19 | 5.1 | 28.4 | 66.5 | 40.7 | 25.8 | | | | | 7.6E-08 | 1.4E-07 | 1.0E-07 | 24 | | |
| | TI - CS02 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Sandy Clay Loam | 11.4 | | | 28 | 12 | 16 | 3.6 | 44.7 | 51.7 | 30.4 | 21.3 | | | | | | | | | | |
| | TI - CS04 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Clay Loam | 15.0 | 2.68 | 113.5 @ 15.0 | 35 | 15 | 20 | 0.9 | 35.0 | 68.2 | 37.2 | 26.9 | | | | | 4.6E-06 | 6.2E-06 | 2.3E-07 | 8 | | |
| | TI - CS01 - 04A | Bulk | 11 | 24 | Sandy Lean Clay | CL | Loam | 9.2 | 2.68 | 117.3 @ 13.0 | 29 | 15 | 14 | 2.0 | 39.8 | 58.2 | 39.0 | 19.2 | | | | ND3 | 3.0E-04 | 4.6E-05 | 7.8E-07 | 8 | 8.6 / 9.6 | 21.7 / 19.0 |

- Notes:** 1. Sample Types: Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. Bulk saturated surface dry (SSD) specific gravity of the gravel fraction, average of three results (ASTM C127).
5. LL = liquid limit, PL = plastic limit, PI = plasticity index
6. L.A. abrasion results are percent loss, by mass, for 100 revolutions.
7. Weighted percent loss for the 3/4-inch to 3/8-inch size range
8. Average of three results for the gravel fraction of the cover gravel/soil mixture samples
9. Pinhole dispersion test (ASTM method A) conducted on a specimen remolded to approximately 95% of the maximum standard Proctor density at optimum water content. ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head.
10. Flexible wall permeameter tests conducted on specimens remolded to approximately 90, 95 and 100% of the maximum standard Proctor density and tested at the confining stresses shown in the table.
11. SWCC test conducted on material passing the No. 10 sieve, remolded to approximately 95% of the maximum standard Proctor density and optimum water content. SWCC tests performed with pairs of specimens for each test.

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | saturation (%) | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | LL | Atterberg limits (%) PLPI | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) | |
|--------------|--------|----------------------------|-----------------------------|-------|-------------------------------------|----------------------------|---------------------------------|--------------------------------|----------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----|------------------------------|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|--|
| CENTRAL | TI-B1 | CA | 36 | 36.5 | Alluvium Clayey Sand | coarse | 21.0 | 19.9 | 76% | 36.3 / 33.2 | 85.2 / 88.0 | 97.3 | | 2.73 | | | | 0.0 | 62.5 | 37.5 | 32.8 | 4.7 | 1.7E-06 | 32 | 0.059 | | | |
| CENTRAL | TI-B1 | ST | 45 | 46 | Alluvium Clayey Sand | coarse | 22 | 21.2 | | | | 106.0 | | | | | | | | | | | | 0.058 | | | 34.4 | |
| CENTRAL | TI-B10 | CA | 91 | 91.5 | Alluvium Clayey Sand | coarse | 18.6 | | | | | 105.6 | | 2.66 | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA | 41 | 41.5 | Alluvium Clayey Sand | coarse | 11.4 | 10.1 | | | | 87.1 | 88.1 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA | 66 | 66.5 | Alluvium Clayey Sand | coarse | 12.7 | 11.8 | | | | 100.7 | 101.5 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | CA | 81 | 81.5 | Alluvium Clayey Sand with Gravel | coarse | 11.0 | | | | | 107.6 | | 2.76 | | | | 12.9 | 65.6 | 21.5 | 9.9 | 11.6 | | | | | | |
| CENTRAL | TI-B10 | CA | 46 | 46.5 | Alluvium Silty Sand | coarse | 9.9 | | | | | 95.4 | | 2.74 | | | | 0.0 | 65.8 | 34.2 | 23.4 | 10.8 | | | | | | |
| CENTRAL | TI-B10 | ST | 55 | 56 | Alluvium Silty Sand | coarse | 14.1 | | | 25.7 / 24.8 | 98.0 / 99.9 | 100.8 | | | | | | | | | | | 2.4E-05 | 72 | 0.139 | | | |
| CENTRAL | TI-B10 | CA | 71 | 71.5 | Alluvium Silty Sand | coarse | 18.1 | | | | | 100.8 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | ST | 56 | 57 | Alluvium Silty Sand | coarse | 16.2 | | | 31.0 / 30.8 | 90.6 / 92.8 | 77.9 | | 2.64 | | NP | | 0.0 | 60.4 | 39.6 | 31.9 | 7.7 | 5.6E-04 | 72 | 0.129 | | | |
| CENTRAL | TI-B11 | CA | 66 | 66.5 | Alluvium Silty Sand | coarse | 14.2 | | | | | 96.2 | | | | | | | | | | | | | | | | |
| BORROW PIT 1 | TI-B8 | CA | 56 | 56.5 | Alluvium Silty Sand | coarse | 12.6 | | | | | 97.6 | | 2.70 | | NP | | 0.0 | 57.0 | 43.0 | 30.9 | 12.1 | | | | | | |
| CENTRAL | TI-B15 | CA (top) | 31 | 31.5 | Alluvium Silty Sand | coarse | 22.3 | 21.3 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA (bottom) | 31 | 31.5 | Alluvium Silty Sand | coarse | 17.1 | | | | | 101.8 | | 2.71 | | NP | | 6.2 | 51.9 | 41.9 | 25.9 | 16.0 | | | | | | |
| NORTH CELL | TI-B2 | CA | 15 | 15.5 | Alluvium Silty Sand | coarse | 6.9 | | | | | 90.4 | | 2.68 | | | | | | | | | | | | | | |
| NORTH CELL | TI-B2 | CA | 21 | 21.5 | Alluvium Silty Sand | coarse | 7.0 | | | | | 91.4 | | 2.74 | | | | 0.0 | 82.9 | 17.1 | 11.5 | 5.6 | | | | | | |
| CENTRAL | TI-B10 | CA | 66 | 66.5 | Alluvium Silty Sand / Sandy Silt | coarse | SM/ML | 13.8 | | | | 94.5 | | | | NP | | 0.0 | 50.1 | 49.9 | 33.4 | 16.5 | | | | | | |
| NORTH CELL | TI-B23 | ST | 26 | 27 | Alluvium Lean Clay | fine | CL | 21.6 | | | | 101.7 | | 2.73 | | 49 | 18 | 31 | 0.0 | 8.8 | 91.2 | 43.8 | 47.5 | | 0.046 | | | |
| DAM | TI-B3 | ST | 56 | 57 | Alluvium Lean Clay | fine | CL | 22.1 | 21.1 | | | 105.3 | 106.2 | 2.72 | | 43 | 14 | 29 | 0.0 | 11.7 | 88.3 | 48.4 | 39.9 | | | -1.5 (7,204) | 22.2, 494 | |
| CENTRAL | TI-B1 | CA | 41 | 41.5 | Alluvium Lean Clay with Sand | fine | CL | 26.7 | | | | 98.6 | | | | 31 | 15 | 16 | 0.0 | 18.2 | 81.8 | 54.7 | 27.1 | 1.2E-07 | 35 | | | |
| BORROW PIT 1 | TI-B8 | CA | 46 | 46.5 | Alluvium Lean Clay with Sand | fine | CL | 21.9 | | | | 95.2 | | 2.72 | | 30 | 16 | 14 | 0.0 | 27.9 | 72.1 | 55.6 | 16.5 | | | | | |
| NORTH CELL | TI-B2 | CA | 26 | 26.5 | Alluvium Lean Clay with Sand | fine | CL | 23.5 | | | | 93.2 | | | | 34 | 16 | 18 | 0.0 | 20.9 | 79.1 | 51.5 | 27.6 | | | | | |
| CENTRAL | TI-B11 | CA | 61 | 61.5 | Alluvium Sandy Clay | fine | | 16.0 | | | | 95.4 | | | | | | 0.0 | 38.7 | 61.3 | 44.1 | 17.2 | | | | | | |
| NORTH CELL | TI-B23 | ST | 17.25 | 17.5 | Alluvium Sandy Clay | fine | | 22.5 | | | | 101.9 | | 2.73 | | | | 0.0 | 31.1 | 68.9 | 46.5 | 22.5 | | | | | | |
| CENTRAL | TI-B15 | CA (top) | 46 | 46.5 | Alluvium Sandy Silt | fine | | 25.8 | 24.0 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CA (bottom) | 46 | 46.5 | Alluvium Sandy Silt | fine | ML | 17.3 | | | | 99.3 | | 2.81 | | NP | | 0.0 | 37.0 | 63.0 | 55.7 | 7.3 | | | | | | |
| CENTRAL | TI-B15 | CA | 56 | 56.5 | Alluvium Silty Clay | fine | | 11.7 | 10.5 | | | 104.2 | 105.3 | | | | | | | | | | | | | | | |
| DAM | TI-B3 | CA | 61 | 61.5 | Alluvium Silty Clay | fine | | 25.8 | | | | 99.0 | | | | | | 0.0 | 22.0 | 78.0 | 54.9 | 23.1 | | | | | | |
| CENTRAL | TI-B10 | ST (top) | 10 | 11 | Coarse Tailings | | | 9.7 | 9.1 | | | 110 | 110.5 | 2.63 | 2.65 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | ST (bottom) | 10 | 11 | Coarse Tailings | Clayey Sand | | 9.0 | | | 20.7 / 21.5 | 102.6 / 101.2 | 96.8 | | | | | 0.2 | 71.9 | 27.9 | 16.6 | 11.3 | 4.3E-04 | 34 | 0.094 | | | |
| CENTRAL | TI-B10 | CC-AC ⁽⁴⁾ (top) | 12.5 | 14 | Coarse Tailings | | | 6.7 | 6.3 | | | | | 2.61 | 2.64 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CC-AC ⁽⁴⁾ (bot) | 12.5 | 14 | Coarse Tailings | Clayey Sand | | 7.5 | | | 31.3 / 31.4 | 85.0 / 85.0 | 99.1 | | | | | 0.7 | 71.5 | 27.8 | 18.9 | 8.9 | 6.7E-05 | 36 | | | | |
| CENTRAL | TI-B10 | CA | 15 | 15.5 | Coarse Tailings | | | 9.3 | | | | 103.0 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CA | 16 | 16.5 | Coarse Tailings | Silty Sand | SM | 6.5 | | | | 100.0 | | 2.65 | | NP | | 2.4 | 82.3 | 15.3 | 10.2 | 5.1 | | | | | | |
| CENTRAL | TI-B10 | ST | 32 | 32.5 | Coarse Tailings | | SM | 15.4 | | | | 100.1 | | 2.67 | | NP | | 0.0 | 83.1 | 16.9 | 12.6 | 4.3 | | | | | | |
| CENTRAL | TI-B1 | CA | 20.5 | 21 | Coarse Tailings | | | 6.1 | 5.7 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B1 | CA | 21 | 21.5 | Coarse Tailings | Poorly Graded Sand w/ Clay | | 7.5 | | | 21.9 / 19.8 | 96.5 / 99.6 | 105.5 | | | | | 0.0 | 90.7 | 9.3 | 5.5 | 3.8 | 3.7E-04 | 18 | 0.024 | | | |
| CENTRAL | TI-B1 | ST | 27 | 27.5 | Coarse Tailings | | SP | 4.0 | | | | 97.6 | | 2.67 | | NP | | 0.0 | 92.7 | 7.3 | 5.2 | 2.1 | 2.9E-03 | 14 | | | 34.9 | |
| CENTRAL | TI-B1 | CA | 30 | 30.5 | Coarse Tailings | | | 13.9 | 13.5 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B1 | CA | 30.5 | 31 | Coarse Tailings | | | 14.6 | | | 29.6 / 33.8 | 84.2 / 83.6 | 91.6 | | | | | | | | | | 3.0E-07 | 25 | 0.092 | | | |
| CENTRAL | TI-B1 | CA (top) | 31 | 31.5 | Coarse Tailings | | | 0.8 | 0.4 | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CA | 25 | 25.5 | Coarse Tailings | | | 9.0 | 8.4 | | | 103.7 | 104.2 | 2.72 | 2.72 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CA ⁽⁵⁾ | 25.5 | 26 | Coarse Tailings | | | 6.2 | | | 25.7 | 94.6 | 99.6 | | | | | 0.0 | 87.9 | 12.7 | 7.9 | 4.8 | 3.6E-04 | 46 | | | | |
| CENTRAL | TI-B8 | CA ⁽⁵⁾ | 26 | 26.5 | Coarse Tailings | Silty Sand | SM | 16.8 | | | 27.0 | 94.8 | 91.7 | | | NP | | 0.0 | 76.0 | 24.0 | 19.0 | 5.0 | | | | | | |
| CENTRAL | TI-B8 | ST (top) | 35 | 36 | Coarse Tailings | | | 14.3 | 13.6 | | | 90.9 | 91.4 | 2.66 | 2.67 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST (bottom) | 35 | 36 | Coarse Tailings | | | 16.5 | | | 31.2 / 39.3 | 89.3 / 82.3 | 89.6 | | | | | | | | | | 1.6E-05 | 43 | | | | |
| CENTRAL | TI-B15 | CA | 6 | 6.5 | Coarse Tailings | | | 5.4 | | | | 101.1 | | | | | | 0.0 | 87.5 | 12.5 | 9.8 | 2.7 | | | | | | |
| CENTRAL | TI-B15 | CA | 11 | 11.5 | Coarse Tailings | | | 6.8 | | | | 93.8 | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B15 | CC-AC | 13.5 | 14 | Coarse Tailings | | SM | 19.0 | 18.4 | | | | | 2.68 | | NP | | 0.0 | 69.6 | 30.4 | 22.6 | 7.8 | | | | | | |
| CENTRAL | TI-B15 | ST | 15.5 | 16 | Coarse Tailings | | SM | 14.2 | | | | 90.4 | | 2.66 | | NP | | 0.0 | 54.9 | 15.1 | 10.1 | 5.0 | 8.3E-04 | 38 | 0.126 | | | |
| CENTRAL | TI-B15 | CA | 21 | 21.5 | Coarse Tailings | | SM | 12.7 | | | | 99.8 | | 2.68 | | NP | | 0.0 | 80.6 | 19.4 | 13.3 | 6.1 | | | | | | |
| CENTRAL | TI-B15 | CC-AC | 28.5 | 29.5 | Coarse Tailings | | SM | 19.3 | | | | | | 2.66 | | NP | | 0.0 | 65.4 | 34.6 | 24.4 | 10.2 | | | | | | |
| NORTH CELL | TI-B23 | ST | 15.5 | 15.75 | Coarse Tailings | | | 20.7 | 19.6 | | | 87.7 | | 2.77 | | | | 0.0 | 62.8 | 37.2 | 34.1 | 3.1 | | | | | | |
| CENTRAL | TI-B10 | ST | 21.5 | 22.5 | Coarse/Fine Tailings | | CL | 28.1 | 26.7 | | | 91.9 | 92.9 | | | 43 | 19 | 24 | 0.0 | 43.0 | 57.0 | 51.4 | 5.6 | | 0.111 | | | |
| CENTRAL | TI-B1 | CC-AC | 32 | 33 | Coarse/Fine Tailings | | CL | 29.3 | 27.8 | | | | | | | 33 | 16 | 17 | 0.0 | 46.7 | 53.3 | 37.4 | 15.9 | | | | | |
| CENTRAL | TI-B10 | CA | 36 | 36.5 | Coarse/Fine Tailings | Clayey Sand / Sandy Clay | SC/CL | 33.9 | 32.2 | 94% | | 86.7 | 87.8 | 2.68 | 2.72 | 36 | 16 | 20 | 0.0 | 50.6 | 49.4 | 31.1 | 18.3 | | | | | |
| CENTRAL | TI-B8 | ST (bottom) | 41 | 42 | Coarse/Fine Tailings | Clayey Sand / Sandy Clay | SC/CL | 35.6 | 34.3 | | 33.1 / 31.6 | 88.7 / 90.7 | 82.8 | 83.6 | | 35 | 16 | 19 | 0.0 | 51.2 | 48.8 | 40.7 | 8.1 | 1.3E-07 | 53 | 0.262 | | |
| CENTRAL | TI-B8 | CC-AC (top) | 43.5 | 44.5 | Coarse/Fine Tailings | | | 31.2 | 29.3 | | | 91.0 | 92.3 | | | | | | | | | | | | | | | |
| DAM | TI-B3 | ST (top) | 35 | 36 | Dam Clayey Sand (dam) | | | 10.5 | 10.2 | | | | | | | | | | | | | | | | | | | |
| DAM | TI-B3 | ST (bottom) | 35 | 36 | Dam Clayey Sand (dam) | | SC | 14.7 | | | | 102.2 | | 2.67 | | 23 | 14 | 9 | 2.1 | 50.2 | 47.7 | 30.9 | 16.8 | | | -0.7 (4,608) | 33.7, 135 | |
| DAM | TI-B3 | ST | 21 | 22 | Dam Sandy Clay (dam) | | CL | 16.0 | | | | 111.1 | | | | 30 | 12 | 18 | 0.0 | 32.8 | 67.2 | 41.7 | 25.5 | | | -0.03 (2,709) | 32.2, 195 | |
| DAM | TI-B3 | CA | 26 | 26.5 | Dam Sandy Clay (dam) | | | 12.0 | | | | 106.8 | | | | 25 | 13 | 12 | | | | | | | | | | |
| DAM | TI-B3 | CA | 31 | 31.5 | Dam Sandy Clay (dam) | | | 16.1 | | | | 108.4 | | | | | | | | | | | | | | | | |
| DAM | TI-B3 | CA | 11 | 11.5 | Dam Silty Sand (dam) | | | 5.1 | | | | 108.4 | | 2.64 | | | | 5.4 | 74.7 | 19.9 | 13.5 | 6.4 | | | | | | |
| DAM | TI-B3 | CA | 16 | 16.5 | Dam Silty Sand (dam) | | | 4.7 | | | | 105.3 | | | | | | | | | | | | | | -2.8 (2,236) | | |
| DAM | TI-B3 | CA | 41 | 41.5 | Dam Sandy Clay (dam) | | | 21.5 | | | | 90.6 | | | | | | 0.0 | 33.8 | 66.2 | 41.7 | 24.5 | | </ | | | | |

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment

(continued)

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | saturation (%) | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | Atterberg limits (%) | | | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) |
|--------------|--------|----------------------------|-----------------------------|-------|-------------------------------------|--------------------|---------------------|---------------------------------|--------------------------------|----------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----------------------|----|----|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|
| CENTRAL | TI-B10 | CA | 26 | 26.5 | Fine Tailings | Fat Clay | CH | 60.4 | 57.4 | | | | 63.1 | 64.3 | 2.71 | 2.80 | 74 | 27 | 47 | 0.0 | 10.0 | 90.0 | 82.6 | 7.4 | | | | | |
| CENTRAL | TI-B10 | ST | 30.3 | 30.7 | Fine Tailings | | CH | 47.7 | 45.3 | 92% | | | 72.2 | 73.4 | 2.71 | 2.78 | 57 | 22 | 35 | 0.0 | 24.3 | 75.7 | 68.4 | 7.3 | | | | | |
| CENTRAL | TI-B10 | ST (top) | 40 | 41 | Fine Tailings | Fat Clay with Sand | | 47.3 | 45.7 | | | | 70.5 | 73.7 | 2.54 | 2.56 | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | ST (bottom) | 40 | 41 | Fine Tailings | Fat Clay with Sand | CH | 49.7 | 47.2 | | 47.7 / 55.7 | 75.3 / 67.9 | 73.3 | 74.5 | | | 61 | 21 | 40 | 0.0 | 20.7 | 79.3 | 46.5 | 32.9 | 2.9E-08 | 58 | 0.315 | | |
| CENTRAL | TI-B11 | ST | 51.5 | 52.5 | Fine Tailings | | CH | 63.0 | 59.9 | 95% | | | 62.5 | 63.7 | 2.75 | 2.84 | 91 | 30 | 61 | 0.0 | 2.7 | 97.3 | 90 | 7.3 | 3.1E-08 | 67 | 0.482 | | |
| CENTRAL | TI-B1 | CA (bottom) | 31 | 31.5 | Fine Tailings | | CL | | 41.6 | 94% | | | | 76.5 | 2.68 | 2.69 | 44 | 17 | 27 | 0.0 | 30.9 | 69.1 | 54.6 | 14.5 | | | | | 33.3 |
| CENTRAL | TI-B10 | CA | 35 | 35.5 | Fine Tailings | | | 50.2 | 47.7 | | | | 71.3 | 72.5 | | | | | | | | | | | | | | | |
| CENTRAL | TI-B10 | CA | 35.5 | 36 | Fine Tailings | | | 54.2 | 51.4 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B11 | CA | 45.5 | 46 | Fine Tailings | | | 117.2 | 88.7 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST | 30 | 31 | Fine Tailings | | CH | 65.1 | 61.8 | | | | 61.5 | 62.7 | | | 74 | 25 | 49 | 0.0 | 9.2 | 90.8 | 81.2 | 9.6 | | | 0.426 | | |
| CENTRAL | TI-B8 | ST | 31 | 31.5 | Fine Tailings | | | 44.3 | 41.4 | | | | | | | | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | ST (top) | 41 | 42 | Fine Tailings | | | 41.8 | 39.7 | 100% | | | 79.2 | 80.4 | 2.60 | 2.63 | | | | | | | | | | | | | |
| CENTRAL | TI-B8 | CC-AC ⁽⁶⁾ (bot) | 43.5 | 44.5 | Fine Tailings | | | 45.6 | 43.3 | 96% | 47.9 / 49.0 | 74.4 / 73.6 | 73.6 | 74.8 | | | | | | 0.0 | 14.5 | 85.5 | 74.7 | 10.8 | 3.0E-08 | 61 | | | |
| BORROW PIT 1 | TI-B8 | CC-AC | 44.5 | 45 | Fine Tailings | | | | | | | | | | 2.59 | 2.60 | | | | | | | | | | | | | |
| NORTH CELL | TI-B2 | CC-AC | 13.5 | 14.5 | Fine Tailings | Sandy Clay | | 41.7 | 39.6 | | | | | | | | | | | 0.0 | 23.1 | 76.9 | 49.2 | 27.7 | | | | | |
| CENTRAL | TI-B10 | CC | 106.9 | 107.3 | Sandstone | | | 14.2 | | | | | 109.1 | | | | | | | | | | | 1.4E-07 | 115 | | | | |
| CENTRAL | TI-B11 | CA | 100 | 100.2 | Sandstone | | | 21.1 | | | | | 103.9 | | | | | | | | | | | 1.3E-05 | 112 | | | | |
| NORTH CELL | TI-B23 | CA | 45.2 | 45.7 | Sandstone | | | 13.8 | | | | | 108.7 | | | | | | | | | | | 2.4E-07 | 43 | | | | |
| NORTH CELL | TI-B2 | BULK | 38.4 | 38.7 | Sandstone | | | 13.5 | | | | | X | | | | | | | | | | | | | | | | |
| BORROW PIT 1 | TI-B8 | BULK | 63.5 | 64 | Shale | | | X | | | | | X | | | | | | | | | | | X | X | | | | |
| NORTH CELL | TI-B23 | CA ⁽⁸⁾ | 65.5 | 66 | Shale | | | 10.2 | | | | | 103.0 | | | | | | | | | | | 9.7E-08 | 62 | | | | |

Notes: 1. Material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available.
2. SWCC tests conducted with pairs of specimens for each test.
3. Flexible wall permeameter tests conducted at confining pressures representing confining stresses for the proposed design fill. Confining stresses were estimated as the existing overburden stress on the specimens (depth times total unit weight of material above) plus the maximum anticipated fill height for the location times the estimated unit weight of fill.
4. Specimen remolded to the in-situ water content and density of the Shelby tube sample from 10-12.5 for the SWCC.
5. Remolded SWCC and permeability tests conducted on a 50-50 mixture of the materials from these two specimens, remolded to the average measured density of the two CA samples.
6. SWCC specimen remolded to the in-situ water content and density of the Shelby tube sample from 41-42 feet.
7. Compression indices estimated using the maximum anticipated loading during fill placement and the range of loading during testing. Initial void ratios are calculated using the average specific gravity for all samples of 2.70.
8. Shale sample had multiple horizontal fractures and was likely disturbed during sampling.

9. Sample Types: CC = continuous core, CC-AC = continuous core in acrylic liner, top/bottom indicates the specimen was taken from the top or bottom of the sample interval
10. Values in italics were calculated based on the relationship $(WC60=0.951*(WC110)-.0611)$ between the water content results measured for 15 tailings samples at the two oven temperatures.
11. Shaded cells are alluvium.
12. Consolidated undrained (CU) triaxial shear, staged loading of one specimen with pore pressure measurements

ST = 3" diam. Shelby tube, CA = California sample
R = remolded, nc = Cc not calculated, because fill will not be placed in this location
X = testing not possible due to sample disturbance
LL = liquid limit, PL = plastic limit, PI = plasticity index

Table 3-5 Summary of Geotechnical Laboratory Data - Borrow Areas

| Area | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (ft) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass, %) | Dry Density (pcf) | Porosity | Specific Gravity | Standard Proctor (max. dd@opt. w.c.), (pcf @ %) | Atterberg Limits (%) ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | Pinhole Dispersion ^(5,6) | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽⁷⁾ | | | SWCC: -5 bar Water Content (by mass, %) ⁽⁸⁾ | | SWCC: Saturated Water Content (by mass, %) ⁽⁸⁾ | |
|--------------|------------|----------------------------|----------------------------|--------------------|-------------------------------------|---------------------|------------------------------------|----------------------------|-------------------|----------|------------------|---|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|-------------------------------------|---|---------|---------|--|-------------|---|--|
| | | | | | | | | | | | | | LL | PL | PI | | | | | | | 80% | 85% | 90% | | | | |
| West Borrow | WB-B1-01A | CA | 3.0 | 3.5 | Clayey Sand | | | 3.8 | 88.8 | 46.7 | 2.67 | | | | | | | | | | | | | | | | | |
| | WB-B1-03A | CA | 11.0 | 11.5 | Clayey Sand | SC | Sandy Loam | 6.4 | 111.0 | 33.3 | 2.67 | | 28 | 18 | 10 | 2.8 | 48.6 | 48.6 | 32.8 | 15.8 | | | | | | | | |
| | WB-B1-06 | Bulk | 5.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.64 | 112.5 @ 13.7 | 26 | 20 | 6 | 0.8 | 52.3 | 46.9 | 31.0 | 15.9 | ND3 | 7.2E-04 | 5.8E-04 | 2.1E-04 | 6.6 / 6.2 | 31.7 / 32.4 | | |
| | WB-B2-02A | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 5.6 | 87.1 | 47.8 | 2.67 | | | | | 8.6 | 53.5 | 37.9 | 23.8 | 14.1 | | | | | | | | |
| | WB-B2-05 | Bulk | 10.0 | 20.0 | Clayey Sand | SC | Sandy Loam | | | | | | 26 | 17 | 9 | 9.9 | 46.3 | 43.8 | 27.7 | 16.1 | ND3 | 8.5E-05 | 1.2E-04 | 6.4E-05 | 6.4 / 6.7 | 30.9 / 33.7 | | |
| | WB-B5-001B | CA | 3.0 | 3.5 | Clayey Sand | | | 3.7 | 92.5 | 44.3 | 2.66 | | | | | | | | | | | | | | | | | |
| | WB-B5-002A | CA | 6.0 | 6.5 | Silty, Clayey Sand | SC-SM | Sandy Loam | 5.1 | 86.9 | 47.7 | 2.66 | | 24 | 17 | 7 | 0.0 | 56.3 | 43.7 | 27.8 | 15.9 | | | | | | | | |
| WB-B5-005 | Bulk | 0.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | | 117.3 @ 12.7 | | | | | 0.0 | 61.6 | 38.4 | 22.8 | 15.6 | | | | | | | | |
| East Borrow | EB-B2-001A | CA | 3.0 | 3.5 | Weath. Sandstone | | | 5.8 | 107.1 | 35.8 | 2.67 | | | | | | | | | | | | | | | | | |
| | EB-B3-003B | CA | 10.5 | 11.0 | Sandy Lean Clay | CL | Sandy Loam | 6.0 | 83.1 | 50.7 | 2.70 | | 26 | 15 | 11 | 0.0 | 46.3 | 53.7 | 34.9 | 18.8 | | | | | | | | |
| | EB-B4-02A | CA | 6.0 | 6.5 | Sandy Lean Clay | CL | Sandy Loam | 5.4 | 80.7 | 51.2 | 2.65 | | | | | 0.0 | 48.5 | 51.5 | 33.9 | 17.6 | | | | | | | | |
| | EB-B4-06 | Bulk | 10.0 | 20.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.67 | 117.1 @ 12.9 | 23 | 17 | 6 | 0.0 | 50.5 | 49.5 | 32.0 | 17.5 | ND3 | 8.7E-04 | 9.0E-04 | 4.4E-04 | 4.6 / 4.2 | 30.8 / 29.8 | | |
| | EB-B5-02B | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 6.7 | 93.8 | 44.4 | 2.71 | | 27 | 15 | 12 | 8.8 | 45.7 | 45.5 | 28.8 | 16.7 | | | | | | | | |
| | EB-B6-01B | CA | 3.0 | 3.5 | Sandy Clay | | | 7.6 | 91.2 | 46.1 | 2.71 | | | | | | | | | | | | | | | | | |
| | EB-B6-03 | Bulk | 0.0 | 10.0 | Lean Clay with Sand | CL | Clay Loam | | | | | 114.8 @ 14.1 | | | | | 0.0 | 26.6 | 73.4 | 44.3 | 29.1 | ND3 | 2.3E-04 | 3.6E-05 | 2.9E-05 | 9.4 / 9.3 | 32.8 / 32.2 | |
| EB-B6-04A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 8.6 | 95.2 | 43.3 | 2.69 | | | 31 | 13 | 18 | 0.0 | 31.1 | 68.9 | 43.8 | 25.1 | | | | | | | | |
| South Borrow | SB-B1-01A | CA | 3.5 | 4.0 | Sandy Lean Clay | CL | Sandy Loam | 7.1 | 91.4 | 49.3 | 2.89 | | | | | 0.0 | 43.1 | 56.9 | 39.2 | 17.7 | | | | | | | | |
| | SB-B1-03A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 6.6 | 82.6 | 50.7 | 2.69 | | 31 | 15 | 16 | 0.0 | 46.7 | 53.3 | 32.9 | 20.4 | | | | | | | | |
| | SB-B1-04 | Bulk | 0.0 | 25.0 | Sandy Lean Clay | CL | Sandy Clay Loam | | | | 2.70 | 115.5 @ 14.2 | 33 | 14 | 19 | 0.0 | 42.6 | 57.4 | 30.7 | 26.7 | ND1 | 2.3E-04 | 5.7E-05 | 1.4E-04 | 6.4 / 5.9 | 31.9 / 30.3 | | |
| | SB-B2-02B | CA | 5.5 | 6.0 | Sandy Lean Clay | CL | Loam | 7.7 | 80.1 | 52.6 | 2.70 | | 36 | 15 | 21 | 0.0 | 29.8 | 70.2 | 45.4 | 24.8 | | | | | | | | |
| | SB-B3-02A | CA | 6.0 | 6.5 | Lean Clay with Sand | CL | Clay Loam | 10.2 | 84.3 | 49.7 | 2.69 | | 40 | 17 | 23 | 0.0 | 21.6 | 78.4 | 46.2 | 32.2 | | | | | | | | |
| | SB-B4-01 | Bulk | 0.0 | 15.0 | Sandy Lean Clay | CL | Sandy Clay Loam | 7.1 | | | 2.67 | 114.1 @ 14.4 | 33 | 15 | 18 | 0.8 | 39.6 | 59.6 | 35.7 | 23.9 | ND3 | 3.4E-04 | 2.0E-04 | 7.4E-05 | 9.1 / 8.6 | 29.6 / 33.5 | | |
| North Borrow | NB-B1-03B | CA | 10.5 | 11.0 | Silty Sand | SM | Sandy Loam | 5.4 | 84.4 | 49.5 | 2.68 | | 25 | 22 | 3 | 0.0 | 55.6 | 44.4 | 30.3 | 14.1 | | | | | | | | |
| | NB-B2-01B | CA | 3.0 | 3.5 | Silty Sand | SM | Sandy Loam | 4.9 | 81.9 | 50.3 | 2.64 | | 27 | 23 | 4 | 0.0 | 51.2 | 48.8 | 33.9 | 15.0 | | | | | | | | |
| | NB-B2-04 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | | | | | 113.9 @ 14.5 | 26 | 19 | 7 | 0.0 | 49.0 | 51.0 | 32.5 | 18.5 | ND3 | 4.0E-04 | 2.7E-04 | 7.5E-05 | 4.9 / 4.7 | 29.5 / 29.9 | | |
| Dilco Hill | DH-B1-01B | CA | 3.0 | 3.5 | Silty Sand | | | 3.5 | 88.8 | 46.6 | 2.66 | | | | | | | | | | | | | | | | | |
| | DH-B1-03 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | 5.4 | | | 2.67 | 117.5 @ 13.8 | 25 | 19 | 6 | 2.0 | 47.4 | 50.6 | 35.0 | 15.6 | ND4 | 6.3E-04 | 7.1E-04 | 2.5E-04 | 4.2 / 4.1 | 39.6 / 35.0 | | |
| | DH-B1-10 | Bulk | 35.0 | 45.0 | Lean Clay with Sand | CL | Loam | 10.3 | | | 2.38 | | | | | 1.5 | 20.9 | 77.6 | 60.9 | 16.7 | ND3 | 1.6E-04 | 2.5E-05 | 3.2E-06 | 5.8 / 6.0 | 25.7 / 24.5 | | |
| | DH-B2-03 | CA | 15.0 | 15.5 | Silty Clay with Sand | CL-ML | Sandy Loam | 10.5 | 96.7 | 39.2 | 2.55 | | 29 | 24 | 5 | 0.0 | 27.7 | 72.3 | 66.9 | 5.4 | | | | | | | | |
| | DH-B3-05 | Bulk | 20.0 | 30.0 | Sandy Lean Clay | CL | Loam | 7.3 | | | 2.66 | 116.3 @ 13.0 | 29 | 18 | 11 | 2.5 | 34.6 | 62.9 | 45.5 | 17.4 | | | | | | | | |

Notes: 1. Sample Types: CA = California sample, Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. LL = liquid limit, PL = plastic limit, PI = plasticity index
5. With the exception of DH-B1-03, which was tested at a density based on the natural in-situ density measured from the CA samples, specimens were remolded to approximately 85% of standard Proctor density and between the estimated natural and optimum water contents for the soil.
6. ND1 = nondispersive clay with very slight to no colloidal erosion under 15-inch or 40-inch head; ND4, ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head (ASTM test method A)
7. Specimens remolded to approximately 80%, 85%, and 90% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil.
8. Specimens remolded to approximately 85% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil. SWCC tests performed with pairs of speciments for each test.

Table 3-6 Summary of Geotechnical Laboratory Data - Site Stockpiles

| Area | Sample | Sample Type ⁽¹⁾ | Material Description | USCS ⁽²⁾ | Specific Gravity | Atterberg Limits ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | L.A. Abrasion ⁽⁵⁾ (% loss) | Sodium Sulfate Soundness ⁽⁶⁾ (% loss) | Absorption ⁽⁷⁾ (%) | Unconfined Compressive Strength ⁽⁸⁾ (psi) | Splitting Tensile Strength ⁽⁸⁾ (psi) |
|------------|--------------|----------------------------|------------------------------|---------------------|---------------------|---------------------------------|----|----|---------------|-------------|---------------------------------|---------------------------------------|--|-------------------------------|--|---|
| | | | | | | LL | PL | PI | | | | | | | | |
| Stockpiles | Topsoil-01 | Bulk | Sandy Clay | CL | 2.68 | 33 | 10 | 23 | 2.6 | 32.4 | 65.0 | | | | | |
| | Topsoil-02 | Bulk | Sandy Clay | CL | 2.71 | 39 | 12 | 27 | 0.5 | 26.8 | 72.7 | | | | | |
| | TI-SP1-01 | Bulk | Crusher Fines | | | | | | 1.9 | 80.8 | 17.3 | | | | | |
| | TI-SP2-01A | Bulk | Erosion Protection Gravel | | 2.78 ⁽³⁾ | | | | 93.0 | 6.3 | 0.7 | 5.7 | 8.26 | 1.868 | | |
| | TI-SP2-01C | Bulk | Erosion Protection Gravel | | | | | | 83.3 | 4.9 | 11.8 | | | | | |
| | TI-SP3-01A | Bulk | Road Base (gravel with sand) | | | | | | 67.4 | 24.6 | 8.0 | | | | | |
| | TI-SP4-01A | Bulk | Erosion Protection Gravel | | 2.75 ⁽³⁾ | | | | 98.0 | 1.2 | 0.8 | 6.1 | 10.47 | 2.091 | | |
| | TI-SP6 (56A) | Bulk | 9-inch riprap | | | | | | | | | | | | 20,780 and 23,630 | 1,320 and 1,400 |
| | TI-SP6 (56B) | Bulk | 9-inch riprap | | | | | | | | | | | | 19,100 and 14,440 | 1,530 and 1,720 |

Notes: 1. Bulk = bucket/grab sample

2. USCS = Unified Soil Classification System, material descriptions are based on field observations, and refined with laboratory data, if available.

USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay

3. Average of three bulk saturated surface dry (SSD) specific gravity results for the rock samples (ASTM C127)

4. LL = liquid limit, PL = plastic limit, PI = plasticity index

5. L.A. Abrasion results are percent loss, by mass, for 100 revolutions.

6. Weighted percentage loss for 0.75 to 1.5-inch size range

7. Average of three absorption results

8. Specimens were collected from the 9-inch stockpile and cored for strength testing.

Table 3-6 Geotechnical Test Results

| Sample ID ¹ | Sample Location | Sample Type | Sample Depth Interval | | Gravimetric Water content | Dry Density | Specific gravity | Standard Proctor | |
|------------------------|-----------------|-----------------|-----------------------|-----------------|---------------------------|----------------|----------------------|-------------------------------------|---------------------------|
| | | Units: | top (ft bgs) | bottom (ft bgs) | (% by mass) | (pcf) | (g/cm ³) | max. dry density (pcf) ³ | optimum water content (%) |
| NECR1-CC01 | NECR-1 | Bulk | 10 | 20 | | | 2.68 | 120.7 | 11.9 |
| NECR1-CC17 | | CA ² | 5.5 | 6 | 4.9 | 92.3 | | | |
| NECR1-CC17 | | CA | 10.5 | 11 | 6.2 | 96.5 | | | |
| NECR1-CC17 | | CA | 15.5 | 16 | 2 | 106.7 | | | |
| NECR1-CC17 | | CA | 20.5 | 21 | 19.1 | 95.8 | | | |
| NECR1-CC17 | | Bulk | 0 | 10 | | | | 120.3 | 11.3 |
| NECR1-CC17 | NECR-2 | Bulk | 10 | 20 | | | | 125.1 | 10 |
| NECR2-CC05 | | Bulk | 0 | 10 | | | | 118.8 | 11.9 |
| NECR2-CC07 | | Bulk | 0 | 10 | | | 2.71 | 117.8 | 11.6 |
| NECR2-CC05 | | CA | 2.5 | 3 | 8.1 | 93.7 | | | |
| NECR2-CC05 | | CA | 5 | 5.5 | 10 | D ³ | | | |
| NECR2-CC06 | | CA | 3.5 | 4 | 4.7 | 101.1 | | | |
| NECR2-CC07 | | CA | 6 | 6.5 | 2.7 | 101 | | | |
| NECR2-CC07 | | CA | 5.5 | 6 | 4.5 | 101.3 | | | |
| NECR2-CC07 | | CA | 10 | 10.5 | 4.1 | 97.1 | | | |
| NECR2-CC01 | | CA | 5.5 | 6 | 7.4 | 99.1 | | | |
| NECR2-CC06 | NECR-2 Drainage | CA | 3 | 3.5 | 5 | 103.4 | | | |
| N2D-CC01 | | Bulk | 0 | 10 | | | | 115.6 | 13.4 |
| N2D-CC01 | | CA | 3.5 | 4 | 8.6 | 91.2 | | | |
| N2D-CC01 | | CA | 6 | 6.5 | 4.7 | 87.2 | | | |
| N2D-CC01 | NEMSA | CA | 11 | 11.5 | 4 | 91.8 | | | |
| NMSA-CC02 | | CA | 3 | 3.5 | 8.1 | 110.6 | | | |
| NMSA-CC02 | | CA | 6 | 6.5 | 20 | 97.5 | | | |
| NMSA-CC02 | | CA | 10.5 | 11 | 15 | 86.6 | | | |
| NMSA-CC04 | Pond 2 | Bulk | 0 | 15 | | | 2.66 | 125.2 | 9.8 |
| P2-CC04 | | Bulk | 0 | 3 | | | 2.66 | 102.0 | 20.6 |
| P3-CC07 | Pond 3 | Bulk | 0 | 5 | | | 2.63 | 109.7 | 13.7 |
| SF2-CC01 | Sandfill 2 | Bulk | 0 | 10 | | | 2.65 | 121.5 | 10.5 |
| SF3-CC01 | Sandfill 3 | Bulk | 0 | 10 | | | 2.68 | 121.7 | 11.1 |
| SF3-CC01 | | CA | 3.5 | 4 | 17 | 99.3 | | | |
| SF3-CC01 | | CA | 6 | 6.5 | 10.5 | 96.4 | | | |
| SF3-CC01 | | CA | 11 | 11.5 | 8.2 | 83.5 | | | |
| SP-CC13 | Sediment Pad | CA | 5.5 | 6 | 10.2 | 101.4 | | | |
| SP-CC13 | | CA | 11 | 11.5 | 3.5 | 100.8 | | | |
| SP-CC13 | | CA | 15.5 | 16 | 6.9 | 97.5 | | | |
| SP-CC13 | | Bulk | 0 | 15 | | | 2.62 | 120.6 | 11.5 |

Notes:

pcf=pounds per cubic foot

1. Samples collected October-December 2013 during the Pre-Design Studies
2. CA = 2-inch diameter California sample, Bulk = 5-gallon bucket sample
3. Maximum dry density listed includes rock correction
4. D = Disturbed, moisture content only

ATTACHMENT B

MEASURED SHEAR WAVE VELOCITIES IN TDA AND UNDERLYING ALLUVIUM (MWH, 2014A)



Shear Wave Velocity Calculations

Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-01
Location: Church Rock Mill Site TSF
Date: November 7, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 5.08 | 4.43 | 4.68 | | | | | | |
| 10.01 | 9.35 | 9.47 | 4.79 | 6.36 | 230 | 2.10 | 754 | 6.9 |
| 15.09 | 14.44 | 14.51 | 5.04 | 6.12 | 251 | 3.62 | 824 | 11.9 |
| 20.01 | 19.36 | 19.41 | 4.90 | 6.81 | 219 | 5.15 | 720 | 16.9 |
| 25.10 | 24.44 | 24.49 | 5.07 | 7.19 | 215 | 6.67 | 705 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 7.68 | 195 | 8.20 | 639 | 26.9 |
| 40.03 | 39.37 | 39.40 | 10.00 | 13.12 | 232 | 10.47 | 762 | 34.4 |
| 45.11 | 44.46 | 44.48 | 5.08 | 6.58 | 235 | 12.77 | 772 | 41.9 |
| 50.03 | 49.38 | 49.40 | 4.92 | 6.19 | 242 | 14.30 | 794 | 46.9 |
| 55.12 | 54.46 | 54.48 | 5.08 | 6.72 | 231 | 15.82 | 757 | 51.9 |
| 60.04 | 59.38 | 59.40 | 4.92 | 7.42 | 202 | 17.35 | 663 | 56.9 |
| 65.12 | 64.47 | 64.49 | 5.08 | 7.64 | 203 | 18.87 | 666 | 61.9 |
| 70.05 | 69.39 | 69.41 | 4.92 | 6.65 | 225 | 20.40 | 740 | 66.9 |
| 75.13 | 74.47 | 74.49 | 5.08 | 5.30 | 292 | 21.92 | 959 | 71.9 |
| 79.72 | 79.07 | 79.08 | 4.59 | 5.17 | 271 | 23.40 | 888 | 76.8 |
| 85.14 | 84.48 | 84.49 | 5.41 | 5.95 | 277 | 24.92 | 910 | 81.8 |
| | | | | | | | | |



Shear Wave Velocity Calculations

Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-02
Location: Church Rock Mill Site TSF
Date: November 5, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 4.43 | 3.77 | 4.06 | | | | | | |
| 10.99 | 10.33 | 10.44 | 6.38 | 6.82 | 285 | 2.15 | 936 | 7.1 |
| 15.09 | 14.44 | 14.51 | 4.07 | 7.31 | 170 | 3.77 | 557 | 12.4 |
| 20.01 | 19.36 | 19.41 | 4.90 | 4.91 | 304 | 5.15 | 998 | 16.9 |
| 25.10 | 24.44 | 24.49 | 5.07 | 5.67 | 273 | 6.67 | 894 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 5.69 | 263 | 8.20 | 864 | 26.9 |
| 33.96 | 33.30 | 33.33 | 3.93 | 3.09 | 389 | 9.55 | 1275 | 31.3 |
| | | | | | | | | |



Shear Wave Velocity Calculations

Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-08
Location: Church Rock Mill Site TSF
Date: November 7, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 5.08 | 4.43 | 4.68 | | | | | | |
| 10.01 | 9.35 | 9.47 | 4.79 | 4.07 | 359 | 2.10 | 1177 | 6.9 |
| 15.09 | 14.44 | 14.51 | 5.04 | 5.91 | 260 | 3.62 | 853 | 11.9 |
| 20.01 | 19.36 | 19.41 | 4.90 | 6.06 | 247 | 5.15 | 809 | 16.9 |
| 25.10 | 24.44 | 24.49 | 5.07 | 6.67 | 232 | 6.67 | 761 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 8.27 | 181 | 8.20 | 594 | 26.9 |
| 35.10 | 34.45 | 34.48 | 5.08 | 8.65 | 179 | 9.72 | 587 | 31.9 |
| 40.35 | 39.70 | 39.73 | 5.25 | 8.20 | 195 | 11.30 | 639 | 37.1 |
| 46.26 | 45.60 | 45.63 | 5.90 | 8.61 | 209 | 13.00 | 685 | 42.7 |
| 50.03 | 49.38 | 49.40 | 3.77 | 2.97 | 388 | 14.47 | 1272 | 47.5 |
| 55.12 | 54.46 | 54.48 | 5.08 | 4.17 | 372 | 15.82 | 1220 | 51.9 |
| 60.04 | 59.38 | 59.40 | 4.92 | 3.79 | 396 | 17.35 | 1298 | 56.9 |
| | | | | | | | | |



Shear Wave Velocity Calculations

Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-10
Location: Church Rock Mill Site TSF
Date: November 6, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 5.08 | 4.43 | 4.68 | | | | | | |
| 10.01 | 9.35 | 9.47 | 4.79 | 5.18 | 282 | 2.10 | 926 | 6.9 |
| 15.09 | 14.44 | 14.51 | 5.04 | 6.94 | 222 | 3.62 | 727 | 11.9 |
| 20.01 | 19.36 | 19.41 | 4.90 | 8.32 | 180 | 5.15 | 589 | 16.9 |
| 25.10 | 24.44 | 24.49 | 5.07 | 10.81 | 143 | 6.67 | 469 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 9.68 | 155 | 8.20 | 508 | 26.9 |
| 35.27 | 34.61 | 34.65 | 5.24 | 10.72 | 149 | 9.75 | 489 | 32.0 |
| 40.03 | 39.37 | 39.40 | 4.75 | 9.45 | 153 | 11.27 | 503 | 37.0 |
| 46.26 | 45.60 | 45.63 | 6.23 | 9.24 | 206 | 12.95 | 674 | 42.5 |
| 50.03 | 49.38 | 49.40 | 3.77 | 3.16 | 364 | 14.47 | 1194 | 47.5 |
| 55.12 | 54.46 | 54.48 | 5.08 | 3.37 | 459 | 15.82 | 1507 | 51.9 |
| 60.04 | 59.38 | 59.40 | 4.92 | 2.69 | 558 | 17.35 | 1829 | 56.9 |
| 63.16 | 62.50 | 62.52 | 3.12 | 2.06 | 461 | 18.57 | 1514 | 60.9 |
| | | | | | | | | |



Shear Wave Velocity Calculations

Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-11
Location: Church Rock Mill Site TSF
Date: November 7, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 5.08 | 4.43 | 4.68 | | | | | | |
| 10.01 | 9.35 | 9.47 | 4.79 | 5.03 | 291 | 2.10 | 953 | 6.9 |
| 15.26 | 14.60 | 14.68 | 5.21 | 6.35 | 250 | 3.65 | 821 | 12.0 |
| 20.01 | 19.36 | 19.41 | 4.74 | 5.65 | 255 | 5.17 | 838 | 17.0 |
| 25.10 | 24.44 | 24.49 | 5.07 | 5.95 | 260 | 6.67 | 853 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 5.48 | 273 | 8.20 | 897 | 26.9 |
| 35.10 | 34.45 | 34.48 | 5.08 | 5.15 | 301 | 9.72 | 987 | 31.9 |
| 40.03 | 39.37 | 39.40 | 4.92 | 5.36 | 280 | 11.25 | 918 | 36.9 |
| 45.11 | 44.46 | 44.48 | 5.08 | 6.13 | 253 | 12.77 | 829 | 41.9 |
| 50.03 | 49.38 | 49.40 | 4.92 | 7.59 | 197 | 14.30 | 648 | 46.9 |
| 55.12 | 54.46 | 54.48 | 5.08 | 7.94 | 195 | 15.82 | 640 | 51.9 |
| 60.04 | 59.38 | 59.40 | 4.92 | 4.00 | 375 | 17.35 | 1229 | 56.9 |
| 65.29 | 64.63 | 64.65 | 5.25 | 4.81 | 332 | 18.90 | 1091 | 62.0 |
| 70.21 | 69.55 | 69.57 | 4.92 | 4.64 | 323 | 20.45 | 1060 | 67.1 |
| 75.13 | 74.47 | 74.49 | 4.92 | 4.90 | 306 | 21.95 | 1003 | 72.0 |
| 80.05 | 79.40 | 79.41 | 4.92 | 4.90 | 306 | 23.45 | 1005 | 76.9 |
| 85.14 | 84.48 | 84.49 | 5.08 | 4.12 | 376 | 24.97 | 1235 | 81.9 |
| 90.06 | 89.40 | 89.42 | 4.92 | 4.84 | 310 | 26.50 | 1016 | 86.9 |
| 95.14 | 94.49 | 94.50 | 5.08 | 3.11 | 499 | 28.02 | 1637 | 91.9 |
| | | | | | | | | |



Shear Wave Velocity Calculations




Job No.: 13-52118
Client: MWH Americas, Inc.
CPT No.: RCPT-15
Location: Church Rock Mill Site TSF
Date: November 6, 2013

Geophone Offset: 0.66 (ft)
Source Offset: 1.50 (ft)

| Test Depth (ft) | Geophone Depth (ft) | Ray Path (ft) | Incremental Distance (ft) | Time Interval (ms) | Interval Velocity (m/s) | Interval Depth (m) | Interval Velocity (ft/s) | Interval Depth (ft) |
|-----------------|---------------------|---------------|---------------------------|--------------------|-------------------------|--------------------|--------------------------|---------------------|
| 5.08 | 4.43 | 4.68 | | | | | | |
| 10.01 | 9.35 | 9.47 | 4.79 | 7.05 | 207 | 2.10 | 680 | 6.9 |
| 15.09 | 14.44 | 14.51 | 5.04 | 8.31 | 185 | 3.62 | 607 | 11.9 |
| 20.01 | 19.36 | 19.41 | 4.90 | 8.07 | 185 | 5.15 | 607 | 16.9 |
| 25.10 | 24.44 | 24.49 | 5.07 | 8.24 | 188 | 6.67 | 616 | 21.9 |
| 30.02 | 29.36 | 29.40 | 4.91 | 7.72 | 194 | 8.20 | 636 | 26.9 |
| 35.10 | 34.45 | 34.48 | 5.08 | 6.35 | 244 | 9.72 | 801 | 31.9 |
| 40.03 | 39.37 | 39.40 | 4.92 | 4.49 | 334 | 11.25 | 1095 | 36.9 |
| 45.93 | 45.28 | 45.30 | 5.90 | 5.41 | 333 | 12.90 | 1092 | 42.3 |
| 50.03 | 49.38 | 49.40 | 4.10 | 3.40 | 367 | 14.42 | 1206 | 47.3 |
| 55.12 | 54.46 | 54.48 | 5.08 | 4.45 | 348 | 15.82 | 1142 | 51.9 |
| | | | | | | | | |

ATTACHMENT C

TAILINGS DISPOSAL AREA BOREHOLE LOGS (MWH, 2014A)

| | | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|---|-----------------|--|------------|-----------------------------|-----------------|-------------------|------------------|--|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B1 | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | | DRILL RIG INFORMATION | | | | BOREHOLE INFORMATION | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | | | | | | | | | |
| DRILLER: M. CAIN | | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6969.7 | | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): N/A | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 70.0 | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/P _I) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 14" | | | | | NA | (0' - 8") SILTY CLAY (FILL) - Light brown, soft, moist silty clay, trace to few very fine to fine sand. | | | | | | | | | | | | |
| 1 | | | | | | (8" - 12") ROCK - 1/2" to 3" crushed basalt. | | | | | | | | | | | | |
| 2 | | | | | | (1' - 18.5') SILTY CLAY WITH SAND (FILL) - Dark brown, firm to hard, slightly moist silty clay, little to some very fine to fine sand, occasional coarse sand and gravel (upper ~5' may be compacted radon barrier). | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 24" | CA 18" | 1C | 8 | | | | | | | | | | | | | | |
| 6 | | | 1B | 9 | | | | | | | | | | | | | | |
| 7 | | | 1A | 11 | | | | | | | | | | | | | | |
| 8 | | | AC | 2 | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 30" | CA 18" | 3C | 10 | | [Below ~10', occasional elevated rad readings indicating possible sand tailings mixed with silty clay fill.] | | | | | | | | | | | | |
| 11 | | | 3B | 12 | | | | | | | | | | | | | | |
| 12 | | | 3A | 14 | | (~11' - ~11.5') 1/2" to 1" gravel observed. | | | | | | | | | | | | |
| 13 | | | AC | 4 | | | | | | | | | | | | | | |




LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:

Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.






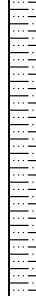
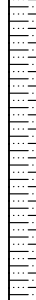
Page 1 of 5

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | |
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NOTES:
 Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

Page 2 of 5

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | | | |
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| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | | |
| 28 | 30" | AC | 10 | NA | (29.6' - 34.3') Very fine grained sand tailings, abundant clayey zones. | CL |  | 13.9 | | | | | | | | | | | |
| 29 | | | | | | | | 14.6 | 91.6 | | | | | | | | 3.0E-7 | 0.092 | |
| 30 | 36" | CA 18" | 11C | 5 | | | | 41.6 | 76.5 | 2.69 | 44/17/27 | 0.0 | 30.9 | 69.1 | | | | | 33.3 |
| 31 | | | 11B | 5 | | | | | | | | | | | | | | | |
| | | | 11A | 4 | | | | | | | | | | | | | | | |
| 32 | | AC | 12 | | (34.3' - ~41') CLAYEY SAND - Loose to medium dense, very moist to wet, very fine-grained clayey sand, increasing clay content with depth. | CL |  | 27.8 | | | 33/16/17 | 0.0 | 46.7 | 53.3 | | | | | |
| 33 | | | | | | | | | | | | | | | | | | | |
| 34 | | | | | | | | | | | | | | | | | | | |
| 35 | 40" | CA 18" | 13C | 7 | | | | | | | | | | | | | | | |
| | | | 13B | 20 | (35' - 36.5') High blow counts due to rock in CA shoe. | CL |  | | | | | | | | | | | | |
| 36 | | | 13A | 22 | | | | 21.0 | 97.3 | 2.73 | | 0.0 | 62.5 | 37.5 | 1.7E-6 | 0.059 | | | |
| 37 | | AC | 14B | | | | | | | | | | | | | | | | |
| 38 | | AC | 14A | | | | | | | | | | | | | | | | |
| 39 | | | | | (41' - ~45') SANDY CLAY - Very moist to wet sandy clay, sand is very fine-grained. | CL |  | | | | | | | | | | | | |
| 40 | 42" | CA 18" | 15C | 3 | | | | | | | | | | | | | | | |
| | | | 15B | 5 | | | | | | | | | | | | | | | |
| 41 | | | 15A | 5 | | | | 26.7 | 98.6 | | 31/15/16 | 0.0 | 18.2 | 81.8 | 1.2E-7 | | | | |
| 42 | | | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:

Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

Page 3 of 5




LEGEND:
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | TI-B1 | | | |
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| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY




NOTES:
 Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.




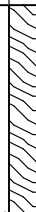







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|  PROJ. LOC.: GALLUP, NM | | CLIENT:  NECR - PRE DESIGN STUDY INVESTIGATION | |  FID. NO. 007 Gallup, New Mexico 87301-0071 | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 60" | | | | NA | | | | | | | | | | | | | |
| 58 | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | |
| 60 | 60" | CA 18" | 5 | | | | | | | | | | | | | | |
| | | 20B | 8 | | | | | | | | | | | | | | |
| 61 | | 20A | 12 | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | |
| 65 | 60" | CA 18" | 5 | | | | | | | | | | | | | | |
| | | 21B | 7 | | | | | | | | | | | | | | |
| 66 | | 21A | 11 | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
| 69 | | | | | (68.2' - E.O.B.) SILTY SAND - Brown, silty, moist very fine to fine sand. | | | | | | | | | | | | |
| 70 | | | | | E.O.B. at 70.0' | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

Page 5 of 5

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B2 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/20/2013 | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6959.9 | | FINISH: 11/21/2013 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 33.5 | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 38.7 | | | |
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|  | | CLIENT: | |  | |  | | BORING LOG | | | | BOREHOLE ID: TI-B2 | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|---|------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) |
| 40" | AC | 7 | | | | tailings, very fine silty sand from 12.8' to 13.2', fine to medium sand from 13.2' to 15'. | |  | 39.6 | | | | 0.0 | 23.1 | 76.9 | | | |
| 14 | | | | | | (15' - 25.7') SILTY SAND - Brown, medium dense, moist silty very fine to fine sand, occasional roots. Appears to be natural "alluvium." Occasional dark brown clay lenses. Rad levels ~ background. | |  | 6.9 | 90.4 | 2.68 | | | | | | | |
| 15 | 42" | 8C | 5 | | | | | | | | | | | | | | | |
| | | 8B | 5 | | | | | | | | | | | | | | | |
| 16 | | 8A | 7 | | | | | | | | | | | | | | | |
| 17 | AC | 9 | | | | | |  | | | | | | | | | | |
| 18 | AC | 10 | | | | | |  | | | | | | | | | | |
| 19 | | | | | | | |  | | | | | | | | | | |
| 20 | 42" | 11C | 4 | | | | |  | | | | | | | | | | |
| | | 11B | 4 | | | | |  | | | | | | | | | | |
| 21 | | 11A | 6 | | | | |  | 7.0 | 91.4 | 2.74 | | 0.0 | 82.9 | 17.1 | | | |
| 22 | AC | 12 | | | | | |  | | | | | | | | | | |
| 23 | AC | 13 | | | | | |  | | | | | | | | | | |
| 24 | | | | | | | |  | | | | | | | | | | |
| 25 | 48" | 14C | 5 | | | | |  | | | | | | | | | | |
| | | 14B | 6 | | | | |  | | | | | | | | | | |
| 26 | | 14A | 6 | | | (25.7' - 33.5') SILTY CLAY - Dark brown, moist, firm to hard, silty clay, trace to few very fine to fine sand, occasional coarse sand. | CL |  | 23.5 | 93.2 | | 34/16/18 | 0.0 | 20.9 | 79.1 | | | |
| 27 | | | | | | | |  | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout. At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation as hole was left open overnight).</div> | | | | | | | | | | | | | | | | | | |
| Page 2 of 3 | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | TI-B2 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
| | | USCS CLASS | | | | | | | | | | | |
| | | GRAPHIC | | | | | | | | | | | |
| | | WATER CONT. (%) | | | | | | | | | | | |
| | | DRY DENSITY (PCF) | | | | | | | | | | | |
| | | SPECIFIC GRAVITY | | | | | | | | | | | |
| | | ATTERBERG LIMITS (LL/PL/PI) | | | | | | | | | | | |
| | | % GRAVEL | | | | | | | | | | | |
| | | % SAND | | | | | | | | | | | |
| | | % FINES | | | | | | | | | | | |
| | | SAT. HYD. COND. (cm/s) | | | | | | | | | | | |
| | | CONSOLIDATION (Cc) | | | | | | | | | | | |
| | | TRIAxIAL (PHI, C [PSF]) | | | | | | | | | | | |
| DEPTH (FT) | | | | | | | | | | | | | |
| CORE RECOV. (IN) | | | | | | | | | | | | | |
| SAMPLES & RECOV. | | | | | | | | | | | | | |
| SAMPLE NO. | | | | | | | | | | | | | |
| BLOW COUNT | | | | | | | | | | | | | |
| BULK SAMPLE NO. | | | | | | | | | | | | | |
| 28-48" | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | |
| 30-54" | | | | | | | | | | | | | |
| | | CA 18" | | | | | | 15C 6 | | | | | |
| | | | | | | | | 15B 11 | | | | | |
| 31 | | | | | | | | 15A 12 | | | | | |
| 32 | | | | | | | | | | | | | |
| 33 | | | | | | | | | | | | | |
| 34 | | | | | | | | | | | | | |
| 35-48" | | NR | | | | | | 50/1" | | | | | |
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|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | | | |
|--|------------------|--|------------|----------------------|---|---------------------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28 | 54" | AC | 3A | | (28.8' - 31') Pale gray, no sand. | CH |  | 61.8 | 62.7 | | 74/25/49 | 0.0 | 9.2 | 90.8 | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 24" | ST 23" | 4 | | (~31' - ~32.5') SAND TAILINGS - Pale yellowish brown, medium dense, moist, fine to medium sand, trace silt. | |  | 41.4 | | | | | | | | | |
| 31 | | | | | | | | | | | | | | | | | |
| 32 | | | | | (~32.5' - 35') FINE TAILINGS WITH SAND - Pale gray, soft, moist, very fine to fine sand. | |  | | | | | | | | | | |
| 33 | | AC | 5 | | | | | | | | | | | | | | |
| 34 | | | | | (35' - 38.6') CLAYEY/SILTY SAND TAILINGS - Pale yellowish gray, soft, moist, very fine to fine sand. | |  | 14.3 | 90.9 | 2.66 | | | | | | | |
| 35 | 30" | ST 28" | 6 | | | | | | | | | | | | | | |
| 36 | | | | | | |  | 16.5 | 89.6 | 2.67 | | | | | 1.6E-5 | | |
| 37 | | | | | | | | | | | | | | | | | |
| 38 | | AC | 7 | | (38.6' - 44.5') FINE TAILINGS - Pale gray, firm, moist, trace to few very fine sand. | |  | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | |
| 40 | 37" | ST 27" | 8 | | | SC / CL |  | 39.7 | 80.4 | 2.63 | | | | | | | |
| 41 | | | | | | | | | | | | | | | | | |
| 42 | | | | | | |  | 34.3 | 83.6 | | 35/16/19 | 0.0 | 51.2 | 48.8 | 1.3E-7 | 0.262 | |




LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:
Hole backfilled with cement/bentonite grout.




Page 3 of 5




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | |
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|  PROJ. LOC.: GALLUP, NM | | CLIENT:  NECR - PRE DESIGN STUDY INVESTIGATION | |  FACILITY: 100-001 GALLUP, NM (Area) 100-001 | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 58 | | | | | (58.7' - 59.5') Silty clay with sand. | | | | | | | | | | | | |
| 59 | | | | | (59.5' - 60') Reddish brown, fine to medium sand. | | | | | | | | | | | | |
| 60 | 48" | CA 18" | 13C | 16 | | | | | | | | | | | | | |
| | | | 13B | 22 | (60.5' - 61') COAL - sandy. | | | | | | | | | | | | |
| 61 | | | 13A | 50/ 4" | (61' - E.O.B.) SHALE - Dark grayish brown, hard to very hard, moist, silty, trace very fine sand. | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| 64 | 12" | | | 14 | (bagged core) At 64' - becomes fissile, very hard, brittle, more sand (few to little). | | | | | | | | | | | | |
| 65 | | CA 2" | 15 | 50/ 2" | 65.2' E.O.B. (Practical Auger Refusal at 65.0') | | | | | | | | | | | | |
| 66 | | | | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
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| 70 | | | | | | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

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|--|------------------|---------------------------------------|------------|--|-----------------|--|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | | | BOREHOLE INFORMATION | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/26/2013 | | | | | | | | | | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6973.3 | | FINISH: 11/27/2013 | | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 105.0 | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 108.2 | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 21" | | | | | | (0' - 0.6') SANDY CLAY - Light brown, soft, very moist sandy clay, very fine sand, some roots, silty. | | | | | | | | | | | | |
| 1 | | | | | | (0.6' - 0.9') ROCK - 1/2" to 3" crushed basalt. | | | | | | | | | | | | |
| 2 | | | | | | (0.9' - 6.8') SANDY CLAY - Dark brown, hard, slightly moist to moist sandy clay, very fine to fine sand, occasional coarse sand to fine gravel, silty. | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 45" | CA 17" | | 12 | | | | | | | | | | | | | | |
| 6 | | 1B | | 13 | | | | | | | | | | | | | | |
| 7 | | 1A | | 19 | | | | | | | | | | | | | | |
| 8 | | | | | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 33" | ST 27.5" | 2 | | | (6.8' - 18.9') SILTY SAND TAILINGS - Pale yellowish gray, loose to medium dense, moist, fine to medium, silty sand tailings, occasional more clayey/silty zones. | | | 9.7 | 110.0 | 2.63 | | | | | | | |
| 11 | | | | | | | | | 9.0 | 96.8 | | | 0.2 | 71.9 | 27.9 | 4.3E-4 | 0.094 | |
| 12 | | | | | | | | | | | | | | | | | | |
| 13 | | AC | 3 | | | | | | 6.7 | | 2.61 | | | | | | | |
| | | | | | | | | | 7.5 | 99.1 | | | 0.7 | 71.5 | 27.8 | 6.7E-5 | | |
| LEGEND: | | | | | | NOTES: | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | | |
| Page 1 of 8 | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B10 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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| MWH | | CLIENT: | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
|----------------------------|------------------|---------------------------------------|------------|----------------------|---|---------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28 | 42" | AC 9A | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 42" | ST 27" 10 | | | | CH | | 45.3 | 73.4 | 2.78 | 57/22/35 | 0.0 | 24.3 | 75.7 | | | |
| 31 | | | | | (~31' - 33.2') SAND TAILINGS - See description for 24.4' - 25.7'. | | | | | | | | | | | | |
| 32 | | | | | | SM | | 15.4 | 100.1 | 2.67 | NP | 0.0 | 83.1 | 16.9 | | | |
| 33 | | AC 11 | | | | | | | | | | | | | | | |
| 34 | | | | | (33.2' - 44.6') FINE TAILINGS - Pale gray, soft, moist to very moist. | | | | | | | | | | | | |
| 35 | 48" | CA 18" 12C 2 | | | | | | 47.7 | 72.5 | | | | | | | | |
| | | 12B 2 | | | | | | 51.4 | | | | | | | | | |
| 36 | | 12A 2 | | | | SC / CL | | 32.2 | 87.8 | 2.72 | 36/16/20 | 0.0 | 50.6 | 49.4 | | | |
| 37 | | AC 13B | | | (36.3' - 37.8') Some very fine to fine sand. | | | | | | | | | | | | |
| 38 | | AC 13A | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | |
| 40 | 30" | ST 30" 14 | | | | CH | | 45.7 | 73.7 | 2.56 | | | | | | | |
| 41 | | | | | | | | 47.2 | 74.5 | | 61/21/40 | 0.0 | 20.7 | 79.3 | 2.9E-8 | 0.315 | |
| 42 | | | | | | | | | | | | | | | | | |






LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




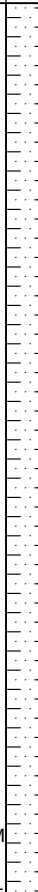
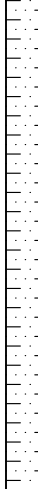
NOTES:
Hole backfilled with cement/bentonite grout.

Page 3 of 8

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
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NOTES:
 Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) |
| 43 | 30" | ST 30" AC | 14 15 | | | (44.3' - 44.6') Appears finer grained (clayey), lighter gray, more moist. (44.6' - 85.5') SILTY SAND - Light brown, medium dense, moist, silty very fine to fine sand, occasional coarse sand and fine gravel. | |  | | | | | | | | | | |
| 45 | 42" | CA 17" | 16B 16A | 12 12 14 | | | | | | 9.9 | 95.4 | 2.74 | | 0.0 | 65.8 | 34.2 | | |
| 50 | 48" | CA 18" | 17B 17A | 10 11 14 | | | | | | | | | | | | | | |
| 55 | 42" | ST 17" | 18 | | | (~56' - 57.5') Gravelly. (Shelby Tube refusal at 56.5') | |  | 14.1 | 100.8 | | | | | | 2.4E-5 | 0.139 | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| Page 4 of 8 | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | (62.5' - 65.2') Weathered Sandstone (?) - Hard, moist, gravelly. | |  | | | | | | | | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | | | | | | | | | | | | | |
| 60" | 39" | CA 18" | 11 | | | | | | | | | | | | | | |
| | | 19B | 11 | | | | | | | | | | | | | | |
| 61" | | 19A | 14 | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 48" | CA 18" | 14 | | | | | | | | | | | | | | |
| | | 20B | 14 | | | | | | | | | | | | | | |
| 66" | | 20A | 15 | | SM / ML | | 13.8 | 94.5 | | NP | 0.0 | 50.1 | 49.9 | | | | |
| 67" | | | | | (70.5' - 71.5') Moist to very moist, increased clay. | |  | | | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | 30" | CA 18" | 4 | | | | | | | | | | | | | | |
| | | 21B | 6 | | | | | | | | | | | | | | |
| 71" | | 21A | 10 | | | | 18.1 | 100.8 | | | | | | | | | |

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NR = NO RECOVERY




NOTES:




Hole backfilled with cement/bentonite grout.




Page 5 of 8




LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 72-30" | | | | | | | | | | | | | | | | | |
| 73- | | | | | | | | | | | | | | | | | |
| 74- | | | | | | | | | | | | | | | | | |
| 75-42" | CA 18" | | 5 | | | | | | | | | | | | | | |
| | | 22B | 7 | | | | | | | | | | | | | | |
| 76- | | 22A | 11 | | | | | | | | | | | | | | |
| 77- | | | | | | | | | | | | | | | | | |
| 78- | | | | | | | | | | | | | | | | | |
| 79- | | | | | | | | | | | | | | | | | |
| 80-36" | CA 18" | | 9 | | (80' - 82') Gravelly (sandstone fragments) | | | | | | | | | | | | |
| | | 23B | 14 | | | | | | | | | | | | | | |
| 81- | | 23A | 17 | | | | | | | | | | | | | | |
| 82- | | | | | (82' - 85.5') Weathered Sandstone - Mottled red/gray/brown, moist, fine to medium weathered sandstone. | | | | | | | | | | | | |
| 83- | | | | | | | | | | | | | | | | | |
| 84-NA | 3" | 24 | 50/3" | | | | | | | | | | | | | | |
| 85-50" | | | | | | | | | | | | | | | | | |
| 86- | | | | | (85.5' - 105') CLAYEY SAND - Dark brown, firm, very moist to wet, fine to medium clayey sand, occasional sandstone fragments. | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| Page 6 of 8 | | | | | | | | | | | | | | | | | |

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|--|------------------|---|------------|-----------------|---|-------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 50" | | | | | | | | | | | | | | | | | |
| 87 | | | | | | | | | | | | | | | | | |
| 88 | | | | | | | | | | | | | | | | | |
| 89 | | | | | | | | | | | | | | | | | |
| 90 | 40" | CA 18" | 7 | | [CA sampler wet 11/26/13.] | | | | | | | | | | | | |
| | | 25B | 12 | | [Water measured at approximately 90.2' bgs at 9:30 11/27/13.] | | | | | | | | | | | | |
| 91 | | 25A | 10 | | | | | 18.6 | 105.6 | 2.66 | | | | | | | |
| 92 | | | | | | | | | | | | | | | | | |
| 93 | | | | | [Core barrel wet 11/27/13.] | | | | | | | | | | | | |
| 94 | | | | | | | | | | | | | | | | | |
| 95 | 52" | NR | 1 | | | | | | | | | | | | | | |
| | | | 5 | | | | | | | | | | | | | | |
| 96 | | | 8 | | | | | | | | | | | | | | |
| 97 | | | | | | | | | | | | | | | | | |
| 98 | | | | | [Core barrel wet.] | | | | | | | | | | | | |
| 99 | | | | | | | | | | | | | | | | | |
| 100 | 44" | | | | | | | | | | | | | | | | |
| 101 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | TI-B10 | | | |
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| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
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|  | | CLIENT:   | | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | DRILL RIG INFORMATION | | | BOREHOLE INFORMATION | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | | | | | | | | | | |
| DRILLER: M. CAIN | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6977.3 | | | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 96.9 | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 103.0 | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Co) | TRIAxIAL (PHI, C (PSF)) |
| 19" | | | | | | (0' - 0.6') SANDY CLAY - Brown, soft, moist to very moist sandy clay, very fine to fine sand, roots, silty. | | | | | | | | | | | | |
| 1 | | | | | | (0.6' - 0.9') ROCK - 1/2" to 3" crushed basalt, silty clay in voids. | | | | | | | | | | | | |
| 2 | | | | | | (0.9' - 10.3') SANDY CLAY - Dark brown, hard, slightly moist sandy clay, very fine to fine sand, occasional coarse sand and gravel up to 1.5". | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 30" | CA 17" | 1C | 12 | | | | | | | | | | | | | | |
| 6 | | | 1B | 16 | | | | | | | | | | | | | | |
| 7 | | | 1A | 15 | | | | | | | | | | | | | | |
| 8 | | | | | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 42" | CA 18" | 2C | 7 | | | | | | | | | | | | | | |
| 11 | | | 2B | 7 | | (10.3' - 12') SILTY SAND - Light brown, medium dense, slightly moist, silty very fine to fine sand. | | | | | | | | | | | | |
| 12 | | | 2A | 8 | | | | | | | | | | | | | | |
| 13 | | | | | | (12' - 15') SANDY CLAY - Dark brown, firm to hard, slightly moist sandy clay, very fine to fine sand, occasional gravel up to 3" size. | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)

ST = SHELBY TUBE (3-INCH OD)

AC = ACRYLIC LINER




HSA = HOLLOW-STEM AUGER






CC = CONTINUOUS CORE




NR = NO RECOVERY




NOTES:

Hole backfilled with cement/bentonite grout.

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|--|------------------|---------------------------------------|------------|--|---|----------------------|---------|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | |
| 15 | 45" | ST 13" | 3 | | (15' - 18') CLAYEY SAND - Light yellowish brown, medium dense, slightly moist, fine to medium clayey sand, occasional gravel up to 1". | | | 8.2 | 110.4 | 2.67 | | 3.9 | 57.6 | 38.5 | 2.5E-5 | 0.09 | |
| 16 | | | | | | | | | | | | | | | | | |
| 17 | | | | | | | | | | | | | | | | | |
| 18 | | | | | (18' - 32.9') SANDY CLAY - Predominantly dark brown, hard, slightly moist sandy clay, silty, very fine to medium sand, few to little coarse sand and gravel up to ~1" size. | | | | | | | | | | | | |
| 19 | | | | | (19.2' - 19.4') Sand, very fine to fine. | | | | | | | | | | | | |
| 20 | 48" | CA 18" | 4C | 4 | | | | | | | | | | | | | |
| | | | 4B | 7 | | | | | | | | | | | | | |
| 21 | | | 4A | 10 | | | | 12.3 | 107.6 | | | | | | | | |
| 22 | | | | | | | | | | | | | | | | | |
| 23 | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | |
| 25 | 56" | CA 18" | 5C | 7 | | | | | | | | | | | | | |
| | | | 5B | 8 | | | | | | | | | | | | | |
| 26 | | | 5A | 13 | | | | | | | | | | | | | |
| 27 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 2 of 8 | | | | | | | | | | | | | | | | | |




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|--|------------------|---------------------------------------|------------|--|-----------------|---|----------------------|---|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28.56" | | | | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | | |
| 30.47" | ST 21" | 6 | | | | | CL |  | 13.7 | 112.4 | | 30/13/17 | 7.1 | 41.3 | 51.6 | 9.0E-7 | 0.06 | |
| 31 | | | | | | | | | | | | | | | | | | |
| 32 | | | | | | | | | | | | | | | | | | |
| 33 | | | | | | (32.9' - 34') SAND (TAILINGS?) - Pale yellowish gray, slightly moist, fine to medium sand. | |  | | | | | | | | | | |
| 34 | NA | | | | | (34' - 45.5') SANDY CLAY WITH GRAVEL - Dark brown, firm to hard, moist sandy clay with very fine to coarse sand and gravel up to ~3", some metallic and fibrous debris. | | | | | | | | | | | | |
| 35 | | | | | | [34' - 38' Drilling through metallic debris (appears to be metal siding). Center bit required to penetrate debris. No core collected. CA sample attempted at 34' and 35' - no penetration or recovery.] | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | | |
| 40.51" | CA 3" | | | 25 | | [Metallic debris in CA shoe - no sample.] | | | | | | | | | | | | |
| 41 | | | | 27 | | | | | | | | | | | | | | |
| 42 | | | | 22 | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |
| Page 3 of 8 | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 51" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 60" | 7C | 7 | | (Photo 310 at 46'.) | | | | | | | | | | | | |
| | | 7B | 7 | | (45.5' - 53.9') FINE TAILINGS - Mottled orange and dark greenish gray (to 50'), pale yellowish gray (50' - 53.9'), firm, moist tailings. | | | 88.7 | | | | | | | | | |
| 46 | | 7A | 8 | | | | | | | | | | | | | | |
| | | | | | [Photo 311 at 46.5'] | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 43" | ST 28" | 8 | | | | | | | | | | | | | | |
| 51 | | | | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | |
| | | AC | 9 | | [Photo 312 at 52.5'] | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | (53.9' - 55') SILTY CLAY - Dark brown, hard, moist silty clay, trace very fine sand. | | | | | | | | | | | | |
| 55 | 48" | ST 25" | 10 | | (55' - 77.5') SILTY SAND - Yellowish brown, medium dense, slightly moist to moist, silty, very fine to fine sand. | | | | | | | | | | | | |
| 56 | | | | | | SM | | 16.2 | 77.9 | 2.64 | NP | 0.0 | 60.4 | 39.6 | 5.6E-4 | 0.129 | |
| 57 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> | | | | | | | | | | | | | | | | | |
| <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 4 of 8 | | | | | | | | | | | | | | | | | |

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|--|----------------------------|---------------------------------------|------------|--|-----------------------------|------------|----------------------|-------------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| DEPTH (FT) | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 48" | AC | 11 | | | (61.1' - 62.1') Sandy clay. | | | 16.0 | 95.4 | | | 0.0 | 38.7 | 61.3 | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | (63.1' - 64') Sandy clay. | | | | | | | | | | | | |
| 60" | 48" | CA 17" | 9 | | | | | | | | | | | | | | |
| | | 12B | 11 | | | | | | | | | | | | | | |
| 61" | | 12A | 12 | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 49" | CA 18" | 13C | 7 | | | | | | | | | | | | | |
| | | 13B | 7 | | | | | | | | | | | | | | |
| 66" | | 13A | 12 | | | | | 14.2 | 96.2 | | | | | | | | |
| 67" | | | | | | | | | | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | 44" | CA 18" | 14C | 7 | | | | | | | | | | | | | |
| | | 14B | 9 | | | | | | | | | | | | | | |
| 71" | | 14A | 10 | | | | | | | | | | | | | | |
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| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 5 of 8 | | | | | | | | | | | | | | | | | |





LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|-------------------|--|-----------------|---|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | |
| 72-74 | 44" | | | | | (71.5' - 73.5') Abundant clayey sand zones. | | | | | | | | | | | | | |
| 75-76 | 38" | CA 18" | 15C 15B 15A | 7 8 11 | | | | | | | | | | | | | | | |
| 77-78 | | | | | | | | | | | | | | | | | | | |
| 78-79 | | | | | 16 | (77.5' - 78') WEATHERED SANDSTONE - Rusty red, moist, fine to medium grained. (Sample #16 is bagged core.) | | | | | | | | | | | | | |
| 79-80 | | | | | | (78' - 96.9') GRAVELLY SAND - Mottled rusty red/brown/yellow, dense, moist fine to medium sand, silty throughout, some clayey zones, abundant coarse material from coarse sand up to 3" gravel comprised of cemented sandstone. | | | | | | | | | | | | | |
| 80-81 | 42" | CA 18" | 17C 17B 17A | 16 21 21 | | | | | 11.0 | 107.6 | 2.76 | | 12.9 | 65.6 | 21.5 | | | | |
| 81-82 | | | | | | | | | | | | | | | | | | | |
| 82-83 | | | | | | | | | | | | | | | | | | | |
| 83-84 | | | | | | | | | | | | | | | | | | | |
| 84-85 | | | | | | | | | | | | | | | | | | | |
| 85-86 | 36" | CA 17" | 18C 18B 18A | 18 21 19 | | | | | | | | | | | | | | | |
| LEGEND: | | | | | | NOTES: | | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | | | |
| Page 6 of 8 | | | | | | | | | | | | | | | | | | | |




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
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 CC = CONTINUOUS CORE
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NOTES:
 Hole backfilled with cement/bentonite grout.

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|---|------------------|---|------------|----------------------|-----------------|---|---|---|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | |
| 102 | | | | | | (102.5' - 103') Reddish brown, strongly cemented sandstone. | |  | | | | | | | | | | | |
| 103 | | | | | | | E.O.B. at 103.0' at 10:00 (practical auger refusal) | | | | | | | | | | | | |
| 104 | | | | | | | | | | | | | | | | | | | |
| 105 | | | | | | | | | | | | | | | | | | | |
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| 114 | | | | | | | | | | | | | | | | | | | |
| 115 | | | | | | | | | | | | | | | | | | | |




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
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 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
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NOTES:
 Hole backfilled with cement/bentonite grout.

|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | | |
|---|------------------|---|------------|-----------------------------|-----------------|---|------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | | | | | | | | | | | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6976.8 | | | | | | | | | | | | |
| DRILLER'S HELPER: L. ALDAZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | FINISH: 12/5/2013 | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | DEPTH TO BEDROCK (FT): N/A | | | | | | | | | | | | |
| TOTAL DEPTH (FT): 71.5 | | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Co) | TRIAxIAL (Phi, C (PSF)) |
| 18" | | | | | | (0' - 0.5') SANDY CLAY - Brown, soft, moist to very moist sandy clay, very fine sand, roots. | | | | | | | | | | | | |
| 1 | | | | | | (0.5' - 0.8') ROCK - Crushed basalt, up to 3" size, sandy clay in voids. | | | | | | | | | | | | |
| 2 | | | | | | (0.8' - ~3') SANDY CLAY - Dark yellowish brown, hard, moist sandy clay, very fine to fine sand. | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | |
| 5 | 30" | CA 18" | 1C | 10 | | | | | | | | | | | | | | |
| 6 | | | 1B | 11 | | | | | | | | | | | | | | |
| 7 | | | 1A | 12 | | | | | | | | | | | | | | |
| 8 | | | AC | 2 | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | |
| 10 | 30" | CA 18" | | 3 | | | | | | | | | | | | | | |
| 11 | | | 3B | 3 | | | | | | | | | | | | | | |
| 12 | | | 3A | 3 | | | | | | | | | | | | | | |
| 13 | | | AC | 4 | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
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 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B15 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 48" | | | | | | | | | | | | | | | | | |
| 43- | | | | | | | | | | | | | | | | | |
| 44- | | | | | | | | | | | | | | | | | |
| 45- | 26" | CA 18" 15C | 13 | | (45' - 50') SANDY SILT - Dark yellowish brown, hard, moist, very fine to fine sand, occasional clayey sand zones. | | | | | | | | | | | | |
| | | 15B | 25 | | | | | | | | | | | | | | |
| 46- | | 15A | 26 | | | | | 25.8 17.3 | 99.3 | 2.81 | NP | 0.0 | 37.0 | 63.0 | | | |
| 47- | | | | | | | | | | | | | | | | | |
| 48- | | | | | | | | | | | | | | | | | |
| 49- | | | | | | | | | | | | | | | | | |
| 50- | 24" | CA 18" 16C | 6 | | (50' - 52') CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine sand, silty. | | | | | | | | | | | | |
| | | 16B | 8 | | | | | | | | | | | | | | |
| 51- | | 16A | 11 | | | | | | | | | | | | | | |
| 52- | | | | | (52' - 65') SILTY CLAY - Dark yellowish brown, firm to hard, moist silty clay, trace to few very fine to fine sand, occasional thin (1-6") clayey sand zones. (~53' - 55') Very hard, very dense clay. | | | | | | | | | | | | |
| 53- | | | | | | | | | | | | | | | | | |
| 54- | | | | | | | | | | | | | | | | | |
| 55- | 18" | CA 18" 17C | 10 | | | | | | | | | | | | | | |
| | | 17B | 11 | | | | | | | | | | | | | | |
| 56- | | 17A | 12 | | | | | 11.7 | 104.2 | | | | | | | | |
| 57- | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:

Hole backfilled with cement/bentonite grout.

Page 4 of 5

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
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NOTES:
 Hole backfilled with cement/bentonite grout.

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|  | | CLIENT: | |  | |  P.O. BOX 9877 Gallup, New Mexico 87309-0277 | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| DEPTH (FT) | | CORE RECOV. (IN) | | SAMPLES & RECOV. | | SAMPLE NO. | | BLOW COUNT | | BULK SAMPLE NO. | | MATERIAL DESCRIPTION | | | | | | USCS CLASS | | GRAPHIC | | WATER CONT. (%) | | DRY DENSITY (PCF) | | SPECIFIC GRAVITY | | ATTERBERG LIMITS (LL/PL/PI) | | % GRAVEL | | % SAND | | % FINES | | SAT. HYD. COND. (cm/s) | | CONSOLIDATION (Cc) | | TRIAxIAL (PHI, C [PSF]) | |
| 18" | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 58 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 60 | | 60" | | CA 18" | | 18C | | 8 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | 18B | | 11 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 61 | | | | | | 18A | | 15 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 65 | | 40" | | CA 18" | | 19B | | 8 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | 19A | | 10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 66 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 70 | | | | CA 18" | | 20B | | 6 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 71 | | | | | | 20A | | 9 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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ATTACHMENT D

TAILINGS DISPOSAL AREA CONE PENETRATION TEST RESULTS (MWH, 2014A)



MWH Americas

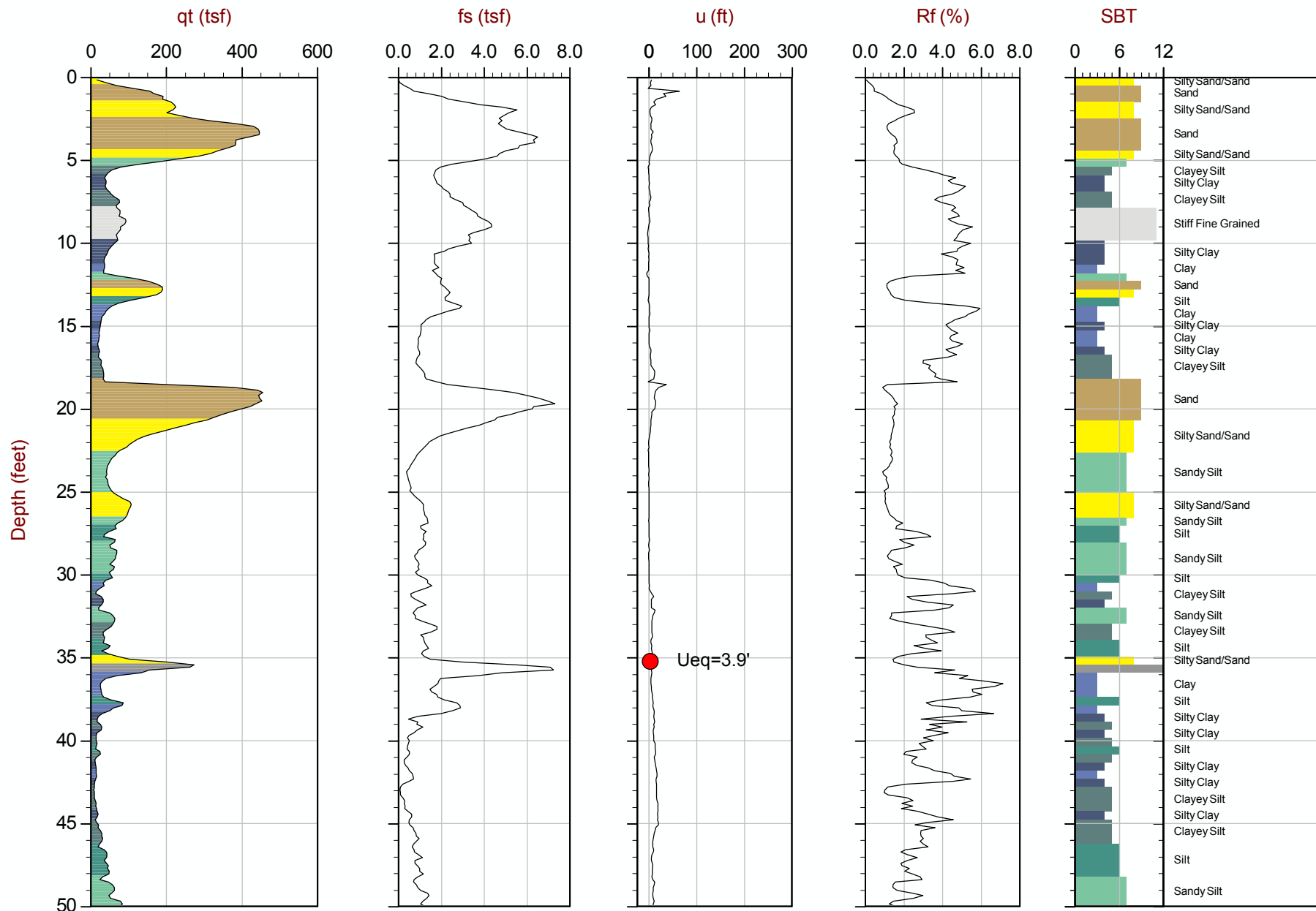
Job No: 13-52118

Date: 11:07:13 15:36

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-01

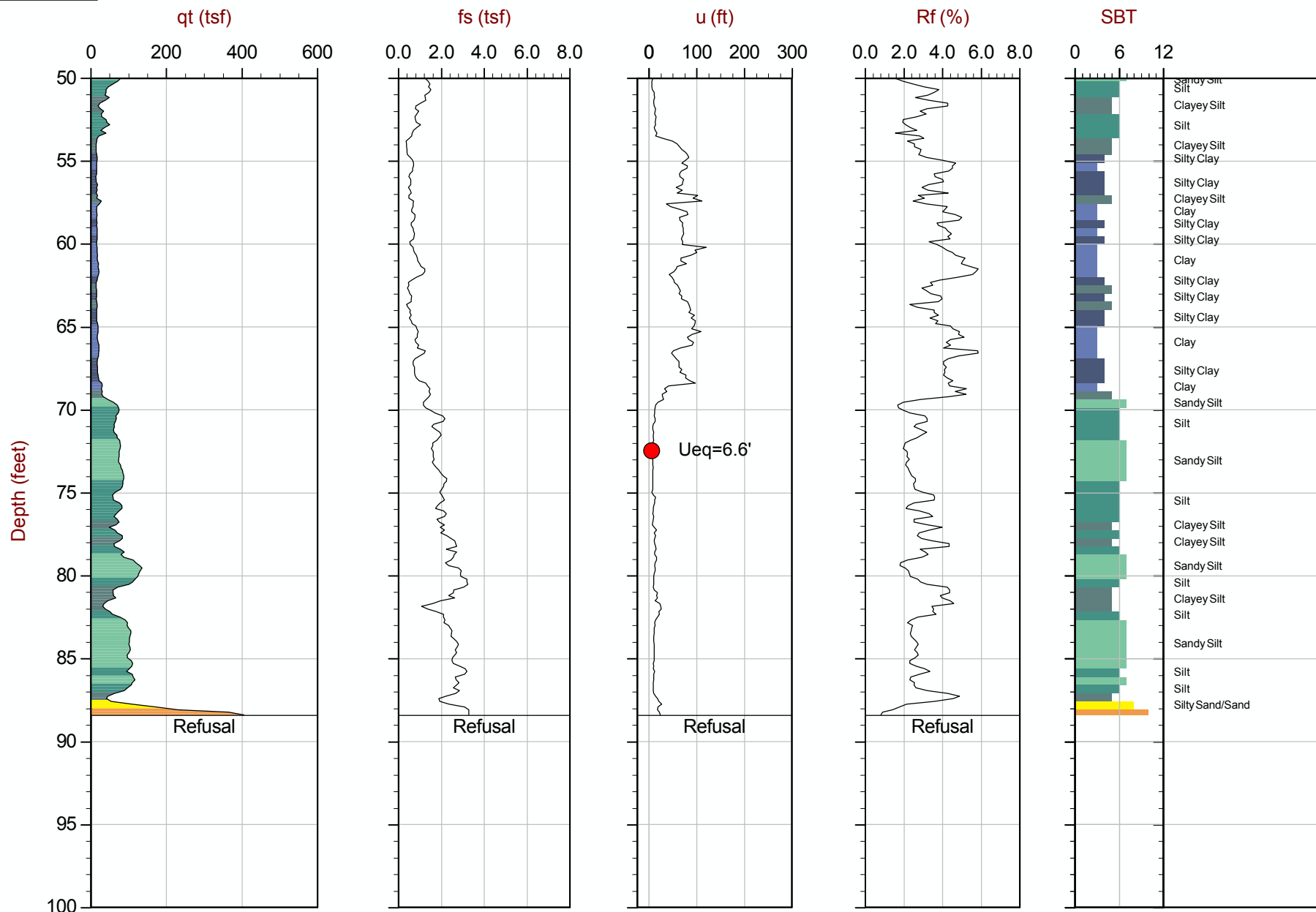
Cone: 155:T1500F15U500



Max Depth: 26.950 m / 88.42 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP01.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.649117 Long: -108.501667
● Equilibrium Pore Pressure from Dissipation



Max Depth: 26.950 m / 88.42 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP01.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
 Coords: Lat: 35.649117 Long: -108.501667
 ● Equilibrium Pore Pressure from Dissipation



MWH Americas

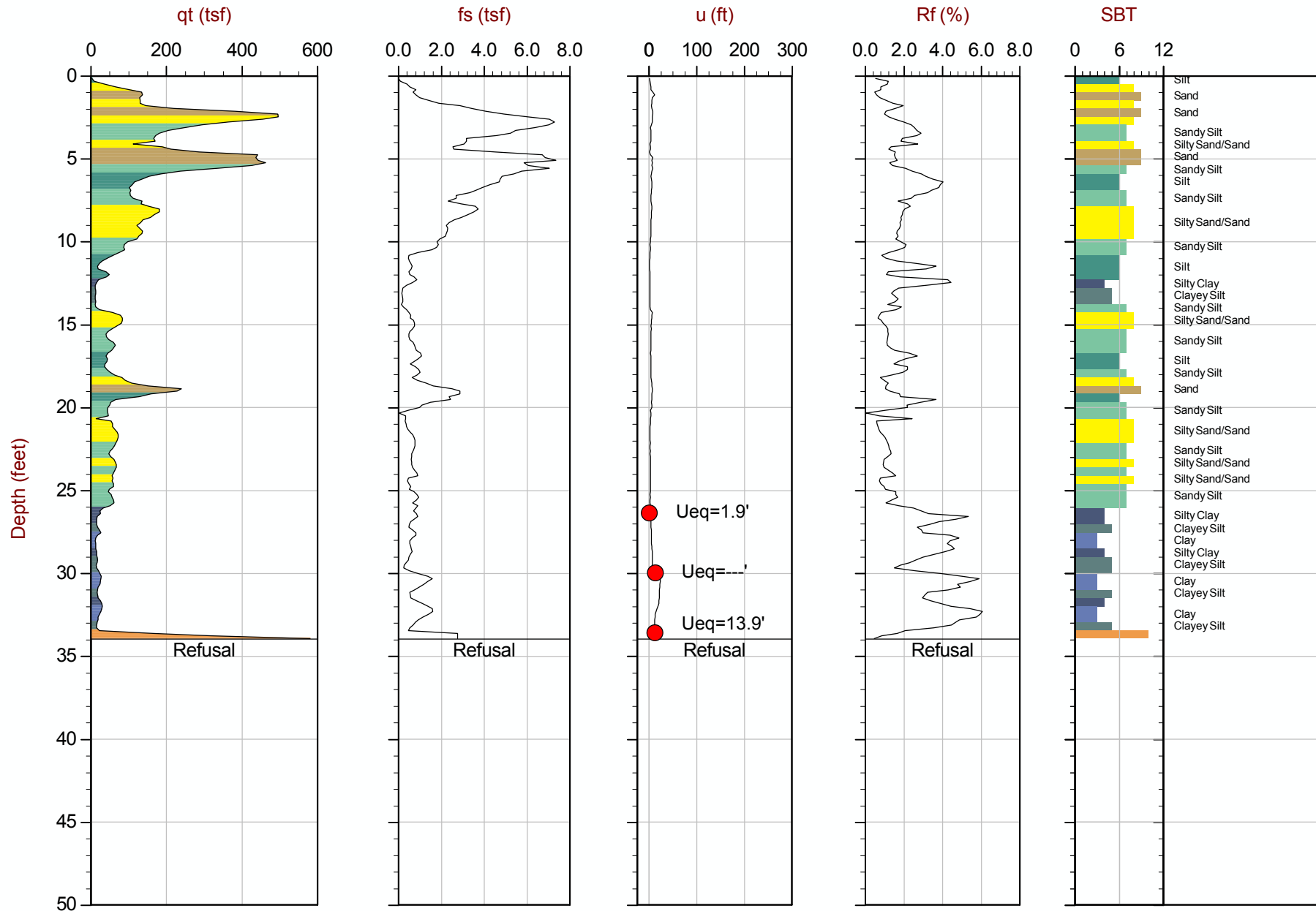
Job No: 13-52118

Date: 11:05:13 13:37

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-02

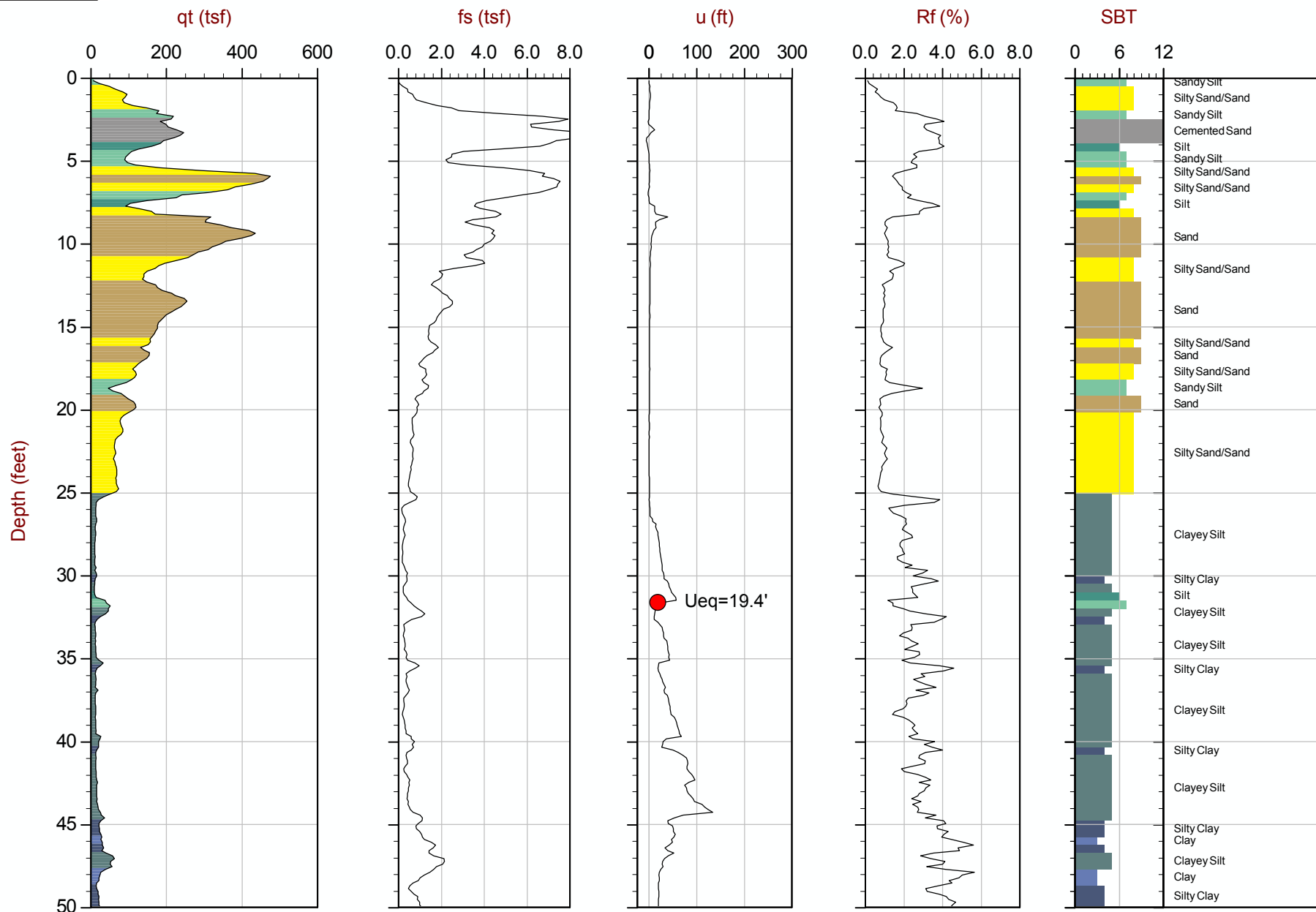
Cone: 155:T1500F15U500



Max Depth: 10.350 m / 33.96 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP02.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.650200 Long: -108.499750
● Equilibrium Pore Pressure from Dissipation



Max Depth: 18.550 m / 60.86 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP08.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
 Coords: Lat: 35.647250 Long: -108.497250
 ● Equilibrium Pore Pressure from Dissipation



MWH Americas

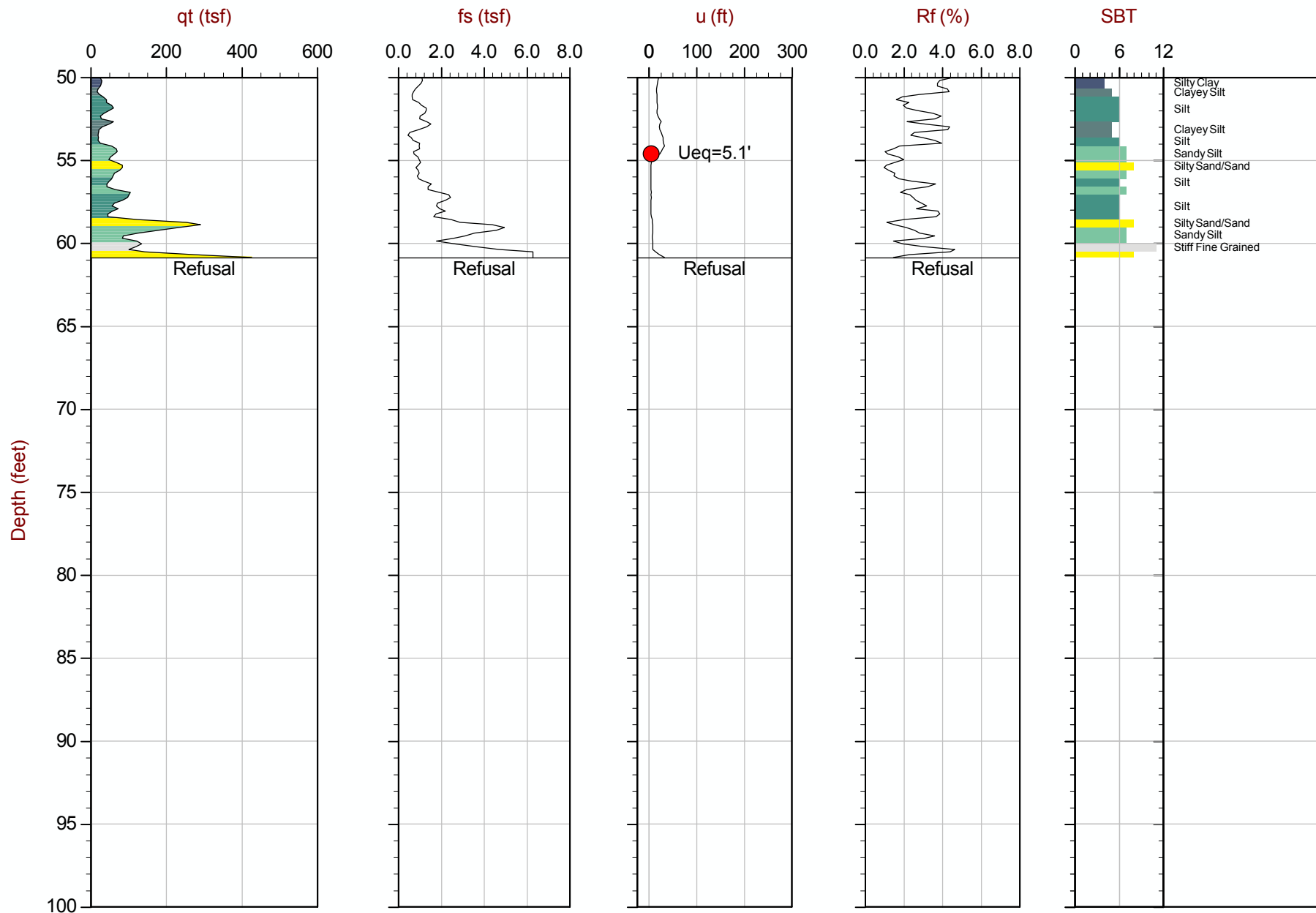
Job No: 13-52118

Date: 11:07:13 08:21

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-08

Cone: 155:T1500F15U500



Max Depth: 18.550 m / 60.86 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP08.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647250 Long: -108.497250
● Equilibrium Pore Pressure from Dissipation



MWH Americas

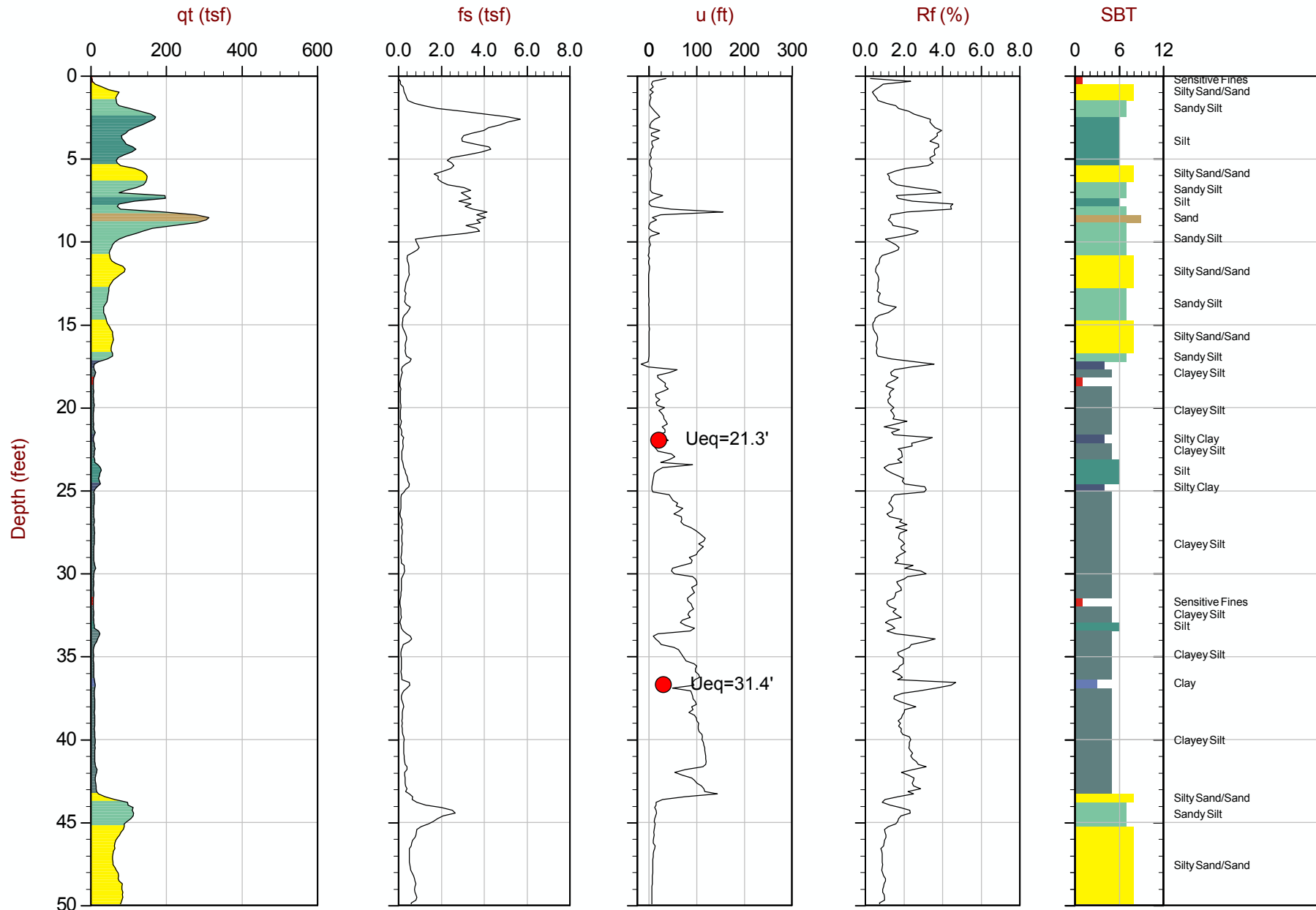
Job No: 13-52118

Date: 11:06:13 10:23

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-10

Cone: 155:T1500F15U500



Max Depth: 19.250 m / 63.16 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP10.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647833 Long: -108.497217
● Equilibrium Pore Pressure from Dissipation



MWH Americas

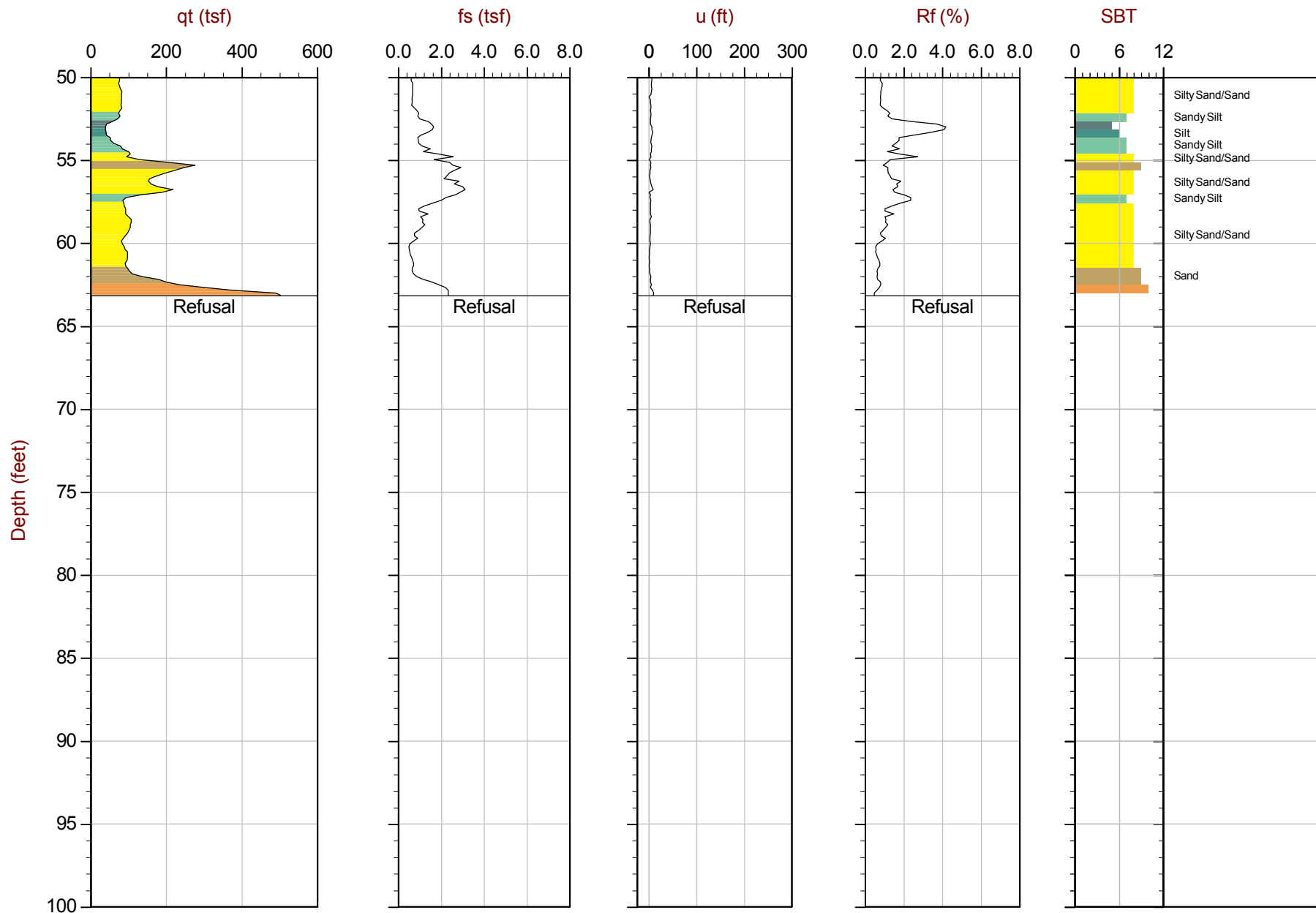
Job No: 13-52118

Date: 11:06:13 10:23

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-10

Cone: 155:T1500F15U500



Max Depth: 19.250 m / 63.16 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP10.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647833 Long: -108.497217
● Equilibrium Pore Pressure from Dissipation



MWH Americas

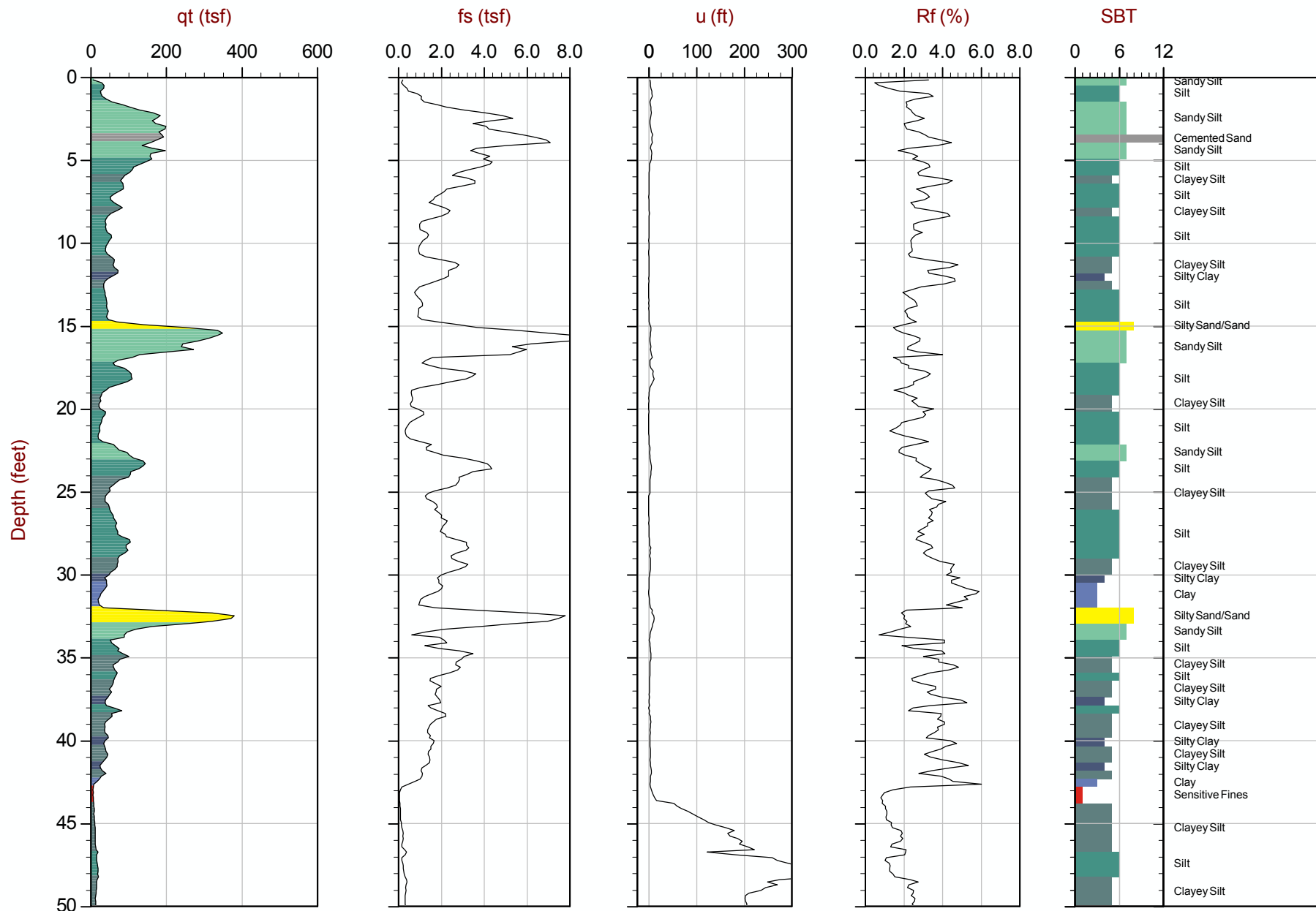
Job No: 13-52118

Date: 11:07:13 12:13

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-11

Cone: 155:T1500F15U500



Max Depth: 29.500 m / 96.78 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP11.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647650 Long: -108.495850
● Equilibrium Pore Pressure from Dissipation



MWH Americas

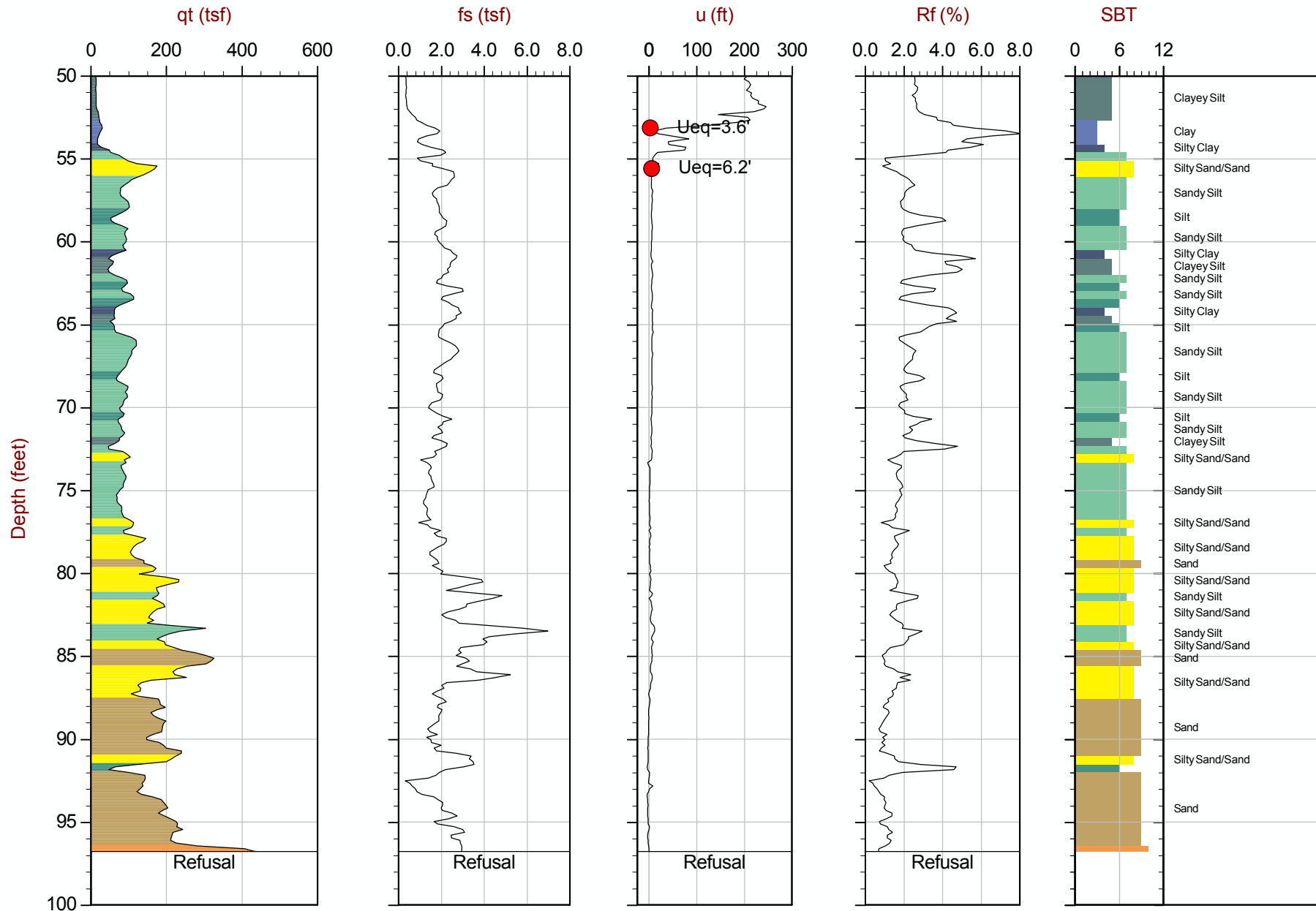
Job No: 13-52118

Date: 11:07:13 12:13

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-11

Cone: 155:T1500F15U500



Max Depth: 29.500 m / 96.78 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP11.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647650 Long: -108.495850
● Equilibrium Pore Pressure from Dissipation



MWH Americas

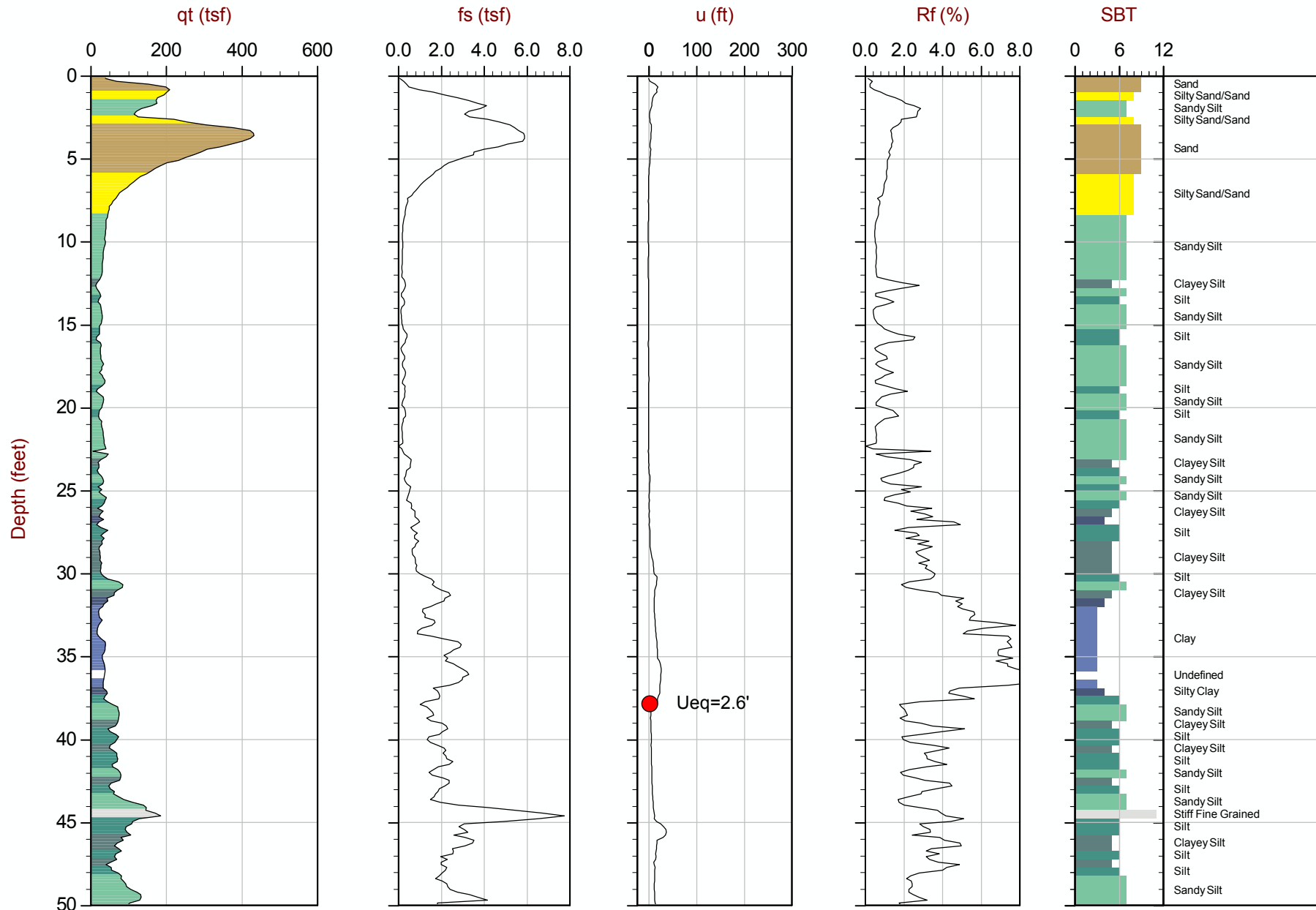
Job No: 13-52118

Date: 11:06:13 16:32

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-15

Cone: 155:T1500F15U500



Max Depth: 16.800 m / 55.12 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP15.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647583 Long: -108.499800
● Equilibrium Pore Pressure from Dissipation



MWH Americas

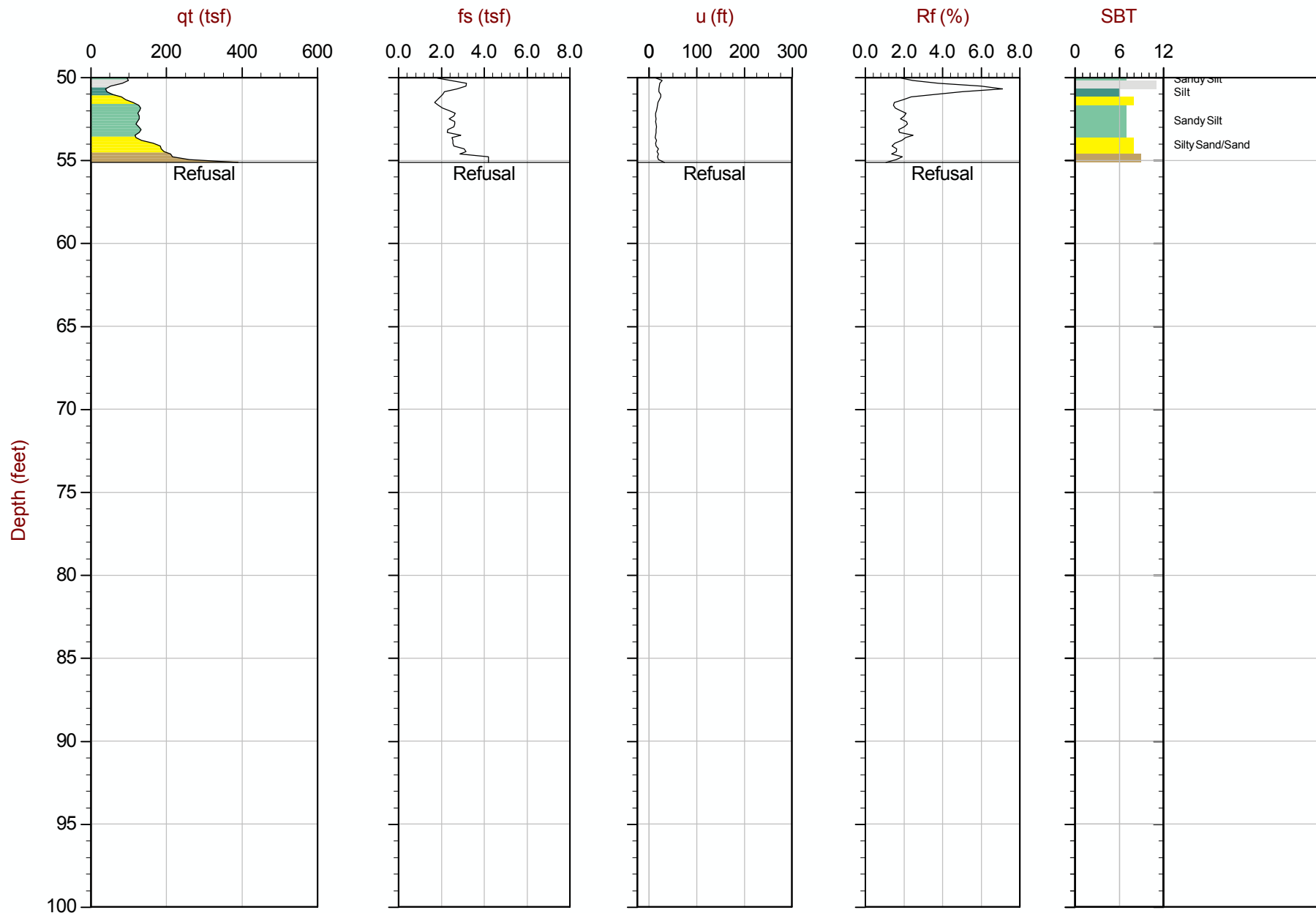
Job No: 13-52118

Date: 11:06:13 16:32

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-15

Cone: 155:T1500F15U500



Max Depth: 16.800 m / 55.12 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP15.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647583 Long: -108.499800
● Equilibrium Pore Pressure from Dissipation

ATTACHMENT E

RECORDED WATER LEVELS AT THE CHURCH ROCK SITE

| Well ID | Measurement Date | Measurement Time | Historical Reference Elev | Water Level Depth | Water Level Elev |
|---------|------------------|------------------|---------------------------|-------------------|------------------|
| 0509 D | 1/4/2016 | 8:37 | 6949.44 | 82.89 | 6866.55 |
| EPA 23 | 1/4/2016 | 9:30 | 6926.31 | 59.52 | 6866.79 |
| GW 1 | 1/4/2016 | 14:20 | 6916.46 | 65.01 | 6851.45 |
| GW 2 | 7/6/2015 | 14:25 | 6912.88 | 58.96 | 6853.92 |
| GW 3 | 7/7/2015 | 10:50 | 6910.04 | 56 | 6854.04 |
| 632 | 1/4/2016 | 12:35 | 6903.49 | 48.05 | 6855.44 |
| EPA 25 | 1/5/2016 | 10:35 | 6903.38 | 56.62 | 6846.76 |
| EPA 27 | 1/12/1999 | | 6910.95 | 55.45 | 6855.5 |
| EPA 28 | 1/4/2016 | 15:20 | 6917.86 | 65.83 | 6852.03 |
| 624 | 1/4/2016 | 16:35 | 6898.57 | 53.61 | 6844.96 |

Note: Water levels provided by email from Chester Engineers, on April 20, 2016.

ATTACHMENT F

SEISMIC SETTLEMENT ANALYSIS CALCULATIONS

| UNC-NECR WASTE REPOSITORY SEISMIC SETTLEMENT ANALYSIS - CPT-01 | | | | | | | | | |
|---|--|---|--|---------------------------------------|--|------|---|--|--|
| Data File: 13-52118_RP01-BSC-CPT | | Cells Requiring User Input/Manipulation | | Idriss and Boulanger (2008) | | 0.00 | Water surface elevation during CPT investigation (ft amsl) | | |
| Location: UNC-NECR 2013 Mill Site PDS | | | | Max. Horiz. Acceleration, Amax/g: 0.3 | | | | | |
| http://projects.mwhglobal.com/.../13-52118_RP02-BSC-CPT_XLS | | | | Earthquake Moment Magnitude, M: 5.5 | | 0.00 | Water surface elevation at t ₀ (ft amsl) | | |
| | | | | Magnitude Scaling Factor, MSF: 1.69 | | 0.00 | Water surface elevation at t ₁ (ft amsl) | | |
| Erosion Protection | | Coarse Tailings | | Youd, et al (2001) | | 1.44 | Scaling Factor for stress ration, r _m | | |
| Cover Soil | | Coarse/Fine Tailings | | | | 0.47 | Volumetric Strain Ratio for Site-Specific Design Earthquake | | |
| Mine Spoils | | Fine Tailings | | Max. Horiz. Acceleration, Amax/g: 0.3 | | | | | |
| Radon Barrier | | Coarse Alluvium | | Earthquake Moment Magnitude, M: 6.3 | | 8.26 | Equiv. Number of Uniform Strain Cycles, N | | |
| General Fill | | Fine Alluvium | | Magnitude Scaling Factor, MSF: 1.59 | | | | | |

| Midpoint Depth at t_1, z_1 (m) | Shear Wave Velocity, V_s (ft/sec) | Soil Density, ρ (pcf) | Max Shear Strain Modulus, G_{max} (tsf) | Coefficient a_s for Stress Reduction Factor | Stress Reduction Factor, r_d | $P =$ $\gamma_{eff}(G_{eff}/G_{max})$ (tsf) | Plasticity Index, PI | g_1 for PI $= 0$ | g_1 for PI $= 15$ | g_1 for PI $= 30$ | g_2 | g_2 for PI $= 0$ | g_2 for PI $= 15$ | g_2 for PI $= 30$ | g_2 | Shear Strain, γ (%) | a | b | Threshold Shear Strain, γ_{th} (%) | Volumetric Strain at 15 Cycles, ϵ_{v-15} (%) | R | c | C_u | Volumetric Strain for Design Event, ϵ_v (%) | Incremental Consolidation n (ft) |
|--|--|----------------------------------|--|---|--------------------------------------|---|----------------------------|-----------------------|------------------------|------------------------|-------|-----------------------|------------------------|------------------------|-------|----------------------------------|------|------|--|--|------|-------|-------|---|--|
| 0.23 | 866 | 1.9E-03 | 1.4E+03 | 1249 | 0.9990 | 6.3E-06 | 12 | 0.102 | 0.091 | 4 | 0.093 | 35267 | 24912 | 1400 | 26983 | 0.001% | 2.00 | 0.65 | 0.03% | 0.000000 | 0.34 | 0.079 | | 0.00% | 0.0000 |
| 0.55 | 866 | 1.8E-03 | 1.3E+03 | 1121 | 0.9974 | 1.8E-05 | 12 | 0.129 | 0.118 | 4 | 0.120 | 23525 | 16487 | 1400 | 17895 | 0.002% | 2.00 | 0.65 | 0.03% | 0.000000 | 0.34 | 0.079 | | 0.00% | 0.0000 |
| 0.64 | 866 | 1.8E-03 | 1.4E+03 | 1087 | 0.9969 | 2.2E-05 | 12 | 0.136 | 0.125 | 4 | 0.127 | 21433 | 14994 | 1400 | 16282 | 0.002% | 1.18 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | | 0.00% | 0.0000 |

| Seismic Settlement Analysis - Stewart et al (2004) | | | | | | | | | |
|--|------|-------------|-----------|-----------------|------------------|----------------------|-----------------------|-----------------------|----------------------|
| Case | Soil | Foundation | Load (kN) | Settlement (mm) | Settlement Ratio | Settlement Ratio (%) | Settlement Ratio (in) | Settlement Ratio (ft) | Settlement Ratio (m) |
| 1 | Clay | 1.0m x 1.0m | 1000 | 10 | 0.01 | 1.0 | 0.0004 | 0.0013 | 0.0004 |
| 2 | Clay | 1.0m x 1.0m | 2000 | 20 | 0.02 | 2.0 | 0.0008 | 0.0026 | 0.0008 |
| 3 | Clay | 1.0m x 1.0m | 3000 | 30 | 0.03 | 3.0 | 0.0012 | 0.0039 | 0.0012 |
| 4 | Clay | 1.0m x 1.0m | 4000 | 40 | 0.04 | 4.0 | 0.0016 | 0.0052 | 0.0016 |
| 5 | Clay | 1.0m x 1.0m | 5000 | 50 | 0.05 | 5.0 | 0.0020 | 0.0065 | 0.0020 |
| 6 | Clay | 1.0m x 1.0m | 6000 | 60 | 0.06 | 6.0 | 0.0024 | 0.0078 | 0.0024 |
| 7 | Clay | 1.0m x 1.0m | 7000 | 70 | 0.07 | 7.0 | 0.0028 | 0.0091 | 0.0028 |
| 8 | Clay | 1.0m x 1.0m | 8000 | 80 | 0.08 | 8.0 | 0.0032 | 0.0104 | 0.0032 |
| 9 | Clay | 1.0m x 1.0m | 9000 | 90 | 0.09 | 9.0 | 0.0036 | 0.0117 | 0.0036 |
| 10 | Clay | 1.0m x 1.0m | 10000 | 100 | 0.10 | 10.0 | 0.0040 | 0.0130 | 0.0040 |
| 11 | Clay | 1.0m x 1.0m | 11000 | 110 | 0.11 | 11.0 | 0.0044 | 0.0143 | 0.0044 |
| 12 | Clay | 1.0m x 1.0m | 12000 | 120 | 0.12 | 12.0 | 0.0048 | 0.0156 | 0.0048 |
| 13 | Clay | 1.0m x 1.0m | 13000 | 130 | 0.13 | 13.0 | 0.0052 | 0.0169 | 0.0052 |
| 14 | Clay | 1.0m x 1.0m | 14000 | 140 | 0.14 | 14.0 | 0.0056 | 0.0182 | 0.0056 |
| 15 | Clay | 1.0m x 1.0m | 15000 | 150 | 0.15 | 15.0 | 0.0060 | 0.0195 | 0.0060 |
| 16 | Clay | 1.0m x 1.0m | 16000 | 160 | 0.16 | 16.0 | 0.0064 | 0.0208 | 0.0064 |
| 17 | Clay | 1.0m x 1.0m | 17000 | 170 | 0.17 | 17.0 | 0.0068 | 0.0221 | 0.0068 |
| 18 | Clay | 1.0m x 1.0m | 18000 | 180 | 0.18 | 18.0 | 0.0072 | 0.0234 | 0.0072 |
| 19 | Clay | 1.0m x 1.0m | 19000 | 190 | 0.19 | 19.0 | 0.0076 | 0.0247 | 0.0076 |
| 20 | Clay | 1.0m x 1.0m | 20000 | 200 | 0.20 | 20.0 | 0.0080 | 0.0260 | 0.0080 |
| 21 | Clay | 1.0m x 1.0m | 21000 | 210 | 0.21 | 21.0 | 0.0084 | 0.0273 | 0.0084 |
| 22 | Clay | 1.0m x 1.0m | 22000 | 220 | 0.22 | 22.0 | 0.0088 | 0.0286 | 0.0088 |
| 23 | Clay | 1.0m x 1.0m | 23000 | 230 | 0.23 | 23.0 | 0.0092 | 0.0299 | 0.0092 |
| 24 | Clay | 1.0m x 1.0m | 24000 | 240 | 0.24 | 24.0 | 0.0096 | 0.0312 | 0.0096 |
| 25 | Clay | 1.0m x 1.0m | 25000 | 250 | 0.25 | 25.0 | 0.0100 | 0.0325 | 0.0100 |
| 26 | Clay | 1.0m x 1.0m | 26000 | 260 | 0.26 | 26.0 | 0.0104 | 0.0338 | 0.0104 |
| 27 | Clay | 1.0m x 1.0m | 27000 | 270 | 0.27 | 27.0 | 0.0108 | 0.0351 | 0.0108 |
| 28 | Clay | 1.0m x 1.0m | 28000 | 280 | 0.28 | 28.0 | 0.0112 | 0.0364 | 0.0112 |
| 29 | Clay | 1.0m x 1.0m | 29000 | 290 | 0.29 | 29.0 | 0.0116 | 0.0377 | 0.0116 |
| 30 | Clay | 1.0m x 1.0m | 30000 | 300 | 0.30 | 30.0 | 0.0120 | 0.0390 | 0.0120 |
| 31 | Clay | 1.0m x 1.0m | 31000 | 310 | 0.31 | 31.0 | 0.0124 | 0.0403 | 0.0124 |
| 32 | Clay | 1.0m x 1.0m | 32000 | 320 | 0.32 | 32.0 | 0.0128 | 0.0416 | 0.0128 |
| 33 | Clay | 1.0m x 1.0m | 33000 | 330 | 0.33 | 33.0 | 0.0132 | 0.0429 | 0.0132 |
| 34 | Clay | 1.0m x 1.0m | 34000 | 340 | 0.34 | 34.0 | 0.0136 | 0.0442 | 0.0136 |
| 35 | Clay | 1.0m x 1.0m | 35000 | 350 | 0.35 | 35.0 | | | |

CPT-01 - Page 1 of 7

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|-------|-------|-------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|----|-------|-------|---|-------|-------|------|------|-------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 10.499 | 6959.46 | 45.0 | 2.147 | 45.0 | -0.8 | -0.36 | 4.77% | General Fill | 0.057 | 113.8 | 0.61 | 0.000 | 0.61 | 0 | 73 | 4.84% | 2.5 | 48% | 0.76 | 0.000 | 0.76 | 3.84 | 824 | 1.8E-03 | 1.2E+03 | 376 | 0.9666 | 1.2E-04 | 19 | 0.196 | 0.190 | 4 | 1.206 | 11169 | 7715 | 1400 | 6031 | 0.019% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 10.663 | 6959.30 | 42.4 | 1.673 | 42.4 | -0.2 | -0.08 | 3.95% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 68 | 4.00% | 2.5 | 48% | 0.77 | 0.000 | 0.77 | 3.89 | 824 | 1.8E-03 | 1.2E+03 | 370 | 0.9659 | 1.2E-04 | 19 | 0.196 | 0.191 | 4 | 1.207 | 11113 | 7675 | 1400 | 6002 | 0.019% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 10.827 | 6959.13 | 37.6 | 1.695 | 37.6 | -0.8 | -0.36 | 4.51% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 59 | 4.58% | 2.5 | 48% | 0.78 | 0.000 | 0.78 | 3.94 | 824 | 1.8E-03 | 1.2E+03 | 364 | 0.9651 | 1.2E-04 | 19 | 0.197 | 0.192 | 4 | 1.207 | 11059 | 7637 | 1400 | 5974 | 0.019% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 10.991 | 6958.97 | 35.2 | 1.694 | 35.3 | -0.1 | -0.04 | 4.81% | General Fill | 0.057 | 113.8 | 0.63 | 0.000 | 0.63 | 0 | 55 | 4.89% | 2.6 | 48% | 0.79 | 0.000 | 0.79 | 3.99 | 824 | 1.8E-03 | 1.2E+03 | 358 | 0.9643 | 1.2E-04 | 19 | 0.197 | 0.192 | 4 | 1.208 | 11005 | 7599 | 1400 | 5946 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 11.155 | 6958.81 | 35.4 | 1.677 | 35.4 | -0.9 | -0.41 | 4.74% | General Fill | 0.057 | 113.8 | 0.64 | 0.000 | 0.64 | 0 | 54 | 4.83% | 2.6 | 48% | 0.80 | 0.000 | 0.80 | 4.04 | 824 | 1.8E-03 | 1.2E+03 | 352 | 0.9635 | 1.3E-04 | 19 | 0.198 | 0.193 | 4 | 1.208 | 10952 | 7562 | 1400 | 5919 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 11.319 | 6958.64 | 36.7 | 1.729 | 36.7 | -0.7 | -0.29 | 4.71% | General Fill | 0.057 | 113.8 | 0.65 | 0.000 | 0.65 | 0 | 55 | 4.80% | 2.6 | 48% | 0.81 | 0.000 | 0.81 | 4.09 | 824 | 1.8E-03 | 1.2E+03 | 347 | 0.9627 | 1.3E-04 | 19 | 0.198 | 0.193 | 4 | 1.209 | 10900 | 7525 | 1400 | 5892 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000117 | 0.34 | 0.079 | 0.797 | 0.02% | 0.0000 |
| 11.483 | 6958.48 | 37.0 | 1.894 | 37.0 | -0.7 | -0.29 | 5.12% | General Fill | 0.057 | 113.8 | 0.66 | 0.000 | 0.66 | 0 | 55 | 5.21% | 2.6 | 48% | 0.82 | 0.000 | 0.82 | 4.14 | 824 | 1.8E-03 | 1.2E+03 | 341 | 0.9619 | 1.3E-04 | 19 | 0.199 | 0.194 | 4 | 1.209 | 10849 | 7489 | 1400 | 5865 | 0.021% | 1.70 | 0.75 | 0.02% | 0.000197 | 0.34 | 0.079 | 0.797 | 0.03% | 0.0001 |
| 11.647 | 6958.31 | 34.0 | 1.595 | 34.0 | -0.8 | -0.36 | 4.69% | General Fill | 0.057 | 113.8 | 0.67 | 0.000 | 0.67 | 0 | 50 | 4.78% | 2.6 | 48% | 0.83 | 0.000 | 0.83 | 4.19 | 824 | 1.8E-03 | 1.2E+03 | 335 | 0.9610 | 1.3E-04 | 19 | 0.200 | 0.195 | 4 | 1.209 | 10798 | 7454 | 1400 | 5839 | 0.021% | 1.70 | 0.75 | 0.02% | 0.000268 | 0.34 | 0.079 | 0.797 | 0.04% | 0.0001 |
| 11.811 | 6958.15 | 33.6 | 1.739 | 33.7 | -4.6 | -1.97 | 5.17% | General Fill | 0.057 | 113.8 | 0.68 | 0.000 | 0.68 | 0 | 48 | 5.28% | 2.6 | 48% | 0.84 | 0.000 | 0.84 | 4.24 | 824 | 1.8E-03 | 1.2E+03 | 330 | 0.9602 | 1.3E-04 | 19 | 0.200 | 0.195 | 4 | 1.210 | 10749 | 7419 | 1400 | 5814 | 0.021% | 1.70 | 0.75 | 0.02% | 0.000333 | 0.34 | 0.079 | 0.797 | 0.05% | 0.0001 |
| 11.975 | 6957.99 | 74.4 | 1.842 | 74.4 | -3.9 | -1.71 | 2.48% | General Fill | 0.057 | 113.8 | 0.69 | 0.000 | 0.69 | 0 | 107 | 2.50% | 2.2 | 48% | 0.84 | 0.000 | 0.84 | 4.29 | 824 | 1.8E-03 | 1.2E+03 | 325 | 0.9593 | 1.3E-04 | 19 | 0.201 | 0.196 | 4 | 1.210 | 10700 | 7384 | 1400 | 5788 | 0.021% | 1.70 | 0.75 | 0.02% | 0.000395 | 0.34 | 0.079 | 0.797 | 0.06% | 0.0001 |
| 12.139 | 6957.82 | 114.3 | 2.031 | 114.3 | 0.4 | 0.18 | 1.78% | General Fill | 0.057 | 113.8 | 0.70 | 0.000 | 0.70 | 0 | 162 | 1.79% | 1.9 | 48% | 0.85 | 0.000 | 0.85 | 4.34 | 824 | 1.8E-03 | 1.2E+03 | 320 | 0.9585 | 1.3E-04 | 19 | 0.201 | 0.196 | 4 | 1.211 | 10652 | 7350 | 1400 | 5764 | 0.022% | 1.70 | 0.75 | 0.02% | 0.000453 | 0.34 | 0.079 | 0.797 | 0.07% | 0.0001 |
| 12.303 | 6957.66 | 152.5 | 1.987 | 152.5 | 0.4 | 0.18 | 1.30% | General Fill | 0.057 | 113.8 | 0.71 | 0.000 | 0.71 | 0 | 214 | 1.31% | 1.8 | 48% | 0.86 | 0.000 | 0.86 | 4.39 | 824 | 1.8E-03 | 1.2E+03 | 314 | 0.9576 | 1.3E-04 | 19 | 0.202 | 0.197 | 4 | 1.211 | 10604 | 7317 | 1400 | 5739 | 0.022% | 1.70 | 0.75 | 0.02% | 0.000509 | 0.34 | 0.079 | 0.797 | 0.08% | 0.0001 |
| 12.467 | 6957.49 | 174.4 | 1.981 | 174.4 | 0.1 | 0.04 | 1.14% | General Fill | 0.057 | 113.8 | 0.72 | 0.000 | 0.72 | 0 | 242 | 1.14% | 1.7 | 48% | 0.87 | 0.000 | 0.87 | 4.44 | 824 | 1.8E-03 | 1.2E+03 | 309 | 0.9567 | 1.4E-04 | 19 | 0.202 | 0.197 | 4 | 1.211 | 10558 | 7284 | 1400 | 5715 | 0.022% | 1.70 | 0.75 | 0.02% | 0.000563 | 0.34 | 0.079 | 0.797 | 0.09% | 0.0001 |
| 12.631 | 6957.33 | 188.8 | 2.144 | 188.8 | 0.0 | 0.00 | 1.14% | General Fill | 0.057 | 113.8 | 0.73 | 0.000 | 0.73 | 0 | 259 | 1.14% | 1.7 | 48% | 0.88 | 0.000 | 0.88 | 4.49 | 824 | 1.8E-03 | 1.2E+03 | 304 | 0.9558 | 1.4E-04 | 19 | 0.203 | 0.198 | 4 | 1.212 | 10512 | 7252 | 1400 | 5691 | 0.023% | 1.70 | 0.75 | 0.02% | 0.000615 | 0.34 | 0.079 | 0.797 | 0.10% | 0.0002 |
| 12.795 | 6957.17 | 190.8 | 2.281 | 190.8 | 0.0 | 0.00 | 1.20% | General Fill | 0.057 | 113.8 | 0.74 | 0.000 | 0.74 | 0 | 258 | 1.20% | 1.7 | 48% | 0.89 | 0.000 | 0.89 | 4.54 | 824 | 1.8E-03 | 1.2E+03 | 300 | 0.9548 | 1.4E-04 | 19 | 0.203 | 0.199 | 4 | 1.212 | 10467 | 7220 | 1400 | 5668 | 0.023% | 1.70 | 0.75 | 0.02% | 0.000666 | 0.34 | 0.079 | 0.797 | 0.11% | 0.0002 |
| 12.959 | 6957.00 | 185.8 | 2.421 | 185.8 | 0.0 | 0.00 | 1.30% | General Fill | 0.057 | 113.8 | 0.75 | 0.000 | 0.75 | 0 | 248 | 1.31% | 1.7 | 48% | 0.90 | 0.000 | 0.90 | 4.59 | 824 | 1.8E-03 | 1.2E+03 | 295 | 0.9539 | 1.4E-04 | 19 | 0.204 | 0.199 | 4 | 1.213 | 10422 | 7189 | 1400 | 5645 | 0.023% | 1.70 | 0.75 | 0.02% | 0.000716 | 0.34 | 0.079 | 0.797 | 0.11% | 0.0002 |
| 13.123 | 6956.84 | 173.6 | 2.319 | 173.6 | -0.2 | -0.08 | 1.34% | Coarse Tailings | 0.054 | 108.1 | 0.75 | 0.000 | 0.75 | 0 | 229 | 1.34% | 1.7 | 21% | 0.91 | 0.000 | 0.91 | 4.64 | 824 | 1.8E-03 | 1.1E+03 | 290 | 0.9529 | 1.5E-04 | 0 | 0.204 | 0.200 | 4 | 1.213 | 10380 | 7159 | 1400 | 10380 | 0.024% | 1.79 | 1.00 | 0.01% | 0.000251 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 13.287 | 6956.67 | 141.9 | 2.166 | 141.9 | -0.6 | -0.26 | 1.53% | Coarse Tailings | 0.054 | 108.1 | 0.76 | 0.000 | 0.76 | 0 | 185 | 1.53% | 1.9 | 21% | 0.92 | 0.000 | 0.92 | 4.69 | 824 | 1.7E-03 | 1.1E+03 | 285 | 0.9520 | 1.5E-04 | 0 | 0.204 | 0.200 | 4 | 0.204 | 10339 | 7131 | 1400 | 10339 | 0.024% | 1.79 | 1.00 | 0.01% | 0.000257 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 13.451 | 6956.51 | 104.4 | 2.194 | 104.4 | -1.4 | -0.61 | 2.10% | Coarse Tailings | 0.054 | 108.1 | 0.77 | 0.000 | 0.77 | 0 | 134 | 2.12% | 2.0 | 21% | 0.93 | 0.000 | 0.93 | 4.74 | 824 | 1.7E-03 | 1.1E+03 | 281 | 0.9510 | 1.5E-04 | 0 | 0.205 | 0.201 | 4 | 0.205 | 10298 | 7102 | 1400 | 10298 | 0.025% | 1.79 | 1.00 | 0.01% | 0.000262 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 13.615 | 6956.35 | 74.1 | 2.570 | 74.1 | -0.1 | -0.06 | 3.47% | Coarse Tailings | 0.054 | 108.1 | 0.78 | 0.000 | 0.78 | 0 | 94 | 3.50% | 2.3 | 21% | 0.94 | 0.000 | 0.94 | 4.79 | 824 | 1.7E-03 | 1.1E+03 | 276 | 0.9500 | 1.5E-04 | 0 | 0.205 | 0.201 | 4 | 0.205 | 10258 | 7074 | 1400 | 10258 | 0.025% | 1.79 | 1.00 | 0.01% | 0.000268 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 13.779 | 6956.18 | 56.9 | 2.954 | 56.9 | 0.8 | 0.36 | 5.20% | Coarse Tailings | 0.054 | 108.1 | 0.79 | 0.000 | 0.79 | 0 | 71 | 5.27% | 2.5 | 21% | 0.95 | 0.000 | 0.95 | 4.84 | 824 | 1.7E-03 | 1.1E+03 | 272 | 0.9490 | 1.5E-04 | 0 | 0.206 | 0.202 | 4 | 0.206 | 10219 | 7046 | 1400 | 10219 | 0.025% | 1.79 | 1.00 | 0.01% | 0.000274 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 13.943 | 6956.02 | 48.2 | 2.863 | 48.2 | 1.1 | 0.47 | 5.94% | Coarse Tailings | 0.054 | 108.1 | 0.80 | 0.000 | 0.80 | 0 | 59 | 6.04% | 2.6 | 21% | 0.95 | 0.000 | 0.95 | 4.89 | 824 | 1.7E-03 | 1.1E+03 | 268 | 0.9479 | 1.5E-04 | 0 | 0.206 | 0.202 | 4 | 0.206 | 10180 | 7018 | 1400 | 10180 | 0.026% | 1.79 | 1.00 | 0.01% | 0.000279 | 0.36 | 0.025 | 0.785 | 0.04% | 0.0001 |
| 14.107 | 6955.85 | 40.4 | 2.323 | 40.4 | 0.9 | 0.39 | 5.76% | General Fill | 0.057 | 113.8 | 0.81 | 0.000 | 0.81 | 0 | 49 | 5.87% | 2.7 | 48% | 0.96 | 0.000 | 0.96 | 4.94 | 824 | 1.8E-03 | 1.2E+03 | 263 | 0.9469 | 1.5E-04 | 19 | 0.207 | 0.203 | 4 | 1.215 | 10139 | 6990 | 1400 | 5499 | 0.025% | 1.70 | 0.75 | 0.02% | 0.001023 | 0.34 | 0.079 | 0.797 | 0.16% | 0.0003 |
| 14.271 | 6955.69 | 36.0 | 1.938 | 36.0 | 1.9 | 0.81 | 5.38% | General Fill | 0.057 | 113.8 | 0.82 | 0.000 | 0.82 | 0 | 43 | 5.50% | 2.7 | 48% | 0.97 | 0.000 | 0.97 | 4.99 | 824 | 1.8E-03 | 1.2E+03 | 259 | 0.9458 | 1.5E-04 | 19 | 0.207 | 0.203 | 4 | 1.216 | 10099 | 6962 | 1400 | 5479 | 0.025% | 1.70 | 0.75 | 0.02% | 0.001066 | 0.34 | 0.079 | 0.797 | 0.17% | 0.0003 |
| 14.436 | 6955.52 | 29.2 | 1.513 | 29.2 | -0.2 | -0.08 | 5.18% | General Fill | 0.057 | 113.8 | 0.83 | 0.000 | 0.83 | 0 | 34 | 5.33% | 2.7 | 48% | 0.98 | 0.000 | 0.98 | 5.04 | 824 | 1.8E-03 | 1.2E+03 | 255 | 0.9447 | 1.5E-04 | 19 | 0.208 | 0.204 | 4 | 1.216 | 10059 | 6934 | 1400 | 5458 | 0.026% | 1.70 | 0.75 | 0.02% | 0.001109 | 0.34 | 0.079 | 0.797 | 0.18% | 0.0003 |
| 14.600 | 6955.36 | 27.8 | 1.299 | 27.9 | -0.2 | -0.08 | 4.66% | General Fill | 0.057 | 113.8 | 0.84 | 0.000 | 0.84 | 0 | 32 | 4.81% | 2.7 | 48% | 0.99 | 0.000 | 0.99 | 5.09 | 720 | 1.8E-03 | 9.2E+02 | 251 | 0.9436 | 2.0E-04 | 19 | 0.208 | 0.204 | 4 | 1.216 | 10020 | 6907 | 1400 | 5438 | 0.041% | 1.70 | 0.75 | 0.02% | 0.002995 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 14.764 | 6955.20 | 27.1 | 1.213 | 27.1 | 0.4 | 0.16 | 4.48% | General Fill | 0.057 | 113.8 | 0.85 | 0.000 | 0.85 | 0 | 31 | 4.62% | 2.7 | 48% | 1.00 | 0.000 | 1.00 | 5.14 | 720 | 1.8E-03 | 9.2E+02 | 247 | 0.9425 | 2.0E-04 | 19 | 0.209 | 0.205 | 4 | 1.217 | 9982 | 6880 | 1400 | 5418 | 0.042% | 1.70 | 0.75 | 0.02% | 0.003048 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 14.928 | 6955.03 | 25.5 | 1.069 | 25.5 | 0.4 | 0.16 | 4.19% | General Fill | 0.057 | 113.8 | 0.85 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|-------|-------|-------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|---|-------|-------|---|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 23.458 | 6946.50 | 44.5 | 0.554 | 44.5 | -0.5 | -0.23 | 1.25% | Coarse Tailings | 0.054 | 108.1 | 1.33 | 0.000 | 1.33 | 0 | 33 | 1.28% | 2.4 | 21% | 1.48 | 0.000 | 1.48 | 7.79 | 705 | 1.7E-03 | 8.3E+02 | 110 | 0.8545 | 3.0E-04 | 0 | 0.228 | 0.227 | 4 | 0.228 | 8500 | 5840 | 1400 | 8500 | 0.092% | 1.79 | 1.00 | 0.01% | 0.001467 | 0.36 | 0.025 | 0.785 | 0.23% | 0.0004 |
| 23.622 | 6946.34 | 43.1 | 0.488 | 43.1 | -0.3 | -0.14 | 1.13% | Coarse Tailings | 0.054 | 108.1 | 1.33 | 0.000 | 1.33 | 0 | 31 | 1.17% | 2.4 | 21% | 1.49 | 0.000 | 1.49 | 7.84 | 705 | 1.7E-03 | 8.3E+02 | 108 | 0.8521 | 3.0E-04 | 0 | 0.229 | 0.228 | 4 | 0.229 | 8479 | 5825 | 1400 | 8479 | 0.092% | 1.79 | 1.00 | 0.01% | 0.001476 | 0.36 | 0.025 | 0.785 | 0.23% | 0.0004 |
| 23.786 | 6946.17 | 41.2 | 0.375 | 41.2 | -0.1 | -0.04 | 0.91% | Coarse Tailings | 0.054 | 108.1 | 1.34 | 0.000 | 1.34 | 0 | 30 | 0.94% | 2.3 | 21% | 1.50 | 0.000 | 1.50 | 7.89 | 705 | 1.7E-03 | 8.3E+02 | 107 | 0.8498 | 3.0E-04 | 0 | 0.229 | 0.228 | 4 | 0.229 | 8459 | 5811 | 1400 | 8459 | 0.093% | 1.79 | 1.00 | 0.01% | 0.001485 | 0.36 | 0.025 | 0.785 | 0.23% | 0.0004 |
| 23.950 | 6946.01 | 42.3 | 0.401 | 42.3 | -0.5 | -0.20 | 0.95% | Coarse Tailings | 0.054 | 108.1 | 1.35 | 0.000 | 1.35 | 0 | 30 | 0.98% | 2.3 | 21% | 1.51 | 0.000 | 1.51 | 7.94 | 705 | 1.7E-03 | 8.3E+02 | 105 | 0.8474 | 3.0E-04 | 0 | 0.229 | 0.228 | 4 | 0.229 | 8438 | 5797 | 1400 | 8438 | 0.093% | 1.79 | 1.00 | 0.01% | 0.001493 | 0.36 | 0.025 | 0.785 | 0.23% | 0.0004 |
| 24.114 | 6945.85 | 39.3 | 0.439 | 39.3 | -0.1 | -0.02 | 1.12% | Coarse Tailings | 0.054 | 108.1 | 1.36 | 0.000 | 1.36 | 0 | 28 | 1.16% | 2.4 | 21% | 1.52 | 0.000 | 1.52 | 7.99 | 705 | 1.7E-03 | 8.3E+02 | 104 | 0.8450 | 3.0E-04 | 0 | 0.230 | 0.229 | 4 | 0.230 | 8418 | 5783 | 1400 | 8418 | 0.094% | 1.79 | 1.00 | 0.01% | 0.001502 | 0.36 | 0.025 | 0.785 | 0.24% | 0.0004 |
| 24.278 | 6945.68 | 43.9 | 0.477 | 43.9 | -0.2 | -0.10 | 1.09% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 31 | 1.12% | 2.4 | 21% | 1.53 | 0.000 | 1.53 | 8.04 | 705 | 1.7E-03 | 8.3E+02 | 102 | 0.8426 | 3.0E-04 | 0 | 0.230 | 0.229 | 4 | 0.230 | 8398 | 5768 | 1400 | 8398 | 0.094% | 1.79 | 1.00 | 0.01% | 0.001510 | 0.36 | 0.025 | 0.785 | 0.24% | 0.0004 |
| 24.442 | 6945.52 | 45.2 | 0.519 | 45.2 | -0.4 | -0.18 | 1.15% | Coarse Tailings | 0.054 | 108.1 | 1.38 | 0.000 | 1.38 | 0 | 32 | 1.18% | 2.4 | 21% | 1.53 | 0.000 | 1.53 | 8.09 | 705 | 1.7E-03 | 8.3E+02 | 101 | 0.8402 | 3.0E-04 | 0 | 0.230 | 0.229 | 4 | 0.230 | 8378 | 5755 | 1400 | 8378 | 0.095% | 1.79 | 1.00 | 0.01% | 0.001517 | 0.36 | 0.025 | 0.785 | 0.24% | 0.0004 |
| 24.606 | 6945.35 | 45.9 | 0.556 | 46.0 | -0.5 | -0.23 | 1.21% | Coarse Tailings | 0.054 | 108.1 | 1.39 | 0.000 | 1.39 | 0 | 32 | 1.25% | 2.4 | 21% | 1.54 | 0.000 | 1.54 | 8.14 | 639 | 1.7E-03 | 6.9E+02 | 99 | 0.8377 | 3.7E-04 | 0 | 0.231 | 0.230 | 4 | 0.231 | 8358 | 5741 | 1400 | 8358 | 0.179% | 1.79 | 1.00 | 0.01% | 0.003020 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 24.770 | 6945.19 | 50.0 | 0.600 | 50.0 | -0.1 | -0.02 | 1.20% | Coarse Tailings | 0.054 | 108.1 | 1.40 | 0.000 | 1.40 | 0 | 35 | 1.23% | 2.3 | 21% | 1.55 | 0.000 | 1.55 | 8.19 | 639 | 1.7E-03 | 6.9E+02 | 98 | 0.8352 | 3.7E-04 | 0 | 0.231 | 0.230 | 4 | 0.231 | 8339 | 5727 | 1400 | 8339 | 0.180% | 1.79 | 1.00 | 0.01% | 0.003034 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 24.934 | 6945.03 | 54.6 | 0.540 | 54.6 | 0.4 | 0.18 | 0.99% | Coarse Tailings | 0.054 | 108.1 | 1.41 | 0.000 | 1.41 | 0 | 38 | 1.02% | 2.3 | 21% | 1.56 | 0.000 | 1.56 | 8.24 | 639 | 1.7E-03 | 6.9E+02 | 96 | 0.8326 | 3.7E-04 | 0 | 0.231 | 0.230 | 4 | 0.231 | 8319 | 5713 | 1400 | 8319 | 0.180% | 1.79 | 1.00 | 0.01% | 0.003048 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 25.098 | 6944.86 | 61.3 | 0.648 | 61.3 | -0.1 | -0.04 | 1.06% | Coarse Tailings | 0.054 | 108.1 | 1.41 | 0.000 | 1.41 | 0 | 42 | 1.08% | 2.2 | 21% | 1.57 | 0.000 | 1.57 | 8.29 | 639 | 1.7E-03 | 6.9E+02 | 95 | 0.8301 | 3.7E-04 | 0 | 0.231 | 0.231 | 4 | 0.231 | 8300 | 5700 | 1400 | 8300 | 0.181% | 1.79 | 1.00 | 0.01% | 0.003061 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 25.262 | 6944.70 | 74.3 | 0.758 | 74.3 | 0.1 | 0.04 | 1.02% | Coarse Tailings | 0.054 | 108.1 | 1.42 | 0.000 | 1.42 | 0 | 51 | 1.04% | 2.2 | 21% | 1.58 | 0.000 | 1.58 | 8.34 | 639 | 1.7E-03 | 6.9E+02 | 94 | 0.8275 | 3.7E-04 | 0 | 0.232 | 0.231 | 4 | 0.232 | 8281 | 5686 | 1400 | 8281 | 0.182% | 1.79 | 1.00 | 0.01% | 0.003074 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 25.426 | 6944.53 | 86.5 | 0.916 | 86.5 | 0.2 | 0.08 | 1.06% | Coarse Tailings | 0.054 | 108.1 | 1.43 | 0.000 | 1.43 | 0 | 59 | 1.08% | 2.1 | 21% | 1.59 | 0.000 | 1.59 | 8.39 | 639 | 1.7E-03 | 6.9E+02 | 92 | 0.8248 | 3.7E-04 | 0 | 0.232 | 0.231 | 4 | 0.232 | 8262 | 5673 | 1400 | 8262 | 0.182% | 1.79 | 1.00 | 0.01% | 0.003085 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 25.590 | 6944.37 | 103.5 | 1.055 | 103.5 | 0.1 | 0.02 | 1.02% | Coarse Tailings | 0.054 | 108.1 | 1.44 | 0.000 | 1.44 | 0 | 71 | 1.03% | 2.0 | 21% | 1.60 | 0.000 | 1.60 | 8.44 | 639 | 1.7E-03 | 6.9E+02 | 91 | 0.8222 | 3.7E-04 | 0 | 0.232 | 0.232 | 4 | 0.232 | 8243 | 5660 | 1400 | 8243 | 0.183% | 1.79 | 1.00 | 0.01% | 0.003096 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 25.754 | 6944.21 | 107.1 | 1.159 | 107.1 | 0.1 | 0.06 | 1.08% | Coarse Tailings | 0.054 | 108.1 | 1.45 | 0.000 | 1.45 | 0 | 73 | 1.10% | 2.0 | 21% | 1.61 | 0.000 | 1.61 | 8.49 | 639 | 1.7E-03 | 6.9E+02 | 90 | 0.8195 | 3.7E-04 | 0 | 0.233 | 0.232 | 4 | 0.233 | 8224 | 5647 | 1400 | 8224 | 0.184% | 1.79 | 1.00 | 0.01% | 0.003106 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 25.918 | 6944.04 | 104.7 | 1.177 | 104.7 | -0.3 | -0.14 | 1.12% | Coarse Tailings | 0.054 | 108.1 | 1.46 | 0.000 | 1.46 | 0 | 71 | 1.14% | 2.1 | 21% | 1.61 | 0.000 | 1.61 | 8.54 | 639 | 1.7E-03 | 6.9E+02 | 89 | 0.8168 | 3.7E-04 | 0 | 0.233 | 0.232 | 4 | 0.233 | 8206 | 5634 | 1400 | 8206 | 0.184% | 1.79 | 1.00 | 0.01% | 0.003116 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.082 | 6943.88 | 101.2 | 1.179 | 101.2 | -0.2 | -0.08 | 1.16% | Coarse Tailings | 0.054 | 108.1 | 1.47 | 0.000 | 1.47 | 0 | 68 | 1.18% | 2.1 | 21% | 1.62 | 0.000 | 1.62 | 8.59 | 639 | 1.7E-03 | 6.9E+02 | 87 | 0.8140 | 3.8E-04 | 0 | 0.233 | 0.233 | 4 | 0.233 | 8187 | 5621 | 1400 | 8187 | 0.185% | 1.79 | 1.00 | 0.01% | 0.003124 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.246 | 6943.71 | 98.4 | 1.211 | 98.4 | -0.1 | -0.06 | 1.23% | Coarse Tailings | 0.054 | 108.1 | 1.48 | 0.000 | 1.48 | 0 | 66 | 1.25% | 2.1 | 21% | 1.63 | 0.000 | 1.63 | 8.64 | 639 | 1.7E-03 | 6.9E+02 | 86 | 0.8112 | 3.8E-04 | 0 | 0.234 | 0.233 | 4 | 0.234 | 8169 | 5608 | 1400 | 8169 | 0.185% | 1.79 | 1.00 | 0.01% | 0.003132 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.410 | 6943.55 | 96.0 | 1.245 | 96.0 | -0.3 | -0.12 | 1.30% | Coarse Tailings | 0.054 | 108.1 | 1.49 | 0.000 | 1.49 | 0 | 64 | 1.32% | 2.1 | 21% | 1.64 | 0.000 | 1.64 | 8.69 | 639 | 1.7E-03 | 6.9E+02 | 85 | 0.8084 | 3.8E-04 | 0 | 0.234 | 0.233 | 4 | 0.234 | 8151 | 5595 | 1400 | 8151 | 0.185% | 1.79 | 1.00 | 0.01% | 0.003139 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.574 | 6943.39 | 90.8 | 1.350 | 90.8 | -0.3 | -0.14 | 1.49% | Coarse Tailings | 0.054 | 108.1 | 1.49 | 0.000 | 1.49 | 0 | 60 | 1.51% | 2.2 | 21% | 1.65 | 0.000 | 1.65 | 8.74 | 639 | 1.7E-03 | 6.9E+02 | 84 | 0.8056 | 3.8E-04 | 0 | 0.234 | 0.234 | 4 | 0.234 | 8133 | 5583 | 1400 | 8133 | 0.186% | 1.79 | 1.00 | 0.01% | 0.003145 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.739 | 6943.22 | 83.7 | 1.366 | 83.7 | -0.4 | -0.16 | 1.63% | Coarse Tailings | 0.054 | 108.1 | 1.50 | 0.000 | 1.50 | 0 | 55 | 1.66% | 2.3 | 21% | 1.66 | 0.000 | 1.66 | 8.79 | 639 | 1.7E-03 | 6.9E+02 | 83 | 0.8027 | 3.8E-04 | 0 | 0.234 | 0.234 | 4 | 0.234 | 8115 | 5570 | 1400 | 8115 | 0.186% | 1.79 | 1.00 | 0.01% | 0.003150 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 26.903 | 6943.06 | 71.4 | 1.393 | 71.4 | -0.2 | -0.08 | 1.95% | Coarse Tailings | 0.054 | 108.1 | 1.51 | 0.000 | 1.51 | 0 | 46 | 1.99% | 2.4 | 21% | 1.67 | 0.000 | 1.67 | 8.84 | 639 | 1.7E-03 | 6.9E+02 | 82 | 0.7998 | 3.8E-04 | 0 | 0.235 | 0.234 | 4 | 0.235 | 8097 | 5558 | 1400 | 8097 | 0.186% | 1.79 | 1.00 | 0.01% | 0.003154 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.067 | 6942.89 | 63.3 | 1.043 | 63.3 | -0.3 | -0.12 | 1.65% | Coarse Tailings | 0.054 | 108.1 | 1.52 | 0.000 | 1.52 | 0 | 41 | 1.69% | 2.4 | 21% | 1.68 | 0.000 | 1.68 | 8.89 | 639 | 1.7E-03 | 6.9E+02 | 81 | 0.7969 | 3.8E-04 | 0 | 0.235 | 0.235 | 4 | 0.235 | 8080 | 5546 | 1400 | 8080 | 0.186% | 1.79 | 1.00 | 0.01% | 0.003158 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.231 | 6942.73 | 67.0 | 1.061 | 67.0 | 0.1 | 0.04 | 1.58% | Coarse Tailings | 0.054 | 108.1 | 1.53 | 0.000 | 1.53 | 0 | 43 | 1.62% | 2.3 | 21% | 1.69 | 0.000 | 1.69 | 8.94 | 639 | 1.7E-03 | 6.9E+02 | 79 | 0.7940 | 3.8E-04 | 0 | 0.235 | 0.235 | 4 | 0.235 | 8062 | 5533 | 1400 | 8062 | 0.187% | 1.79 | 1.00 | 0.01% | 0.003161 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.395 | 6942.57 | 49.0 | 1.302 | 49.0 | 0.1 | 0.02 | 2.66% | Coarse Tailings | 0.054 | 108.1 | 1.54 | 0.000 | 1.54 | 0 | 31 | 2.74% | 2.6 | 21% | 1.69 | 0.000 | 1.69 | 8.99 | 639 | 1.7E-03 | 6.9E+02 | 78 | 0.7910 | 3.8E-04 | 0 | 0.236 | 0.235 | 4 | 0.236 | 8045 | 5521 | 1400 | 8045 | 0.187% | 1.79 | 1.00 | 0.01% | 0.003163 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.559 | 6942.40 | 37.0 | 1.179 | 37.0 | -0.2 | -0.10 | 3.19% | Coarse Tailings | 0.054 | 108.1 | 1.55 | 0.000 | 1.55 | 0 | 23 | 3.33% | 2.7 | 21% | 1.70 | 0.000 | 1.70 | 9.04 | 639 | 1.7E-03 | 6.9E+02 | 77 | 0.7880 | 3.8E-04 | 0 | 0.236 | 0.236 | 4 | 0.236 | 8028 | 5509 | 1400 | 8028 | 0.187% | 1.79 | 1.00 | 0.01% | 0.003164 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.723 | 6942.24 | 34.6 | 1.181 | 34.6 | 0.0 | 0.00 | 3.41% | Coarse Tailings | 0.054 | 108.1 | 1.56 | 0.000 | 1.56 | 0 | 21 | 3.57% | 2.8 | 21% | 1.71 | 0.000 | 1.71 | 9.09 | 639 | 1.7E-03 | 6.9E+02 | 76 | 0.7849 | 3.8E-04 | 0 | 0.236 | 0.236 | 4 | 0.236 | 8011 | 5497 | 1400 | 8011 | 0.187% | 1.79 | 1.00 | 0.01% | 0.003164 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 27.887 | 6942.07 | 64.8 | 1.158 | 64.8 | 0.8 | 0.36 | 1.79% | Coarse Tailings | 0.054 | 108.1 | 1.57 | 0.000 | 1.57 | 0 | 40 | 1.83% | 2.4 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|---|-------|-------|---|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 36.417 | 6933.54 | 31.4 | 1.901 | 31.4 | 4.3 | 1.85 | 6.05% | Coarse Alluvium | 0.056 | 111.0 | 2.03 | 0.000 | 2.03 | 0 | 14 | 6.46% | 3.1 | 36% | 2.18 | 0.000 | 2.18 | 11.74 | 762 | 1.7E-03 | 1.0E+03 | 41 | 0.5843 | 2.5E-04 | 0 | 0.250 | 0.252 | 4 | 0.250 | 7248 | 4964 | 1400 | 7248 | 0.050% | 2.00 | 1.00 | 0.01% | 0.000800 | 0.36 | 0.025 | 0.785 | 0.13% | 0.0002 |
| 36.581 | 6933.38 | 25.8 | 1.845 | 25.8 | 4.4 | 1.89 | 7.14% | Coarse Alluvium | 0.056 | 111.0 | 2.04 | 0.000 | 2.04 | 0 | 12 | 7.75% | 3.2 | 36% | 2.19 | 0.000 | 2.19 | 11.79 | 762 | 1.7E-03 | 1.0E+03 | 40 | 0.5799 | 2.5E-04 | 0 | 0.250 | 0.252 | 4 | 0.250 | 7235 | 4955 | 1400 | 7235 | 0.050% | 2.00 | 1.00 | 0.01% | 0.000791 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 36.745 | 6933.22 | 25.0 | 1.678 | 25.0 | 5.1 | 2.22 | 6.71% | Coarse Alluvium | 0.056 | 111.0 | 2.05 | 0.000 | 2.05 | 0 | 11 | 7.31% | 3.2 | 36% | 2.20 | 0.000 | 2.20 | 11.84 | 762 | 1.7E-03 | 1.0E+03 | 40 | 0.5755 | 2.5E-04 | 0 | 0.250 | 0.252 | 4 | 0.250 | 7223 | 4947 | 1400 | 7223 | 0.049% | 2.00 | 1.00 | 0.01% | 0.000783 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 36.909 | 6933.05 | 26.5 | 1.469 | 26.5 | 5.5 | 2.38 | 5.53% | Coarse Alluvium | 0.056 | 111.0 | 2.06 | 0.000 | 2.06 | 0 | 12 | 6.00% | 3.1 | 36% | 2.21 | 0.000 | 2.21 | 11.89 | 762 | 1.7E-03 | 1.0E+03 | 39 | 0.5711 | 2.5E-04 | 0 | 0.251 | 0.253 | 4 | 0.251 | 7211 | 4938 | 1400 | 7211 | 0.049% | 2.00 | 1.00 | 0.01% | 0.000774 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 37.073 | 6932.89 | 27.8 | 1.554 | 27.8 | 5.7 | 2.46 | 5.58% | Coarse Alluvium | 0.056 | 111.0 | 2.07 | 0.000 | 2.07 | 0 | 12 | 6.03% | 3.1 | 36% | 2.22 | 0.000 | 2.22 | 11.94 | 762 | 1.7E-03 | 1.0E+03 | 39 | 0.5666 | 2.5E-04 | 0 | 0.251 | 0.253 | 4 | 0.251 | 7199 | 4930 | 1400 | 7199 | 0.048% | 2.00 | 1.00 | 0.01% | 0.000765 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 37.237 | 6932.72 | 29.5 | 1.787 | 29.5 | 6.8 | 2.95 | 6.06% | Coarse Alluvium | 0.056 | 111.0 | 2.08 | 0.000 | 2.08 | 0 | 13 | 6.52% | 3.1 | 36% | 2.23 | 0.000 | 2.23 | 11.99 | 762 | 1.7E-03 | 1.0E+03 | 39 | 0.5622 | 2.4E-04 | 0 | 0.251 | 0.253 | 4 | 0.251 | 7187 | 4921 | 1400 | 7187 | 0.048% | 2.00 | 1.00 | 0.01% | 0.000757 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 37.401 | 6932.56 | 34.2 | 1.855 | 34.2 | 7.1 | 3.09 | 5.42% | Coarse Alluvium | 0.056 | 111.0 | 2.08 | 0.000 | 2.08 | 0 | 15 | 5.78% | 3.0 | 36% | 2.24 | 0.000 | 2.24 | 12.04 | 762 | 1.7E-03 | 1.0E+03 | 38 | 0.5577 | 2.4E-04 | 0 | 0.251 | 0.253 | 4 | 0.251 | 7175 | 4913 | 1400 | 7175 | 0.047% | 2.00 | 1.00 | 0.01% | 0.000748 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 37.565 | 6932.40 | 54.5 | 2.149 | 54.4 | 7.8 | 3.40 | 3.94% | Coarse Alluvium | 0.056 | 111.0 | 2.09 | 0.000 | 2.09 | 0 | 25 | 4.10% | 2.8 | 36% | 2.25 | 0.000 | 2.25 | 12.09 | 762 | 1.7E-03 | 1.0E+03 | 38 | 0.5533 | 2.4E-04 | 0 | 0.251 | 0.254 | 4 | 0.251 | 7163 | 4905 | 1400 | 7163 | 0.047% | 2.00 | 1.00 | 0.01% | 0.000739 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 37.729 | 6932.23 | 86.0 | 2.718 | 86.0 | 9.0 | 3.88 | 3.16% | Coarse Alluvium | 0.056 | 111.0 | 2.10 | 0.000 | 2.10 | 0 | 40 | 3.24% | 2.5 | 36% | 2.26 | 0.000 | 2.26 | 12.14 | 762 | 1.7E-03 | 1.0E+03 | 37 | 0.5488 | 2.4E-04 | 0 | 0.252 | 0.254 | 4 | 0.252 | 7151 | 4897 | 1400 | 7151 | 0.047% | 2.00 | 1.00 | 0.01% | 0.000730 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 37.893 | 6932.07 | 83.0 | 2.883 | 82.9 | 8.3 | 3.58 | 3.47% | Coarse Alluvium | 0.056 | 111.0 | 2.11 | 0.000 | 2.11 | 0 | 38 | 3.57% | 2.6 | 36% | 2.27 | 0.000 | 2.27 | 12.19 | 762 | 1.7E-03 | 1.0E+03 | 37 | 0.5443 | 2.4E-04 | 0 | 0.252 | 0.254 | 4 | 0.252 | 7139 | 4888 | 1400 | 7139 | 0.046% | 2.00 | 1.00 | 0.01% | 0.000721 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.057 | 6931.90 | 59.6 | 2.894 | 59.5 | 9.2 | 3.99 | 4.86% | Coarse Alluvium | 0.056 | 111.0 | 2.12 | 0.000 | 2.12 | 0 | 27 | 5.04% | 2.8 | 36% | 2.28 | 0.000 | 2.28 | 12.24 | 762 | 1.7E-03 | 1.0E+03 | 37 | 0.5398 | 2.4E-04 | 0 | 0.252 | 0.255 | 4 | 0.252 | 7127 | 4880 | 1400 | 7127 | 0.046% | 2.00 | 1.00 | 0.01% | 0.000712 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.221 | 6931.74 | 50.5 | 2.527 | 50.4 | 10.8 | 4.68 | 5.01% | Coarse Alluvium | 0.056 | 111.0 | 2.13 | 0.000 | 2.13 | 0 | 23 | 5.23% | 2.9 | 36% | 2.28 | 0.000 | 2.28 | 12.29 | 762 | 1.7E-03 | 1.0E+03 | 36 | 0.5353 | 2.4E-04 | 0 | 0.252 | 0.255 | 4 | 0.252 | 7116 | 4872 | 1400 | 7116 | 0.045% | 2.00 | 1.00 | 0.01% | 0.000703 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.385 | 6931.58 | 30.0 | 1.994 | 29.9 | 10.5 | 4.55 | 6.66% | Coarse Alluvium | 0.056 | 111.0 | 2.14 | 0.000 | 2.14 | 0 | 13 | 7.17% | 3.1 | 36% | 2.29 | 0.000 | 2.29 | 12.34 | 762 | 1.7E-03 | 1.0E+03 | 36 | 0.5308 | 2.4E-04 | 0 | 0.253 | 0.255 | 4 | 0.253 | 7104 | 4864 | 1400 | 7104 | 0.045% | 2.00 | 1.00 | 0.01% | 0.000694 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.549 | 6931.41 | 20.8 | 0.913 | 20.7 | 10.1 | 4.39 | 4.40% | Coarse Alluvium | 0.056 | 111.0 | 2.15 | 0.000 | 2.15 | 0 | 9 | 4.90% | 3.2 | 36% | 2.30 | 0.000 | 2.30 | 12.39 | 762 | 1.7E-03 | 1.0E+03 | 36 | 0.5263 | 2.4E-04 | 0 | 0.253 | 0.255 | 4 | 0.253 | 7093 | 4856 | 1400 | 7093 | 0.044% | 2.00 | 1.00 | 0.01% | 0.000685 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.713 | 6931.25 | 16.1 | 0.467 | 16.1 | 10.6 | 4.58 | 2.90% | Coarse Alluvium | 0.056 | 111.0 | 2.16 | 0.000 | 2.16 | 0 | 6 | 3.34% | 3.2 | 36% | 2.31 | 0.000 | 2.31 | 12.44 | 762 | 1.7E-03 | 1.0E+03 | 35 | 0.5218 | 2.3E-04 | 0 | 0.253 | 0.256 | 4 | 0.253 | 7081 | 4848 | 1400 | 7081 | 0.044% | 2.00 | 1.00 | 0.01% | 0.000676 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 38.877 | 6931.08 | 17.2 | 0.909 | 17.2 | 11.5 | 4.96 | 5.28% | Coarse Alluvium | 0.056 | 111.0 | 2.17 | 0.000 | 2.17 | 0 | 7 | 6.04% | 3.3 | 36% | 2.32 | 0.000 | 2.32 | 12.49 | 762 | 1.7E-03 | 1.0E+03 | 35 | 0.5172 | 2.3E-04 | 0 | 0.253 | 0.256 | 4 | 0.253 | 7070 | 4840 | 1400 | 7070 | 0.043% | 2.00 | 1.00 | 0.01% | 0.000666 | 0.36 | 0.025 | 0.785 | 0.10% | 0.0002 |
| 39.042 | 6930.92 | 25.8 | 0.860 | 25.8 | 10.0 | 4.31 | 3.33% | Coarse Alluvium | 0.056 | 111.0 | 2.18 | 0.000 | 2.18 | 0 | 11 | 3.64% | 3.0 | 36% | 2.33 | 0.000 | 2.33 | 12.54 | 762 | 1.7E-03 | 1.0E+03 | 35 | 0.5127 | 2.3E-04 | 0 | 0.254 | 0.256 | 4 | 0.254 | 7058 | 4832 | 1400 | 7058 | 0.043% | 2.00 | 1.00 | 0.01% | 0.000657 | 0.36 | 0.025 | 0.785 | 0.10% | 0.0002 |
| 39.206 | 6930.75 | 28.7 | 1.144 | 28.7 | 10.8 | 4.66 | 3.98% | Coarse Alluvium | 0.056 | 111.0 | 2.18 | 0.000 | 2.18 | 0 | 12 | 4.31% | 3.0 | 36% | 2.34 | 0.000 | 2.34 | 12.59 | 762 | 1.7E-03 | 1.0E+03 | 34 | 0.5081 | 2.3E-04 | 0 | 0.254 | 0.256 | 4 | 0.254 | 7047 | 4824 | 1400 | 7047 | 0.042% | 2.00 | 1.00 | 0.01% | 0.000648 | 0.36 | 0.025 | 0.785 | 0.10% | 0.0002 |
| 39.370 | 6930.59 | 28.0 | 0.889 | 28.0 | 9.1 | 3.93 | 3.17% | Coarse Alluvium | 0.056 | 111.0 | 2.19 | 0.000 | 2.19 | 0 | 12 | 3.44% | 3.0 | 36% | 2.35 | 0.000 | 2.35 | 12.64 | 762 | 1.7E-03 | 1.0E+03 | 34 | 0.5036 | 2.3E-04 | 0 | 0.254 | 0.257 | 4 | 0.254 | 7036 | 4816 | 1400 | 7036 | 0.042% | 2.00 | 1.00 | 0.01% | 0.000639 | 0.36 | 0.025 | 0.785 | 0.10% | 0.0002 |
| 39.534 | 6930.43 | 17.1 | 0.734 | 17.0 | 9.1 | 3.94 | 4.30% | Coarse Alluvium | 0.056 | 111.0 | 2.20 | 0.000 | 2.20 | 0 | 7 | 4.93% | 3.3 | 36% | 2.36 | 0.000 | 2.36 | 12.69 | 772 | 1.7E-03 | 1.0E+03 | 34 | 0.4990 | 2.2E-04 | 0 | 0.254 | 0.257 | 4 | 0.254 | 7025 | 4808 | 1400 | 7025 | 0.040% | 2.00 | 1.00 | 0.01% | 0.000590 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0002 |
| 39.698 | 6930.26 | 13.8 | 0.491 | 13.8 | 9.9 | 4.29 | 3.55% | Coarse Alluvium | 0.056 | 111.0 | 2.21 | 0.000 | 2.21 | 0 | 5 | 4.23% | 3.3 | 36% | 2.37 | 0.000 | 2.37 | 12.74 | 772 | 1.7E-03 | 1.0E+03 | 34 | 0.4945 | 2.2E-04 | 0 | 0.254 | 0.257 | 4 | 0.254 | 7014 | 4801 | 1400 | 7014 | 0.039% | 2.00 | 1.00 | 0.01% | 0.000581 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0001 |
| 39.862 | 6930.10 | 13.8 | 0.419 | 13.8 | 10.0 | 4.35 | 3.03% | Coarse Alluvium | 0.056 | 111.0 | 2.22 | 0.000 | 2.22 | 0 | 5 | 3.61% | 3.3 | 36% | 2.38 | 0.000 | 2.38 | 12.79 | 772 | 1.7E-03 | 1.0E+03 | 33 | 0.4899 | 2.2E-04 | 0 | 0.255 | 0.257 | 4 | 0.255 | 7003 | 4793 | 1400 | 7003 | 0.039% | 2.00 | 1.00 | 0.01% | 0.000573 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0001 |
| 40.026 | 6929.93 | 14.7 | 0.519 | 14.6 | 10.3 | 4.47 | 3.54% | Coarse Alluvium | 0.056 | 111.0 | 2.23 | 0.000 | 2.23 | 0 | 6 | 4.17% | 3.3 | 36% | 2.39 | 0.000 | 2.39 | 12.84 | 772 | 1.7E-03 | 1.0E+03 | 33 | 0.4854 | 2.2E-04 | 0 | 0.255 | 0.258 | 4 | 0.255 | 6992 | 4785 | 1400 | 6992 | 0.038% | 2.00 | 1.00 | 0.01% | 0.000564 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0001 |
| 40.190 | 6929.77 | 17.1 | 0.478 | 17.0 | 12.7 | 5.51 | 2.80% | Coarse Alluvium | 0.056 | 111.0 | 2.24 | 0.000 | 2.24 | 0 | 7 | 3.22% | 3.2 | 36% | 2.39 | 0.000 | 2.39 | 12.89 | 772 | 1.7E-03 | 1.0E+03 | 33 | 0.4808 | 2.2E-04 | 0 | 0.255 | 0.258 | 4 | 0.255 | 6981 | 4778 | 1400 | 6981 | 0.038% | 2.00 | 1.00 | 0.01% | 0.000556 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0001 |
| 40.354 | 6929.61 | 14.2 | 0.422 | 14.2 | 12.9 | 5.59 | 2.97% | Coarse Alluvium | 0.056 | 111.0 | 2.25 | 0.000 | 2.25 | 0 | 5 | 3.52% | 3.3 | 36% | 2.40 | 0.000 | 2.40 | 12.94 | 772 | 1.7E-03 | 1.0E+03 | 32 | 0.4763 | 2.2E-04 | 0 | 0.255 | 0.258 | 4 | 0.255 | 6970 | 4770 | 1400 | 6970 | 0.037% | 2.00 | 1.00 | 0.01% | 0.000547 | 0.36 | 0.025 | 0.785 | 0.09% | 0.0001 |
| 40.518 | 6929.44 | 13.0 | 0.410 | 12.9 | 12.9 | 5.57 | 3.17% | Coarse Alluvium | 0.056 | 111.0 | 2.26 | 0.000 | 2.26 | 0 | 5 | 3.83% | 3.3 | 36% | 2.41 | 0.000 | 2.41 | 12.99 | 772 | 1.7E-03 | 1.0E+03 | 32 | 0.4717 | 2.2E-04 | 0 | 0.256 | 0.259 | 4 | 0.256 | 6959 | 4763 | 1400 | 6959 | 0.037% | 2.00 | 1.00 | 0.01% | 0.000539 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 40.682 | 6929.28 | 23.9 | 0.504 | 23.8 | 14.0 | 6.06 | 2.11% | Coarse Alluvium | 0.056 | 111.0 | 2.27 | 0.000 | 2.27 | 0 | 10 | 2.33% | 3.0 | 36% | 2.42 | 0.000 | 2.42 | 13.04 | 772 | 1.7E-03 | 1.0E+03 | 32 | 0.4671 | 2.1E-04 | 0 | 0.256 | 0.259 | 4 | 0.256 | 6948 | 4755 | 1400 | 6948 | 0.037% | 2.00 | 1.00 | 0.01% | 0.000530 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 40.846 | 6929.11 | 25.6 | 0.512 | 25.5 | 13.5 | 5.86 | 2.00% | Coarse Alluvium | 0.056 | 111.0 | 2.28 | 0.0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|---|-------|-------|---|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 49.376 | 6920.58 | 47.5 | 1.425 | 47.4 | 7.9 | 3.41 | 3.00% | Coarse Alluvium | 0.056 | 111.0 | 2.77 | 0.000 | 2.77 | 0 | 16 | 3.19% | 2.8 | 36% | 2.92 | 0.000 | 2.92 | 15.69 | 794 | 1.7E-03 | 1.1E+03 | 23 | 0.2427 | 1.3E-04 | 0 | 0.267 | 0.272 | 4 | 0.267 | 6431 | 4395 | 1400 | 6431 | 0.016% | 2.00 | 1.00 | 0.01% | 0.000122 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 49.540 | 6920.42 | 52.9 | 1.319 | 52.8 | 9.3 | 4.05 | 2.49% | Coarse Alluvium | 0.056 | 111.0 | 2.78 | 0.000 | 2.78 | 0 | 18 | 2.63% | 2.8 | 36% | 2.93 | 0.000 | 2.93 | 15.74 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2390 | 1.4E-04 | 0 | 0.267 | 0.272 | 4 | 0.267 | 6423 | 4389 | 1400 | 6423 | 0.018% | 2.00 | 1.00 | 0.01% | 0.000160 | 0.36 | 0.025 | 0.785 | 0.03% | 0.0000 |
| 49.704 | 6920.26 | 79.2 | 1.156 | 79.1 | 11.2 | 4.84 | 1.46% | Coarse Alluvium | 0.056 | 111.0 | 2.79 | 0.000 | 2.79 | 0 | 27 | 1.51% | 2.5 | 36% | 2.94 | 0.000 | 2.94 | 15.79 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2354 | 1.4E-04 | 0 | 0.268 | 0.272 | 4 | 0.268 | 6415 | 4383 | 1400 | 6415 | 0.018% | 2.00 | 1.00 | 0.01% | 0.000154 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 49.868 | 6920.09 | 84.1 | 1.040 | 84.0 | 9.0 | 3.88 | 1.24% | Coarse Alluvium | 0.056 | 111.0 | 2.80 | 0.000 | 2.80 | 0 | 29 | 1.28% | 2.4 | 36% | 2.95 | 0.000 | 2.95 | 15.84 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2318 | 1.3E-04 | 0 | 0.268 | 0.273 | 4 | 0.268 | 6407 | 4378 | 1400 | 6407 | 0.017% | 2.00 | 1.00 | 0.01% | 0.000148 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.032 | 6919.93 | 78.1 | 1.275 | 78.0 | 8.2 | 3.56 | 1.63% | Coarse Alluvium | 0.056 | 111.0 | 2.81 | 0.000 | 2.81 | 0 | 27 | 1.69% | 2.5 | 36% | 2.96 | 0.000 | 2.96 | 15.89 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2282 | 1.3E-04 | 0 | 0.268 | 0.273 | 4 | 0.268 | 6399 | 4372 | 1400 | 6399 | 0.017% | 2.00 | 1.00 | 0.01% | 0.000142 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.196 | 6919.76 | 71.0 | 1.386 | 70.9 | 6.5 | 2.81 | 1.95% | Coarse Alluvium | 0.056 | 111.0 | 2.81 | 0.000 | 2.81 | 0 | 24 | 2.03% | 2.6 | 36% | 2.97 | 0.000 | 2.97 | 15.94 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2246 | 1.3E-04 | 0 | 0.268 | 0.273 | 4 | 0.268 | 6391 | 4366 | 1400 | 6391 | 0.017% | 2.00 | 1.00 | 0.01% | 0.000137 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.360 | 6919.60 | 58.9 | 1.466 | 58.8 | 5.6 | 2.42 | 2.49% | Coarse Alluvium | 0.056 | 111.0 | 2.82 | 0.000 | 2.82 | 0 | 20 | 2.62% | 2.7 | 36% | 2.98 | 0.000 | 2.98 | 15.99 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2211 | 1.3E-04 | 0 | 0.268 | 0.273 | 4 | 0.268 | 6383 | 4361 | 1400 | 6383 | 0.017% | 2.00 | 1.00 | 0.01% | 0.000131 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.524 | 6919.44 | 46.0 | 1.404 | 46.0 | 6.0 | 2.58 | 3.05% | Coarse Alluvium | 0.056 | 111.0 | 2.83 | 0.000 | 2.83 | 0 | 15 | 3.25% | 2.9 | 36% | 2.99 | 0.000 | 2.99 | 16.04 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2176 | 1.3E-04 | 0 | 0.269 | 0.274 | 4 | 0.269 | 6375 | 4355 | 1400 | 6375 | 0.016% | 2.00 | 1.00 | 0.01% | 0.000125 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.688 | 6919.27 | 39.8 | 1.521 | 39.8 | 7.5 | 3.23 | 3.82% | Coarse Alluvium | 0.056 | 111.0 | 2.84 | 0.000 | 2.84 | 0 | 13 | 4.11% | 3.0 | 36% | 3.00 | 0.000 | 3.00 | 16.09 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2141 | 1.3E-04 | 0 | 0.269 | 0.274 | 4 | 0.269 | 6367 | 4350 | 1400 | 6367 | 0.016% | 2.00 | 1.00 | 0.01% | 0.000120 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 50.852 | 6919.11 | 39.8 | 1.421 | 39.8 | 10.9 | 4.74 | 3.57% | Coarse Alluvium | 0.056 | 111.0 | 2.85 | 0.000 | 2.85 | 0 | 13 | 3.84% | 3.0 | 36% | 3.01 | 0.000 | 3.01 | 16.14 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2107 | 1.2E-04 | 0 | 0.269 | 0.274 | 4 | 0.269 | 6359 | 4344 | 1400 | 6359 | 0.016% | 2.00 | 1.00 | 0.01% | 0.000114 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 51.016 | 6918.94 | 38.2 | 1.237 | 38.2 | 10.5 | 4.54 | 3.23% | Coarse Alluvium | 0.056 | 111.0 | 2.86 | 0.000 | 2.86 | 0 | 12 | 3.50% | 3.0 | 36% | 3.02 | 0.000 | 3.02 | 16.19 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2073 | 1.2E-04 | 0 | 0.269 | 0.274 | 4 | 0.269 | 6351 | 4339 | 1400 | 6351 | 0.015% | 2.00 | 1.00 | 0.01% | 0.000109 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 51.180 | 6918.78 | 48.6 | 1.265 | 48.6 | 11.5 | 5.00 | 2.60% | Coarse Alluvium | 0.056 | 111.0 | 2.87 | 0.000 | 2.87 | 0 | 16 | 2.76% | 2.8 | 36% | 3.02 | 0.000 | 3.02 | 16.24 | 757 | 1.7E-03 | 9.9E+02 | 22 | 0.2039 | 1.2E-04 | 0 | 0.269 | 0.274 | 4 | 0.269 | 6343 | 4333 | 1400 | 6343 | 0.015% | 2.00 | 1.00 | 0.01% | 0.000103 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 51.345 | 6918.62 | 38.1 | 1.273 | 38.0 | 10.0 | 4.33 | 3.35% | Coarse Alluvium | 0.056 | 111.0 | 2.88 | 0.000 | 2.88 | 0 | 12 | 3.62% | 3.0 | 36% | 3.03 | 0.000 | 3.03 | 16.29 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.2006 | 1.2E-04 | 0 | 0.269 | 0.275 | 4 | 0.269 | 6335 | 4328 | 1400 | 6335 | 0.015% | 2.00 | 1.00 | 0.01% | 0.000098 | 0.36 | 0.025 | 0.785 | 0.02% | 0.0000 |
| 51.509 | 6918.45 | 24.4 | 1.043 | 24.4 | 9.6 | 4.17 | 4.27% | Coarse Alluvium | 0.056 | 111.0 | 2.89 | 0.000 | 2.89 | 0 | 7 | 4.85% | 3.2 | 36% | 3.04 | 0.000 | 3.04 | 16.34 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1972 | 1.2E-04 | 0 | 0.270 | 0.275 | 4 | 0.270 | 6328 | 4322 | 1400 | 6328 | 0.015% | 2.00 | 1.00 | 0.01% | 0.000093 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 51.673 | 6918.29 | 18.9 | 0.809 | 18.9 | 11.0 | 4.76 | 4.27% | Coarse Alluvium | 0.056 | 111.0 | 2.90 | 0.000 | 2.90 | 0 | 6 | 5.04% | 3.3 | 36% | 3.05 | 0.000 | 3.05 | 16.39 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1939 | 1.2E-04 | 0 | 0.270 | 0.275 | 4 | 0.270 | 6320 | 4317 | 1400 | 6320 | 0.014% | 2.00 | 1.00 | 0.01% | 0.000088 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 51.837 | 6918.12 | 25.2 | 0.801 | 25.1 | 13.9 | 6.04 | 3.18% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 8 | 3.59% | 3.1 | 36% | 3.06 | 0.000 | 3.06 | 16.44 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1907 | 1.2E-04 | 0 | 0.270 | 0.275 | 4 | 0.270 | 6312 | 4312 | 1400 | 6312 | 0.014% | 2.00 | 1.00 | 0.01% | 0.000083 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.001 | 6917.96 | 33.6 | 0.957 | 33.5 | 13.3 | 5.75 | 2.85% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 11 | 3.12% | 3.0 | 36% | 3.07 | 0.000 | 3.07 | 16.49 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1875 | 1.1E-04 | 0 | 0.270 | 0.276 | 4 | 0.270 | 6304 | 4306 | 1400 | 6304 | 0.014% | 2.00 | 1.00 | 0.01% | 0.000078 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.165 | 6917.80 | 27.5 | 0.873 | 27.5 | 12.3 | 5.35 | 3.17% | Coarse Alluvium | 0.056 | 111.0 | 2.92 | 0.000 | 2.92 | 0 | 8 | 3.55% | 3.1 | 36% | 3.08 | 0.000 | 3.08 | 16.54 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1843 | 1.1E-04 | 0 | 0.270 | 0.276 | 4 | 0.270 | 6297 | 4301 | 1400 | 6297 | 0.014% | 2.00 | 1.00 | 0.01% | 0.000073 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.329 | 6917.63 | 28.7 | 0.771 | 28.6 | 14.6 | 6.31 | 2.69% | Coarse Alluvium | 0.056 | 111.0 | 2.93 | 0.000 | 2.93 | 0 | 9 | 2.99% | 3.0 | 36% | 3.09 | 0.000 | 3.09 | 16.59 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1811 | 1.1E-04 | 0 | 0.271 | 0.276 | 4 | 0.271 | 6289 | 4296 | 1400 | 6289 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000068 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.493 | 6917.47 | 40.7 | 0.801 | 40.6 | 14.3 | 6.20 | 1.97% | Coarse Alluvium | 0.056 | 111.0 | 2.94 | 0.000 | 2.94 | 0 | 13 | 2.12% | 2.8 | 36% | 3.10 | 0.000 | 3.10 | 16.64 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1779 | 1.1E-04 | 0 | 0.271 | 0.276 | 4 | 0.271 | 6282 | 4290 | 1400 | 6282 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000063 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.657 | 6917.30 | 42.7 | 0.837 | 42.6 | 13.0 | 5.61 | 1.96% | Coarse Alluvium | 0.056 | 111.0 | 2.95 | 0.000 | 2.95 | 0 | 13 | 2.11% | 2.8 | 36% | 3.11 | 0.000 | 3.11 | 16.69 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1748 | 1.1E-04 | 0 | 0.271 | 0.276 | 4 | 0.271 | 6274 | 4285 | 1400 | 6274 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000058 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.821 | 6917.14 | 49.9 | 1.049 | 49.8 | 13.7 | 5.92 | 2.09% | Coarse Alluvium | 0.056 | 111.0 | 2.96 | 0.000 | 2.96 | 0 | 16 | 2.22% | 2.8 | 36% | 3.12 | 0.000 | 3.12 | 16.74 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1718 | 1.1E-04 | 0 | 0.271 | 0.277 | 4 | 0.271 | 6267 | 4280 | 1400 | 6267 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000053 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 52.985 | 6916.98 | 36.6 | 0.853 | 36.5 | 11.4 | 4.94 | 2.33% | Coarse Alluvium | 0.056 | 111.0 | 2.97 | 0.000 | 2.97 | 0 | 11 | 2.54% | 2.9 | 36% | 3.12 | 0.000 | 3.12 | 16.79 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1687 | 1.0E-04 | 0 | 0.271 | 0.277 | 4 | 0.271 | 6259 | 4275 | 1400 | 6259 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000049 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 53.149 | 6916.81 | 26.6 | 0.712 | 26.5 | 13.3 | 5.78 | 2.68% | Coarse Alluvium | 0.056 | 111.0 | 2.98 | 0.000 | 2.98 | 0 | 8 | 3.02% | 3.1 | 36% | 3.13 | 0.000 | 3.13 | 16.84 | 757 | 1.7E-03 | 9.9E+02 | 21 | 0.1657 | 1.0E-04 | 0 | 0.271 | 0.277 | 4 | 0.271 | 6252 | 4269 | 1400 | 6252 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000044 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 53.313 | 6916.65 | 40.8 | 0.641 | 40.7 | 17.1 | 7.40 | 1.57% | Coarse Alluvium | 0.056 | 111.0 | 2.99 | 0.000 | 2.99 | 0 | 13 | 1.69% | 2.8 | 36% | 3.14 | 0.000 | 3.14 | 16.89 | 757 | 1.7E-03 | 9.9E+02 | 20 | 0.1627 | 1.0E-04 | 0 | 0.272 | 0.277 | 4 | 0.272 | 6244 | 4264 | 1400 | 6244 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000040 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 53.477 | 6916.48 | 22.2 | 0.629 | 22.1 | 14.2 | 6.16 | 2.83% | Coarse Alluvium | 0.056 | 111.0 | 3.00 | 0.000 | 3.00 | 0 | 6 | 3.27% | 3.2 | 36% | 3.15 | 0.000 | 3.15 | 16.94 | 757 | 1.7E-03 | 9.9E+02 | 20 | 0.1597 | 9.9E-05 | 0 | 0.272 | 0.277 | 4 | 0.272 | 6237 | 4259 | 1400 | 6237 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000035 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 53.641 | 6916.32 | 17.0 | 0.519 | 16.8 | 28.5 | 12.36 | 3.05% | Coarse Alluvium | 0.056 | 111.0 | 3.01 | 0.000 | 3.01 | 0 | 5 | 3.71% | 3.3 | 36% | 3.16 | 0.000 | 3.16 | 16.99 | 757 | 1.7E-03 | 9.9E+02 | 20 | 0.1568 | 9.8E-05 | 0 | 0.272 | 0.278 | 4 | 0.272 | 6229 | 4254 | 1400 | 6229 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000031 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.805 | 6916.16 | 16.1 | 0.351 | 15.7 | 49.7 | 21.54 | 2.19% | Coarse Alluvium | 0.056 | 111.0 | 3.0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-------|-------|-------|---------------|-------|-------|------|-------|------|---|---|-------|-----|-----|------|-------|------|--|-------|-----|---------|---------|----|--------|---------|----|-------|-------|---|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 62.499 | 6907.46 | 14.0 | 0.488 | 13.6 | 60.4 | 26.15 | 3.50% | Fine Alluvium | 0.060 | 120.7 | 3.54 | 0.000 | 3.54 | 0 | 3 | 4.68% | 3.5 | 76% | 3.69 | 0.000 | 3.69 | | 19.69 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0398 | 3.4E-05 | 22 | 0.282 | 0.289 | 4 | 2.021 | 5844 | 3986 | 1400 | 2779 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 62.663 | 6907.30 | 14.7 | 0.435 | 14.3 | 62.7 | 27.19 | 2.95% | Fine Alluvium | 0.060 | 120.7 | 3.55 | 0.000 | 3.55 | 0 | 3 | 3.89% | 3.5 | 76% | 3.70 | 0.000 | 3.70 | | 19.74 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0383 | 3.3E-05 | 22 | 0.282 | 0.290 | 4 | 2.021 | 5838 | 3981 | 1400 | 2777 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 62.827 | 6907.13 | 15.5 | 0.494 | 15.1 | 65.6 | 28.43 | 3.18% | Fine Alluvium | 0.060 | 120.7 | 3.56 | 0.000 | 3.56 | 0 | 3 | 4.13% | 3.5 | 76% | 3.71 | 0.000 | 3.71 | | 19.79 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0368 | 3.2E-05 | 22 | 0.282 | 0.290 | 4 | 2.021 | 5831 | 3977 | 1400 | 2774 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 62.991 | 6906.97 | 15.4 | 0.525 | 15.0 | 63.5 | 27.50 | 3.40% | Fine Alluvium | 0.060 | 120.7 | 3.57 | 0.000 | 3.57 | 0 | 3 | 4.42% | 3.5 | 76% | 3.72 | 0.000 | 3.72 | | 19.84 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0354 | 3.1E-05 | 22 | 0.283 | 0.290 | 4 | 2.021 | 5825 | 3972 | 1400 | 2772 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.155 | 6906.81 | 15.9 | 0.619 | 15.4 | 69.0 | 29.89 | 3.91% | Fine Alluvium | 0.060 | 120.7 | 3.58 | 0.000 | 3.58 | 0 | 3 | 5.04% | 3.5 | 76% | 3.73 | 0.000 | 3.73 | | 19.89 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0339 | 3.0E-05 | 22 | 0.283 | 0.290 | 4 | 2.021 | 5818 | 3968 | 1400 | 2770 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.319 | 6906.64 | 14.8 | 0.592 | 14.4 | 67.8 | 29.36 | 3.99% | Fine Alluvium | 0.060 | 120.7 | 3.59 | 0.000 | 3.59 | 0 | 3 | 5.27% | 3.6 | 76% | 3.74 | 0.000 | 3.74 | | 19.94 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0325 | 2.9E-05 | 22 | 0.283 | 0.290 | 4 | 2.022 | 5812 | 3964 | 1400 | 2767 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.483 | 6906.48 | 15.7 | 0.596 | 15.2 | 80.7 | 34.98 | 3.80% | Fine Alluvium | 0.060 | 120.7 | 3.60 | 0.000 | 3.60 | 0 | 3 | 4.94% | 3.5 | 76% | 3.75 | 0.000 | 3.75 | | 19.99 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0311 | 2.7E-05 | 22 | 0.283 | 0.291 | 4 | 2.022 | 5806 | 3959 | 1400 | 2765 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.648 | 6906.31 | 16.7 | 0.386 | 16.1 | 83.7 | 36.26 | 2.32% | Fine Alluvium | 0.060 | 120.7 | 3.61 | 0.000 | 3.61 | 0 | 4 | 2.96% | 3.4 | 76% | 3.76 | 0.000 | 3.76 | | 20.04 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0307 | 2.7E-05 | 22 | 0.283 | 0.291 | 4 | 2.022 | 5800 | 3955 | 1400 | 2763 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.812 | 6906.15 | 16.6 | 0.447 | 16.1 | 86.2 | 37.34 | 2.69% | Fine Alluvium | 0.060 | 120.7 | 3.62 | 0.000 | 3.62 | 0 | 4 | 3.44% | 3.4 | 76% | 3.77 | 0.000 | 3.77 | | 20.09 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0304 | 2.7E-05 | 22 | 0.283 | 0.291 | 4 | 2.022 | 5793 | 3951 | 1400 | 2760 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 63.976 | 6905.98 | 15.6 | 0.557 | 15.0 | 87.4 | 37.89 | 3.57% | Fine Alluvium | 0.060 | 120.7 | 3.63 | 0.000 | 3.63 | 0 | 3 | 4.66% | 3.5 | 76% | 3.78 | 0.000 | 3.78 | | 20.14 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0302 | 2.7E-05 | 22 | 0.284 | 0.291 | 4 | 2.022 | 5787 | 3946 | 1400 | 2758 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.140 | 6905.82 | 15.2 | 0.546 | 14.7 | 83.5 | 36.18 | 3.58% | Fine Alluvium | 0.060 | 120.7 | 3.64 | 0.000 | 3.64 | 0 | 3 | 4.71% | 3.5 | 76% | 3.79 | 0.000 | 3.79 | | 20.19 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0300 | 2.7E-05 | 22 | 0.284 | 0.291 | 4 | 2.022 | 5781 | 3942 | 1400 | 2756 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.304 | 6905.66 | 16.0 | 0.606 | 15.4 | 95.2 | 41.26 | 3.80% | Fine Alluvium | 0.060 | 120.7 | 3.65 | 0.000 | 3.65 | 0 | 3 | 4.92% | 3.5 | 76% | 3.80 | 0.000 | 3.80 | | 20.24 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0297 | 2.7E-05 | 22 | 0.284 | 0.292 | 4 | 2.022 | 5775 | 3938 | 1400 | 2753 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.468 | 6905.49 | 15.3 | 0.515 | 14.8 | 88.8 | 38.50 | 3.36% | Fine Alluvium | 0.060 | 120.7 | 3.66 | 0.000 | 3.66 | 0 | 3 | 4.41% | 3.5 | 76% | 3.81 | 0.000 | 3.81 | | 20.29 | 666 | 1.9E-03 | 8.3E+02 | 18 | 0.0295 | 2.6E-05 | 22 | 0.284 | 0.292 | 4 | 2.022 | 5769 | 3933 | 1400 | 2751 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.632 | 6905.33 | 15.8 | 0.598 | 15.2 | 97.1 | 42.09 | 3.78% | Fine Alluvium | 0.060 | 120.7 | 3.67 | 0.000 | 3.67 | 0 | 3 | 4.91% | 3.5 | 76% | 3.82 | 0.000 | 3.82 | | 20.34 | 740 | 1.9E-03 | 1.0E+03 | 18 | 0.0293 | 2.1E-05 | 22 | 0.284 | 0.292 | 4 | 2.022 | 5762 | 3929 | 1400 | 2749 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.796 | 6905.16 | 17.5 | 0.637 | 16.9 | 96.8 | 41.95 | 3.64% | Fine Alluvium | 0.060 | 120.7 | 3.68 | 0.000 | 3.68 | 0 | 4 | 4.61% | 3.5 | 76% | 3.83 | 0.000 | 3.83 | | 20.39 | 740 | 1.9E-03 | 1.0E+03 | 18 | 0.0290 | 2.1E-05 | 22 | 0.284 | 0.292 | 4 | 2.023 | 5756 | 3925 | 1400 | 2747 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 64.960 | 6905.00 | 18.6 | 0.824 | 18.0 | 95.1 | 41.20 | 4.42% | Fine Alluvium | 0.060 | 120.7 | 3.69 | 0.000 | 3.69 | 0 | 4 | 5.51% | 3.5 | 76% | 3.84 | 0.000 | 3.84 | | 20.44 | 740 | 1.9E-03 | 1.0E+03 | 18 | 0.0288 | 2.1E-05 | 22 | 0.285 | 0.292 | 4 | 2.023 | 5750 | 3921 | 1400 | 2744 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.124 | 6904.84 | 18.8 | 0.855 | 18.3 | 89.3 | 38.68 | 4.54% | Fine Alluvium | 0.060 | 120.7 | 3.70 | 0.000 | 3.70 | 0 | 4 | 5.64% | 3.5 | 76% | 3.85 | 0.000 | 3.85 | | 20.49 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0286 | 2.1E-05 | 22 | 0.285 | 0.293 | 4 | 2.023 | 5744 | 3916 | 1400 | 2742 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.288 | 6904.67 | 19.2 | 0.934 | 18.5 | 109.3 | 47.38 | 4.87% | Fine Alluvium | 0.060 | 120.7 | 3.71 | 0.000 | 3.71 | 0 | 4 | 6.04% | 3.5 | 76% | 3.86 | 0.000 | 3.86 | | 20.54 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0284 | 2.1E-05 | 22 | 0.285 | 0.293 | 4 | 2.023 | 5738 | 3912 | 1400 | 2740 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.452 | 6904.51 | 18.2 | 0.883 | 17.7 | 90.4 | 39.16 | 4.85% | Fine Alluvium | 0.060 | 120.7 | 3.72 | 0.000 | 3.72 | 0 | 4 | 6.09% | 3.5 | 76% | 3.87 | 0.000 | 3.87 | | 20.59 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0281 | 2.1E-05 | 22 | 0.285 | 0.293 | 4 | 2.023 | 5732 | 3908 | 1400 | 2738 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.616 | 6904.34 | 17.3 | 0.887 | 16.8 | 80.4 | 34.84 | 5.14% | Fine Alluvium | 0.060 | 120.7 | 3.73 | 0.000 | 3.73 | 0 | 4 | 6.55% | 3.6 | 76% | 3.88 | 0.000 | 3.88 | | 20.64 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0279 | 2.1E-05 | 22 | 0.285 | 0.293 | 4 | 2.023 | 5726 | 3904 | 1400 | 2735 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.780 | 6904.18 | 17.6 | 0.780 | 17.1 | 83.5 | 36.20 | 4.43% | Fine Alluvium | 0.060 | 120.7 | 3.74 | 0.000 | 3.74 | 0 | 4 | 5.63% | 3.5 | 76% | 3.89 | 0.000 | 3.89 | | 20.69 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0277 | 2.0E-05 | 22 | 0.285 | 0.293 | 4 | 2.023 | 5720 | 3900 | 1400 | 2733 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 65.944 | 6904.02 | 19.6 | 0.823 | 19.0 | 94.1 | 40.79 | 4.20% | Fine Alluvium | 0.060 | 120.7 | 3.75 | 0.000 | 3.75 | 0 | 4 | 5.20% | 3.4 | 76% | 3.90 | 0.000 | 3.90 | | 20.74 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0274 | 2.0E-05 | 22 | 0.286 | 0.294 | 4 | 2.023 | 5714 | 3896 | 1400 | 2731 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 66.108 | 6903.85 | 21.3 | 0.943 | 20.7 | 90.9 | 39.37 | 4.43% | Fine Alluvium | 0.060 | 120.7 | 3.76 | 0.000 | 3.76 | 0 | 5 | 5.38% | 3.4 | 76% | 3.91 | 0.000 | 3.91 | | 20.79 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0272 | 2.0E-05 | 22 | 0.286 | 0.294 | 4 | 2.023 | 5708 | 3891 | 1400 | 2729 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 66.272 | 6903.69 | 21.7 | 0.880 | 21.3 | 66.0 | 28.59 | 4.05% | Fine Alluvium | 0.060 | 120.7 | 3.77 | 0.000 | 3.77 | 0 | 5 | 4.91% | 3.4 | 76% | 3.92 | 0.000 | 3.92 | | 20.84 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0270 | 2.0E-05 | 22 | 0.286 | 0.294 | 4 | 2.023 | 5702 | 3887 | 1400 | 2727 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 66.436 | 6903.52 | 21.3 | 1.246 | 21.0 | 50.9 | 22.07 | 5.84% | Fine Alluvium | 0.060 | 120.7 | 3.78 | 0.000 | 3.78 | 0 | 5 | 7.09% | 3.5 | 76% | 3.93 | 0.000 | 3.93 | | 20.89 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0267 | 2.0E-05 | 22 | 0.286 | 0.294 | 4 | 2.024 | 5696 | 3883 | 1400 | 2724 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 66.600 | 6903.36 | 20.6 | 1.207 | 20.3 | 46.9 | 20.34 | 5.85% | Fine Alluvium | 0.060 | 120.7 | 3.79 | 0.000 | 3.79 | 0 | 4 | 7.17% | 3.5 | 76% | 3.94 | 0.000 | 3.94 | | 20.94 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0265 | 2.0E-05 | 22 | 0.286 | 0.294 | 4 | 2.024 | 5691 | 3879 | 1400 | 2722 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 66.764 | 6903.20 | 20.8 | 1.003 | 20.5 | 52.3 | 22.68 | 4.83% | Fine Alluvium | 0.060 | 120.7 | 3.80 | 0.000 | 3.80 | 0 | 4 | 5.90% | 3.5 | 76% | 3.95 | 0.000 | 3.95 | | 20.99 | 740 | 1.9E-03 | 1.0E+03 | 17 | 0.0263 | 2.0E-05 | 22 | 0.286 | 0.295 | 4 | 2.024 | 5685 | 3875 | 1400 | 2720 | 0.002% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0. |

| Proposed Repository | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer (tsf) | Total Stress at Midpoint of Layer (tsf) | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Effective Stress at Midpoint of Layer (tsf) |
|---------------------|----------------------------|---------------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------------|---|--|--|---|---|
| Erosion Protection | 6972.1 | 6971.3 | 6970.6 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6970.6 | 6970.3 | 6970.0 | 1.1 | 0.057 | 114.7 | 0.155 | 0.124 | 0.00 | 0.00 | 0.155 | 0.124 |
| Mine Spoils | 6970.0 | 6970.0 | 6970.0 | 0.0 | 0.058 | 116.4 | 0.155 | 0.155 | 0.00 | 0.00 | 0.155 | 0.155 |

| | |
|---------|--|
| 6969.96 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6972.06 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 2.50 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|--|
| 0.16 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{\text{repos}}$ (psf) |
| 6954.95 | Elevation of bottom of tailings (ft amsl) |

| UNC-NECR WASTE REPOSITORY SEISMIC SETTLEMENT ANALYSIS - CPT-02 | | | | | | | | | | | |
|---|--|---|--|---|--|------|--|------|--|---|--|
| Data File: | | 13-52118_RP02-BSC-CPT | | | | | | | | | |
| Location: | | UNC-NECR 2013 Mill Site PDS | | Cells Requiring User Input/Manipulation | | | | | | | |
| | | http://projects.mwhglobal.com/.../13-52118_RP02-BSC-CPT.XLS | | | | | | | | | |
| <div>Erosion Protection</div> <div>Cover Soil</div> <div>Mine Spoils</div> <div>Radon Barrier</div> <div>General Fill</div> | | <div>Coarse Tailings</div> <div>Coarse/Fine Tailings</div> <div>Fine Tailings</div> <div>Coarse Alluvium</div> <div>Fine Alluvium</div> | | Idriss and Boulanger (2008) | | | | | | | |
| | | | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | | 0.00 | | Water surface elevation during CPT investigation (ft amsl) | |
| | | | | Earthquake Moment Magnitude, M: | | 5.5 | | 0.00 | | Water surface elevation at t ₀ (ft amsl) | |
| | | | | Magnitude Scaling Factor, MSF: | | 1.69 | | 0.00 | | Water surface elevation at t ₁ (ft amsl) | |
| | | | | Youd, et al (2001) | | | | 1.44 | | Scaling Factor for stress ration, r _m | |
| | | | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | | 0.47 | | Volumetric Strain Ratio for Site-Specific Design Earthquake | |
| | | | | Earthquake Moment Magnitude, M: | | 6.3 | | | | | |
| | | | | Magnitude Scaling Factor, MSF: | | 1.59 | | 8.26 | | Equiv. Number of Uniform Strain Cycles, N | |

| | | | | | | | | | | | | | | | | | | | | | |
|-------|---------|-------|-------|-------|------|------|-------|---------------|-------|-------|------|-------|------|---|------|-------|-----|-----|------|-------|------|
| 0.000 | 6960.00 | 2.6 | 0.014 | 2.6 | 0.8 | 0.35 | 0.55% | Radon Barrier | 0.061 | 122.3 | 0.01 | 0.000 | 0.01 | 0 | 254 | 0.55% | 1.4 | 59% | 0.84 | 0.000 | 0.84 |
| 0.328 | 6959.83 | 8.7 | 0.105 | 8.7 | 1.8 | 0.77 | 1.21% | Radon Barrier | 0.061 | 122.3 | 0.02 | 0.000 | 0.02 | 0 | 432 | 1.21% | 1.5 | 59% | 0.85 | 0.000 | 0.85 |
| 0.492 | 6959.67 | 36.0 | 0.414 | 36.0 | 3.4 | 1.46 | 1.15% | Radon Barrier | 0.061 | 122.3 | 0.03 | 0.000 | 0.03 | 0 | 1196 | 1.15% | 1.3 | 59% | 0.86 | 0.000 | 0.86 |
| 0.656 | 6959.50 | 66.4 | 0.539 | 66.4 | 4.5 | 1.95 | 0.81% | Radon Barrier | 0.061 | 122.3 | 0.04 | 0.000 | 0.04 | 0 | 1655 | 0.81% | 1.2 | 59% | 0.87 | 0.000 | 0.87 |
| 0.820 | 6959.34 | 102.2 | 0.826 | 102.2 | 3.9 | 1.67 | 0.81% | Radon Barrier | 0.061 | 122.3 | 0.05 | 0.000 | 0.05 | 0 | 2036 | 0.81% | 1.1 | 59% | 0.88 | 0.000 | 0.88 |
| 0.984 | 6959.18 | 135.1 | 0.687 | 135.1 | 7.5 | 3.25 | 0.51% | Radon Barrier | 0.061 | 122.3 | 0.06 | 0.000 | 0.06 | 0 | 2434 | 0.51% | 0.9 | 59% | 0.89 | 0.000 | 0.89 |
| 1.148 | 6959.01 | 136.4 | 0.823 | 136.3 | 11.6 | 5.04 | 0.60% | Radon Barrier | 0.061 | 122.3 | 0.07 | 0.000 | 0.07 | 0 | 1941 | 0.60% | 1.0 | 59% | 0.90 | 0.000 | 0.90 |
| 1.312 | 6958.85 | 130.0 | 0.996 | 130.0 | 7.5 | 3.23 | 0.77% | Radon Barrier | 0.061 | 122.3 | 0.08 | 0.000 | 0.08 | 0 | 1619 | 0.77% | 1.1 | 59% | 0.91 | 0.000 | 0.91 |
| 1.476 | 6958.68 | 130.4 | 1.428 | 130.3 | 6.1 | 2.62 | 1.10% | Radon Barrier | 0.061 | 122.3 | 0.09 | 0.000 | 0.09 | 0 | 1443 | 1.10% | 1.3 | 59% | 0.92 | 0.000 | 0.92 |
| 1.640 | 6958.52 | 131.0 | 1.189 | 130.9 | 5.7 | 2.48 | 1.44% | Radon Barrier | 0.061 | 122.3 | 0.10 | 0.000 | 0.10 | 0 | 1304 | 1.44% | 1.4 | 59% | 0.93 | 0.000 | 0.93 |
| 1.804 | 6958.36 | 145.6 | 2.873 | 145.6 | 5.4 | 2.34 | 1.97% | Radon Barrier | 0.061 | 122.3 | 0.11 | 0.000 | 0.11 | 0 | 1319 | 1.97% | 1.6 | 59% | 0.94 | 0.000 | 0.94 |
| 1.968 | 6958.19 | 219.5 | 3.426 | 219.5 | 6.1 | 2.62 | 1.56% | Radon Barrier | 0.061 | 122.3 | 0.12 | 0.000 | 0.12 | 0 | 1822 | 1.56% | 1.4 | 59% | 0.95 | 0.000 | 0.95 |
| 2.133 | 6958.03 | 381.6 | 4.079 | 381.5 | 7.7 | 3.34 | 1.07% | General Fill | 0.057 | 113.8 | 0.13 | 0.000 | 0.13 | 0 | 2941 | 1.07% | 1.2 | 48% | 0.96 | 0.000 | 0.96 |
| 2.297 | 6957.86 | 494.8 | 4.928 | 494.8 | 7.8 | 3.38 | 1.00% | General Fill | 0.057 | 113.8 | 0.14 | 0.000 | 0.14 | 0 | 3558 | 1.00% | 1.2 | 48% | 0.97 | 0.000 | 0.97 |
| 2.461 | 6957.70 | 497.1 | 6.052 | 497.0 | 7.5 | 3.32 | 1.22% | General Fill | 0.057 | 113.8 | 0.15 | 0.000 | 0.15 | 0 | 3349 | 1.22% | 1.3 | 48% | 0.98 | 0.000 | 0.98 |
| 2.625 | 6957.54 | 455.8 | 7.051 | 455.8 | 7.3 | 3.15 | 1.55% | General Fill | 0.057 | 113.8 | 0.16 | 0.000 | 0.16 | 0 | 2889 | 1.55% | 1.4 | 48% | 0.99 | 0.000 | 0.99 |
| 2.789 | 6957.37 | 364.3 | 7.294 | 364.3 | 6.8 | 2.93 | 2.00% | General Fill | 0.057 | 113.8 | 0.17 | 0.000 | 0.17 | 0 | 2180 | 2.00% | 1.5 | 48% | 1.00 | 0.000 | 1.00 |
| 2.953 | 6957.21 | 294.3 | 7.021 | 294.3 | 5.4 | 2.32 | 2.39% | General Fill | 0.057 | 113.8 | 0.18 | 0.000 | 0.18 | 0 | 1668 | 2.39% | 1.6 | 48% | 1.01 | 0.000 | 1.01 |
| 3.117 | 6957.04 | 248.5 | 6.372 | 248.5 | 3.4 | 1.49 | 2.56% | General Fill | 0.057 | 113.8 | 0.19 | 0.000 | 0.19 | 0 | 1337 | 2.57% | 1.7 | 48% | 1.02 | 0.000 | 1.02 |
| 3.281 | 6956.88 | 205.2 | 5.484 | 205.2 | 3.6 | 1.55 | 2.67% | General Fill | 0.057 | 113.8 | 0.20 | 0.000 | 0.20 | 0 | 1051 | 2.68% | 1.7 | 48% | 1.03 | 0.000 | 1.03 |
| 3.445 | 6 | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | | | | |
|------|-----|---------|---------|-----|--------|---------|----|-------|-------|----|-------|-------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 4.40 | 936 | 1.9E-03 | 1.7E+03 | 313 | 0.9574 | 9.5E-05 | 16 | 0.200 | 0.196 | 4 | 0.449 | 10708 | 7390 | 1400 | 6991 | 0.012% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.45 | 936 | 1.9E-03 | 1.7E+03 | 308 | 0.9564 | 9.6E-05 | 16 | 0.201 | 0.197 | 5 | 0.517 | 10657 | 7354 | 1401 | 6957 | 0.013% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.50 | 936 | 1.9E-03 | 1.7E+03 | 303 | 0.9555 | 9.7E-05 | 16 | 0.202 | 0.196 | 6 | 0.584 | 10606 | 7318 | 1402 | 6924 | 0.013% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.55 | 936 | 1.9E-03 | 1.7E+03 | 298 | 0.9546 | 9.8E-05 | 16 | 0.202 | 0.197 | 7 | 0.651 | 10556 | 7283 | 1403 | 6891 | 0.013% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.60 | 936 | 1.9E-03 | 1.7E+03 | 294 | 0.9537 | 9.9E-05 | 16 | 0.203 | 0.198 | 8 | 0.718 | 10506 | 7248 | 1404 | 6859 | 0.014% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.65 | 936 | 1.9E-03 | 1.7E+03 | 289 | 0.9527 | 1.0E-04 | 16 | 0.203 | 0.199 | 9 | 0.785 | 10458 | 7214 | 1405 | 6827 | 0.014% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.70 | 936 | 1.9E-03 | 1.7E+03 | 284 | 0.9517 | 1.0E-04 | 16 | 0.204 | 0.199 | 10 | 0.853 | 10410 | 7180 | 1406 | 6793 | 0.015% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.75 | 936 | 1.9E-03 | 1.7E+03 | 280 | 0.9507 | 1.0E-04 | 16 | 0.204 | 0.200 | 11 | 0.920 | 10363 | 7147 | 1407 | 6765 | 0.015% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.80 | 936 | 1.9E-03 | 1.7E+03 | 275 | 0.9497 | 1.0E-04 | 16 | 0.205 | 0.200 | 12 | 0.987 | 10317 | 7115 | 1408 | 6734 | 0.015% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.85 | 936 | 1.9E-03 | 1.7E+03 | 271 | 0.9487 | 1.0E-04 | 16 | 0.205 | 0.201 | 13 | 1.054 | 10271 | 7083 | 1409 | 6704 | 0.016% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.90 | 936 | 1.9E-03 | 1.7E+03 | 267 | 0.9477 | 1.0E-04 | 16 | 0.206 | 0.202 | 14 | 1.121 | 10226 | 7051 | 1410 | 6675 | 0.016% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 4.95 | 936 | 1.9E-03 | 1.7E+03 | 262 | 0.9466 | 1.1E-04 | 16 | 0.206 | 0.202 | 15 | 1.189 | 10182 | 7020 | 1411 | 6646 | 0.016% | 0.65 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 5.00 | 936 | 1.8E-03 | 1.5E+03 | 258 | 0.9455 | 1.1E-04 | 19 | 0.207 | 0.203 | 16 | 1.415 | 10141 | 6992 | 1412 | 5504 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000000 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 5.05 | 936 | 1.8E-03 | 1.5E+03 | 254 | 0.9445 | 1.2E-04 | 19 | 0.207 | 0.203 | 17 | 4.682 | 10101 | 6963 | 1413 | 5483 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000008 | 0.34 | 0.079 | 0.797 | 0.00% | 0.0000 |
| 5.10 | 936 | 1.8E-03 | 1.5E+03 | 250 | 0.9434 | 1.2E-04 | 19 | 0.208 | 0.204 | 18 | 4.949 | 10062 | 6936 | 1414 | 5463 | 0.020% | 1.70 | 0.75 | 0.02% | 0.000127 | 0.34 | 0.079 | 0.797 | 0.02% | 0.0000 |
| 5.15 | 936 | 1.8E-03 | 1.5E+03 | 246 | 0.9422 | 1.2E-04 | 19 | 0.208 | 0.204 | 19 | 5.216 | 10023 | 6908 | 1415 | 5443 | 0.021% | 1.70 | 0.75 | 0.02% | 0.000210 | 0.34 | 0.079 | 0.797 | 0.03% | 0.0001 |
| | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|----|-------|-------|----|--------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 10.663 | 6949.50 | 74.8 | 0.915 | 74.8 | 2.5 | 1.10 | 1.22% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 121 | 1.23% | 1.9 | 48% | 1.45 | 0.000 | 1.45 | 7.60 | 557 | 1.8E-03 | 5.5E+02 | 116 | 0.8629 | 4.4E-04 | 19 | 0.227 | 0.226 | 68 | 18.299 | 8579 | 5895 | 1464 | 4713 | 0.345% | 1.70 | 0.75 | 0.02% | 0.023134 | 0.34 | 0.079 | 0.797 | 3.69% | 0.0060 |
| 10.827 | 6949.33 | 58.3 | 0.502 | 58.3 | 2.1 | 0.91 | 0.86% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 92 | 0.87% | 1.9 | 48% | 1.46 | 0.000 | 1.46 | 7.65 | 557 | 1.8E-03 | 5.5E+02 | 114 | 0.8607 | 4.5E-04 | 19 | 0.227 | 0.226 | 69 | 18.566 | 8556 | 5879 | 1465 | 4702 | 0.347% | 1.70 | 0.75 | 0.02% | 0.023273 | 0.34 | 0.079 | 0.797 | 3.71% | 0.0061 |
| 10.991 | 6949.17 | 43.9 | 0.468 | 43.9 | 2.1 | 0.89 | 1.07% | General Fill | 0.057 | 113.8 | 0.63 | 0.000 | 0.63 | 0 | 68 | 1.08% | 2.1 | 48% | 1.47 | 0.000 | 1.47 | 7.70 | 557 | 1.8E-03 | 5.5E+02 | 113 | 0.8585 | 4.5E-04 | 19 | 0.228 | 0.227 | 70 | 18.833 | 8534 | 5863 | 1466 | 4691 | 0.350% | 1.70 | 0.75 | 0.02% | 0.023408 | 0.34 | 0.079 | 0.797 | 3.73% | 0.0061 |
| 11.155 | 6949.01 | 30.7 | 0.504 | 30.7 | 1.2 | 0.51 | 1.64% | General Fill | 0.057 | 113.8 | 0.64 | 0.000 | 0.64 | 0 | 47 | 1.68% | 2.3 | 48% | 1.48 | 0.000 | 1.48 | 7.75 | 557 | 1.8E-03 | 5.5E+02 | 111 | 0.8562 | 4.5E-04 | 19 | 0.228 | 0.227 | 71 | 19.100 | 8512 | 5848 | 1467 | 4680 | 0.353% | 1.70 | 0.75 | 0.02% | 0.023541 | 0.34 | 0.079 | 0.797 | 3.75% | 0.0062 |
| 11.319 | 6948.84 | 22.6 | 0.588 | 22.6 | 1.1 | 0.47 | 2.61% | General Fill | 0.057 | 113.8 | 0.65 | 0.000 | 0.65 | 0 | 34 | 2.68% | 2.5 | 48% | 1.49 | 0.000 | 1.49 | 7.80 | 557 | 1.8E-03 | 5.5E+02 | 109 | 0.8539 | 4.5E-04 | 19 | 0.228 | 0.227 | 72 | 19.367 | 8490 | 5833 | 1468 | 4669 | 0.355% | 1.70 | 0.75 | 0.02% | 0.023669 | 0.34 | 0.079 | 0.797 | 3.77% | 0.0062 |
| 11.483 | 6948.68 | 17.4 | 0.639 | 17.4 | 1.3 | 0.57 | 3.67% | General Fill | 0.057 | 113.8 | 0.66 | 0.000 | 0.66 | 0 | 25 | 3.82% | 2.7 | 48% | 1.49 | 0.000 | 1.49 | 7.85 | 557 | 1.8E-03 | 5.5E+02 | 108 | 0.8516 | 4.5E-04 | 19 | 0.229 | 0.228 | 73 | 19.634 | 8468 | 5817 | 1469 | 4658 | 0.357% | 1.70 | 0.75 | 0.02% | 0.023794 | 0.34 | 0.079 | 0.797 | 3.79% | 0.0062 |
| 11.647 | 6948.51 | 19.0 | 0.599 | 19.0 | 1.4 | 0.59 | 3.15% | General Fill | 0.057 | 113.8 | 0.67 | 0.000 | 0.67 | 0 | 27 | 3.27% | 2.7 | 48% | 1.50 | 0.000 | 1.50 | 7.90 | 557 | 1.8E-03 | 5.5E+02 | 106 | 0.8492 | 4.5E-04 | 19 | 0.229 | 0.228 | 74 | 19.901 | 8446 | 5802 | 1470 | 4647 | 0.360% | 1.70 | 0.75 | 0.02% | 0.023916 | 0.34 | 0.079 | 0.797 | 3.81% | 0.0063 |
| 11.811 | 6948.35 | 40.7 | 0.487 | 40.7 | 1.8 | 0.79 | 1.20% | General Fill | 0.057 | 113.8 | 0.68 | 0.000 | 0.68 | 0 | 59 | 1.22% | 2.1 | 48% | 1.51 | 0.000 | 1.51 | 7.95 | 557 | 1.8E-03 | 5.5E+02 | 105 | 0.8468 | 4.6E-04 | 19 | 0.229 | 0.228 | 75 | 20.168 | 8425 | 5787 | 1471 | 4636 | 0.362% | 1.70 | 0.75 | 0.02% | 0.024034 | 0.34 | 0.079 | 0.797 | 3.83% | 0.0063 |
| 11.975 | 6948.19 | 49.0 | 0.546 | 49.0 | 2.3 | 0.98 | 1.11% | General Fill | 0.057 | 113.8 | 0.69 | 0.000 | 0.69 | 0 | 70 | 1.13% | 2.1 | 48% | 1.52 | 0.000 | 1.52 | 8.00 | 557 | 1.8E-03 | 5.5E+02 | 103 | 0.8444 | 4.6E-04 | 19 | 0.230 | 0.229 | 76 | 20.434 | 8404 | 5772 | 1472 | 4626 | 0.364% | 1.70 | 0.75 | 0.02% | 0.024148 | 0.34 | 0.079 | 0.797 | 3.85% | 0.0063 |
| 12.139 | 6948.02 | 41.0 | 0.743 | 41.0 | 2.0 | 0.85 | 1.81% | General Fill | 0.057 | 113.8 | 0.70 | 0.000 | 0.70 | 0 | 58 | 1.84% | 2.3 | 48% | 1.53 | 0.000 | 1.53 | 8.05 | 557 | 1.8E-03 | 5.5E+02 | 102 | 0.8420 | 4.6E-04 | 19 | 0.230 | 0.229 | 77 | 20.701 | 8383 | 5758 | 1473 | 4615 | 0.366% | 1.70 | 0.75 | 0.02% | 0.024258 | 0.34 | 0.079 | 0.797 | 3.87% | 0.0063 |
| 12.303 | 6947.86 | 20.4 | 0.871 | 20.4 | 1.8 | 0.77 | 4.27% | General Fill | 0.057 | 113.8 | 0.71 | 0.000 | 0.71 | 0 | 28 | 4.42% | 2.8 | 48% | 1.54 | 0.000 | 1.54 | 8.10 | 557 | 1.8E-03 | 5.5E+02 | 100 | 0.8395 | 4.6E-04 | 19 | 0.230 | 0.230 | 78 | 20.968 | 8362 | 5743 | 1474 | 4605 | 0.368% | 1.70 | 0.75 | 0.02% | 0.024364 | 0.34 | 0.079 | 0.797 | 3.88% | 0.0064 |
| 12.467 | 6947.69 | 14.9 | 0.663 | 14.9 | 1.8 | 0.79 | 4.44% | General Fill | 0.057 | 113.8 | 0.72 | 0.000 | 0.72 | 0 | 20 | 4.67% | 2.9 | 48% | 1.55 | 0.000 | 1.55 | 8.15 | 557 | 1.8E-03 | 5.5E+02 | 99 | 0.8370 | 4.6E-04 | 19 | 0.231 | 0.230 | 79 | 21.235 | 8341 | 5729 | 1475 | 4594 | 0.370% | 1.70 | 0.75 | 0.02% | 0.024466 | 0.34 | 0.079 | 0.797 | 3.90% | 0.0064 |
| 12.631 | 6947.53 | 12.1 | 0.383 | 12.1 | 1.9 | 0.81 | 3.16% | General Fill | 0.057 | 113.8 | 0.73 | 0.000 | 0.73 | 0 | 16 | 3.36% | 2.9 | 48% | 1.56 | 0.000 | 1.56 | 8.20 | 557 | 1.8E-03 | 5.5E+02 | 98 | 0.8345 | 4.6E-04 | 19 | 0.231 | 0.230 | 80 | 21.502 | 8321 | 5714 | 1476 | 4584 | 0.372% | 1.70 | 0.75 | 0.02% | 0.024565 | 0.34 | 0.079 | 0.797 | 3.92% | 0.0064 |
| 12.795 | 6947.36 | 12.4 | 0.214 | 12.4 | 2.0 | 0.88 | 1.73% | General Fill | 0.057 | 113.8 | 0.74 | 0.000 | 0.74 | 0 | 16 | 1.84% | 2.7 | 48% | 1.57 | 0.000 | 1.57 | 8.25 | 557 | 1.8E-03 | 5.5E+02 | 96 | 0.8320 | 4.6E-04 | 19 | 0.231 | 0.231 | 81 | 21.769 | 8300 | 5700 | 1477 | 4574 | 0.374% | 1.70 | 0.75 | 0.02% | 0.024659 | 0.34 | 0.079 | 0.797 | 3.93% | 0.0064 |
| 12.959 | 6947.20 | 12.5 | 0.185 | 12.5 | 1.9 | 0.83 | 1.48% | Fine Tailings | 0.054 | 107.6 | 0.75 | 0.000 | 0.75 | 0 | 16 | 1.57% | 2.7 | 83% | 1.58 | 0.000 | 1.58 | 8.30 | 557 | 1.7E-03 | 5.2E+02 | 95 | 0.8294 | 4.9E-04 | 43 | 0.232 | 0.231 | 82 | 82.000 | 8281 | 5687 | 1478 | 1478 | 0.101% | 0.90 | 0.75 | 0.06% | 0.002600 | 0.25 | 0.323 | 0.851 | 0.44% | 0.0007 |
| 13.123 | 6947.04 | 12.6 | 0.173 | 12.6 | 2.3 | 0.98 | 1.38% | Fine Tailings | 0.054 | 107.6 | 0.75 | 0.000 | 0.75 | 0 | 16 | 1.46% | 2.7 | 83% | 1.59 | 0.000 | 1.59 | 8.35 | 557 | 1.7E-03 | 5.2E+02 | 93 | 0.8268 | 4.9E-04 | 43 | 0.232 | 0.231 | 83 | 83.000 | 8262 | 5673 | 1479 | 1479 | 0.102% | 0.90 | 0.75 | 0.06% | 0.002630 | 0.25 | 0.323 | 0.851 | 0.45% | 0.0007 |
| 13.287 | 6946.87 | 12.3 | 0.189 | 12.3 | 2.3 | 0.98 | 1.53% | Fine Tailings | 0.054 | 107.6 | 0.76 | 0.000 | 0.76 | 0 | 15 | 1.64% | 2.7 | 83% | 1.60 | 0.000 | 1.60 | 8.40 | 557 | 1.7E-03 | 5.2E+02 | 92 | 0.8242 | 4.9E-04 | 43 | 0.232 | 0.232 | 84 | 84.000 | 8244 | 5660 | 1480 | 1480 | 0.102% | 0.90 | 0.75 | 0.06% | 0.002653 | 0.25 | 0.323 | 0.851 | 0.45% | 0.0007 |
| 13.451 | 6946.71 | 12.4 | 0.213 | 12.4 | 2.2 | 0.96 | 1.72% | Fine Tailings | 0.054 | 107.6 | 0.77 | 0.000 | 0.77 | 0 | 15 | 1.83% | 2.7 | 83% | 1.60 | 0.000 | 1.60 | 8.45 | 557 | 1.7E-03 | 5.2E+02 | 91 | 0.8215 | 5.0E-04 | 43 | 0.233 | 0.232 | 85 | 85.000 | 8225 | 5647 | 1481 | 1481 | 0.103% | 0.90 | 0.75 | 0.06% | 0.002674 | 0.25 | 0.323 | 0.851 | 0.46% | 0.0007 |
| 13.615 | 6946.54 | 13.0 | 0.203 | 13.0 | 2.3 | 1.00 | 1.56% | Fine Tailings | 0.054 | 107.6 | 0.78 | 0.000 | 0.78 | 0 | 16 | 1.66% | 2.7 | 83% | 1.61 | 0.000 | 1.61 | 8.50 | 557 | 1.7E-03 | 5.2E+02 | 90 | 0.8188 | 5.0E-04 | 43 | 0.233 | 0.232 | 86 | 86.000 | 8206 | 5634 | 1482 | 1482 | 0.103% | 0.90 | 0.75 | 0.06% | 0.002695 | 0.25 | 0.323 | 0.851 | 0.46% | 0.0008 |
| 13.779 | 6946.38 | 12.0 | 0.140 | 12.0 | 2.4 | 1.06 | 1.17% | Fine Tailings | 0.054 | 107.6 | 0.79 | 0.000 | 0.79 | 0 | 14 | 1.25% | 2.7 | 83% | 1.62 | 0.000 | 1.62 | 8.55 | 557 | 1.7E-03 | 5.2E+02 | 88 | 0.8161 | 5.0E-04 | 43 | 0.233 | 0.233 | 87 | 87.000 | 8188 | 5621 | 1483 | 1483 | 0.104% | 0.90 | 0.75 | 0.06% | 0.002716 | 0.25 | 0.323 | 0.851 | 0.46% | 0.0008 |
| 13.943 | 6946.22 | 12.7 | 0.238 | 12.7 | 2.4 | 1.04 | 1.87% | Fine Tailings | 0.054 | 107.6 | 0.80 | 0.000 | 0.80 | 0 | 15 | 2.00% | 2.8 | 83% | 1.63 | 0.000 | 1.63 | 8.60 | 557 | 1.7E-03 | 5.2E+02 | 87 | 0.8133 | 5.0E-04 | 43 | 0.233 | 0.233 | 88 | 88.000 | 8170 | 5609 | 1484 | 1484 | 0.104% | 0.90 | 0.75 | 0.06% | 0.002736 | 0.25 | 0.323 | 0.851 | 0.47% | 0.0008 |
| 14.107 | 6946.05 | 23.4 | 0.375 | 23.4 | 2.8 | 1.22 | 1.60% | Fine Tailings | 0.054 | 107.6 | 0.81 | 0.000 | 0.81 | 0 | 28 | 1.66% | 2.5 | 83% | 1.64 | 0.000 | 1.64 | 8.65 | 557 | 1.7E-03 | 5.2E+02 | 86 | 0.8105 | 5.0E-04 | 43 | 0.234 | 0.233 | 89 | 89.000 | 8152 | 5596 | 1485 | 1485 | 0.104% | 0.90 | 0.75 | 0.06% | 0.002755 | 0.25 | 0.323 | 0.851 | 0.47% | 0.0008 |
| 14.271 | 6945.89 | 57.4 | 0.484 | 57.4 | 7.2 | 3.13 | 0.84% | Fine Tailings | 0.054 | 107.6 | 0.82 | 0.000 | 0.82 | 0 | 69 | 0.86% | 2.0 | 83% | 1.65 | 0.000 | 1.65 | 8.70 | 557 | 1.7E-03 | 5.2E+02 | 85 | 0.8077 | 5.0E-04 | 43 | 0.234 | 0.234 | 90 | 90.000 | 8134 | 5584 | 1486 | 1486 | 0.105% | 0.90 | 0.75 | 0.06% | 0.002774 | 0.25 | 0.323 | 0.851 | 0.47% | 0.0008 |
| 14.436 | 6945.72 | 78.4 | 0.581 | 78.3 | 5.7 | 2.46 | 0.74% | Fine Tailings | 0.054 | 107.6 | 0.82 | 0.000 | 0.82 | 0 | 94 | 0.75% | 1.9 | 83% | 1.66 | 0.000 | 1.66 | 8.75 | 557 | 1.7E-03 | 5.2E+02 | 84 | 0.8049 | 5.0E-04 | 43 | 0.234 | 0.234 | 91 | 91.000 | 8116 | 5571 | 1487 | 1487 | 0.105% | 0.90 | 0.75 | 0.06% | 0.002792 | 0.25 | 0.323 | 0.851 | 0.48% | 0.0008 |
| 14.600 | 6945.56 | 84.1 | 0.551 | 84.1 | 5.5 | 2.40 | 0.66% | Fine Tailings | 0.054 | 107.6 | 0.83 | 0.000 | 0.83 | 0 | 100 | 0.66% | 1.8 | 83% | 1.67 | 0.000 | 1.67 | 8.80 | 998 | 1.7E-03 | 1.7E+03 | 82 | 0.8020 | 1.6E-04 | 43 | 0.235 | 0.234 | 92 | 92.000 | 8099 | 5559 | 1488 | 1488 | 0.020% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 14.764 | 6945.40 | 84.2 | 0.709 | 84.2 | 5.0 | 2.18 | 0.84% | Fine Tailings | 0.054 | 107.6 | 0.84 | 0.000 | 0.84 | 0 | 99 | 0.85% | 1.9 | 83% | 1.68 | 0.000 | 1.68 | 8.85 | 998 | 1.7E-03 | 1.7E+03 | 81 | 0.7991 | 1.6E-04 | 43 | 0.235 | 0.235 | 93 | 93.000 | 8081 | 5547 | 1489 | 1489 | 0.020% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 14.928 | 6945.23 | 81.7 | 0.753 | 81.7 | 4.7 | 2.02 | 0.92% | Fine Tailings | 0.054 | 107.6 | 0.85 | 0.000 | 0.85 | 0 | 95 | 0.93% | 1.9 | 83% | 1.68 | 0.000 | 1.68 | 8.90 | 998 | 1.7E-03 | 1.7E+03 | 80 | 0.7962 | 1.6E-04 | 43 | 0.235 | 0.235 | 94 | 94.000 | 8064 | 5534 | 1490 | 1490 | 0.020% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 15.092 | 6945.07 | 69.2 | 0.752 | 69.2 | 4.5 | 1.93 | 1.09% | Coarse Alluvium | 0.056 | 111.0 | 0.86 | 0.000 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|--------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 23.786 | 6936.37 | 63.8 | 0.763 | 63.8 | 3.1 | 1.36 | 1.20% | Coarse Alluvium | 0.056 | 111.0 | 1.34 | 0.000 | 1.34 | 0 | 47 | 1.22% | 2.2 | 36% | 2.18 | 0.000 | 2.18 | 11.60 | 894 | 1.7E-03 | 1.4E+03 | 42 | 0.5963 | 1.8E-04 | 0 | 0.250 | 0.252 | 148 | 0.250 | 7260 | 4972 | 1544 | 7260 | 0.029% | 2.00 | 1.00 | 0.01% | 0.000372 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 23.950 | 6936.21 | 60.6 | 0.864 | 60.6 | 3.0 | 1.28 | 1.43% | Coarse Alluvium | 0.056 | 111.0 | 1.35 | 0.000 | 1.35 | 0 | 44 | 1.46% | 2.3 | 36% | 2.19 | 0.000 | 2.19 | 11.65 | 894 | 1.7E-03 | 1.4E+03 | 41 | 0.5919 | 1.8E-04 | 0 | 0.250 | 0.252 | 149 | 0.250 | 7247 | 4964 | 1545 | 7247 | 0.028% | 2.00 | 1.00 | 0.01% | 0.000368 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 24.114 | 6936.05 | 56.6 | 0.899 | 56.6 | 3.1 | 1.36 | 1.59% | Coarse Alluvium | 0.056 | 111.0 | 1.36 | 0.000 | 1.36 | 0 | 41 | 1.63% | 2.3 | 36% | 2.19 | 0.000 | 2.19 | 11.70 | 894 | 1.7E-03 | 1.4E+03 | 41 | 0.5875 | 1.8E-04 | 0 | 0.250 | 0.252 | 150 | 0.250 | 7235 | 4955 | 1546 | 7235 | 0.028% | 2.00 | 1.00 | 0.01% | 0.000365 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 24.278 | 6935.88 | 58.4 | 0.463 | 58.4 | 3.0 | 1.30 | 0.79% | Coarse Alluvium | 0.056 | 111.0 | 1.37 | 0.000 | 1.37 | 0 | 42 | 0.81% | 2.2 | 36% | 2.20 | 0.000 | 2.20 | 11.75 | 894 | 1.7E-03 | 1.4E+03 | 40 | 0.5832 | 1.8E-04 | 0 | 0.250 | 0.252 | 151 | 0.250 | 7223 | 4946 | 1547 | 7223 | 0.028% | 2.00 | 1.00 | 0.01% | 0.000361 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 24.442 | 6935.72 | 56.5 | 0.419 | 56.5 | 2.9 | 1.24 | 0.74% | Coarse Alluvium | 0.056 | 111.0 | 1.38 | 0.000 | 1.38 | 0 | 40 | 0.76% | 2.2 | 36% | 2.21 | 0.000 | 2.21 | 11.80 | 894 | 1.7E-03 | 1.4E+03 | 40 | 0.5788 | 1.8E-04 | 0 | 0.251 | 0.253 | 152 | 0.251 | 7210 | 4938 | 1548 | 7210 | 0.028% | 2.00 | 1.00 | 0.01% | 0.000358 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 24.606 | 6935.55 | 59.6 | 0.491 | 59.6 | 3.4 | 1.49 | 0.82% | Coarse Alluvium | 0.056 | 111.0 | 1.39 | 0.000 | 1.39 | 0 | 42 | 0.84% | 2.2 | 36% | 2.22 | 0.000 | 2.22 | 11.85 | 864 | 1.7E-03 | 1.3E+03 | 40 | 0.5744 | 1.9E-04 | 0 | 0.251 | 0.253 | 153 | 0.251 | 7198 | 4929 | 1549 | 7198 | 0.031% | 2.00 | 1.00 | 0.01% | 0.000420 | 0.36 | 0.025 | 0.785 | 0.07% | 0.0001 |
| 24.770 | 6935.39 | 59.6 | 0.584 | 59.6 | 3.5 | 1.50 | 0.98% | Coarse Alluvium | 0.056 | 111.0 | 1.40 | 0.000 | 1.40 | 0 | 42 | 1.00% | 2.2 | 36% | 2.23 | 0.000 | 2.23 | 11.90 | 864 | 1.7E-03 | 1.3E+03 | 39 | 0.5700 | 1.9E-04 | 0 | 0.251 | 0.253 | 154 | 0.251 | 7186 | 4921 | 1550 | 7186 | 0.031% | 2.00 | 1.00 | 0.01% | 0.000416 | 0.36 | 0.025 | 0.785 | 0.07% | 0.0001 |
| 24.934 | 6935.23 | 48.5 | 0.515 | 48.4 | 3.4 | 1.46 | 1.06% | Coarse Alluvium | 0.056 | 111.0 | 1.41 | 0.000 | 1.41 | 0 | 33 | 1.09% | 2.3 | 36% | 2.24 | 0.000 | 2.24 | 11.95 | 864 | 1.7E-03 | 1.3E+03 | 39 | 0.5655 | 1.9E-04 | 0 | 0.251 | 0.253 | 155 | 0.251 | 7174 | 4913 | 1551 | 7174 | 0.031% | 2.00 | 1.00 | 0.01% | 0.000412 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 25.098 | 6935.06 | 46.2 | 0.732 | 46.2 | 3.4 | 1.46 | 1.59% | Coarse Alluvium | 0.056 | 111.0 | 1.42 | 0.000 | 1.42 | 0 | 32 | 1.64% | 2.4 | 36% | 2.25 | 0.000 | 2.25 | 12.00 | 864 | 1.7E-03 | 1.3E+03 | 38 | 0.5611 | 1.9E-04 | 0 | 0.251 | 0.254 | 156 | 0.251 | 7162 | 4904 | 1552 | 7162 | 0.030% | 2.00 | 1.00 | 0.01% | 0.000407 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 25.262 | 6934.90 | 54.3 | 0.862 | 54.3 | 2.5 | 1.10 | 1.59% | Coarse Alluvium | 0.056 | 111.0 | 1.42 | 0.000 | 1.42 | 0 | 37 | 1.63% | 2.4 | 36% | 2.26 | 0.000 | 2.26 | 12.05 | 864 | 1.7E-03 | 1.3E+03 | 38 | 0.5566 | 1.9E-04 | 0 | 0.252 | 0.254 | 157 | 0.252 | 7150 | 4896 | 1553 | 7150 | 0.030% | 2.00 | 1.00 | 0.01% | 0.000403 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 25.426 | 6934.73 | 57.0 | 0.957 | 57.0 | 2.9 | 1.26 | 1.68% | Coarse Alluvium | 0.056 | 111.0 | 1.43 | 0.000 | 1.43 | 0 | 39 | 1.72% | 2.4 | 36% | 2.27 | 0.000 | 2.27 | 12.10 | 864 | 1.7E-03 | 1.3E+03 | 38 | 0.5522 | 1.9E-04 | 0 | 0.252 | 0.254 | 158 | 0.252 | 7139 | 4888 | 1554 | 7139 | 0.030% | 2.00 | 1.00 | 0.01% | 0.000398 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 25.590 | 6934.57 | 60.2 | 0.831 | 60.2 | 2.5 | 1.08 | 1.38% | Coarse Alluvium | 0.056 | 111.0 | 1.44 | 0.000 | 1.44 | 0 | 41 | 1.41% | 2.3 | 36% | 2.28 | 0.000 | 2.28 | 12.15 | 864 | 1.7E-03 | 1.3E+03 | 37 | 0.5477 | 1.9E-04 | 0 | 0.252 | 0.255 | 159 | 0.252 | 7127 | 4880 | 1555 | 7127 | 0.030% | 2.00 | 1.00 | 0.01% | 0.000394 | 0.36 | 0.025 | 0.785 | 0.06% | 0.0001 |
| 25.754 | 6934.41 | 61.6 | 0.658 | 61.6 | 2.5 | 1.10 | 1.07% | Fine Alluvium | 0.060 | 120.7 | 1.45 | 0.000 | 1.45 | 0 | 41 | 1.09% | 2.2 | 76% | 2.29 | 0.000 | 2.29 | 12.20 | 864 | 1.9E-03 | 1.4E+03 | 37 | 0.5432 | 1.7E-04 | 22 | 0.252 | 0.255 | 160 | 74.803 | 7114 | 4871 | 1556 | 3324 | 0.031% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 25.918 | 6934.24 | 52.0 | 0.903 | 52.0 | 2.6 | 1.14 | 1.74% | Fine Alluvium | 0.060 | 120.7 | 1.46 | 0.000 | 1.46 | 0 | 35 | 1.79% | 2.4 | 76% | 2.30 | 0.000 | 2.30 | 12.25 | 864 | 1.9E-03 | 1.4E+03 | 37 | 0.5387 | 1.7E-04 | 22 | 0.253 | 0.255 | 161 | 75.269 | 7102 | 4862 | 1557 | 3320 | 0.030% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.082 | 6934.08 | 32.6 | 0.823 | 32.6 | 2.5 | 1.08 | 2.52% | Fine Alluvium | 0.060 | 120.7 | 1.47 | 0.000 | 1.47 | 0 | 21 | 2.64% | 2.7 | 76% | 2.31 | 0.000 | 2.31 | 12.30 | 864 | 1.9E-03 | 1.4E+03 | 36 | 0.5342 | 1.7E-04 | 22 | 0.253 | 0.255 | 162 | 75.736 | 7089 | 4853 | 1558 | 3315 | 0.030% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.246 | 6933.91 | 24.2 | 0.702 | 24.2 | 2.7 | 1.18 | 2.90% | Fine Alluvium | 0.060 | 120.7 | 1.48 | 0.000 | 1.48 | 0 | 15 | 3.09% | 2.9 | 76% | 2.32 | 0.000 | 2.32 | 12.35 | 864 | 1.9E-03 | 1.4E+03 | 36 | 0.5297 | 1.7E-04 | 22 | 0.253 | 0.256 | 163 | 76.203 | 7077 | 4845 | 1559 | 3311 | 0.030% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.410 | 6933.75 | 25.5 | 0.836 | 25.4 | 3.7 | 1.59 | 3.28% | Fine Alluvium | 0.060 | 120.7 | 1.49 | 0.000 | 1.49 | 0 | 16 | 3.49% | 2.9 | 76% | 2.33 | 0.000 | 2.33 | 12.40 | 864 | 1.9E-03 | 1.4E+03 | 36 | 0.5252 | 1.7E-04 | 22 | 0.253 | 0.256 | 164 | 76.670 | 7064 | 4836 | 1560 | 3307 | 0.030% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.574 | 6933.59 | 16.9 | 0.905 | 16.9 | 2.4 | 1.02 | 5.35% | Fine Alluvium | 0.060 | 120.7 | 1.50 | 0.000 | 1.50 | 0 | 10 | 5.88% | 3.2 | 76% | 2.34 | 0.000 | 2.34 | 12.45 | 864 | 1.9E-03 | 1.4E+03 | 35 | 0.5206 | 1.7E-04 | 22 | 0.254 | 0.256 | 165 | 77.137 | 7052 | 4827 | 1561 | 3303 | 0.030% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.739 | 6933.42 | 15.7 | 0.736 | 15.7 | 3.1 | 1.32 | 4.69% | Fine Alluvium | 0.060 | 120.7 | 1.51 | 0.000 | 1.51 | 0 | 9 | 5.19% | 3.2 | 76% | 2.35 | 0.000 | 2.35 | 12.50 | 864 | 1.9E-03 | 1.4E+03 | 35 | 0.5161 | 1.7E-04 | 22 | 0.254 | 0.257 | 166 | 77.604 | 7040 | 4819 | 1562 | 3299 | 0.029% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 26.903 | 6933.26 | 15.0 | 0.577 | 15.0 | 3.9 | 1.71 | 3.85% | Fine Alluvium | 0.060 | 120.7 | 1.52 | 0.000 | 1.52 | 0 | 9 | 4.28% | 3.1 | 76% | 2.36 | 0.000 | 2.36 | 12.55 | 864 | 1.9E-03 | 1.4E+03 | 35 | 0.5116 | 1.7E-04 | 22 | 0.254 | 0.257 | 167 | 78.070 | 7028 | 4810 | 1563 | 3295 | 0.029% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.067 | 6933.09 | 15.1 | 0.503 | 15.1 | 4.2 | 1.81 | 3.32% | Fine Alluvium | 0.060 | 120.7 | 1.53 | 0.000 | 1.53 | 0 | 9 | 3.70% | 3.1 | 76% | 2.37 | 0.000 | 2.37 | 12.60 | 864 | 1.9E-03 | 1.4E+03 | 34 | 0.5070 | 1.7E-04 | 22 | 0.254 | 0.257 | 168 | 78.537 | 7016 | 4802 | 1564 | 3291 | 0.029% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.231 | 6932.93 | 18.4 | 0.498 | 18.4 | 4.4 | 1.91 | 2.70% | Fine Alluvium | 0.060 | 120.7 | 1.54 | 0.000 | 1.54 | 0 | 11 | 2.95% | 3.0 | 76% | 2.38 | 0.000 | 2.38 | 12.65 | 864 | 1.9E-03 | 1.4E+03 | 34 | 0.5025 | 1.7E-04 | 22 | 0.255 | 0.257 | 169 | 79.004 | 7004 | 4794 | 1565 | 3287 | 0.029% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.395 | 6932.77 | 23.2 | 0.676 | 23.2 | 4.6 | 1.99 | 2.91% | Fine Alluvium | 0.060 | 120.7 | 1.55 | 0.000 | 1.55 | 0 | 14 | 3.12% | 2.9 | 76% | 2.39 | 0.000 | 2.39 | 12.70 | 864 | 1.9E-03 | 1.4E+03 | 34 | 0.4979 | 1.7E-04 | 22 | 0.255 | 0.258 | 170 | 79.471 | 6992 | 4785 | 1566 | 3283 | 0.028% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.559 | 6932.60 | 27.1 | 0.811 | 27.0 | 5.3 | 2.30 | 3.00% | Fine Alluvium | 0.060 | 120.7 | 1.56 | 0.000 | 1.56 | 0 | 16 | 3.18% | 2.8 | 76% | 2.40 | 0.000 | 2.40 | 12.75 | 864 | 1.9E-03 | 1.4E+03 | 33 | 0.4934 | 1.6E-04 | 22 | 0.255 | 0.258 | 171 | 79.938 | 6980 | 4777 | 1567 | 3279 | 0.028% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.723 | 6932.44 | 18.4 | 0.811 | 18.4 | 4.9 | 2.14 | 4.41% | Fine Alluvium | 0.060 | 120.7 | 1.57 | 0.000 | 1.57 | 0 | 11 | 4.82% | 3.1 | 76% | 2.41 | 0.000 | 2.41 | 12.80 | 864 | 1.9E-03 | 1.4E+03 | 33 | 0.4888 | 1.6E-04 | 22 | 0.255 | 0.258 | 172 | 80.404 | 6968 | 4769 | 1568 | 3275 | 0.028% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 27.887 | 6932.27 | 13.3 | 0.646 | 13.3 | 4.7 | 2.03 | 4.86% | Fine Alluvium | 0.060 | 120.7 | 1.58 | 0.000 | 1.58 | 0 | 7 | 5.52% | 3.3 | 76% | 2.41 | 0.000 | 2.41 | 12.85 | 864 | 1.9E-03 | 1.4E+03 | 33 | 0.4842 | 1.6E-04 | 22 | 0.256 | 0.259 | 173 | 80.871 | 6956 | 4761 | 1569 | 3271 | 0.028% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 28.051 | 6932.11 | 12.3 | 0.540 | 12.2 | 5.0 | 2.15 | 4.40% | Fine Alluvium | 0.060 | 120.7 | 1.59 | 0.000 | 1.59 | 0 | 7 | 5.06% | 3.3 | 76% | 2.42 | 0.000 | 2.42 | 12.90 | 864 | 1.9E-03 | 1.4E+03 | 33 | 0.4797 | 1.6E-04 | 22 | 0.256 | 0.259 | 174 | 81.338 | 6945 | 4752 | 1570 | 3267 | 0.027% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 28.215 | 6931.95 | 12.7 | 0.537 | 12.6 | 5.4 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Proposed Repository | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer (tsf) | Total Stress at Midpoint of Layer (tsf) | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Effective Stress at Midpoint of Layer (tsf) |
|---------------------|----------------------------|---------------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------------|---|--|--|---|---|
| Erosion Protection | 6974.4 | 6973.7 | 6972.9 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6972.9 | 6971.4 | 6969.9 | 3.0 | 0.057 | 114.7 | 0.264 | 0.178 | 0.00 | 0.00 | 0.264 | 0.178 |
| Mine Spoils | 6969.9 | 6965.1 | 6960.2 | 9.8 | 0.058 | 116.4 | 0.833 | 0.549 | 0.00 | 0.00 | 0.833 | 0.549 |

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|---------|--|
| 6960.16 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6974.44 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|---|
| 0.83 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{repos}$ (psf) |
| 6945.15 | Elevation of bottom of tailings (ft amsl) |

| UNC-NECR WASTE REPOSITORY SEISMIC SETTLEMENT ANALYSIS - CPT-08 | | | | | | | | | |
|---|--|-----------------------------|--|---|--|-----------------------------------|--|------|--|
| Data File: | | 13-52118_RP08-BSC-CPT | | | | Idriss and Boulanger (2008) | | | |
| Location: | | UNC-NECR 2013 Mill Site PDS | | Cells Requiring User Input/Manipulation | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | |
| http://projects.mwhglobal.com/_/13-52118_RP08-BSC-CPT_XLS | | | | | | Earthquake Moment Magnitude, M: | | 5.5 | |
| | | | | | | Magnitude Scaling Factor, MSF: | | 1.69 | |
| | | | | | | Youd, et al (2001) | | | |
| | | | | | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | |
| | | | | | | Earthquake Moment Magnitude, M: | | 6.3 | |
| | | | | | | Magnitude Scaling Factor, MSF: | | 1.59 | |
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|--------|---------|-------|-------|-------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|---|-------|-------|----|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 10.991 | 6965.05 | 228.0 | 3.932 | 228.0 | 2.4 | 1.02 | 1.72% | Coarse Tailings | 0.054 | 108.1 | 0.62 | 0.000 | 0.62 | 0 | 366 | 1.73% | 1.7 | 21% | 1.45 | 0.000 | 1.45 | 7.68 | 853 | 1.7E-03 | 1.2E+03 | 113 | 0.8593 | 2.0E-04 | 0 | 0.227 | 0.226 | 70 | 0.227 | 8570 | 5889 | 1466 | 8570 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000475 | 0.36 | 0.025 | 0.785 | 0.07% | 0.0001 |
| 11.155 | 6964.89 | 196.5 | 4.039 | 196.4 | 2.8 | 1.22 | 2.06% | Coarse Tailings | 0.054 | 108.1 | 0.63 | 0.000 | 0.63 | 0 | 310 | 2.06% | 1.8 | 21% | 1.46 | 0.000 | 1.46 | 7.73 | 853 | 1.7E-03 | 1.2E+03 | 112 | 0.8570 | 2.0E-04 | 0 | 0.228 | 0.226 | 71 | 0.228 | 8549 | 5874 | 1467 | 8549 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000478 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 11.319 | 6964.72 | 180.6 | 3.584 | 180.6 | 3.1 | 1.32 | 1.98% | Coarse Tailings | 0.054 | 108.1 | 0.64 | 0.000 | 0.64 | 0 | 281 | 1.99% | 1.8 | 21% | 1.47 | 0.000 | 1.47 | 7.78 | 853 | 1.7E-03 | 1.2E+03 | 110 | 0.8547 | 2.0E-04 | 0 | 0.228 | 0.227 | 72 | 0.228 | 8527 | 5859 | 1468 | 8527 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000481 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 11.483 | 6964.56 | 171.0 | 2.616 | 170.9 | 2.3 | 0.98 | 1.53% | Coarse Tailings | 0.054 | 108.1 | 0.65 | 0.000 | 0.65 | 0 | 263 | 1.54% | 1.8 | 21% | 1.48 | 0.000 | 1.48 | 7.83 | 853 | 1.7E-03 | 1.2E+03 | 108 | 0.8524 | 2.0E-04 | 0 | 0.228 | 0.227 | 73 | 0.228 | 8506 | 5844 | 1469 | 8506 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000484 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 11.647 | 6964.39 | 149.9 | 1.910 | 149.9 | 1.9 | 0.83 | 1.27% | Coarse Tailings | 0.054 | 108.1 | 0.66 | 0.000 | 0.66 | 0 | 227 | 1.28% | 1.7 | 21% | 1.49 | 0.000 | 1.49 | 7.88 | 853 | 1.7E-03 | 1.2E+03 | 107 | 0.8501 | 2.0E-04 | 0 | 0.229 | 0.227 | 74 | 0.229 | 8486 | 5830 | 1470 | 8486 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000487 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 11.811 | 6964.23 | 141.0 | 2.061 | 141.0 | 2.0 | 0.88 | 1.46% | Coarse Tailings | 0.054 | 108.1 | 0.67 | 0.000 | 0.67 | 0 | 211 | 1.47% | 1.8 | 21% | 1.50 | 0.000 | 1.50 | 7.93 | 853 | 1.7E-03 | 1.2E+03 | 105 | 0.8477 | 2.0E-04 | 0 | 0.229 | 0.228 | 75 | 0.229 | 8465 | 5815 | 1471 | 8465 | 0.037% | 1.79 | 1.00 | 0.01% | 0.000490 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 11.975 | 6964.07 | 141.3 | 2.024 | 141.3 | 1.7 | 0.73 | 1.43% | Coarse Tailings | 0.054 | 108.1 | 0.68 | 0.000 | 0.68 | 0 | 208 | 1.44% | 1.8 | 21% | 1.51 | 0.000 | 1.51 | 7.98 | 853 | 1.7E-03 | 1.2E+03 | 104 | 0.8453 | 2.0E-04 | 0 | 0.229 | 0.228 | 76 | 0.229 | 8444 | 5801 | 1472 | 8444 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000493 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.139 | 6963.90 | 137.2 | 1.911 | 137.2 | 1.6 | 0.69 | 1.39% | Coarse Tailings | 0.054 | 108.1 | 0.68 | 0.000 | 0.68 | 0 | 200 | 1.40% | 1.8 | 21% | 1.51 | 0.000 | 1.51 | 8.03 | 853 | 1.7E-03 | 1.2E+03 | 102 | 0.8429 | 2.0E-04 | 0 | 0.229 | 0.228 | 77 | 0.229 | 8424 | 5787 | 1473 | 8424 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000496 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.303 | 6963.74 | 148.3 | 1.655 | 148.3 | 1.6 | 0.71 | 1.12% | Coarse Tailings | 0.054 | 108.1 | 0.69 | 0.000 | 0.69 | 0 | 213 | 1.12% | 1.7 | 21% | 1.52 | 0.000 | 1.52 | 8.08 | 853 | 1.7E-03 | 1.2E+03 | 101 | 0.8404 | 2.0E-04 | 0 | 0.230 | 0.229 | 78 | 0.230 | 8404 | 5773 | 1474 | 8404 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000499 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.467 | 6963.57 | 171.9 | 1.533 | 171.9 | 1.8 | 0.79 | 0.89% | Coarse Tailings | 0.054 | 108.1 | 0.70 | 0.000 | 0.70 | 0 | 244 | 0.90% | 1.6 | 21% | 1.53 | 0.000 | 1.53 | 8.13 | 853 | 1.7E-03 | 1.2E+03 | 99 | 0.8380 | 2.0E-04 | 0 | 0.230 | 0.229 | 79 | 0.230 | 8384 | 5759 | 1475 | 8384 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000501 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.631 | 6963.41 | 177.1 | 1.749 | 177.1 | 1.9 | 0.81 | 0.99% | Coarse Tailings | 0.054 | 108.1 | 0.71 | 0.000 | 0.71 | 0 | 248 | 0.99% | 1.6 | 21% | 1.54 | 0.000 | 1.54 | 8.18 | 853 | 1.7E-03 | 1.2E+03 | 98 | 0.8355 | 2.1E-04 | 0 | 0.230 | 0.230 | 80 | 0.230 | 8364 | 5745 | 1476 | 8364 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000504 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.795 | 6963.24 | 188.7 | 1.928 | 188.7 | 2.1 | 0.89 | 1.02% | Coarse Tailings | 0.054 | 108.1 | 0.72 | 0.000 | 0.72 | 0 | 261 | 1.03% | 1.6 | 21% | 1.55 | 0.000 | 1.55 | 8.23 | 853 | 1.7E-03 | 1.2E+03 | 97 | 0.8329 | 2.1E-04 | 0 | 0.231 | 0.230 | 81 | 0.231 | 8345 | 5731 | 1477 | 8345 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000506 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 12.959 | 6963.08 | 214.1 | 2.066 | 214.1 | 1.9 | 0.81 | 0.96% | Coarse Tailings | 0.054 | 108.1 | 0.73 | 0.000 | 0.73 | 0 | 293 | 0.97% | 1.6 | 21% | 1.56 | 0.000 | 1.56 | 8.28 | 853 | 1.7E-03 | 1.2E+03 | 95 | 0.8303 | 2.1E-04 | 0 | 0.231 | 0.230 | 82 | 0.231 | 8325 | 5717 | 1478 | 8325 | 0.038% | 1.79 | 1.00 | 0.01% | 0.000508 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.123 | 6962.92 | 223.6 | 2.279 | 223.6 | 2.0 | 0.88 | 1.02% | Coarse Tailings | 0.054 | 108.1 | 0.74 | 0.000 | 0.74 | 0 | 302 | 1.02% | 1.6 | 21% | 1.57 | 0.000 | 1.57 | 8.33 | 853 | 1.7E-03 | 1.2E+03 | 94 | 0.8278 | 2.1E-04 | 0 | 0.231 | 0.231 | 83 | 0.231 | 8306 | 5704 | 1479 | 8306 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000511 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.287 | 6962.75 | 247.1 | 2.375 | 247.1 | 1.8 | 0.79 | 0.96% | Coarse Tailings | 0.054 | 108.1 | 0.75 | 0.000 | 0.75 | 0 | 330 | 0.96% | 1.5 | 21% | 1.58 | 0.000 | 1.58 | 8.38 | 853 | 1.7E-03 | 1.2E+03 | 93 | 0.8251 | 2.1E-04 | 0 | 0.232 | 0.231 | 84 | 0.232 | 8286 | 5690 | 1480 | 8286 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000513 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.451 | 6962.59 | 254.8 | 2.530 | 254.8 | 1.6 | 0.71 | 0.99% | Coarse Tailings | 0.054 | 108.1 | 0.76 | 0.000 | 0.76 | 0 | 336 | 1.00% | 1.5 | 21% | 1.58 | 0.000 | 1.58 | 8.43 | 853 | 1.7E-03 | 1.2E+03 | 91 | 0.8225 | 2.1E-04 | 0 | 0.232 | 0.231 | 85 | 0.232 | 8267 | 5677 | 1481 | 8267 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000515 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.615 | 6962.42 | 246.9 | 2.534 | 246.9 | 1.8 | 0.77 | 1.03% | Coarse Tailings | 0.054 | 108.1 | 0.76 | 0.000 | 0.76 | 0 | 322 | 1.03% | 1.6 | 21% | 1.59 | 0.000 | 1.59 | 8.48 | 853 | 1.7E-03 | 1.2E+03 | 90 | 0.8198 | 2.1E-04 | 0 | 0.232 | 0.232 | 86 | 0.232 | 8249 | 5664 | 1482 | 8249 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000517 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.779 | 6962.26 | 238.6 | 2.398 | 238.6 | 1.5 | 0.63 | 1.01% | Coarse Tailings | 0.054 | 108.1 | 0.77 | 0.000 | 0.77 | 0 | 308 | 1.01% | 1.6 | 21% | 1.60 | 0.000 | 1.60 | 8.53 | 853 | 1.7E-03 | 1.2E+03 | 89 | 0.8171 | 2.1E-04 | 0 | 0.233 | 0.232 | 87 | 0.233 | 8230 | 5651 | 1483 | 8230 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000519 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 13.943 | 6962.10 | 226.5 | 2.107 | 226.5 | 1.5 | 0.63 | 0.93% | Coarse Tailings | 0.054 | 108.1 | 0.78 | 0.000 | 0.78 | 0 | 289 | 0.93% | 1.6 | 21% | 1.61 | 0.000 | 1.61 | 8.58 | 853 | 1.7E-03 | 1.2E+03 | 88 | 0.8143 | 2.1E-04 | 0 | 0.233 | 0.232 | 88 | 0.233 | 8211 | 5638 | 1484 | 8211 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000520 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 14.107 | 6961.93 | 214.2 | 2.003 | 214.2 | 1.5 | 0.63 | 0.93% | Coarse Tailings | 0.054 | 108.1 | 0.79 | 0.000 | 0.79 | 0 | 270 | 0.94% | 1.6 | 21% | 1.62 | 0.000 | 1.62 | 8.63 | 853 | 1.7E-03 | 1.2E+03 | 86 | 0.8116 | 2.1E-04 | 0 | 0.233 | 0.233 | 89 | 0.233 | 8193 | 5625 | 1485 | 8193 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000522 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 14.271 | 6961.77 | 200.1 | 1.891 | 200.1 | 2.1 | 0.89 | 0.95% | Coarse Tailings | 0.054 | 108.1 | 0.80 | 0.000 | 0.80 | 0 | 249 | 0.95% | 1.6 | 21% | 1.63 | 0.000 | 1.63 | 8.68 | 853 | 1.7E-03 | 1.2E+03 | 85 | 0.8088 | 2.1E-04 | 0 | 0.233 | 0.233 | 90 | 0.233 | 8174 | 5612 | 1486 | 8174 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000524 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 14.436 | 6961.60 | 192.5 | 1.806 | 192.5 | 2.3 | 0.98 | 0.94% | Coarse Tailings | 0.054 | 108.1 | 0.81 | 0.000 | 0.81 | 0 | 237 | 0.94% | 1.6 | 21% | 1.64 | 0.000 | 1.64 | 8.73 | 853 | 1.7E-03 | 1.2E+03 | 84 | 0.8059 | 2.1E-04 | 0 | 0.234 | 0.233 | 91 | 0.234 | 8156 | 5599 | 1487 | 8156 | 0.039% | 1.79 | 1.00 | 0.01% | 0.000525 | 0.36 | 0.025 | 0.785 | 0.08% | 0.0001 |
| 14.600 | 6961.44 | 184.8 | 1.764 | 184.8 | 1.6 | 0.67 | 0.95% | Coarse Tailings | 0.054 | 108.1 | 0.82 | 0.000 | 0.82 | 0 | 225 | 0.96% | 1.6 | 21% | 1.65 | 0.000 | 1.65 | 8.78 | 809 | 1.7E-03 | 1.1E+03 | 83 | 0.8031 | 2.3E-04 | 0 | 0.234 | 0.234 | 92 | 0.234 | 8138 | 5587 | 1488 | 8138 | 0.049% | 1.79 | 1.00 | 0.01% | 0.000699 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 14.764 | 6961.28 | 179.0 | 1.664 | 179.0 | 1.1 | 0.49 | 0.93% | Coarse Tailings | 0.054 | 108.1 | 0.83 | 0.000 | 0.83 | 0 | 216 | 0.93% | 1.6 | 21% | 1.66 | 0.000 | 1.66 | 8.83 | 809 | 1.7E-03 | 1.1E+03 | 82 | 0.8002 | 2.4E-04 | 0 | 0.234 | 0.234 | 93 | 0.234 | 8120 | 5574 | 1489 | 8120 | 0.049% | 1.79 | 1.00 | 0.01% | 0.000701 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 14.928 | 6961.11 | 176.7 | 1.460 | 176.7 | 1.3 | 0.55 | 0.83% | Coarse Tailings | 0.054 | 108.1 | 0.83 | 0.000 | 0.83 | 0 | 211 | 0.83% | 1.6 | 21% | 1.66 | 0.000 | 1.66 | 8.88 | 809 | 1.7E-03 | 1.1E+03 | 81 | 0.7973 | 2.4E-04 | 0 | 0.235 | 0.234 | 94 | 0.235 | 8103 | 5562 | 1490 | 8103 | 0.049% | 1.79 | 1.00 | 0.01% | 0.000702 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 15.092 | 6960.95 | 176.2 | 1.439 | 176.2 | 1.2 | 0.53 | 0.82% | Coarse Tailings | 0.054 | 108.1 | 0.84 | 0.000 | 0.84 | 0 | 208 | 0.82% | 1.6 | 21% | 1.67 | 0.000 | 1.67 | 8.93 | 809 | 1.7E-03 | 1.1E+03 | 80 | 0.7943 | 2.4E-04 | 0 | 0.235 | 0.235 | 95 | 0.235 | 8085 | 5549 | 1491 | 8085 | 0.049% | 1.79 | 1.00 | 0.01% | 0.000704 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 15.256 | 6960.78 | 172.2 | 1.406 | 172.2 | 1.1 | 0.49 | 0.82% | Coarse Tailings | 0.054 | 108.1 | 0.85 | 0.000 | 0.85 | 0 | 201 | 0.82% | 1.6 | 21% | 1.68 | 0.000 | 1.68 | 8.98 | 809 | 1.7E-03 | 1.1E+03 | 79 | 0.7913 | 2.4E-04 | 0 | 0.235 | 0.235 | 96 | 0.235 | 8067 | 5537 | 1492 | 8067 | 0.049% | 1.79 | 1.00 | 0.01% | 0.000705 | 0.36 | 0.025 | 0.785 | 0.11% | 0.0002 |
| 15.420 | 6960.62 | 168.5 | 1.435 | 168.5 | 1.2 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 24.442 | 6951.60 | 67.6 | 0.479 | 67.6 | 0.9 | 0.39 | 0.71% | Coarse Tailings | 0.054 | 108.1 | 1.36 | 0.000 | 1.36 | 0 | 49 | 0.72% | 2.1 | 21% | 2.19 | 0.000 | 2.19 | 11.78 | 761 | 1.7E-03 | 9.7E+02 | 40 | 0.5804 | 2.5E-04 | 0 | 0.250 | 0.252 | 152 | 0.250 | 7245 | 4962 | 1548 | 7245 | 0.053% | 1.79 | 1.00 | 0.01% | 0.000761 | 0.36 | 0.025 | 0.785 | 0.12% | 0.0002 |
| 24.606 | 6951.43 | 69.7 | 0.468 | 69.7 | 0.7 | 0.30 | 0.67% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 50 | 0.69% | 2.1 | 21% | 2.20 | 0.000 | 2.20 | 11.83 | 594 | 1.7E-03 | 5.9E+02 | 40 | 0.5760 | 4.2E-04 | 0 | 0.250 | 0.252 | 153 | 0.250 | 7233 | 4954 | 1549 | 7233 | 0.202% | 1.79 | 1.00 | 0.01% | 0.003445 | 0.36 | 0.025 | 0.785 | 0.54% | 0.0009 |
| 24.770 | 6951.27 | 73.4 | 0.534 | 73.4 | 0.8 | 0.35 | 0.73% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 52 | 0.74% | 2.1 | 21% | 2.20 | 0.000 | 2.20 | 11.88 | 594 | 1.7E-03 | 5.9E+02 | 39 | 0.5716 | 4.1E-04 | 0 | 0.250 | 0.252 | 154 | 0.250 | 7221 | 4945 | 1550 | 7221 | 0.199% | 1.79 | 1.00 | 0.01% | 0.003886 | 0.36 | 0.025 | 0.785 | 0.53% | 0.0009 |
| 24.934 | 6951.11 | 67.6 | 0.558 | 67.6 | 0.8 | 0.33 | 0.83% | Coarse Tailings | 0.054 | 108.1 | 1.38 | 0.000 | 1.38 | 0 | 48 | 0.84% | 2.1 | 21% | 2.21 | 0.000 | 2.21 | 11.93 | 594 | 1.7E-03 | 5.9E+02 | 39 | 0.5671 | 4.1E-04 | 0 | 0.251 | 0.253 | 155 | 0.251 | 7209 | 4937 | 1551 | 7209 | 0.196% | 1.79 | 1.00 | 0.01% | 0.003327 | 0.36 | 0.025 | 0.785 | 0.52% | 0.0009 |
| 25.098 | 6950.94 | 51.9 | 0.730 | 51.9 | 0.8 | 0.33 | 1.41% | Coarse Tailings | 0.054 | 108.1 | 1.39 | 0.000 | 1.39 | 0 | 36 | 1.45% | 2.4 | 21% | 2.22 | 0.000 | 2.22 | 11.98 | 594 | 1.7E-03 | 5.9E+02 | 39 | 0.5627 | 4.1E-04 | 0 | 0.251 | 0.253 | 156 | 0.251 | 7197 | 4929 | 1552 | 7197 | 0.193% | 1.79 | 1.00 | 0.01% | 0.003268 | 0.36 | 0.025 | 0.785 | 0.51% | 0.0008 |
| 25.262 | 6950.78 | 33.5 | 0.875 | 33.5 | 0.7 | 0.30 | 2.61% | Coarse Tailings | 0.054 | 108.1 | 1.40 | 0.000 | 1.40 | 0 | 23 | 2.72% | 2.7 | 21% | 2.23 | 0.000 | 2.23 | 12.03 | 594 | 1.7E-03 | 5.9E+02 | 38 | 0.5583 | 4.1E-04 | 0 | 0.251 | 0.253 | 157 | 0.251 | 7186 | 4921 | 1553 | 7186 | 0.189% | 1.79 | 1.00 | 0.01% | 0.003210 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 25.426 | 6950.61 | 21.0 | 0.814 | 21.0 | 0.3 | 0.12 | 3.87% | Coarse Tailings | 0.054 | 108.1 | 1.41 | 0.000 | 1.41 | 0 | 14 | 4.15% | 3.0 | 21% | 2.24 | 0.000 | 2.24 | 12.08 | 594 | 1.7E-03 | 5.9E+02 | 38 | 0.5538 | 4.1E-04 | 0 | 0.251 | 0.253 | 158 | 0.251 | 7174 | 4912 | 1554 | 7174 | 0.186% | 1.79 | 1.00 | 0.01% | 0.003152 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 25.590 | 6950.45 | 14.9 | 0.530 | 14.9 | 0.9 | 0.41 | 3.57% | Coarse Tailings | 0.054 | 108.1 | 1.42 | 0.000 | 1.42 | 0 | 9 | 3.94% | 3.1 | 21% | 2.25 | 0.000 | 2.25 | 12.13 | 594 | 1.7E-03 | 5.9E+02 | 38 | 0.5493 | 4.1E-04 | 0 | 0.251 | 0.254 | 159 | 0.251 | 7162 | 4904 | 1555 | 7162 | 0.183% | 1.79 | 1.00 | 0.01% | 0.003094 | 0.36 | 0.025 | 0.785 | 0.49% | 0.0008 |
| 25.754 | 6950.29 | 13.7 | 0.284 | 13.7 | 1.5 | 0.65 | 2.07% | Coarse Tailings | 0.054 | 108.1 | 1.43 | 0.000 | 1.43 | 0 | 9 | 2.31% | 3.0 | 21% | 2.26 | 0.000 | 2.26 | 12.18 | 594 | 1.7E-03 | 5.9E+02 | 37 | 0.5448 | 4.0E-04 | 0 | 0.252 | 0.254 | 160 | 0.252 | 7151 | 4896 | 1556 | 7151 | 0.180% | 1.79 | 1.00 | 0.01% | 0.003037 | 0.36 | 0.025 | 0.785 | 0.48% | 0.0008 |
| 25.918 | 6950.12 | 13.1 | 0.160 | 13.1 | 1.6 | 0.69 | 1.22% | Coarse Tailings | 0.054 | 108.1 | 1.44 | 0.000 | 1.44 | 0 | 8 | 1.37% | 2.9 | 21% | 2.27 | 0.000 | 2.27 | 12.23 | 594 | 1.7E-03 | 5.9E+02 | 37 | 0.5403 | 4.0E-04 | 0 | 0.252 | 0.254 | 161 | 0.252 | 7139 | 4888 | 1557 | 7139 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002980 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 26.082 | 6949.96 | 12.9 | 0.169 | 12.9 | 1.7 | 0.73 | 1.31% | Coarse Tailings | 0.054 | 108.1 | 1.45 | 0.000 | 1.45 | 0 | 8 | 1.48% | 2.9 | 21% | 2.28 | 0.000 | 2.28 | 12.28 | 594 | 1.7E-03 | 5.9E+02 | 36 | 0.5358 | 4.0E-04 | 0 | 0.252 | 0.255 | 162 | 0.252 | 7128 | 4880 | 1558 | 7128 | 0.173% | 1.79 | 1.00 | 0.01% | 0.002923 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.246 | 6949.79 | 13.2 | 0.194 | 13.2 | 1.9 | 0.83 | 1.47% | Coarse Tailings | 0.054 | 108.1 | 1.45 | 0.000 | 1.45 | 0 | 8 | 1.65% | 2.9 | 21% | 2.28 | 0.000 | 2.28 | 12.33 | 594 | 1.7E-03 | 5.9E+02 | 36 | 0.5313 | 4.0E-04 | 0 | 0.252 | 0.255 | 163 | 0.252 | 7116 | 4872 | 1559 | 7116 | 0.170% | 1.79 | 1.00 | 0.01% | 0.002867 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 26.410 | 6949.63 | 13.5 | 0.250 | 13.5 | 2.5 | 1.08 | 1.86% | Fine Tailings | 0.054 | 107.6 | 1.46 | 0.003 | 1.46 | 1 | 8 | 2.08% | 3.0 | 83% | 2.29 | 0.003 | 2.29 | 12.38 | 594 | 1.7E-03 | 5.9E+02 | 36 | 0.5268 | 4.0E-04 | 43 | 0.253 | 0.255 | 164 | 164.000 | 7110 | 4868 | 1560 | 1560 | 0.074% | 0.90 | 0.75 | 0.06% | 0.001177 | 0.25 | 0.323 | 0.851 | 0.20% | 0.0003 |
| 26.574 | 6949.47 | 15.1 | 0.321 | 15.0 | 6.5 | 2.83 | 2.13% | Fine Tailings | 0.054 | 107.6 | 1.47 | 0.009 | 1.46 | 1 | 9 | 2.36% | 3.0 | 83% | 2.30 | 0.009 | 2.29 | 12.43 | 594 | 1.7E-03 | 5.9E+02 | 35 | 0.5223 | 4.0E-04 | 43 | 0.253 | 0.255 | 165 | 165.000 | 7105 | 4864 | 1561 | 1561 | 0.074% | 0.90 | 0.75 | 0.06% | 0.001144 | 0.25 | 0.323 | 0.851 | 0.19% | 0.0003 |
| 26.739 | 6949.30 | 15.7 | 0.328 | 15.6 | 8.2 | 3.56 | 2.09% | Fine Tailings | 0.054 | 107.6 | 1.48 | 0.014 | 1.47 | 1 | 10 | 2.31% | 2.9 | 83% | 2.31 | 0.014 | 2.30 | 12.48 | 594 | 1.7E-03 | 5.9E+02 | 35 | 0.5178 | 4.0E-04 | 43 | 0.253 | 0.255 | 166 | 166.000 | 7100 | 4861 | 1562 | 1562 | 0.073% | 0.90 | 0.75 | 0.06% | 0.001109 | 0.25 | 0.323 | 0.851 | 0.19% | 0.0003 |
| 26.903 | 6949.14 | 13.4 | 0.287 | 13.3 | 14.4 | 6.24 | 2.14% | Fine Tailings | 0.054 | 107.6 | 1.49 | 0.019 | 1.47 | 1 | 8 | 2.41% | 3.0 | 83% | 2.32 | 0.019 | 2.30 | 12.53 | 594 | 1.7E-03 | 5.9E+02 | 35 | 0.5132 | 3.9E-04 | 43 | 0.253 | 0.255 | 167 | 167.000 | 7095 | 4858 | 1563 | 1563 | 0.073% | 0.90 | 0.75 | 0.06% | 0.001074 | 0.25 | 0.323 | 0.851 | 0.18% | 0.0003 |
| 27.067 | 6948.97 | 12.1 | 0.242 | 12.0 | 14.3 | 6.20 | 2.01% | Fine Tailings | 0.054 | 107.6 | 1.50 | 0.024 | 1.47 | 1 | 7 | 2.29% | 3.1 | 83% | 2.33 | 0.024 | 2.30 | 12.58 | 594 | 1.7E-03 | 5.9E+02 | 34 | 0.5087 | 3.9E-04 | 43 | 0.253 | 0.255 | 168 | 168.000 | 7091 | 4854 | 1564 | 1564 | 0.072% | 0.90 | 0.75 | 0.06% | 0.001038 | 0.25 | 0.323 | 0.851 | 0.18% | 0.0003 |
| 27.231 | 6948.81 | 12.3 | 0.235 | 12.2 | 15.7 | 6.79 | 1.92% | Fine Tailings | 0.054 | 107.6 | 1.51 | 0.029 | 1.48 | 1 | 7 | 2.18% | 3.0 | 83% | 2.34 | 0.029 | 2.31 | 12.63 | 594 | 1.7E-03 | 5.9E+02 | 34 | 0.5041 | 3.9E-04 | 43 | 0.253 | 0.255 | 169 | 169.000 | 7086 | 4851 | 1565 | 1565 | 0.072% | 0.90 | 0.75 | 0.06% | 0.001001 | 0.25 | 0.323 | 0.851 | 0.17% | 0.0003 |
| 27.395 | 6948.65 | 12.9 | 0.282 | 12.8 | 18.8 | 8.13 | 2.18% | Fine Tailings | 0.054 | 107.6 | 1.52 | 0.034 | 1.48 | 1 | 8 | 2.47% | 3.0 | 83% | 2.35 | 0.034 | 2.31 | 12.68 | 594 | 1.7E-03 | 5.9E+02 | 34 | 0.4996 | 3.9E-04 | 43 | 0.253 | 0.256 | 170 | 170.000 | 7081 | 4848 | 1566 | 1566 | 0.071% | 0.90 | 0.75 | 0.06% | 0.000963 | 0.25 | 0.323 | 0.851 | 0.16% | 0.0003 |
| 27.559 | 6948.48 | 13.0 | 0.314 | 12.9 | 19.3 | 8.36 | 2.42% | Fine Tailings | 0.054 | 107.6 | 1.53 | 0.039 | 1.49 | 1 | 8 | 2.74% | 3.1 | 83% | 2.36 | 0.039 | 2.32 | 12.73 | 594 | 1.7E-03 | 5.9E+02 | 34 | 0.4950 | 3.9E-04 | 43 | 0.253 | 0.256 | 171 | 171.000 | 7077 | 4845 | 1567 | 1567 | 0.070% | 0.90 | 0.75 | 0.06% | 0.000924 | 0.25 | 0.323 | 0.851 | 0.16% | 0.0003 |
| 27.723 | 6948.32 | 11.5 | 0.282 | 11.4 | 19.8 | 8.58 | 2.46% | Fine Tailings | 0.054 | 107.6 | 1.53 | 0.044 | 1.49 | 1 | 7 | 2.84% | 3.1 | 83% | 2.36 | 0.044 | 2.32 | 12.78 | 594 | 1.7E-03 | 5.9E+02 | 33 | 0.4905 | 3.8E-04 | 43 | 0.253 | 0.256 | 172 | 172.000 | 7072 | 4841 | 1568 | 1568 | 0.070% | 0.90 | 0.75 | 0.06% | 0.000884 | 0.25 | 0.323 | 0.851 | 0.15% | 0.0002 |
| 27.887 | 6948.15 | 11.4 | 0.219 | 11.3 | 20.6 | 8.93 | 1.92% | Fine Tailings | 0.054 | 107.6 | 1.54 | 0.050 | 1.49 | 1 | 7 | 2.22% | 3.1 | 83% | 2.37 | 0.050 | 2.32 | 12.83 | 594 | 1.7E-03 | 5.9E+02 | 33 | 0.4859 | 3.8E-04 | 43 | 0.253 | 0.256 | 173 | 173.000 | 7067 | 4838 | 1569 | 1569 | 0.069% | 0.90 | 0.75 | 0.06% | 0.000844 | 0.25 | 0.323 | 0.851 | 0.14% | 0.0002 |
| 28.051 | 6947.99 | 10.9 | 0.198 | 10.8 | 21.4 | 9.25 | 1.82% | Fine Tailings | 0.054 | 107.6 | 1.55 | 0.055 | 1.50 | 1 | 6 | 2.12% | 3.1 | 83% | 2.38 | 0.055 | 2.33 | 12.88 | 594 | 1.7E-03 | 5.9E+02 | 33 | 0.4813 | 3.8E-04 | 43 | 0.253 | 0.256 | 174 | 174.000 | 7063 | 4835 | 1570 | 1570 | 0.069% | 0.90 | 0.75 | 0.06% | 0.000802 | 0.25 | 0.323 | 0.851 | 0.14% | 0.0002 |
| 28.215 | 6947.83 | 10.5 | 0.189 | 10.3 | 22.1 | 9.56 | 1.81% | Fine Tailings | 0.054 | 107.6 | 1.56 | 0.060 | 1.50 | 1 | 6 | 2.12% | 3.1 | 83% | 2.39 | 0.060 | 2.33 | 12.93 | 594 | 1.7E-03 | 5.9E+02 | 32 | 0.4768 | 3.8E-04 | 43 | 0.254 | 0.256 | 175 | 175.000 | 7058 | 4832 | 1571 | 1571 | 0.068% | 0.90 | 0.75 | 0.06% | 0.000759 | 0.25 | 0.323 | 0.851 | 0.13% | 0.0002 |
| 28.379 | 6947.66 | 10.0 | 0.193 | 9.9 | 22.6 | 9.78 | 1.93% | Fine Tailings | 0.054 | 107.6 | 1.57 | 0.065 | 1.50 | 1 | 6 | 2.28% | 3.1 | 83% | 2.40 | 0.065 | 2.33 | 12.98 | 594 | 1.7E-03 | 5.9E+02 | 32 | 0.4722 | 3.7E-04 | 43 | 0.254 | 0.256 | 176 | 176.000 | 7054 | 4829 | 1572 | 1572 | 0.067% | 0.90 | 0.75 | 0.06% | 0.000715 | 0.25 | 0.323 | 0.851 | 0.12% | 0.0002 |
| 28.543 | 6947.50 | 10.3 | 0.200 | 10.1 | 23.1 | 10.01 | 1.95% | Fine Tailings | 0.054 | 107.6 | 1.58 | 0.070 | 1.51 | 1 | 6 | 2.30% | 3.1 | 83% | 2.41 | 0.070 | 2.34 | 13.03 | 594 | 1.7E-03 | 5.9E+02 | 32 | 0.4677 | 3.7E-04 | 43 | 0.254 | 0.256 | 177 | 177.000 | 7049 | 4825 | 1573 | 1573 | 0.067% | 0.90 | 0.75 | 0.06% | 0.000670 | 0.25 | 0.323 | 0.851 | 0.11% | 0.0002 |
| 28.707 | 6947.33 | 10.5 | 0.214 | 10.3 | 24.2 | 10.50 | 2.04% | Fine Tailings | 0.054 | 107.6 | 1.59 | 0.075 | 1.51 | 1 | 6 | 2.41% | 3.1 | 83% | 2.42 | 0.075 | 2.34 | 13.08 | 594 | 1.7E-03 | 5.9E+02 | 32 | 0.4631 | 3.7E-04 | 43 | 0.254 | 0.256 | 178 | 178.000 | 7044 | 4822 | 1574 | 1574 | 0.066% | 0.90 | 0.75 | 0.06% | 0.000623 | 0.25 | 0.323 | 0.851 | 0.11% | 0.0002 |
| 28.871 | 6947.17 | 11.2 | 0.187 | 11.0 | 24.5 | 10.62 | 1. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|-------|-------|------------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 37.893 | 6938.15 | 12.7 | 0.266 | 12.4 | 44.0 | 19.08 | 2.09% | Coarse/Fine Tail | 0.058 | 116.0 | 2.09 | 0.000 | 2.09 | 0 | 5 | 2.51% | 3.2 | 52% | 2.92 | 0.000 | 2.92 | 15.88 | 639 | 1.8E-03 | 7.4E+02 | 22 | 0.2286 | 1.8E-04 | 20 | 0.267 | 0.272 | 234 | 78.181 | 6432 | 4395 | 1630 | 3473 | 0.033% | 2.00 | 0.65 | 0.03% | 0.002103 | 0.34 | 0.079 | 0.797 | 0.34% | 0.0005 |
| 38.057 | 6937.98 | 13.1 | 0.256 | 12.9 | 44.4 | 19.24 | 1.95% | Coarse/Fine Tail | 0.058 | 116.0 | 2.10 | 0.000 | 2.10 | 0 | 5 | 2.32% | 3.2 | 52% | 2.93 | 0.000 | 2.93 | 15.93 | 639 | 1.8E-03 | 7.4E+02 | 22 | 0.2250 | 1.8E-04 | 20 | 0.267 | 0.272 | 235 | 78.515 | 6423 | 4389 | 1631 | 3470 | 0.032% | 2.00 | 0.65 | 0.03% | 0.001739 | 0.34 | 0.079 | 0.797 | 0.28% | 0.0005 |
| 38.221 | 6937.82 | 13.4 | 0.205 | 13.1 | 45.1 | 19.54 | 1.54% | Coarse/Fine Tail | 0.058 | 116.0 | 2.11 | 0.000 | 2.11 | 0 | 5 | 1.82% | 3.1 | 52% | 2.94 | 0.000 | 2.94 | 15.98 | 639 | 1.8E-03 | 7.4E+02 | 22 | 0.2215 | 1.7E-04 | 20 | 0.268 | 0.272 | 236 | 78.848 | 6415 | 4383 | 1632 | 3466 | 0.031% | 2.00 | 0.65 | 0.03% | 0.001334 | 0.34 | 0.079 | 0.797 | 0.21% | 0.0003 |
| 38.385 | 6937.65 | 12.6 | 0.178 | 12.3 | 46.5 | 20.13 | 1.41% | Coarse/Fine Tail | 0.058 | 116.0 | 2.12 | 0.000 | 2.12 | 0 | 5 | 1.70% | 3.1 | 52% | 2.95 | 0.000 | 2.95 | 16.03 | 639 | 1.8E-03 | 7.4E+02 | 22 | 0.2180 | 1.7E-04 | 20 | 0.268 | 0.273 | 237 | 79.182 | 6406 | 4377 | 1633 | 3462 | 0.031% | 2.00 | 0.65 | 0.03% | 0.000855 | 0.34 | 0.079 | 0.797 | 0.14% | 0.0002 |
| 38.549 | 6937.49 | 12.0 | 0.236 | 11.7 | 52.9 | 22.92 | 1.97% | Coarse/Fine Tail | 0.058 | 116.0 | 2.13 | 0.000 | 2.13 | 0 | 5 | 2.39% | 3.2 | 52% | 2.96 | 0.000 | 2.96 | 16.08 | 639 | 1.8E-03 | 7.4E+02 | 22 | 0.2145 | 1.7E-04 | 20 | 0.268 | 0.273 | 238 | 79.515 | 6398 | 4371 | 1634 | 3459 | 0.030% | 2.00 | 0.65 | 0.03% | 0.000096 | 0.34 | 0.079 | 0.797 | 0.02% | 0.0000 |
| 38.713 | 6937.33 | 12.4 | 0.281 | 12.1 | 56.4 | 24.42 | 2.27% | Fine Tailings | 0.054 | 107.6 | 2.14 | 0.004 | 2.14 | 1 | 5 | 2.74% | 3.2 | 83% | 2.97 | 0.004 | 2.97 | 16.13 | 639 | 1.7E-03 | 6.8E+02 | 22 | 0.2111 | 1.8E-04 | 43 | 0.268 | 0.273 | 239 | 239.000 | 6393 | 4368 | 1635 | 1635 | 0.024% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.877 | 6937.16 | 12.4 | 0.304 | 12.1 | 58.1 | 25.18 | 2.45% | Fine Tailings | 0.054 | 107.6 | 2.15 | 0.009 | 2.14 | 1 | 5 | 2.96% | 3.3 | 83% | 2.98 | 0.009 | 2.97 | 16.18 | 639 | 1.7E-03 | 6.8E+02 | 22 | 0.2077 | 1.8E-04 | 43 | 0.268 | 0.273 | 240 | 240.000 | 6390 | 4366 | 1636 | 1636 | 0.024% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.042 | 6937.00 | 12.5 | 0.323 | 12.1 | 59.5 | 25.79 | 2.59% | Fine Tailings | 0.054 | 107.6 | 2.16 | 0.014 | 2.14 | 1 | 5 | 3.13% | 3.3 | 83% | 2.99 | 0.014 | 2.97 | 16.23 | 639 | 1.7E-03 | 6.8E+02 | 22 | 0.2043 | 1.7E-04 | 43 | 0.268 | 0.273 | 241 | 241.000 | 6386 | 4363 | 1637 | 1637 | 0.023% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.206 | 6936.83 | 13.0 | 0.317 | 12.6 | 60.8 | 26.36 | 2.44% | Fine Tailings | 0.054 | 107.6 | 2.17 | 0.019 | 2.15 | 1 | 5 | 2.93% | 3.2 | 83% | 3.00 | 0.019 | 2.98 | 16.28 | 639 | 1.7E-03 | 6.8E+02 | 21 | 0.2009 | 1.7E-04 | 43 | 0.268 | 0.273 | 242 | 242.000 | 6383 | 4361 | 1638 | 1638 | 0.023% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.370 | 6936.67 | 13.9 | 0.354 | 13.5 | 63.3 | 27.43 | 2.55% | Fine Tailings | 0.054 | 107.6 | 2.18 | 0.024 | 2.15 | 1 | 5 | 3.02% | 3.2 | 83% | 3.01 | 0.024 | 2.98 | 16.33 | 639 | 1.7E-03 | 6.8E+02 | 21 | 0.1976 | 1.7E-04 | 43 | 0.268 | 0.273 | 243 | 243.000 | 6380 | 4359 | 1639 | 1639 | 0.022% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.534 | 6936.51 | 13.2 | 0.359 | 12.8 | 63.9 | 27.68 | 2.72% | Fine Tailings | 0.054 | 107.6 | 2.19 | 0.029 | 2.16 | 1 | 5 | 3.26% | 3.3 | 83% | 3.01 | 0.029 | 2.99 | 16.38 | 639 | 1.7E-03 | 6.8E+02 | 21 | 0.1943 | 1.7E-04 | 43 | 0.268 | 0.274 | 244 | 244.000 | 6377 | 4357 | 1640 | 1640 | 0.022% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.698 | 6936.34 | 26.5 | 0.601 | 26.1 | 67.9 | 29.43 | 2.27% | Fine Tailings | 0.054 | 107.6 | 2.19 | 0.034 | 2.16 | 1 | 11 | 2.47% | 2.9 | 83% | 3.02 | 0.034 | 2.99 | 16.43 | 639 | 1.7E-03 | 6.8E+02 | 21 | 0.1911 | 1.7E-04 | 43 | 0.269 | 0.274 | 245 | 245.000 | 6373 | 4354 | 1641 | 1641 | 0.022% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.862 | 6936.18 | 24.6 | 0.611 | 24.4 | 36.8 | 15.93 | 2.48% | Fine Tailings | 0.054 | 107.6 | 2.20 | 0.039 | 2.16 | 1 | 10 | 2.72% | 3.0 | 83% | 3.03 | 0.039 | 2.99 | 16.48 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1878 | 1.4E-04 | 43 | 0.269 | 0.274 | 246 | 246.000 | 6370 | 4352 | 1642 | 1642 | 0.018% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.026 | 6936.01 | 20.7 | 0.747 | 20.5 | 30.1 | 13.03 | 3.61% | Fine Tailings | 0.054 | 107.6 | 2.21 | 0.044 | 2.17 | 1 | 9 | 4.04% | 3.1 | 83% | 3.04 | 0.044 | 3.00 | 16.53 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1846 | 1.4E-04 | 43 | 0.269 | 0.274 | 247 | 247.000 | 6367 | 4350 | 1643 | 1643 | 0.018% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.190 | 6935.85 | 20.7 | 0.634 | 20.5 | 28.6 | 12.41 | 3.06% | Fine Tailings | 0.054 | 107.6 | 2.22 | 0.050 | 2.17 | 1 | 9 | 3.43% | 3.1 | 83% | 3.05 | 0.050 | 3.00 | 16.58 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1815 | 1.4E-04 | 43 | 0.269 | 0.274 | 248 | 248.000 | 6364 | 4348 | 1644 | 1644 | 0.017% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.354 | 6935.69 | 20.7 | 0.720 | 20.5 | 26.8 | 11.59 | 3.48% | Fine Tailings | 0.054 | 107.6 | 2.23 | 0.055 | 2.17 | 1 | 8 | 3.90% | 3.1 | 83% | 3.06 | 0.055 | 3.00 | 16.63 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1783 | 1.4E-04 | 43 | 0.269 | 0.274 | 249 | 249.000 | 6360 | 4345 | 1645 | 1645 | 0.017% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.518 | 6935.52 | 15.0 | 0.602 | 14.7 | 50.2 | 21.76 | 4.02% | Fine Tailings | 0.054 | 107.6 | 2.24 | 0.060 | 2.18 | 1 | 6 | 4.73% | 3.3 | 83% | 3.07 | 0.060 | 3.01 | 16.68 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1752 | 1.3E-04 | 43 | 0.269 | 0.274 | 250 | 250.000 | 6357 | 4343 | 1646 | 1646 | 0.017% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.682 | 6935.36 | 13.1 | 0.409 | 12.7 | 60.9 | 26.39 | 3.13% | Fine Tailings | 0.054 | 107.6 | 2.25 | 0.065 | 2.18 | 1 | 5 | 3.78% | 3.3 | 83% | 3.08 | 0.065 | 3.01 | 16.73 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1721 | 1.3E-04 | 43 | 0.269 | 0.274 | 251 | 251.000 | 6354 | 4341 | 1647 | 1647 | 0.016% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.846 | 6935.19 | 13.1 | 0.370 | 12.7 | 71.3 | 30.89 | 2.82% | Fine Tailings | 0.054 | 107.6 | 2.26 | 0.070 | 2.19 | 1 | 5 | 3.40% | 3.3 | 83% | 3.09 | 0.070 | 3.02 | 16.78 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1691 | 1.3E-04 | 43 | 0.269 | 0.274 | 252 | 252.000 | 6351 | 4339 | 1648 | 1648 | 0.016% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.010 | 6935.03 | 13.8 | 0.383 | 13.3 | 79.0 | 34.25 | 2.78% | Fine Tailings | 0.054 | 107.6 | 2.26 | 0.075 | 2.19 | 1 | 5 | 3.33% | 3.3 | 83% | 3.09 | 0.075 | 3.02 | 16.83 | 685 | 1.7E-03 | 7.8E+02 | 21 | 0.1660 | 1.3E-04 | 43 | 0.269 | 0.274 | 253 | 253.000 | 6348 | 4336 | 1649 | 1649 | 0.016% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.174 | 6934.87 | 13.5 | 0.420 | 13.0 | 81.1 | 35.14 | 3.11% | Fine Tailings | 0.054 | 107.6 | 2.27 | 0.080 | 2.19 | 1 | 5 | 3.74% | 3.3 | 83% | 3.10 | 0.080 | 3.02 | 16.88 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1630 | 1.3E-04 | 43 | 0.269 | 0.274 | 254 | 254.000 | 6344 | 4334 | 1650 | 1650 | 0.015% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.338 | 6934.70 | 13.5 | 0.417 | 12.9 | 81.7 | 35.40 | 3.10% | Fine Tailings | 0.054 | 107.6 | 2.28 | 0.085 | 2.20 | 1 | 5 | 3.73% | 3.3 | 83% | 3.11 | 0.085 | 3.03 | 16.93 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1601 | 1.2E-04 | 43 | 0.269 | 0.275 | 255 | 255.000 | 6341 | 4332 | 1651 | 1651 | 0.015% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.502 | 6934.54 | 13.1 | 0.315 | 12.6 | 80.2 | 34.75 | 2.41% | Fine Tailings | 0.054 | 107.6 | 2.29 | 0.091 | 2.20 | 1 | 5 | 2.92% | 3.3 | 83% | 3.12 | 0.091 | 3.03 | 16.98 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1571 | 1.2E-04 | 43 | 0.269 | 0.275 | 256 | 256.000 | 6338 | 4330 | 1652 | 1652 | 0.015% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.666 | 6934.37 | 13.1 | 0.247 | 12.6 | 80.3 | 34.78 | 1.88% | Fine Tailings | 0.054 | 107.6 | 2.30 | 0.096 | 2.20 | 1 | 5 | 2.28% | 3.2 | 83% | 3.13 | 0.096 | 3.03 | 17.03 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1542 | 1.2E-04 | 43 | 0.269 | 0.275 | 257 | 257.000 | 6335 | 4327 | 1653 | 1653 | 0.015% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.830 | 6934.21 | 13.8 | 0.279 | 13.3 | 84.1 | 36.42 | 2.02% | Fine Tailings | 0.054 | 107.6 | 2.31 | 0.101 | 2.21 | 1 | 5 | 2.42% | 3.2 | 83% | 3.14 | 0.101 | 3.04 | 17.08 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1514 | 1.2E-04 | 43 | 0.270 | 0.275 | 258 | 258.000 | 6332 | 4325 | 1654 | 1654 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.994 | 6934.05 | 14.1 | 0.381 | 13.6 | 90.1 | 39.04 | 2.69% | Fine Tailings | 0.054 | 107.6 | 2.32 | 0.106 | 2.21 | 1 | 5 | 3.22% | 3.2 | 83% | 3.15 | 0.106 | 3.04 | 17.13 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1485 | 1.2E-04 | 43 | 0.270 | 0.275 | 259 | 259.000 | 6329 | 4323 | 1655 | 1655 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 42.158 | 6933.88 | 14.4 | 0.449 | 13.8 | 94.1 | 40.77 | 3.11% | Fine Tailings | 0.054 | 107.6 | 2.33 | 0.111 | 2.22 | 1 | 5 | 3.71% | 3.3 | 83% | 3.16 | 0.111 | 3.05 | 17.18 | 685 | 1.7E-03 | 7.8E+02 | 20 | 0.1457 | 1.1E-04 | 43 | 0.270 | 0.275 | 260 | 260.000 | 6325 | 4321 | 1656 | 1656 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 42.322 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|-------|-------|-------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|
| 51.345 | 6924.70 | 41.1 | 0.665 | 41.0 | 16.7 | 7.24 | 1.62% | Coarse Alluvium | 0.056 | 111.0 | 2.83 | 0.000 | 2.83 | 0 | 14 | 1.74% | 2.8 | 36% | 3.66 | 0.000 | 3.66 |
| 51.509 | 6924.53 | 41.8 | 0.943 | 41.7 | 17.0 | 7.38 | 2.26% | Coarse Alluvium | 0.056 | 111.0 | 2.84 | 0.000 | 2.84 | 0 | 14 | 2.42% | 2.8 | 36% | 3.67 | 0.000 | 3.67 |
| 51.673 | 6924.37 | 55.4 | 1.092 | 55.3 | 17.4 | 7.52 | 1.97% | Coarse Alluvium | 0.056 | 111.0 | 2.85 | 0.000 | 2.85 | 0 | 18 | 2.08% | 2.7 | 36% | 3.68 | 0.000 | 3.68 |
| 51.837 | 6924.20 | 60.7 | 1.308 | 60.6 | 17.4 | 7.54 | 2.15% | Coarse Alluvium | 0.056 | 111.0 | 2.86 | 0.000 | 2.86 | 0 | 20 | 2.26% | 2.7 | 36% | 3.69 | 0.000 | 3.69 |
| 52.001 | 6924.04 | 48.7 | 1.307 | 48.6 | 16.9 | 7.32 | 2.68% | Coarse Alluvium | 0.056 | 111.0 | 2.87 | 0.000 | 2.87 | 0 | 16 | 2.85% | 2.8 | 36% | 3.70 | 0.000 | 3.70 |
| 52.165 | 6923.88 | 35.0 | 1.231 | 34.9 | 16.5 | 7.16 | 3.51% | Coarse Alluvium | 0.056 | 111.0 | 2.88 | 0.000 | 2.88 | 0 | 11 | 3.83% | 3.0 | 36% | 3.71 | 0.000 | 3.71 |
| 52.329 | 6923.71 | 25.8 | 1.019 | 25.7 | 18.4 | 7.97 | 3.95% | Coarse Alluvium | 0.056 | 111.0 | 2.89 | 0.000 | 2.89 | 0 | 8 | 4.45% | 3.2 | 36% | 3.72 | 0.000 | 3.72 |
| 52.493 | 6923.55 | 27.7 | 1.001 | 27.5 | 21.5 | 9.31 | 3.62% | Coarse Alluvium | 0.056 | 111.0 | 2.90 | 0.000 | 2.90 | 0 | 9 | 4.04% | 3.1 | 36% | 3.73 | 0.000 | 3.73 |
| 52.657 | 6923.38 | 60.6 | 1.317 | 60.5 | 25.5 | 11.06 | 2.17% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 20 | 2.28% | 2.7 | 36% | 3.74 | 0.000 | 3.74 |
| 52.821 | 6923.21 | 49.3 | 1.528 | 49.1 | 22.5 | 9.76 | 3.10% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 16 | 3.30% | 2.9 | 36% | 3.74 | 0.000 | 3.74 |
| 52.985 | 6923.06 | 30.4 | 1.330 | 30.3 | 21.5 | 9.31 | 4.37% | Coarse Alluvium | 0.056 | 111.0 | 2.92 | 0.000 | 2.92 | 0 | 9 | 4.84% | 3.1 | 36% | 3.75 | 0.000 | 3.75 |
| 53.149 | 6922.89 | 22.3 | 0.950 | 22.1 | 23.0 | 9.96 | 4.27% | Coarse Alluvium | 0.056 | 111.0 | 2.93 | 0.000 | 2.93 | 0 | 7 | 4.91% | 3.3 | 36% | 3.76 | 0.000 | 3.76 |
| 53.313 | 6922.73 | 20.4 | 0.525 | 20.3 | 25.7 | 11.15 | 2.57% | Coarse Alluvium | 0.056 | 111.0 | 2.94 | 0.000 | 2.94 | 0 | 6 | 3.00% | 3.2 | 36% | 3.77 | 0.000 | 3.77 |
| 53.477 | 6922.56 | 18.8 | 0.445 | 18.6 | 27.4 | 11.88 | 2.37% | Coarse Alluvium | 0.056 | 111.0 | 2.95 | 0.000 | 2.95 | 0 | 5 | 2.81% | 3.2 | 36% | 3.78 | 0.000 | 3.78 |
| 53.641 | 6922.40 | 19.9 | 0.592 | 19.7 | 29.5 | 12.77 | 2.98% | Coarse Alluvium | 0.056 | 111.0 | 2.96 | 0.000 | 2.96 | 0 | 6 | 3.50% | 3.2 | 36% | 3.79 | 0.000 | 3.79 |
| 53.805 | 6922.23 | 19.2 | 0.692 | 19.1 | 29.6 | 12.83 | 3.60% | Coarse Alluvium | 0.056 | 111.0 | 2.97 | 0.000 | 2.97 | 0 | 5 | 4.25% | 3.3 | 36% | 3.80 | 0.000 | 3.80 |
| 53.969 | 6922.07 | 25.0 | 0.992 | 24.8 | 30.4 | 13.16 | 3.97% | Coarse Alluvium | 0.056 | 111.0 | 2.98 | 0.000 | 2.98 | 0 | 7 | 4.51% | 3.2 | 36% | 3.81 | 0.000 | 3.81 |
| 54.133 | 6921.91 | 55.5 | 0.983 | 55.3 | 32.3 | 13.99 | 1.77% | Coarse Alluvium | 0.056 | 111.0 | 2.99 | 0.000 | 2.99 | 0 | 18 | 1.87% | 2.7 | 36% | 3.82 | 0.000 | 3.82 |
| 54.297 | 6921.74 | 67.1 | 0.985 | 66.9 | 28.4 | 12.32 | 1.47% | Coarse Alluvium | 0.056 | 111.0 | 3.00 | 0.000 | 3.00 | 0 | 21 | 1.54% | 2.6 | 36% | 3.83 | 0.000 | 3.83 |
| 54.461 | 6921.58 | 69.8 | 0.720 | 69.6 | 25.3 | 10.94 | 1.03% | Coarse Alluvium | 0.056 | 111.0 | 3.01 | 0.000 | 3.01 | 0 | 22 | 1.08% | 2.5 | 36% | 3.84 | 0.000 | 3.84 |
| 54.625 | 6921.41 | 62.1 | 0.726 | 62.0 | 22.7 | 9.82 | 1.17% | Coarse Alluvium | 0.056 | 111.0 | 3.01 | 0.000 | 3.01 | 0 | 20 | 1.23% | 2.5 | 36% | 3.84 | 0.000 | 3.84 |
| 54.789 | 6921.25 | 52.4 | 0.899 | 52.3 | 5.0 | 2.18 | 1.72% | Coarse Alluvium | 0.056 | 111.0 | 3.02 | 0.000 | 3.02 | 0 | 16 | 1.82% | 2.7 | 36% | 3.85 | 0.000 | 3.85 |
| 54.953 | 6921.09 | 48.3 | 0.966 | 48.3 | 5.0 | 2.15 | 2.00% | Coarse Alluvium | 0.056 | 111.0 | 3.03 | 0.000 | 3.03 | 0 | 15 | 2.13% | 2.8 | 36% | 3.86 | 0.000 | 3.86 |
| 55.117 | 6920.92 | 67.2 | 1.039 | 67.1 | 5.4 | 2.36 | 1.55% | Coarse Alluvium | 0.056 | 111.0 | 3.04 | 0.000 | 3.04 | 0 | 21 | 1.62% | 2.6 | 36% | 3.87 | 0.000 | 3.87 |
| 55.281 | 6920.76 | 84.1 | 0.932 | 84.1 | 4.3 | 1.85 | 1.11% | Coarse Alluvium | 0.056 | 111.0 | 3.05 | 0.000 | 3.05 | 0 | 27 | 1.15% | 2.4 | 36% | 3.88 | 0.000 | 3.88 |
| 55.446 | 6920.59 | 83.1 | 0.812 | 83.1 | 4.3 | 1.85 | 0.98% | Coarse Alluvium | 0.056 | 111.0 | 3.06 | 0.000 | 3.06 | 0 | 26 | 1.01% | 2.4 | 36% | 3.89 | 0.000 | 3.89 |
| 55.610 | 6920.43 | 75.9 | 0.925 | 75.9 | 4.4 | 1.91 | 1.22% | Coarse Alluvium | 0.056 | 111.0 | 3.07 | 0.000 | 3.07 | 0 | 24 | 1.27% | 2.5 | 36% | 3.90 | 0.000 | 3.90 |
| 55.774 | 6920.27 | 62.9 | 0.964 | 62.9 | 4.0 | 1.73 | 1.53% | Coarse Alluvium | 0.056 | 111.0 | 3.08 | 0.000 | 3.08 | 0 | 19 | 1.61% | 2.6 | 36% | 3.91 | 0.000 | 3.91 |
| 55.938 | 6920.10 | 59.2 | 0.890 | 59.2 | 4.0 | 1.73 | 1.50% | Coarse Alluvium | 0.056 | 111.0 | 3.09 | 0.000 | 3.09 | 0 | 18 | 1.59% | 2.6 | 36% | 3.92 | 0.000 | 3.92 |
| 56.102 | 6919.94 | 55.0 | 0.960 | 54.9 | 4.1 | 1.77 | 1.75% | Coarse Alluvium | 0.056 | 111.0 | 3.10 | 0.000 | 3.10 | 0 | 17 | 1.85% | 2.7 | 36% | 3.93 | 0.000 | 3.93 |
| 56.266 | 6919.77 | 49.2 | 1.187 | 49.2 | 3.9 | 1.71 | 2.41% | Coarse Alluvium | 0.056 | 111.0 | 3.11 | 0.000 | 3.11 | 0 | 15 | 2.57% | 2.8 | 36% | 3.94 | 0.000 | 3.94 |
| 56.430 | 6919.61 | 42.4 | 1.546 | 42.4 | 3.9 | 1.71 | 3.64% | Coarse Alluvium | 0.056 | 111.0 | 3.11 | 0.000 | 3.11 | 0 | 13 | 3.93% | 3.0 | 36% | 3.94 | 0.000 | 3.94 |
| 56.594 | 6919.45 | 42.7 | 1.380 | 42.7 | 4.0 | 1.73 | 3.23% | Coarse Alluvium | 0.056 | 111.0 | 3.12 | 0.000 | 3.12 | 0 | 13 | 3.48% | 3.0 | 36% | 3.95 | 0.000 | 3.95 |
| 56.758 | 6919.28 | 65.0 | 1.396 | 65.0 | 4.0 | 1.75 | 2.15% | Coarse Alluvium | 0.056 | 111.0 | 3.13 | 0.000 | 3.13 | 0 | 20 | 2.26% | 2.7 | 36% | 3.96 | 0.000 | 3.96 |
| 56.922 | 6919.12 | 104.5 | 1.907 | 104.5 | 4.8 | 2.08 | 1.82% | Coarse Alluvium | 0.056 | 111.0 | 3.14 | 0.000 | 3.14 | 0 | 32 | 1.88% | 2.5 | 36% | 3.97 | 0.000 | 3.97 |
| 57.086 | 6918.95 | 101.6 | 2.343 | 101.6 | 4.4 | 1.89 | 2.31% | Coarse Alluvium | 0.056 | 111.0 | 3.15 | 0.000 | 3.15 | 0 | 31 | 2.38% | 2.5 | 36% | 3.98 | 0.000 | 3.98 |
| 57.250 | 6918.79 | 97.9 | 2.432 | 97.9 | 4.6 | 1.97 | 2.48% | Coarse Alluvium | 0.056 | 111.0 | 3.16 | 0.000 | 3.16 | 0 | 30 | 2.57% | 2.6 | 36% | 3.99 | 0.000 | 3.99 |
| 57.414 | 6918.63 | 83.6 | 2.203 | 83.6 | 4.3 | 1.87 | 2.64% | Coarse Alluvium | 0.056 | 111.0 | 3.17 | 0.000 | 3.17 | 0 | 25 | 2.74% | 2.6 | 36% | 4.00 | 0.000 | 4.00 |
| 57.578 | 6918.46 | 62.2 | 1.815 | 62.2 | 3.7 | 1.59 | 2.92% | Coarse Alluvium | 0.056 | 111.0 | 3.18 | 0.000 | 3.18 | 0 | 19 | 3.07% | 2.8 | 36% | 4.01 | 0.000 | 4.01 |
| 57.742 | 6918.30 | 56.2 | 1.789 | 56.2 | 3.9 | 1.67 | 3.18% | Coarse Alluvium | 0.056 | 111.0 | 3.19 | 0.000 | 3.19 | 0 | 17 | 3.38% | 2.8 | 36% | 4.02 | 0.000 | 4.02 |
| 57.906 | 6918.13 | 72.6 | 1.932 | 72.6 | 4.2 | 1.81 | 2.66% | Coarse Alluvium | 0.056 | 111.0 | 3.20 | 0.000 | 3.20 | 0 | 22 | 2.78% | 2.7 | 36% | 4.03 | 0.000 | 4.03 |
| 58.070 | 6917.97 | 58.2 | 2.197 | 58.1 | 4.3 | 1.85 | 3.78% | Coarse Alluvium | 0.056 | 111.0 | 3.21 | 0.000 | 3.21 | 0 | 17 | 4.00% | 2.9 | 36% | 4.04 | 0.000 | 4.04 |
| 58.234 | 6917.81 | 45.7 | 1.766 | 45.6 | 4.1 | 1.77 | 3.87% | Coarse Alluvium | 0.056 | 111.0 | 3.22 | 0.000 | 3.22 | 0 | 13 | 4.16% | 3.0 | 36% | 4.04 | 0.000 | 4.04 |
| 58.398 | 6917.64 | 44.7 | 1.643 | 44.7 | 4.9 | 2.14 | 3.67% | Coarse Alluvium | 0.056 | 111.0 | 3.22 | 0.000 | 3.22 | 0 | 13 | 3.96% | 3.0 | 36% | 4.05 | 0.000 | 4.05 |
| 58.562 | 6917.48 | 121.5 | 2.456 | 121.5 | 6.8 | 2.95 | 2.02% | Coarse Alluvium | 0.056 | 111.0 | 3.23 | 0.000 | 3.23 | 0 | 37 | 2.08% | 2.4 | 36% | 4.06 | 0.000 | 4.06 |
| 58.726 | 6917.31 | 252.7 | 2.868 | 252.7 | 7.4 | 3.21 | 1.13% | Coarse Alluvium | 0.056 | 111.0 | 3.24 | 0.000 | 3.24 | 0 | 77 | 1.15% | 2.0 | 36% | 4.07 | 0.000 | 4.07 |
| 58.890 | 6917.15 | 290.8 | 4.380 | 290.8 | 8.2 | 3.54 | 1.51% | Coarse Alluvium | 0.056 | 111.0 | 3.25 | 0.000 | 3.25 | 0 | 88 | 1.52% | 2.1 | 36% | 4.08 | 0.000 | 4.08 |
| 59.054 | 6916.99 | 233.5 | 4.940 | 233.5 | 8.2 | 3.54 | 2.12% | Coarse Alluvium | 0.056 | 111.0 | 3.26 | 0.000 | 3.26 | 0 | 71 | 2.15% | 2.2 | 36% | 4.09 | 0.000 | 4.09 |
| 59.218 | 6916.82 | 179.0 | 4.573 | 178.9 | 7.9 | 3.44 | 2.56% | Coarse Alluvium | 0.056 | 111.0 | 3.27 | 0.000 | 3.27 | 0 | 54 | 2.60% | 2.4 | 36% | 4.10 | 0.000 | 4.10 |
| 59.382 | 6916.66 | 125.7 | 3.556 | 125.7 | 7.6 | 3.28 | 2.83% | Coarse Alluvium | 0.056 | 111.0 | 3.28 | 0.000 | 3.28 | 0 | 37 | 2.90% | 2.5 | 36% | 4.11 | 0.000 | 4.11 |
| 59.547 | 6916.49 | 86.3 | 3.115 | 86.3 | 7.2 | 3.13 | 3.61% | Coarse Alluvium | 0.056 | 111.0 | 3.29 | 0.000 | 3.29 | 0 | 25 | 3.75% | 2.7 | 36% | 4.12 | 0.000 | 4.12 |
| 59.711 | 6916.33 | 83.3 | 2.586 | 83.3 | 7.5 | 3.23 | 3.10% | Coarse Alluvium | 0.056 | 111.0 | 3.30 | 0.000 | 3.30 | 0 | 24 | 3.23% | 2.7 | 36% | 4.13 | 0.000 | 4.13 |
| 59.875 | 6916.17 | 121.5 | 1.784 | 121.5 | 8.1 | 3.52 | 1.47% | Coarse Alluvium | 0.056 | 111.0 | 3.31 | 0.000 | 3.31 | 0 | 36 | 1.51% | 2.4 | 36% | 4.14 | 0.000 | 4.14 |
| 60.039 | 6916.00 | 134.5 | 2.632 | 134.4 | 8.2 | 3.54 | 1.96% | Coarse Alluvium | 0.056 | 111.0 | 3.32 | 0.000 | 3.32 | 0 | 40 | 2.01% | 2.4 | 36% | 4.14 | 0.000 | 4.14 |
| 60.203 | 6915.84 | 119.0 | 3.618 | 119.0 | 7.9 | 3.44 | 3.04% | Coarse Alluvium | 0.056 | 111.0 | 3.32 | 0.000 | 3.32 | 0 | 35 | 3.13% | 2.6 | 36% | 4.15 | 0.000 | 4.15 |
| 60.367 | 6915.67 | 101.5 | 4.702 | 101.5 | 8.0 | 3.48 | 4.63% | Coarse Alluvium | 0.056 | 111.0 | 3.33 | 0.000 | 3.33 | 0 | 29 | 4.79% | 2.8 | 36% | 4.16 | 0.000 | 4.16 |
| 60.531 | 6915.51 | 143.1 | 6.286 | 143.0 | 14.4 | 6.22 | 4.39% | Coal | - | - | - | - | - | - | - | - | - | - | - | - | |
| 60.695 | 6915.35 | 273.4 | 6.286 | 273.3 | 21.3 | 9.23 | 2.30% | Coal | - | - | - | - | - | - | - | - | - | - | - | - | |
| 60.859 | 6915.18 | 426.4 | 6.286 | 426.2 | 32.6 | 14.14 | 1.47% | Coal | - | - | - | - | - | - | - | - | - | - | - | - | |

| | | | | | | | | | | | | | | | | | | | | | | | | | |
|-------|------|---------|---------|----|--------|---------|---|-------|-------|-----|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 19.98 | 1220 | 1.7E-03 | 2.6E+03 | 18 | 0.0313 | 8.7E-06 | 0 | 0.281 | 0.289 | 316 | 0.281 | 5864 | 4000 | 1712 | 5864 | 0.001% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 20.03 | 1220 | 1.7E-03 | 2.6E+03 | 18 | 0.0307 | 8.6E-06 | 0 | 0.282 | 0.289 | 317 | 0.282 | 5858 | 3996 | 1713 | 585 | | | | | | | | | | |

| | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer (tsf) | Total Stress at Midpoint of Layer (tsf) | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Effectiv e Stress at Midpoin t of Layer |
|---------------------|----------------------------------|--|-------------------------------------|-------------------------------|-------------------------|----------------------|--|--|---|---|--|--|
| Proposed Repository | | | | | | | | | | | | |
| Erosion Protection | 6990.3 | 6989.5 | 6988.8 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6988.8 | 6987.3 | 6985.8 | 3.0 | 0.057 | 114.7 | 0.264 | 0.178 | 0.00 | 0.00 | 0.264 | 0.178 |
| Mine Spoils | 6985.8 | 6980.9 | 6976.0 | 9.7 | 0.058 | 116.4 | 0.830 | 0.547 | 0.00 | 0.00 | 0.830 | 0.547 |

| | |
|---------|--|
| 6976.04 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6990.26 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|---|
| 0.83 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{repos}$ (psf) |
| 6931.50 | Elevation of bottom of tailings (ft amsl) |

| UNC-NECR WASTE REPOSITORY SEISMIC SETTLEMENT ANALYSIS - CPT-10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--|-----------|-------|-------|-------|---------|---------|-------|--|-------------------|---------------------------------------|-----------------------------|------------------------------------|---------------------------------------|--------------------------------|--|---|----------------------------|------|--------------------------------------|--|--|--|--|--|--|--|--|--|--|
| Data File: 13-52118_RP10-BSC-CPT | | | | | | | | | | Idriss and Boulanger (2008) | | | | | 6883.40 | | | | | | | | | | | | | | |
| Location: UNC-NECR 2013 Mill Site PDS | | | | | | | | | | Max. Horiz. Acceleration, Amax/g: 0.3 | | | | | Water surface elevation during CPT investigation (ft amsl) | | | | | | | | | | | | | | |
| http://projects.mwhglobal.com/.../13-52118_RP10-BSC-CPT.XLS | | | | | | | | | | Earthquake Moment Magnitude, M: 5.5 | | | | | 6883.40 | | | | | | | | | | | | | | |
| Erosion Protection | | | | | | | | | | Magnitude Scaling Factor, MSF: 1.69 | | | | | 6883.40 | | | | | | | | | | | | | | |
| Cover Soil | | | | | | | | | | Youd, et al (2001) | | | | | 1.44 | | | | | | | | | | | | | | |
| Mine Spoils | | | | | | | | | | Max. Horiz. Acceleration, Amax/g: 0.3 | | | | | 0.47 | | | | | | | | | | | | | | |
| Radon Barrier | | | | | | | | | | Earthquake Moment Magnitude, M: 6.3 | | | | | 8.26 | | | | | | | | | | | | | | |
| General Fill | | | | | | | | | | Magnitude Scaling Factor, MSF: 1.59 | | | | | Equiv. Number of Uniform Strain Cycles, N | | | | | | | | | | | | | | |
| 2013 CPT Data from ConeTec | | | | | | | | | | CPT Data Interpretations | | | | | | | | | | Conditions at t _i | | | | | | | | | |
| Depth at time of CPT | Elevation | qt | fs | qc | Pw (u2) | Pw (u2) | fs/qt | Material Type (per drilling log from coupled borehole) | Unit Weight (pcf) | Unit Weight (pcf) | Stress at time of CPT (tsf) | Pore Pressure at time of CPT (tsf) | Effective Stress at time of CPT (tsf) | Saturated at time of CPT 1=Yes | Normalized Cone Penetration Resistance, Q _c | Normalized Friction Ratio, F _r (%) | Type Index, I _c | FC % | Total Stress at t _i (tsf) | Equip Pore Pressure at t _i , u _i (tsf) | Effective Stress at t _i (tsf) | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 0.164 | 6973.44 | 3.5 | 0.010 | 3.3 | 35.8 | 15.52 | 0.29% | Radon Barrier | 0.061 | 122.3 | 0.01 | 0.000 | 0.01 | 0 | 345 | 0.29% | 1.2 | 59% | 1.23 | 0.000 | 1.23 | | | | | | | | |
| 0.328 | 6973.27 | 4.6 | 0.108 | 4.5 | 8.5 | 3.66 | 2.36% | Radon Barrier | 0.061 | 122.3 | 0.02 | 0.000 | 0.02 | 0 | 227 | 2.37% | 1.9 | 59% | 1.24 | 0.000 | 1.24 | | | | | | | | |
| 0.492 | 6973.11 | 11.3 | 0.106 | 11.3 | 5.3 | 2.28 | 0.94% | Radon Barrier | 0.061 | 122.3 | 0.03 | 0.000 | 0.03 | 0 | 375 | 0.94% | 1.5 | 59% | 1.25 | 0.000 | 1.25 | | | | | | | | |
| 0.656 | 6972.94 | 30.1 | 0.223 | 30.0 | 9.0 | 3.90 | 0.74% | Radon Barrier | 0.061 | 122.3 | 0.04 | 0.000 | 0.04 | 0 | 749 | 0.74% | 1.2 | 59% | 1.26 | 0.000 | 1.26 | | | | | | | | |
| 0.820 | 6972.78 | 48.2 | 0.257 | 48.2 | 1.7 | 0.73 | 0.53% | Radon Barrier | 0.061 | 122.3 | 0.05 | 0.000 | 0.05 | 0 | 960 | 0.53% | 1.1 | 59% | 1.27 | 0.000 | 1.27 | | | | | | | | |
| 0.984 | 6972.62 | 75.1 | 0.280 | 75.0 | 8.6 | 3.74 | 0.37% | Radon Barrier | 0.061 | 122.3 | 0.06 | 0.000 | 0.06 | 0 | 1246 | 0.37% | 0.9 | 59% | 1.28 | 0.000 | 1.28 | | | | | | | | |
| 1.148 | 6972.45 | 70.6 | 0.336 | 70.6 | 2.4 | 1.02 | 0.48% | Radon Barrier | 0.061 | 122.3 | 0.07 | 0.000 | 0.07 | 0 | 1005 | 0.48% | 1.0 | 59% | 1.29 | 0.000 | 1.29 | | | | | | | | |
| 1.312 | 6972.29 | 66.8 | 0.393 | 66.8 | 3.9 | 1.69 | 0.59% | Radon Barrier | 0.061 | 122.3 | 0.08 | 0.000 | 0.08 | 0 | 832 | 0.59% | 1.1 | 59% | 1.30 | 0.000 | 1.30 | | | | | | | | |
| 1.476 | 6972.12 | 68.0 | 0.456 | 68.0 | 1.7 | 0.75 | 0.67% | Radon Barrier | 0.061 | 122.3 | 0.09 | 0.000 | 0.09 | 0 | 752 | 0.67% | 1.2 | 59% | 1.31 | 0.000 | 1.31 | | | | | | | | |
| 1.640 | 6971.96 | 68.8 | 0.760 | 68.8 | 0.9 | 0.41 | 1.11% | Radon Barrier | 0.061 | 122.3 | 0.10 | 0.000 | 0.10 | 0 | 685 | 1.11% | 1.4 | 59% | 1.32 | 0.000 | 1.32 | | | | | | | | |
| 1.804 | 6971.80 | 75.0 | 1.185 | 75.0 | 2.1 | 0.89 | 1.58% | Radon Barrier | 0.061 | 122.3 | 0.11 | 0.000 | 0.11 | 0 | 679 | 1.58% | 1.6 | 59% | 1.33 | 0.000 | 1.33 | | | | | | | | |
| 1.968 | 6971.63 | 105.1 | 1.817 | 105.1 | 7.3 | 3.17 | 1.73% | Radon Barrier | 0.061 | 122.3 | 0.12 | 0.000 | 0.12 | 0 | 872 | 1.73% | 1.6 | 59% | 1.34 | 0.000 | 1.34 | | | | | | | | |
| 2.133 | 6971.47 | 132.3 | 3.007 | 132.2 | 11.7 | 5.08 | 2.27% | General Fill | 0.057 | 113.8 | 0.13 | 0.000 | 0.13 | 0 | 1019 | 2.28% | 1.6 | 48% | 1.35 | 0.000 | 1.35 | | | | | | | | |
| 2.297 | 6971.30 | 159.7 | 4.044 | 159.6 | 17.5 | 7.57 | 2.53% | General Fill | 0.057 | 113.8 | 0.14 | 0.000 | 0.14 | 0 | 1148 | 2.53% | 1.7 | 48% | 1.36 | 0.000 | 1.36 | | | | | | | | |
| 2.461 | 6971.14 | 171.5 | 5.127 | 171.4 | 23.0 | 9.95 | 2.99% | General Fill | 0.057 | 113.8 | 0.15 | 0.000 | 0.15 | 0 | 1155 | 2.99% | 1.7 | 48% | 1.37 | 0.000 | 1.37 | | | | | | | | |
| 2.625 | 6970.98 | 168.1 | 5.694 | 168.1 | 11.6 | 5.02 | 3.39% | General Fill | 0.057 | 113.8 | 0.16 | 0.000 | 0.16 | 0 | 1065 | 3.39% | 1.8 | 48% | 1.38 | 0.000 | 1.38 | | | | | | | | |
| 2.789 | 6970.81 | 152.4 | 5.098 | 152.3 | 5.4 | 2.36 | 3.35% | General Fill | 0.057 | 113.8 | 0.17 | 0.000 | 0.17 | 0 | 911 | 3.35% | 1.8 | 48% | 1.39 | 0.000 | 1.39 | | | | | | | | |
| 2.953 | 6970.65 | 136.3 | 4.745 | 136.3 | 2.2 | 0.94 | 3.48% | General Fill | 0.057 | 113.8 | 0.18 | 0.000 | 0.18 | 0 | 772 | 3.49% | 1.9 | 48% | 1.40 | 0.000 | 1.40 | | | | | | | | |
| 3.117 | 6970.48 | 116.0 | 4.208 | 115.9 | 1.7 | 0.73 | 3.63% | General Fill | 0.057 | 113.8 | 0.19 | 0.000 | 0.19 | 0 | 623 | 3.63% | 1.9 | 48% | 1.41 | 0.000 | 1.41 | | | | | | | | |
| 3.281 | 6970.32 | 100.2 | 3.982 | 100.1 | 22.8 | 9.86 | 3.97% | General Fill | 0.057 | 113.8 | 0.20 | 0.000 | 0.20 | 0 | 513 | 3.98% | 2.0 | 48% | 1.42 | 0.000 | 1.42 | | | | | | | | |
| 3.445 | 6970.16 | 91.8 | 3.420 | 91.7 | 5.0 | 2.15 | 3.73% | General Fill | 0.057 | 113.8 | 0.20 | 0.000 | 0.20 | 0 | 448 | 3.74% | 2.0 | 48% | 1.43 | 0.000 | 1.43 | | | | | | | | |
| 3.609 | 6969.99 | 81.4 | 3.048 | 81.4 | 5.0 | 2.18 | 3.74% | General Fill | 0.057 | 113.8 | 0.21 | 0.000 | 0.21 | 0 | 380 | 3.75% | 2.0 | 48% | 1.44 | 0.000 | 1.44 | | | | | | | | |
| 3.773 | 6969.83 | 82.5 | 2.961 | 82.4 | 20.8 | 9.01 | 3.59% | General Fill | 0.057 | 113.8 | 0.22 | 0.000 | 0.22 | 0 | 369 | 3.60% | 2.0 | 48% | 1.45 | 0.000 | 1.45 | | | | | | | | |
| 3.937 | 6969.66 | 88.9 | 2.980 | 88.9 | 6.2 | 2.70 | 3.35% | General Fill | 0.057 | 113.8 | 0.23 | 0.000 | 0.23 | 0 | 382 | 3.36% | 2.0 | 48% | 1.46 | 0.000 | 1.46 | | | | | | | | |
| 4.101 | 6969.50 | 94.1 | 3.569 | 94.1 | 6.1 | 2.64 | 3.79% | General Fill | 0.057 | 113.8 | 0.24 | 0.000 | 0.24 | 0 | 388 | 3.80% | 2.0 | 48% | 1.47 | 0.000 | 1.47 | | | | | | | | |
| 4.265 | 6969.33 | 110.6 | 4.227 | 110.6 | 8.8 | 3.80 | 3.82% | General Fill | 0.057 | 113.8 | 0.25 | 0.000 | 0.25 | 0 | 440 | 3.83% | 2.0 | 48% | 1.48 | 0.000 | 1.48 | | | | | | | | |
| 4.429 | 6969.17 | 119.7 | 4.319 | 119.6 | 5.1 | 2.22 | 3.61% | General Fill | 0.057 | 113.8 | 0.26 | 0.000 | 0.26 | 0 | 459 | 3.62% | 2.0 | 48% | 1.48 | 0.000 | 1.48 | | | | | | | | |
| 4.593 | 6969.01 | 108.1 | 3.869 | 108.0 | 4.5 | 1.93 | 3.58% | General Fill | 0.057 | 113.8 | 0.27 | 0.000 | 0.27 | 0 | 400 | 3.59% | 2.0 | 48% | 1.49 | 0.000 | 1.49 | | | | | | | | |
| 4.757 | 6968.84 | 88.7 | 3.214 | 88.7 | 1.7 | 0.75 | 3.62% | General Fill | 0.057 | 113.8 | 0.28 | 0.000 | 0.28 | 0 | 317 | 3.64% | 2.0 | 48% | 1.50 | 0.000 | 1.50 | | | | | | | | |
| 4.921 | 6968.68 | 73.0 | 2.450 | 72.9 | 4.8 | 2.09 | 3.36% | General Fill | 0.057 | 113.8 | 0.29 | 0.000 | 0.29 | 0 | 252 | 3.37% | 2.0 | 48% | 1.51 | 0.000 | 1.51 | | | | | | | | |
| 5.085 | 6968.51 | 67.2 | 2.274 | 67.2 | 1.4 | 0.61 | 3.38% | General Fill | 0.057 | 113.8 | 0.30 | 0.000 | 0.30 | 0 | 225 | 3.40% | 2.1 | 48% | 1.52 | 0.000 | 1.52 | | | | | | | | |
| 5.249 | 6968.35 | 70.1 | 2.479 | 70.1 | 1.2 | 0.53 | 3.54% | General Fill | 0.057 | 113.8 | 0.31 | 0.000 | 0.31 | 0 | 227 | 3.55% | 2.1 | 48% | 1.53 | 0.000 | 1.53 | | | | | | | | |
| 5.413 | 6968.19 | 79.2 | 2.592 | 79.2 | 2.5 | 1.10 | 3.27% | General Fill | 0.057 | 113.8 | 0.32 | 0.000 | 0.32 | 0 | 249 | 3.28% | 2.0 | 48% | 1.54 | 0.000 | 1.54 | | | | | | | | |
| 5.577 | 6968.02 | 117.1 | 2.461 | 117.1 | 6.0 | 2.58 | 2.10% | General Fill | 0.057 | 113.8 | 0.33 | 0.000 | 0.33 | 0 | 359 | 2.11% | 1.8 | 48% | 1.55 | 0.000 | 1.55 | | | | | | | | |
| 5.741 | 6967.86 | 136.7 | 2.097 | 136.7 | 4.2 | 1.83 | 1.53% | General Fill | 0.057 | 113.8 | 0.34 | 0.000 | 0.34 | 0 | 407 | 1.54% | 1.6 | 48% | 1.56 | 0.000 | 1.56 | | | | | | | | |
| 5.905 | 6967.69 | 145.9 | 1.683 | 145.9 | 5.5 | 2.38 | 1.15% | General Fill | 0.057 | 113.8 | 0.34 | 0.000 | 0.34 | 0 | 423 | 1.16% | 1.5 | 48% | 1.57 | 0.000 | 1.57 | | | | | | | | |
| 6.069 | 6967.53 | 149.2 | 1.876 | 149.2 | 5.4 | 2.34 | 1.26% | General Fill | 0.057 | 113.8 | 0.35 | 0.000 | 0.35 | 0 | 421 | 1.26% | 1.6 | 48% | 1.58 | 0.000 | 1.58 | | | | | | | | |
| 6.234 | 6967.37 | 147.8 | 1.838 | 147.6 | 4.2 | 1.83 | 1.25% | General Fill | 0.057 | 113.8 | 0.36 | 0.000 | 0.36 | 0 | 406 | 1.25% | 1.6 | 48% | 1.59 | 0.000 | 1.59 | | | | | | | | |
| 6.398 | 6967.20 | 145.8 | 2.024 | 145.8 | 3.0 | 1.30 | 1.39% | General Fill | 0.057 | 113.8 | 0.37 | 0.000 | 0.37 | 0 | 391 | 1.39% | 1.6 | 48% | 1.60 | 0.000 | 1.60 | | | | | | | | |
| 6.562 | 6967.04 | 139.7 | 2.297 | 139.7 | 2.9 | 1.26 | 1.64% | General Fill | 0.057 | 113.8 | 0.38 | 0.000 | 0.38 | 0 | 365 | 1.65% | 1.7 | 48% | 1.61 | 0.000 | 1.61 | | | | | | | | |
| 6.726 | 6966.87 | 120.4 | 3.073 | 120.4 | 2.8 | 1.22 | 2.55% | General Fill | 0.057 | 113.8 | 0.39 | 0.000 | 0.39 | 0 | 307 | 2.56% | 1.9 | 48% | 1.62 | 0.000 | 1.62 | | | | | | | | |
| 6.890 | 6966.71 | 92.0 | 3.381 | 92.0 | 3.3 | 1.42 | 3.68% | Coarse Tailings | 0.054 | 108.1 | 0.40 | 0.000 | 0.40 | 0 | 229 | 3.69% | 2.1 | 21% | 1.62 | 0.000 | 1.62 | | | | | | | | |
| 7.054 | 6966.55 | 74.6 | 2.934 | 74.6 | 7.3 | 3.15 | 3.93% | Coarse Tailings | 0.054 | 108.1 | 0.41 | 0.000 | 0.41 | 0 | 182 | 3.95% | 2.2 | 21% | 1.63 | 0.000 | 1.63 | | | | | | | | |
| 7.218 | 6966.38 | 196.1 | 3.150 | 195.9 | 28.4 | 12.30 | 1.61% | Coarse Tailings | 0.054 | 108.1 | 0.42 | 0.000 | 0.42 | 0 | 469 | 1.61% | 1.6 | 21% | 1.64 | 0.000 | 1.64 | | | | | | | | |
| 7.382 | 6966.22 | 197.9 | 3.877 | 197.8 | 12.8 | 5.53 | 1.71% | Coarse Tailings | 0.054 | 108.1 | 0.43 | 0.000 | 0.43 | 0 | 463 | 1.71% | 1.7 | 21% | 1.65 | 0.000 | 1.65 | | | | | | | | |
| 7.546 | 6966.05 | 115.4 | 2.330 | 115.4 | 0.7 | 0.30 | 2.45% | Coarse Tailings | 0.054 | 108.1 | 0.44 | 0.000 | 0.44 | 0 | 264 | 2.46% | 1.9 | 21% | 1.66 | 0.000 | 1.66 | | | | | | | | |
| 7.710 | 6965.89 | 74.5 | 3.389 | 74.5 | 0.1 | 0.06 | 4.55% | Coarse Tailings | 0.054 | 108.1 | 0.44 | 0.000 | 0.44 | 0 | 167 | 4.58% | 2.3 | 21% | 1.67 | 0.000 | 1.67 | | | | | | | | |
| 7.874 | 6965.73 | 70.5 | 3.113 | 70.5 | 4.7 | 2.03 | 4.41% | Coarse Tailings | 0.054 | 108.1 | 0.45 | 0.000 | 0.45 | 0 | 155 | 4.44% | 2.3 | 21% | 1.68 | 0.000 | 1.68 | | | | | | | | |
| 8.038 | 6965.56 | 78.3 | 3.495 | 78.0 | 0.2 | 20.25 | 4.46% | Coarse Tailings | 0.054 | 108.1 | 0.46 | 0.000 | 0.46 | 0 | 168 | 4.49% | 2.2 | 21% | 1.69 | 0.000 | 1.69 | | | | | | | | |
| 8.202 | 6965.40 | 192.2 | 4.149 | 191.3 | 155.3 | 67.31 | 2.16% | Coarse Tailings | 0.054 | 108.1 | 0.47 | 0.000 | 0.47 | 0 | 407 | 2.16% | 1.8 | 21% | 1.70 | 0.000 | 1.70 | | | | | | | | |
| 8.366 | 6965.23 | 278.0 | 3.670 | 277.8 | 25.1 | 10.88 | 1.32% | Coarse Tailings | 0.054 | 108.1 | 0.48 | 0.000 | 0.48 | 0 | 578 | 1.32% | 1.5 | 21% | 1.70 | 0.000 | 1.70 | | | | | | | | |
| 8.530 | 6965.07 | 312.7 | 4.080 | 312.7 | 6.6 | 2.87 | 1.30% | Coarse Tailings | 0.054 | 108.1 | 0.49 | 0.000 | 0.49 | 0 | 639 | 1.31% | 1.5 | 21% | 1.71 | 0.000 | 1.71 | | | | | | | | |
| 8.694 | 6964.91 | 305.6 | 3.655 | 305.5 | 16.0 | 6.93 | 1.20% | Coarse Tailings | 0.054 | 108.1 | 0.50 | 0.000 | 0.50 | 0 | 613 | 1.20% | 1.5 | 21% | 1.72 | 0.000 | 1.72 | | | | | | | | |
| 8.858 | 6964.74 | 279.3 | 3.841 | 279.2 | 3.2 | 1.40 | 1.38% | Coarse Tailings | 0.054 | 108.1 | 0.51 | 0.000 | 0.51 | 0 | 551 | 1.38% | 1.5 | 21% | 1.73 | 0.000 | 1.73 | | | | | | | | |
| 9.022 | 6964.58 | 219.5 | 3.165 | 219.5 | 0.0 | 0.00 | 1.44% | Coarse Tailings | 0.054 | 108.1 | 0.52 | 0.000 | 0.52 | 0 | 425 | 1.45% | 1.6 | 21% | 1.74 | 0.000 | 1.74 | | | | | | | | |
| 9.186 | 6964.41 | 163.2 | 3.657 | 163.2 | -0.9 | -0.41 | 2.24% | Coarse Tailings | 0.054 | 108.1 | 0.52 | 0.000 | 0.52 | 0 | 310 | 2.25% | 1.9 | 21% | 1.75 | 0.000 | 1.75 | | | | | | | | |
| 9.350 | 6964.25 | 138.1 | 3.792 | 138.1 | 7.2 | 3.13 | 2.75% | Coarse Tailings | 0.054 | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|---|-------|-------|----|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 10.827 | 6962.77 | 50.0 | 0.437 | 50.0 | -1.6 | -0.67 | 0.87% | Coarse Tailings | 0.054 | 108.1 | 0.61 | 0.000 | 0.61 | 0 | 81 | 0.88% | 2.0 | 21% | 1.84 | 0.000 | 1.84 | 9.70 | 727 | 1.7E-03 | 8.9E+02 | 65 | 0.7453 | 3.0E-04 | 0 | 0.240 | 0.240 | 69 | 0.240 | 7782 | 5337 | 1465 | 7782 | 0.085% | 1.79 | 1.00 | 0.01% | 0.001339 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 10.991 | 6962.61 | 52.0 | 0.400 | 52.0 | 0.8 | 0.33 | 0.77% | Coarse Tailings | 0.054 | 108.1 | 0.62 | 0.000 | 0.62 | 0 | 83 | 0.78% | 1.9 | 21% | 1.85 | 0.000 | 1.85 | 9.75 | 727 | 1.7E-03 | 8.9E+02 | 64 | 0.7419 | 3.0E-04 | 0 | 0.240 | 0.241 | 70 | 0.240 | 7766 | 5327 | 1466 | 7766 | 0.085% | 1.79 | 1.00 | 0.01% | 0.001335 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.155 | 6962.45 | 56.5 | 0.416 | 56.5 | -1.3 | -0.55 | 0.74% | Coarse Tailings | 0.054 | 108.1 | 0.63 | 0.000 | 0.63 | 0 | 89 | 0.74% | 1.9 | 21% | 1.85 | 0.000 | 1.85 | 9.80 | 727 | 1.7E-03 | 8.9E+02 | 63 | 0.7384 | 3.0E-04 | 0 | 0.241 | 0.241 | 71 | 0.241 | 7751 | 5316 | 1467 | 7751 | 0.084% | 1.79 | 1.00 | 0.01% | 0.001332 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.319 | 6962.28 | 66.0 | 0.471 | 66.0 | -0.7 | -0.29 | 0.71% | Coarse Tailings | 0.054 | 108.1 | 0.64 | 0.000 | 0.64 | 0 | 102 | 0.72% | 1.8 | 21% | 1.86 | 0.000 | 1.86 | 9.85 | 727 | 1.7E-03 | 8.9E+02 | 63 | 0.7349 | 3.0E-04 | 0 | 0.241 | 0.241 | 72 | 0.241 | 7736 | 5305 | 1468 | 7736 | 0.084% | 1.79 | 1.00 | 0.01% | 0.001328 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.483 | 6962.12 | 84.8 | 0.502 | 84.8 | 0.6 | 0.26 | 0.59% | Coarse Tailings | 0.054 | 108.1 | 0.65 | 0.000 | 0.65 | 0 | 130 | 0.60% | 1.7 | 21% | 1.87 | 0.000 | 1.87 | 9.90 | 727 | 1.7E-03 | 8.9E+02 | 62 | 0.7314 | 3.0E-04 | 0 | 0.241 | 0.242 | 73 | 0.241 | 7721 | 5295 | 1469 | 7721 | 0.084% | 1.79 | 1.00 | 0.01% | 0.001324 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.647 | 6961.95 | 91.2 | 0.499 | 91.2 | 1.4 | 0.59 | 0.55% | Coarse Tailings | 0.054 | 108.1 | 0.66 | 0.000 | 0.66 | 0 | 138 | 0.55% | 1.6 | 21% | 1.88 | 0.000 | 1.88 | 9.95 | 727 | 1.7E-03 | 8.9E+02 | 61 | 0.7279 | 3.0E-04 | 0 | 0.241 | 0.242 | 74 | 0.241 | 7706 | 5284 | 1470 | 7706 | 0.084% | 1.79 | 1.00 | 0.01% | 0.001319 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.811 | 6961.79 | 88.1 | 0.486 | 88.1 | -0.3 | -0.12 | 0.50% | Coarse Tailings | 0.054 | 108.1 | 0.67 | 0.000 | 0.67 | 0 | 131 | 0.56% | 1.7 | 21% | 1.89 | 0.000 | 1.89 | 10.00 | 727 | 1.7E-03 | 8.9E+02 | 60 | 0.7243 | 3.0E-04 | 0 | 0.242 | 0.242 | 75 | 0.242 | 7691 | 5274 | 1471 | 7691 | 0.083% | 1.79 | 1.00 | 0.01% | 0.001314 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 11.975 | 6961.63 | 77.7 | 0.504 | 77.7 | -1.0 | -0.43 | 0.65% | Coarse Tailings | 0.054 | 108.1 | 0.67 | 0.000 | 0.67 | 0 | 114 | 0.65% | 1.8 | 21% | 1.90 | 0.000 | 1.90 | 10.05 | 727 | 1.7E-03 | 8.9E+02 | 60 | 0.7207 | 3.0E-04 | 0 | 0.242 | 0.243 | 76 | 0.242 | 7677 | 5264 | 1472 | 7677 | 0.083% | 1.79 | 1.00 | 0.01% | 0.001309 | 0.36 | 0.025 | 0.785 | 0.21% | 0.0003 |
| 12.139 | 6961.46 | 69.6 | 0.473 | 69.6 | -0.9 | -0.41 | 0.68% | Coarse Tailings | 0.054 | 108.1 | 0.68 | 0.000 | 0.68 | 0 | 101 | 0.69% | 1.8 | 21% | 1.91 | 0.000 | 1.91 | 10.10 | 727 | 1.7E-03 | 8.9E+02 | 59 | 0.7171 | 3.0E-04 | 0 | 0.242 | 0.243 | 77 | 0.242 | 7662 | 5253 | 1473 | 7662 | 0.083% | 1.79 | 1.00 | 0.01% | 0.001304 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 12.303 | 6961.30 | 60.0 | 0.435 | 60.0 | -1.2 | -0.51 | 0.72% | Coarse Tailings | 0.054 | 108.1 | 0.69 | 0.000 | 0.69 | 0 | 86 | 0.73% | 1.9 | 21% | 1.92 | 0.000 | 1.92 | 10.15 | 727 | 1.7E-03 | 8.9E+02 | 58 | 0.7134 | 3.0E-04 | 0 | 0.242 | 0.243 | 78 | 0.242 | 7647 | 5243 | 1474 | 7647 | 0.083% | 1.79 | 1.00 | 0.01% | 0.001298 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 12.467 | 6961.13 | 54.6 | 0.370 | 54.6 | -0.8 | -0.36 | 0.68% | Coarse Tailings | 0.054 | 108.1 | 0.70 | 0.000 | 0.70 | 0 | 77 | 0.69% | 1.9 | 21% | 1.93 | 0.000 | 1.93 | 10.20 | 727 | 1.7E-03 | 8.9E+02 | 57 | 0.7097 | 3.0E-04 | 0 | 0.243 | 0.244 | 79 | 0.243 | 7633 | 5233 | 1475 | 7633 | 0.082% | 1.79 | 1.00 | 0.01% | 0.001292 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 12.631 | 6960.97 | 50.6 | 0.332 | 50.6 | -0.9 | -0.39 | 0.66% | Coarse Tailings | 0.054 | 108.1 | 0.71 | 0.000 | 0.71 | 0 | 70 | 0.67% | 1.9 | 21% | 1.93 | 0.000 | 1.93 | 10.25 | 727 | 1.7E-03 | 8.9E+02 | 57 | 0.7060 | 3.0E-04 | 0 | 0.243 | 0.244 | 80 | 0.243 | 7618 | 5223 | 1476 | 7618 | 0.082% | 1.79 | 1.00 | 0.01% | 0.001286 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 12.795 | 6960.80 | 48.2 | 0.317 | 48.3 | -0.9 | -0.39 | 0.66% | Coarse Tailings | 0.054 | 108.1 | 0.72 | 0.000 | 0.72 | 0 | 66 | 0.67% | 2.0 | 21% | 1.94 | 0.000 | 1.94 | 10.30 | 727 | 1.7E-03 | 8.9E+02 | 56 | 0.7022 | 3.0E-04 | 0 | 0.243 | 0.244 | 81 | 0.243 | 7604 | 5213 | 1477 | 7604 | 0.081% | 1.79 | 1.00 | 0.01% | 0.001280 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 12.959 | 6960.64 | 47.5 | 0.298 | 47.5 | -0.9 | -0.41 | 0.63% | Coarse Tailings | 0.054 | 108.1 | 0.73 | 0.000 | 0.73 | 0 | 64 | 0.64% | 2.0 | 21% | 1.95 | 0.000 | 1.95 | 10.35 | 727 | 1.7E-03 | 8.9E+02 | 55 | 0.6985 | 3.0E-04 | 0 | 0.243 | 0.244 | 82 | 0.243 | 7590 | 5203 | 1478 | 7590 | 0.081% | 1.79 | 1.00 | 0.01% | 0.001273 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 13.123 | 6960.48 | 46.3 | 0.361 | 46.3 | -1.0 | -0.43 | 0.78% | Coarse Tailings | 0.054 | 108.1 | 0.74 | 0.000 | 0.74 | 0 | 62 | 0.79% | 2.0 | 21% | 1.96 | 0.000 | 1.96 | 10.40 | 727 | 1.7E-03 | 8.9E+02 | 55 | 0.6947 | 3.0E-04 | 0 | 0.244 | 0.245 | 83 | 0.244 | 7576 | 5193 | 1479 | 7576 | 0.081% | 1.79 | 1.00 | 0.01% | 0.001266 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 13.287 | 6960.31 | 45.1 | 0.318 | 45.1 | -0.4 | -0.16 | 0.70% | Coarse Tailings | 0.054 | 108.1 | 0.75 | 0.000 | 0.75 | 0 | 60 | 0.72% | 2.0 | 21% | 1.97 | 0.000 | 1.97 | 10.45 | 727 | 1.7E-03 | 8.9E+02 | 54 | 0.6908 | 3.0E-04 | 0 | 0.244 | 0.245 | 84 | 0.244 | 7562 | 5184 | 1480 | 7562 | 0.080% | 1.79 | 1.00 | 0.01% | 0.001259 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 13.451 | 6960.15 | 44.2 | 0.310 | 44.2 | 0.2 | 0.08 | 0.70% | Coarse Tailings | 0.054 | 108.1 | 0.75 | 0.000 | 0.75 | 0 | 58 | 0.71% | 2.0 | 21% | 1.98 | 0.000 | 1.98 | 10.50 | 727 | 1.7E-03 | 8.9E+02 | 53 | 0.6870 | 3.0E-04 | 0 | 0.244 | 0.245 | 85 | 0.244 | 7548 | 5174 | 1481 | 7548 | 0.080% | 1.79 | 1.00 | 0.01% | 0.001251 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 13.615 | 6959.98 | 42.6 | 0.299 | 42.6 | -0.3 | -0.12 | 0.70% | Coarse Tailings | 0.054 | 108.1 | 0.76 | 0.000 | 0.76 | 0 | 55 | 0.71% | 2.0 | 21% | 1.99 | 0.000 | 1.99 | 10.55 | 727 | 1.7E-03 | 8.9E+02 | 53 | 0.6831 | 3.0E-04 | 0 | 0.244 | 0.246 | 86 | 0.244 | 7534 | 5164 | 1482 | 7534 | 0.079% | 1.79 | 1.00 | 0.01% | 0.001243 | 0.36 | 0.025 | 0.785 | 0.20% | 0.0003 |
| 13.779 | 6959.82 | 37.7 | 0.382 | 37.7 | -1.1 | -0.49 | 1.03% | Coarse Tailings | 0.054 | 108.1 | 0.77 | 0.000 | 0.77 | 0 | 48 | 1.03% | 2.2 | 21% | 2.00 | 0.000 | 2.00 | 10.60 | 727 | 1.7E-03 | 8.9E+02 | 52 | 0.6792 | 3.0E-04 | 0 | 0.245 | 0.246 | 87 | 0.245 | 7520 | 5155 | 1483 | 7520 | 0.079% | 1.79 | 1.00 | 0.01% | 0.001235 | 0.36 | 0.025 | 0.785 | 0.19% | 0.0003 |
| 13.943 | 6959.66 | 34.4 | 0.553 | 34.4 | 0.5 | 0.23 | 1.61% | Coarse Tailings | 0.054 | 108.1 | 0.78 | 0.000 | 0.78 | 0 | 43 | 1.64% | 2.3 | 21% | 2.01 | 0.000 | 2.01 | 10.65 | 727 | 1.7E-03 | 8.9E+02 | 52 | 0.6753 | 3.0E-04 | 0 | 0.245 | 0.246 | 88 | 0.245 | 7507 | 5145 | 1484 | 7507 | 0.079% | 1.79 | 1.00 | 0.01% | 0.001227 | 0.36 | 0.025 | 0.785 | 0.19% | 0.0003 |
| 14.107 | 6959.49 | 34.0 | 0.484 | 34.0 | 0.5 | 0.23 | 1.42% | Coarse Tailings | 0.054 | 108.1 | 0.79 | 0.000 | 0.79 | 0 | 42 | 1.46% | 2.3 | 21% | 2.01 | 0.000 | 2.01 | 10.70 | 727 | 1.7E-03 | 8.9E+02 | 51 | 0.6713 | 3.0E-04 | 0 | 0.245 | 0.246 | 89 | 0.245 | 7493 | 5136 | 1485 | 7493 | 0.078% | 1.79 | 1.00 | 0.01% | 0.001219 | 0.36 | 0.025 | 0.785 | 0.19% | 0.0003 |
| 14.271 | 6959.33 | 34.0 | 0.409 | 34.0 | -0.2 | -0.10 | 1.20% | Coarse Tailings | 0.054 | 108.1 | 0.80 | 0.000 | 0.80 | 0 | 42 | 1.23% | 2.3 | 21% | 2.02 | 0.000 | 2.02 | 10.75 | 727 | 1.7E-03 | 8.9E+02 | 50 | 0.6674 | 3.0E-04 | 0 | 0.245 | 0.247 | 90 | 0.245 | 7480 | 5126 | 1486 | 7480 | 0.078% | 1.79 | 1.00 | 0.01% | 0.001210 | 0.36 | 0.025 | 0.785 | 0.19% | 0.0003 |
| 14.436 | 6959.16 | 37.3 | 0.267 | 37.3 | 0.1 | 0.06 | 0.72% | Coarse Tailings | 0.054 | 108.1 | 0.81 | 0.000 | 0.81 | 0 | 45 | 0.73% | 2.1 | 21% | 2.03 | 0.000 | 2.03 | 10.80 | 727 | 1.7E-03 | 8.9E+02 | 50 | 0.6634 | 3.0E-04 | 0 | 0.246 | 0.247 | 91 | 0.246 | 7466 | 5117 | 1487 | 7466 | 0.077% | 1.79 | 1.00 | 0.01% | 0.001201 | 0.36 | 0.025 | 0.785 | 0.19% | 0.0003 |
| 14.600 | 6959.00 | 40.5 | 0.207 | 40.5 | 0.4 | 0.16 | 0.51% | Coarse Tailings | 0.054 | 108.1 | 0.82 | 0.000 | 0.82 | 0 | 49 | 0.52% | 2.0 | 21% | 2.04 | 0.000 | 2.04 | 10.85 | 589 | 1.7E-03 | 5.8E+02 | 49 | 0.6593 | 4.5E-04 | 0 | 0.246 | 0.247 | 92 | 0.246 | 7453 | 5107 | 1488 | 7453 | 0.291% | 1.79 | 1.00 | 0.01% | 0.005038 | 0.36 | 0.025 | 0.785 | 0.79% | 0.0013 |
| 14.764 | 6958.84 | 42.1 | 0.212 | 42.1 | 0.2 | 0.08 | 0.50% | Coarse Tailings | 0.054 | 108.1 | 0.83 | 0.000 | 0.83 | 0 | 50 | 0.51% | 2.0 | 21% | 2.05 | 0.000 | 2.05 | 10.90 | 589 | 1.7E-03 | 5.8E+02 | 49 | 0.6553 | 4.5E-04 | 0 | 0.246 | 0.248 | 93 | 0.246 | 7440 | 5098 | 1489 | 7440 | 0.288% | 1.79 | 1.00 | 0.01% | 0.004978 | 0.36 | 0.025 | 0.785 | 0.78% | 0.0013 |
| 14.928 | 6958.67 | 44.0 | 0.175 | 44.0 | -0.1 | -0.04 | 0.40% | Coarse Tailings | 0.054 | 108.1 | 0.83 | 0.000 | 0.83 | 0 | 52 | 0.41% | 1.9 | 21% | 2.06 | 0.000 | 2.06 | 10.95 | 589 | 1.7E-03 | 5.8E+02 | 48 | 0.6512 | 4.5E-04 | 0 | 0.246 | 0.248 | 94 | 0.246 | 7427 | 5089 | 1490 | 7427 | 0.285% | 1.79 | 1.00 | 0.01% | 0.004916 | 0.36 | 0.025 | 0.785 | 0.77% | 0.0013 |
| 15.092 | 6958.51 | 48.6 | 0.191 | 48.6 | 0.3 | 0.14 | 0.39% | Coarse Tailings | 0.054 | 108.1 | 0.84 | 0.000 | 0.84 | 0 | 57 | 0.40% | 1.9 | 21% | 2.07 | 0.000 | 2.07 | 11.00 | 589 | 1.7E-03 | 5.8E+02 | 48 | 0.6471 | 4.5E-04 | 0 | 0.247 | 0.248 | 95 | 0.247 | 7414 | 5080 | 1491 | 7414 | 0.281% | 1.79 | 1.00 | 0.01% | 0.004854 | 0.36 | 0.025 | 0.785 | 0.76% | 0.0013 |
| 15.256 | 6958.34 | 53.3 | 0.228 | 53.3 | 0.7 | 0.30 | 0.43% | Coarse Tailings | 0.054 | 108.1 | 0.85 | 0.000 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-------|-------|---------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 23.950 | 6949.65 | 24.2 | 0.338 | 24.2 | 11.4 | 4.94 | 1.39% | Fine Tailings | 0.054 | 107.6 | 1.32 | 0.158 | 1.16 | 1 | 20 | 1.48% | 2.6 | 83% | 2.55 | 0.158 | 2.39 | 13.70 | 469 | 1.7E-03 | 3.7E+02 | 29 | 0.4073 | 5.5E-04 | 43 | 0.255 | 0.258 | 149 | 149.000 | 6989 | 4783 | 1545 | 1545 | 0.128% | 0.90 | 0.75 | 0.06% | 0.003795 | 0.25 | 0.323 | 0.851 | 0.65% | 0.0011 |
| 24.114 | 6949.49 | 22.9 | 0.395 | 22.8 | 9.9 | 4.29 | 1.73% | Fine Tailings | 0.054 | 107.6 | 1.33 | 0.163 | 1.17 | 1 | 18 | 1.83% | 2.7 | 83% | 2.55 | 0.163 | 2.39 | 13.75 | 469 | 1.7E-03 | 3.7E+02 | 29 | 0.4028 | 5.5E-04 | 43 | 0.255 | 0.258 | 150 | 150.000 | 6984 | 4780 | 1546 | 1546 | 0.126% | 0.90 | 0.75 | 0.06% | 0.003723 | 0.25 | 0.323 | 0.851 | 0.63% | 0.0010 |
| 24.278 | 6949.32 | 20.9 | 0.416 | 20.8 | 8.9 | 3.84 | 1.99% | Fine Tailings | 0.054 | 107.6 | 1.34 | 0.168 | 1.17 | 1 | 17 | 2.13% | 2.7 | 83% | 2.56 | 0.168 | 2.40 | 13.80 | 469 | 1.7E-03 | 3.7E+02 | 28 | 0.3983 | 5.4E-04 | 43 | 0.255 | 0.258 | 151 | 151.000 | 6980 | 4777 | 1547 | 1547 | 0.125% | 0.90 | 0.75 | 0.06% | 0.003651 | 0.25 | 0.323 | 0.851 | 0.62% | 0.0010 |
| 24.442 | 6949.16 | 23.1 | 0.444 | 23.0 | 7.7 | 3.34 | 1.93% | Fine Tailings | 0.054 | 107.6 | 1.35 | 0.000 | 1.35 | 0 | 16 | 2.05% | 2.7 | 83% | 2.57 | 0.000 | 2.57 | 13.85 | 469 | 1.7E-03 | 3.7E+02 | 28 | 0.3939 | 5.4E-04 | 43 | 0.259 | 0.263 | 152 | 152.000 | 6779 | 4637 | 1548 | 1548 | 0.123% | 0.90 | 0.75 | 0.06% | 0.003579 | 0.25 | 0.323 | 0.851 | 0.61% | 0.0010 |
| 24.606 | 6948.99 | 25.3 | 0.519 | 25.2 | 7.0 | 3.03 | 2.05% | Coarse Tailings | 0.054 | 108.1 | 1.36 | 0.000 | 1.36 | 0 | 18 | 2.17% | 2.7 | 21% | 2.58 | 0.000 | 2.58 | 13.90 | 508 | 1.7E-03 | 4.3E+02 | 28 | 0.3894 | 4.5E-04 | 0 | 0.260 | 0.263 | 153 | 0.260 | 6770 | 4630 | 1549 | 6770 | 0.235% | 1.79 | 1.00 | 0.01% | 0.004028 | 0.36 | 0.025 | 0.785 | 0.63% | 0.0010 |
| 24.770 | 6948.83 | 16.2 | 0.500 | 16.2 | 5.7 | 2.48 | 3.08% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 11 | 3.36% | 3.0 | 21% | 2.59 | 0.000 | 2.59 | 13.95 | 508 | 1.7E-03 | 4.3E+02 | 28 | 0.3850 | 4.5E-04 | 0 | 0.260 | 0.263 | 154 | 0.260 | 6760 | 4624 | 1550 | 6760 | 0.228% | 1.79 | 1.00 | 0.01% | 0.003897 | 0.36 | 0.025 | 0.785 | 0.61% | 0.0010 |
| 24.934 | 6948.67 | 10.7 | 0.338 | 10.6 | 5.8 | 2.52 | 3.17% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 7 | 3.63% | 3.2 | 21% | 2.60 | 0.000 | 2.60 | 14.00 | 508 | 1.7E-03 | 4.3E+02 | 28 | 0.3806 | 4.5E-04 | 0 | 0.260 | 0.264 | 155 | 0.260 | 6751 | 4617 | 1551 | 6751 | 0.221% | 1.79 | 1.00 | 0.01% | 0.003769 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 25.098 | 6948.50 | 8.6 | 0.263 | 8.5 | 8.3 | 3.58 | 3.06% | Coarse Tailings | 0.054 | 108.1 | 1.38 | 0.000 | 1.38 | 0 | 5 | 3.65% | 3.3 | 21% | 2.61 | 0.000 | 2.61 | 14.05 | 508 | 1.7E-03 | 4.3E+02 | 27 | 0.3762 | 4.4E-04 | 0 | 0.260 | 0.264 | 156 | 0.260 | 6741 | 4611 | 1552 | 6741 | 0.214% | 1.79 | 1.00 | 0.01% | 0.003645 | 0.36 | 0.025 | 0.785 | 0.57% | 0.0009 |
| 25.262 | 6948.34 | 9.6 | 0.146 | 9.4 | 42.8 | 18.53 | 1.52% | Coarse Tailings | 0.054 | 108.1 | 1.39 | 0.000 | 1.39 | 0 | 6 | 1.77% | 3.1 | 21% | 2.62 | 0.000 | 2.62 | 14.10 | 508 | 1.7E-03 | 4.3E+02 | 27 | 0.3718 | 4.4E-04 | 0 | 0.260 | 0.264 | 157 | 0.260 | 6732 | 4604 | 1553 | 6732 | 0.207% | 1.79 | 1.00 | 0.01% | 0.003525 | 0.36 | 0.025 | 0.785 | 0.55% | 0.0009 |
| 25.426 | 6948.17 | 9.6 | 0.131 | 9.3 | 46.3 | 20.07 | 1.36% | Coarse Tailings | 0.054 | 108.1 | 1.40 | 0.000 | 1.40 | 0 | 6 | 1.60% | 3.1 | 21% | 2.62 | 0.000 | 2.62 | 14.15 | 508 | 1.7E-03 | 4.3E+02 | 27 | 0.3674 | 4.3E-04 | 0 | 0.261 | 0.264 | 158 | 0.261 | 6722 | 4598 | 1554 | 6722 | 0.200% | 1.79 | 1.00 | 0.01% | 0.003409 | 0.36 | 0.025 | 0.785 | 0.54% | 0.0009 |
| 25.590 | 6948.01 | 9.2 | 0.124 | 8.9 | 50.5 | 21.90 | 1.34% | Coarse Tailings | 0.054 | 108.1 | 1.41 | 0.000 | 1.41 | 0 | 6 | 1.58% | 3.1 | 21% | 2.63 | 0.000 | 2.63 | 14.20 | 508 | 1.7E-03 | 4.3E+02 | 27 | 0.3631 | 4.3E-04 | 0 | 0.261 | 0.265 | 159 | 0.261 | 6713 | 4591 | 1555 | 6713 | 0.194% | 1.79 | 1.00 | 0.01% | 0.003296 | 0.36 | 0.025 | 0.785 | 0.52% | 0.0008 |
| 25.754 | 6947.85 | 9.4 | 0.116 | 9.1 | 60.6 | 26.25 | 1.23% | Coarse Tailings | 0.054 | 108.1 | 1.42 | 0.002 | 1.42 | 1 | 6 | 1.45% | 3.0 | 21% | 2.64 | 0.002 | 2.64 | 14.25 | 508 | 1.7E-03 | 4.3E+02 | 27 | 0.3587 | 4.3E-04 | 0 | 0.261 | 0.265 | 160 | 0.261 | 6706 | 4586 | 1556 | 6706 | 0.188% | 1.79 | 1.00 | 0.01% | 0.003189 | 0.36 | 0.025 | 0.785 | 0.50% | 0.0008 |
| 25.918 | 6947.68 | 8.3 | 0.114 | 7.9 | 57.1 | 24.73 | 1.38% | Fine Tailings | 0.054 | 107.6 | 1.43 | 0.007 | 1.42 | 1 | 5 | 1.67% | 3.1 | 83% | 2.65 | 0.007 | 2.64 | 14.30 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3544 | 4.2E-04 | 43 | 0.261 | 0.265 | 161 | 161.000 | 6702 | 4583 | 1557 | 1557 | 0.082% | 0.90 | 0.75 | 0.06% | 0.001631 | 0.25 | 0.323 | 0.851 | 0.28% | 0.0005 |
| 26.082 | 6947.52 | 7.7 | 0.112 | 7.3 | 71.0 | 30.77 | 1.45% | Fine Tailings | 0.054 | 107.6 | 1.44 | 0.012 | 1.42 | 1 | 4 | 1.78% | 3.2 | 83% | 2.66 | 0.012 | 2.65 | 14.35 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3501 | 4.2E-04 | 43 | 0.261 | 0.265 | 162 | 162.000 | 6698 | 4580 | 1558 | 1558 | 0.081% | 0.90 | 0.75 | 0.06% | 0.001566 | 0.25 | 0.323 | 0.851 | 0.27% | 0.0004 |
| 26.246 | 6947.35 | 7.7 | 0.108 | 7.3 | 63.1 | 27.35 | 1.40% | Fine Tailings | 0.054 | 107.6 | 1.44 | 0.017 | 1.43 | 1 | 4 | 1.72% | 3.2 | 83% | 2.67 | 0.017 | 2.65 | 14.40 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3458 | 4.2E-04 | 43 | 0.261 | 0.265 | 163 | 163.000 | 6694 | 4578 | 1559 | 1559 | 0.080% | 0.90 | 0.75 | 0.06% | 0.001500 | 0.25 | 0.323 | 0.851 | 0.26% | 0.0004 |
| 26.410 | 6947.19 | 7.5 | 0.085 | 7.1 | 52.0 | 22.53 | 1.14% | Fine Tailings | 0.054 | 107.6 | 1.45 | 0.022 | 1.43 | 1 | 4 | 1.41% | 3.2 | 83% | 2.68 | 0.022 | 2.66 | 14.45 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3415 | 4.1E-04 | 43 | 0.261 | 0.265 | 164 | 164.000 | 6690 | 4575 | 1560 | 1560 | 0.079% | 0.90 | 0.75 | 0.06% | 0.001434 | 0.25 | 0.323 | 0.851 | 0.24% | 0.0004 |
| 26.574 | 6947.03 | 8.0 | 0.101 | 7.6 | 67.8 | 29.36 | 1.26% | Fine Tailings | 0.054 | 107.6 | 1.46 | 0.027 | 1.44 | 1 | 5 | 1.54% | 3.1 | 83% | 2.69 | 0.027 | 2.66 | 14.50 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3373 | 4.1E-04 | 43 | 0.261 | 0.265 | 165 | 165.000 | 6687 | 4572 | 1561 | 1561 | 0.077% | 0.90 | 0.75 | 0.06% | 0.001367 | 0.25 | 0.323 | 0.851 | 0.23% | 0.0004 |
| 26.739 | 6946.86 | 8.8 | 0.168 | 8.4 | 69.2 | 30.00 | 1.91% | Fine Tailings | 0.054 | 107.6 | 1.47 | 0.032 | 1.44 | 1 | 5 | 2.30% | 3.2 | 83% | 2.70 | 0.032 | 2.66 | 14.55 | 508 | 1.7E-03 | 4.3E+02 | 26 | 0.3331 | 4.1E-04 | 43 | 0.261 | 0.265 | 166 | 166.000 | 6683 | 4570 | 1562 | 1562 | 0.076% | 0.90 | 0.75 | 0.06% | 0.001299 | 0.25 | 0.323 | 0.851 | 0.22% | 0.0004 |
| 26.903 | 6946.70 | 9.5 | 0.168 | 9.1 | 67.4 | 29.20 | 1.77% | Fine Tailings | 0.054 | 107.6 | 1.48 | 0.038 | 1.44 | 1 | 6 | 2.10% | 3.1 | 83% | 2.70 | 0.038 | 2.67 | 14.60 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3288 | 4.0E-04 | 43 | 0.262 | 0.265 | 167 | 167.000 | 6679 | 4567 | 1563 | 1563 | 0.075% | 0.90 | 0.75 | 0.06% | 0.001231 | 0.25 | 0.323 | 0.851 | 0.21% | 0.0003 |
| 27.067 | 6946.53 | 8.1 | 0.176 | 7.7 | 72.9 | 31.60 | 2.17% | Fine Tailings | 0.054 | 107.6 | 1.49 | 0.043 | 1.45 | 1 | 5 | 2.66% | 3.3 | 83% | 2.71 | 0.043 | 2.67 | 14.65 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3246 | 4.0E-04 | 43 | 0.262 | 0.266 | 168 | 168.000 | 6675 | 4564 | 1564 | 1564 | 0.074% | 0.90 | 0.75 | 0.06% | 0.001161 | 0.25 | 0.323 | 0.851 | 0.20% | 0.0003 |
| 27.231 | 6946.37 | 9.1 | 0.146 | 8.6 | 87.3 | 37.83 | 1.60% | Fine Tailings | 0.054 | 107.6 | 1.50 | 0.048 | 1.45 | 1 | 5 | 1.91% | 3.1 | 83% | 2.72 | 0.048 | 2.67 | 14.70 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3205 | 3.9E-04 | 43 | 0.262 | 0.266 | 169 | 169.000 | 6671 | 4562 | 1565 | 1565 | 0.073% | 0.90 | 0.75 | 0.06% | 0.001091 | 0.25 | 0.323 | 0.851 | 0.19% | 0.0003 |
| 27.395 | 6946.21 | 8.9 | 0.194 | 8.3 | 97.2 | 42.14 | 2.18% | Fine Tailings | 0.054 | 107.6 | 1.51 | 0.053 | 1.45 | 1 | 5 | 2.63% | 3.2 | 83% | 2.73 | 0.053 | 2.68 | 14.75 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3163 | 3.9E-04 | 43 | 0.262 | 0.266 | 170 | 170.000 | 6667 | 4559 | 1566 | 1566 | 0.072% | 0.90 | 0.75 | 0.06% | 0.001020 | 0.25 | 0.323 | 0.851 | 0.17% | 0.0003 |
| 27.559 | 6946.04 | 10.1 | 0.182 | 9.4 | 105.1 | 45.53 | 1.80% | Fine Tailings | 0.054 | 107.6 | 1.52 | 0.058 | 1.46 | 1 | 6 | 2.12% | 3.1 | 83% | 2.74 | 0.058 | 2.68 | 14.80 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3122 | 3.9E-04 | 43 | 0.262 | 0.266 | 171 | 171.000 | 6664 | 4557 | 1567 | 1567 | 0.071% | 0.90 | 0.75 | 0.06% | 0.000947 | 0.25 | 0.323 | 0.851 | 0.16% | 0.0003 |
| 27.723 | 6945.88 | 9.1 | 0.160 | 8.4 | 111.3 | 48.23 | 1.76% | Fine Tailings | 0.054 | 107.6 | 1.52 | 0.063 | 1.46 | 1 | 5 | 2.11% | 3.2 | 83% | 2.75 | 0.063 | 2.69 | 14.85 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3081 | 3.8E-04 | 43 | 0.262 | 0.266 | 172 | 172.000 | 6660 | 4554 | 1568 | 1568 | 0.070% | 0.90 | 0.75 | 0.06% | 0.000873 | 0.25 | 0.323 | 0.851 | 0.15% | 0.0002 |
| 27.887 | 6945.71 | 9.0 | 0.156 | 8.3 | 118.2 | 51.21 | 1.73% | Fine Tailings | 0.054 | 107.6 | 1.53 | 0.068 | 1.46 | 1 | 5 | 2.08% | 3.2 | 83% | 2.76 | 0.068 | 2.69 | 14.90 | 508 | 1.7E-03 | 4.3E+02 | 25 | 0.3040 | 3.8E-04 | 43 | 0.262 | 0.266 | 173 | 173.000 | 6656 | 4551 | 1569 | 1569 | 0.069% | 0.90 | 0.75 | 0.06% | 0.000797 | 0.25 | 0.323 | 0.851 | 0.14% | 0.0002 |
| 28.051 | 6945.55 | 9.0 | 0.173 | 8.3 | 114.9 | 49.78 | 1.92% | Fine Tailings | 0.054 | 107.6 | 1.54 | 0.073 | 1.47 | 1 | 5 | 2.32% | 3.2 | 83% | 2.77 | 0.073 | 2.69 | 14.95 | 508 | 1.7E-03 | 4.3E+02 | 24 | 0.2999 | 3.8E-04 | 43 | 0.262 | 0.266 | 174 | 174.000 | 6652 | 4549 | 1570 | 1570 | 0.067% | 0.90 | 0.75 | 0.06% | 0.000720 | 0.25 | 0.323 | 0.851 | 0.12% | 0.0002 |
| 28.215 | 6945.39 | 8.7 | 0.177 | 8.0 | 104.5 | 45.29 | 2.04% | Fine Tailings | 0.054 | 107.6 | 1.55 | 0.078 | 1.47 | 1 | 5 | 2.48% | 3.2 | 83% | 2.78 | 0.078 | 2.70 | 15.00 | 508 | 1.7E-03 | 4.3E+02 | 24 | 0.2959 | 3.7E-04 | 43 | 0.262 | 0.266 | 175 | 175.000 | 6649 | 4546 | 1571 | 1571 | 0.066% | 0.90 | 0.75 | 0.06% | 0.000640 | 0.25 | 0.323 | 0.851 | 0.11% | 0.0002 |
| 28.379 | 6945.22 | 8.4 | 0.157 | 7.7 | 114.3 | 49.54 | 1.86%</ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-------|-------|-------|---------------|-------|-------|------|-------|------|---|---|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 37.073 | 6936.53 | 9.1 | 0.257 | 8.6 | 87.9 | 38.11 | 2.81% | Fine Tailings | 0.054 | 107.6 | 2.03 | 0.121 | 1.91 | 1 | 4 | 3.61% | 3.4 | 83% | 3.25 | 0.121 | 3.13 | 17.70 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1183 | 1.8E-04 | 43 | 0.271 | 0.277 | 229 | 229.000 | 6253 | 4271 | 1625 | 1625 | 0.024% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.237 | 6936.36 | 9.5 | 0.185 | 8.9 | 90.0 | 39.00 | 1.95% | Fine Tailings | 0.054 | 107.6 | 2.04 | 0.126 | 1.91 | 1 | 4 | 2.48% | 3.3 | 83% | 3.26 | 0.126 | 3.14 | 17.75 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1158 | 1.7E-04 | 43 | 0.272 | 0.277 | 230 | 230.000 | 6250 | 4269 | 1626 | 1626 | 0.023% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.401 | 6936.20 | 9.2 | 0.138 | 8.6 | 91.6 | 39.68 | 1.50% | Fine Tailings | 0.054 | 107.6 | 2.05 | 0.131 | 1.91 | 1 | 4 | 1.94% | 3.3 | 83% | 3.27 | 0.131 | 3.14 | 17.80 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1134 | 1.7E-04 | 43 | 0.272 | 0.277 | 231 | 231.000 | 6247 | 4266 | 1627 | 1627 | 0.023% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.565 | 6936.03 | 9.9 | 0.149 | 9.4 | 92.3 | 39.98 | 1.50% | Fine Tailings | 0.054 | 107.6 | 2.05 | 0.136 | 1.92 | 1 | 4 | 1.89% | 3.2 | 83% | 3.28 | 0.136 | 3.14 | 17.85 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1110 | 1.7E-04 | 43 | 0.272 | 0.277 | 232 | 232.000 | 6244 | 4264 | 1628 | 1628 | 0.022% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.729 | 6935.87 | 10.3 | 0.186 | 9.7 | 98.0 | 42.48 | 1.81% | Fine Tailings | 0.054 | 107.6 | 2.06 | 0.141 | 1.92 | 1 | 4 | 2.26% | 3.2 | 83% | 3.29 | 0.141 | 3.15 | 17.90 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1086 | 1.6E-04 | 43 | 0.272 | 0.277 | 233 | 233.000 | 6241 | 4262 | 1629 | 1629 | 0.021% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.893 | 6935.71 | 10.4 | 0.235 | 9.8 | 99.4 | 43.09 | 2.25% | Fine Tailings | 0.054 | 107.6 | 2.07 | 0.146 | 1.93 | 1 | 4 | 2.81% | 3.3 | 83% | 3.30 | 0.146 | 3.15 | 17.95 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1062 | 1.6E-04 | 43 | 0.272 | 0.277 | 234 | 234.000 | 6238 | 4260 | 1630 | 1630 | 0.021% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.057 | 6935.54 | 9.9 | 0.261 | 9.4 | 90.1 | 39.04 | 2.63% | Fine Tailings | 0.054 | 107.6 | 2.08 | 0.152 | 1.93 | 1 | 4 | 3.32% | 3.3 | 83% | 3.31 | 0.152 | 3.15 | 18.00 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1038 | 1.6E-04 | 43 | 0.272 | 0.278 | 235 | 235.000 | 6235 | 4258 | 1631 | 1631 | 0.020% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.221 | 6935.38 | 10.1 | 0.207 | 9.5 | 92.4 | 40.02 | 2.05% | Fine Tailings | 0.054 | 107.6 | 2.09 | 0.157 | 1.93 | 1 | 4 | 2.59% | 3.3 | 83% | 3.31 | 0.157 | 3.16 | 18.05 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.1015 | 1.6E-04 | 43 | 0.272 | 0.278 | 236 | 236.000 | 6232 | 4256 | 1632 | 1632 | 0.020% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.385 | 6935.21 | 9.3 | 0.187 | 8.7 | 83.9 | 36.34 | 2.02% | Fine Tailings | 0.054 | 107.6 | 2.10 | 0.162 | 1.94 | 1 | 4 | 2.61% | 3.3 | 83% | 3.32 | 0.162 | 3.16 | 18.10 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0992 | 1.5E-04 | 43 | 0.272 | 0.278 | 237 | 237.000 | 6229 | 4254 | 1633 | 1633 | 0.019% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.549 | 6935.05 | 10.0 | 0.192 | 9.4 | 96.0 | 41.59 | 1.93% | Fine Tailings | 0.054 | 107.6 | 2.11 | 0.167 | 1.94 | 1 | 4 | 2.44% | 3.3 | 83% | 3.33 | 0.167 | 3.16 | 18.15 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0970 | 1.5E-04 | 43 | 0.272 | 0.278 | 238 | 238.000 | 6226 | 4252 | 1634 | 1634 | 0.019% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.713 | 6934.89 | 10.1 | 0.189 | 9.5 | 99.9 | 43.29 | 1.87% | Fine Tailings | 0.054 | 107.6 | 2.12 | 0.172 | 1.94 | 1 | 4 | 2.36% | 3.3 | 83% | 3.34 | 0.172 | 3.17 | 18.20 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0947 | 1.5E-04 | 43 | 0.272 | 0.278 | 239 | 239.000 | 6223 | 4250 | 1635 | 1635 | 0.019% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.877 | 6934.72 | 10.1 | 0.174 | 9.5 | 100.2 | 43.44 | 1.72% | Fine Tailings | 0.054 | 107.6 | 2.13 | 0.177 | 1.95 | 1 | 4 | 2.17% | 3.3 | 83% | 3.35 | 0.177 | 3.17 | 18.25 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0925 | 1.4E-04 | 43 | 0.272 | 0.278 | 240 | 240.000 | 6220 | 4248 | 1636 | 1636 | 0.018% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.042 | 6934.56 | 10.2 | 0.185 | 9.5 | 104.2 | 45.14 | 1.82% | Fine Tailings | 0.054 | 107.6 | 2.13 | 0.182 | 1.95 | 1 | 4 | 2.31% | 3.3 | 83% | 3.36 | 0.182 | 3.18 | 18.30 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0903 | 1.4E-04 | 43 | 0.272 | 0.278 | 241 | 241.000 | 6217 | 4246 | 1637 | 1637 | 0.018% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.206 | 6934.39 | 10.0 | 0.172 | 9.3 | 103.9 | 45.02 | 1.73% | Fine Tailings | 0.054 | 107.6 | 2.14 | 0.187 | 1.96 | 1 | 4 | 2.20% | 3.3 | 83% | 3.37 | 0.187 | 3.18 | 18.35 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0882 | 1.4E-04 | 43 | 0.272 | 0.278 | 242 | 242.000 | 6214 | 4243 | 1638 | 1638 | 0.017% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.370 | 6934.23 | 9.6 | 0.179 | 8.9 | 103.1 | 44.66 | 1.87% | Fine Tailings | 0.054 | 107.6 | 2.15 | 0.192 | 1.96 | 1 | 4 | 2.42% | 3.3 | 83% | 3.38 | 0.192 | 3.18 | 18.40 | 503 | 1.7E-03 | 4.2E+02 | 19 | 0.0861 | 1.3E-04 | 43 | 0.272 | 0.278 | 243 | 243.000 | 6211 | 4241 | 1639 | 1639 | 0.017% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.534 | 6934.07 | 10.1 | 0.186 | 9.4 | 105.8 | 45.86 | 1.84% | Fine Tailings | 0.054 | 107.6 | 2.16 | 0.198 | 1.96 | 1 | 4 | 2.35% | 3.3 | 83% | 3.38 | 0.198 | 3.19 | 18.45 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0840 | 7.3E-05 | 43 | 0.273 | 0.278 | 244 | 244.000 | 6208 | 4238 | 1640 | 1640 | 0.008% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.698 | 6933.90 | 11.1 | 0.218 | 10.5 | 111.9 | 48.48 | 1.96% | Fine Tailings | 0.054 | 107.6 | 2.17 | 0.203 | 1.97 | 1 | 5 | 2.43% | 3.2 | 83% | 3.39 | 0.203 | 3.19 | 18.50 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0819 | 7.1E-05 | 43 | 0.273 | 0.278 | 245 | 245.000 | 6205 | 4237 | 1641 | 1641 | 0.008% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 39.862 | 6933.74 | 10.9 | 0.250 | 10.2 | 112.4 | 48.69 | 2.29% | Fine Tailings | 0.054 | 107.6 | 2.18 | 0.208 | 1.97 | 1 | 4 | 2.87% | 3.3 | 83% | 3.40 | 0.208 | 3.19 | 18.55 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0798 | 7.0E-05 | 43 | 0.273 | 0.278 | 246 | 246.000 | 6202 | 4235 | 1642 | 1642 | 0.008% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.026 | 6933.57 | 11.2 | 0.266 | 10.5 | 111.8 | 48.46 | 2.37% | Fine Tailings | 0.054 | 107.6 | 2.19 | 0.213 | 1.97 | 1 | 5 | 2.95% | 3.3 | 83% | 3.41 | 0.213 | 3.20 | 18.60 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0778 | 6.8E-05 | 43 | 0.273 | 0.279 | 247 | 247.000 | 6199 | 4233 | 1643 | 1643 | 0.008% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.190 | 6933.41 | 11.4 | 0.259 | 10.6 | 114.6 | 49.64 | 2.28% | Fine Tailings | 0.054 | 107.6 | 2.20 | 0.218 | 1.98 | 1 | 5 | 2.83% | 3.3 | 83% | 3.42 | 0.218 | 3.20 | 18.65 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0758 | 6.7E-05 | 43 | 0.273 | 0.279 | 248 | 248.000 | 6197 | 4231 | 1644 | 1644 | 0.007% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.354 | 6933.25 | 11.1 | 0.254 | 10.4 | 115.1 | 49.89 | 2.29% | Fine Tailings | 0.054 | 107.6 | 2.20 | 0.223 | 1.98 | 1 | 4 | 2.85% | 3.3 | 83% | 3.43 | 0.223 | 3.21 | 18.70 | 674 | 1.7E-03 | 7.6E+02 | 19 | 0.0739 | 6.5E-05 | 43 | 0.273 | 0.279 | 249 | 249.000 | 6194 | 4229 | 1645 | 1645 | 0.007% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.518 | 6933.08 | 11.2 | 0.254 | 10.5 | 117.1 | 50.74 | 2.27% | Fine Tailings | 0.054 | 107.6 | 2.21 | 0.228 | 1.99 | 1 | 5 | 2.83% | 3.3 | 83% | 3.44 | 0.228 | 3.21 | 18.75 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0719 | 6.3E-05 | 43 | 0.273 | 0.279 | 250 | 250.000 | 6191 | 4227 | 1646 | 1646 | 0.007% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.682 | 6932.92 | 11.0 | 0.256 | 10.3 | 117.2 | 50.80 | 2.33% | Fine Tailings | 0.054 | 107.6 | 2.22 | 0.233 | 1.99 | 1 | 4 | 2.92% | 3.3 | 83% | 3.45 | 0.233 | 3.21 | 18.80 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0700 | 6.2E-05 | 43 | 0.273 | 0.279 | 251 | 251.000 | 6188 | 4225 | 1647 | 1647 | 0.007% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 40.846 | 6932.75 | 10.5 | 0.259 | 9.8 | 118.3 | 51.25 | 2.47% | Fine Tailings | 0.054 | 107.6 | 2.23 | 0.239 | 1.99 | 1 | 4 | 3.14% | 3.3 | 83% | 3.46 | 0.239 | 3.22 | 18.85 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0681 | 6.0E-05 | 43 | 0.273 | 0.279 | 252 | 252.000 | 6185 | 4223 | 1648 | 1648 | 0.007% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.010 | 6932.59 | 11.1 | 0.263 | 10.4 | 119.3 | 51.69 | 2.36% | Fine Tailings | 0.054 | 107.6 | 2.24 | 0.244 | 2.00 | 1 | 4 | 2.96% | 3.3 | 83% | 3.46 | 0.244 | 3.22 | 18.90 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0662 | 5.9E-05 | 43 | 0.273 | 0.279 | 253 | 253.000 | 6182 | 4221 | 1649 | 1649 | 0.006% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.174 | 6932.43 | 11.1 | 0.273 | 10.3 | 119.3 | 51.69 | 2.47% | Fine Tailings | 0.054 | 107.6 | 2.25 | 0.249 | 2.00 | 1 | 4 | 3.09% | 3.3 | 83% | 3.47 | 0.249 | 3.22 | 18.95 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0644 | 5.7E-05 | 43 | 0.273 | 0.279 | 254 | 254.000 | 6179 | 4219 | 1650 | 1650 | 0.006% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.338 | 6932.26 | 11.1 | 0.294 | 10.3 | 120.1 | 52.04 | 2.65% | Fine Tailings | 0.054 | 107.6 | 2.26 | 0.254 | 2.00 | 1 | 4 | 3.33% | 3.3 | 83% | 3.48 | 0.254 | 3.23 | 19.00 | 674 | 1.7E-03 | 7.6E+02 | 18 | 0.0626 | 5.6E-05 | 43 | 0.273 | 0.279 | 255 | 255.000 | 6176 | 4217 | 1651 | 1651 | 0.006% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 41.502 | 6932.10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|-------|-------|-------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|------|---------|---------|----|--------|---------|---|-------|-------|-----|-------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 50.196 | 6923.40 | 76.5 | 0.611 | 76.4 | 5.6 | 2.44 | 0.80% | Coarse Alluvium | 0.056 | 111.0 | 2.74 | 0.000 | 2.74 | 0 | 27 | 0.83% | 2.3 | 36% | 3.97 | 0.000 | 3.97 | 21.70 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0230 | 4.5E-06 | 0 | 0.287 | 0.295 | 309 | 0.287 | 5675 | 3868 | 1705 | 5675 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 50.360 | 6923.24 | 73.7 | 0.656 | 73.7 | 4.8 | 2.09 | 0.89% | Coarse Alluvium | 0.056 | 111.0 | 2.75 | 0.000 | 2.75 | 0 | 26 | 0.92% | 2.4 | 36% | 3.98 | 0.000 | 3.98 | 21.75 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0228 | 4.5E-06 | 0 | 0.287 | 0.295 | 310 | 0.287 | 5669 | 3864 | 1706 | 5669 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 50.524 | 6923.08 | 76.5 | 0.669 | 76.5 | 4.9 | 2.11 | 0.87% | Coarse Alluvium | 0.056 | 111.0 | 2.76 | 0.000 | 2.76 | 0 | 27 | 0.91% | 2.4 | 36% | 3.99 | 0.000 | 3.99 | 21.80 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0226 | 4.5E-06 | 0 | 0.287 | 0.295 | 311 | 0.287 | 5664 | 3861 | 1707 | 5664 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 50.688 | 6922.91 | 79.2 | 0.663 | 79.2 | 5.6 | 2.42 | 0.84% | Coarse Alluvium | 0.056 | 111.0 | 2.77 | 0.000 | 2.77 | 0 | 28 | 0.87% | 2.3 | 36% | 4.00 | 0.000 | 4.00 | 21.85 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0223 | 4.4E-06 | 0 | 0.287 | 0.295 | 312 | 0.287 | 5659 | 3857 | 1708 | 5659 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 50.852 | 6922.75 | 81.9 | 0.668 | 81.9 | 4.7 | 2.05 | 0.82% | Coarse Alluvium | 0.056 | 111.0 | 2.78 | 0.000 | 2.78 | 0 | 28 | 0.84% | 2.3 | 36% | 4.00 | 0.000 | 4.00 | 21.90 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0221 | 4.4E-06 | 0 | 0.287 | 0.296 | 313 | 0.287 | 5653 | 3853 | 1709 | 5653 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.016 | 6922.58 | 81.3 | 0.656 | 81.3 | 3.8 | 1.63 | 0.81% | Coarse Alluvium | 0.056 | 111.0 | 2.79 | 0.000 | 2.79 | 0 | 28 | 0.84% | 2.3 | 36% | 4.01 | 0.000 | 4.01 | 21.95 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0219 | 4.4E-06 | 0 | 0.287 | 0.296 | 314 | 0.287 | 5648 | 3850 | 1710 | 5648 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.180 | 6922.42 | 81.3 | 0.639 | 81.3 | 0.3 | 0.14 | 0.79% | Coarse Alluvium | 0.056 | 111.0 | 2.80 | 0.000 | 2.80 | 0 | 28 | 0.81% | 2.3 | 36% | 4.02 | 0.000 | 4.02 | 22.00 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0216 | 4.3E-06 | 0 | 0.288 | 0.296 | 315 | 0.288 | 5643 | 3846 | 1711 | 5643 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.345 | 6922.26 | 80.6 | 0.649 | 80.6 | 3.3 | 1.42 | 0.81% | Coarse Alluvium | 0.056 | 111.0 | 2.81 | 0.000 | 2.81 | 0 | 28 | 0.83% | 2.3 | 36% | 4.03 | 0.000 | 4.03 | 22.05 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0214 | 4.3E-06 | 0 | 0.288 | 0.296 | 316 | 0.288 | 5638 | 3842 | 1712 | 5638 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.509 | 6922.09 | 81.0 | 0.644 | 81.0 | 3.5 | 1.50 | 0.79% | Coarse Alluvium | 0.056 | 111.0 | 2.82 | 0.000 | 2.82 | 0 | 28 | 0.82% | 2.3 | 36% | 4.04 | 0.000 | 4.04 | 22.10 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0212 | 4.3E-06 | 0 | 0.288 | 0.296 | 317 | 0.288 | 5632 | 3839 | 1713 | 5632 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.673 | 6921.93 | 80.1 | 0.626 | 80.1 | 3.6 | 1.55 | 0.78% | Coarse Alluvium | 0.056 | 111.0 | 2.83 | 0.000 | 2.83 | 0 | 27 | 0.81% | 2.3 | 36% | 4.05 | 0.000 | 4.05 | 22.15 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0209 | 4.2E-06 | 0 | 0.288 | 0.297 | 318 | 0.288 | 5627 | 3835 | 1714 | 5627 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 51.837 | 6921.76 | 81.9 | 0.764 | 81.9 | 3.7 | 1.61 | 0.93% | Coarse Alluvium | 0.056 | 111.0 | 2.84 | 0.000 | 2.84 | 0 | 28 | 0.97% | 2.4 | 36% | 4.06 | 0.000 | 4.06 | 22.20 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0207 | 4.2E-06 | 0 | 0.288 | 0.297 | 319 | 0.288 | 5622 | 3832 | 1715 | 5622 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.001 | 6921.60 | 77.2 | 0.870 | 77.2 | 3.5 | 1.53 | 1.13% | Coarse Alluvium | 0.056 | 111.0 | 2.84 | 0.000 | 2.84 | 0 | 26 | 1.17% | 2.4 | 36% | 4.07 | 0.000 | 4.07 | 22.25 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0205 | 4.2E-06 | 0 | 0.288 | 0.297 | 320 | 0.288 | 5617 | 3828 | 1716 | 5617 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.165 | 6921.44 | 74.2 | 0.943 | 74.1 | 4.1 | 1.77 | 1.27% | Coarse Alluvium | 0.056 | 111.0 | 2.85 | 0.000 | 2.85 | 0 | 25 | 1.32% | 2.5 | 36% | 4.08 | 0.000 | 4.08 | 22.30 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0203 | 4.1E-06 | 0 | 0.289 | 0.297 | 321 | 0.289 | 5612 | 3824 | 1717 | 5612 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.329 | 6921.27 | 77.0 | 0.912 | 76.9 | 3.9 | 1.67 | 1.19% | Coarse Alluvium | 0.056 | 111.0 | 2.86 | 0.000 | 2.86 | 0 | 26 | 1.23% | 2.4 | 36% | 4.09 | 0.000 | 4.09 | 22.35 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0200 | 4.1E-06 | 0 | 0.289 | 0.297 | 322 | 0.289 | 5607 | 3821 | 1718 | 5607 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.493 | 6921.11 | 71.8 | 0.996 | 71.8 | 3.0 | 1.30 | 1.39% | Coarse Alluvium | 0.056 | 111.0 | 2.87 | 0.000 | 2.87 | 0 | 24 | 1.44% | 2.5 | 36% | 4.10 | 0.000 | 4.10 | 22.40 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0198 | 4.0E-06 | 0 | 0.289 | 0.297 | 323 | 0.289 | 5602 | 3817 | 1719 | 5602 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.657 | 6920.94 | 56.5 | 1.402 | 56.5 | 2.6 | 1.14 | 2.48% | Coarse Alluvium | 0.056 | 111.0 | 2.88 | 0.000 | 2.88 | 0 | 19 | 2.61% | 2.7 | 36% | 4.11 | 0.000 | 4.10 | 22.45 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0196 | 4.0E-06 | 0 | 0.289 | 0.298 | 324 | 0.289 | 5596 | 3814 | 1720 | 5596 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.821 | 6920.78 | 42.0 | 1.549 | 42.0 | 3.1 | 1.34 | 3.69% | Coarse Alluvium | 0.056 | 111.0 | 2.89 | 0.000 | 2.89 | 0 | 14 | 3.96% | 3.0 | 36% | 4.11 | 0.000 | 4.11 | 22.50 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0193 | 4.0E-06 | 0 | 0.289 | 0.298 | 325 | 0.289 | 5591 | 3810 | 1721 | 5591 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 52.985 | 6920.62 | 39.3 | 1.641 | 39.3 | 5.6 | 2.42 | 4.18% | Coarse Alluvium | 0.056 | 111.0 | 2.90 | 0.000 | 2.90 | 0 | 13 | 4.51% | 3.0 | 36% | 4.12 | 0.000 | 4.12 | 22.55 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0191 | 3.9E-06 | 0 | 0.289 | 0.298 | 326 | 0.289 | 5586 | 3807 | 1722 | 5586 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.149 | 6920.45 | 38.8 | 1.586 | 38.8 | 6.5 | 2.81 | 4.09% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 12 | 4.42% | 3.0 | 36% | 4.13 | 0.000 | 4.13 | 22.60 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0189 | 3.9E-06 | 0 | 0.289 | 0.298 | 327 | 0.289 | 5581 | 3803 | 1723 | 5581 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.313 | 6920.29 | 40.7 | 1.370 | 40.6 | 8.2 | 3.54 | 3.37% | Coarse Alluvium | 0.056 | 111.0 | 2.92 | 0.000 | 2.92 | 0 | 13 | 3.63% | 3.0 | 36% | 4.14 | 0.000 | 4.14 | 22.65 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0186 | 3.8E-06 | 0 | 0.290 | 0.298 | 328 | 0.290 | 5576 | 3800 | 1724 | 5576 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.477 | 6920.12 | 41.8 | 1.035 | 41.8 | 6.2 | 2.70 | 2.47% | Coarse Alluvium | 0.056 | 111.0 | 2.93 | 0.000 | 2.93 | 0 | 13 | 2.66% | 2.9 | 36% | 4.15 | 0.000 | 4.15 | 22.70 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0184 | 3.8E-06 | 0 | 0.290 | 0.298 | 329 | 0.290 | 5571 | 3796 | 1725 | 5571 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.641 | 6919.96 | 52.0 | 0.917 | 52.0 | 4.9 | 2.11 | 1.76% | Coarse Alluvium | 0.056 | 111.0 | 2.94 | 0.000 | 2.94 | 0 | 17 | 1.87% | 2.7 | 36% | 4.16 | 0.000 | 4.16 | 22.75 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0182 | 3.8E-06 | 0 | 0.290 | 0.299 | 330 | 0.290 | 5566 | 3793 | 1726 | 5566 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.805 | 6919.79 | 53.2 | 0.932 | 53.2 | 4.5 | 1.95 | 1.75% | Coarse Alluvium | 0.056 | 111.0 | 2.94 | 0.000 | 2.94 | 0 | 17 | 1.85% | 2.7 | 36% | 4.17 | 0.000 | 4.17 | 22.80 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0180 | 3.7E-06 | 0 | 0.290 | 0.299 | 331 | 0.290 | 5561 | 3789 | 1727 | 5561 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 53.969 | 6919.63 | 60.2 | 0.957 | 60.2 | 4.5 | 1.95 | 1.59% | Coarse Alluvium | 0.056 | 111.0 | 2.95 | 0.000 | 2.95 | 0 | 19 | 1.67% | 2.6 | 36% | 4.18 | 0.000 | 4.18 | 22.85 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0177 | 3.7E-06 | 0 | 0.290 | 0.299 | 332 | 0.290 | 5556 | 3786 | 1728 | 5556 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 54.133 | 6919.47 | 78.9 | 1.096 | 78.8 | 5.6 | 2.42 | 1.39% | Coarse Alluvium | 0.056 | 111.0 | 2.96 | 0.000 | 2.96 | 0 | 26 | 1.44% | 2.5 | 36% | 4.19 | 0.000 | 4.19 | 22.90 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0175 | 3.6E-06 | 0 | 0.290 | 0.299 | 333 | 0.290 | 5551 | 3782 | 1729 | 5551 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 54.297 | 6919.30 | 84.2 | 1.495 | 84.2 | 5.4 | 2.34 | 1.77% | Coarse Alluvium | 0.056 | 111.0 | 2.97 | 0.000 | 2.97 | 0 | 27 | 1.84% | 2.5 | 36% | 4.20 | 0.000 | 4.20 | 22.95 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0173 | 3.6E-06 | 0 | 0.290 | 0.299 | 334 | 0.290 | 5546 | 3779 | 1730 | 5546 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 54.461 | 6919.14 | 100.2 | 1.165 | 100.2 | 5.4 | 2.36 | 1.16% | Coarse Alluvium | 0.056 | 111.0 | 2.98 | 0.000 | 2.98 | 0 | 33 | 1.20% | 2.3 | 36% | 4.21 | 0.000 | 4.21 | 23.00 | 1507 | 1.7E-03 | 3.9E+03 | 17 | 0.0170 | 3.6E-06 | 0 | 0.291 | 0.300 | 335 | 0.291 | 5541 | 3776 | 1731 | 5541 | 0.000% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 54.625 | 6918.97 | 105.0 | 1.871 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Proposed Repository | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer | Total Stress at Midpoint of Layer | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint | Effective Stress at Bottom of Layer (tsf) | Effective Stress at Midpoint of Layer |
|---------------------|----------------------------|---------------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------|-----------------------------------|--|---------------------------------|---|---------------------------------------|
| Erosion Protection | 6994.6 | 6993.9 | 6993.1 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6993.1 | 6991.6 | 6990.1 | 3.0 | 0.057 | 114.7 | 0.264 | 0.178 | 0.00 | 0.00 | 0.264 | 0.178 |
| Mine Spoils | 6990.1 | 6981.9 | 6973.6 | 16.5 | 0.058 | 116.4 | 1.224 | 0.744 | 0.00 | 0.00 | 1.224 | 0.744 |

| | |
|---------|--|
| 6973.60 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6994.60 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|--|
| 1.22 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{\text{repos}}$ (psf) |
| 6929.06 | Elevation of bottom of tailings (ft amsl) |

| UNC-NECR WASTE REPOSITORY SEISMIC SETTLEMENT ANALYSIS - CPT-11 | | | | | | | | | | | | | | | | | | | | | | | | |
|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|
| Data File: 13-52118_RP11-BSC-CPT | | | | | | | | | | | | | | | | | | | | | | | | |
| Location: UNC-NECR 2013 Mill Site PDS | | | | | | | | | | | | | | | | | | | | | | | | |
| http://projects.mwhglobal.com/.../13-52118_RP11-BSC-CPT.XLS | | | | | | | | | | | | | | | | | | | | | | | | |
| Cells Requiring User Input/Manipulation | | | | | | | | | | | | | | | | | | | | | | | | |
| Erosion Protection | | | | | | | | | | | | | | | | | | | | | | | | |
| Cover Soil | | | | | | | | | | | | | | | | | | | | | | | | |
| Mine Spoils | | | | | | | | | | | | | | | | | | | | | | | | |
| Radon Barrier | | | | | | | | | | | | | | | | | | | | | | | | |
| General Fill | | | | | | | | | | | | | | | | | | | | | | | | |
| Coarse Tailings | | | | | | | | | | | | | | | | | | | | | | | | |
| Coarse/Fine Tailings | | | | | | | | | | | | | | | | | | | | | | | | |
| Fine Tailings | | | | | | | | | | | | | | | | | | | | | | | | |
| Coarse Alluvium | | | | | | | | | | | | | | | | | | | | | | | | |
| Fine Alluvium | | | | | | | | | | | | | | | | | | | | | | | | |
| Idriss and Boulanger (2008) | | | | | | | | | | | | | | | | | | | | | | | | |
| Max. Horiz. Acceleration, A _{max} /g: | | | | | | | | | | | | | | | | | | | | | | | | |
| Earthquake Moment Magnitude, M: | | | | | | | | | | | | | | | | | | | | | | | | |
| Magnitude Scaling Factor, MSF: | | | | | | | | | | | | | | | | | | | | | | | | |
| Youd, et al (2001) | | | | | | | | | | | | | | | | | | | | | | | | |
| Max. Horiz. Acceleration, A _{max} /g: | | | | | | | | | | | | | | | | | | | | | | | | |
| Earthquake Moment Magnitude, M: | | | | | | | | | | | | | | | | | | | | | | | | |
| Magnitude Scaling Factor, MSF: | | | | | | | | | | | | | | | | | | | | | | | | |
| 6887.43 | | | | | | | | | | | | | | | | | | | | | | | | |
| Water surface elevation during CPT investigation (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | |
| 6887.43 | | | | | | | | | | | | | | | | | | | | | | | | |
| Water surface elevation at t ₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | |
| 6887.43 | | | | | | | | | | | | | | | | | | | | | | | | |
| Water surface elevation at t ₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.44 | | | | | | | | | | | | | | | | | | | | | | | | |
| Scaling Factor for stress ration, r _m | | | | | | | | | | | | | | | | | | | | | | | | |
| 0.47 | | | | | | | | | | | | | | | | | | | | | | | | |
| Volumetric Strain Ratio for Site-Specific Design Earthquake | | | | | | | | | | | | | | | | | | | | | | | | |
| 8.26 | | | | | | | | | | | | | | | | | | | | | | | | |
| Equiv. Number of Uniform Strain Cycles, N | | | | | | | | | | | | | | | | | | | | | | | | |

| 2013 CPT Data from ConeTec | | | | | | | | | | CPT Data Interpretations | | | | | | | | | | Conditions at t _i | | | | |
|----------------------------|-----------|-------|-------|-------|---------|---------|-------|---------------|-------|--|-------------|-------------|-----------------------|------------------------------|---------------------------------|--------------------------|--|---|----------------------------|------------------------------|--------------------------------|--------------------------------------|------------------------------------|--|
| Depth at time of CPT | Elevation | qt | fs | qc | Pw (u2) | Pw (u2) | fs/qt | | | Material Type (per drilling log from coupled borehole) | Unit Weight | Unit Weight | Stress at time of CPT | Pore Pressure at time of CPT | Effective Stress at time of CPT | Saturated at time of CPT | Normalized Cone Penetration Resistance, Q _c | Normalized Friction Ratio, F _r (%) | Type Index, I _c | FC | Total Stress at t _i | Equi Pore Pressure at t _i | Effective Stress at t _i | |
| (ft) | (ft amsl) | TSF | TSF | TSF | (ft) | PSI | (%) | | | | (pcf) | (pcf) | (tsf) | (tsf) | (tsf) | 1=Yes 0=No | | | | % | (tsf) | (tsf) | (tsf) | |
| 0.164 | 6977.27 | 6.5 | 0.212 | 6.4 | 2.1 | 0.91 | 3.29% | Radon Barrier | 0.061 | 122.3 | 0.01 | 0.000 | 0.01 | 0 | 642 | 3.29% | 1.9 | 59% | 0.27 | 0.000 | 0.27 | | | |
| 0.328 | 6977.10 | 29.7 | 0.150 | 29.7 | 3.2 | 1.40 | 0.50% | Radon Barrier | 0.061 | 122.3 | 0.02 | 0.000 | 0.02 | 0 | 1480 | 0.51% | 1.0 | 59% | 0.28 | 0.000 | 0.28 | | | |
| 0.492 | 6976.94 | 35.1 | 0.256 | 35.1 | 3.3 | 1.42 | 0.73% | Radon Barrier | 0.061 | 122.3 | 0.03 | 0.000 | 0.03 | 0 | 1165 | 0.73% | 1.2 | 59% | 0.29 | 0.000 | 0.29 | | | |
| 0.856 | 6976.77 | 33.5 | 0.409 | 33.5 | 3.6 | 1.55 | 1.22% | Radon Barrier | 0.061 | 122.3 | 0.04 | 0.000 | 0.04 | 0 | 833 | 1.22% | 1.4 | 59% | 0.30 | 0.000 | 0.30 | | | |
| 0.820 | 6976.61 | 25.8 | 0.463 | 25.8 | 5.9 | 2.56 | 1.80% | Radon Barrier | 0.061 | 122.3 | 0.05 | 0.000 | 0.05 | 0 | 513 | 1.80% | 1.7 | 59% | 0.31 | 0.000 | 0.31 | | | |
| 0.984 | 6976.45 | 26.6 | 0.869 | 26.6 | 5.8 | 2.50 | 3.26% | Radon Barrier | 0.061 | 122.3 | 0.06 | 0.000 | 0.06 | 0 | 441 | 3.27% | 1.9 | 59% | 0.32 | 0.000 | 0.32 | | | |
| 1.148 | 6976.28 | 30.7 | 1.085 | 30.7 | 6.5 | 2.83 | 3.53% | Radon Barrier | 0.061 | 122.3 | 0.07 | 0.000 | 0.07 | 0 | 436 | 3.54% | 2.0 | 59% | 0.33 | 0.000 | 0.33 | | | |
| 1.312 | 6976.12 | 41.4 | 1.070 | 41.4 | 4.0 | 1.75 | 2.58% | Radon Barrier | 0.061 | 122.3 | 0.08 | 0.000 | 0.08 | 0 | 515 | 2.59% | 1.8 | 59% | 0.34 | 0.000 | 0.34 | | | |
| 1.476 | 6975.95 | 56.7 | 1.208 | 56.7 | 2.3 | 0.98 | 2.13% | Radon Barrier | 0.061 | 122.3 | 0.09 | 0.000 | 0.09 | 0 | 628 | 2.13% | 1.7 | 59% | 0.35 | 0.000 | 0.35 | | | |
| 1.640 | 6975.79 | 84.8 | 1.832 | 84.8 | 2.7 | 1.16 | 2.16% | Radon Barrier | 0.061 | 122.3 | 0.10 | 0.000 | 0.10 | 0 | 844 | 2.16% | 1.6 | 59% | 0.36 | 0.000 | 0.36 | | | |
| 1.804 | 6975.63 | 103.7 | 2.230 | 103.6 | 3.7 | 1.59 | 2.15% | Radon Barrier | 0.061 | 122.3 | 0.11 | 0.000 | 0.11 | 0 | 938 | 2.15% | 1.6 | 59% | 0.37 | 0.000 | 0.37 | | | |
| 1.968 | 6975.46 | 128.1 | 3.024 | 128.1 | 4.4 | 1.89 | 2.36% | Radon Barrier | 0.061 | 122.3 | 0.12 | 0.000 | 0.12 | 0 | 1063 | 2.36% | 1.7 | 59% | 0.38 | 0.000 | 0.38 | | | |
| 2.133 | 6975.30 | 163.8 | 4.014 | 163.8 | 4.6 | 1.99 | 2.45% | General Fill | 0.057 | 113.8 | 0.13 | 0.000 | 0.13 | 0 | 1262 | 2.45% | 1.7 | 48% | 0.39 | 0.000 | 0.39 | | | |
| 2.297 | 6975.13 | 183.5 | 4.842 | 183.5 | 4.1 | 1.77 | 2.64% | General Fill | 0.057 | 113.8 | 0.14 | 0.000 | 0.14 | 0 | 1319 | 2.64% | 1.7 | 48% | 0.40 | 0.000 | 0.40 | | | |
| 2.461 | 6974.97 | 174.0 | 5.348 | 174.0 | 3.0 | 1.28 | 3.07% | General Fill | 0.057 | 113.8 | 0.15 | 0.000 | 0.15 | 0 | 1172 | 3.08% | 1.8 | 48% | 0.41 | 0.000 | 0.41 | | | |
| 2.625 | 6974.81 | 163.0 | 4.389 | 163.0 | 1.8 | 0.79 | 2.69% | General Fill | 0.057 | 113.8 | 0.16 | 0.000 | 0.16 | 0 | 1032 | 2.70% | 1.7 | 48% | 0.42 | 0.000 | 0.42 | | | |
| 2.789 | 6974.64 | 171.9 | 3.486 | 171.8 | 2.5 | 1.08 | 2.03% | General Fill | 0.057 | 113.8 | 0.17 | 0.000 | 0.17 | 0 | 1028 | 2.03% | 1.6 | 48% | 0.43 | 0.000 | 0.43 | | | |
| 2.953 | 6974.48 | 198.9 | 4.100 | 198.9 | 3.4 | 1.49 | 2.06% | General Fill | 0.057 | 113.8 | 0.18 | 0.000 | 0.18 | 0 | 1127 | 2.06% | 1.6 | 48% | 0.44 | 0.000 | 0.44 | | | |
| 3.117 | 6974.31 | 196.5 | 4.240 | 196.4 | 4.3 | 1.87 | 2.16% | General Fill | 0.057 | 113.8 | 0.19 | 0.000 | 0.19 | 0 | 1057 | 2.16% | 1.6 | 48% | 0.45 | 0.000 | 0.45 | | | |
| 3.281 | 6974.15 | 180.5 | 5.026 | 180.5 | 5.1 | 2.20 | 2.78 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|-------|-------|-------|------|-------|-------|--------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|----|-------|-------|----|--------|-------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 10.663 | 6966.77 | 44.1 | 0.988 | 44.1 | -0.5 | -0.20 | 2.24% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 71 | 2.27% | 2.3 | 48% | 0.88 | 0.000 | 0.88 | 4.55 | 821 | 1.8E-03 | 1.2E+03 | 298 | 0.9546 | 1.4E-04 | 19 | 0.202 | 0.198 | 67 | 18.012 | 10527 | 7263 | 1463 | 5716 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001607 | 0.34 | 0.079 | 0.797 | 0.26% | 0.0004 |
| 10.827 | 6966.60 | 54.5 | 1.270 | 54.5 | 0.1 | 0.06 | 2.33% | General Fill | 0.057 | 113.8 | 0.62 | 0.000 | 0.62 | 0 | 86 | 2.36% | 2.2 | 48% | 0.89 | 0.000 | 0.89 | 4.60 | 821 | 1.8E-03 | 1.2E+03 | 293 | 0.9536 | 1.4E-04 | 19 | 0.203 | 0.198 | 68 | 18.279 | 10481 | 7231 | 1464 | 5693 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001661 | 0.34 | 0.079 | 0.797 | 0.26% | 0.0004 |
| 10.991 | 6966.44 | 62.1 | 1.974 | 62.1 | 0.3 | 0.12 | 3.18% | General Fill | 0.057 | 113.8 | 0.63 | 0.000 | 0.63 | 0 | 97 | 3.21% | 2.3 | 48% | 0.90 | 0.000 | 0.90 | 4.65 | 821 | 1.8E-03 | 1.2E+03 | 289 | 0.9527 | 1.4E-04 | 19 | 0.203 | 0.199 | 69 | 18.546 | 10436 | 7199 | 1465 | 5670 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001715 | 0.34 | 0.079 | 0.797 | 0.27% | 0.0004 |
| 11.155 | 6966.28 | 61.7 | 2.643 | 61.7 | -0.1 | -0.06 | 4.28% | General Fill | 0.057 | 113.8 | 0.64 | 0.000 | 0.64 | 0 | 95 | 4.33% | 2.4 | 48% | 0.91 | 0.000 | 0.91 | 4.70 | 821 | 1.8E-03 | 1.2E+03 | 284 | 0.9517 | 1.4E-04 | 19 | 0.204 | 0.199 | 70 | 18.813 | 10392 | 7168 | 1466 | 5647 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001768 | 0.34 | 0.079 | 0.797 | 0.28% | 0.0005 |
| 11.319 | 6966.11 | 58.6 | 2.825 | 58.6 | 0.3 | 0.12 | 4.82% | General Fill | 0.057 | 113.8 | 0.65 | 0.000 | 0.65 | 0 | 89 | 4.87% | 2.4 | 48% | 0.92 | 0.000 | 0.92 | 4.75 | 821 | 1.8E-03 | 1.2E+03 | 280 | 0.9507 | 1.4E-04 | 19 | 0.204 | 0.200 | 71 | 19.080 | 10349 | 7137 | 1467 | 5625 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001820 | 0.34 | 0.079 | 0.797 | 0.29% | 0.0005 |
| 11.483 | 6965.95 | 62.0 | 2.694 | 62.0 | 0.5 | 0.23 | 4.34% | General Fill | 0.057 | 113.8 | 0.66 | 0.000 | 0.66 | 0 | 93 | 4.39% | 2.4 | 48% | 0.93 | 0.000 | 0.93 | 4.80 | 821 | 1.8E-03 | 1.2E+03 | 275 | 0.9497 | 1.4E-04 | 19 | 0.205 | 0.201 | 72 | 19.347 | 10306 | 7107 | 1468 | 5603 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001873 | 0.34 | 0.079 | 0.797 | 0.30% | 0.0005 |
| 11.647 | 6965.78 | 72.6 | 2.346 | 72.6 | 0.2 | 0.08 | 3.23% | General Fill | 0.057 | 113.8 | 0.67 | 0.000 | 0.67 | 0 | 107 | 3.26% | 2.3 | 48% | 0.94 | 0.000 | 0.94 | 4.85 | 821 | 1.8E-03 | 1.2E+03 | 271 | 0.9487 | 1.5E-04 | 19 | 0.205 | 0.201 | 73 | 19.614 | 10264 | 7077 | 1469 | 5582 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001924 | 0.34 | 0.079 | 0.797 | 0.31% | 0.0005 |
| 11.811 | 6965.62 | 70.7 | 2.347 | 70.7 | 0.1 | 0.04 | 3.32% | General Fill | 0.057 | 113.8 | 0.68 | 0.000 | 0.68 | 0 | 103 | 3.35% | 2.3 | 48% | 0.94 | 0.000 | 0.94 | 4.90 | 821 | 1.8E-03 | 1.2E+03 | 266 | 0.9476 | 1.5E-04 | 19 | 0.206 | 0.202 | 74 | 19.881 | 10222 | 7048 | 1470 | 5561 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001976 | 0.34 | 0.079 | 0.797 | 0.31% | 0.0005 |
| 11.975 | 6965.46 | 57.0 | 2.344 | 57.0 | -0.2 | -0.10 | 4.11% | General Fill | 0.057 | 113.8 | 0.69 | 0.000 | 0.69 | 0 | 82 | 4.16% | 2.4 | 48% | 0.95 | 0.000 | 0.95 | 4.95 | 821 | 1.8E-03 | 1.2E+03 | 262 | 0.9466 | 1.5E-04 | 19 | 0.206 | 0.202 | 75 | 20.148 | 10181 | 7019 | 1471 | 5540 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002027 | 0.34 | 0.079 | 0.797 | 0.32% | 0.0005 |
| 12.139 | 6965.29 | 46.1 | 2.132 | 46.2 | -0.4 | -0.18 | 4.62% | General Fill | 0.057 | 113.8 | 0.70 | 0.000 | 0.70 | 0 | 65 | 4.69% | 2.5 | 48% | 0.96 | 0.000 | 0.96 | 5.00 | 821 | 1.8E-03 | 1.2E+03 | 258 | 0.9455 | 1.5E-04 | 19 | 0.207 | 0.203 | 76 | 20.415 | 10140 | 6991 | 1472 | 5519 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002077 | 0.34 | 0.079 | 0.797 | 0.33% | 0.0005 |
| 12.303 | 6965.13 | 38.0 | 1.770 | 38.0 | -0.2 | -0.10 | 4.66% | General Fill | 0.057 | 113.8 | 0.71 | 0.000 | 0.71 | 0 | 53 | 4.75% | 2.6 | 48% | 0.97 | 0.000 | 0.97 | 5.05 | 821 | 1.8E-03 | 1.2E+03 | 254 | 0.9444 | 1.5E-04 | 19 | 0.207 | 0.203 | 77 | 20.682 | 10100 | 6963 | 1473 | 5499 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002128 | 0.34 | 0.079 | 0.797 | 0.34% | 0.0006 |
| 12.467 | 6964.96 | 34.0 | 1.387 | 34.0 | -0.6 | -0.24 | 4.08% | General Fill | 0.057 | 113.8 | 0.72 | 0.000 | 0.72 | 0 | 46 | 4.17% | 2.6 | 48% | 0.98 | 0.000 | 0.98 | 5.10 | 821 | 1.8E-03 | 1.2E+03 | 250 | 0.9433 | 1.5E-04 | 19 | 0.208 | 0.204 | 78 | 20.949 | 10061 | 6935 | 1474 | 5479 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002177 | 0.34 | 0.079 | 0.797 | 0.35% | 0.0006 |
| 12.631 | 6964.80 | 33.8 | 0.990 | 33.8 | -0.5 | -0.20 | 2.93% | General Fill | 0.057 | 113.8 | 0.73 | 0.000 | 0.73 | 0 | 45 | 3.00% | 2.5 | 48% | 0.99 | 0.000 | 0.99 | 5.15 | 821 | 1.8E-03 | 1.2E+03 | 246 | 0.9422 | 1.5E-04 | 19 | 0.208 | 0.204 | 79 | 21.216 | 10022 | 6907 | 1475 | 5459 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002227 | 0.34 | 0.079 | 0.797 | 0.35% | 0.0006 |
| 12.795 | 6964.63 | 35.2 | 0.855 | 35.3 | -0.5 | -0.20 | 2.43% | General Fill | 0.057 | 113.8 | 0.74 | 0.000 | 0.74 | 0 | 47 | 2.48% | 2.4 | 48% | 1.00 | 0.000 | 1.00 | 5.20 | 821 | 1.8E-03 | 1.2E+03 | 242 | 0.9411 | 1.5E-04 | 19 | 0.209 | 0.205 | 80 | 21.483 | 9983 | 6880 | 1476 | 5439 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002276 | 0.34 | 0.079 | 0.797 | 0.36% | 0.0006 |
| 12.959 | 6964.47 | 38.4 | 0.748 | 38.4 | -0.5 | -0.23 | 1.95% | General Fill | 0.057 | 113.8 | 0.75 | 0.000 | 0.75 | 0 | 50 | 1.99% | 2.3 | 48% | 1.01 | 0.000 | 1.01 | 5.25 | 821 | 1.8E-03 | 1.2E+03 | 238 | 0.9399 | 1.6E-04 | 19 | 0.209 | 0.205 | 81 | 21.751 | 9945 | 6854 | 1477 | 5420 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002325 | 0.34 | 0.079 | 0.797 | 0.37% | 0.0006 |
| 13.123 | 6964.31 | 38.4 | 0.813 | 38.4 | -0.4 | -0.16 | 2.12% | General Fill | 0.057 | 113.8 | 0.76 | 0.000 | 0.76 | 0 | 50 | 2.16% | 2.4 | 48% | 1.02 | 0.000 | 1.02 | 5.30 | 821 | 1.8E-03 | 1.2E+03 | 235 | 0.9387 | 1.6E-04 | 19 | 0.209 | 0.206 | 82 | 22.018 | 9908 | 6828 | 1478 | 5401 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002373 | 0.34 | 0.079 | 0.797 | 0.38% | 0.0006 |
| 13.287 | 6964.14 | 40.0 | 0.924 | 40.0 | -0.4 | -0.18 | 2.31% | General Fill | 0.057 | 113.8 | 0.76 | 0.000 | 0.76 | 0 | 51 | 2.36% | 2.4 | 48% | 1.03 | 0.000 | 1.03 | 5.35 | 821 | 1.8E-03 | 1.2E+03 | 231 | 0.9376 | 1.6E-04 | 19 | 0.210 | 0.206 | 83 | 22.285 | 9871 | 6802 | 1479 | 5382 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002421 | 0.34 | 0.079 | 0.797 | 0.39% | 0.0006 |
| 13.451 | 6963.98 | 41.2 | 1.059 | 41.2 | -0.2 | -0.10 | 2.57% | General Fill | 0.057 | 113.8 | 0.77 | 0.000 | 0.77 | 0 | 52 | 2.62% | 2.4 | 48% | 1.04 | 0.000 | 1.04 | 5.40 | 821 | 1.8E-03 | 1.2E+03 | 227 | 0.9364 | 1.6E-04 | 19 | 0.210 | 0.207 | 84 | 22.552 | 9834 | 6776 | 1480 | 5364 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002469 | 0.34 | 0.079 | 0.797 | 0.39% | 0.0006 |
| 13.615 | 6963.81 | 42.6 | 1.130 | 42.6 | -0.4 | -0.16 | 2.65% | General Fill | 0.057 | 113.8 | 0.78 | 0.000 | 0.78 | 0 | 53 | 2.70% | 2.4 | 48% | 1.05 | 0.000 | 1.05 | 5.45 | 821 | 1.8E-03 | 1.2E+03 | 224 | 0.9351 | 1.6E-04 | 19 | 0.211 | 0.207 | 85 | 22.819 | 9798 | 6751 | 1481 | 5345 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002516 | 0.34 | 0.079 | 0.797 | 0.40% | 0.0007 |
| 13.779 | 6963.65 | 42.1 | 1.133 | 42.1 | -0.3 | -0.12 | 2.69% | General Fill | 0.057 | 113.8 | 0.79 | 0.000 | 0.79 | 0 | 52 | 2.74% | 2.4 | 48% | 1.06 | 0.000 | 1.06 | 5.50 | 821 | 1.8E-03 | 1.2E+03 | 220 | 0.9339 | 1.6E-04 | 19 | 0.211 | 0.208 | 86 | 23.086 | 9763 | 6726 | 1482 | 5327 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002563 | 0.34 | 0.079 | 0.797 | 0.41% | 0.0007 |
| 13.943 | 6963.49 | 42.6 | 0.951 | 42.6 | -0.6 | -0.26 | 2.23% | General Fill | 0.057 | 113.8 | 0.80 | 0.000 | 0.80 | 0 | 52 | 2.28% | 2.4 | 48% | 1.07 | 0.000 | 1.07 | 5.55 | 821 | 1.8E-03 | 1.2E+03 | 217 | 0.9326 | 1.6E-04 | 19 | 0.212 | 0.208 | 87 | 23.353 | 9728 | 6701 | 1483 | 5309 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002609 | 0.34 | 0.079 | 0.797 | 0.42% | 0.0007 |
| 14.107 | 6963.32 | 46.4 | 0.948 | 46.4 | -0.1 | -0.04 | 2.04% | General Fill | 0.057 | 113.8 | 0.81 | 0.000 | 0.81 | 0 | 56 | 2.08% | 2.3 | 48% | 1.08 | 0.000 | 1.08 | 5.60 | 821 | 1.8E-03 | 1.2E+03 | 213 | 0.9314 | 1.6E-04 | 19 | 0.212 | 0.209 | 88 | 23.620 | 9693 | 6677 | 1484 | 5292 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002655 | 0.34 | 0.079 | 0.797 | 0.42% | 0.0007 |
| 14.271 | 6963.16 | 43.4 | 0.939 | 43.4 | -0.3 | -0.12 | 2.16% | General Fill | 0.057 | 113.8 | 0.82 | 0.000 | 0.82 | 0 | 52 | 2.20% | 2.4 | 48% | 1.08 | 0.000 | 1.08 | 5.65 | 821 | 1.8E-03 | 1.2E+03 | 210 | 0.9301 | 1.7E-04 | 19 | 0.212 | 0.209 | 89 | 23.887 | 9659 | 6652 | 1485 | 5274 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002701 | 0.34 | 0.079 | 0.797 | 0.43% | 0.0007 |
| 14.436 | 6962.99 | 41.3 | 0.907 | 41.3 | -0.1 | -0.06 | 2.20% | General Fill | 0.057 | 113.8 | 0.83 | 0.000 | 0.83 | 0 | 49 | 2.24% | 2.4 | 48% | 1.09 | 0.000 | 1.09 | 5.70 | 821 | 1.8E-03 | 1.2E+03 | 207 | 0.9288 | 1.7E-04 | 19 | 0.213 | 0.210 | 90 | 24.154 | 9625 | 6629 | 1486 | 5257 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002746 | 0.34 | 0.079 | 0.797 | 0.44% | 0.0007 |
| 14.600 | 6962.83 | 46.0 | 1.104 | 46.0 | 0.1 | 0.02 | 2.40% | General Fill | 0.057 | 113.8 | 0.84 | 0.000 | 0.84 | 0 | 54 | 2.44% | 2.4 | 48% | 1.10 | 0.000 | 1.10 | 5.75 | 821 | 1.8E-03 | 1.2E+03 | 204 | 0.9274 | 1.7E-04 | 19 | 0.213 | 0.210 | 91 | 24.421 | 9591 | 6605 | 1487 | 5240 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002791 | 0.34 | 0.079 | 0.797 | 0.44% | 0.0007 |
| 14.764 | 6962.67 | 68.8 | 1.832 | 68.8 | 0.6 | 0.24 | 2.66% | General Fill | 0.057 | 113.8 | 0.85 | 0.000 | 0.85 | 0 | 80 | 2.69% | 2.3 | 48% | 1.11 | 0.000 | 1.11 | 5.80 | 838 | 1.8E-03 | 1.2E+03 | 200 | 0.9261 | 1.6E-04 | 19 | 0.214 | 0.211 | 92 | 24.688 | 9558 | 6582 | 1488 | 5224 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002818 | 0.34 | 0.079 | 0.797 | 0.44% | 0.0007 |
| 14.928 | 6962.50 | 139.9 | 2.813 | 139.9 | 2.2 | 0.96 | 2.01% | General Fill | 0.057 | 113.8 | 0.86 | 0.000 | 0.86 | 0 | 162 | 2.02% | 2.0 | 48% | 1.12 | 0.000 | 1.12 | 5.85 | 838 | 1.8E-03 | 1.2E+03 | 197 | 0.9247 | 1.6E-04 | 19 | 0.214 | 0.211 | 93 | 24.955 | 9526 | 6559 | 1489 | 5207 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002860 | 0.34 | 0.079 | 0.797 | 0.41% | 0.0007 |
| 15.092 | 6962.34 | 251.0 | 3.685 | 251.0 | 3.9 | 1.67 | 1.47% | General Fill | 0.057 | 113.8 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|-------|-------|-------|------|-------|-------|--------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|--------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 23.786 | 6953.64 | 105.8 | 3.484 | 105.7 | 3.6 | 1.55 | 3.29% | General Fill | 0.057 | 113.8 | 1.36 | 0.000 | 1.36 | 0 | 77 | 3.34% | 2.4 | 48% | 1.63 | 0.000 | 1.63 | 8.55 | 853 | 1.8E-03 | 1.3E+03 | 88 | 0.8160 | 2.0E-04 | 19 | 0.233 | 0.233 | 147 | 39.371 | 8181 | 5617 | 1543 | 4530 | 0.05% | 1.70 | 0.75 | 0.020% | 0.003810 | 0.34 | 0.079 | 0.797 | 0.61% | 0.0010 |
| 23.950 | 6953.48 | 104.9 | 3.193 | 104.9 | 3.9 | 1.69 | 3.04% | General Fill | 0.057 | 113.8 | 1.37 | 0.000 | 1.37 | 0 | 76 | 3.08% | 2.3 | 48% | 1.64 | 0.000 | 1.64 | 8.60 | 853 | 1.8E-03 | 1.3E+03 | 87 | 0.8132 | 2.0E-04 | 19 | 0.234 | 0.233 | 148 | 39.638 | 8162 | 5603 | 1544 | 4521 | 0.05% | 1.70 | 0.75 | 0.020% | 0.003823 | 0.34 | 0.079 | 0.797 | 0.61% | 0.0010 |
| 24.114 | 6953.32 | 100.0 | 2.838 | 99.9 | 3.3 | 1.44 | 2.84% | General Fill | 0.057 | 113.8 | 1.38 | 0.000 | 1.38 | 0 | 71 | 2.88% | 2.3 | 48% | 1.64 | 0.000 | 1.64 | 8.65 | 853 | 1.8E-03 | 1.3E+03 | 86 | 0.8104 | 2.0E-04 | 19 | 0.234 | 0.234 | 149 | 39.905 | 8143 | 5590 | 1545 | 4511 | 0.05% | 1.70 | 0.75 | 0.020% | 0.003835 | 0.34 | 0.079 | 0.797 | 0.61% | 0.0010 |
| 24.278 | 6953.15 | 77.9 | 2.845 | 77.9 | 2.2 | 0.94 | 3.65% | General Fill | 0.057 | 113.8 | 1.39 | 0.000 | 1.39 | 0 | 55 | 3.72% | 2.5 | 48% | 1.65 | 0.000 | 1.65 | 8.70 | 853 | 1.8E-03 | 1.3E+03 | 85 | 0.8076 | 2.0E-04 | 19 | 0.234 | 0.234 | 150 | 40.172 | 8124 | 5577 | 1546 | 4502 | 0.05% | 1.70 | 0.75 | 0.020% | 0.003846 | 0.34 | 0.079 | 0.797 | 0.61% | 0.0010 |
| 24.442 | 6952.99 | 67.3 | 2.777 | 67.3 | 1.8 | 0.77 | 4.13% | General Fill | 0.057 | 113.8 | 1.40 | 0.000 | 1.40 | 0 | 47 | 4.21% | 2.6 | 48% | 1.66 | 0.000 | 1.66 | 8.75 | 853 | 1.8E-03 | 1.3E+03 | 84 | 0.8048 | 2.0E-04 | 19 | 0.235 | 0.234 | 151 | 40.438 | 8106 | 5564 | 1547 | 4493 | 0.05% | 1.70 | 0.75 | 0.020% | 0.003857 | 0.34 | 0.079 | 0.797 | 0.61% | 0.0010 |
| 24.606 | 6952.82 | 58.9 | 2.669 | 58.9 | 1.9 | 0.81 | 4.53% | General Fill | 0.057 | 113.8 | 1.41 | 0.000 | 1.41 | 0 | 41 | 4.64% | 2.6 | 48% | 1.67 | 0.000 | 1.67 | 8.80 | 897 | 1.8E-03 | 1.4E+03 | 82 | 0.8019 | 1.8E-04 | 19 | 0.235 | 0.235 | 152 | 40.705 | 8087 | 5551 | 1548 | 4483 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003008 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 24.770 | 6952.66 | 49.3 | 2.286 | 49.3 | 1.7 | 0.75 | 4.64% | General Fill | 0.057 | 113.8 | 1.42 | 0.000 | 1.42 | 0 | 34 | 4.78% | 2.7 | 48% | 1.68 | 0.000 | 1.68 | 8.85 | 897 | 1.8E-03 | 1.4E+03 | 81 | 0.7990 | 1.8E-04 | 19 | 0.235 | 0.235 | 153 | 40.972 | 8069 | 5538 | 1549 | 4474 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003016 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 24.934 | 6952.50 | 52.0 | 1.715 | 52.0 | 1.9 | 0.83 | 3.30% | General Fill | 0.057 | 113.8 | 1.43 | 0.000 | 1.43 | 0 | 35 | 3.39% | 2.6 | 48% | 1.69 | 0.000 | 1.69 | 8.90 | 897 | 1.8E-03 | 1.4E+03 | 80 | 0.7961 | 1.8E-04 | 19 | 0.235 | 0.235 | 154 | 41.239 | 8050 | 5525 | 1550 | 4465 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003024 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 25.098 | 6952.33 | 45.4 | 1.412 | 45.4 | 1.6 | 0.71 | 3.11% | General Fill | 0.057 | 113.8 | 1.44 | 0.000 | 1.44 | 0 | 31 | 3.21% | 2.6 | 48% | 1.70 | 0.000 | 1.70 | 8.95 | 897 | 1.8E-03 | 1.4E+03 | 79 | 0.7931 | 1.8E-04 | 19 | 0.236 | 0.236 | 155 | 41.506 | 8032 | 5512 | 1551 | 4456 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003031 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 25.262 | 6952.17 | 38.8 | 1.265 | 38.8 | -0.5 | -0.20 | 3.26% | General Fill | 0.057 | 113.8 | 1.45 | 0.000 | 1.45 | 0 | 26 | 3.38% | 2.7 | 48% | 1.71 | 0.000 | 1.71 | 9.00 | 897 | 1.8E-03 | 1.4E+03 | 78 | 0.7901 | 1.9E-04 | 19 | 0.236 | 0.236 | 156 | 41.773 | 8014 | 5500 | 1552 | 4447 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003038 | 0.34 | 0.079 | 0.797 | 0.48% | 0.0008 |
| 25.426 | 6952.00 | 37.9 | 1.312 | 37.9 | -0.5 | -0.23 | 3.47% | General Fill | 0.057 | 113.8 | 1.46 | 0.000 | 1.46 | 0 | 25 | 3.60% | 2.7 | 48% | 1.72 | 0.000 | 1.72 | 9.05 | 897 | 1.8E-03 | 1.4E+03 | 77 | 0.7871 | 1.9E-04 | 19 | 0.236 | 0.236 | 157 | 42.040 | 7996 | 5487 | 1553 | 4438 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003044 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 25.590 | 6951.84 | 38.4 | 1.611 | 38.4 | -0.6 | -0.26 | 4.19% | General Fill | 0.057 | 113.8 | 1.46 | 0.000 | 1.46 | 0 | 25 | 4.36% | 2.8 | 48% | 1.73 | 0.000 | 1.73 | 9.10 | 897 | 1.8E-03 | 1.4E+03 | 76 | 0.7840 | 1.9E-04 | 19 | 0.237 | 0.237 | 158 | 42.307 | 7978 | 5475 | 1554 | 4429 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003050 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 25.754 | 6951.68 | 47.0 | 1.787 | 47.0 | -0.5 | -0.23 | 3.80% | General Fill | 0.057 | 113.8 | 1.47 | 0.000 | 1.47 | 0 | 31 | 3.92% | 2.7 | 48% | 1.74 | 0.000 | 1.74 | 9.15 | 897 | 1.8E-03 | 1.4E+03 | 75 | 0.7810 | 1.9E-04 | 19 | 0.237 | 0.237 | 159 | 42.574 | 7961 | 5462 | 1555 | 4420 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003054 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 25.918 | 6951.51 | 49.2 | 1.819 | 49.2 | -0.6 | -0.26 | 3.70% | General Fill | 0.057 | 113.8 | 1.48 | 0.000 | 1.48 | 0 | 32 | 3.81% | 2.7 | 48% | 1.75 | 0.000 | 1.75 | 9.20 | 897 | 1.8E-03 | 1.4E+03 | 74 | 0.7778 | 1.9E-04 | 19 | 0.237 | 0.237 | 160 | 42.841 | 7943 | 5450 | 1556 | 4412 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003058 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.082 | 6951.35 | 50.7 | 1.690 | 50.7 | -0.5 | -0.23 | 3.33% | General Fill | 0.057 | 113.8 | 1.49 | 0.000 | 1.49 | 0 | 33 | 3.43% | 2.6 | 48% | 1.76 | 0.000 | 1.76 | 9.25 | 897 | 1.8E-03 | 1.4E+03 | 73 | 0.7747 | 1.9E-04 | 19 | 0.238 | 0.238 | 161 | 43.108 | 7926 | 5438 | 1557 | 4403 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003062 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.246 | 6951.18 | 53.8 | 1.866 | 53.8 | -0.5 | -0.20 | 3.47% | General Fill | 0.057 | 113.8 | 1.50 | 0.000 | 1.50 | 0 | 35 | 3.57% | 2.6 | 48% | 1.77 | 0.000 | 1.77 | 9.30 | 897 | 1.8E-03 | 1.4E+03 | 72 | 0.7715 | 1.9E-04 | 19 | 0.238 | 0.238 | 162 | 43.375 | 7909 | 5426 | 1558 | 4395 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003065 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.410 | 6951.02 | 59.2 | 2.031 | 59.2 | -0.5 | -0.23 | 3.43% | General Fill | 0.057 | 113.8 | 1.51 | 0.000 | 1.51 | 0 | 38 | 3.52% | 2.6 | 48% | 1.78 | 0.000 | 1.78 | 9.35 | 897 | 1.8E-03 | 1.4E+03 | 71 | 0.7683 | 1.9E-04 | 19 | 0.238 | 0.238 | 163 | 43.641 | 7892 | 5414 | 1559 | 4386 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003067 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.574 | 6950.86 | 60.6 | 1.994 | 60.6 | -0.1 | -0.06 | 3.29% | General Fill | 0.057 | 113.8 | 1.52 | 0.000 | 1.52 | 0 | 39 | 3.37% | 2.6 | 48% | 1.78 | 0.000 | 1.78 | 9.40 | 897 | 1.8E-03 | 1.4E+03 | 70 | 0.7651 | 1.9E-04 | 19 | 0.238 | 0.239 | 164 | 43.908 | 7875 | 5402 | 1560 | 4378 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003069 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.739 | 6950.69 | 64.6 | 2.277 | 64.6 | -0.3 | -0.12 | 3.53% | General Fill | 0.057 | 113.8 | 1.53 | 0.000 | 1.53 | 0 | 41 | 3.61% | 2.6 | 48% | 1.79 | 0.000 | 1.79 | 9.45 | 897 | 1.8E-03 | 1.4E+03 | 69 | 0.7618 | 1.9E-04 | 19 | 0.239 | 0.239 | 165 | 44.175 | 7858 | 5391 | 1561 | 4369 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003070 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 26.903 | 6950.53 | 68.4 | 2.223 | 68.5 | -0.4 | -0.16 | 3.25% | General Fill | 0.057 | 113.8 | 1.54 | 0.000 | 1.54 | 0 | 43 | 3.32% | 2.5 | 48% | 1.80 | 0.000 | 1.80 | 9.50 | 897 | 1.8E-03 | 1.4E+03 | 68 | 0.7585 | 1.9E-04 | 19 | 0.239 | 0.239 | 166 | 44.442 | 7841 | 5379 | 1562 | 4361 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003070 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.067 | 6950.36 | 64.6 | 2.093 | 64.6 | 0.1 | 0.04 | 3.24% | General Fill | 0.057 | 113.8 | 1.55 | 0.000 | 1.55 | 0 | 41 | 3.32% | 2.5 | 48% | 1.81 | 0.000 | 1.81 | 9.55 | 897 | 1.8E-03 | 1.4E+03 | 68 | 0.7552 | 1.9E-04 | 19 | 0.239 | 0.240 | 167 | 44.709 | 7825 | 5367 | 1563 | 4353 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003070 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.231 | 6950.20 | 67.9 | 2.017 | 67.9 | -0.1 | -0.02 | 2.97% | General Fill | 0.057 | 113.8 | 1.56 | 0.000 | 1.56 | 0 | 43 | 3.04% | 2.5 | 48% | 1.82 | 0.000 | 1.82 | 9.60 | 897 | 1.8E-03 | 1.4E+03 | 67 | 0.7519 | 1.9E-04 | 19 | 0.240 | 0.240 | 168 | 44.976 | 7808 | 5356 | 1564 | 4345 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003069 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.395 | 6950.04 | 71.5 | 1.948 | 71.5 | 0.1 | 0.04 | 2.72% | General Fill | 0.057 | 113.8 | 1.57 | 0.000 | 1.57 | 0 | 45 | 2.79% | 2.5 | 48% | 1.83 | 0.000 | 1.83 | 9.65 | 897 | 1.8E-03 | 1.4E+03 | 66 | 0.7485 | 1.9E-04 | 19 | 0.240 | 0.240 | 169 | 45.243 | 7792 | 5344 | 1565 | 4337 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003068 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.559 | 6949.87 | 71.8 | 2.207 | 71.8 | -0.2 | -0.08 | 3.07% | General Fill | 0.057 | 113.8 | 1.58 | 0.000 | 1.58 | 0 | 45 | 3.14% | 2.5 | 48% | 1.84 | 0.000 | 1.84 | 9.70 | 897 | 1.8E-03 | 1.4E+03 | 65 | 0.7451 | 1.9E-04 | 19 | 0.240 | 0.241 | 170 | 45.510 | 7776 | 5333 | 1566 | 4328 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003066 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.723 | 6949.71 | 81.8 | 2.242 | 81.8 | 0.3 | 0.14 | 2.74% | General Fill | 0.057 | 113.8 | 1.59 | 0.000 | 1.59 | 0 | 51 | 2.79% | 2.4 | 48% | 1.85 | 0.000 | 1.85 | 9.75 | 897 | 1.8E-03 | 1.4E+03 | 64 | 0.7416 | 1.9E-04 | 19 | 0.240 | 0.241 | 171 | 45.777 | 7760 | 5322 | 1567 | 4320 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003063 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 27.887 | 6949.54 | 102.4 | 2.701 | 102.4 | 1.2 | 0.51 | 2.64% | General Fill | 0.057 | 113.8 | 1.60 | 0.000 | 1.60 | 0 | 63 | 2.68% | 2.3 | 48% | 1.86 | 0.000 | 1.86 | 9.80 | 897 | 1.8E-03 | 1.4E+03 | 63 | 0.7382 | 1.9E-04 | 19 | 0.241 | 0.241 | 172 | 46.044 | 7744 | 5311 | 1568 | 4313 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003059 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 28.051 | 6949.38 | 105.0 | 3.171 | 105.0 | 0.8 | 0.35 | 3.02% | General Fill | 0.057 | 113.8 | 1.60 | 0.000 | 1.60 | 0 | 64 | 3.07% | 2.4 | 48% | 1.87 | 0.000 | 1.87 | 9.85 | 897 | 1.8E-03 | 1.4E+03 | 63 | 0.7347 | 1.9E-04 | 19 | 0.241 | 0.242 | 173 | 46.310 | 7728 | 5299 | 1569 | 4305 | 0.04% | 1.70 | 0.75 | 0.020% | 0.003055 | 0.34 | 0.079 | 0.797 | 0.49% | 0.0008 |
| 28.215 | 6949.22 | 93.3 | 3.179 | 93.3 | 1.5 | 0.63 | 3.41% | General Fill | 0.057 | 113.8 | 1.61 | 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|-------|-------|--------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|--------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 36.909 | 6940.52 | 48.6 | 1.774 | 48.6 | 0.6 | 0.26 | 3.65% | General Fill | 0.057 | 113.8 | 2.11 | 0.000 | 2.11 | 0 | 22 | 3.82% | 2.8 | 48% | 2.37 | 0.000 | 2.37 | 12.55 | 918 | 1.8E-03 | 1.5E+03 | 35 | 0.5114 | 1.6E-04 | 19 | 0.255 | 0.257 | 227 | 60.722 | 7007 | 4796 | 1623 | 3950 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001640 | 0.34 | 0.079 | 0.797 | 0.26% | 0.0004 |
| 37.073 | 6940.36 | 54.7 | 1.759 | 54.7 | 1.1 | 0.49 | 3.22% | General Fill | 0.057 | 113.8 | 2.12 | 0.000 | 2.12 | 0 | 25 | 3.35% | 2.7 | 48% | 2.38 | 0.000 | 2.38 | 12.60 | 918 | 1.8E-03 | 1.5E+03 | 34 | 0.5068 | 1.6E-04 | 19 | 0.255 | 0.258 | 228 | 60.989 | 6996 | 4788 | 1624 | 3944 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001606 | 0.34 | 0.079 | 0.797 | 0.26% | 0.0004 |
| 37.237 | 6940.19 | 50.4 | 1.722 | 50.4 | 0.5 | 0.23 | 3.42% | General Fill | 0.057 | 113.8 | 2.13 | 0.000 | 2.13 | 0 | 23 | 3.57% | 2.8 | 48% | 2.39 | 0.000 | 2.39 | 12.65 | 918 | 1.8E-03 | 1.5E+03 | 34 | 0.5023 | 1.6E-04 | 19 | 0.255 | 0.258 | 229 | 61.256 | 6985 | 4780 | 1625 | 3939 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001572 | 0.34 | 0.079 | 0.797 | 0.25% | 0.0004 |
| 37.401 | 6940.03 | 45.6 | 1.817 | 45.6 | 1.0 | 0.43 | 3.98% | General Fill | 0.057 | 113.8 | 2.14 | 0.000 | 2.14 | 0 | 20 | 4.18% | 2.8 | 48% | 2.40 | 0.000 | 2.40 | 12.70 | 918 | 1.8E-03 | 1.5E+03 | 34 | 0.4977 | 1.6E-04 | 19 | 0.255 | 0.258 | 230 | 61.523 | 6973 | 4773 | 1626 | 3933 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001537 | 0.34 | 0.079 | 0.797 | 0.24% | 0.0004 |
| 37.565 | 6939.86 | 38.9 | 1.931 | 38.9 | 0.5 | 0.23 | 4.97% | General Fill | 0.057 | 113.8 | 2.15 | 0.000 | 2.15 | 0 | 17 | 5.26% | 3.0 | 48% | 2.41 | 0.000 | 2.41 | 12.75 | 918 | 1.8E-03 | 1.5E+03 | 33 | 0.4932 | 1.6E-04 | 19 | 0.256 | 0.258 | 231 | 61.790 | 6962 | 4765 | 1627 | 3928 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001501 | 0.34 | 0.079 | 0.797 | 0.24% | 0.0004 |
| 37.729 | 6939.70 | 37.5 | 1.975 | 37.5 | 0.6 | 0.24 | 5.27% | General Fill | 0.057 | 113.8 | 2.16 | 0.000 | 2.16 | 0 | 16 | 5.59% | 3.0 | 48% | 2.42 | 0.000 | 2.42 | 12.80 | 918 | 1.8E-03 | 1.5E+03 | 33 | 0.4886 | 1.5E-04 | 19 | 0.256 | 0.259 | 232 | 62.056 | 6951 | 4757 | 1628 | 3923 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001466 | 0.34 | 0.079 | 0.797 | 0.23% | 0.0004 |
| 37.893 | 6939.54 | 41.1 | 1.391 | 41.1 | 0.8 | 0.35 | 3.38% | General Fill | 0.057 | 113.8 | 2.16 | 0.000 | 2.16 | 0 | 18 | 3.57% | 2.8 | 48% | 2.43 | 0.000 | 2.43 | 12.85 | 918 | 1.8E-03 | 1.5E+03 | 33 | 0.4841 | 1.5E-04 | 19 | 0.256 | 0.259 | 233 | 62.323 | 6940 | 4749 | 1629 | 3917 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001429 | 0.34 | 0.079 | 0.797 | 0.23% | 0.0004 |
| 38.057 | 6939.37 | 60.9 | 1.509 | 60.9 | -0.6 | -0.24 | 2.48% | General Fill | 0.057 | 113.8 | 2.17 | 0.000 | 2.17 | 0 | 27 | 2.57% | 2.6 | 48% | 2.44 | 0.000 | 2.44 | 12.90 | 918 | 1.8E-03 | 1.5E+03 | 33 | 0.4795 | 1.5E-04 | 19 | 0.256 | 0.259 | 234 | 62.590 | 6929 | 4742 | 1630 | 3912 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001392 | 0.34 | 0.079 | 0.797 | 0.22% | 0.0004 |
| 38.221 | 6939.21 | 82.0 | 1.841 | 82.0 | 0.8 | 0.33 | 3.25% | General Fill | 0.057 | 113.8 | 2.18 | 0.000 | 2.18 | 0 | 37 | 2.31% | 2.5 | 48% | 2.45 | 0.000 | 2.45 | 12.95 | 918 | 1.8E-03 | 1.5E+03 | 32 | 0.4749 | 1.5E-04 | 19 | 0.256 | 0.259 | 235 | 62.857 | 6919 | 4734 | 1631 | 3907 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001355 | 0.34 | 0.079 | 0.797 | 0.22% | 0.0004 |
| 38.385 | 6939.04 | 55.6 | 2.193 | 55.6 | 0.5 | 0.23 | 3.95% | General Fill | 0.057 | 113.8 | 2.19 | 0.000 | 2.19 | 0 | 24 | 4.11% | 2.8 | 48% | 2.46 | 0.000 | 2.46 | 13.00 | 918 | 1.8E-03 | 1.5E+03 | 32 | 0.4704 | 1.5E-04 | 19 | 0.257 | 0.260 | 236 | 63.124 | 6908 | 4727 | 1632 | 3901 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001317 | 0.34 | 0.079 | 0.797 | 0.21% | 0.0003 |
| 38.549 | 6938.88 | 56.7 | 2.214 | 56.7 | 2.8 | 1.20 | 3.91% | General Fill | 0.057 | 113.8 | 2.20 | 0.000 | 2.20 | 0 | 25 | 4.06% | 2.8 | 48% | 2.47 | 0.000 | 2.47 | 13.05 | 918 | 1.8E-03 | 1.5E+03 | 32 | 0.4658 | 1.5E-04 | 19 | 0.257 | 0.260 | 237 | 63.391 | 6897 | 4719 | 1633 | 3896 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001279 | 0.34 | 0.079 | 0.797 | 0.20% | 0.0003 |
| 38.713 | 6938.72 | 47.8 | 1.786 | 47.7 | 2.8 | 1.22 | 3.74% | General Fill | 0.057 | 113.8 | 2.21 | 0.000 | 2.21 | 0 | 21 | 3.92% | 2.8 | 48% | 2.48 | 0.000 | 2.48 | 13.10 | 918 | 1.8E-03 | 1.5E+03 | 32 | 0.4613 | 1.5E-04 | 19 | 0.257 | 0.260 | 238 | 63.658 | 6886 | 4712 | 1634 | 3891 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001240 | 0.34 | 0.079 | 0.797 | 0.20% | 0.0003 |
| 38.877 | 6938.55 | 39.5 | 1.625 | 39.5 | 4.2 | 1.81 | 4.12% | General Fill | 0.057 | 113.8 | 2.22 | 0.000 | 2.22 | 0 | 17 | 4.36% | 2.9 | 48% | 2.48 | 0.000 | 2.48 | 13.15 | 918 | 1.8E-03 | 1.5E+03 | 31 | 0.4567 | 1.5E-04 | 19 | 0.257 | 0.261 | 239 | 63.924 | 6876 | 4704 | 1635 | 3886 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001201 | 0.34 | 0.079 | 0.797 | 0.19% | 0.0003 |
| 39.042 | 6938.39 | 36.7 | 1.507 | 36.7 | 3.3 | 1.44 | 4.10% | General Fill | 0.057 | 113.8 | 2.23 | 0.000 | 2.23 | 0 | 15 | 4.37% | 2.9 | 48% | 2.49 | 0.000 | 2.49 | 13.20 | 918 | 1.8E-03 | 1.5E+03 | 31 | 0.4522 | 1.5E-04 | 19 | 0.258 | 0.261 | 240 | 64.191 | 6865 | 4697 | 1636 | 3881 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001161 | 0.34 | 0.079 | 0.797 | 0.19% | 0.0003 |
| 39.206 | 6938.22 | 37.9 | 1.421 | 37.9 | 3.3 | 1.42 | 3.75% | General Fill | 0.057 | 113.8 | 2.24 | 0.000 | 2.24 | 0 | 16 | 3.98% | 2.9 | 48% | 2.50 | 0.000 | 2.50 | 13.25 | 918 | 1.8E-03 | 1.5E+03 | 31 | 0.4476 | 1.5E-04 | 19 | 0.258 | 0.261 | 241 | 64.458 | 6855 | 4690 | 1637 | 3876 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001121 | 0.34 | 0.079 | 0.797 | 0.18% | 0.0003 |
| 39.370 | 6938.06 | 36.3 | 1.375 | 36.3 | 2.2 | 0.96 | 3.79% | General Fill | 0.057 | 113.8 | 2.25 | 0.000 | 2.25 | 0 | 15 | 4.04% | 2.9 | 48% | 2.51 | 0.000 | 2.51 | 13.30 | 918 | 1.8E-03 | 1.5E+03 | 31 | 0.4431 | 1.5E-04 | 19 | 0.258 | 0.261 | 242 | 64.725 | 6844 | 4682 | 1638 | 3871 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001080 | 0.34 | 0.079 | 0.797 | 0.17% | 0.0003 |
| 39.534 | 6937.90 | 38.1 | 1.361 | 38.1 | 2.7 | 1.16 | 3.58% | General Fill | 0.057 | 113.8 | 2.26 | 0.000 | 2.26 | 0 | 16 | 3.80% | 2.9 | 48% | 2.52 | 0.000 | 2.52 | 13.35 | 829 | 1.8E-03 | 1.2E+03 | 30 | 0.4385 | 1.8E-04 | 19 | 0.258 | 0.262 | 243 | 64.992 | 6834 | 4675 | 1639 | 3866 | 0.04% | 1.70 | 0.75 | 0.020% | 0.002306 | 0.34 | 0.079 | 0.797 | 0.37% | 0.0006 |
| 39.698 | 6937.73 | 45.3 | 1.490 | 45.3 | 2.4 | 1.02 | 3.29% | General Fill | 0.057 | 113.8 | 2.27 | 0.000 | 2.27 | 0 | 19 | 3.47% | 2.8 | 48% | 2.53 | 0.000 | 2.53 | 13.40 | 829 | 1.8E-03 | 1.2E+03 | 30 | 0.4340 | 1.8E-04 | 19 | 0.258 | 0.262 | 244 | 65.259 | 6823 | 4668 | 1640 | 3860 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002257 | 0.34 | 0.079 | 0.797 | 0.36% | 0.0006 |
| 39.862 | 6937.57 | 46.0 | 1.458 | 46.0 | 2.4 | 1.04 | 3.17% | General Fill | 0.057 | 113.8 | 2.28 | 0.000 | 2.28 | 0 | 19 | 3.34% | 2.8 | 48% | 2.54 | 0.000 | 2.54 | 13.45 | 829 | 1.8E-03 | 1.2E+03 | 30 | 0.4295 | 1.8E-04 | 19 | 0.259 | 0.262 | 245 | 65.526 | 6813 | 4661 | 1641 | 3856 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002208 | 0.34 | 0.079 | 0.797 | 0.35% | 0.0006 |
| 40.026 | 6937.40 | 37.9 | 1.662 | 37.9 | 2.2 | 0.96 | 4.39% | General Fill | 0.057 | 113.8 | 2.29 | 0.000 | 2.29 | 0 | 16 | 4.67% | 3.0 | 48% | 2.55 | 0.000 | 2.55 | 13.50 | 829 | 1.8E-03 | 1.2E+03 | 30 | 0.4249 | 1.7E-04 | 19 | 0.259 | 0.262 | 246 | 65.792 | 6803 | 4654 | 1642 | 3851 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002159 | 0.34 | 0.079 | 0.797 | 0.34% | 0.0006 |
| 40.190 | 6937.24 | 34.3 | 1.625 | 34.3 | 2.5 | 1.08 | 4.74% | General Fill | 0.057 | 113.8 | 2.30 | 0.000 | 2.30 | 0 | 14 | 5.08% | 3.0 | 48% | 2.56 | 0.000 | 2.56 | 13.55 | 829 | 1.8E-03 | 1.2E+03 | 29 | 0.4204 | 1.7E-04 | 19 | 0.259 | 0.263 | 247 | 66.059 | 6793 | 4647 | 1643 | 3846 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002110 | 0.34 | 0.079 | 0.797 | 0.34% | 0.0006 |
| 40.354 | 6937.08 | 36.5 | 1.530 | 36.5 | 2.4 | 1.04 | 4.19% | General Fill | 0.057 | 113.8 | 2.30 | 0.000 | 2.30 | 0 | 15 | 4.47% | 3.0 | 48% | 2.57 | 0.000 | 2.57 | 13.60 | 829 | 1.8E-03 | 1.2E+03 | 29 | 0.4159 | 1.7E-04 | 19 | 0.259 | 0.263 | 248 | 66.326 | 6783 | 4639 | 1644 | 3841 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002060 | 0.34 | 0.079 | 0.797 | 0.33% | 0.0005 |
| 40.518 | 6936.91 | 38.6 | 1.514 | 38.6 | 3.1 | 1.36 | 3.92% | General Fill | 0.057 | 113.8 | 2.31 | 0.000 | 2.31 | 0 | 16 | 4.17% | 2.9 | 48% | 2.58 | 0.000 | 2.58 | 13.65 | 829 | 1.8E-03 | 1.2E+03 | 29 | 0.4114 | 1.7E-04 | 19 | 0.260 | 0.263 | 249 | 66.593 | 6773 | 4632 | 1645 | 3836 | 0.03% | 1.70 | 0.75 | 0.020% | 0.002010 | 0.34 | 0.079 | 0.797 | 0.32% | 0.0005 |
| 40.682 | 6936.75 | 40.1 | 1.406 | 40.1 | 2.7 | 1.16 | 3.51% | General Fill | 0.057 | 113.8 | 2.32 | 0.000 | 2.32 | 0 | 16 | 3.72% | 2.9 | 48% | 2.59 | 0.000 | 2.59 | 13.70 | 829 | 1.8E-03 | 1.2E+03 | 29 | 0.4070 | 1.7E-04 | 19 | 0.260 | 0.263 | 250 | 66.860 | 6763 | 4625 | 1646 | 3831 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001959 | 0.34 | 0.079 | 0.797 | 0.31% | 0.0005 |
| 40.846 | 6936.58 | 45.4 | 1.394 | 45.4 | 2.8 | 1.22 | 3.07% | General Fill | 0.057 | 113.8 | 2.33 | 0.000 | 2.33 | 0 | 18 | 3.24% | 2.8 | 48% | 2.60 | 0.000 | 2.60 | 13.75 | 829 | 1.8E-03 | 1.2E+03 | 28 | 0.4025 | 1.7E-04 | 19 | 0.260 | 0.264 | 251 | 67.127 | 6753 | 4618 | 1647 | 3826 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001909 | 0.34 | 0.079 | 0.797 | 0.30% | 0.0005 |
| 41.010 | 6936.42 | 43.1 | 1.454 | 43.1 | 2.6 | 1.12 | 3.37% | General Fill | 0.057 | 113.8 | 2.34 | 0.000 | 2.34 | 0 | 17 | 3.57% | 2.8 | 48% | 2.61 | 0.000 | 2.61 | 13.80 | 829 | 1.8E-03 | 1.2E+03 | 28 | 0.3980 | 1.7E-04 | 19 | 0.260 | 0.264 | 252 | 67.393 | 6743 | 4612 | 1648 | 3821 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001858 | 0.34 | 0.079 | 0.797 | 0.30% | 0.0005 |
| 41.174 | 6936.26 | 36.9 | 1.481 | 36.8 | 2.2 | 0.94 | 4.02% | General Fill | 0.057 | 113.8 | 2.35 | 0.000 | 2.35 | 0 | 15 | 4.29% | 3.0 | 48% | 2.62 | 0.000 | 2.62 | 13.85 | 829 | 1.8E-03 | 1.2E+03 | 28 | 0.3936 | 1.7E-04 | 19 | 0.260 | 0.264 | 253 | 67.660 | 6733 | 4605 | 1649 | 3817 | 0.03% | 1.70 | 0.75 | 0.020% | 0.001807 | 0.34 | 0.079 | 0.797 | 0.29% | 0.0005 |
| 41.338 | 6936.09 | 30.0 | 1.456 | 30.0 | 2.7 | 1.16 | 4.85% | General Fill | 0.057 | 11 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|-------|-------|-------|---------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 50.032 | 6927.40 | 13.1 | 0.334 | 11.8 | 204.5 | 88.62 | 2.56% | Fine Tailings | 0.054 | 107.6 | 2.84 | 0.173 | 2.67 | 1 | 4 | 3.27% | 3.4 | 83% | 3.10 | 0.173 | 2.93 | 16.55 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1833 | 1.6E-04 | 43 | 0.267 | 0.272 | 307 | 307.000 | 6424 | 4390 | 1703 | 1703 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 50.196 | 6927.23 | 13.9 | 0.358 | 12.7 | 200.5 | 86.87 | 2.57% | Fine Tailings | 0.054 | 107.6 | 2.85 | 0.178 | 2.67 | 1 | 4 | 3.23% | 3.3 | 83% | 3.11 | 0.178 | 2.94 | 16.60 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1802 | 1.6E-04 | 43 | 0.267 | 0.272 | 308 | 308.000 | 6421 | 4387 | 1704 | 1704 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 50.360 | 6927.07 | 14.2 | 0.369 | 12.9 | 209.8 | 90.90 | 2.59% | Fine Tailings | 0.054 | 107.6 | 2.86 | 0.183 | 2.68 | 1 | 4 | 3.24% | 3.3 | 83% | 3.12 | 0.183 | 2.94 | 16.65 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1770 | 1.6E-04 | 43 | 0.268 | 0.272 | 309 | 309.000 | 6417 | 4385 | 1705 | 1705 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 50.524 | 6926.91 | 14.3 | 0.368 | 13.0 | 213.5 | 92.51 | 2.57% | Fine Tailings | 0.054 | 107.6 | 2.87 | 0.188 | 2.68 | 1 | 4 | 3.21% | 3.3 | 83% | 3.13 | 0.188 | 2.94 | 16.70 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1739 | 1.6E-04 | 43 | 0.268 | 0.272 | 310 | 310.000 | 6414 | 4383 | 1706 | 1706 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 50.688 | 6926.74 | 13.6 | 0.372 | 12.3 | 210.7 | 91.31 | 2.73% | Fine Tailings | 0.054 | 107.6 | 2.88 | 0.193 | 2.68 | 1 | 4 | 3.46% | 3.4 | 83% | 3.14 | 0.193 | 2.95 | 16.75 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1709 | 1.5E-04 | 43 | 0.268 | 0.273 | 311 | 311.000 | 6411 | 4380 | 1707 | 1707 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 50.852 | 6926.58 | 13.5 | 0.365 | 12.2 | 205.0 | 88.85 | 2.70% | Fine Tailings | 0.054 | 107.6 | 2.88 | 0.198 | 2.69 | 1 | 4 | 3.43% | 3.4 | 83% | 3.15 | 0.198 | 2.95 | 16.80 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1678 | 1.5E-04 | 43 | 0.268 | 0.273 | 312 | 312.000 | 6407 | 4378 | 1708 | 1708 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.016 | 6926.41 | 13.8 | 0.358 | 12.4 | 215.9 | 93.54 | 2.60% | Fine Tailings | 0.054 | 107.6 | 2.89 | 0.203 | 2.69 | 1 | 4 | 3.29% | 3.3 | 83% | 3.16 | 0.203 | 2.95 | 16.85 | 640 | 1.7E-03 | 6.8E+02 | 21 | 0.1648 | 1.5E-04 | 43 | 0.268 | 0.273 | 313 | 313.000 | 6404 | 4376 | 1709 | 1709 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.180 | 6926.25 | 14.1 | 0.345 | 12.8 | 213.5 | 92.51 | 2.44% | Fine Tailings | 0.054 | 107.6 | 2.90 | 0.208 | 2.69 | 1 | 4 | 3.07% | 3.3 | 83% | 3.17 | 0.208 | 2.96 | 16.90 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1618 | 1.5E-04 | 43 | 0.268 | 0.273 | 314 | 314.000 | 6401 | 4373 | 1710 | 1710 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.345 | 6926.09 | 14.1 | 0.366 | 12.8 | 217.8 | 94.36 | 2.59% | Fine Tailings | 0.054 | 107.6 | 2.91 | 0.214 | 2.70 | 1 | 4 | 3.27% | 3.3 | 83% | 3.18 | 0.214 | 2.96 | 16.95 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1589 | 1.4E-04 | 43 | 0.268 | 0.273 | 315 | 315.000 | 6398 | 4371 | 1711 | 1711 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.509 | 6925.92 | 14.3 | 0.372 | 12.9 | 231.0 | ##### | 2.60% | Fine Tailings | 0.054 | 107.6 | 2.92 | 0.219 | 2.70 | 1 | 4 | 3.27% | 3.3 | 83% | 3.18 | 0.219 | 2.97 | 17.00 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1560 | 1.4E-04 | 43 | 0.268 | 0.273 | 316 | 316.000 | 6394 | 4369 | 1712 | 1712 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.673 | 6925.76 | 14.4 | 0.384 | 12.9 | 229.7 | 99.52 | 2.67% | Fine Tailings | 0.054 | 107.6 | 2.93 | 0.224 | 2.71 | 1 | 4 | 3.36% | 3.3 | 83% | 3.19 | 0.224 | 2.97 | 17.05 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1531 | 1.4E-04 | 43 | 0.268 | 0.273 | 317 | 317.000 | 6391 | 4367 | 1713 | 1713 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 51.837 | 6925.59 | 14.9 | 0.395 | 13.3 | 246.7 | ##### | 2.66% | Fine Tailings | 0.054 | 107.6 | 2.94 | 0.229 | 2.71 | 1 | 4 | 3.31% | 3.3 | 83% | 3.20 | 0.229 | 2.97 | 17.10 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1502 | 1.4E-04 | 43 | 0.268 | 0.273 | 318 | 318.000 | 6388 | 4364 | 1714 | 1714 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.001 | 6925.43 | 16.1 | 0.433 | 14.6 | 241.5 | ##### | 2.69% | Fine Tailings | 0.054 | 107.6 | 2.95 | 0.234 | 2.71 | 1 | 5 | 3.29% | 3.3 | 83% | 3.21 | 0.234 | 2.98 | 17.15 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1474 | 1.3E-04 | 43 | 0.268 | 0.273 | 319 | 319.000 | 6384 | 4362 | 1715 | 1715 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.165 | 6925.27 | 19.4 | 0.543 | 18.0 | 220.4 | 95.52 | 2.80% | Fine Tailings | 0.054 | 107.6 | 2.96 | 0.239 | 2.72 | 1 | 6 | 3.30% | 3.2 | 83% | 3.22 | 0.239 | 2.98 | 17.20 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1446 | 1.3E-04 | 43 | 0.268 | 0.273 | 320 | 320.000 | 6381 | 4360 | 1716 | 1716 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.329 | 6925.10 | 20.7 | 0.636 | 19.8 | 146.1 | 63.31 | 3.08% | Fine Tailings | 0.054 | 107.6 | 2.96 | 0.244 | 2.72 | 1 | 7 | 3.59% | 3.2 | 83% | 3.23 | 0.244 | 2.98 | 17.25 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1418 | 1.3E-04 | 43 | 0.268 | 0.273 | 321 | 321.000 | 6378 | 4357 | 1717 | 1717 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.493 | 6924.94 | 21.4 | 0.788 | 20.1 | 208.1 | 90.17 | 3.69% | Fine Tailings | 0.054 | 107.6 | 2.97 | 0.249 | 2.72 | 1 | 7 | 4.28% | 3.2 | 83% | 3.24 | 0.249 | 2.98 | 17.30 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1390 | 1.3E-04 | 43 | 0.269 | 0.274 | 322 | 322.000 | 6375 | 4355 | 1718 | 1718 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.657 | 6924.77 | 23.1 | 0.866 | 21.7 | 213.1 | 92.33 | 3.76% | Fine Tailings | 0.054 | 107.6 | 2.98 | 0.254 | 2.73 | 1 | 7 | 4.31% | 3.2 | 83% | 3.25 | 0.254 | 2.99 | 17.35 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1363 | 1.3E-04 | 43 | 0.269 | 0.274 | 323 | 323.000 | 6371 | 4353 | 1719 | 1719 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.821 | 6924.61 | 24.4 | 1.078 | 23.3 | 183.3 | 79.43 | 4.42% | Fine Tailings | 0.054 | 107.6 | 2.99 | 0.260 | 2.73 | 1 | 8 | 5.03% | 3.2 | 83% | 3.26 | 0.260 | 3.00 | 17.40 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1336 | 1.2E-04 | 43 | 0.269 | 0.274 | 324 | 324.000 | 6368 | 4351 | 1720 | 1720 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 52.985 | 6924.45 | 28.8 | 1.315 | 28.0 | 126.1 | 54.62 | 4.56% | Fine Tailings | 0.054 | 107.6 | 3.00 | 0.265 | 2.73 | 1 | 9 | 5.09% | 3.2 | 83% | 3.26 | 0.265 | 3.00 | 17.45 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1310 | 1.2E-04 | 43 | 0.269 | 0.274 | 325 | 325.000 | 6365 | 4348 | 1721 | 1721 | 0.02% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.149 | 6924.28 | 30.6 | 1.722 | 30.3 | 36.8 | 15.96 | 5.63% | Fine Tailings | 0.054 | 107.6 | 3.01 | 0.270 | 2.74 | 1 | 10 | 6.25% | 3.2 | 83% | 3.27 | 0.270 | 3.00 | 17.50 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1284 | 1.2E-04 | 43 | 0.269 | 0.274 | 326 | 326.000 | 6362 | 4346 | 1722 | 1722 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.313 | 6924.12 | 26.6 | 1.938 | 26.5 | 11.0 | 4.78 | 7.29% | Fine Tailings | 0.054 | 107.6 | 3.02 | 0.275 | 2.74 | 1 | 9 | 8.23% | 3.3 | 83% | 3.28 | 0.275 | 3.01 | 17.55 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1258 | 1.2E-04 | 43 | 0.269 | 0.274 | 327 | 327.000 | 6359 | 4344 | 1723 | 1723 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.477 | 6923.95 | 22.4 | 1.813 | 22.3 | 18.9 | 8.19 | 8.08% | Fine Tailings | 0.054 | 107.6 | 3.03 | 0.280 | 2.75 | 1 | 7 | 9.34% | 3.4 | 83% | 3.29 | 0.280 | 3.01 | 17.60 | 640 | 1.7E-03 | 6.8E+02 | 20 | 0.1232 | 1.2E-04 | 43 | 0.269 | 0.274 | 328 | 328.000 | 6355 | 4342 | 1724 | 1724 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.641 | 6923.79 | 20.1 | 1.307 | 19.8 | 48.4 | 20.96 | 6.50% | Fine Tailings | 0.054 | 107.6 | 3.04 | 0.285 | 2.75 | 1 | 6 | 7.65% | 3.4 | 83% | 3.30 | 0.285 | 3.01 | 17.65 | 640 | 1.7E-03 | 6.8E+02 | 19 | 0.1206 | 1.1E-04 | 43 | 0.269 | 0.274 | 329 | 329.000 | 6352 | 4339 | 1725 | 1725 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.805 | 6923.62 | 18.1 | 0.954 | 17.5 | 84.0 | 36.40 | 5.28% | Fine Tailings | 0.054 | 107.6 | 3.04 | 0.290 | 2.75 | 1 | 5 | 6.36% | 3.4 | 83% | 3.31 | 0.290 | 3.02 | 17.70 | 640 | 1.7E-03 | 6.8E+02 | 19 | 0.1181 | 1.1E-04 | 43 | 0.269 | 0.274 | 330 | 330.000 | 6349 | 4337 | 1726 | 1726 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 53.969 | 6923.46 | 17.7 | 0.885 | 17.4 | 40.9 | 17.71 | 5.01% | Fine Alluvium | 0.060 | 120.7 | 3.05 | 0.000 | 3.05 | 0 | 5 | 6.06% | 3.4 | 76% | 3.32 | 0.000 | 3.32 | 17.75 | 640 | 1.9E-03 | 7.7E+02 | 19 | 0.1157 | 9.7E-05 | 22 | 0.275 | 0.281 | 331 | 154.617 | 6107 | 4169 | 1727 | 3029 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 54.133 | 6923.30 | 18.0 | 1.099 | 17.7 | 42.0 | 18.18 | 6.11% | Fine Alluvium | 0.060 | 120.7 | 3.06 | 0.000 | 3.06 | 0 | 5 | 7.37% | 3.5 | 76% | 3.33 | 0.000 | 3.33 | 17.80 | 640 | 1.9E-03 | 7.7E+02 | 19 | 0.1132 | 9.6E-05 | 22 | 0.275 | 0.282 | 332 | 155.083 | 6099 | 4163 | 1728 | 3027 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 54.297 | 6923.13 | 27.8 | 1.519 | 27.3 | 77.6 | 33.61 | 5.46% | Fine Alluvium | 0.060 | 120.7 | 3.07 | 0.000 | 3.07 | 0 | 8 | 6.14% | 3.3 | 76% | 3.34 | 0.000 | 3.34 | 17.85 | 640 | 1.9E-03 | 7.7E+02 | 19 | 0.1108 | 9.4E-05 | 22 | 0.275 | 0.282 | 333 | 155.550 | 6092 | 4158 | 1729 | 3025 | 0.01% | 0.90 | 0.75 | 0.060% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 54.461 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|-------|-------|-------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|------|---------|---------|----|--------|---------|---|-------|-------|-----|-------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 63.155 | 6914.27 | 104.6 | 2.560 | 104.5 | 7.0 | 3.01 | 2.45% | Coarse Alluvium | 0.056 | 111.0 | 3.57 | 0.000 | 3.57 | 0 | 28 | 2.53% | 2.6 | 36% | 3.83 | 0.000 | 3.83 | 20.55 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0283 | 1.0E-05 | 0 | 0.284 | 0.292 | 387 | 0.284 | 5756 | 3925 | 1783 | 5756 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 63.319 | 6914.11 | 113.6 | 2.098 | 113.5 | 6.5 | 2.83 | 1.85% | Coarse Alluvium | 0.056 | 111.0 | 3.58 | 0.000 | 3.58 | 0 | 31 | 1.91% | 2.5 | 36% | 3.84 | 0.000 | 3.84 | 20.60 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0281 | 1.0E-05 | 0 | 0.285 | 0.292 | 388 | 0.285 | 5751 | 3921 | 1784 | 5751 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 63.483 | 6913.95 | 113.9 | 2.009 | 113.8 | 6.7 | 2.89 | 1.76% | Coarse Alluvium | 0.056 | 111.0 | 3.59 | 0.000 | 3.59 | 0 | 31 | 1.82% | 2.5 | 36% | 3.85 | 0.000 | 3.85 | 20.65 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0278 | 1.0E-05 | 0 | 0.285 | 0.293 | 389 | 0.285 | 5745 | 3917 | 1785 | 5745 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 63.648 | 6913.78 | 95.7 | 2.337 | 95.7 | 5.5 | 2.40 | 2.44% | Coarse Alluvium | 0.056 | 111.0 | 3.60 | 0.000 | 3.60 | 0 | 26 | 2.54% | 2.6 | 36% | 3.86 | 0.000 | 3.86 | 20.70 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0276 | 1.0E-05 | 0 | 0.285 | 0.293 | 390 | 0.285 | 5739 | 3913 | 1786 | 5739 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 63.812 | 6913.62 | 77.3 | 2.500 | 77.3 | 5.3 | 2.28 | 3.23% | Coarse Alluvium | 0.056 | 111.0 | 3.60 | 0.000 | 3.60 | 0 | 20 | 3.39% | 2.8 | 36% | 3.87 | 0.000 | 3.87 | 20.75 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0274 | 1.0E-05 | 0 | 0.285 | 0.293 | 391 | 0.285 | 5734 | 3909 | 1787 | 5734 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 63.976 | 6913.45 | 65.3 | 2.806 | 65.3 | 5.2 | 2.24 | 4.30% | Coarse Alluvium | 0.056 | 111.0 | 3.61 | 0.000 | 3.61 | 0 | 17 | 4.55% | 2.9 | 36% | 3.88 | 0.000 | 3.88 | 20.80 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0271 | 1.0E-05 | 0 | 0.285 | 0.293 | 392 | 0.285 | 5728 | 3905 | 1788 | 5728 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.140 | 6913.29 | 62.1 | 2.838 | 62.1 | 7.6 | 3.28 | 4.57% | Coarse Alluvium | 0.056 | 111.0 | 3.62 | 0.000 | 3.62 | 0 | 16 | 4.85% | 3.0 | 36% | 3.89 | 0.000 | 3.89 | 20.85 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0269 | 9.9E-06 | 0 | 0.285 | 0.293 | 393 | 0.285 | 5723 | 3902 | 1789 | 5723 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.304 | 6913.13 | 62.1 | 2.935 | 62.0 | 7.3 | 3.17 | 4.73% | Coarse Alluvium | 0.056 | 111.0 | 3.63 | 0.000 | 3.63 | 0 | 16 | 5.02% | 3.0 | 36% | 3.90 | 0.000 | 3.90 | 20.90 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0267 | 9.9E-06 | 0 | 0.286 | 0.294 | 394 | 0.286 | 5717 | 3898 | 1790 | 5717 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.468 | 6912.96 | 60.8 | 2.689 | 60.7 | 6.7 | 2.91 | 4.42% | Coarse Alluvium | 0.056 | 111.0 | 3.64 | 0.000 | 3.64 | 0 | 16 | 4.71% | 3.0 | 36% | 3.91 | 0.000 | 3.91 | 20.95 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0264 | 9.8E-06 | 0 | 0.286 | 0.294 | 395 | 0.286 | 5712 | 3894 | 1791 | 5712 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.632 | 6912.80 | 64.0 | 2.694 | 63.9 | 7.3 | 3.15 | 4.21% | Coarse Alluvium | 0.056 | 111.0 | 3.65 | 0.000 | 3.65 | 0 | 17 | 4.47% | 2.9 | 36% | 3.91 | 0.000 | 3.91 | 21.00 | 1091 | 1.7E-03 | 2.1E+03 | 17 | 0.0262 | 9.8E-06 | 0 | 0.286 | 0.294 | 396 | 0.286 | 5706 | 3890 | 1792 | 5706 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.796 | 6912.63 | 51.7 | 2.453 | 51.6 | 6.5 | 2.83 | 4.75% | Coarse Alluvium | 0.056 | 111.0 | 3.66 | 0.000 | 3.66 | 0 | 13 | 5.11% | 3.0 | 36% | 3.92 | 0.000 | 3.92 | 21.05 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0260 | 1.0E-05 | 0 | 0.286 | 0.294 | 397 | 0.286 | 5701 | 3886 | 1793 | 5701 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 64.960 | 6912.47 | 58.3 | 2.154 | 58.3 | 7.8 | 3.40 | 3.69% | Coarse Alluvium | 0.056 | 111.0 | 3.67 | 0.000 | 3.67 | 0 | 15 | 3.94% | 2.9 | 36% | 3.93 | 0.000 | 3.93 | 21.10 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0258 | 1.0E-05 | 0 | 0.286 | 0.294 | 398 | 0.286 | 5696 | 3883 | 1794 | 5696 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.124 | 6912.31 | 62.3 | 2.066 | 62.2 | 6.8 | 2.95 | 3.32% | Coarse Alluvium | 0.056 | 111.0 | 3.68 | 0.000 | 3.68 | 0 | 16 | 3.53% | 2.9 | 36% | 3.94 | 0.000 | 3.94 | 21.15 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0255 | 1.0E-05 | 0 | 0.286 | 0.294 | 399 | 0.286 | 5690 | 3879 | 1795 | 5690 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.288 | 6912.14 | 62.0 | 1.918 | 62.0 | 7.0 | 3.05 | 3.09% | Coarse Alluvium | 0.056 | 111.0 | 3.69 | 0.000 | 3.69 | 0 | 16 | 3.29% | 2.9 | 36% | 3.95 | 0.000 | 3.95 | 21.20 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0253 | 1.0E-05 | 0 | 0.286 | 0.295 | 400 | 0.286 | 5685 | 3875 | 1796 | 5685 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.452 | 6911.98 | 66.9 | 1.901 | 66.9 | 8.6 | 3.72 | 2.84% | Coarse Alluvium | 0.056 | 111.0 | 3.70 | 0.000 | 3.70 | 0 | 17 | 3.01% | 2.8 | 36% | 3.96 | 0.000 | 3.96 | 21.25 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0251 | 1.0E-05 | 0 | 0.287 | 0.295 | 401 | 0.287 | 5679 | 3871 | 1797 | 5679 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.616 | 6911.81 | 88.3 | 1.871 | 88.3 | 7.1 | 3.09 | 2.12% | Coarse Alluvium | 0.056 | 111.0 | 3.71 | 0.000 | 3.71 | 0 | 23 | 2.21% | 2.6 | 36% | 3.97 | 0.000 | 3.97 | 21.30 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0248 | 9.9E-06 | 0 | 0.287 | 0.295 | 402 | 0.287 | 5674 | 3868 | 1798 | 5674 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.780 | 6911.65 | 107.8 | 1.886 | 107.8 | 6.5 | 2.81 | 1.75% | Coarse Alluvium | 0.056 | 111.0 | 3.71 | 0.000 | 3.71 | 0 | 28 | 1.81% | 2.5 | 36% | 3.98 | 0.000 | 3.98 | 21.35 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0246 | 9.9E-06 | 0 | 0.287 | 0.295 | 403 | 0.287 | 5669 | 3864 | 1799 | 5669 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 65.944 | 6911.49 | 120.1 | 2.143 | 120.0 | 6.2 | 2.70 | 1.78% | Coarse Alluvium | 0.056 | 111.0 | 3.72 | 0.000 | 3.72 | 0 | 31 | 1.84% | 2.5 | 36% | 3.99 | 0.000 | 3.99 | 21.40 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0244 | 9.8E-06 | 0 | 0.287 | 0.295 | 404 | 0.287 | 5663 | 3860 | 1800 | 5663 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.108 | 6911.32 | 120.9 | 2.433 | 120.9 | 6.2 | 2.70 | 2.01% | Coarse Alluvium | 0.056 | 111.0 | 3.73 | 0.000 | 3.73 | 0 | 31 | 2.08% | 2.5 | 36% | 4.00 | 0.000 | 4.00 | 21.45 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0241 | 9.7E-06 | 0 | 0.287 | 0.296 | 405 | 0.287 | 5658 | 3857 | 1801 | 5658 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.272 | 6911.16 | 120.6 | 2.662 | 120.6 | 6.1 | 2.64 | 2.21% | Coarse Alluvium | 0.056 | 111.0 | 3.74 | 0.000 | 3.74 | 0 | 31 | 2.28% | 2.5 | 36% | 4.01 | 0.000 | 4.01 | 21.50 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0239 | 9.6E-06 | 0 | 0.287 | 0.296 | 406 | 0.287 | 5653 | 3853 | 1802 | 5653 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.436 | 6910.99 | 113.0 | 2.772 | 113.0 | 6.0 | 2.58 | 2.45% | Coarse Alluvium | 0.056 | 111.0 | 3.75 | 0.000 | 3.75 | 0 | 29 | 2.54% | 2.6 | 36% | 4.01 | 0.000 | 4.01 | 21.55 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0237 | 9.6E-06 | 0 | 0.287 | 0.296 | 407 | 0.287 | 5648 | 3849 | 1803 | 5648 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.600 | 6910.83 | 108.1 | 2.836 | 108.0 | 6.0 | 2.58 | 2.62% | Coarse Alluvium | 0.056 | 111.0 | 3.76 | 0.000 | 3.76 | 0 | 28 | 2.72% | 2.6 | 36% | 4.02 | 0.000 | 4.02 | 21.60 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0235 | 9.5E-06 | 0 | 0.288 | 0.296 | 408 | 0.288 | 5642 | 3846 | 1804 | 5642 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.764 | 6910.67 | 108.1 | 2.718 | 108.0 | 7.9 | 3.41 | 2.51% | Coarse Alluvium | 0.056 | 111.0 | 3.77 | 0.000 | 3.77 | 0 | 28 | 2.61% | 2.6 | 36% | 4.03 | 0.000 | 4.03 | 21.65 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0232 | 9.4E-06 | 0 | 0.288 | 0.296 | 409 | 0.288 | 5637 | 3842 | 1805 | 5637 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 66.928 | 6910.50 | 104.4 | 2.567 | 104.4 | 6.6 | 2.87 | 2.46% | Coarse Alluvium | 0.056 | 111.0 | 3.78 | 0.000 | 3.78 | 0 | 27 | 2.55% | 2.6 | 36% | 4.04 | 0.000 | 4.04 | 21.70 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0230 | 9.4E-06 | 0 | 0.288 | 0.296 | 410 | 0.288 | 5632 | 3838 | 1806 | 5632 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 67.092 | 6910.34 | 99.3 | 2.456 | 99.3 | 6.2 | 2.70 | 2.47% | Coarse Alluvium | 0.056 | 111.0 | 3.79 | 0.000 | 3.79 | 0 | 25 | 2.57% | 2.6 | 36% | 4.05 | 0.000 | 4.05 | 21.75 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0228 | 9.3E-06 | 0 | 0.288 | 0.297 | 411 | 0.288 | 5627 | 3835 | 1807 | 5627 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 67.256 | 6910.17 | 97.1 | 2.223 | 97.1 | 6.3 | 2.75 | 2.29% | Coarse Alluvium | 0.056 | 111.0 | 3.80 | 0.000 | 3.80 | 0 | 25 | 2.38% | 2.6 | 36% | 4.06 | 0.000 | 4.06 | 21.80 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0225 | 9.2E-06 | 0 | 0.288 | 0.297 | 412 | 0.288 | 5622 | 3831 | 1808 | 5622 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 67.420 | 6910.01 | 94.4 | 2.017 | 94.3 | 6.2 | 2.67 | 2.14% | Coarse Alluvium | 0.056 | 111.0 | 3.81 | 0.000 | 3.81 | 0 | 24 | 2.23% | 2.6 | 36% | 4.07 | 0.000 | 4.07 | 21.85 | 1060 | 1.7E-03 | 1.9E+03 | 17 | 0.0223 | 9.1E-06 | 0 | 0.288 | 0.297 | 413 | 0.288 | 5616 | 3828 | 1809 | 5616 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 67.584 | 6909.85 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|-------|-------|-------|-----|------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|------|---------|---------|----|--------|---------|---|-------|-------|-----|-------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 76.279 | 6901.15 | 81.5 | 1.347 | 81.5 | 0.8 | 0.35 | 1.65% | Coarse Alluvium | 0.056 | 111.0 | 4.30 | 0.000 | 4.30 | 0 | 18 | 1.74% | 2.7 | 36% | 4.56 | 0.000 | 4.56 | 24.55 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0099 | 5.1E-06 | 0 | 0.296 | 0.306 | 467 | 0.296 | 5360 | 3649 | 1863 | 5360 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 76.443 | 6900.99 | 83.7 | 1.312 | 83.7 | 1.1 | 0.49 | 1.57% | Coarse Alluvium | 0.056 | 111.0 | 4.31 | 0.000 | 4.31 | 0 | 18 | 1.65% | 2.6 | 36% | 4.57 | 0.000 | 4.57 | 24.60 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0097 | 4.9E-06 | 0 | 0.296 | 0.306 | 468 | 0.296 | 5355 | 3646 | 1864 | 5355 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 76.607 | 6900.82 | 86.8 | 1.384 | 86.8 | 2.2 | 0.96 | 1.59% | Coarse Alluvium | 0.056 | 111.0 | 4.32 | 0.000 | 4.32 | 0 | 19 | 1.68% | 2.6 | 36% | 4.58 | 0.000 | 4.58 | 24.65 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0094 | 4.8E-06 | 0 | 0.296 | 0.306 | 469 | 0.296 | 5351 | 3643 | 1865 | 5351 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 76.771 | 6900.66 | 102.4 | 1.512 | 102.4 | 2.0 | 0.85 | 1.48% | Coarse Alluvium | 0.056 | 111.0 | 4.32 | 0.000 | 4.32 | 0 | 23 | 1.54% | 2.5 | 36% | 4.59 | 0.000 | 4.59 | 24.70 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0092 | 4.7E-06 | 0 | 0.296 | 0.307 | 470 | 0.296 | 5347 | 3640 | 1866 | 5347 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 76.935 | 6900.50 | 113.5 | 0.957 | 113.5 | 1.5 | 0.63 | 0.84% | Coarse Alluvium | 0.056 | 111.0 | 4.33 | 0.000 | 4.33 | 0 | 25 | 0.88% | 2.4 | 36% | 4.60 | 0.000 | 4.60 | 24.75 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0090 | 4.6E-06 | 0 | 0.297 | 0.307 | 471 | 0.297 | 5342 | 3637 | 1867 | 5342 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.099 | 6900.33 | 111.7 | 1.361 | 111.7 | 1.2 | 0.53 | 1.22% | Coarse Alluvium | 0.056 | 111.0 | 4.34 | 0.000 | 4.34 | 0 | 25 | 1.27% | 2.5 | 36% | 4.61 | 0.000 | 4.61 | 24.80 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0087 | 4.5E-06 | 0 | 0.297 | 0.307 | 472 | 0.297 | 5338 | 3634 | 1868 | 5338 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.263 | 6900.17 | 106.8 | 1.506 | 106.8 | 4.4 | 1.91 | 1.41% | Coarse Alluvium | 0.056 | 111.0 | 4.35 | 0.000 | 4.35 | 0 | 24 | 1.47% | 2.5 | 36% | 4.62 | 0.000 | 4.62 | 24.85 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0085 | 4.4E-06 | 0 | 0.297 | 0.307 | 473 | 0.297 | 5334 | 3631 | 1869 | 5334 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.427 | 6900.00 | 86.2 | 1.982 | 86.2 | 0.4 | 0.16 | 2.30% | Coarse Alluvium | 0.056 | 111.0 | 4.36 | 0.000 | 4.36 | 0 | 19 | 2.42% | 2.7 | 36% | 4.63 | 0.000 | 4.63 | 24.90 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0083 | 4.3E-06 | 0 | 0.297 | 0.307 | 474 | 0.297 | 5329 | 3628 | 1870 | 5329 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.591 | 6899.84 | 88.7 | 1.678 | 88.7 | 2.9 | 1.26 | 1.89% | Coarse Alluvium | 0.056 | 111.0 | 4.37 | 0.000 | 4.37 | 0 | 19 | 1.99% | 2.7 | 36% | 4.63 | 0.000 | 4.63 | 24.95 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0080 | 4.2E-06 | 0 | 0.297 | 0.307 | 475 | 0.297 | 5325 | 3625 | 1871 | 5325 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.755 | 6899.68 | 120.4 | 1.831 | 120.3 | 4.2 | 1.83 | 1.52% | Coarse Alluvium | 0.056 | 111.0 | 4.38 | 0.000 | 4.38 | 0 | 26 | 1.58% | 2.5 | 36% | 4.64 | 0.000 | 4.64 | 25.00 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0078 | 4.1E-06 | 0 | 0.297 | 0.307 | 476 | 0.297 | 5321 | 3622 | 1872 | 5321 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 77.919 | 6899.51 | 145.7 | 2.244 | 145.6 | 2.1 | 0.89 | 1.54% | Coarse Alluvium | 0.056 | 111.0 | 4.39 | 0.000 | 4.39 | 0 | 32 | 1.59% | 2.4 | 36% | 4.65 | 0.000 | 4.65 | 25.05 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0076 | 4.0E-06 | 0 | 0.297 | 0.308 | 477 | 0.297 | 5316 | 3619 | 1873 | 5316 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.083 | 6899.35 | 137.9 | 2.233 | 137.9 | 1.0 | 0.43 | 1.62% | Coarse Alluvium | 0.056 | 111.0 | 4.40 | 0.000 | 4.40 | 0 | 30 | 1.67% | 2.5 | 36% | 4.66 | 0.000 | 4.66 | 25.10 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0074 | 3.8E-06 | 0 | 0.298 | 0.308 | 478 | 0.298 | 5312 | 3616 | 1874 | 5312 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.247 | 6899.18 | 122.6 | 2.131 | 122.6 | 2.5 | 1.08 | 1.74% | Coarse Alluvium | 0.056 | 111.0 | 4.41 | 0.000 | 4.41 | 0 | 27 | 1.80% | 2.5 | 36% | 4.67 | 0.000 | 4.67 | 25.15 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0071 | 3.7E-06 | 0 | 0.298 | 0.308 | 479 | 0.298 | 5308 | 3613 | 1875 | 5308 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.411 | 6899.02 | 112.8 | 1.876 | 112.8 | 0.5 | 0.23 | 1.66% | Coarse Alluvium | 0.056 | 111.0 | 4.42 | 0.000 | 4.42 | 0 | 25 | 1.73% | 2.5 | 36% | 4.68 | 0.000 | 4.68 | 25.20 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0069 | 3.6E-06 | 0 | 0.298 | 0.308 | 480 | 0.298 | 5304 | 3610 | 1876 | 5304 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.575 | 6898.85 | 108.9 | 1.646 | 108.9 | 1.6 | 0.71 | 1.51% | Coarse Alluvium | 0.056 | 111.0 | 4.42 | 0.000 | 4.42 | 0 | 24 | 1.58% | 2.5 | 36% | 4.69 | 0.000 | 4.69 | 25.25 | 1005 | 1.7E-03 | 1.7E+03 | 17 | 0.0067 | 3.5E-06 | 0 | 0.298 | 0.308 | 481 | 0.298 | 5299 | 3608 | 1877 | 5299 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.739 | 6898.69 | 104.2 | 1.479 | 104.2 | 1.1 | 0.67 | 1.42% | Coarse Alluvium | 0.056 | 111.0 | 4.43 | 0.000 | 4.43 | 0 | 23 | 1.48% | 2.5 | 36% | 4.70 | 0.000 | 4.70 | 25.30 | 1005 | 1.7E-03 | 1.7E+03 | 16 | 0.0064 | 3.4E-06 | 0 | 0.298 | 0.308 | 482 | 0.298 | 5295 | 3605 | 1878 | 5295 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 78.903 | 6898.53 | 107.9 | 1.467 | 107.9 | 1.1 | 0.49 | 1.36% | Coarse Alluvium | 0.056 | 111.0 | 4.44 | 0.000 | 4.44 | 0 | 23 | 1.42% | 2.5 | 36% | 4.71 | 0.000 | 4.71 | 25.35 | 1005 | 1.7E-03 | 1.7E+03 | 16 | 0.0062 | 3.3E-06 | 0 | 0.298 | 0.309 | 483 | 0.298 | 5291 | 3602 | 1879 | 5291 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.067 | 6898.36 | 116.8 | 1.667 | 116.8 | 1.4 | 0.59 | 1.43% | Coarse Alluvium | 0.056 | 111.0 | 4.45 | 0.000 | 4.45 | 0 | 25 | 1.48% | 2.5 | 36% | 4.72 | 0.000 | 4.72 | 25.40 | 1005 | 1.7E-03 | 1.7E+03 | 16 | 0.0060 | 3.2E-06 | 0 | 0.298 | 0.309 | 484 | 0.298 | 5287 | 3599 | 1880 | 5287 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.231 | 6898.20 | 140.5 | 1.836 | 140.5 | 1.2 | 0.51 | 1.31% | Coarse Alluvium | 0.056 | 111.0 | 4.46 | 0.000 | 4.46 | 0 | 30 | 1.35% | 2.4 | 36% | 4.73 | 0.000 | 4.73 | 25.45 | 1005 | 1.7E-03 | 1.7E+03 | 16 | 0.0057 | 3.0E-06 | 0 | 0.299 | 0.309 | 485 | 0.299 | 5283 | 3596 | 1881 | 5283 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.395 | 6898.03 | 140.6 | 1.890 | 140.6 | 1.4 | 0.59 | 1.34% | Coarse Alluvium | 0.056 | 111.0 | 4.47 | 0.000 | 4.47 | 0 | 30 | 1.39% | 2.4 | 36% | 4.73 | 0.000 | 4.73 | 25.50 | 1005 | 1.7E-03 | 1.7E+03 | 16 | 0.0055 | 2.9E-06 | 0 | 0.299 | 0.309 | 486 | 0.299 | 5278 | 3593 | 1882 | 5278 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.559 | 6897.87 | 161.2 | 1.585 | 161.2 | 3.3 | 1.44 | 0.98% | Coarse Alluvium | 0.056 | 111.0 | 4.48 | 0.000 | 4.48 | 0 | 35 | 1.01% | 2.3 | 36% | 4.74 | 0.000 | 4.74 | 25.55 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0053 | 1.9E-06 | 0 | 0.299 | 0.309 | 487 | 0.299 | 5274 | 3590 | 1883 | 5274 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.723 | 6897.71 | 172.6 | 1.841 | 172.5 | 2.7 | 1.18 | 1.07% | Coarse Alluvium | 0.056 | 111.0 | 4.49 | 0.000 | 4.49 | 0 | 37 | 1.10% | 2.3 | 36% | 4.75 | 0.000 | 4.75 | 25.60 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0051 | 1.8E-06 | 0 | 0.299 | 0.309 | 488 | 0.299 | 5270 | 3587 | 1884 | 5270 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 79.887 | 6897.54 | 164.9 | 2.077 | 164.9 | 4.3 | 1.85 | 1.26% | Coarse Alluvium | 0.056 | 111.0 | 4.50 | 0.000 | 4.50 | 0 | 36 | 1.29% | 2.3 | 36% | 4.76 | 0.000 | 4.76 | 25.65 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0048 | 1.7E-06 | 0 | 0.299 | 0.310 | 489 | 0.299 | 5266 | 3584 | 1885 | 5266 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 80.052 | 6897.38 | 128.8 | 1.981 | 128.8 | 1.4 | 0.59 | 1.54% | Coarse Alluvium | 0.056 | 111.0 | 4.51 | 0.000 | 4.51 | 0 | 28 | 1.59% | 2.5 | 36% | 4.77 | 0.000 | 4.77 | 25.70 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0046 | 1.6E-06 | 0 | 0.299 | 0.310 | 490 | 0.299 | 5262 | 3581 | 1886 | 5262 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 80.216 | 6897.21 | 193.6 | 3.060 | 193.6 | 2.7 | 1.16 | 1.58% | Coarse Alluvium | 0.056 | 111.0 | 4.52 | 0.000 | 4.52 | 0 | 42 | 1.62% | 2.3 | 36% | 4.78 | 0.000 | 4.78 | 25.75 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0044 | 1.5E-06 | 0 | 0.299 | 0.310 | 491 | 0.299 | 5258 | 3579 | 1887 | 5258 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 80.380 | 6897.05 | 234.3 | 3.869 | 234.2 | 3.9 | 1.71 | 1.65% | Coarse Alluvium | 0.056 | 111.0 | 4.52 | 0.000 | 4.52 | 0 | 51 | 1.68% | 2.3 | 36% | 4.79 | 0.000 | 4.79 | 25.80 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0041 | 1.5E-06 | 0 | 0.299 | 0.310 | 492 | 0.299 | 5254 | 3576 | 1888 | 5254 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 80.544 | 6896.89 | 232.5 | 3.947 | 232.5 | 3.4 | 1.46 | 1.70% | Coarse Alluvium | 0.056 | 111.0 | 4.53 | 0.000 | 4.53 | 0 | 50 | 1.73% | 2.3 | 36% | 4.80 | 0.000 | 4.80 | 25.85 | 1235 | 1.7E-03 | 2.6E+03 | 16 | 0.0039 | 1.4E-06 | 0 | 0.300 | 0.310 | 493 | 0.300 | 5250 | 3573 | 1889 | 5250 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|-------|-------|-------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|
| 89.402 | 6888.03 | 188.9 | 1.368 | 188.9 | -1.1 | -0.47 | 0.72% | Coarse Alluvium | 0.056 | 111.0 | 5.03 | 0.000 | 5.03 | 0 | 37 | 0.74% | 2.2 | 36% | 5.29 | 0.000 | 5.29 |
| 89.566 | 6887.86 | 188.1 | 1.479 | 188.1 | -1.3 | -0.55 | 0.79% | Coarse Alluvium | 0.056 | 111.0 | 5.03 | 0.000 | 5.03 | 0 | 36 | 0.81% | 2.2 | 36% | 5.30 | 0.000 | 5.30 |
| 89.730 | 6887.70 | 164.1 | 1.834 | 164.1 | -0.8 | -0.36 | 1.12% | Coarse Alluvium | 0.056 | 111.0 | 5.04 | 0.000 | 5.04 | 0 | 32 | 1.15% | 2.4 | 36% | 5.31 | 0.000 | 5.31 |
| 89.894 | 6887.54 | 147.7 | 1.329 | 147.7 | -1.5 | -0.65 | 0.90% | Coarse Alluvium | 0.056 | 111.0 | 5.05 | 0.000 | 5.05 | 0 | 28 | 0.93% | 2.3 | 36% | 5.32 | 0.000 | 5.32 |
| 90.058 | 6887.37 | 148.4 | 1.521 | 148.4 | -1.6 | -0.71 | 1.02% | Coarse Alluvium | 0.056 | 111.0 | 5.06 | 0.000 | 5.06 | 0 | 28 | 1.06% | 2.4 | 36% | 5.33 | 0.000 | 5.33 |
| 90.222 | 6887.21 | 181.2 | 1.550 | 181.2 | -1.7 | -0.75 | 0.86% | Coarse Alluvium | 0.056 | 111.0 | 5.07 | 0.000 | 5.07 | 0 | 35 | 0.88% | 2.3 | 36% | 5.34 | 0.000 | 5.34 |
| 90.386 | 6887.04 | 193.4 | 1.996 | 193.4 | -1.6 | -0.69 | 1.03% | Coarse Alluvium | 0.056 | 111.0 | 5.08 | 0.000 | 5.08 | 0 | 37 | 1.06% | 2.3 | 36% | 5.34 | 0.000 | 5.34 |
| 90.550 | 6886.88 | 200.4 | 1.665 | 200.4 | -2.5 | -1.08 | 0.83% | Coarse Alluvium | 0.056 | 111.0 | 5.09 | 0.000 | 5.09 | 0 | 38 | 0.85% | 2.2 | 36% | 5.35 | 0.000 | 5.35 |
| 90.714 | 6886.72 | 240.4 | 1.781 | 240.4 | -1.7 | -0.75 | 0.74% | Coarse Alluvium | 0.056 | 111.0 | 5.10 | 0.000 | 5.10 | 0 | 46 | 0.76% | 2.1 | 36% | 5.36 | 0.000 | 5.36 |
| 90.878 | 6886.55 | 240.0 | 2.697 | 240.0 | -1.7 | -0.73 | 1.12% | Coarse Alluvium | 0.056 | 111.0 | 5.11 | 0.000 | 5.11 | 0 | 46 | 1.15% | 2.2 | 36% | 5.37 | 0.000 | 5.37 |
| 91.042 | 6886.39 | 225.2 | 3.396 | 225.2 | -1.9 | -0.81 | 1.51% | Coarse Alluvium | 0.056 | 111.0 | 5.12 | 0.000 | 5.12 | 0 | 43 | 1.54% | 2.3 | 36% | 5.38 | 0.000 | 5.38 |
| 91.206 | 6886.22 | 213.7 | 3.313 | 213.7 | -1.5 | -0.65 | 1.55% | Coarse Alluvium | 0.056 | 111.0 | 5.13 | 0.000 | 5.13 | 0 | 41 | 1.59% | 2.3 | 36% | 5.39 | 0.000 | 5.39 |
| 91.370 | 6886.06 | 200.7 | 3.490 | 200.7 | -1.6 | -0.71 | 1.74% | Coarse Alluvium | 0.056 | 111.0 | 5.14 | 0.000 | 5.14 | 0 | 38 | 1.78% | 2.4 | 36% | 5.40 | 0.000 | 5.40 |
| 91.534 | 6885.90 | 123.1 | 3.538 | 123.1 | -2.7 | -1.16 | 2.87% | Coarse Alluvium | 0.056 | 111.0 | 5.14 | 0.000 | 5.14 | 0 | 23 | 3.00% | 2.7 | 36% | 5.41 | 0.000 | 5.41 |
| 91.698 | 6885.73 | 64.5 | 3.045 | 64.5 | -3.7 | -1.59 | 4.72% | Coarse Alluvium | 0.056 | 111.0 | 5.15 | 0.000 | 5.15 | 0 | 12 | 5.13% | 3.1 | 36% | 5.42 | 0.000 | 5.42 |
| 91.862 | 6885.57 | 47.7 | 2.189 | 47.7 | -3.3 | -1.44 | 4.59% | Coarse Alluvium | 0.056 | 111.0 | 5.16 | 0.000 | 5.16 | 0 | 8 | 5.14% | 3.2 | 36% | 5.43 | 0.000 | 5.43 |
| 92.026 | 6885.40 | 99.9 | 1.913 | 99.9 | 1.2 | 0.51 | 1.92% | Coarse Alluvium | 0.056 | 111.0 | 5.17 | 0.000 | 5.17 | 0 | 18 | 2.02% | 2.7 | 36% | 5.44 | 0.000 | 5.44 |
| 92.190 | 6885.24 | 142.8 | 1.768 | 142.8 | 1.3 | 0.55 | 1.24% | Coarse Alluvium | 0.056 | 111.0 | 5.18 | 0.000 | 5.18 | 0 | 27 | 1.28% | 2.4 | 36% | 5.44 | 0.000 | 5.44 |
| 92.355 | 6885.08 | 144.2 | 1.412 | 144.2 | 0.1 | 0.02 | 0.98% | Coarse Alluvium | 0.056 | 111.0 | 5.19 | 0.000 | 5.19 | 0 | 27 | 1.02% | 2.4 | 36% | 5.45 | 0.000 | 5.45 |
| 92.519 | 6884.91 | 142.1 | 0.308 | 142.1 | -0.7 | -0.30 | 0.22% | Coarse Alluvium | 0.056 | 111.0 | 5.20 | 0.000 | 5.20 | 0 | 26 | 0.23% | 2.1 | 36% | 5.46 | 0.000 | 5.46 |
| 92.683 | 6884.75 | 135.4 | 0.530 | 135.4 | -0.8 | -0.35 | 0.39% | Coarse Alluvium | 0.056 | 111.0 | 5.21 | 0.000 | 5.21 | 0 | 25 | 0.41% | 2.2 | 36% | 5.47 | 0.000 | 5.47 |
| 92.847 | 6884.58 | 138.6 | 0.653 | 138.5 | 8.5 | 3.66 | 0.47% | Coarse Alluvium | 0.056 | 111.0 | 5.22 | 0.000 | 5.22 | 0 | 26 | 0.49% | 2.3 | 36% | 5.48 | 0.000 | 5.48 |
| 93.011 | 6884.42 | 131.0 | 0.808 | 131.0 | 0.9 | 0.39 | 0.62% | Coarse Alluvium | 0.056 | 111.0 | 5.23 | 0.000 | 5.23 | 0 | 24 | 0.64% | 2.3 | 36% | 5.49 | 0.000 | 5.49 |
| 93.175 | 6884.26 | 122.0 | 0.867 | 122.0 | -0.6 | -0.26 | 0.71% | Coarse Alluvium | 0.056 | 111.0 | 5.24 | 0.000 | 5.24 | 0 | 22 | 0.74% | 2.4 | 36% | 5.50 | 0.000 | 5.50 |
| 93.339 | 6884.09 | 134.9 | 1.098 | 134.9 | -4.0 | -1.75 | 0.81% | Coarse Alluvium | 0.056 | 111.0 | 5.24 | 0.000 | 5.24 | 0 | 25 | 0.85% | 2.4 | 36% | 5.51 | 0.000 | 5.51 |
| 93.503 | 6883.93 | 163.2 | 1.641 | 163.2 | -3.4 | -1.46 | 1.01% | Coarse Alluvium | 0.056 | 111.0 | 5.25 | 0.000 | 5.25 | 0 | 30 | 1.04% | 2.3 | 36% | 5.52 | 0.000 | 5.52 |
| 93.667 | 6883.76 | 186.9 | 1.857 | 186.9 | -2.7 | -1.18 | 0.99% | Coarse Alluvium | 0.056 | 111.0 | 5.26 | 0.000 | 5.26 | 0 | 35 | 1.02% | 2.3 | 36% | 5.53 | 0.000 | 5.53 |
| 93.831 | 6883.60 | 193.0 | 2.073 | 193.0 | -2.5 | -1.08 | 1.07% | Coarse Alluvium | 0.056 | 111.0 | 5.27 | 0.000 | 5.27 | 0 | 36 | 1.10% | 2.3 | 36% | 5.54 | 0.000 | 5.54 |
| 93.995 | 6883.44 | 199.4 | 2.059 | 199.4 | -2.3 | -0.98 | 1.03% | Coarse Alluvium | 0.056 | 111.0 | 5.28 | 0.000 | 5.28 | 0 | 37 | 1.06% | 2.3 | 36% | 5.55 | 0.000 | 5.55 |
| 94.159 | 6883.27 | 204.2 | 2.008 | 204.2 | -2.5 | -1.08 | 0.98% | Coarse Alluvium | 0.056 | 111.0 | 5.29 | 0.000 | 5.29 | 0 | 38 | 1.01% | 2.3 | 36% | 5.55 | 0.000 | 5.55 |
| 94.323 | 6883.11 | 190.8 | 2.045 | 190.8 | -2.7 | -1.16 | 1.07% | Coarse Alluvium | 0.056 | 111.0 | 5.30 | 0.000 | 5.30 | 0 | 35 | 1.10% | 2.3 | 36% | 5.56 | 0.000 | 5.56 |
| 94.487 | 6882.94 | 178.7 | 2.495 | 178.7 | -2.6 | -1.14 | 1.40% | Coarse Alluvium | 0.056 | 111.0 | 5.31 | 0.000 | 5.31 | 0 | 33 | 1.44% | 2.4 | 36% | 5.57 | 0.000 | 5.57 |
| 94.651 | 6882.78 | 195.5 | 2.741 | 195.5 | -2.4 | -1.02 | 1.40% | Coarse Alluvium | 0.056 | 111.0 | 5.32 | 0.000 | 5.32 | 0 | 36 | 1.44% | 2.4 | 36% | 5.58 | 0.000 | 5.58 |

| | | | | | | | | | | | | | | | | | | | | | | | | | |
|-------|------|---------|---------|----|---------|-------|---|-------|-------|-----|-------|------|------|------|------|-------|------|------|--------|----------|------|-------|-------|-------|--------|
| 28.55 | 1016 | 1.7E+03 | 1.8E+03 | 16 | -0.0085 | ##### | 0 | 0.306 | 0.318 | 547 | 0.306 | 5044 | 3430 | 1943 | 5044 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.60 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0087 | ##### | 0 | 0.307 | 0.318 | 548 | 0.307 | 5040 | 3428 | 1944 | 5040 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.65 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0090 | ##### | 0 | 0.307 | 0.319 | 549 | 0.307 | 5037 | 3425 | 1945 | 5037 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.70 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0092 | ##### | 0 | 0.307 | 0.319 | 550 | 0.307 | 5033 | 3423 | 1946 | 5033 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.75 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0094 | ##### | 0 | 0.307 | 0.319 | 551 | 0.307 | 5030 | 3420 | 1947 | 5030 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.80 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0097 | ##### | 0 | 0.307 | 0.319 | 552 | 0.307 | 5026 | 3418 | 1948 | 5026 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.85 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0099 | ##### | 0 | 0.307 | 0.319 | 553 | 0.307 | 5022 | 3415 | 1949 | 5022 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.90 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0101 | ##### | 0 | 0.307 | 0.319 | 554 | 0.307 | 5019 | 3413 | 1950 | 5019 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 28.95 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0103 | ##### | 0 | 0.307 | 0.319 | 555 | 0.307 | 5015 | 3411 | 1951 | 5015 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.00 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0106 | ##### | 0 | 0.307 | 0.320 | 556 | 0.307 | 5012 | 3408 | 1952 | 5012 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.05 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0108 | ##### | 0 | 0.308 | 0.320 | 557 | 0.308 | 5009 | 3406 | 1953 | 5009 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.10 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0110 | ##### | 0 | 0.308 | 0.320 | 558 | 0.308 | 5005 | 3403 | 1954 | 5005 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.15 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0113 | ##### | 0 | 0.308 | 0.320 | 559 | 0.308 | 5002 | 3401 | 1955 | 5002 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.20 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0115 | ##### | 0 | 0.308 | 0.320 | 560 | 0.308 | 4998 | 3399 | 1956 | 4998 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.25 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0117 | ##### | 0 | 0.308 | 0.320 | 561 | 0.308 | 4995 | 3396 | 1957 | 4995 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.30 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0120 | ##### | 0 | 0.308 | 0.320 | 562 | 0.308 | 4991 | 3394 | 1958 | 4991 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.35 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0122 | ##### | 0 | 0.308 | 0.321 | 563 | 0.308 | 4988 | 3391 | 1959 | 4988 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.40 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0124 | ##### | 0 | 0.308 | 0.321 | 564 | 0.308 | 4984 | 3389 | 1960 | 4984 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.45 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0126 | ##### | 0 | 0.309 | 0.321 | 565 | 0.309 | 4981 | 3387 | 1961 | 4981 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.50 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0129 | ##### | 0 | 0.309 | 0.321 | 566 | 0.309 | 4978 | 3384 | 1962 | 4978 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.55 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0131 | ##### | 0 | 0.309 | 0.321 | 567 | 0.309 | 4974 | 3382 | 1963 | 4974 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.60 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0133 | ##### | 0 | 0.309 | 0.321 | 568 | 0.309 | 4971 | 3380 | 1964 | 4971 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.65 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0136 | ##### | 0 | 0.309 | 0.321 | 569 | 0.309 | 4967 | 3377 | 1965 | 4967 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.70 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0138 | ##### | 0 | 0.309 | 0.322 | 570 | 0.309 | 4964 | 3375 | 1966 | 4964 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.75 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0140 | ##### | 0 | 0.309 | 0.322 | 571 | 0.309 | 4961 | 3373 | 1967 | 4961 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.80 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0143 | ##### | 0 | 0.309 | 0.322 | 572 | 0.309 | 4957 | 3370 | 1968 | 4957 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.85 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0145 | ##### | 0 | 0.310 | 0.322 | 573 | 0.310 | 4954 | 3368 | 1969 | 4954 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.90 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0147 | ##### | 0 | 0.310 | 0.322 | 574 | 0.310 | 4951 | 3366 | 1970 | 4951 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 29.95 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0149 | ##### | 0 | 0.310 | 0.322 | 575 | 0.310 | 4947 | 3363 | 1971 | 4947 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 30.00 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0152 | ##### | 0 | 0.310 | 0.322 | 576 | 0.310 | 4944 | 3361 | 1972 | 4944 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 30.05 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0154 | ##### | 0 | 0.310 | 0.323 | 577 | 0.310 | 4941 | 3359 | 1973 | 4941 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 30.10 | 1637 | 1.7E+03 | 4.6E+03 | 16 | -0.0156 | ##### | 0 | 0.310 | 0.323 | 578 | 0.310 | 4937 | 3356 | 1974 | 4937 | 0.00% | 2.00 | 1.00 | 0.010% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 30.15 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |

| Proposed Repository | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer (tsf) | Total Stress at Midpoint of Layer (tsf) | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Effective Stress at Midpoint of Layer (tsf) |
|---------------------|----------------------------|---------------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------------|---|--|--|---|---|
| Erosion Protection | 6981.7 | 6981.0 | 6980.2 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6980.2 | 6978.7 | 6977.2 | 3.0 | 0.057 | 114.7 | 0.264 | 0.178 | 0.00 | 0.00 | 0.264 | 0.178 |
| Mine Spoils | 6977.2 | 6977.3 | 6977.4 | 0.0 | 0.058 | 116.4 | 0.264 | 0.264 | 0.00 | 0.00 | 0.264 | 0.264 |

| | |
|---------|--|
| 6977.43 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6981.71 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|--|
| 0.26 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{\text{repos}}$ (psf) |
| 6923.54 | Elevation of bottom of tailings (ft amsl) |

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|------|-----|---------|---------|-----|--------|---------|---|-------|-------|----|-------|-------|------|------|-------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 10.663 | 6965.98 | 33.1 | 0.190 | 33.1 | -1.3 | -0.57 | 0.57% | Coarse Tailings | 0.054 | 108.1 | 0.59 | 0.000 | 0.59 | 0 | 55 | 0.59% | 2.0 | 21% | 0.89 | 0.000 | 0.89 | 4.80 | 607 | 1.7E-03 | 6.2E+02 | 276 | 0.9499 | 2.7E-04 | 0 | 0.203 | 0.199 | 67 | 0.203 | 10472 | 7224 | 1463 | 10472 | 0.096% | 1.79 | 1.00 | 0.01% | 0.001532 | 0.36 | 0.025 | 0.785 | 0.24% | 0.0004 |
| 10.827 | 6965.81 | 32.8 | 0.193 | 32.8 | -1.3 | -0.57 | 0.59% | Coarse Tailings | 0.054 | 108.1 | 0.60 | 0.000 | 0.60 | 0 | 53 | 0.60% | 2.0 | 21% | 0.90 | 0.000 | 0.90 | 4.85 | 607 | 1.7E-03 | 6.2E+02 | 272 | 0.9488 | 2.7E-04 | 0 | 0.203 | 0.199 | 68 | 0.203 | 10429 | 7194 | 1464 | 10429 | 0.098% | 1.79 | 1.00 | 0.01% | 0.001567 | 0.36 | 0.025 | 0.785 | 0.25% | 0.0004 |
| 10.991 | 6965.65 | 32.4 | 0.189 | 32.4 | -1.5 | -0.63 | 0.58% | Coarse Tailings | 0.054 | 108.1 | 0.61 | 0.000 | 0.61 | 0 | 52 | 0.59% | 2.0 | 21% | 0.91 | 0.000 | 0.91 | 4.90 | 607 | 1.7E-03 | 6.2E+02 | 267 | 0.9478 | 2.7E-04 | 0 | 0.204 | 0.200 | 69 | 0.204 | 10387 | 7164 | 1465 | 10387 | 0.100% | 1.79 | 1.00 | 0.01% | 0.001603 | 0.36 | 0.025 | 0.785 | 0.25% | 0.0004 |
| 11.155 | 6965.49 | 31.7 | 0.178 | 31.7 | -1.9 | -0.83 | 0.56% | Coarse Tailings | 0.054 | 108.1 | 0.62 | 0.000 | 0.62 | 0 | 50 | 0.57% | 2.0 | 21% | 0.92 | 0.000 | 0.92 | 4.95 | 607 | 1.7E-03 | 6.2E+02 | 263 | 0.9468 | 2.7E-04 | 0 | 0.204 | 0.200 | 70 | 0.204 | 10346 | 7135 | 1466 | 10346 | 0.102% | 1.79 | 1.00 | 0.01% | 0.001639 | 0.36 | 0.025 | 0.785 | 0.26% | 0.0004 |
| 11.319 | 6965.32 | 31.0 | 0.176 | 31.0 | -1.6 | -0.71 | 0.57% | Coarse Tailings | 0.054 | 108.1 | 0.63 | 0.000 | 0.63 | 0 | 48 | 0.58% | 2.0 | 21% | 0.93 | 0.000 | 0.93 | 5.00 | 607 | 1.7E-03 | 6.2E+02 | 259 | 0.9457 | 2.8E-04 | 0 | 0.205 | 0.201 | 71 | 0.205 | 10305 | 7107 | 1467 | 10305 | 0.104% | 1.79 | 1.00 | 0.01% | 0.001675 | 0.36 | 0.025 | 0.785 | 0.26% | 0.0004 |
| 11.483 | 6965.16 | 30.8 | 0.169 | 30.8 | -1.8 | -0.77 | 0.55% | Coarse Tailings | 0.054 | 108.1 | 0.64 | 0.000 | 0.64 | 0 | 47 | 0.56% | 2.0 | 21% | 0.93 | 0.000 | 0.93 | 5.05 | 607 | 1.7E-03 | 6.2E+02 | 255 | 0.9446 | 2.8E-04 | 0 | 0.205 | 0.201 | 72 | 0.205 | 10265 | 7078 | 1468 | 10265 | 0.106% | 1.79 | 1.00 | 0.01% | 0.001712 | 0.36 | 0.025 | 0.785 | 0.27% | 0.0004 |
| 11.647 | 6964.99 | 30.2 | 0.170 | 30.2 | -1.6 | -0.67 | 0.56% | Coarse Tailings | 0.054 | 108.1 | 0.65 | 0.000 | 0.65 | 0 | 46 | 0.58% | 2.1 | 21% | 0.94 | 0.000 | 0.94 | 5.10 | 607 | 1.7E-03 | 6.2E+02 | 251 | 0.9435 | 2.8E-04 | 0 | 0.206 | 0.202 | 73 | 0.206 | 10225 | 7051 | 1469 | 10225 | 0.108% | 1.79 | 1.00 | 0.01% | 0.001750 | 0.36 | 0.025 | 0.785 | 0.27% | 0.0005 |
| 11.811 | 6964.83 | 30.5 | 0.175 | 30.5 | -1.4 | -0.61 | 0.57% | Coarse Tailings | 0.054 | 108.1 | 0.66 | 0.000 | 0.66 | 0 | 45 | 0.59% | 2.1 | 21% | 0.95 | 0.000 | 0.95 | 5.15 | 607 | 1.7E-03 | 6.2E+02 | 247 | 0.9424 | 2.8E-04 | 0 | 0.206 | 0.202 | 74 | 0.206 | 10186 | 7023 | 1470 | 10186 | 0.110% | 1.79 | 1.00 | 0.01% | 0.001787 | 0.36 | 0.025 | 0.785 | 0.28% | 0.0005 |
| 11.975 | 6964.67 | 29.2 | 0.173 | 29.2 | -1.8 | -0.79 | 0.59% | Coarse Tailings | 0.054 | 108.1 | 0.66 | 0.000 | 0.66 | 0 | 43 | 0.61% | 2.1 | 21% | 0.96 | 0.000 | 0.96 | 5.20 | 607 | 1.7E-03 | 6.2E+02 | 243 | 0.9413 | 2.9E-04 | 0 | 0.207 | 0.203 | 75 | 0.207 | 10148 | 6996 | 1471 | 10148 | 0.112% | 1.79 | 1.00 | 0.01% | 0.001825 | 0.36 | 0.025 | 0.785 | 0.29% | 0.0005 |
| 12.139 | 6964.50 | 26.9 | 0.168 | 27.0 | -2.0 | -0.88 | 0.62% | Coarse Tailings | 0.054 | 108.1 | 0.67 | 0.000 | 0.67 | 0 | 39 | 0.64% | 2.1 | 21% | 0.97 | 0.000 | 0.97 | 5.25 | 607 | 1.7E-03 | 6.2E+02 | 239 | 0.9401 | 2.9E-04 | 0 | 0.207 | 0.203 | 76 | 0.207 | 10109 | 6969 | 1472 | 10109 | 0.114% | 1.79 | 1.00 | 0.01% | 0.001863 | 0.36 | 0.025 | 0.785 | 0.29% | 0.0005 |
| 12.303 | 6964.34 | 23.4 | 0.280 | 23.4 | -1.2 | -0.51 | 1.20% | Coarse Tailings | 0.054 | 108.1 | 0.68 | 0.000 | 0.68 | 0 | 33 | 1.23% | 2.3 | 21% | 0.98 | 0.000 | 0.98 | 5.30 | 607 | 1.7E-03 | 6.2E+02 | 235 | 0.9390 | 2.9E-04 | 0 | 0.208 | 0.204 | 77 | 0.208 | 10072 | 6943 | 1473 | 10072 | 0.116% | 1.79 | 1.00 | 0.01% | 0.001902 | 0.36 | 0.025 | 0.785 | 0.30% | 0.0005 |
| 12.467 | 6964.17 | 16.2 | 0.310 | 16.3 | -0.7 | -0.30 | 1.91% | Coarse Tailings | 0.054 | 108.1 | 0.69 | 0.000 | 0.69 | 0 | 23 | 1.99% | 2.6 | 21% | 0.99 | 0.000 | 0.99 | 5.35 | 607 | 1.7E-03 | 6.2E+02 | 232 | 0.9378 | 2.9E-04 | 0 | 0.208 | 0.204 | 78 | 0.208 | 10035 | 6917 | 1474 | 10035 | 0.118% | 1.79 | 1.00 | 0.01% | 0.001941 | 0.36 | 0.025 | 0.785 | 0.30% | 0.0005 |
| 12.631 | 6964.01 | 12.4 | 0.349 | 12.4 | -0.7 | -0.29 | 2.81% | Coarse Tailings | 0.054 | 108.1 | 0.70 | 0.000 | 0.70 | 0 | 17 | 2.98% | 2.8 | 21% | 1.00 | 0.000 | 1.00 | 5.40 | 607 | 1.7E-03 | 6.2E+02 | 228 | 0.9366 | 2.9E-04 | 0 | 0.208 | 0.205 | 79 | 0.208 | 9998 | 6891 | 1475 | 9998 | 0.121% | 1.79 | 1.00 | 0.01% | 0.001980 | 0.36 | 0.025 | 0.785 | 0.31% | 0.0005 |
| 12.795 | 6963.84 | 15.2 | 0.291 | 15.2 | -1.0 | -0.45 | 1.91% | Coarse Tailings | 0.054 | 108.1 | 0.71 | 0.000 | 0.71 | 0 | 20 | 2.00% | 2.6 | 21% | 1.01 | 0.000 | 1.01 | 5.45 | 607 | 1.7E-03 | 6.2E+02 | 224 | 0.9354 | 3.0E-04 | 0 | 0.209 | 0.205 | 80 | 0.209 | 9962 | 6865 | 1476 | 9962 | 0.123% | 1.79 | 1.00 | 0.01% | 0.002019 | 0.36 | 0.025 | 0.785 | 0.32% | 0.0005 |
| 12.959 | 6963.68 | 20.6 | 0.228 | 20.6 | -0.9 | -0.39 | 1.11% | Coarse Tailings | 0.054 | 108.1 | 0.72 | 0.000 | 0.72 | 0 | 28 | 1.15% | 2.4 | 21% | 1.01 | 0.000 | 1.01 | 5.50 | 607 | 1.7E-03 | 6.2E+02 | 221 | 0.9341 | 3.0E-04 | 0 | 0.209 | 0.205 | 81 | 0.209 | 9926 | 6840 | 1477 | 9926 | 0.125% | 1.79 | 1.00 | 0.01% | 0.002059 | 0.36 | 0.025 | 0.785 | 0.32% | 0.0005 |
| 13.123 | 6963.52 | 24.8 | 0.134 | 24.8 | -0.8 | -0.35 | 0.54% | Coarse Tailings | 0.054 | 108.1 | 0.73 | 0.000 | 0.73 | 0 | 33 | 0.56% | 2.2 | 21% | 1.02 | 0.000 | 1.02 | 5.55 | 607 | 1.7E-03 | 6.2E+02 | 217 | 0.9329 | 3.0E-04 | 0 | 0.210 | 0.206 | 82 | 0.210 | 9891 | 6815 | 1478 | 9891 | 0.127% | 1.79 | 1.00 | 0.01% | 0.002099 | 0.36 | 0.025 | 0.785 | 0.33% | 0.0005 |
| 13.287 | 6963.35 | 26.3 | 0.145 | 26.3 | -1.2 | -0.51 | 0.55% | Coarse Tailings | 0.054 | 108.1 | 0.74 | 0.000 | 0.74 | 0 | 35 | 0.57% | 2.2 | 21% | 1.03 | 0.000 | 1.03 | 5.60 | 607 | 1.7E-03 | 6.2E+02 | 214 | 0.9316 | 3.0E-04 | 0 | 0.210 | 0.206 | 83 | 0.210 | 9856 | 6791 | 1479 | 9856 | 0.130% | 1.79 | 1.00 | 0.01% | 0.002139 | 0.36 | 0.025 | 0.785 | 0.34% | 0.0006 |
| 13.451 | 6963.19 | 22.3 | 0.254 | 22.3 | -1.2 | -0.53 | 1.14% | Coarse Tailings | 0.054 | 108.1 | 0.74 | 0.000 | 0.74 | 0 | 29 | 1.18% | 2.4 | 21% | 1.04 | 0.000 | 1.04 | 5.65 | 607 | 1.7E-03 | 6.2E+02 | 211 | 0.9303 | 3.1E-04 | 0 | 0.210 | 0.207 | 84 | 0.210 | 9821 | 6767 | 1480 | 9821 | 0.132% | 1.79 | 1.00 | 0.01% | 0.002179 | 0.36 | 0.025 | 0.785 | 0.34% | 0.0006 |
| 13.615 | 6963.02 | 19.6 | 0.294 | 19.6 | -0.5 | -0.23 | 1.50% | Coarse Tailings | 0.054 | 108.1 | 0.75 | 0.000 | 0.75 | 0 | 25 | 1.56% | 2.5 | 21% | 1.05 | 0.000 | 1.05 | 5.70 | 607 | 1.7E-03 | 6.2E+02 | 207 | 0.9290 | 3.1E-04 | 0 | 0.211 | 0.207 | 85 | 0.211 | 9787 | 6743 | 1481 | 9787 | 0.134% | 1.79 | 1.00 | 0.01% | 0.002220 | 0.36 | 0.025 | 0.785 | 0.35% | 0.0006 |
| 13.779 | 6962.86 | 25.8 | 0.270 | 25.8 | -0.7 | -0.29 | 1.05% | Coarse Tailings | 0.054 | 108.1 | 0.76 | 0.000 | 0.76 | 0 | 33 | 1.08% | 2.3 | 21% | 1.06 | 0.000 | 1.06 | 5.75 | 607 | 1.7E-03 | 6.2E+02 | 204 | 0.9277 | 3.1E-04 | 0 | 0.211 | 0.208 | 86 | 0.211 | 9753 | 6719 | 1482 | 9753 | 0.136% | 1.79 | 1.00 | 0.01% | 0.002261 | 0.36 | 0.025 | 0.785 | 0.35% | 0.0006 |
| 13.943 | 6962.70 | 27.2 | 0.156 | 27.2 | -0.6 | -0.26 | 0.57% | Coarse Tailings | 0.054 | 108.1 | 0.77 | 0.000 | 0.77 | 0 | 34 | 0.59% | 2.2 | 21% | 1.07 | 0.000 | 1.07 | 5.80 | 607 | 1.7E-03 | 6.2E+02 | 201 | 0.9263 | 3.1E-04 | 0 | 0.212 | 0.208 | 87 | 0.212 | 9720 | 6696 | 1483 | 9720 | 0.139% | 1.79 | 1.00 | 0.01% | 0.002302 | 0.36 | 0.025 | 0.785 | 0.36% | 0.0006 |
| 14.107 | 6962.53 | 28.2 | 0.119 | 28.2 | -1.2 | -0.51 | 0.42% | Coarse Tailings | 0.054 | 108.1 | 0.78 | 0.000 | 0.78 | 0 | 35 | 0.43% | 2.1 | 21% | 1.08 | 0.000 | 1.08 | 5.84 | 607 | 1.7E-03 | 6.2E+02 | 198 | 0.9250 | 3.1E-04 | 0 | 0.212 | 0.209 | 88 | 0.212 | 9687 | 6672 | 1484 | 9687 | 0.141% | 1.79 | 1.00 | 0.01% | 0.002343 | 0.36 | 0.025 | 0.785 | 0.37% | 0.0006 |
| 14.271 | 6962.37 | 28.9 | 0.121 | 28.9 | -0.5 | -0.23 | 0.42% | Coarse Tailings | 0.054 | 108.1 | 0.79 | 0.000 | 0.79 | 0 | 36 | 0.43% | 2.1 | 21% | 1.09 | 0.000 | 1.09 | 5.89 | 607 | 1.7E-03 | 6.2E+02 | 195 | 0.9236 | 3.2E-04 | 0 | 0.213 | 0.209 | 89 | 0.213 | 9655 | 6650 | 1485 | 9655 | 0.143% | 1.79 | 1.00 | 0.01% | 0.002384 | 0.36 | 0.025 | 0.785 | 0.37% | 0.0006 |
| 14.436 | 6962.20 | 30.1 | 0.133 | 30.1 | -0.7 | -0.29 | 0.44% | Coarse Tailings | 0.054 | 108.1 | 0.80 | 0.000 | 0.80 | 0 | 37 | 0.45% | 2.1 | 21% | 1.09 | 0.000 | 1.09 | 5.94 | 607 | 1.7E-03 | 6.2E+02 | 192 | 0.9222 | 3.2E-04 | 0 | 0.213 | 0.210 | 90 | 0.213 | 9622 | 6627 | 1486 | 9622 | 0.146% | 1.79 | 1.00 | 0.01% | 0.002425 | 0.36 | 0.025 | 0.785 | 0.38% | 0.0006 |
| 14.600 | 6962.04 | 30.0 | 0.143 | 30.0 | -0.6 | -0.26 | 0.48% | Coarse Tailings | 0.054 | 108.1 | 0.81 | 0.000 | 0.81 | 0 | 36 | 0.49% | 2.1 | 21% | 1.10 | 0.000 | 1.10 | 5.99 | 607 | 1.7E-03 | 6.2E+02 | 189 | 0.9207 | 3.2E-04 | 0 | 0.213 | 0.210 | 91 | 0.213 | 9591 | 6605 | 1487 | 9591 | 0.148% | 1.79 | 1.00 | 0.01% | 0.002467 | 0.36 | 0.025 | 0.785 | 0.39% | 0.0006 |
| 14.764 | 6961.88 | 29.6 | 0.161 | 29.6 | -0.9 | -0.41 | 0.54% | Coarse Tailings | 0.054 | 108.1 | 0.81 | 0.000 | 0.81 | 0 | 35 | 0.56% | 2.2 | 21% | 1.11 | 0.000 | 1.11 | 6.04 | 607 | 1.7E-03 | 6.2E+02 | 186 | 0.9193 | 3.2E-04 | 0 | 0.214 | 0.211 | 92 | 0.214 | 9559 | 6583 | 1488 | 9559 | 0.150% | 1.79 | 1.00 | 0.01% | 0.002508 | 0.36 | 0.025 | 0.785 | 0.39% | 0.0006 |
| 14.928 | 6961.71 | 27.3 | 0.174 | 27.3 | -1.1 | -0.47 | 0.64% | Coarse Tailings | 0.054 | 108.1 | 0.82 | 0.000 | 0.82 | 0 | 32 | 0.66% | 2.2 | 21% | 1.12 | 0.000 | 1.12 | 6.09 | 607 | 1.7E-03 | 6.2E+02 | 183 | 0.9178 | 3.2E-04 | 0 | 0.214 | 0.211 | 93 | 0.214 | 9528 | 6561 | 1489 | 9528 | 0.152% | 1.79 | 1.00 | 0.01% | 0.002550 | 0.36 | 0.025 | 0.785 | 0.40% | 0.0007 |
| 15.092 | 6961.55 | 22.9 | 0.199 | 22.9 | -1.4 | -0.59 | 0.87% | Coarse Tailings | 0.054 | 108 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|-----|---------|---------|----|--------|---------|---|-------|-------|-----|-------|------|------|------|------|---------|------|------|-------|----------|------|-------|-------|-------|--------|
| 23.786 | 6952.85 | 17.1 | 0.379 | 17.2 | -0.2 | -0.08 | 2.21% | Coarse Tailings | 0.054 | 108.1 | 1.30 | 0.000 | 1.30 | 0 | 12 | 2.39% | 2.9 | 21% | 1.60 | 0.000 | 1.60 | 8.79 | 616 | 1.7E-03 | 6.4E+02 | 83 | 0.8024 | 3.9E-04 | 0 | 0.232 | 0.232 | 147 | 0.232 | 8235 | 5654 | 1543 | 8235 | 0.2220% | 1.79 | 1.00 | 0.01% | 0.003766 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 23.950 | 6952.69 | 20.5 | 0.361 | 20.5 | 0.4 | 0.18 | 1.76% | Coarse Tailings | 0.054 | 108.1 | 1.31 | 0.000 | 1.31 | 0 | 15 | 1.88% | 2.7 | 21% | 1.61 | 0.000 | 1.61 | 8.84 | 616 | 1.7E-03 | 6.4E+02 | 82 | 0.7995 | 3.9E-04 | 0 | 0.233 | 0.232 | 148 | 0.233 | 8217 | 5641 | 1544 | 8217 | 0.2221% | 1.79 | 1.00 | 0.01% | 0.003773 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 24.114 | 6952.53 | 28.3 | 0.331 | 28.2 | 2.4 | 1.02 | 1.17% | Coarse Tailings | 0.054 | 108.1 | 1.32 | 0.000 | 1.32 | 0 | 20 | 1.23% | 2.5 | 21% | 1.62 | 0.000 | 1.62 | 8.89 | 616 | 1.7E-03 | 6.4E+02 | 80 | 0.7966 | 3.9E-04 | 0 | 0.233 | 0.233 | 149 | 0.233 | 8198 | 5628 | 1545 | 8198 | 0.221% | 1.79 | 1.00 | 0.01% | 0.003779 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 24.278 | 6952.36 | 32.1 | 0.263 | 32.1 | 2.1 | 0.91 | 0.82% | Coarse Tailings | 0.054 | 108.1 | 1.33 | 0.000 | 1.33 | 0 | 23 | 0.85% | 2.4 | 21% | 1.63 | 0.000 | 1.63 | 8.94 | 616 | 1.7E-03 | 6.4E+02 | 79 | 0.7937 | 4.0E-04 | 0 | 0.233 | 0.233 | 150 | 0.233 | 8180 | 5616 | 1546 | 8180 | 0.221% | 1.79 | 1.00 | 0.01% | 0.003784 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 24.442 | 6952.20 | 34.6 | 0.308 | 34.6 | 1.8 | 0.79 | 0.89% | Coarse Tailings | 0.054 | 108.1 | 1.34 | 0.000 | 1.34 | 0 | 25 | 0.93% | 2.4 | 21% | 1.64 | 0.000 | 1.64 | 8.99 | 616 | 1.7E-03 | 6.4E+02 | 78 | 0.7907 | 4.0E-04 | 0 | 0.234 | 0.233 | 151 | 0.234 | 8161 | 5603 | 1547 | 8161 | 0.222% | 1.79 | 1.00 | 0.01% | 0.003787 | 0.36 | 0.025 | 0.785 | 0.59% | 0.0010 |
| 24.606 | 6952.03 | 31.4 | 0.412 | 31.4 | 1.3 | 0.57 | 1.31% | Coarse Tailings | 0.054 | 108.1 | 1.35 | 0.000 | 1.35 | 0 | 22 | 1.37% | 2.5 | 21% | 1.64 | 0.000 | 1.64 | 9.04 | 636 | 1.7E-03 | 6.8E+02 | 77 | 0.7876 | 3.7E-04 | 0 | 0.234 | 0.234 | 152 | 0.234 | 8143 | 5590 | 1548 | 8143 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002968 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 24.770 | 6951.87 | 19.6 | 0.574 | 19.6 | 1.3 | 0.57 | 2.92% | Coarse Tailings | 0.054 | 108.1 | 1.36 | 0.000 | 1.36 | 0 | 13 | 3.14% | 2.9 | 21% | 1.65 | 0.000 | 1.65 | 9.09 | 636 | 1.7E-03 | 6.8E+02 | 76 | 0.7846 | 3.7E-04 | 0 | 0.234 | 0.234 | 153 | 0.234 | 8125 | 5578 | 1549 | 8125 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002969 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 24.934 | 6951.71 | 28.9 | 0.546 | 28.9 | 1.2 | 0.51 | 1.89% | Coarse Tailings | 0.054 | 108.1 | 1.36 | 0.000 | 1.36 | 0 | 20 | 1.98% | 2.6 | 21% | 1.66 | 0.000 | 1.66 | 9.14 | 636 | 1.7E-03 | 6.8E+02 | 75 | 0.7815 | 3.7E-04 | 0 | 0.235 | 0.234 | 154 | 0.235 | 8108 | 5565 | 1550 | 8108 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002970 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.098 | 6951.54 | 21.7 | 0.511 | 21.7 | 0.8 | 0.36 | 2.35% | Coarse Tailings | 0.054 | 108.1 | 1.37 | 0.000 | 1.37 | 0 | 15 | 2.51% | 2.8 | 21% | 1.67 | 0.000 | 1.67 | 9.19 | 636 | 1.7E-03 | 6.8E+02 | 74 | 0.7784 | 3.7E-04 | 0 | 0.235 | 0.235 | 155 | 0.235 | 8090 | 5553 | 1551 | 8090 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002970 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.262 | 6951.38 | 29.9 | 0.463 | 29.9 | 0.1 | 0.04 | 1.55% | Coarse Tailings | 0.054 | 108.1 | 1.38 | 0.000 | 1.38 | 0 | 21 | 1.62% | 2.6 | 21% | 1.68 | 0.000 | 1.68 | 9.24 | 636 | 1.7E-03 | 6.8E+02 | 73 | 0.7753 | 3.7E-04 | 0 | 0.235 | 0.235 | 156 | 0.235 | 8072 | 5541 | 1552 | 8072 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002969 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.426 | 6951.21 | 41.6 | 0.432 | 41.6 | 0.2 | 0.10 | 1.04% | Coarse Tailings | 0.054 | 108.1 | 1.39 | 0.000 | 1.39 | 0 | 29 | 1.08% | 2.4 | 21% | 1.69 | 0.000 | 1.69 | 9.29 | 636 | 1.7E-03 | 6.8E+02 | 72 | 0.7721 | 3.7E-04 | 0 | 0.235 | 0.235 | 157 | 0.235 | 8055 | 5528 | 1553 | 8055 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002968 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.590 | 6951.05 | 37.9 | 0.373 | 37.9 | 0.7 | 0.30 | 0.98% | Coarse Tailings | 0.054 | 108.1 | 1.40 | 0.000 | 1.40 | 0 | 26 | 1.02% | 2.4 | 21% | 1.70 | 0.000 | 1.70 | 9.34 | 636 | 1.7E-03 | 6.8E+02 | 71 | 0.7689 | 3.7E-04 | 0 | 0.236 | 0.236 | 158 | 0.236 | 8038 | 5516 | 1554 | 8038 | 0.176% | 1.79 | 1.00 | 0.01% | 0.002965 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.754 | 6950.89 | 35.0 | 0.602 | 35.0 | 0.0 | 0.00 | 1.72% | Coarse Tailings | 0.054 | 108.1 | 1.41 | 0.000 | 1.41 | 0 | 24 | 1.79% | 2.6 | 21% | 1.71 | 0.000 | 1.71 | 9.39 | 636 | 1.7E-03 | 6.8E+02 | 70 | 0.7657 | 3.8E-04 | 0 | 0.236 | 0.236 | 159 | 0.236 | 8021 | 5504 | 1555 | 8021 | 0.175% | 1.79 | 1.00 | 0.01% | 0.002962 | 0.36 | 0.025 | 0.785 | 0.47% | 0.0008 |
| 25.918 | 6950.72 | 28.9 | 0.617 | 28.9 | 0.5 | 0.23 | 2.14% | Coarse Tailings | 0.054 | 108.1 | 1.42 | 0.000 | 1.42 | 0 | 19 | 2.25% | 2.7 | 21% | 1.72 | 0.000 | 1.72 | 9.44 | 636 | 1.7E-03 | 6.8E+02 | 69 | 0.7624 | 3.8E-04 | 0 | 0.236 | 0.236 | 160 | 0.236 | 8004 | 5492 | 1556 | 8004 | 0.175% | 1.79 | 1.00 | 0.01% | 0.002958 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.082 | 6950.56 | 17.6 | 0.606 | 17.6 | 0.4 | 0.18 | 3.44% | Coarse Tailings | 0.054 | 108.1 | 1.43 | 0.000 | 1.43 | 0 | 11 | 3.75% | 3.0 | 21% | 1.72 | 0.000 | 1.72 | 9.49 | 636 | 1.7E-03 | 6.8E+02 | 69 | 0.7592 | 3.8E-04 | 0 | 0.236 | 0.236 | 161 | 0.236 | 7987 | 5481 | 1557 | 7987 | 0.175% | 1.79 | 1.00 | 0.01% | 0.002953 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.246 | 6950.39 | 32.7 | 0.775 | 32.7 | 0.8 | 0.35 | 2.37% | Coarse Tailings | 0.054 | 108.1 | 1.44 | 0.000 | 1.44 | 0 | 22 | 2.48% | 2.7 | 21% | 1.73 | 0.000 | 1.73 | 9.54 | 636 | 1.7E-03 | 6.8E+02 | 68 | 0.7558 | 3.8E-04 | 0 | 0.237 | 0.237 | 162 | 0.237 | 7970 | 5469 | 1558 | 7970 | 0.175% | 1.79 | 1.00 | 0.01% | 0.002947 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.410 | 6950.23 | 24.9 | 0.769 | 24.9 | 0.6 | 0.24 | 3.09% | Coarse Tailings | 0.054 | 108.1 | 1.44 | 0.000 | 1.44 | 0 | 16 | 3.28% | 2.8 | 21% | 1.74 | 0.000 | 1.74 | 9.59 | 636 | 1.7E-03 | 6.8E+02 | 67 | 0.7525 | 3.8E-04 | 0 | 0.237 | 0.237 | 163 | 0.237 | 7953 | 5457 | 1559 | 7953 | 0.174% | 1.79 | 1.00 | 0.01% | 0.002941 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.574 | 6950.07 | 22.2 | 0.776 | 22.2 | 0.4 | 0.16 | 3.50% | Coarse Tailings | 0.054 | 108.1 | 1.45 | 0.000 | 1.45 | 0 | 14 | 3.74% | 2.9 | 21% | 1.75 | 0.000 | 1.75 | 9.64 | 636 | 1.7E-03 | 6.8E+02 | 66 | 0.7491 | 3.8E-04 | 0 | 0.237 | 0.237 | 164 | 0.237 | 7937 | 5446 | 1560 | 7937 | 0.174% | 1.79 | 1.00 | 0.01% | 0.002933 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.739 | 6949.90 | 34.4 | 0.919 | 34.4 | 0.8 | 0.33 | 2.68% | Coarse Tailings | 0.054 | 108.1 | 1.46 | 0.000 | 1.46 | 0 | 22 | 2.79% | 2.7 | 21% | 1.76 | 0.000 | 1.76 | 9.69 | 636 | 1.7E-03 | 6.8E+02 | 65 | 0.7457 | 3.8E-04 | 0 | 0.238 | 0.238 | 165 | 0.238 | 7920 | 5434 | 1561 | 7920 | 0.173% | 1.79 | 1.00 | 0.01% | 0.002925 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 26.903 | 6949.74 | 21.7 | 0.997 | 21.7 | 0.8 | 0.35 | 4.59% | Coarse Tailings | 0.054 | 108.1 | 1.47 | 0.000 | 1.47 | 0 | 14 | 4.92% | 3.0 | 21% | 1.77 | 0.000 | 1.77 | 9.74 | 636 | 1.7E-03 | 6.8E+02 | 64 | 0.7423 | 3.8E-04 | 0 | 0.238 | 0.238 | 166 | 0.238 | 7904 | 5423 | 1562 | 7904 | 0.173% | 1.79 | 1.00 | 0.01% | 0.002916 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0008 |
| 27.067 | 6949.57 | 16.1 | 0.792 | 16.1 | 0.4 | 0.16 | 4.93% | Coarse Tailings | 0.054 | 108.1 | 1.48 | 0.000 | 1.48 | 0 | 10 | 5.43% | 3.2 | 21% | 1.78 | 0.000 | 1.78 | 9.79 | 636 | 1.7E-03 | 6.8E+02 | 63 | 0.7388 | 3.8E-04 | 0 | 0.238 | 0.238 | 167 | 0.238 | 7888 | 5411 | 1563 | 7888 | 0.172% | 1.79 | 1.00 | 0.01% | 0.002906 | 0.36 | 0.025 | 0.785 | 0.46% | 0.0007 |
| 27.231 | 6949.41 | 26.1 | 0.589 | 26.1 | 0.8 | 0.35 | 2.25% | Coarse Tailings | 0.054 | 108.1 | 1.49 | 0.000 | 1.49 | 0 | 17 | 2.39% | 2.8 | 21% | 1.79 | 0.000 | 1.79 | 9.84 | 636 | 1.7E-03 | 6.8E+02 | 63 | 0.7353 | 3.8E-04 | 0 | 0.238 | 0.239 | 168 | 0.238 | 7872 | 5400 | 1564 | 7872 | 0.172% | 1.79 | 1.00 | 0.01% | 0.002896 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 27.395 | 6949.25 | 45.0 | 0.692 | 45.0 | 1.9 | 0.81 | 1.54% | Coarse Tailings | 0.054 | 108.1 | 1.50 | 0.000 | 1.50 | 0 | 29 | 1.59% | 2.5 | 21% | 1.80 | 0.000 | 1.80 | 9.89 | 636 | 1.7E-03 | 6.8E+02 | 62 | 0.7318 | 3.8E-04 | 0 | 0.239 | 0.239 | 169 | 0.239 | 7856 | 5389 | 1565 | 7856 | 0.171% | 1.79 | 1.00 | 0.01% | 0.002885 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 27.559 | 6949.08 | 33.5 | 0.889 | 33.5 | 1.9 | 0.81 | 2.65% | Coarse Tailings | 0.054 | 108.1 | 1.51 | 0.000 | 1.51 | 0 | 21 | 2.78% | 2.7 | 21% | 1.80 | 0.000 | 1.80 | 9.94 | 636 | 1.7E-03 | 6.8E+02 | 61 | 0.7283 | 3.8E-04 | 0 | 0.239 | 0.239 | 170 | 0.239 | 7840 | 5378 | 1566 | 7840 | 0.170% | 1.79 | 1.00 | 0.01% | 0.002872 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 27.723 | 6948.92 | 26.5 | 0.744 | 26.5 | 1.9 | 0.81 | 2.81% | Coarse Tailings | 0.054 | 108.1 | 1.52 | 0.000 | 1.52 | 0 | 16 | 2.98% | 2.8 | 21% | 1.81 | 0.000 | 1.81 | 9.99 | 636 | 1.7E-03 | 6.8E+02 | 60 | 0.7247 | 3.8E-04 | 0 | 0.239 | 0.240 | 171 | 0.239 | 7824 | 5367 | 1567 | 7824 | 0.170% | 1.79 | 1.00 | 0.01% | 0.002860 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 27.887 | 6948.75 | 35.5 | 0.749 | 35.4 | 2.1 | 0.89 | 2.11% | Coarse Tailings | 0.054 | 108.1 | 1.52 | 0.000 | 1.52 | 0 | 22 | 2.21% | 2.6 | 21% | 1.82 | 0.000 | 1.82 | 10.04 | 636 | 1.7E-03 | 6.8E+02 | 60 | 0.7211 | 3.8E-04 | 0 | 0.240 | 0.240 | 172 | 0.240 | 7808 | 5356 | 1568 | 7808 | 0.169% | 1.79 | 1.00 | 0.01% | 0.002846 | 0.36 | 0.025 | 0.785 | 0.45% | 0.0007 |
| 28.051 | 6948.59 | 28.6 | 0.945 | 28.6 | 1.8 | 0.77 | 3.30% | Coarse Tailings | 0.054 | 108.1 | 1.53 | 0.000 | 1.53 | 0 | 18 | 3.49% | 2.8 | 21% | 1.83 | 0.000 | 1.83 | 10.09 | 636 | 1.7E-03 | 6.8E+02 | 59 | 0.7175 | 3.8E-04 | 0 | 0.240 | 0.240 | 173 | 0.240 | 7793 | 5345 | 1569 | 7793 | 0.168% | 1.79 | 1.00 | 0.01% | 0.002832 | 0.36 | 0.025 | 0.785 | 0.44% | 0.0007 |
| 28.215 | 6948.43 | 30.2 | 0.826 | 30.2 | 1.8 | 0.77 | 2.74% | Coarse Tailings | 0.054 | 108.1 | 1.54 | 0.00 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--------|---------|------|-------|------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|----|-------|-----|-----|------|-------|------|-------|------|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 36.909 | 6939.73 | 33.3 | 1.627 | 33.1 | 23.0 | 9.95 | 4.89% | Fine Alluvium | 0.060 | 120.7 | 2.06 | 0.000 | 2.06 | 0 | 15 | 5.21% | 3.0 | 76% | 2.35 | 0.000 | 2.35 | 12.79 | 1095 | 1.9E-03 | 2.2E+03 | 33 | 0.4895 | 1.0E-04 | 22 | 0.254 | 0.257 | 227 | 106.070 | 7031 | 4813 | 1623 | 3324 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.073 | 6939.57 | 41.6 | 1.831 | 41.4 | 23.0 | 9.95 | 4.40% | Fine Alluvium | 0.060 | 120.7 | 2.07 | 0.000 | 2.07 | 0 | 19 | 4.63% | 2.9 | 76% | 2.36 | 0.000 | 2.36 | 12.84 | 1095 | 1.9E-03 | 2.2E+03 | 33 | 0.4849 | 9.9E-05 | 22 | 0.254 | 0.257 | 228 | 106.537 | 7019 | 4804 | 1624 | 3320 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.237 | 6939.40 | 43.7 | 1.892 | 43.6 | 23.2 | 10.04 | 4.33% | Fine Alluvium | 0.060 | 120.7 | 2.08 | 0.000 | 2.08 | 0 | 20 | 4.54% | 2.9 | 76% | 2.37 | 0.000 | 2.37 | 12.89 | 1095 | 1.9E-03 | 2.2E+03 | 33 | 0.4803 | 9.9E-05 | 22 | 0.255 | 0.257 | 229 | 107.004 | 7007 | 4796 | 1625 | 3316 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.401 | 6939.24 | 38.4 | 1.935 | 38.2 | 19.9 | 8.64 | 5.04% | Fine Alluvium | 0.060 | 120.7 | 2.09 | 0.000 | 2.09 | 0 | 17 | 5.33% | 3.0 | 76% | 2.38 | 0.000 | 2.38 | 12.94 | 1095 | 1.9E-03 | 2.2E+03 | 32 | 0.4758 | 9.8E-05 | 22 | 0.255 | 0.258 | 230 | 107.471 | 6995 | 4788 | 1626 | 3312 | 0.014% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.565 | 6939.07 | 33.6 | 1.901 | 33.5 | 17.9 | 7.75 | 5.66% | Fine Alluvium | 0.060 | 120.7 | 2.09 | 0.000 | 2.09 | 0 | 15 | 6.04% | 3.0 | 76% | 2.39 | 0.000 | 2.39 | 12.99 | 1095 | 1.9E-03 | 2.2E+03 | 32 | 0.4712 | 9.8E-05 | 22 | 0.255 | 0.258 | 231 | 107.938 | 6983 | 4779 | 1627 | 3308 | 0.013% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.729 | 6938.91 | 43.8 | 1.261 | 43.7 | 18.1 | 7.83 | 2.88% | Fine Alluvium | 0.060 | 120.7 | 2.10 | 0.000 | 2.10 | 0 | 20 | 3.02% | 2.8 | 76% | 2.40 | 0.000 | 2.40 | 13.04 | 1095 | 1.9E-03 | 2.2E+03 | 32 | 0.4667 | 9.7E-05 | 22 | 0.255 | 0.258 | 232 | 108.404 | 6971 | 4771 | 1628 | 3304 | 0.013% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 37.893 | 6938.75 | 57.0 | 1.013 | 56.9 | 17.3 | 7.51 | 1.78% | Fine Alluvium | 0.060 | 120.7 | 2.11 | 0.000 | 2.11 | 0 | 26 | 1.84% | 2.5 | 76% | 2.41 | 0.000 | 2.41 | 13.09 | 1095 | 1.9E-03 | 2.2E+03 | 32 | 0.4621 | 9.7E-05 | 22 | 0.256 | 0.258 | 233 | 108.871 | 6960 | 4763 | 1629 | 3300 | 0.013% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 38.057 | 6938.58 | 70.7 | 1.333 | 70.7 | 3.4 | 1.46 | 1.89% | Coarse Alluvium | 0.056 | 111.0 | 2.12 | 0.000 | 2.12 | 0 | 32 | 1.94% | 2.5 | 36% | 2.42 | 0.000 | 2.42 | 13.14 | 1095 | 1.7E-03 | 2.1E+03 | 31 | 0.4575 | 1.0E-04 | 0 | 0.256 | 0.259 | 234 | 0.256 | 6949 | 4755 | 1630 | 6949 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000054 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 38.221 | 6938.42 | 72.4 | 1.485 | 72.3 | 3.7 | 1.59 | 2.05% | Coarse Alluvium | 0.056 | 111.0 | 2.13 | 0.000 | 2.13 | 0 | 33 | 2.11% | 2.5 | 36% | 2.43 | 0.000 | 2.43 | 13.19 | 1095 | 1.7E-03 | 2.1E+03 | 31 | 0.4530 | 1.0E-04 | 0 | 0.256 | 0.259 | 235 | 0.256 | 6938 | 4748 | 1631 | 6938 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000052 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 38.385 | 6938.25 | 75.2 | 1.598 | 75.2 | 4.0 | 1.73 | 2.13% | Coarse Alluvium | 0.056 | 111.0 | 2.14 | 0.000 | 2.14 | 0 | 34 | 2.19% | 2.5 | 36% | 2.44 | 0.000 | 2.44 | 13.24 | 1095 | 1.7E-03 | 2.1E+03 | 31 | 0.4484 | 1.0E-04 | 0 | 0.256 | 0.259 | 236 | 0.256 | 6928 | 4741 | 1632 | 6928 | 0.013% | 2.00 | 1.00 | 0.01% | 0.000050 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 38.549 | 6938.09 | 74.8 | 1.648 | 74.8 | 3.7 | 1.59 | 2.20% | Coarse Alluvium | 0.056 | 111.0 | 2.15 | 0.000 | 2.15 | 0 | 34 | 2.27% | 2.5 | 36% | 2.45 | 0.000 | 2.45 | 13.29 | 1095 | 1.7E-03 | 2.1E+03 | 31 | 0.4439 | 1.0E-04 | 0 | 0.256 | 0.260 | 237 | 0.256 | 6917 | 4733 | 1633 | 6917 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000048 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 38.713 | 6937.93 | 73.4 | 1.333 | 73.4 | 3.5 | 1.53 | 1.82% | Coarse Alluvium | 0.056 | 111.0 | 2.16 | 0.000 | 2.16 | 0 | 33 | 1.87% | 2.5 | 36% | 2.46 | 0.000 | 2.46 | 13.34 | 1095 | 1.7E-03 | 2.1E+03 | 30 | 0.4394 | 1.0E-04 | 0 | 0.257 | 0.260 | 238 | 0.257 | 6906 | 4726 | 1634 | 6906 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000046 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 38.877 | 6937.76 | 72.7 | 1.512 | 72.7 | 3.9 | 1.67 | 2.08% | Coarse Alluvium | 0.056 | 111.0 | 2.17 | 0.000 | 2.17 | 0 | 33 | 2.14% | 2.5 | 36% | 2.47 | 0.000 | 2.47 | 13.39 | 1095 | 1.7E-03 | 2.1E+03 | 30 | 0.4348 | 1.0E-04 | 0 | 0.257 | 0.260 | 239 | 0.257 | 6896 | 4719 | 1635 | 6896 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000044 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.042 | 6937.60 | 70.3 | 2.033 | 70.2 | 3.7 | 1.61 | 2.89% | Coarse Alluvium | 0.056 | 111.0 | 2.18 | 0.000 | 2.18 | 0 | 31 | 2.99% | 2.6 | 36% | 2.48 | 0.000 | 2.48 | 13.44 | 1095 | 1.7E-03 | 2.1E+03 | 30 | 0.4303 | 1.0E-04 | 0 | 0.257 | 0.260 | 240 | 0.257 | 6886 | 4711 | 1636 | 6886 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000042 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.206 | 6937.43 | 64.6 | 2.247 | 64.6 | 4.1 | 1.79 | 3.48% | Coarse Alluvium | 0.056 | 111.0 | 2.19 | 0.000 | 2.19 | 0 | 29 | 3.60% | 2.7 | 36% | 2.48 | 0.000 | 2.48 | 13.49 | 1095 | 1.7E-03 | 2.1E+03 | 30 | 0.4258 | 1.0E-04 | 0 | 0.257 | 0.261 | 241 | 0.257 | 6875 | 4704 | 1637 | 6875 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000040 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.370 | 6937.27 | 44.7 | 2.304 | 44.7 | 3.8 | 1.63 | 5.15% | Coarse Alluvium | 0.056 | 111.0 | 2.20 | 0.000 | 2.20 | 0 | 19 | 5.42% | 2.9 | 36% | 2.49 | 0.000 | 2.49 | 13.54 | 1095 | 1.7E-03 | 2.1E+03 | 29 | 0.4213 | 9.9E-05 | 0 | 0.258 | 0.261 | 242 | 0.258 | 6865 | 4697 | 1638 | 6865 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000038 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.534 | 6937.11 | 51.7 | 2.063 | 51.7 | 4.4 | 1.89 | 3.99% | Coarse Alluvium | 0.056 | 111.0 | 2.21 | 0.000 | 2.21 | 0 | 22 | 4.17% | 2.8 | 36% | 2.50 | 0.000 | 2.50 | 13.59 | 1092 | 1.7E-03 | 2.1E+03 | 29 | 0.4168 | 9.9E-05 | 0 | 0.258 | 0.261 | 243 | 0.258 | 6855 | 4690 | 1639 | 6855 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000037 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.698 | 6936.94 | 66.3 | 1.912 | 66.3 | 5.1 | 2.22 | 2.88% | Coarse Alluvium | 0.056 | 111.0 | 2.21 | 0.000 | 2.21 | 0 | 29 | 2.98% | 2.6 | 36% | 2.51 | 0.000 | 2.51 | 13.64 | 1092 | 1.7E-03 | 2.1E+03 | 29 | 0.4123 | 9.8E-05 | 0 | 0.258 | 0.261 | 244 | 0.258 | 6844 | 4683 | 1640 | 6844 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000035 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 39.862 | 6936.78 | 74.4 | 1.421 | 74.3 | 5.0 | 2.15 | 1.91% | Coarse Alluvium | 0.056 | 111.0 | 2.22 | 0.000 | 2.22 | 0 | 32 | 1.97% | 2.5 | 36% | 2.52 | 0.000 | 2.52 | 13.69 | 1092 | 1.7E-03 | 2.1E+03 | 29 | 0.4078 | 9.8E-05 | 0 | 0.258 | 0.262 | 245 | 0.258 | 6834 | 4675 | 1641 | 6834 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000033 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 40.026 | 6936.61 | 68.4 | 1.350 | 68.3 | 4.9 | 2.11 | 1.97% | Coarse Alluvium | 0.056 | 111.0 | 2.23 | 0.000 | 2.23 | 0 | 30 | 2.04% | 2.5 | 36% | 2.53 | 0.000 | 2.53 | 13.74 | 1092 | 1.7E-03 | 2.1E+03 | 29 | 0.4033 | 9.7E-05 | 0 | 0.258 | 0.262 | 246 | 0.258 | 6824 | 4668 | 1642 | 6824 | 0.012% | 2.00 | 1.00 | 0.01% | 0.000031 | 0.36 | 0.025 | 0.785 | 0.01% | 0.0000 |
| 40.190 | 6936.45 | 64.4 | 1.507 | 64.4 | 5.0 | 2.15 | 2.34% | Coarse Alluvium | 0.056 | 111.0 | 2.24 | 0.000 | 2.24 | 0 | 28 | 2.42% | 2.6 | 36% | 2.54 | 0.000 | 2.54 | 13.79 | 1092 | 1.7E-03 | 2.1E+03 | 28 | 0.3988 | 9.6E-05 | 0 | 0.259 | 0.262 | 247 | 0.259 | 6814 | 4661 | 1643 | 6814 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000029 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 40.354 | 6936.29 | 51.8 | 1.815 | 51.8 | 4.5 | 1.95 | 3.50% | Coarse Alluvium | 0.056 | 111.0 | 2.25 | 0.000 | 2.25 | 0 | 22 | 3.66% | 2.8 | 36% | 2.55 | 0.000 | 2.55 | 13.84 | 1092 | 1.7E-03 | 2.1E+03 | 28 | 0.3944 | 9.5E-05 | 0 | 0.259 | 0.262 | 248 | 0.259 | 6804 | 4654 | 1644 | 6804 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000026 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 40.518 | 6936.12 | 49.2 | 2.141 | 49.2 | 4.8 | 2.08 | 4.35% | Coarse Alluvium | 0.056 | 111.0 | 2.26 | 0.000 | 2.26 | 0 | 21 | 4.56% | 2.9 | 36% | 2.56 | 0.000 | 2.56 | 13.89 | 1092 | 1.7E-03 | 2.1E+03 | 28 | 0.3899 | 9.5E-05 | 0 | 0.259 | 0.263 | 249 | 0.259 | 6794 | 4648 | 1645 | 6794 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000024 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 40.682 | 6935.96 | 58.5 | 2.216 | 58.5 | 5.3 | 2.28 | 3.79% | Coarse Alluvium | 0.056 | 111.0 | 2.27 | 0.000 | 2.27 | 0 | 25 | 3.94% | 2.8 | 36% | 2.57 | 0.000 | 2.57 | 13.94 | 1092 | 1.7E-03 | 2.1E+03 | 28 | 0.3855 | 9.4E-05 | 0 | 0.259 | 0.263 | 250 | 0.259 | 6784 | 4641 | 1646 | 6784 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000022 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 40.846 | 6935.79 | 68.3 | 2.094 | 68.3 | 5.3 | 2.30 | 3.07% | Coarse Alluvium | 0.056 | 111.0 | 2.28 | 0.000 | 2.28 | 0 | 29 | 3.17% | 2.6 | 36% | 2.58 | 0.000 | 2.58 | 13.99 | 1092 | 1.7E-03 | 2.1E+03 | 28 | 0.3811 | 9.3E-05 | 0 | 0.259 | 0.263 | 251 | 0.259 | 6774 | 4634 | 1647 | 6774 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000020 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 41.010 | 6935.63 | 69.3 | 2.215 | 69.2 | 5.2 | 2.24 | 3.20% | Coarse Alluvium | 0.056 | 111.0 | 2.29 | 0.000 | 2.29 | 0 | 29 | 3.31% | 2.7 | 36% | 2.59 | 0.000 | 2.59 | 14.04 | 1092 | 1.7E-03 | 2.1E+03 | 27 | 0.3767 | 9.2E-05 | 0 | 0.260 | 0.263 | 252 | 0.260 | 6765 | 4627 | 1648 | 6765 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000018 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 41.174 | 6935.47 | 70.8 | 2.270 | 70.8 | 4.8 | 2.08 | 3.21% | Coarse Alluvium | 0.056 | 111.0 | 2.30 | 0.000 | 2.30 | 0 | 30 | 3.31% | 2.6 | 36% | 2.59 | 0.000 | 2.59 | 14.09 | 1092 | 1.7E-03 | 2.1E+03 | 27 | 0.3723 | 9.2E-05 | 0 | 0.260 | 0.264 | 253 | 0.260 | 6755 | 4620 | 1649 | 6755 | 0.011% | 2.00 | 1.00 | 0.01% | 0.000016 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 41.338 | 6935.30 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|-------|-------|-------|------|-------|-------|-----------------|-------|-------|------|-------|------|---|-----|-------|-----|-----|------|-------|------|--|
| 50.032 | 6926.61 | 96.1 | 1.817 | 96.1 | 15.0 | 6.49 | 1.89% | Fine Alluvium | 0.060 | 120.7 | 2.81 | 0.000 | 2.81 | 0 | 33 | 1.95% | 2.5 | 76% | 3.11 | 0.000 | 3.11 | |
| 50.196 | 6926.44 | 99.3 | 2.434 | 99.1 | 27.7 | 12.00 | 2.45% | Coarse Alluvium | 0.056 | 111.0 | 2.82 | 0.000 | 2.82 | 0 | 34 | 2.52% | 2.5 | 36% | 3.12 | 0.000 | 3.12 | |
| 50.360 | 6926.28 | 84.0 | 3.182 | 83.8 | 22.8 | 9.86 | 3.79% | Coarse Alluvium | 0.056 | 111.0 | 2.83 | 0.000 | 2.83 | 0 | 29 | 3.92% | 2.7 | 36% | 3.13 | 0.000 | 3.13 | |
| 50.524 | 6926.12 | 52.6 | 3.156 | 52.5 | 21.9 | 9.48 | 6.00% | Coarse Alluvium | 0.056 | 111.0 | 2.84 | 0.000 | 2.84 | 0 | 18 | 6.34% | 3.0 | 36% | 3.14 | 0.000 | 3.14 | |
| 50.688 | 6925.95 | 38.6 | 2.742 | 38.4 | 20.4 | 8.83 | 7.11% | Coarse Alluvium | 0.056 | 111.0 | 2.85 | 0.000 | 2.85 | 0 | 13 | 7.68% | 3.2 | 36% | 3.15 | 0.000 | 3.15 | |
| 50.852 | 6925.79 | 43.0 | 2.157 | 42.8 | 21.0 | 9.09 | 5.02% | Coarse Alluvium | 0.056 | 111.0 | 2.86 | 0.000 | 2.86 | 0 | 14 | 5.38% | 3.0 | 36% | 3.16 | 0.000 | 3.16 | |
| 51.016 | 6925.62 | 57.6 | 2.068 | 57.5 | 24.7 | 10.69 | 3.59% | Coarse Alluvium | 0.056 | 111.0 | 2.87 | 0.000 | 2.87 | 0 | 19 | 3.78% | 2.8 | 36% | 3.17 | 0.000 | 3.17 | |
| 51.180 | 6925.46 | 81.2 | 1.947 | 81.0 | 25.0 | 10.82 | 2.40% | Coarse Alluvium | 0.056 | 111.0 | 2.88 | 0.000 | 2.88 | 0 | 27 | 2.49% | 2.6 | 36% | 3.17 | 0.000 | 3.17 | |
| 51.345 | 6925.30 | 92.8 | 1.829 | 92.7 | 21.4 | 9.27 | 1.97% | Coarse Alluvium | 0.056 | 111.0 | 2.89 | 0.000 | 2.89 | 0 | 31 | 2.03% | 2.5 | 36% | 3.18 | 0.000 | 3.18 | |
| 51.509 | 6925.13 | 111.8 | 1.691 | 111.7 | 18.6 | 8.05 | 1.51% | Coarse Alluvium | 0.056 | 111.0 | 2.90 | 0.000 | 2.90 | 0 | 38 | 1.55% | 2.4 | 36% | 3.19 | 0.000 | 3.19 | |
| 51.673 | 6924.97 | 127.5 | 1.862 | 127.4 | 17.4 | 7.52 | 1.46% | Coarse Alluvium | 0.056 | 111.0 | 2.90 | 0.000 | 2.90 | 0 | 43 | 1.49% | 2.3 | 36% | 3.20 | 0.000 | 3.20 | |
| 51.837 | 6924.80 | 131.5 | 2.071 | 131.4 | 16.6 | 7.20 | 1.57% | Coarse Alluvium | 0.056 | 111.0 | 2.91 | 0.000 | 2.91 | 0 | 44 | 1.61% | 2.3 | 36% | 3.21 | 0.000 | 3.21 | |
| 52.001 | 6924.64 | 130.1 | 2.340 | 130.0 | 15.8 | 6.85 | 1.80% | Fine Alluvium | 0.060 | 120.7 | 2.92 | 0.000 | 2.92 | 0 | 43 | 1.84% | 2.4 | 76% | 3.22 | 0.000 | 3.22 | |
| 52.165 | 6924.48 | 124.5 | 2.653 | 124.4 | 14.4 | 6.22 | 2.13% | Fine Alluvium | 0.060 | 120.7 | 2.93 | 0.000 | 2.93 | 0 | 41 | 2.18% | 2.4 | 76% | 3.23 | 0.000 | 3.23 | |
| 52.329 | 6924.31 | 128.5 | 2.597 | 128.4 | 14.4 | 6.22 | 2.02% | Fine Alluvium | 0.060 | 120.7 | 2.94 | 0.000 | 2.94 | 0 | 43 | 2.07% | 2.4 | 76% | 3.24 | 0.000 | 3.24 | |
| 52.493 | 6924.15 | 128.9 | 2.368 | 128.8 | 14.9 | 6.47 | 1.84% | Fine Alluvium | 0.060 | 120.7 | 2.95 | 0.000 | 2.95 | 0 | 43 | 1.88% | 2.4 | 76% | 3.25 | 0.000 | 3.25 | |
| 52.657 | 6923.98 | 123.4 | 2.644 | 123.3 | 14.1 | 6.12 | 2.14% | Fine Alluvium | 0.060 | 120.7 | 2.96 | 0.000 | 2.96 | 0 | 41 | 2.20% | 2.4 | 76% | 3.26 | 0.000 | 3.26 | |
| 52.821 | 6923.82 | 120.1 | 2.625 | 120.0 | 15.1 | 6.53 | 2.19% | Fine Alluvium | 0.060 | 120.7 | 2.97 | 0.000 | 2.97 | 0 | 39 | 2.24% | 2.4 | 76% | 3.27 | 0.000 | 3.27 | |
| 52.985 | 6923.66 | 127.6 | 2.583 | 127.5 | 15.8 | 6.83 | 2.02% | Fine Alluvium | 0.060 | 120.7 | 2.98 | 0.000 | 2.98 | 0 | 42 | 2.07% | 2.4 | 76% | 3.28 | 0.000 | 3.28 | |
| 53.149 | 6923.49 | 133.7 | 2.313 | 133.6 | 14.8 | 6.43 | 1.73% | Fine Alluvium | 0.060 | 120.7 | 2.99 | 0.000 | 2.99 | 0 | 44 | 1.77% | 2.3 | 76% | 3.29 | 0.000 | 3.29 | |
| 53.313 | 6923.33 | 128.5 | 2.286 | 128.4 | 14.5 | 6.26 | 1.78% | Fine Alluvium | 0.060 | 120.7 | 3.00 | 0.000 | 3.00 | 0 | 42 | 1.82% | 2.4 | 76% | 3.30 | 0.000 | 3.30 | |
| 53.477 | 6923.16 | 117.4 | 2.912 | 117.3 | 13.5 | 5.86 | 2.48% | Fine Alluvium | 0.060 | 120.7 | 3.01 | 0.000 | 3.01 | 0 | 38 | 2.55% | 2.5 | 76% | 3.31 | 0.000 | 3.31 | |
| 53.641 | 6923.00 | 119.7 | 2.512 | 119.6 | 13.1 | 5.69 | 2.10% | Fine Alluvium | 0.060 | 120.7 | 3.02 | 0.000 | 3.02 | 0 | 39 | 2.15% | 2.4 | 76% | 3.32 | 0.000 | 3.32 | |
| 53.805 | 6922.83 | 134.1 | 2.551 | 134.0 | 15.6 | 6.77 | 1.90% | Fine Alluvium | 0.060 | 120.7 | 3.03 | 0.000 | 3.03 | 0 | 43 | 1.95% | 2.4 | 76% | 3.33 | 0.000 | 3.33 | |
| 53.969 | 6922.67 | 165.3 | 2.556 | 165.2 | 14.0 | 6.08 | 1.55% | Fine Alluvium | 0.060 | 120.7 | 3.04 | 0.000 | 3.04 | 0 | 53 | 1.58% | 2.2 | 76% | 3.34 | 0.000 | 3.34 | |
| 54.133 | 6922.51 | 184.4 | 2.581 | 184.3 | 14.6 | 6.32 | 1.40% | Fine Alluvium | 0.060 | 120.7 | 3.05 | 0.000 | 3.05 | 0 | 59 | 1.42% | 2.2 | 76% | 3.35 | 0.000 | 3.35 | |
| 54.297 | 6922.34 | 187.0 | 3.073 | 186.9 | 19.9 | 8.64 | 1.64% | Fine Alluvium | 0.060 | 120.7 | 3.06 | 0.000 | 3.06 | 0 | 60 | 1.67% | 2.2 | 76% | 3.36 | 0.000 | 3.36 | |
| 54.461 | 6922.18 | 194.5 | 3.149 | 194.4 | 17.1 | 7.42 | 1.62% | Fine Alluvium | 0.060 | 120.7 | 3.07 | 0.000 | 3.07 | 0 | 62 | 1.64% | 2.2 | 76% | 3.37 | 0.000 | 3.37 | |
| 54.625 | 6922.01 | 211.5 | 2.885 | 211.4 | 19.8 | 8.58 | 1.36% | Fine Alluvium | 0.060 | 120.7 | 3.08 | 0.000 | 3.08 | 0 | 68 | 1.38% | 2.1 | 76% | 3.38 | 0.000 | 3.38 | |
| 54.789 | 6921.85 | 216.9 | 4.198 | 216.8 | 18.3 | 7.93 | 1.94% | Fine Alluvium | 0.060 | 120.7 | 3.09 | 0.000 | 3.09 | 0 | 69 | 1.96% | 2.2 | 76% | 3.39 | 0.000 | 3.39 | |
| 54.953 | 6921.69 | 260.9 | 4.198 | 260.8 | 19.8 | 8.58 | 1.61% | Fine Alluvium | 0.060 | 120.7 | 3.10 | 0.000 | 3.10 | 0 | 83 | 1.63% | 2.1 | 76% | 3.40 | 0.000 | 3.40 | |
| 55.117 | 6921.52 | 390.0 | 4.198 | 389.8 | 32.8 | 14.21 | 1.08% | Fine Alluvium | 0.060 | 120.7 | 3.11 | 0.000 | 3.11 | 0 | 124 | 1.08% | 1.9 | 76% | 3.41 | 0.000 | 3.41 | |

| | | | | | | | | | | | | | | | | | | | | | | | | | |
|-------|------|---------|---------|----|--------|---------|----|-------|-------|-----|---------|------|------|------|------|--------|------|------|-------|----------|------|-------|-------|-------|--------|
| 16.79 | 1142 | 1.9E-03 | 2.4E+03 | 21 | 0.1684 | 4.2E-05 | 22 | 0.271 | 0.277 | 307 | 143.414 | 6270 | 4283 | 1703 | 3079 | 0.005% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 16.84 | 1142 | 1.7E-03 | 2.2E+03 | 21 | 0.1654 | 4.5E-05 | 0 | 0.271 | 0.277 | 308 | 0.271 | 6263 | 4277 | 1704 | 6263 | 0.005% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 16.89 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1624 | 4.4E-05 | 0 | 0.271 | 0.277 | 309 | 0.271 | 6255 | 4272 | 1705 | 6255 | 0.005% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 16.94 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1594 | 4.3E-05 | 0 | 0.272 | 0.277 | 310 | 0.272 | 6248 | 4267 | 1706 | 6248 | 0.005% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 16.99 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1565 | 4.3E-05 | 0 | 0.272 | 0.277 | 311 | 0.272 | 6241 | 4262 | 1707 | 6241 | 0.005% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.04 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1536 | 4.2E-05 | 0 | 0.272 | 0.278 | 312 | 0.272 | 6233 | 4257 | 1708 | 6233 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.09 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1507 | 4.1E-05 | 0 | 0.272 | 0.278 | 313 | 0.272 | 6226 | 4252 | 1709 | 6226 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.14 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1479 | 4.1E-05 | 0 | 0.272 | 0.278 | 314 | 0.272 | 6219 | 4246 | 1710 | 6219 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.19 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1451 | 4.0E-05 | 0 | 0.272 | 0.278 | 315 | 0.272 | 6211 | 4241 | 1711 | 6211 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.24 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1423 | 3.9E-05 | 0 | 0.273 | 0.278 | 316 | 0.273 | 6204 | 4236 | 1712 | 6204 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.29 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1395 | 3.9E-05 | 0 | 0.273 | 0.279 | 317 | 0.273 | 6197 | 4231 | 1713 | 6197 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.34 | 1142 | 1.7E-03 | 2.2E+03 | 20 | 0.1368 | 3.8E-05 | 0 | 0.273 | 0.279 | 318 | 0.273 | 6190 | 4226 | 1714 | 6190 | 0.004% | 2.00 | 1.00 | 0.01% | 0.000000 | 0.36 | 0.025 | 0.785 | 0.00% | 0.0000 |
| 17.39 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1341 | 3.4E-05 | 22 | 0.273 | 0.279 | 319 | 149.016 | 6182 | 4221 | 1715 | 3051 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.44 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1315 | 3.4E-05 | 22 | 0.273 | 0.279 | 320 | 149.482 | 6174 | 4215 | 1716 | 3049 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.49 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1288 | 3.3E-05 | 22 | 0.274 | 0.280 | 321 | 149.949 | 6166 | 4210 | 1717 | 3047 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.54 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1262 | 3.3E-05 | 22 | 0.274 | 0.280 | 322 | 150.416 | 6158 | 4205 | 1718 | 3044 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.59 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1237 | 3.2E-05 | 22 | 0.274 | 0.280 | 323 | 150.883 | 6151 | 4199 | 1719 | 3042 | 0.004% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.64 | 1142 | 1.9E-03 | 2.4E+03 | 20 | 0.1211 | 3.2E-05 | 22 | 0.274 | 0.280 | 324 | 151.349 | 6143 | 4194 | 1720 | 3039 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.69 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1186 | 3.1E-05 | 22 | 0.274 | 0.280 | 325 | 151.816 | 6136 | 4189 | 1721 | 3037 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.74 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1161 | 3.0E-05 | 22 | 0.275 | 0.281 | 326 | 152.283 | 6128 | 4183 | 1722 | 3035 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.79 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1136 | 3.0E-05 | 22 | 0.275 | 0.281 | 327 | 152.750 | 6120 | 4178 | 1723 | 3032 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.84 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1112 | 2.9E-05 | 22 | 0.275 | 0.281 | 328 | 153.217 | 6113 | 4173 | 1724 | 3030 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.89 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1088 | 2.9E-05 | 22 | 0.275 | 0.281 | 329 | 153.683 | 6105 | 4168 | 1725 | 3028 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.94 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1064 | 2.8E-05 | 22 | 0.275 | 0.282 | 330 | 154.150 | 6098 | 4163 | 1726 | 3025 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 17.99 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1041 | 2.8E-05 | 22 | 0.276 | 0.282 | 331 | 154.617 | 6091 | 4157 | 1727 | 3023 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 18.04 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.1018 | 2.7E-05 | 22 | 0.276 | 0.282 | 332 | 155.084 | 6083 | 4152 | 1728 | 3021 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 18.09 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.0995 | 2.7E-05 | 22 | 0.276 | 0.282 | 333 | 155.551 | 6076 | 4147 | 1729 | 3019 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 18.14 | 1142 | 1.9E-03 | 2.4E+03 | 19 | 0.0972 | 2.6E-05 | 22 | 0.276 | 0.282 | 334 | 156.017 | 6069 | 4142 | 1730 | 3016 | 0.003% | 0.90 | 0.75 | 0.06% | 0.000000 | 0.25 | 0.323 | 0.851 | 0.00% | 0.0000 |
| 18.19 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| 18.24 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| 18.29 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| 18.34 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |

| Proposed Repository | Elev. at Top of Layer (ft) | Elev. At Midpoint of Layer (ft) | Elev. At Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Total Stress at Bottom of Layer (tsf) | Total Stress at Midpoint of Layer (tsf) | Equil Pore Pressure at Bottom of Layer (tsf) | Equil Pore Pressure at Midpoint of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Effective Stress at Midpoint of Layer (tsf) |
|---------------------|----------------------------|---------------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------------|---|--|--|---|---|
| Erosion Protection | 6981.7 | 6981.0 | 6980.2 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| Cover Soil | 6980.2 | 6978.7 | 6977.2 | 3.0 | 0.057 | 114.7 | 0.264 | 0.178 | 0.00 | 0.00 | 0.264 | 0.178 |
| Mine Spoils | 6977.2 | 6976.9 | 6976.6 | 0.6 | 0.058 | 116.4 | 0.297 | 0.281 | 0.00 | 0.00 | 0.297 | 0.281 |

| | |
|---------|--|
| 6976.64 | Ground Surface Elevation at time of CPT (ft amsl) |
| 6981.71 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoils) Immediately after placement (ft) |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|--|
| 0.30 | Additional Stress due to Proposed Repository Construction, $\Delta\sigma_{\text{repos}}$ (psf) |
| 6946.54 | Elevation of bottom of tailings (ft amsl) |

ATTACHMENT G.5
Repository (Existing Radon Barrier) Cover Cracking Analysis

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Cover Cracking of the Existing Radon Barrier*

Sheet: 1 of 6
Job No: *10508639*

ATTACHMENT G.5: REPOSITORY (EXISTING RADON BARRIER) COVER CRACKING ANALYSIS

| Revisoning | | | | | |
|------------|------------|--------------------------|-----------|------------|----------|
| Rev. | Date | Description | By | Checked | Date |
| 0 | 06/02/2016 | Preliminary (30%) Design | S. Moore | J. Cumbers | 6/23/16 |
| 1 | July 2017 | 95% Design | S. Downey | M. Davis | 10/17/17 |
| 2 | Oct 2019 | Update Cover Thickness | S. Downey | M. Davis | 11/11/19 |

| Location and Format |
|---|
| <p>Electronic copies of these calculations are located on the Stantec internal project teamsite.</p> <p>The following calculations were generated using the following software:</p> <p style="text-align: center;">Microsoft Excel 2013</p> |

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| Conclusions | 5 |
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| Objective |
|---|
| <p>The objective of this calculation brief is to analyze the potential for cover cracking of the existing radon barrier, where the new repository cover transitions directly to the existing radon barrier.</p> |

Client: *GE/UNC*Sheet: 2 of 6Project: *Northeast Church Rock Mine Site Removal Action*Description: *Cover Cracking of the Existing Radon Barrier*Job No: *10508639*

Background

This analysis was performed as part of the design of the Removal Action (RA) at the Northeast Church Rock Mine Site (Mine Site) and the related Remedial Action (RA) at the Church Rock Mill Site (Mill Site). The Mine Site and Mill Site are located in close proximity to one another, approximately 16 miles northeast of Gallup, in McKinley County, New Mexico. They are located on adjacent Sections approximately one-half mile apart. The sites are temporarily being treated as one facility for purposes of the RA. The combined site is referred to as the "Settlement Agreement Site" (SA Site).

Site History

The NECR mine is a historical uranium mine operated by United Nuclear Corporation (UNC). Mining development began in 1967 and ended in 1982. While the mine operated, it served as the principal mineral source for the UNC uranium mill. The uranium mill and its adjacent disposal cells make up the UNC Superfund Site (the "UNC Mill Site"). Remedial activities addressing source control and on-site surface reclamation are being implemented by General Electric/United Nuclear Corporation (GE/UNC) under the direction of the U.S. Nuclear Regulatory Commission (NRC), pursuant to the UNC facility's NRC license, and integrated with the US Environmental Protection Agency's (USEPA's) selected remedy for the groundwater.

The tailings disposal area (TDA) is an unlined facility bounded by an embankment and subdivided by cross-dikes into three cells, which are identified as the South Cell, Central Cell, and North Cell. An estimated 3.5 million tons of tailings were pumped in slurry from the mill to the TDA.

Proposed Remedial Action

The proposed repository will be constructed on top of the existing TDA and will incorporate controlled placement of mine spoils on top of the existing TDA cover/radon barrier and a final evapotranspirative (ET) cover placed over the mine spoils. Improvements to the existing TDA cover/radon barrier within the footprint of the proposed repository will be completed prior to placement of mine spoils.

The design for the selected repository alternative will be evaluated as part of a NRC license amendment request for the existing licensed facility. The repository features that affect the licensed facility will meet the performance standards outlined in NRC regulations and areas of the existing facility affected by the repository construction will be evaluated for compliance. However, existing conditions of the facility not affected by the proposed repository were not evaluated as part of this analysis, as they are managed by the existing NRC license.

Site Description

The natural stratigraphy at the Mill Site is divided into two main components: the surficial unconsolidated deposits (alluvium) and the underlying consolidated bedrock units. The alluvium consists of a mixture of sand, silt, and clay with minor portions of gravel. Alluvial thicknesses at the site are usually around 50 feet, but exceed 120 feet in some locations. Generally, the uppermost bedrock unit at the site is the Upper Gallup Sandstone, though in some locations it is overlain by coal or the Mancos shale.

The TDA was constructed on top of the native alluvium and deposition of tailings via slurry within the TDA has resulted in an interbedded accumulation of tailings. TDA closure construction began in 1989 and was completed in 1995. Closure construction included placement of an interim cover (general fill) from 1989 through 1991 followed by placement of the final cover (radon barrier and erosion protection layer) from 1993 through 1995.

Client: *GE/UNC*
Sheet: 3 **of** 6
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Cover Cracking of the Existing Radon Barrier*
Job No: *10508639*

Site Investigation

In 2013, MWH performed pre-design studies (PDS) at the Mill Site and Mine Site to supplement previous site investigations and collect pre-design data necessary to perform the Removal Design (RD). Activities performed as part of the Mill Site PDS included: surveying, cone penetration tests (CPTs), drilling, standard penetration tests (SPTs), excavation and soil sampling, and subsequent laboratory testing. Geotechnical data collected during the PDS are presented in the PDS reports (MWH, 2014a and MWH, 2014b).

Results of the settlement analyses and seismic settlement analysis (Attachments G.3 and G.4 of Appendix G) were used for the cover cracking analysis. The potential for cover cracking was analyzed at three critical locations, CPT-01, CPT-15, and CPT-26, shown on **Figure 1**, located where the repository cover transitions directly to the TDA cover.

Applicable Codes and Standards

NUREG 1620 (NRC, 2003)

Methods

Results of the immediate, primary, and secondary consolidation, as well as the seismic settlement, were used to evaluate the potential for differential settlement and cover cracking. Differential settlement was determined by the difference between the estimated total potential future settlement at each of three locations and the settlement at the nearest point on the edge of the repository fill (assumed to be zero). The slope reduction at each location was calculated from the maximum differential settlement divided by the shortest distance between the point and edge of cover. The addition of minor amounts of grading fill (up to 3 feet) on the west and southwest sides of the cover may affect these selected locations, however the differential settlement results will not change significantly.

The evaluation of potential for cover cracking used the critical location determined in the differential settlement analysis. The method presented in Morrison-Knudsen Environmental Corporation (1993) was used to determine the tensile strain required to cause cracking of the radon barrier as a function of the plasticity index (PI) of the soil. The PI was estimated as the average of the measured PIs of radon barrier samples collected during the PDS (MWH, 2014a). The horizontal movement at the top of the radon barrier was calculated based on the method presented in Lee and Shen (1969). The peak horizontal movement is assumed to be twice the average horizontal movement based on relationships presented in Gourc et al. (2010) and Rajesh and Viswanadham (2010). The horizontal strain was calculated using the horizontal distance between the point and edge of cover. The calculated strain was compared to the maximum allowable strain to determine the potential for cover cracking.

Assumptions

Three critical locations were selected for the cover cracking analysis, CPT-01, CPT-15, and CPT-26. The greatest potential for detrimental cracking of the radon barrier is in the area where the radon barrier remains in its current configuration (i.e., at the edge of fill to be placed) and the fill differential adjacent to it is the highest. These locations were selected as critical because they are nearest to the repository slope where the final cover will transition directly to the existing radon barrier, and has the potential to cause cover cracking in the existing radon barrier. In the remaining area of the repository, differential settlements will be less since the fill will completely cover the existing radon barrier. The radon barrier will be recompacted and, therefore, this analysis only considers how the radon barrier performs after recompaction then loading. The critical locations used in the cover cracking analysis are shown on **Figure 1**.

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Cover Cracking of the Existing Radon Barrier*

Sheet: 4 **of** 6
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Differential Settlement

The critical time period where differential settlement is a concern for the cover grading is after active placement of waste and the final cover is complete. Potential maximum future settlement after active placement of waste and the final cover includes immediate, primary and secondary consolidation as well as seismic settlement and is estimated to be 0.26 to 1.17 feet at the three locations evaluated (CPT-01, CPT-15, and CPT-26) (**Attachment A**). The horizontal distances range from 80 to 210 feet from each of these three points to the edge of the cover.

Calculations

Differential Settlement

The total potential differential settlement for the three locations were approximated using the estimated total settlement on the cover and assuming zero settlement at the edge of the repository. The maximum differential settlement in feet (equal to the estimated total settlement at each location) was divided by the distance in feet between the respective location and edge of the cover to obtain the slope reduction in percent.

Cover Cracking

Morrison-Knudsen Environmental Corporation (1993) presents a method for determining the tensile strain required to cause cracking of the radon barrier as a function of the PI of the soil. The tensile strain at cracking is calculated by the equation below:

$$\varepsilon_f(\%) = 0.05 + 0.003 * (PI)$$

where:

$\varepsilon_f(\%)$ = tensile strain to cause cracking of the radon barrier

PI = plasticity index of radon barrier

The PI value for the compacted radon attenuation layer was estimated as 16 percent, calculated as the average of the measured PIs of 10 radon barrier samples collected during the PDS (MWH, 2014a). The PI values ranged from 14 to 20, with a median value of 15.5. The average value was used as it is representative of the radon barrier material. Using this value for PI, the minimum tensile strain that will induce cracking is 0.10 percent. The maximum settlement-induced horizontal tensile strain on the radon attenuation layer must be less than 0.10 percent to prevent cover cracking.

The horizontal movement at the top of the radon barrier can be calculated based on the following equation (Lee and Shen, 1969), which is referenced in NUREG 1620 (NRC, 2003) for cover cracking analysis:

$$m = \frac{2}{3} H \alpha$$

where:

m = horizontal movement in feet

H = thickness of relatively incompressible material (in this analysis H is the thickness of the radon barrier)

α = local slope of the settlement profile (expressed as decimal fraction)

Horizontal movement at the maximum tailing thickness is calculated using a maximum thickness of relatively incompressible material of 1.8 feet (21 inches), and the total differential settlement. The thickness of relatively incompressible material was estimated assuming a maximum of 21 inches for the radon barrier, based on the As-Built Reports (Canonie, 1994, 1995). The as-built reports by Canonie do not include details on specific depths of the radon barrier at specific locations. The cover test pits conducted during the PDS were intentionally terminated prior to reaching the bottom of the radon barrier and were therefore not used to estimate the radon barrier thickness. By assuming the greatest thickness of radon barrier documented by Canonie (21 inches), the cover cracking results are conservative. The peak horizontal movement is assumed to be twice the average horizontal movement based on relationships presented in Gourc et al. (2010) and Rajesh and Viswanadham (2010).

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Cover Cracking of the Existing Radon Barrier*

Sheet: 5 **of** 6
Job No: *10508639*

The horizontal strain between any two settlement locations is the maximum horizontal movement divided by the horizontal distance. This value is then compared with the maximum allowable strain of 0.10 percent.

Results

The estimated reduction in slope for the three locations ranges from approximately 0.29 to 0.61 percent. The estimated differential settlement is sufficiently low such that ponding and slope reversal is not expected to occur.

The maximum horizontal strains calculated for the three locations are less than 0.01 percent. The values are lower than the calculated maximum allowable strain of 0.10 percent and indicate that cracking of the radon attenuation layer due to settlement is not likely. The results are summarized in **Table 1** and the spreadsheet calculations and results are presented in **Attachment A** to this brief.

Conclusions

The results indicate that cracking of the radon attenuation layer due to total differential settlement is not expected.

Attachments

- Figure 1 – Differential Settlement and Cover Cracking
- Attachment A – Spreadsheet Calculations

References

- Canonie Environmental, 1994. As-built Report North Cell Final Reclamation, Church Rock Site, Gallup, New Mexico. November.
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- Gourc, J.P., S. Camp, B.V.S. Viswanadham, and S. Rajesh, 2010. "Deformation behavior of clay cap barriers of hazardous waste containment systems: Full-scale and centrifuge tests," *Geotextiles and Geomembranes*. Elsevier. Vol. 28: 281-291.
- Lee, K.L., and C.K. Shen, 1969. "Horizontal Movements Related to Subsidence." *Journal of Soil Mechanics and Foundation Division*, ASCE Volume 95. January.
- Morrison-Knudsen Environmental Corporation (Morrison-Knudsen), 1993. UMTRA-Naturita, Embankment Design, Settlement Analysis and Cracking Potential Evaluation. Calc. No. 17-740-02-01. May.
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- MWH, Inc. (MWH), 2014b. Pre-Design Studies, Northeast Church Rock Mine Site Removal Action, Northeast Church Rock Mine Site. Prepared for United Nuclear Corporation and General Electric Corporation. October 31.

Client: *GE/UNC***Sheet:** 6 of 6**Project:** *Northeast Church Rock Mine Site Removal Action***Description:** *Cover Cracking of the Existing Radon Barrier***Job No:** *10508639*

U.S. Nuclear Regulatory Commission (NRC), 2003. "Standard Review Plan for the Review of a Reclamation Plan for Mill Tailings Sites Under Title II of Uranium Mill Tailings Radiation Control Act of 1978, Final Report." NUREG-1620. June.

Rajesh, S. and B.V.S. Viswanadham, 2010. "Performance Assessment of Deformation Behavior of Landfill Barriers at the Onset of Differential Settlement," International Journal of Environmental Engineering, Vol. 2.1, pp. 269-289.

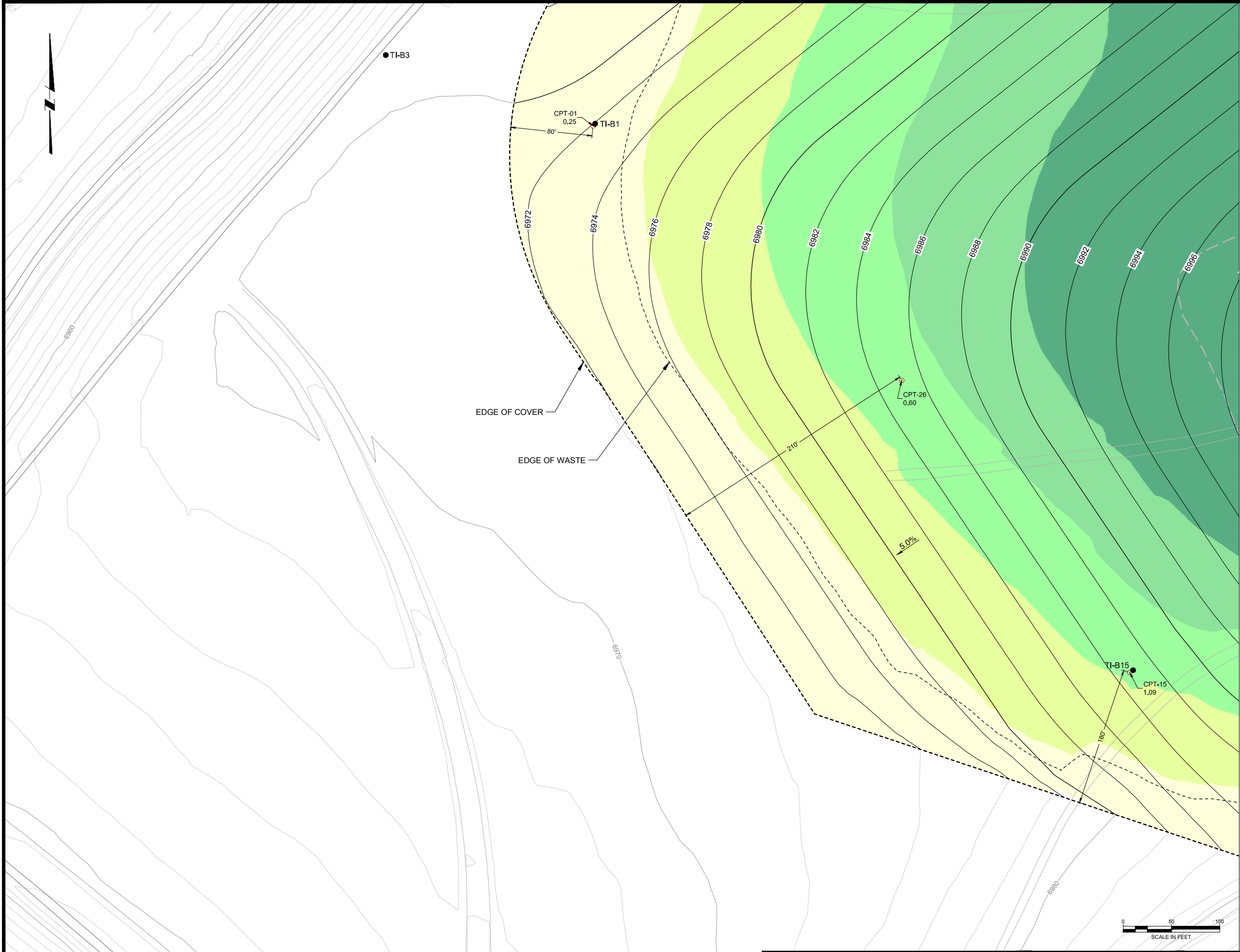
TABLE

Table 1: Estimated Differential Settlement and Cover Cracking Potential near the Edge of the Repository

| Location/Borehole ID | Estimated Total Differential Settlement (ft) | Horizontal Distance to Edge of Cover (ft) | Resulting Slope Reduction (%) | Horizontal Strain (%) |
|----------------------|--|---|-------------------------------|-----------------------|
| TI-B1/CPT-01 | 0.25 | 80 | 0.32 | 0.009 |
| TI-B15/CPT-15 | 1.10 | 180 | 0.61 | 0.008 |
| CPT-26 | 0.60 | 210 | 0.29 | 0.003 |

FIGURE

p:\omev\win103.mwhglobal.com\AM_PROJECTS02\Documents\General Electric\GE_NECR_Design\Civil\Figures\2016-3-30_REPOSITORY_COVER\NECR_REPOSITORY_COVER_THICKNESS_FIG 7-2.dwg LAYOUT:COVER PLOT DATE:7/17/2017 2:45 PM BY:KWEED



LEGEND:

- EXISTING GROUND SURFACE CONTOUR & ELEVATION, FEET
- PROPOSED SURFACE CONTOUR & ELEVATION, FEET
- EXISTING ROAD
- EXISTING DRAINAGE
- FENCE
- BOUNDARY OF REPOSITORY
- FORMER BORROW PIT BOUNDARY
- CPT-17 CPT LOCATION
- TI-B15 BORING LOCATION
- 0.26 ESTIMATED TOTAL SETTLEMENT (FEET)

NOTE:

- THE ESTIMATED TOTAL SETTLEMENT FOR THE COVER CRACKING ANALYSIS INCLUDES IMMEDIATE, PRIMARY, SECONDARY, AND SEISMIC SETTLEMENT.

| REPOSITORY FILL THICKNESS | | | |
|---------------------------|----------|----------|-------|
| NUMBER | MIN (FT) | MAX (FT) | COLOR |
| 1 | 0.0 | 5.0 | |
| 2 | 5.0 | 10.0 | |
| 3 | 10.0 | 15.0 | |
| 4 | 15.0 | 20.0 | |
| 5 | 20.0 | 30.0 | |
| 6 | 30.0 | 42.0 | |

DESIGNED _S.MOORE_
CHECKED _M.DAVIS_
APPROVED _J.CUMBERS_



UNITED NUCLEAR CORPORATION AND NORTHEAST CHURCH ROCK MINE
McKINLEY COUNTY, NEW MEXICO
DIFFERENTIAL SETTLEMENT AND COVER CRACKING

ATTACHMENT A
SPREADSHEET CALCULATIONS

Differential Settlement and Cover Cracking Evaluation

Date: 30-Jun-17 11-Nov-19
Calculated By: S. Downey S. Downey (updated)
Review: J. Cumbers M. Davis (update)

During Placement of Waste and Final Cover

| Location | Immediate Settlement (ft) |
|----------|---------------------------|
| CPT-01 | 0.10 |
| CPT-15 | 1.00 |
| CPT-26 | 0.60 |

After Placement of Waste and Final Cover

| Location | Total Primary, Secondary, and Seismic Settlement (ft) |
|----------|---|
| CPT-01 | 0.15 |
| CPT-15 | 0.10 |
| CPT-26 | 0.00 |

During and After Placement of Waste and Final Cover

| Total Potential Future Settlement (ft) |
|--|
| 0.25 |
| 1.10 |
| 0.60 |

| primary+secondary | seismic | total |
|-------------------|---------|-------|
| 0.07 | 0.079 | 0.149 |
| 0 | 0.101 | 0.101 |
| 0 | | 0 |

Differential Settlement After Active Maintenance

| Location: | CPT-01 | CPT-15 | CPT-26 |
|---|--------|--------|--------|
| Maximum Differential Settlement (ft) = | 0.25 | 1.10 | 0.60 |
| Distance between point and edge of cover (ft) = | 80 | 180 | 210 |
| Slope reduction = | 0.32% | 0.61% | 0.29% |

Evaluation of Potential for Cover Cracking

Maximum Allowable Strain

| | | |
|--------------------------------|------|------------------------------------|
| Plasticity Index (PI) = | 16 | Average of 10 samples from the PDS |
| Maximum Allowable Strain (%) = | 0.10 | (MWH, 2014) |

Peak Horizontal Movement

| Location: | CPT-01 | CPT-15 | CPT-26 |
|---|--------|--------|--------|
| Maximum Differential Settlement (ft) = | 0.25 | 1.10 | 0.60 |
| Distance between points (ft) = | 80 | 180 | 210 |
| Local slope of settlement profile, α = | 0.003 | 0.006 | 0.003 |
| Thickness of relatively incompressible material, H (ft) = | 1.8 | 1.8 | 1.8 |
| Horizontal movement, m (ft) = | 0.004 | 0.007 | 0.003 |
| Peak Horizontal Movement (ft) = | 0.01 | 0.01 | 0.01 |

Horizontal Strain

| | | | |
|-------------------------|-------|-------|-------|
| Horizontal Strain (%) = | 0.009 | 0.008 | 0.003 |
|-------------------------|-------|-------|-------|

Horizontal Strain is < Maximum Allowable Strain

Cover Cracking is Not Likely

ATTACHMENT G.6

Repository Liquefaction Triggering Analysis

Client: *GE/UNC*
Project: *Northeast Church Rock Mine Site Removal Action*
Description: *Liquefaction Triggering Analysis for the Mill Site Repository*

Sheet: 1 of 24
Job No: *10508639*

ATTACHMENT G.6: REPOSITORY LIQUEFACTION TRIGGERING ANALYSIS

| Revisoning | | | | | |
|------------|-----------|--------------------------|------------|-----------|------------|
| Rev. | Date | Description | By | Checked | Date |
| 0 | June 2016 | Preliminary (30%) Design | S. McManus | J. Barber | 06/01/2016 |
| 1 | June 2017 | 95% Design | T. Borden | M. Garton | 06/26/2017 |
| 2 | Oct 2019 | Update Cover Thickness | S. Downey | S. Sun | 11/11/2019 |

| Location and Format |
|--|
| <p>Electronic copies of these calculations are located on the Stantec internal project teamsite.</p> <p>The calculations were generated using the following software:</p> <p>Microsoft Excel</p> |

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| Objective |
|--|
| <p>The purpose of this calculation is to evaluate the potential for liquefaction of the materials beneath the repository. This calculation brief documents evaluation of the potential for liquefaction of saturated layers of tailings and alluvium beneath the proposed repository during the design seismic event. The analysis was performed for the proposed conditions (after completion of repository construction). A liquefaction screening analysis (Bray et al., 2009) was performed to identify potential zones of susceptible to liquefaction. Potential zones of susceptibility, as identified by the screening analysis, were evaluated by a liquefaction triggering analysis using two simplified liquefaction triggering analysis methods (Idriss and Boulanger, 2008; Youd et al., 2001). Liquefaction-induced settlement was estimated using the results of the liquefaction analysis and field data.</p> |

Client: *GE/UNC*Sheet: 2 of 24Project: *Northeast Church Rock Mine Site Removal Action*Description: *Liquefaction Triggering Analysis for the Mill Site Repository*Job No: *10508639*

Background

This analysis was performed as part of the design of the Removal Action (RA) at the Northeast Church Rock Mine Site (Mine Site) and the related Remedial Action (RA) at the Church Rock Mill Site (Mill Site). The Mine Site and Mill Site are located in close proximity to one another, approximately 16 miles northeast of Gallup, in McKinley County, New Mexico. They are located on adjacent Sections approximately one-half mile apart. The sites are temporarily being treated as one facility for purposes of the RA. The combined site is referred to as the "Settlement Agreement Site" (SA Site).

Site History

The NECR mine is a historical uranium mine operated by United Nuclear Corporation (UNC). Mining development began in 1967 and ended in 1982. While the mine operated, it served as the principal mineral source for the UNC uranium mill. The uranium mill and its adjacent disposal cells make up the UNC Superfund Site (the "UNC Mill Site"). Remedial activities addressing source control and on-site surface reclamation are being implemented by General Electric/United Nuclear Corporation (GE/UNC) under the direction of the U.S. Nuclear Regulatory Commission (NRC), pursuant to the UNC facility's NRC license, and integrated with the US Environmental Protection Agency's (USEPA's) selected remedy for the groundwater.

The tailings disposal area (TDA) is an unlined facility bounded by an embankment and subdivided by cross-dikes into three cells, which are identified as the South Cell, Central Cell, and North Cell. An estimated 3.5 million tons of tailings were pumped in slurry from the mill to the TDA.

Proposed Remedial Action

The proposed repository will be constructed on top of the existing TDA and will incorporate controlled placement of mine waste on top of the existing TDA cover/radon barrier and a final evapotranspirative (ET) cover placed over the mine waste. Improvements to the existing TDA cover/radon barrier within the footprint of the proposed repository will be completed prior to placement of mine waste. **Figure 1** shows the location and grading of the proposed repository.

The design for the selected repository alternative will be evaluated as part of an NRC license amendment request for the existing licensed facility. The repository features that affect the licensed facility will meet the performance standards outlined in NRC regulations and areas of the existing facility affected by the repository construction will be evaluated for compliance. However, existing conditions of the facility not affected by the proposed repository were not evaluated as part of this analysis, as they are managed by the existing NRC license.

Site Description

The natural stratigraphy at the Mill Site is divided into two main components: the surficial unconsolidated deposits (alluvium) and the underlying consolidated bedrock units. The alluvium consists of a mixture of sand, silt, and clay with minor portions of gravel. Alluvial thicknesses at the site are usually around 50 feet, but exceed 120 feet in some locations. Generally, the uppermost bedrock unit at the site is the Upper Gallup Sandstone, though in some locations it is overlain by coal or the Mancos shale.

The TDA was constructed on top of the native alluvium and deposition of tailings via slurry within the TDA resulted in an interbedded accumulation of tailings. TDA closure construction began in 1989 and was completed in 1995. Closure construction included placement of an interim cover (general fill) from 1989 through 1991 followed by placement of the final cover (radon barrier and erosion protection layer) from 1993 through 1995.

Measurements taken in alluvial monitoring wells show an alluvial groundwater table in the vicinity of the TDA at approximately 6,867 feet above mean sea level (amsl), which indicates that the alluvium is unsaturated above this

Client: *GE/UNC*
Sheet: 3 **of** 24
Project: *Northeast Church Rock Mine Site Removal Action*
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elevation. Additionally, subsurface investigations of the TDA indicate that there is not a consistent static water level within the tailings or the alluvium above approximately 6,867 feet amsl. However, localized perched zones of saturation exist within the low-permeability, fine-grained tailings. These zones of saturation do not appear to extend beyond the fine-grained tailings into the higher-permeability coarse-grained tailings.

Site Investigation

In 2013, MWH performed pre-design studies (PDS) at the Mill Site and Mine Site to supplement previous site investigations and collect pre-design data necessary to perform the Remedial Design (RD). Activities performed as part of the Mill Site PDS included: surveying, cone penetration tests (CPTs), drilling, standard penetration tests (SPTs), excavation and soil sampling, and subsequent laboratory testing. Geotechnical data collected during the PDS are presented in the PDS reports (MWH, 2014a and MWH, 2014b) and summarized in **Attachment A**. A list of the materials encountered within the TDA during the PDS is presented in the Assumptions section below. Geotechnical properties for these materials and discussion of one-dimensional stratigraphic profiles developed for the liquefaction triggering analysis are also presented in the Assumptions section.

Applicable Codes and Standards

Applicable regulatory guidance documents include the following:

- NUREG 1620, Section 2.4 (NRC, 2003)
- NRC Regulatory Guide 3.11, Section C (NRC, 2008)

Methods

General

The liquefaction triggering analysis evaluated the potential for liquefaction of saturated soil layers beneath the proposed repository to damage the existing TDA radon barrier or compromise the effectiveness of the proposed repository. A liquefaction screening evaluation was performed to identify soil layers that may be susceptible to liquefaction.

One-dimensional soil profiles were developed for analysis, based on conditions observed during the Mill Site PDS field investigation and modified to reflect proposed conditions (after completion of repository construction) (see **Figure 2**). Identified zones of potentially susceptible soils within these profiles, as identified by the screening analysis, were evaluated for liquefaction potential using simplified liquefaction triggering analysis methods (Idriss and Boulanger, 2008; Youd et al., 2001). The “simplified procedure” was first developed by Seed and Idriss (1971). “That procedure has been modified and improved periodically since that time...” (Youd et al., 2001). Detailed earthquake measurements in recent decades have allowed researchers to adjust the simplified procedure for empirical accuracy. It is therefore appropriate to use these updated simplified procedures. Regulatory Guide 3.11 (2008) lists five criteria that any liquefaction analysis shall meet: (1) development of a detailed understanding of site conditions (see Attachment A), (2) development of simplified cross-section amenable to analysis (see Figure 1), (3) calculation of the force required to liquefy the critical zone based on the characteristics of the critical zone (this is the Cyclic Resistance Ratio [CRR] as calculated below) (4) calculation of the design earthquake effect (this is the Cyclic Stress Ratio (CSR) as calculated below), and (5) computation of the factor of safety against liquefaction (see Table 2). Therefore, the liquefaction analysis presented herein meets the NRC Regulatory Guide 3.11 criteria.

The liquefaction triggering analysis used data collected during CPTs, Hollow Stem Auger (HSA) drilling, SPTs, and laboratory testing to calculate the factor of safety (FS) against liquefaction for potentially susceptible soil layers below the proposed repository. Primary analysis of the soil layers was performed using the results of CPTs. Results of SPTs, where available, were analyzed to provide a secondary data point against which the results of the CPT-based analysis were checked. The liquefaction triggering analyses incorporated supplemental data from laboratory testing and were

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performed according to the methods outlined in Idriss and Boulanger (2008) and Youd et al. (2001). The FS was calculated as the average of the FS values calculated by each of the analysis methods. Both analysis methods were based in part on the CPT data, which was collected approximately every two inches within the subsurface profile. Due to the heterogeneity of the subsurface materials, anomalies occurred in the CPT data. By averaging the two calculation methods, the effects of anomalies on the calculated results was reduced for the evaluated profiles. The percent difference between the FS for the two methods was calculated for critical depths (location of minimum FS) and was observed to be at most 7 percent for CPT methods and 13 percent for SPT methods. Therefore, each method independently yields an answer close to the reported average.

Subsurface materials considered in the liquefaction triggering analysis are identified in the Assumptions section. Subsurface material properties relevant to the liquefaction triggering analysis and one-dimensional stratigraphic profiles evaluated in this analysis are also presented in the Assumptions section.

Liquefaction Screening Evaluation

A liquefaction screening evaluation was performed to identify soil layers that are potentially susceptible to liquefaction, considering tailings and alluvial soils at the TDA with perched layers of saturation, high fines content, and high plasticity. Saturated, or nearly-saturated, soils identified as “susceptible” or “moderately susceptible” to liquefaction (Bray et al., 2009) were further evaluated using the liquefaction triggering analysis, as described below. Unsaturated soils with a high degree of saturation (85 percent or higher) may behave like saturated soils and experience strength reduction due to excess pore pressure generation during a seismic event (NRC, 2008). Soils at or near saturation are composed mostly of the fine-grained tailings and fine-grained alluvium that exhibit a low hydraulic conductivity. For the purpose of this analysis, the term “nearly saturated” refers to soils between 85 percent and 99 percent saturation. Discussion of saturated and nearly saturated soil layers are presented in the Assumptions section. Every borehole was evaluated for the entire depth, which was generally in excess of 50 ft. The one borehole evaluated to a bottom depth of 34 ft. experienced refusal at that depth and liquefaction is therefore not a concern.

Soils were identified as “susceptible” or “moderately susceptible” to liquefaction according to the guidelines outlined in Bray et al. (2009), which state that a soil deposit is considered to be susceptible to liquefaction if the ratio of the water content (w_c) to liquid limit (LL) is greater than or equal to 0.85 ($w_c/LL \geq 0.85$), and the soil plasticity index (PI) is less than or equal to 12 ($PI \leq 12$). Soils with $12 < PI \leq 20$ and $w_c/LL \geq 0.8$ may be moderately susceptible to liquefaction. Soils with $PI > 20$ are considered too clayey to liquefy.

NRC Regulatory Guide 3.11 (2008) states that if three or more of the following indicate liquefaction is likely, then “... a more rigorous analysis of the liquefaction potential at a facility is required”: (1) geologic age and origin, (2) fines content and plasticity index, (3) saturation, (4) depth below ground surface, and (5) soil penetration and resistance. As discussed above, materials considered to be susceptible to liquefaction included materials with 85 percent saturation or greater (meets NUREG 3.11), water content to liquid limit ratio of 0.85 or greater (exceeds NUREG 3.11, which limits at a ratio of 0.9), and material depths from the ground surface to depths greater than 50 feet (meets NUREG 3.11). In addition, penetration resistances were used to evaluate liquefaction potential (meets NUREG 3.11). In cases where three or more of the NRC criteria were met, the materials were further evaluated using the Bray et al. (2009) criteria which states a soils plasticity index (PI) is a better indicator of susceptibility than weight of clay-size particles. PI values used in this evaluation come from laboratory testing of samples taken along the length of each borehole.

Analysis of Cone Penetration Testing Results

Two procedures were used to evaluate the potential for liquefaction of the soils beneath the proposed repository based on the results of the CPT soundings and the drilling logs associated with the “paired” hollow-stem auger boreholes. These methods (Idriss and Boulanger, 2008; Youd et al., 2001) are described below. The average FS calculated from the two methods was used to evaluate the liquefaction potential of these soils.

Idriss and Boulanger (2008)

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The Idriss and Boulanger (2008) liquefaction triggering method estimates the cyclic stress ratio (CSR) based on the seismic design criteria and estimates the cyclic resistance ratio (CRR) based on the CPT readings and site conditions. CSR is calculated using a simplified procedure to estimate earthquake induced stresses normalized for a magnitude 7.5 earthquake and confining pressure equal to 1 atm, calculated using the following relationship:

$$CSR_{M=7.5, \sigma'_{vc}=1} = 0.65 \frac{a_{max}}{g} \frac{\sigma_{vc}}{\sigma'_{vc}} r_d \frac{1}{MSF} \frac{1}{K_\sigma} \frac{1}{K_\alpha}$$

where:

a_{max} : maximum horizontal ground surface acceleration

σ'_{vc} : effective vertical confining stress

σ_{vc} : total vertical confining stress

r_d : shear stress reduction coefficient

MSF : earthquake magnitude scaling factor

K_σ : overburden correction factor

K_α : static shear stress correction factor

g : acceleration due to gravity

The equations for the correction factors applied to the CSR for this evaluation are the following:

$$r_d = \exp(\alpha(z) + \beta(z)M)$$

$$\alpha(z) = -1.012 - 1.126 \sin\left(\frac{z}{11.73} + 5.133\right)$$

$$\beta(z) = 0.106 + 0.118 \sin\left(\frac{z}{11.28} + 5.142\right)$$

$$MSF = 6.9 \exp\left(\frac{-M}{4}\right) - 0.058 \leq 1.8$$

$$K_\sigma = 1 - C_\sigma \ln\left(\frac{\sigma'_{vc}}{P_a}\right) \leq 1.1$$

$$C_\sigma = \frac{1}{37.3 - 8.27(q_{c1N})^{0.264}} \leq 0.3$$

where:

r_d : shear stress reduction coefficient

q_{c1N} : tip resistance normalized to atmospheric pressure and overburden pressure

z : depth below ground surface

P_a : atmospheric pressure (calculated for an average elevation of 7,000 feet for the site)

M : design earthquake magnitude

The liquefaction triggering analysis was performed assuming essentially flat ground and ignored the effects of the sloping final surface of the repository. Thus, a static shear stress correction factor of $K_\alpha=1$ was used for all calculations.

The cone tip resistance (q_c) as measured in the field is dependent on the confining stress. To express the material strength independent of the in-situ stress condition, the field readings are normalized for a confining stress of 1 atm (q_{c1}) according to:

$$q_{c1} = C_N * q_c$$

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$$q_{c1N} = \frac{q_{c1}}{P_a}$$

$$\text{For } 21 \leq q_{c1N} \leq 254: C_N = \left(\frac{P_a}{\sigma'_{vc}} \right)^{1.338 - 0.249 * (q_{c1N})^{0.264}} \leq 1.7$$

where:

 q_{c1} : cone tip resistance corrected to an effective overburden stress of 1 atm

 C_N : CPT overburden correction factor

 q_c : cone tip resistance recorded by cone penetrometer

Solving for C_N and q_{c1N} using the above expressions requires iteration, because q_{c1N} is dependent on C_N and C_N is dependent on q_{c1N} . This iteration was accomplished using circular references in a spreadsheet.

The relationship for CRR is based on liquefaction case histories and is expressed as:

$$\text{CRR}_{M=7.5, \sigma'_{vc}=1} = \exp \left(\frac{q_{c1Ncs}}{540} + \left(\frac{q_{c1Ncs}}{67} \right)^2 - \left(\frac{q_{c1Ncs}}{80} \right)^3 + \left(\frac{q_{c1Ncs}}{114} \right)^4 - 3 \right)$$

where:

 q_{c1Ncs} : equivalent clean-sand corrected normalized tip resistance

The cone tip resistance corrected for overburden (q_{c1}) must be further corrected for the fines content (q_{c1Ncs}). A clayey material with the same strength as a clean sand will measure lower cone tip resistances, and this correction accounts for that phenomenon. The correction for clean sand is calculated as follows:

$$q_{c1Ncs} = q_{c1N} + \Delta q_{c1N}$$

$$\Delta q_{c1N} = \left(5.4 + \frac{q_{c1N}}{16} \right) \cdot \exp \left(1.63 + \frac{9.7}{FC + 0.01} - \left(\frac{15.7}{FC + 0.01} \right)^2 \right)$$

where:

 FC = Fines Content in %

The FS against liquefaction was computed as:

$$FS_{liq} = \frac{\text{CRR}_{M=7.5, \sigma'_{vc}=1}}{\text{CSR}_{M=7.5, \sigma'_{vc}=1}}$$

The correlation between CSR, CRR, and q_{c1N} is shown in Figure 67 of Idriss and Boulanger (2008).

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Youd et al. (2001)

The Youd et al. (2001) liquefaction triggering analysis method estimates the CSR based on the seismic design criteria and estimates the CRR based on the CPT readings and site conditions. CSR is calculated using a simplified procedure to estimate earthquake induced stresses normalized for a magnitude of 7.5 and a confining pressure equal to 1 atm, calculated using the following relationship:

$$CSR_{M=7.5, \sigma'_{vc}=1} = 0.65 \frac{a_{max}}{g} \frac{\sigma_{vc}}{\sigma'_{vc}} r_d \frac{1}{MSF} \frac{1}{K_\sigma} \frac{1}{K_\alpha}$$

where:

a_{max} : maximum horizontal ground surface acceleration

σ'_{vc} : effective vertical confining stress

σ_{vc} : total vertical confining stress

r_d : shear stress reduction coefficient

MSF : earthquake magnitude scaling factor

K_σ : overburden correction factor

K_α : static shear stress correction factor

g : acceleration due to gravity

The equations for the correction factors applied to the CSR for this evaluation are the following:

$$r_d = 1.0 - 0.00765 \times z \text{ for } z \leq 9.15m$$

$$r_d = 1.174 - 0.0267 \times z \text{ for } 9.15m < z \leq 23m$$

$$\text{Revised Idriss Scaling Factor: } MSF = \frac{10^{2.24}}{M_w^{2.56}}$$

$$K_\sigma = \left(\frac{\sigma'_{vc}}{P_a} \right)^{(f-1)}$$

where:

r_d : shear stress reduction coefficient

z : depth below ground surface

M_w : design earthquake magnitude

P_a : atmospheric pressure (calculated for an average elevation of 7,000 feet for the site)

σ'_{vc} : effective vertical overburden pressure

$f = 0.7$ to 0.8 for $40\% \leq$ relative density, $D_r \leq 60\%$

$f = 0.6$ to 0.7 for $60\% <$ relative density, $D_r \leq 80\%$

$$D_r = \sqrt{\frac{q_{c1n}}{300}}$$

where:

q_{c1n} : tip resistance normalized to atmospheric pressure and overburden pressure (calculated using the same method as outlined in Idriss and Boulanger, 2008 [see above]).

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The liquefaction triggering analysis was performed assuming essentially flat ground and ignored the effects of the sloping final surface of the repository. Thus, a static shear stress correction factor of $K_a=1$ was used for all calculations.

The relationship for CRR is based on liquefaction case histories and is expressed as:

$$\begin{aligned} \text{If } (q_{c1N})_{cs} < 50 \text{ then } CRR_{7.5} &= 0.833 \left[\frac{(q_{c1N})_{cs}}{1000} \right] + 0.05 \\ \text{If } 50 \leq (q_{c1N})_{cs} < 160 \text{ then } CRR_{7.5} &= 93 \left[\frac{(q_{c1N})_{cs}}{1000} \right]^3 + 0.08 \\ \text{If } (q_{c1N})_{cs} \geq 160 \text{ then } CRR_{7.5} &= 1.0 \end{aligned}$$

where $(q_{c1N})_{cs}$ is the normalized cone penetration resistance adjusted to the clean sand value and is defined by:

$$(q_{c1N})_{cs} = K_c \times q_{c1N}$$

where:

q_{c1N} : tip resistance normalized to atmospheric pressure and overburden pressure
 K_c : correction factor for grain characteristics

$$\text{For } I_c \leq 1.64 \quad K_c = 1.0$$

$$\text{For } I_c > 1.64 \quad K_c = -0.403I_c^4 + 5.581I_c^3 - 21.63I_c^2 + 33.75I_c - 17.88$$

where:

I_c : soil behavior type index

I_c is defined by the following equations (per Robertson and Cabal, 2012, page 29):

$$I_c = \left[(3.47 - \log Q)^2 + (1.22 + \log F)^2 \right]^{0.5}$$

$$Q = \frac{(q_t - \sigma_{vo})}{\sigma'_{vo}}$$

$$F = \left[\frac{f_s}{(q_c - \sigma_{vo})} \right] \times 100\%$$

where:

Q : normalized and dimensionless cone penetration resistance
 F : normalized friction ratio
 q_c : cone tip resistance recorded by cone penetrometer
 f_s : sleeve friction measured by cone penetrometer

The FS against liquefaction was computed as:

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$$FS_{liq} = \frac{CRR_{M=7.5, \sigma'_{vc}=1}}{CSR_{M=7.5, \sigma'_{vc}=1}}$$

The correlation between CSR, CRR, and q_{c1N} is shown in Figure 4 of Youd et al. (2001).

Analysis of Standard Penetration Test Results

Results of SPTs performed during the PDS were analyzed using two procedures to evaluate the potential for liquefaction of the soils beneath the proposed repository. These methods are described below (Idriss and Boulanger, 2008; Youd et al., 2001). The average FS calculated from the two methods was used to evaluate the liquefaction potential of these soils. Analyses using SPT blow counts were performed for the discreet locations for which SPT data was available (e.g. blow counts were not extrapolated to other areas). Analysis of SPT results were used only as a secondary data point against which the results of the CPT-based analysis were checked.

SPTs performed during the Mill Site PDS used an oversized split spoon sampler (2.5-inch outer diameter). To correct the recorded blow counts for this non-standard sampler, the LaCroix and Horn corrective method was applied (Rogers, 2006 after LaCroix and Horn, 1973):

$$N = N_1 \left(\frac{2in}{D_1} \right)^2 \times \frac{12in}{L_1} \times \frac{W_1}{140lbs} \times \frac{H_1}{30in} = \frac{2N_1W_1H_1}{175D^2L_1}$$

$$\text{For } D = 2.5 \text{ in.}: N = N_1 \times 0.64$$

where:

N: standard penetration resistance (blow count)
N₁: nonstandard penetration resistance (blow count)
L: sampler drive distance (12 in.)
D: outside diameter of the nonstandard sampler (2.5 in.)
W: weight of the hammer (140 lb.)
H: height of the hammer drop (30 in.)

Using this relationship, a correction factor of 0.64 was applied to the blow counts recorded during the Mill Site PDS.

After correcting the recorded blow counts for the non-standard sampler, the blow counts were further corrected for an energy ratio of 60 percent (Idriss and Boulanger, 2008), rod length, borehole diameter, and sampler type, using the following equations and correction factors:

$$N_{60} = C_E C_B C_R C_S N_m$$

Rod Length Correction Factor

| Rod Length (m) | C_R |
|----------------|-------|
| < 3 | 0.75 |
| 3 to 4 | 0.80 |
| 4 to 6 | 0.85 |
| 6 to 10 | 0.95 |
| 10 to 30 | 1.00 |

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$$C_E = \frac{ER_m}{60}$$

where:

N_{60} : blow count for an energy ratio of 60%

C_E : energy ratio correction factor

C_B : correction factor for borehole diameter (equal to 1.15 for boreholes with $D \geq 8$ in.)

C_R : correction factor for rod length

C_S : correction factor for samplers without liners (equal to 1.0 for a split spoon sampler with liners)

N_m : measured blow count

ER_m : delivered energy ratio

Idriss and Boulanger (2008)

The Idriss and Boulanger (2008) method of liquefaction triggering analysis using SPT blow counts is similar to the method presented in the same paper for analysis using CPT results (see above). The CSR is estimated using the same method as was used for the CPT-based analysis. However, the CRR is estimated using SPT blow counts. Using this method, the CRR can only be estimated at discrete locations within the soil profile where SPT blow counts are available.

The relationship for CRR is based on liquefaction case histories and is expressed as:

$$CRR_{M=7.5, \sigma'_{vc}=1} = \exp \left(\frac{(N_1)_{60cs}}{14.1} + \left(\frac{(N_1)_{60cs}}{126} \right)^2 - \left(\frac{(N_1)_{60cs}}{23.6} \right)^3 + \left(\frac{(N_1)_{60cs}}{25.4} \right)^4 - 2.8 \right)$$

where:

$(N_1)_{60cs}$: equivalent clean sand corrected $(N_1)_{60}$

$$(N_1)_{60cs} = (N_1)_{60} + \Delta(N_1)_{60}$$

$$\Delta(N_1)_{60} = \exp \left(1.63 + \frac{9.7}{FC + 0.01} - \left(\frac{15.7}{FC + 0.01} \right)^2 \right)$$

where:

FC: fines content (expressed in percent)

$\Delta(N_1)_{60}$: fines content correction factor for $(N_1)_{60}$

$(N_1)_{60}$: SPT blow count corrected to a 60% energy ratio and an overburden stress of 1 atm

$(N_1)_{60}$ is estimated using the following equations:

$$(N_1)_{60} = C_N \cdot N_{60}$$

$$C_N = \left(\frac{P_a}{\sigma'_{vc}} \right)^m$$

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$$m = 0.784 - 0.521 \cdot D_R$$

$$D_R = \sqrt{\frac{(N_1)_{60}}{C_d}}$$

$C_d = 40$ for fine sands in recent fills (per Skempton, 1986)

where:

C_N : SPT overburden correction factor

P_a : atmospheric pressure (calculated for an average elevation of 7,000 feet for the site)

m : parameter dependent on sand properties and relative density

σ'_{vc} : vertical effective stress

D_R : relative density

Solving for $(N_1)_{60}$ and C_N using the above expressions requires iteration, because $(N_1)_{60}$ is dependent on C_N and C_N is dependent on $(N_1)_{60}$. This iteration was accomplished using circular references in a spreadsheet.

As before, the FS against liquefaction was computed as:

$$FS_{liq} = \frac{CRR_{M=7.5, \sigma'_{vc}=1}}{CSR_{M=7.5, \sigma'_{vc}=1}}$$

Youd et al. (2001)

The Youd et al. (2001) liquefaction triggering analysis using SPT blow counts is similar to the method presented in the same paper for analysis using CPT results (see above). The CSR is estimated using the same method as was used for the CPT-based analysis. However, the CRR is estimated using SPT blow counts. Using this method, the CRR can only be estimated at discrete locations within the soil profile where SPT blow counts are available.

The relationship for CRR is based on liquefaction case histories and is expressed as:

$$CRR_{7.5} = \frac{1}{34 - (N_1)_{60cs}} + \frac{(N_1)_{60cs}}{135} + \frac{50}{[10 \cdot (N_1)_{60cs} + 45]^2} - \frac{1}{200}$$

where:

$(N_1)_{60cs}$: equivalent clean sand corrected $(N_1)_{60}$

$(N_1)_{60cs}$ is estimated using the following equations:

$$(N_1)_{60cs} = \alpha + \beta \cdot (N_1)_{60}$$

For $FC \leq 5\%$ $\alpha = 0$

$$\text{For } 5\% < FC < 35\% \quad \alpha = \exp\left[1.76 - \left(\frac{190}{FC^2}\right)\right]$$

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For $FC \geq 35\%$ $\alpha = 5.0$

For $FC \leq 5\%$ $\beta = 1.0$

$$\text{For } 5\% < FC < 35\% \quad \beta = \left[0.99 + \left(\frac{FC^{1.5}}{1,000} \right) \right]$$

For $FC \geq 35\%$ $\beta = 1.2$

where:

FC: fines content (expressed in percent)

(N₁)₆₀ : SPT blow count corrected to a 60% energy ratio and an overburden stress of 1 atm calculated using the same method as outlined in Idriss and Boulanger, 2008 (see above).

As before, the FS against liquefaction was computed as:

$$FS_{liq} = \frac{CRR_{M=7.5, \sigma'_{vc}=1}}{CSR_{M=7.5, \sigma'_{vc}=1}}$$

Post-Liquefaction Reconsolidation Settlement

Following a seismic event during which liquefaction occurs, the soil may experience a volume change as the excess pore water dissipates and the soil particles rearrange themselves. One method for estimating this volume change is outlined in Idriss and Boulanger (2008), Section 4.4 "Post-liquefaction Reconsolidation Settlement". Assuming that displacement occurs only in the vertical direction, the settlement can be calculated from the volumetric strain according to:

$$S_{v-1D} = \int_0^{z_{max}} \varepsilon_v * dz$$

where:

S_{v-1D} : magnitude of vertical settlement with 1D assumptions

ε_v : volumetric strain

z : depth below surface

The volumetric strain can be calculated using either SPT or CPT data. Both methods were implemented for the available data using the following equations:

Estimated volumetric strain using SPT data:

$$\varepsilon_v = 1.5 * e^{(-0.369 * \sqrt{(N1_{60cs})})} * \min(0.08, \gamma_{max})$$

where:

N_{160cs} : equivalent clean sand corrected (N₁)₆₀

γ_{max} : maximum shear strain developed during liquefaction

Estimated volumetric strain using CPT data:

$$\varepsilon_v = 1.5 * e^{(2.551 - 1.147 * q_{c1Ncs}^{0.264})} * \min(0.08, \gamma_{max})$$

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where:

 γ_{max} : maximum shear strain developed during liquefaction

 $q_{c1N_{cs}}$: normalized cone penetration resistance adjusted to the clean sand value

Note that the CPT correlation for volumetric strain is limited to $q_{c1N_{cs}}$ values of greater or equal to 21. Values less than 21 are calculated as being on the asymptote defined by the value calculated at 21. To estimate the maximum shear strain, one must first identify which of three ranges the factor of safety falls in.

$$FS_{liq} \geq 2$$

$$\gamma_{max} = 0$$

$$2 > FS_{liq} > F_{\alpha}$$

$$\gamma_{max} = \min \left[\gamma_{lim}, 0.035 * (2 - FS_{liq}) * \left(\frac{1 - F_{\alpha}}{FS_{liq} - F_{\alpha}} \right) \right]$$

$$F_{\alpha} \geq FS_{liq}$$

$$\gamma_{max} = \gamma_{lim}$$

where:

 γ_{lim} : limiting shear strain

 F_{α} : cut-off for above ranges defined below for both CPT and SPT data

$$F_{\alpha} = 0.032 + 0.69 * \sqrt{N_{1,60cs}} - 0.13 * N_{1,60cs}$$

$$F_{\alpha} = -11.74 + 8.34 * q_{c1N_{cs}}^{0.264} - 1.371 * q_{c1N_{cs}}^{0.528}$$

Note that the SPT correlation for F_{α} is limited to $N_{1,60cs}$ values of greater or equal to 7 and the CPT correlation for F_{α} is limited to $q_{c1N_{cs}}$ values of greater or equal to 69. Once the appropriate expression for the maximum shear strain has been estimated, the final term may be calculated according to (as before, equations for both CPT and SPT data):

$$\gamma_{lim} = 1.859 * \left(1.1 - \sqrt{\frac{N_{1,60cs}}{46}} \right)^3 \geq 0$$

$$\gamma_{lim} = 1.859 * (2.163 - 0.478 * q_{c1N_{cs}}^{0.264})^3 \geq 0$$

There are a few important implications about this series of equations. Firstly, if the FS_{liq} is greater than or equal to 2, there will be zero settlement (as the volumetric strain term is multiplied by zero). The two primary inputs are relative density (alternatively expressed as $N_{1,60cs}$ or $q_{c1N_{cs}}$) and FS_{liq} (which is a function of CRR and CSR). Volumetric strain can therefore be plotted showing a series of solutions on the figure below.

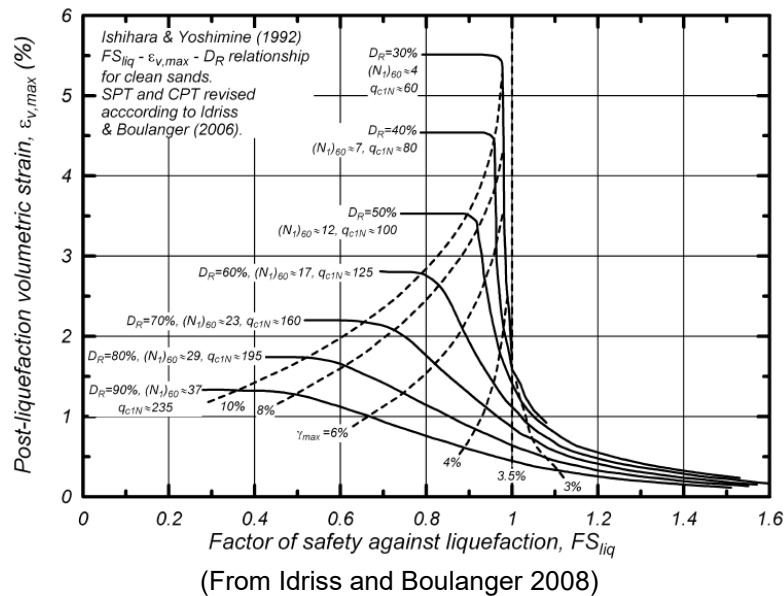
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From the above plot, it can be seen that the settlement calculation is sensitive to an FS breakpoint of 1. Typical volumetric strains for an FS greater than 1 are in the range of 0.5 percent -1.5 percent. The top curve represents strains up to 5.5 percent. The maximum strain is approximately 12 percent, which is for the softest possible soil within the equation constraints (CPT values of less than 21).

Material Properties and General Assumptions

Subsurface Materials

The PDS investigation encountered the following materials within the TDA: general fill, erosion protection admixture, radon barrier (clay), tailings, alluvium, and various bedrock units (including coal, shale, and sandstone). For the purposes of the liquefaction triggering analysis, some of these units were further subdivided according to notation in the borehole logs, results of CPT logs, and results of laboratory analysis.

The material identifications used in this analysis are discussed below and are as follows: erosion protection, cover soil, mine waste, radon barrier, general fill, coarse tailings, fine tailings, coarse/fine tailings, coarse alluvium, and fine alluvium. A comprehensive discussion of material properties is contained in Appendix G: Mine Waste Repository Design. Table 1 presents the material properties associated with each of these materials used in this analysis.

Other General Assumptions

- The ground surface elevations for paired CPT and HSA boreholes have been estimated from the topographic survey using AutoCAD Civil3D.
- The design seismic event is the 10,000-year return period earthquake, which has a maximum peak ground acceleration (PGA) of 0.30 g and a magnitude of 5.5, as identified in the Seismic Hazard Analysis (SHA; see Appendix G, Attachment G.1).
- The minimum acceptable average factor of safety against liquefaction is 1.0 (NRC, 2008). If the FS is less than 1.0 over a minimum depth of 1 foot, the material is considered liquefiable.
- Soils that have a degree of saturation less than 85 percent are assumed to be non-liquefiable (NRC, 2008).

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- Soils at a depth in excess of 50 feet are less susceptible to liquefaction than those within the first 50 feet (NRC, 2008). Nonetheless, liquefaction triggering and susceptibility analysis was conducted for the available CPT/SPT data, which in some places went up to a depth of 80 feet. Two of the liquefiable layers are at depths of greater than 50 feet, and this should be considered a mitigating factor when analyzing the risk they pose.
- The liquefaction triggering analysis was performed assuming essentially flat ground, and ignored the effects of the sloping final surface of the repository. Thus, a static shear stress correction factor of $K_\sigma=1.0$ was used for all calculations (Idriss and Boulanger, 2008).
- The in-field energy ratio (ER_m) for the SPTs performed during the PDS field investigation of the tailings impoundment is 0.75. This is based on published data for CME Automatic SPT hammers (USBR, 1999).

Stratigraphic Profiles

During the Mill Site PDS, eight CPTs were paired with boreholes to correlate CPT results with direct observation of the materials encountered. The borehole logs are presented in **Attachment B** and plots of the CPT measurements are presented in **Attachment C**. Seven of these “paired” CPT locations are within the footprint of the proposed repository. The CPT data combined with the profiles from the borehole logs were used to define the thickness and texture of the soil layers, as well as the location of the contact between the tailings and underlying alluvium. The relationships used to define the tailings-alluvium contact are described in the Mill Site PDS (MWH, 2014a). These subsurface profiles were modified using the proposed repository design to reflect proposed conditions after repository construction. The final one-dimensional profiles (**Figure 2**) and associated CPT results (**Attachment C**) were used in the liquefaction triggering analysis.

Groundwater

Groundwater was encountered during drilling in two of the boreholes (TI-B10 and TI-B11) within the footprint of the proposed repository. In both boreholes, the groundwater elevation was approximately 6,885 feet amsl. Groundwater was also encountered at about 6,903 feet amsl while drilling in boring B3 (drilled through the dam). In addition, alluvial wells 509D and EPA 23 (measured on 1/4/2016) show an alluvial ground water elevation of approximately 6,867 feet amsl. These elevations are below the bottom of the tailings and exceed the depth at which liquefaction is likely to occur. Therefore, the liquefaction triggering analysis did not assume the presence of a consistent static water level within the TDA. Water level measurements taken in the vicinity of the TDA are presented in **Attachment D**.

For the purpose of this analysis, it is assumed that localized perched zones of saturated or nearly saturated soils are present above the water levels in the alluvial wells and encountered while drilling. Unsaturated soils with a high degree of saturation (85 percent or higher) may behave like saturated soils and experience strength reduction due to excess pore pressure generation during a seismic event (NRC, 2008). Soils at or near saturation consist mostly of the fine-grained tailings and fine-grained alluvium that exhibit a low hydraulic conductivity. For the purpose of this analysis, the term “nearly saturated” refers to soils between 85 percent and 99 percent saturation.

For the purposes of this analysis, it has been assumed that hydrostatic conditions are present in tailings at or above 85 percent saturation considering the long-term loading of the site. A layer of clay alluvium exists beneath much of the tailings impoundment. The clay layer appears to have localized zones of saturated or nearly saturated soils that may experience strength reduction due to excess pore pressure generation during a seismic event. For the purpose of this analysis, it is assumed that these soils are nearly saturated and should be included in the liquefaction screening and triggering analysis. It is also assumed that hydrostatic conditions do not exist in the alluvium, above the static water levels observed while drilling the HSA boreholes or the nearby alluvial monitoring wells.

It is also assumed that pore pressure dissipation tests performed in the tailings were unable to reach equilibrium due to the low hydraulic conductivity of the tailings deposited in the TDA. Therefore, the results of the pore pressure dissipation tests are likely to be artificially elevated and are not indicators of saturation. Assumptions regarding perched zones of saturated and nearly saturated soils at each of the paired CPT/boreholes are based on the results of CPTs,

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observations during HSA drilling and sampling, and subsequent laboratory testing. These assumptions are presented below.

CPT-01 and TI-B1

Tailings

- It is assumed that saturated or nearly saturated tailings do not exist at this location.
- Dynamic pore pressures recorded by the CPT in the tailings at CPT-01 were not elevated, indicating that these tailings are unsaturated.
- Eight tailings samples from TI-B1 were analyzed for water contents. All but one of these samples had degrees of saturation below 85 percent (1 percent to 63 percent saturation).
- One tailings sample (from approximately 31.25 ft to 31.5 ft below ground surface [bgs]) was nearly saturated (94 percent saturation). However, it is assumed that this sample is not indicative of the soils in this area and was collected from a discontinuous layer of interbedded fine tailings for the following reasons:
 - Three samples from the approximately 1.25 ft of immediately overlying soil (30 ft to 31.25 ft bgs) did not exhibit this level of saturation (1 percent to 47 percent saturation).
 - Analysis of a soil sample from approximately one foot below this sample (32 ft to 33 ft bgs) did not exhibit this level of saturation (63 percent saturation).
 - The boring log for TI-B1 indicated that the soils from 18.5 ft to 34.3 ft bgs are coarse tailings, an observation that is supported by laboratory analysis (other samples in this layer had fines contents of 7 percent, 9 percent and 53 percent). However, the sample in question had a fines content of 69 percent and was identified as fine tailings.

Alluvium

- At this location, it has been assumed that there are two layers of clay alluvium with increased degrees of saturation that may be susceptible to strength reduction during a seismic event. For the purposes of the analysis, these layers are assumed to behave as saturated soils, but hydrostatic conditions (e.g. perched ground water) are not present:
 - 41.0 ft to 45.0 ft bgs
 - 54.0 ft to 68.2 ft bgs
- These zones correspond to layers of fine alluvium observed during HSA drilling of TI-B1 and are separated by a layer of coarse alluvium that is unsaturated.
- It is assumed that hydrostatic conditions do not exist within the alluvium at this site above approximately elevation 6,867 ft amsl (approximately 103 ft bgs). This is supported by observations in alluvial monitoring wells (509D, EPA23) and boreholes and free water encountered at TI-B10 and TI-B11.
- The uppermost zone of soils assumed to be nearly saturated (41.0 ft to 45.0 ft bgs) corresponds to a layer of fine alluvium observed during HSA drilling at TI-B1. This assumption is based on the:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured in this layer during the CPT.
 - Two samples from this layer that were submitted for laboratory analysis. One sample was 98 percent saturated and one sample was 100 percent saturated.
- It is assumed that the wetter fine-grained zones are separated by a layer of coarse alluvium (45.0 ft to 54.0 ft bgs) identified during HSA drilling. This assumption is based on the following:
 - A sample of this layer was submitted for laboratory analysis, the results of which indicate that the material is 74 percent saturated.
 - Dynamic pore pressures (one potential indicator of higher degrees of saturation) measured in this layer were not elevated, as they were in the overlying and underlying layers of fine alluvium.
- The lowermost zone of soils assumed to be nearly saturated (54.0 ft to 68.2 ft bgs) corresponds to a layer of fine alluvium observed during HSA drilling at TI-B1. This assumption is conservative and is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured at in this layer during the CPT.

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- No samples from this layer were submitted for laboratory analysis.
- Elevated dynamic pore pressure measurements (one potential indicator of higher degrees of saturation) were not observed below 68.2 ft bgs. Therefore, it is assumed that saturated soils do not exist from 68.2 ft to 103 ft bgs at this location.

CPT-02 and TI-B2

Tailings

- It has been assumed that saturated or nearly saturated tailings do not exist at this location.
- Dynamic pore pressures recorded by the CPT in this zone were not elevated, indicating that this zone is unsaturated.
- This assumption is supported by the degree of saturation calculated from laboratory testing results (77 percent saturation).

Alluvium

- It is assumed that the alluvium at this location is moist to wet, but is not saturated or nearly saturated above the weathered sandstone encountered at 33.5 ft bgs (approximate elevation: 6,926.5 ft amsl).
- Despite some elevated dynamic pore pressure measurements during the CPT at this location (one potential indicator of higher degrees of saturation), this assumption is based on the following:
 - No free water was encountered during HSA drilling at this location.
 - Laboratory analysis of samples from the alluvium at TI-B2 indicated that these soils are not at or near saturation (22 percent to 78 percent saturation).
 - The nearest alluvial well indicates that the static water level within the alluvium is approximately 6,867 ft amsl (approximately 93 ft bgs).

CPT-08 and TI-B8

Tailings

- It has been assumed for this analysis that there are three localized perched zones of saturation (hydrostatic conditions) within the tailings at this location.
- These zones of saturation correspond to the layers of fine tailings observed during HSA drilling:
 - 26.3 ft to 31.0 ft bgs
 - 32.5 ft to 35.0 ft bgs
 - 38.6 ft to 44.5 ft bgs
- The uppermost zone that is assumed to be saturated (26.3 ft to 31.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 26.9 ft to 31.5 ft bgs during the CPT.
 - A sample from this layer that was submitted for laboratory analysis, the results of which indicate that the material is fully saturated (100 percent saturation).
- The middle zone of assumed saturation (32.5 ft to 35.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 32.8 ft to 35.1 ft bgs during the CPT.
- The lowermost zone that is assumed to be saturated (38.6 ft to 44.5 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B8. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured in this layer during the CPT.
 - Samples from this layer that were submitted for laboratory analysis, the results of which indicate that the material is 93 percent to 99 percent saturated.
- Laboratory analysis of samples of the coarse tailings and coarse/fine tailings between the layers of fine tailings at TI-B8 indicated that these soils are unsaturated (44 percent to 67 percent saturation).

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- The dynamic pore pressure measurements taken during CPT-08 also support this pattern of nearly-saturated layers of fine tailings, interbedded with layers of coarser unsaturated tailings.
- The pore pressure dissipation test performed at 31.7 ft bgs did not reach zero, which may indicate saturated conditions; however, the material is classified as a CH (USCS) and has a fines content of 91 percent. It is likely that dynamic pore pressures generated by the CPT probe shearing the soils were very slow to dissipate, resulting in misleading measurements during the pore pressure dissipation test.

Alluvium

- It is assumed that the alluvium at this location is unsaturated. This assumption is based on the following:
 - Dynamic pore pressure measurements taken during the CPT were not elevated in the alluvium.
 - No free water was encountered during HSA drilling at this location.
 - The nearest alluvial well indicates that the static water level within the alluvium is approximately 6,867 ft amsl (approximately 109 ft bgs). Bedrock at TI-B8 was encountered at 61 ft bgs. Free water was encountered in the alluvium at B10 and B11 at about 90 ft bgs while drilling.

CPT-10 and TI-B10

Tailings

- At this location, it has been assumed for this analysis that free-draining layers of coarse tailings separate three perched zones of saturated tailings:
 - 18.9 ft to 24.4 ft bgs
 - 25.7 ft to 31.0 ft bgs
 - 33.2 ft to 44.6 ft bgs
- The uppermost zone that is assumed to be saturated (18.9 ft to 24.4 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B10. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 17.7 ft to 24.3 ft bgs during the CPT.
 - A sample from this layer was submitted for laboratory analysis, the results of which indicate that the material is 89 percent saturated.
- The uppermost and middle zones of assumed saturation are separated by a layer of unsaturated coarse tailings (24.4 ft to 25.7 ft bgs) identified during HSA drilling.
- The middle zone of assumed saturation (25.7 ft to 31.0 ft bgs) corresponds to a layer of fine tailings observed during HSA drilling at TI-B10. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured in this layer during the CPT.
 - Samples from this layer that were submitted for laboratory analysis, the results of which indicate that the material is 95 percent to 96 percent saturated.
- The middle and lowermost zones of assumed saturation are separated by a layer of unsaturated coarse tailings (31.0 ft to 33.2 ft bgs) identified during HSA drilling. A sample of this layer was submitted for laboratory analysis, the results of which indicate that the material is unsaturated (62 percent saturation).
- The lowermost zone of assumed saturation (33.2 ft to 44.6 ft bgs) spans two layers of fine tailings (33.2 ft to 36.3 ft bgs and 37.8 ft to 44.6 ft bgs) and the layer of coarse/fine tailings by which they are separated. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured during the CPT at this location to a depth of 43.8 ft bgs.
 - Laboratory analysis of samples of these tailings indicated that these soils are 95 percent to 100 percent saturated.

Alluvium

- It is assumed that the alluvium at this location is unsaturated above 90.2 ft bgs. This assumption is based on the following:

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- Dynamic pore pressure measurements taken during the CPT at this location were not elevated in the alluvium.
- A static water level was measured in TI-B10 at 90.2 ft bgs (approximately 6,883 ft amsl), while drilling and the groundwater measurements in the nearest alluvial well (509D) indicate that the static water level near the tailings impoundment is approximately 6,867 ft amsl.

CPT-11 and TI-B11

Tailings

- It has been assumed that there is a localized perched zone of saturation (hydrostatic conditions) within the tailings at this location (44.5 ft to 53.9 ft bgs).
- This assumption corresponds to a layer of fine tailings observed during HSA drilling at TI-B11 and is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 43.5 ft to 54.8 ft bgs during the CPT.
 - Laboratory analysis of samples of the fine tailings at TI-B11 indicated that these soils are 95 percent to 100 percent saturated.

Alluvium

- It is assumed that the alluvium at this location is unsaturated. This assumption is based on the following:
 - Dynamic pore pressure measurements taken during the CPT were not elevated in the alluvium.
 - Free water was not encountered in the alluvial soils (53.9 ft to 77.5 ft bgs). Free water (wet sampler bit) was encountered at approximately 90 ft bgs (approximately 6,887 ft amsl) in the weathered sandstone.
 - Laboratory analysis of alluvium samples at TI-B11 indicated that these soils are unsaturated (38 percent to 56 percent saturation).

CPT-15 and TI-B15

Tailings

- It has been assumed that there are no saturated tailings at this location. This assumption is based on the following:
 - The tailings at this location are of a coarse nature and therefore have a higher hydraulic conductivity than fine-grained tailings.
 - Results of multiple samples of tailings from this location that were submitted for laboratory analysis. Laboratory analysis indicated that the samples were unsaturated (22 percent to 58 percent saturation).

Alluvium

- At this location, it has been assumed that there are two layers of clay alluvium with increased degrees of saturation that may be susceptible to strength reduction during a seismic event. For the purposes of the analysis, these layers are assumed to behave as saturated soils, but hydrostatic conditions (e.g. perched groundwater) are not present:
 - 30.0 ft to 38.0 ft bgs
 - 45.0 ft to 50.0 ft bgs
- These zones correspond to layers of fine alluvium observed during HSA drilling of TI-B15 and are separated by a layer of coarse alluvium that is unsaturated
- It is assumed that hydrostatic conditions do not exist within the alluvium at this site above elevation approximately 6,867 ft amsl (approximately 110 ft bgs). This is supported by observations in alluvial monitoring wells and boreholes and free water encountered at TI-B10 and TI-B11.
- The uppermost zone of soils assumed to be nearly saturated (30.0 ft to 38.0 ft bgs) corresponds to a layer of fine alluvium observed during HSA drilling at TI-B15. This assumption is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured from approximately 30 ft to 38 ft bgs during the CPT.

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- Results of two samples from this layer that were submitted for laboratory analysis. One sample was 92 percent saturated and one sample was 70 percent saturated.
- It is assumed that the wetter fine-grained zones are separated by a layer of coarse alluvium (38.0 ft to 45.0 ft bgs) identified during HSA drilling. This assumption is based on the following:
 - A sample of this layer was submitted for laboratory analysis, the results of which indicate that the material is 33 percent saturated.
 - Dynamic pore pressures (one potential indicator of higher degrees of saturation) measured in this layer were not elevated, as they were in the overlying and underlying layers of fine alluvium.
- The lowermost zone of soils assumed to be nearly saturated (45.0 ft to 50.0 ft bgs) corresponds to a layer of fine alluvium observed during HSA drilling at TI-B15. This assumption is conservative and is based on the following:
 - Elevated dynamic pore pressures, one potential indicator of higher degrees of saturation, were measured at depths greater than 45 ft bgs during the CPT.
 - Results from two samples from this layer that were submitted for laboratory analysis. One sample was 93 percent saturated and one sample was 67 percent saturated.
- Despite some elevated dynamic pore pressure measurements (one potential indicator of higher degrees of saturation) below 50 ft bgs, it is assumed that nearly saturated soils do not exist between 50 ft and 110 ft bgs at this location. This assumption is based on the following:
 - A layer of coarse alluvium was observed at 50 ft to 52 ft bgs. Due to the coarse nature of this material and the water levels encountered while drilling at nearby locations during the PDS, at or below elevation 6,867 ft amsl (approximately 110 ft bgs) it is assumed that this layer is unsaturated and that the recorded dynamic pore pressures are the result of shearing induced by the penetrometer during CPT.
 - Laboratory analysis of alluvium samples from depths of 56 ft and 66 ft bgs at TI-B15 indicated that these soils were unsaturated (51 percent saturation for both samples).

CPT-23 and TI-B23

Tailings

- It has been assumed that saturated conditions do not exist within the tailings at this location. This assumption is based on the following:
 - Dynamic pore pressures recorded by the CPT in this zone were not elevated, indicating that this zone is unsaturated.
 - Laboratory analysis of a tailings sample collected from TI-B23 indicated these tailings are unsaturated (57 percent saturation).
 - The layer of fine tailings (the material most likely to have a high degree of saturation) did not exhibit positive dynamic pore pressures during the CPT.

Alluvium

- It is assumed that the moist to wet layers of fine alluvium at this location are nearly, but not completely, saturated and hydrostatic conditions are not present:
 - 16.0 ft to 20.3 ft bgs
 - 23.0 ft to 38.6 ft bgs
 - 40.0 ft to 43.0 ft bgs
- This assumption is based on the following:
 - Laboratory analysis of alluvium samples collected at this location indicated that the samples were nearly, but not completely, saturated (87 percent and 91 percent saturation).
 - Elevated dynamic pore pressure measurements, one potential indicator of higher degrees of saturation, were recorded below approximately 25 ft bgs but not in the uppermost layer of fine alluvium (16.0 ft to 20.3 ft bgs) or in the top approximately 2 feet of the middle layer of fine alluvium (23 ft bgs to 38.6 ft bgs).
 - The nearest alluvial well indicates that the static water level within the alluvium is approximately 6,867 feet amsl (approximately 92 ft bgs). Bedrock at TI-B8 was encountered at 65.5 ft bgs.

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Also, the uppermost layer of fine alluvium (16.0 ft to 20.3 ft bgs) did not exhibit positive pore pressures, despite the laboratory results indicating 91 percent saturation.

Other Stratigraphic Assumptions

- The 4.5-foot-thick ET cover will consist of two layers: (1) an erosion protection layer (14-31.5 inches thick) on top of (2) a layer of cover soil (22.5-40 inches thick). The final cover thicknesses were updated to reflect the change in cover thickness after approving use of the Jetty borrow as cover material.
- The top of the ET cover will be the same as the finished grade of the proposed repository grading plan (as shown in **Figure 1**).
- Improvement and reconditioning of the existing TDA cover and radon barrier within the footprint of the proposed repository will result in a minimum 18-inch-thick radon barrier over the general fill. The existing erosion protection material will be removed from within the proposed repository footprint during this process. The finished grade of the improved radon barrier will be equal to existing grade.
- Mine waste will be placed from the top of the radon barrier (existing grade) to the bottom of the ET cover (4.5 ft below finished grade).

Calculations

Figure 3 presents the liquefaction screening evaluation. Results of laboratory analysis on soil samples have been plotted to identify the soils as susceptible, moderately susceptible, or unsusceptible to liquefaction (Bray, et al., 2009). **Attachment E** presents the liquefaction triggering analysis calculations.

Results

The liquefaction triggering analysis evaluated the potential for liquefaction of saturated soil layers during the design seismic event, which could cause settlement of the proposed repository cover, result in damage to the existing TDA radon barrier or compromise the effectiveness of the proposed repository. A liquefaction screening evaluation was performed to identify soil layers that may be susceptible to liquefaction. Results of the liquefaction screening evaluation are presented in **Figure 3**.

Soil layers meeting both of the following criteria were evaluated by a liquefaction triggering analysis:

- A sample taken from the soil layer was identified by the liquefaction screening evaluation as susceptible or moderately susceptible to liquefaction.
- The soil layer is fully saturated, nearly saturated (greater than or equal to 85 percent saturation per NRC, 2008), or has the potential to become saturated in the future.

Results of the liquefaction triggering analysis and liquefaction induced settlement are presented in **Table 2**. The FS values were calculated as the average of the FS values calculated using each of the liquefaction triggering analysis methods. Averaging the results of two methods mitigates the effects of anomalous results and draws upon the data and analyses used to develop both methods. Of the three locations identified as susceptible to liquefaction in the liquefaction screening evaluation, only one location was found to be potentially liquefiable ($FS < 1$): TI-B10 from 33.2 ft to 44.6 ft bgs. Although only one location was found to be potentially liquefiable, two locations were evaluated for post-liquefaction consolidation settlement: TI-B8 (liquefaction analysis at location TI-B8 resulted in a FS of 1.0) and TI-B10. These post-liquefaction settlements were estimated to be approximately 0.06 ft at TI-B8 (considering CPT data – SPT data resulted in zero settlement) and approximately 0.5 ft at TI-B10, considering CPT and SPT data. These settlements fall within the range of possible vertical strains described in the methods section, and therefore seem reasonable.

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A sensitivity analysis was performed to explore how varying material properties would influence the results. Whereas the primary analysis presented herein uses the average (50th percentile) material properties (from multiple tests performed on a certain material), this sensitivity analysis uses the material property of the 30th percentile (see **Table 3**). Using the 30th percentile material properties, the results of the liquefaction analysis and post-liquefaction settlement showed little variation from the base case (50th percentile material properties) values. This is reasonable because the controlling parameters of this analysis are seismic hazard inputs, CPT/SPT values, and the liquefiable criteria outlined above.

Conclusions

Liquefaction Screening Evaluation

The liquefaction screening evaluation identified eight soil samples from four depths/locations as moderately susceptible to liquefaction (see **Figure 3**). None of the soil types encountered within or below the TDA had a significant portion of the profile identified as susceptible or moderately susceptible to liquefaction.

Liquefaction Analysis

Three of the four depths/locations identified as moderately susceptible to liquefaction were also fully saturated or nearly saturated. The minimum calculated average FS for one of these soil layers is 0.79 (calculated for a minimum thickness of 1 foot). This indicates that the layer of fine tailings from 33.2 ft to 44.6 ft bgs at TI-B10 does not meet the minimum acceptable FS criteria for liquefaction (per NRC, 2008 C.2.f). Note that the final depth of this layer will be about 54.2 ft to 65.6 ft. The soil layer at this depth and within this borehole location is susceptible to liquefaction under the design seismic event. However, the layer is at significant depth below the cover and is limited to a localized area within one of the former borrow pits. This layer is also below the pseudo-static failure surfaces identified in the slope stability analyses and is not likely to present a post-seismic slope stability concern, if it were to liquefy following the design seismic event.

Per NUREG 1620 2.4.3(8) (NRC, 2003), liquefiable material may not require mitigation “if minor liquefaction potential is identified and is evaluated to have only a localized effect that may not directly alter the stability of embankments, the effect of liquefaction is adequately accounted for in analyses of both differential and total settlement and is shown not to compromise the intended performance of the radon barrier.” Based on the results of the liquefaction analysis for the 30% Design, mitigation measures are not considered necessary with respect to the post-seismic slope stability. Adding the mine waste and cover material to the site will increase vertical stresses and depths of existing material. From a calculation perspective, this additional material would increase vertical stress and result in higher FS values. The identified liquefaction potential at the Mill Site falls into this category of minor liquefaction. Neither slope stability nor the radon barrier would be compromised due to liquefaction given the location of the potentially liquefiable material, thickness of the layer, and the number of layer.

Liquefaction Induced Settlement

There are two mechanisms by which liquefied soil may experience settlement. The first is displacement caused by lateral spreading. Certain criteria are necessary for this type of displacement to occur, and these criteria are not present at this site. Specifically, this phenomenon may occur if a liquefied layer of soil extends to a free face. Not only have the liquefiable deposits been identified as pockets of material in a localized area, they are either at a depth greater than any free surface and/or hundreds of feet away. Therefore, this type of settlement was not accounted for in this analysis.

Of the three locations identified as liquefiable in the liquefaction analysis, two locations indicated the potential for post-liquefaction consolidation settlement. Note that the liquefaction analysis at location TI-B8 resulted in a FS of 1.0, at a depth of about 52 to 59 ft. While this does not meet the criteria for liquefaction (it meets the minimum required FS against liquefaction), it was evaluated for potential settlement due to the liquefaction. These post-liquefaction settlements were estimated to be about 0.06 ft at TI-B8 (considering CPT data – SPT data resulted in zero settlement).

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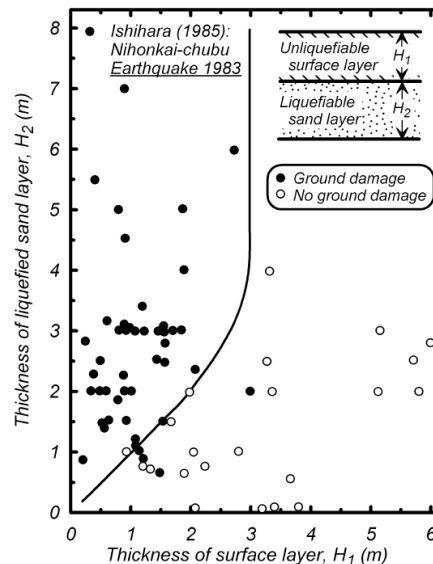
Sheet: *23* of *24*

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Job No: *10508639*

and approximately 0.5 ft at TI-B10 (considering both CPT and SPT data sets). It is unlikely that settlements of this magnitude would be observed at the ground surface, due to depth at which they occur. Per Idriss and Boulanger (2008): "The consequences of one-dimensional settlement may, however, be largely mitigated by the presence of a thick nonliquefied layer above the liquefied soils (e.g., Ishihara 1985, Naesgaard et al. 1998, Bouckovalas and Dakoulas 2007)." At depths of 52 to 59 ft and 54 to 66 ft for TI-B8 and TI-B10, respectively, there is a substantial amount of nonliquefied material above the liquefiable pockets and surficial expressions of the liquefied pockets may not be realized as shown in the figure below from Idriss and Boulanger (2008) based on historic earthquake data.



Example of nonliquefiable surface layer protecting against surface damage from case histories
(From Idriss and Boulanger 2008)

Based on the field data and the results of the calculations presented herein, the potential for liquefaction induced settlement at the site is contained in a localized area and occurs at a depth where surficial expression and damage to the radon barrier or ET cover is considered unlikely. Therefore, remedial action is not required.

Attachments

Figures

- Figure 1 – Borehole and CPT Locations for Liquefaction Triggering Analysis
- Figure 2 – One-Dimensional Stratigraphic Profiles
- Figure 3 – Liquefaction Screening Evaluation Results

Attachments

- Attachment A – Laboratory Results from Pre-Design Studies (MWH, 2014a and MWH, 2014b)
- Attachment B – Tailings Disposal Area Borehole Logs (MWH, 2014a)
- Attachment C – Tailings Disposal Area Cone Penetration Test Results (MWH, 2014a)
- Attachment D – Recorded Water Levels at the Mill Site (Chester Engineers, 2016)
- Attachment E – Liquefaction Triggering Analysis Calculations

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Description: *Liquefaction Triggering Analysis for the Mill Site Repository*
Job No: *10508639*

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TABLES

Table 1: Material Properties Used in Liquefaction Triggering Analysis

| Material Identification | Maximum Dry Unit Weight, γ_d (pcf) | Relative Compaction, C_r (% of Standard Proctor) | Specific Gravity, G_s | Void Ratio, e | Water Content, w (%) | Moist Unit Weight, γ_m (pcf) | Fines Content, FC (%) | Plasticity Index, PI |
|-------------------------|---|--|-------------------------|-----------------|------------------------|-------------------------------------|-----------------------|----------------------|
| Erosion Protection | 130.0 | 90 | 2.71 | 0.45 | 5.0 | 122.9 | N/A | N/A |
| Cover Soil | 115.0 | 90 | 2.69 | 0.62 | 10.8 | 114.7 | 53 | 12 |
| Mine Spoils | 118.3 | 90 | 2.66 | 0.56 | 9.3 | 116.4 | 53 | 12 |
| Radon Barrier | 116.3 | 95 | 2.68 | 0.51 | 9.4 | 122.3 | 59 | 16 |
| Existing Fill | N/A | N/A | 2.69 | 0.67 | 13.0 | 113.8 | 48 | 19 |
| Coarse Tailings | N/A | N/A | 2.67 | 0.71 | 10.9 | 108.1 | 21 | 0 |
| Coarse/Fine Tailings | N/A | N/A | 2.72 | 0.90 | 30.0 | 116.0 | 52 | 20 |
| Fine Tailings | N/A | N/A | 2.70 | 1.35 | 50.1 | 107.6 | 83 | 43 |
| Coarse Alluvium | N/A | N/A | 2.71 | 0.73 | 14.6 | 111.0 | 36 | N/A |
| Fine Alluvium | N/A | N/A | 2.74 | 0.72 | 21.4 | 120.7 | 76 | N/A |

Notes:

All values are the results of laboratory testing, unless otherwise noted.

Table 2: Minimum Average Factors of Safety Against Liquefaction and Predicted Settlement

| Borehole | Soil Type | Depth Range of Soil Layer ¹ (ft.) | Min. Average FS from Analysis of CPT Data | Min Average FS from Analysis of SPT Data | Post-Liq Consol. Settlement from CPT Data (ft.) | Post-Liq Consol. Settlement from SPT Data (ft.) |
|----------|---------------|--|---|--|---|---|
| TI-B1 | Fine Alluvium | 43.2 – 47.1 | 44.9 | 45.1 | 0 | 0 |
| TI-B8 | Fine Tailings | 52.8 – 58.7 | 1.01 | N/A ² | 0.06 | N/A ² |
| TI-B10 | Fine Tailings | 54.2 – 65.6 | 0.88 | 0.79 | 0.47 | 0.51 |

Notes:

¹Depth indicated is the depth considering the final proposed repository configuration

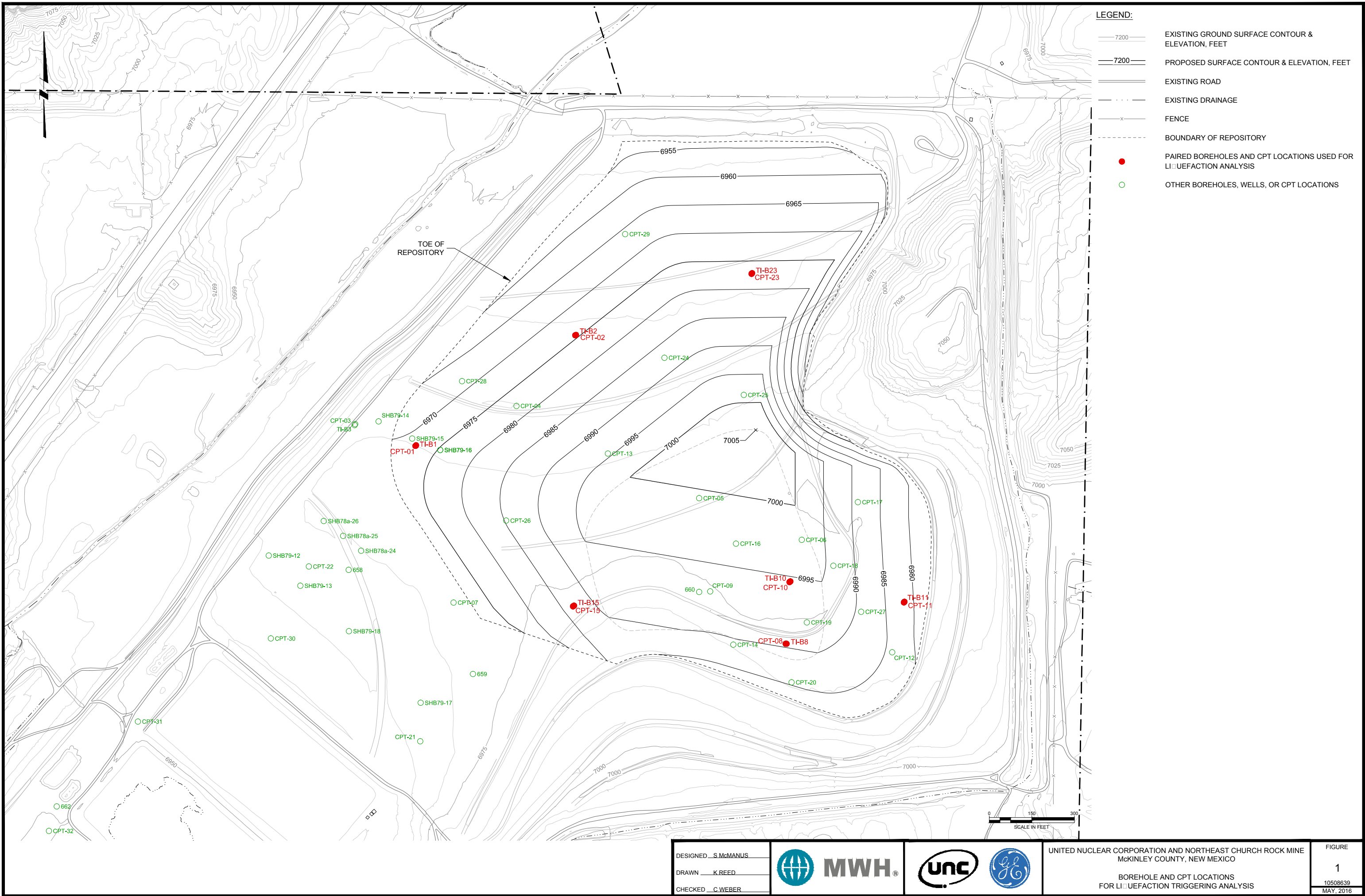
²An SPT (N = value) was not performed within this layer at TI-B8

Table 3: 30th Percentile Material Properties Used in Sensitivity Demonstration

| Material Identification | Specific Gravity, G_s | Water Content, w (%) | Moist Unit Weight, γ_m (pcf) | Fines Content, FC (%) | Plasticity Index, PI |
|-------------------------|-------------------------|------------------------|-------------------------------------|-----------------------|----------------------|
| Existing Fill | 2.67 | 8.4 | 104.4 | 38 | 19 |
| Coarse Tailings | 2.66 | 6.8 | 97.9 | 14 | 0 |
| Coarse/Fine Tailings | 2.72 | 27.6 | 109.4 | 49 | 18 |
| Fine Tailings | 2.61 | 41.6 | 94.6 | 77 | 36 |
| Coarse Alluvium | 2.68 | 11.9 | 105.8 | 30 | 0 |
| Fine Alluvium | 2.72 | 19.9 | 115.5 | 69 | 16 |

FIGURES

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| TI-B1/CPT-01 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 2.1 | Cover Soil |
| 0.0 - 2.0 | 2.1 - 4.1 | Radon Barrier |
| 2.0 - 13.0 | 4.1 - 15.1 | General Fill |
| 13.0 - 15.0 | 15.1 - 16.1 | Coarse Tailings |
| 15.0 - 18.5 | 16.1 - 20.6 | General Fill |
| 18.5 - 34.3 | 20.6 - 36.4 | Coarse Tailings |
| 34.3 - 41.1 | 36.4 - 43.2 | Coarse Alluvium |
| 41.1 - 45.0 | 43.2 - 47.1 | Fine Alluvium |
| 45.0 - 54.0 | 47.1 - 56.1 | Coarse Alluvium |
| 54.0 - 68.2 | 56.1 - 70.3 | Fine Alluvium |
| 68.2 - 70.0 | 70.3 - 72.1 | Coarse Alluvium |

| TI-B2/CPT-02 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 14.3 | Mine Spoils |
| 0.0 - 2.0 | 14.3 - 16.3 | Radon Barrier |
| 2.0 - 12.8 | 16.3 - 27.1 | General Fill |
| 12.8 - 15.0 | 27.1 - 29.3 | Fine Tailings |
| 15.0 - 25.7 | 29.3 - 40.0 | Coarse Alluvium |
| 25.7 - 33.5 | 40.0 - 47.8 | Fine Alluvium |

| TI-B8/CPT-08 | | |
|---|--|----------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 14.2 | Mine Spoils |
| 0.0 - 2.0 | 14.2 - 16.2 | Radon Barrier |
| 2.0 - 7.0 | 16.2 - 21.2 | General Fill |
| 7.0 - 18.0 | 21.2 - 32.2 | Coarse Tailings |
| 18.0 - 20.7 | 32.2 - 34.9 | General Fill |
| 20.7 - 26.3 | 34.9 - 40.5 | Coarse Tailings |
| 26.3 - 31.1 | 40.5 - 45.3 | Fine Tailings |
| 31.1 - 32.5 | 45.3 - 46.7 | Coarse Tailings |
| 32.5 - 35.0 | 46.7 - 49.2 | Fine Tailings |
| 35.0 - 38.6 | 49.2 - 52.8 | Coarse/Fine Tailings |
| 38.6 - 44.5 | 52.8 - 58.7 | Fine Tailings |
| 44.5+ | 58.7+ | Coarse Alluvium |

| TI-B10/CPT-10 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 21.0 | Mine Spoils |
| 0.0 - 2.0 | 21.0 - 23.0 | Radon Barrier |
| 2.0 - 6.8 | 23.0 - 27.8 | General Fill |
| 6.8 - 18.9 | 27.8 - 39.9 | Coarse Tailings |
| 18.9 - 24.4 | 39.9 - 45.4 | Fine Tailings |
| 24.4 - 25.7 | 45.4 - 46.7 | Coarse Tailings |
| 25.7 - 31.0 | 46.7 - 52.0 | Fine Tailings |
| 31.0 - 33.2 | 52.0 - 54.2 | Coarse Tailings |
| 33.2 - 44.6 | 54.2 - 65.6 | Fine Tailings |
| 44.6+ | 65.6+ | Coarse Alluvium |

| TI-B11/CPT-11 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 4.3 | Mine Spoils |
| 0.0 - 2.0 | 4.3 - 6.3 | Radon Barrier |
| 2.0 - 44.5 | 6.3 - 48.8 | General Fill |
| 44.5 - 53.9 | 48.8 - 58.2 | Fine Tailings |
| 53.9 - 55.0 | 58.2 - 59.3 | Fine Alluvium |
| 55.0 - 77.5 | 59.3 - 81.8 | Coarse Alluvium |

| TI-B15/CPT-15 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 5.1 | Mine Spoils |
| 0.0 - 2.0 | 5.1 - 7.1 | Radon Barrier |
| 2.0 - 3.0 | 7.1 - 8.1 | General Fill |
| 3.0 - 30.0 | 8.1 - 35.1 | Coarse Tailings |
| 30.0 - 38.0 | 35.1 - 43.1 | Fine Alluvium |
| 38.0 - 45.0 | 43.1 - 50.1 | Coarse Alluvium |
| 45.0 - 50.0 | 50.1 - 55.1 | Fine Alluvium |
| 50.0 - 52.0 | 55.1 - 57.1 | Coarse Alluvium |
| 52.0 - 65.0 | 57.1 - 70.1 | Fine Alluvium |

| TI-B23/CPT-23 | | |
|---|--|--------------------|
| Depth below Existing Ground Surface at Time of CPT (ft) | Depth below Finished Ground Surface (ft) | Soil Type |
| - | 0 - 1.5 | Erosion Protection |
| - | 1.5 - 3.5 | Cover Soil |
| - | 3.5 - 22.2 | Mine Spoils |
| 0.0 - 2.0 | 22.2 - 24.2 | Radon Barrier |
| 2.0 - 13.4 | 24.2 - 35.6 | General Fill |
| 13.4 - 14.2 | 35.6 - 36.4 | Fine Tailings |
| 14.2 - 16.0 | 36.4 - 38.2 | Coarse Tailings |
| 16.0 - 20.3 | 38.2 - 42.5 | Fine Alluvium |
| 20.3 - 23.0 | 42.5 - 45.2 | Coarse Alluvium |
| 23.0 - 38.6 | 45.2 - 60.8 | Fine Alluvium |
| 38.6 - 40.3 | 60.8 - 62.5 | Coarse Alluvium |
| 40.3 - 43.0 | 62.5 - 65.2 | Fine Alluvium |

PROJECT UNITED NUCLEAR CORPORATION AND
NORTHEAST CHURCH ROCK MINE

TITLE ONE-DIMENSIONAL STRATIGRAPHIC PROFILES

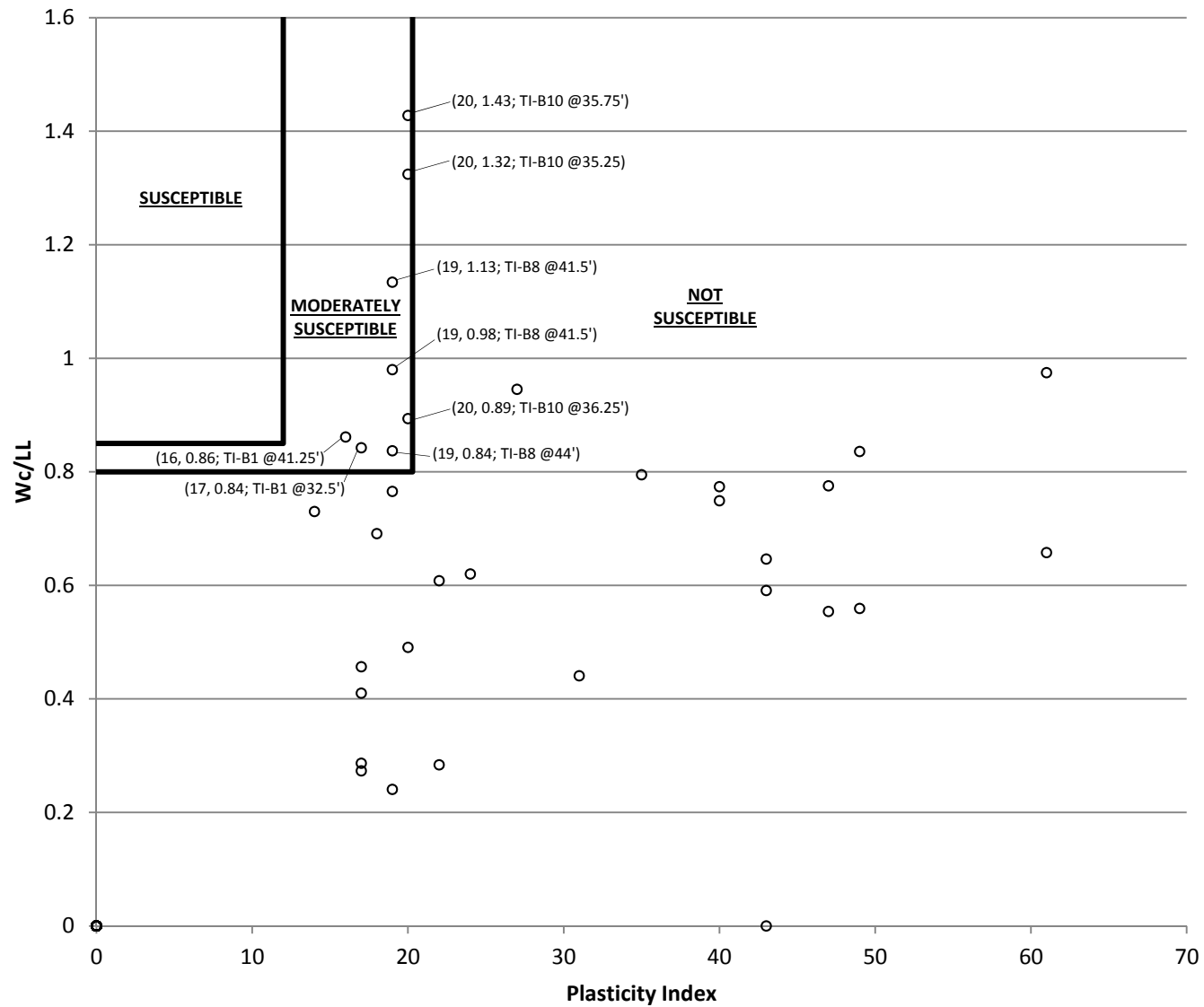



DATE May 2016

FIGURE 2

FILENAME

FIG_2



| | | | |
|---------|---|--|--|
| PROJECT | UNITED NULEAR CORPERATION AND NORTHEAST CHURCH ROCK MINE | |  MWH |
| TITLE | LIQUEFACTION SCREENING EVALUATION RESULTS | | |
| | | | FIGURE 3 |
| | | | FILENAME FIG_3 |

ATTACHMENT A

LABORATORY RESULTS FROM PRE-DESIGN STUDIES (MWH, 2014A AND MWH, 2014B)

Table 3-1 Summary of Geotechnical Laboratory Data - Cover Samples

| Cover Layer | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (in) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass) (%) | Specific Gravity | Standard Proctor (max. dd@opt. w.c.) (pcf @ %) | Atterberg Limits (%) ⁽⁵⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | L.A. Abrasion ⁽⁶⁾ (%) loss | Sodium Soundness ⁽⁷⁾ (%) loss | Absorption ⁽⁸⁾ (%) | Pinhole Dispersion ⁽⁹⁾ | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽¹⁰⁾ | | | Confining Stress (psi) | SWCC: -5 bar Water Content (by mass) (%) ⁽¹⁰⁾ | SWCC: Saturated Water Content (by mass) (%) ⁽¹¹⁾ |
|-------------------------------|-----------------|----------------------------|----------------------------|----|-------------------------------------|---------------------|------------------------------------|-----------------------------|---------------------|--|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|---------------------------------------|--|-------------------------------|-----------------------------------|--|---------|---------|------------------------|--|---|
| | | | | | | | | | | | LL | PL | PI | | | | | | | | | | 90% | 95% | 100% | | | |
| Admix. (Gravel/ Soil Mixture) | TI - CS01 - 02A | Bulk | 0 | 11 | Clayey Gravel with Sand | | Loam | | | | | | | 33.3 | 23.4 | 43.3 | 28.0 | 15.3 | | | | | | | | | | |
| | TI - CS02 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Clay Loam | | 2.81 ⁽⁴⁾ | | | | | 36.9 | 17.0 | 46.1 | 28.8 | 17.3 | 3.8 | 0.37 | 1.06 | | | | | | | |
| | TI - CS03 - 02A | Bulk | 0 | 6 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.7 | 27.7 | 18.1 | 9.6 | | | | | | | | | | |
| | TI - CS04 - 02A | Bulk | 0 | 10 | Clayey Gravel with Sand | | Loam | | | | | | | 53.6 | 18.2 | 28.2 | 18.0 | 10.2 | | | | | | | | | | |
| | TI - CS05 - 02A | Bulk | 0 | 9 | Sandy Lean Clay | | Loam | | | | | | | 13.9 | 34.4 | 51.7 | 31.2 | 20.5 | | | | | | | | | | |
| | TI - CS06 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | 2.77 ⁽⁴⁾ | | | | | 48.4 | 18.5 | 33.1 | 23.4 | 9.7 | 5.7 | 0.14 | 1.91 | | | | | | | |
| | TI - CS07 - 02A | Bulk | 0 | 20 | Sandy Lean Clay | CL | Loam | 7.8 | | | 28 | 13 | 15 | 1.1 | 41.0 | 60.9 | 42.4 | 18.5 | | | | | | | | | | |
| | TI - CS08 - 02A | Bulk | 0 | 8 | Clayey Gravel with Sand | | Loam | | | | | | | 56.7 | 18.5 | 24.8 | 17.2 | 7.6 | | | | | | | | | | |
| | TI - CS09 - 02A | Bulk | 0 | 9 | Clayey Gravel | | Loam | | 2.78 ⁽⁴⁾ | | | | | 53.6 | 14.2 | 32.2 | 21.2 | 11.0 | 5.1 | 1.17 | 1.55 | | | | | | | |
| | TI - CS10 - 02A | Bulk | 0 | 7 | Clayey Gravel with Sand | | Loam | | | | | | | 41.4 | 19.7 | 38.9 | 26.1 | 12.8 | | | | | | | | | | |
| | TI - CS11 - 02A | Bulk | 0 | 9 | Clayey Gravel with Sand | | Sandy Loam | | | | | | | 30.7 | 30.1 | 39.2 | 26.1 | 13.1 | | | | | | | | | | |
| | TI - CS12 - 02A | Bulk | 0 | 14 | Sandy Lean Clay | CL | Loam | 9.1 | | | 33 | 13 | 20 | 1.3 | 28.8 | 69.9 | 43.5 | 26.4 | | | | | | | | | | |
| Radon barrier (clay layer) | TI - CS03 - 04A | Bulk | 6 | 24 | Sandy Lean Clay | CL | Loam | 6.0 | | | 28 | 14 | 14 | 6.3 | 38.7 | 55.0 | 36.1 | 18.9 | | | | | | | | | | |
| | TI - CS06 - 04A | Bulk | 7 | 24 | Sandy Lean Clay | CL | Loam | 11.0 | | | 30 | 13 | 17 | 6.7 | 34.2 | 59.1 | 40.2 | 18.9 | | | | | | | | | | |
| | TI - CS10 - 04A | Bulk | 7 | 25 | Sandy Lean Clay | CL | Loam | 7.7 | | | 29 | 14 | 15 | 2.3 | 39.5 | 58.2 | 36.9 | 21.3 | | | | | | | | | | |
| | TI - CS08 - 04A | Bulk | 8 | 28 | Sandy Lean Clay | CL | Loam | 8.1 | 2.67 | 119.4 @ 11.9 | 27 | 12 | 15 | 11.3 | 35.0 | 53.7 | 36.7 | 17.0 | | | | | 9.1E-06 | 1.1E-05 | 1.5E-06 | 24 | | |
| | TI - CS05 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Loam | 9.6 | | | 29 | 12 | 17 | 1.3 | 37.3 | 61.4 | 42.0 | 19.4 | | | | | | | | | | |
| | TI - CS09 - 04A | Bulk | 9 | 26 | Sandy Lean Clay | CL | Loam | 7.7 | | | 28 | 13 | 15 | 4.0 | 38.1 | 57.9 | 40.0 | 17.9 | | | | | | | | | | |
| | TI - CS11 - 04A | Bulk | 9 | 24 | Sandy Lean Clay | CL | Clay Loam | 8.6 | 2.68 | 115.0 @ 14.9 | 32 | 13 | 19 | 5.1 | 28.4 | 66.5 | 40.7 | 25.8 | | | | | 7.6E-08 | 1.4E-07 | 1.0E-07 | 24 | | |
| | TI - CS02 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Sandy Clay Loam | 11.4 | | | 28 | 12 | 16 | 3.6 | 44.7 | 51.7 | 30.4 | 21.3 | | | | | | | | | | |
| | TI - CS04 - 04A | Bulk | 10 | 24 | Sandy Lean Clay | CL | Clay Loam | 15.0 | 2.68 | 113.5 @ 15.0 | 35 | 15 | 20 | 0.9 | 35.0 | 68.2 | 37.2 | 26.9 | | | | | 4.6E-06 | 6.2E-06 | 2.3E-07 | 8 | | |
| | TI - CS01 - 04A | Bulk | 11 | 24 | Sandy Lean Clay | CL | Loam | 9.2 | 2.68 | 117.3 @ 13.0 | 29 | 15 | 14 | 2.0 | 39.8 | 58.2 | 39.0 | 19.2 | | | | ND3 | 3.0E-04 | 4.6E-05 | 7.8E-07 | 8 | 8.6 / 9.6 | 21.7 / 19.0 |

- Notes:** 1. Sample Types: Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. Bulk saturated surface dry (SSD) specific gravity of the gravel fraction, average of three results (ASTM C127).
5. LL = liquid limit, PL = plastic limit, PI = plasticity index
6. L.A. abrasion results are percent loss, by mass, for 100 revolutions.
7. Weighted percent loss for the 3/4-inch to 3/8-inch size range
8. Average of three results for the gravel fraction of the cover gravel/soil mixture samples
9. Pinhole dispersion test (ASTM method A) conducted on a specimen remolded to approximately 95% of the maximum standard Proctor density at optimum water content. ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head.
10. Flexible wall permeameter tests conducted on specimens remolded to approximately 90, 95 and 100% of the maximum standard Proctor density and tested at the confining stresses shown in the table.
11. SWCC test conducted on material passing the No. 10 sieve, remolded to approximately 95% of the maximum standard Proctor density and optimum water content. SWCC tests performed with pairs of specimens for each test.

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | LL | PL | PI | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) | |
|-----------------------------|--------|----------------------------|-----------------------------|-------------|-------------------------------------|---------------------|---------------------------------|--------------------------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----|----|----|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|--|
| CENTRAL CELL | TI-B1 | CA | 16 | 16.5 | Lean Clay with Sand (Fill) | CL | 16.2 | | | | 104.7 | | | | 33 | 13 | 20 | 0.3 | 27.2 | 72.5 | 42.9 | 29.6 | | | | | | |
| | TI-B1 | CA | 20.5 | 21 | Coarse Tailings | | 6.1 | 5.7 | | | | | | | | | | | | | | | | | | | | |
| | TI-B1 | CA | 21 | 21.5 | Coarse Tailings | | 7.5 | | 21.9 / 19.8 | 96.5 / 99.6 | 105.5 | | | | | | | 0.0 | 90.7 | 9.3 | 5.5 | 3.8 | 3.7E-04 | 18 | 0.024 | | | |
| | TI-B1 | ST | 27 | 27.5 | Coarse Tailings | SP | 4.0 | | | | 97.6 | | 2.67 | | NP | | | 0.0 | 92.7 | 7.3 | 5.2 | 2.1 | 2.9E-03 | 14 | | | 34.9 | |
| | TI-B1 | CA | 30 | 30.5 | Coarse Tailings | | 13.9 | 13.5 | | | | | | | | | | | | | | | | | | | | |
| | TI-B1 | CA | 30.5 | 31 | Coarse Tailings | | 14.6 | | 29.6 / 33.8 | 84.2 / 83.6 | 91.6 | | | | | | | | | | | | | | | | | |
| | TI-B1 | CA (top) | 31 | 31.5 | Coarse Tailings | | 0.8 | 0.4 | | | | | | | | | | | | | | | | | | | | |
| | TI-B1 | CA (bottom) | 31 | 31.5 | Fine Tailings | CL | | 41.6 | | | | 76.5 | 2.68 | 2.69 | 44 | 17 | 27 | 0.0 | 30.9 | 69.1 | 54.6 | 14.5 | | | | | 33.3 | |
| | TI-B1 | CC-AC | 32 | 33 | Coarse/Fine Tailings | CL | 29.3 | 27.8 | | | | | | | 33 | 16 | 17 | 0.0 | 46.7 | 53.3 | 37.4 | 15.9 | | | | | | |
| | TI-B1 | CA | 36 | 36.5 | Clayey Sand | | 21.0 | 19.9 | 36.3 / 33.2 | 85.2 / 88.0 | 97.3 | | 2.73 | | | | | 0.0 | 62.5 | 37.5 | 32.8 | 4.7 | 1.7E-06 | 32 | 0.059 | | | |
| | TI-B1 | CA | 41 | 41.5 | Lean Clay with Sand | CL | 26.7 | | | | 98.6 | | | | 31 | 15 | 16 | 0.0 | 18.2 | 81.8 | 54.7 | 27.1 | 1.2E-07 | 35 | | | | |
| TI-B1 | ST | 45 | 46 | Clayey Sand | | 22 | 21.2 | | | 106.0 | | | | | | | | | | | | | | 0.058 | | | 34.4 | |
| CENTRAL CELL - BORROW PIT 1 | TI-B10 | ST (top) | 10 | 11 | Coarse Tailings | | 9.7 | 9.1 | | | 110 | 110.5 | 2.63 | 2.65 | | | | | | | | | | | | | | |
| | TI-B10 | ST (bottom) | 10 | 11 | Coarse Tailings | | 9.0 | | 20.7 / 21.5 | 102.6 / 101.2 | 96.8 | | | | | | | 0.2 | 71.9 | 27.9 | 16.6 | 11.3 | 4.3E-04 | 34 | 0.094 | | | |
| | TI-B10 | CC-AC ⁽⁴⁾ (top) | 12.5 | 14 | Coarse Tailings | | 6.7 | 6.3 | | | | | 2.61 | 2.64 | | | | | | | | | | | | | | |
| | TI-B10 | CC-AC ⁽⁴⁾ (bot) | 12.5 | 14 | Coarse Tailings | | 7.5 | | 31.3 / 31.4 | 85.0 / 85.0 | 99.1 | | | | | | | 0.7 | 71.5 | 27.8 | 18.9 | 8.9 | 6.7E-05 | 36 | | | | |
| | TI-B10 | CA | 15 | 15.5 | Coarse Tailings | | 9.3 | | | | 103.0 | | | | | | | | | | | | | | | | | |
| | TI-B10 | CA | 16 | 16.5 | Coarse Tailings | SM | 6.5 | | | | 100.0 | | 2.65 | | NP | | | 2.4 | 82.3 | 15.3 | 10.2 | 5.1 | | | | | | |
| | TI-B10 | ST | 21.5 | 22.5 | Coarse/Fine Tailings | CL | 28.1 | 26.7 | | | 91.9 | 92.9 | | | 43 | 19 | 24 | 0.0 | 43.0 | 57.0 | 51.4 | 5.6 | | | 0.111 | | | |
| | TI-B10 | CA | 25.75 | 26 | Fine Tailings | | 43.7 | 41.0 | | | | | | | | | | | | | | | | | | | | |
| | TI-B10 | CA | 26 | 26.5 | Fine Tailings | CH | 60.4 | 57.4 | | | 63.1 | 64.3 | 2.71 | 2.80 | 74 | 27 | 47 | 0.0 | 10.0 | 90.0 | 82.6 | 7.4 | | | | | | |
| | TI-B10 | ST | 30.3 | 30.7 | Fine Tailings | CH | 47.7 | 45.3 | | | 72.2 | 73.4 | 2.71 | 2.78 | 57 | 22 | 35 | 0.0 | 24.3 | 75.7 | 68.4 | 7.3 | | | | | | |
| | TI-B10 | ST | 32 | 32.5 | Coarse Tailings | SM | 15.4 | | | | 100.1 | | 2.67 | | NP | | | 0.0 | 83.1 | 16.9 | 12.6 | 4.3 | | | | | | |
| | TI-B10 | CA | 35 | 35.5 | Fine Tailings | | 50.2 | 47.7 | | | 71.3 | 72.5 | | | | | | | | | | | | | | | | |
| | TI-B10 | CA | 35.5 | 36 | Fine Tailings | | 54.2 | 51.4 | | | | | | | | | | | | | | | | | | | | |
| | TI-B10 | CA | 36 | 36.5 | Coarse/Fine Tailings | SC/CL | 33.9 | 32.2 | | | 86.7 | 87.8 | 2.68 | 2.72 | 36 | 16 | 20 | 0.0 | 50.6 | 49.4 | 31.1 | 18.3 | | | | | | |
| | TI-B10 | ST (top) | 40 | 41 | Fine Tailings | | 47.3 | 45.7 | | | 70.5 | 73.7 | 2.54 | 2.56 | | | | | | | | | | | | | | |
| | TI-B10 | ST (bottom) | 40 | 41 | Fine Tailings | CH | 49.7 | 47.2 | 47.7 / 55.7 | 75.3 / 67.9 | 73.3 | 74.5 | | | 61 | 21 | 40 | 0.0 | 20.7 | 79.3 | 46.5 | 32.9 | 2.9E-08 | 58 | 0.315 | | | |
| | TI-B10 | CA | 46 | 46.5 | Silty Sand | | 9.9 | | | | 95.4 | | 2.74 | | | | | 0.0 | 65.8 | 34.2 | 23.4 | 10.8 | | | | | | |
| | TI-B10 | ST | 55 | 56 | Silty Sand | | 14.1 | | 25.7 / 24.8 | 98.0 / 99.9 | 100.8 | | | | | | | | | | | | 2.4E-05 | 72 | 0.139 | | | |
| | TI-B10 | CA | 66 | 66.5 | Silty Sand / Sandy Silt | SM/ML | 13.8 | | | | 94.5 | | | | NP | | | 0.0 | 50.1 | 49.9 | 33.4 | 16.5 | | | | | | |
| | TI-B10 | CA | 71 | 71.5 | Silty Sand | | 18.1 | | | | 100.8 | | | | | | | | | | | | | | | | | |
| TI-B10 | CA | 91 | 91.5 | Clayey Sand | | 18.6 | | | | 105.6 | | 2.66 | | | | | | | | | | | | | | | | |
| TI-B10 | CC | 106.9 | 107.3 | Sandstone | | 14.2 | | | | 109.1 | | | | | | | | | | | | 1.4E-07 | 115 | | | | | |
| CENTRAL CELL - BORROW PIT 2 | TI-B11 | CA | 6 | 6.5 | Sandy Clay (Fill) | | 8.6 | | | | 93.5 | | | | | | | | | | | | | | | | | |
| | TI-B11 | ST | 15 | 16 | Clayey Sand (Fill) | | 8.2 | | 16.0 / 16.3 | 117.7 / 116.6 | 110.4 | | 2.67 | | | | | 3.9 | 57.6 | 38.5 | 24.6 | 13.9 | 2.5E-05 | 38 | 0.085 | | | |
| | TI-B11 | CA | 21 | 21.5 | Sandy Clay (Fill) | | 12.3 | | | | 107.6 | | | | | | | | | | | | | | | | | |
| | TI-B11 | ST | 30.5 | 31.5 | Sandy Clay (Fill) | CL | 13.7 | | | | 112.4 | | | | 30 | 13 | 17 | 7.1 | 41.3 | 51.6 | 33.9 | 17.7 | 9.0E-07 | 51 | 0.059 | | | |
| | TI-B11 | CA | 45.5 | 46 | Fine Tailings | | 117.2 | 88.7 | | | | | | | | | | | | | | | | | | | | |
| | TI-B11 | ST | 51.5 | 52.5 | Fine Tailings | CH | 63.0 | 59.9 | | | 62.5 | 63.7 | 2.75 | 2.84 | 91 | 30 | 61 | 0.0 | 2.7 | 97.3 | 90 | 7.3 | 3.1E-08 | 67 | 0.482 | | | |
| | TI-B11 | ST | 56 | 57 | Silty Sand | SM | 16.2 | | 31.0 / 30.8 | 90.6 / 92.8 | 77.9 | | 2.64 | | NP | | | 0.0 | 60.4 | 39.6 | 31.9 | 7.7 | 5.6E-04 | 72 | 0.129 | | | |
| | TI-B11 | CA | 61 | 61.5 | Sandy Clay | | 16.0 | | | | 95.4 | | | | | | | 0.0 | 38.7 | 61.3 | 44.1 | 17.2 | | | | | | |
| | TI-B11 | CA | 66 | 66.5 | Silty Sand | | 14.2 | | | | 96.2 | | | | | | | | | | | | | | | | | |
| | TI-B11 | CA | 81 | 81.5 | Clayey Sand with Gravel | | 11.0 | | | | 107.6 | | 2.76 | | | | | 12.9 | 65.6 | 21.5 | 9.9 | 11.6 | | | | | | |
| TI-B11 | CA | 100 | 100.2 | Sandstone | | 21.1 | | | | 103.9 | | | | | | | | | | | | 1.3E-05 | 112 | | | | | |
| CENTRAL CELL - BORROW PIT 1 | TI-B8 | CA | 25 | 25.5 | Coarse Tailings | | 9.0 | 8.4 | | | 103.7 | 104.2 | 2.72 | 2.72 | | | | | | | | | | | | | | |
| | TI-B8 | CA ⁽⁵⁾ | 25.5 | 26 | Coarse Tailings | | 6.2 | | 25.7 | 94.6 | 99.6 | | | | | | | 0.0 | 87.9 | 12.7 | 7.9 | 4.8 | 3.6E-04E | | | | | |

Table 3-4 Summary of Geotechnical Laboratory Data - Mill Site Impoundment (continued)

| Area | Boring | Sample Type ⁽⁹⁾ | Sample Depth Interval (ft.) | | Material Description ⁽¹⁾ | USCS ⁽¹⁾ | Water content (by mass, %) 110C | Water content (by mass, %) 60C | SWCC - Saturated water content (by mass, %) ⁽²⁾ | SWCC - Specimen dry density (pcf) ⁽²⁾ | Dry density (pcf), 110C | Dry density (pcf), 60C | Specific gravity, 110C | Specific gravity, 60C | Atterberg limits (%) | | | USCS % gravel (size) | USCS % sand (size) | % Passing No. 200 sieve | % Silt (size) | USDA % clay (size <0.002 mm) | Saturated Hydraulic conductivity (cm/sec) ⁽³⁾ | Hydraulic conductivity confining stress (psi) | Consolidation (Cc) ⁽⁷⁾ | Collapse potential (%) (inundation load (psf)) | Triaxial ⁽¹²⁾ (peak friction angle (φ) (degrees), cohesion (psf), where applicable) |
|----------------------|--------|----------------------------|-----------------------------|-------|-------------------------------------|---------------------|---------------------------------|--------------------------------|--|--|-------------------------|------------------------|------------------------|-----------------------|----------------------|----|----|----------------------|--------------------|-------------------------|---------------|------------------------------|--|---|-----------------------------------|--|--|
| BORROW PIT 1 (cont.) | TI-B8 | CC-AC | 44.5 | 45 | Fine Tailings | | | | | | | | 2.59 | 2.60 | | | | | | | | | | | | | |
| | TI-B8 | CA | 46 | 46.5 | Lean Clay with Sand | CL | 21.9 | | | | 95.2 | | 2.72 | | 30 | 16 | 14 | 0.0 | 27.9 | 72.1 | 55.6 | 16.5 | | | | | |
| | TI-B8 | CA | 56 | 56.5 | Silty Sand | SM | 12.6 | | | | 97.6 | | 2.70 | | NP | | | 0.0 | 57.0 | 43.0 | 30.9 | 12.1 | | | | | |
| | TI-B8 | BULK | 63.5 | 64 | Shale | | X | | | | X | | | | | | | | | | | X | X | | | | |
| CENTRAL CELL | TI-B15 | CA | 6 | 6.5 | Coarse Tailings | | 5.4 | | | | 101.1 | | | | | | | 0.0 | 87.5 | 12.5 | 9.8 | 2.7 | | | | | |
| | TI-B15 | CA | 11 | 11.5 | Coarse Tailings | | 6.8 | | | | 93.8 | | | | | | | | | | | | | | | | |
| | TI-B15 | CC-AC | 13.5 | 14 | Coarse Tailings | SM | 19.0 | 18.4 | | | | | 2.68 | | NP | | | 0.0 | 69.6 | 30.4 | 22.6 | 7.8 | | | | | |
| | TI-B15 | ST | 15.5 | 16 | Coarse Tailings | SM | 14.2 | | | | 90.4 | | 2.66 | | NP | | | 0.0 | 54.9 | 15.1 | 10.1 | 5.0 | 8.3E-04 | 38 | 0.126 | | |
| | TI-B15 | CA | 21 | 21.5 | Coarse Tailings | SM | 12.7 | | | | 99.8 | | 2.68 | | NP | | | 0.0 | 80.6 | 19.4 | 13.3 | 6.1 | | | | | |
| | TI-B15 | CC-AC | 28.5 | 29.5 | Coarse Tailings | SM | 19.3 | | | | | | 2.66 | | NP | | | 0.0 | 65.4 | 34.6 | 24.4 | 10.2 | | | | | |
| | TI-B15 | CA (top) | 31 | 31.5 | Silty Sand | | 22.3 | 21.3 | | | | | | | | | | | | | | | | | | | |
| | TI-B15 | CA (bottom) | 31 | 31.5 | Silty Sand | SM | 17.1 | | | | 101.8 | | 2.71 | | NP | | | 6.2 | 51.9 | 41.9 | 25.9 | 16.0 | | | | | |
| | TI-B15 | CA | 41 | 41.5 | Clayey Sand | | 11.4 | 10.1 | | | 87.1 | 88.1 | | | | | | | | | | | | | | | |
| | TI-B15 | CA (top) | 46 | 46.5 | Sandy Silt | | 25.8 | 24.0 | | | | | | | | | | | | | | | | | | | |
| | TI-B15 | CA (bottom) | 46 | 46.5 | Sandy Silt | ML | 17.3 | | | | 99.3 | | 2.81 | | NP | | | 0.0 | 37.0 | 63.0 | 55.7 | 7.3 | | | | | |
| | TI-B15 | CA | 56 | 56.5 | Silty Clay | | 11.7 | 10.5 | | | 104.2 | 105.3 | | | | | | | | | | | | | | | |
| | TI-B15 | CA | 66 | 66.5 | Clayey Sand | | 12.7 | 11.8 | | | 100.7 | 101.5 | | | | | | | | | | | | | | | |
| NORTH CELL | TI-B23 | ST | 15.5 | 15.75 | Coarse Tailings | | 20.7 | 19.6 | | | 87.7 | | 2.77 | | | | | 0.0 | 62.8 | 37.2 | 34.1 | 3.1 | | | | | |
| | TI-B23 | ST | 17.25 | 17.5 | Sandy Clay | | 22.5 | | | | 101.9 | | 2.73 | | | | | 0.0 | 31.1 | 68.9 | 46.5 | 22.5 | | | | | |
| | TI-B23 | ST | 26 | 27 | Lean Clay | CL | 21.6 | | | | 101.7 | | 2.73 | | 49 | 18 | 31 | 0.0 | 8.8 | 91.2 | 43.8 | 47.5 | | | 0.046 | | |
| | TI-B23 | CA | 45.2 | 45.7 | Sandstone | | 13.8 | | | | 108.7 | | | | | | | | | | | 2.4E-07 | 43 | | | | |
| | TI-B23 | CA ⁽⁸⁾ | 65.5 | 66 | Shale | | 10.2 | | | | 103.0 | | | | | | | | | | | 9.7E-08 | 62 | | | | |
| | TI-B2 | CA | 6 | 6.5 | Silty Sand with Gravel (Fill) | | 7.7 | | | | 100.4 | | 2.68 | | | | | 26.9 | 29.9 | 43.2 | 30.7 | 12.5 | | | | | |
| | TI-B2 | CA | 11 | 11.5 | Clayey Sand (fill) | | 24.5 | | | | 75.9 | | 2.73 | | | | | 0.0 | 65.4 | 34.6 | 30.3 | 4.3 | | | | | |
| | TI-B2 | CC-AC | 13.5 | 14.5 | Fine Tailings | | 41.7 | 39.6 | | | | | | | | | | 0.0 | 23.1 | 76.9 | 49.2 | 27.7 | | | | | |
| | TI-B2 | CA | 15 | 15.5 | Silty Sand | | 6.9 | | | | 90.4 | | 2.68 | | | | | | | | | | | | | | |
| | TI-B2 | CA | 21 | 21.5 | Silty Sand | | 7.0 | | | | 91.4 | | 2.74 | | | | | 0.0 | 82.9 | 17.1 | 11.5 | 5.6 | | | | | |
| | TI-B2 | CA | 26 | 26.5 | Lean Clay with Sand | CL | 23.5 | | | | 93.2 | | | | 34 | 16 | 18 | 0.0 | 20.9 | 79.1 | 51.5 | 27.6 | | | | | |
| | TI-B2 | BULK | 38.4 | 38.7 | Sandstone | | 13.5 | | | | X | | | | | | | | | | | | | | | | |
| DAM | TI-B3 | CA | 11 | 11.5 | Silty Sand (dam) | | 5.1 | | | | 108.4 | | 2.64 | | | | | 5.4 | 74.7 | 19.9 | 13.5 | 6.4 | | | | | |
| | TI-B3 | CA | 16 | 16.5 | Silty Sand (dam) | | 4.7 | | | | 105.3 | | | | | | | | | | | | | | | -2.8 (2,236) | |
| | TI-B3 | ST | 21 | 22 | Sandy Clay (dam) | CL | 16.0 | | | | 111.1 | | | | 30 | 12 | 18 | 0.0 | 32.8 | 67.2 | 41.7 | 25.5 | | | | -0.03 (2,709) | 32.2, 195 |
| | TI-B3 | CA | 26 | 26.5 | Sandy Clay (dam) | | 12.0 | | | | 106.8 | | | | 25 | 13 | 12 | | | | | | | | | | |
| | TI-B3 | CA | 31 | 31.5 | Sandy Clay (dam) | | 16.1 | | | | 108.4 | | | | | | | | | | | | | | | | |
| | TI-B3 | ST (top) | 35 | 36 | Clayey Sand (dam) | | 10.5 | 10.2 | | | | | | | | | | | | | | | | | | | |
| | TI-B3 | ST (bottom) | 35 | 36 | Clayey Sand (dam) | SC | 14.7 | | | | 102.2 | | 2.67 | | 23 | 14 | 9 | 2.1 | 50.2 | 47.7 | 30.9 | 16.8 | | | nc | -0.7 (4,608) | 33.7, 135 |
| | TI-B3 | CA | 41 | 41.5 | Sandy Clay (dam) | | 21.5 | | | | 90.6 | | | | | | | 0.0 | 33.8 | 66.2 | 41.7 | 24.5 | | | | | |
| | TI-B3 | CA | 45.5 | 46 | Sandy Clay (dam) | | 17.0 | 17.7 | | | 110.1 | 109.4 | | | | | | | | | | | | | | | 29.3, 293 |
| | TI-B3 | CA | 46 | 46.5 | Sandy Clay (dam) | | 18.0 | | | | 104.8 | | | | 28 | 13 | 15 | | | | | | | | | | |
| | TI-B3 | ST | 56 | 57 | Lean Clay | CL | 22.1 | 21.1 | | | 105.3 | 106.2 | 2.72 | | 43 | 14 | 29 | 0.0 | 11.7 | 88.3 | 48.4 | 39.9 | | | | -1.5 (7,204) | 22.2, 494 |
| | TI-B3 | CA | 61 | 61.5 | Silty Clay | | 25.8 | | | | 99.0 | | | | | | | 0.0 | 22.0 | 78.0 | 54.9 | 23.1 | | | | | |

Notes: 1. Material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available.

2. SWCC tests conducted with pairs of specimens for each test.

3. Flexible wall permeameter tests conducted at confining pressures representing confining stresses for the proposed design fill. Confining stresses were estimated as the existing overburden stress on the specimens (depth times total unit weight of material above) plus the maximum anticipated fill height for the location times the estimated unit weight of fill.

4. Specimen remolded to the in-situ water content and density of the Shelby tube sample from 10-12.5 for the SWCC.

5. Remolded SWCC and permeability tests conducted on a 50-50 mixture of the materials from these two specimens, remolded to the average measured density of the two CA samples.

6. SWCC specimen remolded to the in-situ water content and density of the Shelby tube sample from 41-42 feet.

7. Compression indices estimated using the maximum anticipated loading during fill placement and the range of loading during testing. Initial void ratios are calculated using the average specific gravity for all samples of 2.70.

8. Shale sample had multiple horizontal fractures and was likely disturbed during sampling.

9. Sample Types: CC = continuous core, CC-AC = continuous core in acrylic liner, top/bottom indicates the specimen was taken from the top or bottom of the sample interval

10. *Values in italics were calculated based on the relationship (WC60=0.951*(WC110)-.0611) between the water content results measured for 15 tailings samples at the two oven temperatures.*

11. Shaded cells are alluvium.

12. Consolidated undrained (CU) triaxial shear, staged loading of one specimen with pore pressure measurements

ST = 3" diam. Shelby tube, CA = California sample

R = remolded, nc = Cc not calculated, because fill will not be placed in this location

X = testing not possible due to sample disturbance

LL = liquid limit, PL = plastic limit, PI = plasticity index

Table 3-5 Summary of Geotechnical Laboratory Data - Borrow Areas

| Area | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (ft) | | Material Description ⁽²⁾ | USCS ⁽²⁾ | USDA Classification ⁽³⁾ | Water Content (by mass, %) | Dry Density (pcf) | Porosity | Specific Gravity | Standard Proctor (max. dd@opt. w.c.), (pcf @ %) | Atterberg Limits (%) ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | Pinhole Dispersion ^(5,6) | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽⁷⁾ | | | SWCC: -5 bar Water Content (by mass, %) ⁽⁸⁾ | | SWCC: Saturated Water Content (by mass, %) ⁽⁸⁾ | |
|--------------|------------|----------------------------|----------------------------|--------------------|-------------------------------------|---------------------|------------------------------------|----------------------------|-------------------|----------|------------------|---|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|-------------------------------------|---|---------|---------|--|-------------|---|--|
| | | | | | | | | | | | | | LL | PL | PI | | | | | | | 80% | 85% | 90% | | | | |
| West Borrow | WB-B1-01A | CA | 3.0 | 3.5 | Clayey Sand | | | 3.8 | 88.8 | 46.7 | 2.67 | | | | | | | | | | | | | | | | | |
| | WB-B1-03A | CA | 11.0 | 11.5 | Clayey Sand | SC | Sandy Loam | 6.4 | 111.0 | 33.3 | 2.67 | | 28 | 18 | 10 | 2.8 | 48.6 | 48.6 | 32.8 | 15.8 | | | | | | | | |
| | WB-B1-06 | Bulk | 5.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.64 | 112.5 @ 13.7 | 26 | 20 | 6 | 0.8 | 52.3 | 46.9 | 31.0 | 15.9 | ND3 | 7.2E-04 | 5.8E-04 | 2.1E-04 | 6.6 / 6.2 | 31.7 / 32.4 | | |
| | WB-B2-02A | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 5.6 | 87.1 | 47.8 | 2.67 | | | | | 8.6 | 53.5 | 37.9 | 23.8 | 14.1 | | | | | | | | |
| | WB-B2-05 | Bulk | 10.0 | 20.0 | Clayey Sand | SC | Sandy Loam | | | | | | 26 | 17 | 9 | 9.9 | 46.3 | 43.8 | 27.7 | 16.1 | ND3 | 8.5E-05 | 1.2E-04 | 6.4E-05 | 6.4 / 6.7 | 30.9 / 33.7 | | |
| | WB-B5-001B | CA | 3.0 | 3.5 | Clayey Sand | | | 3.7 | 92.5 | 44.3 | 2.66 | | | | | | | | | | | | | | | | | |
| | WB-B5-002A | CA | 6.0 | 6.5 | Silty, Clayey Sand | SC-SM | Sandy Loam | 5.1 | 86.9 | 47.7 | 2.66 | | 24 | 17 | 7 | 0.0 | 56.3 | 43.7 | 27.8 | 15.9 | | | | | | | | |
| WB-B5-005 | Bulk | 0.0 | 10.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | | 117.3 @ 12.7 | | | | | 0.0 | 61.6 | 38.4 | 22.8 | 15.6 | | | | | | | | |
| East Borrow | EB-B2-001A | CA | 3.0 | 3.5 | Weath. Sandstone | | | 5.8 | 107.1 | 35.8 | 2.67 | | | | | | | | | | | | | | | | | |
| | EB-B3-003B | CA | 10.5 | 11.0 | Sandy Lean Clay | CL | Sandy Loam | 6.0 | 83.1 | 50.7 | 2.70 | | 26 | 15 | 11 | 0.0 | 46.3 | 53.7 | 34.9 | 18.8 | | | | | | | | |
| | EB-B4-02A | CA | 6.0 | 6.5 | Sandy Lean Clay | CL | Sandy Loam | 5.4 | 80.7 | 51.2 | 2.65 | | | | | 0.0 | 48.5 | 51.5 | 33.9 | 17.6 | | | | | | | | |
| | EB-B4-06 | Bulk | 10.0 | 20.0 | Silty, Clayey Sand | SC-SM | Sandy Loam | | | | 2.67 | 117.1 @ 12.9 | 23 | 17 | 6 | 0.0 | 50.5 | 49.5 | 32.0 | 17.5 | ND3 | 8.7E-04 | 9.0E-04 | 4.4E-04 | 4.6 / 4.2 | 30.8 / 29.8 | | |
| | EB-B5-02B | CA | 5.5 | 6.0 | Clayey Sand | SC | Sandy Loam | 6.7 | 93.8 | 44.4 | 2.71 | | 27 | 15 | 12 | 8.8 | 45.7 | 45.5 | 28.8 | 16.7 | | | | | | | | |
| | EB-B6-01B | CA | 3.0 | 3.5 | Sandy Clay | | | 7.6 | 91.2 | 46.1 | 2.71 | | | | | | | | | | | | | | | | | |
| | EB-B6-03 | Bulk | 0.0 | 10.0 | Lean Clay with Sand | CL | Clay Loam | | | | | 114.8 @ 14.1 | | | | | 0.0 | 26.6 | 73.4 | 44.3 | 29.1 | ND3 | 2.3E-04 | 3.6E-05 | 2.9E-05 | 9.4 / 9.3 | 32.8 / 32.2 | |
| EB-B6-04A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 8.6 | 95.2 | 43.3 | 2.69 | | | 31 | 13 | 18 | 0.0 | 31.1 | 68.9 | 43.8 | 25.1 | | | | | | | | |
| South Borrow | SB-B1-01A | CA | 3.5 | 4.0 | Sandy Lean Clay | CL | Sandy Loam | 7.1 | 91.4 | 49.3 | 2.89 | | | | | 0.0 | 43.1 | 56.9 | 39.2 | 17.7 | | | | | | | | |
| | SB-B1-03A | CA | 11.0 | 11.5 | Sandy Lean Clay | CL | Sandy Clay Loam | 6.6 | 82.6 | 50.7 | 2.69 | | 31 | 15 | 16 | 0.0 | 46.7 | 53.3 | 32.9 | 20.4 | | | | | | | | |
| | SB-B1-04 | Bulk | 0.0 | 25.0 | Sandy Lean Clay | CL | Sandy Clay Loam | | | | 2.70 | 115.5 @ 14.2 | 33 | 14 | 19 | 0.0 | 42.6 | 57.4 | 30.7 | 26.7 | ND1 | 2.3E-04 | 5.7E-05 | 1.4E-04 | 6.4 / 5.9 | 31.9 / 30.3 | | |
| | SB-B2-02B | CA | 5.5 | 6.0 | Sandy Lean Clay | CL | Loam | 7.7 | 80.1 | 52.6 | 2.70 | | 36 | 15 | 21 | 0.0 | 29.8 | 70.2 | 45.4 | 24.8 | | | | | | | | |
| | SB-B3-02A | CA | 6.0 | 6.5 | Lean Clay with Sand | CL | Clay Loam | 10.2 | 84.3 | 49.7 | 2.69 | | 40 | 17 | 23 | 0.0 | 21.6 | 78.4 | 46.2 | 32.2 | | | | | | | | |
| SB-B4-01 | Bulk | 0.0 | 15.0 | Sandy Lean Clay | CL | Sandy Clay Loam | 7.1 | | | | 2.67 | 114.1 @ 14.4 | 33 | 15 | 18 | 0.8 | 39.6 | 59.6 | 35.7 | 23.9 | ND3 | 3.4E-04 | 2.0E-04 | 7.4E-05 | 9.1 / 8.6 | 29.6 / 33.5 | | |
| North Borrow | NB-B1-03B | CA | 10.5 | 11.0 | Silty Sand | SM | Sandy Loam | 5.4 | 84.4 | 49.5 | 2.68 | | 25 | 22 | 3 | 0.0 | 55.6 | 44.4 | 30.3 | 14.1 | | | | | | | | |
| | NB-B2-01B | CA | 3.0 | 3.5 | Silty Sand | SM | Sandy Loam | 4.9 | 81.9 | 50.3 | 2.64 | | 27 | 23 | 4 | 0.0 | 51.2 | 48.8 | 33.9 | 15.0 | | | | | | | | |
| | NB-B2-04 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | | | | | 113.9 @ 14.5 | 26 | 19 | 7 | 0.0 | 49.0 | 51.0 | 32.5 | 18.5 | ND3 | 4.0E-04 | 2.7E-04 | 7.5E-05 | 4.9 / 4.7 | 29.5 / 29.9 | | |
| Dilco Hill | DH-B1-01B | CA | 3.0 | 3.5 | Silty Sand | | | 3.5 | 88.8 | 46.6 | 2.66 | | | | | | | | | | | | | | | | | |
| | DH-B1-03 | Bulk | 0.0 | 10.0 | Sandy, Silty Clay | CL-ML | Sandy Loam | 5.4 | | | 2.67 | 117.5 @ 13.8 | 25 | 19 | 6 | 2.0 | 47.4 | 50.6 | 35.0 | 15.6 | ND4 | 6.3E-04 | 7.1E-04 | 2.5E-04 | 4.2 / 4.1 | 39.6 / 35.0 | | |
| | DH-B1-10 | Bulk | 35.0 | 45.0 | Lean Clay with Sand | CL | Loam | 10.3 | | | 2.38 | | | | | 1.5 | 20.9 | 77.6 | 60.9 | 16.7 | ND3 | 1.6E-04 | 2.5E-05 | 3.2E-06 | 5.8 / 6.0 | 25.7 / 24.5 | | |
| | DH-B2-03 | CA | 15.0 | 15.5 | Silty Clay with Sand | CL-ML | Sandy Loam | 10.5 | 96.7 | 39.2 | 2.55 | | 29 | 24 | 5 | 0.0 | 27.7 | 72.3 | 66.9 | 5.4 | | | | | | | | |
| | DH-B3-05 | Bulk | 20.0 | 30.0 | Sandy Lean Clay | CL | Loam | 7.3 | | | 2.66 | 116.3 @ 13.0 | 29 | 18 | 11 | 2.5 | 34.6 | 62.9 | 45.5 | 17.4 | | | | | | | | |

Notes: 1. Sample Types: CA = California sample, Bulk = bucket/grab sample
2. USCS = Unified Soil Classification Sysytem, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay
3. USDA = United States Department of Agriculture, USDA classifications are based on the sand/silt/clay fraction of the sample and on USDA grain-size designations.
4. LL = liquid limit, PL = plastic limit, PI = plasticity index
5. With the exception of DH-B1-03, which was tested at a density based on the natural in-situ density measured from the CA samples, specimens were remolded to approximately 85% of standard Proctor density and between the estimated natural and optimum water contents for the soil.
6. ND1 = nondispersive clay with very slight to no colloidal erosion under 15-inch or 40-inch head; ND4, ND3 = slightly to moderately dispersive clays that erode slowly under 2-inch or 7-inch head (ASTM test method A)
7. Specimens remolded to approximately 80%, 85%, and 90% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil.
8. Specimens remolded to approximately 85% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil. SWCC tests performed with pairs of speciments for each test.

Table 3-6 Summary of Geotechnical Laboratory Data - Site Stockpiles

| Area | Sample | Sample Type ⁽¹⁾ | Material Description | USCS ⁽²⁾ | Specific Gravity | Atterberg Limits (%) ⁽⁴⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | L.A. Abrasion (% loss) ⁽⁵⁾ | Sodium Sulfate Soundness (% loss) ⁽⁶⁾ | Absorption (%) ⁽⁷⁾ | Unconfined Compressive Strength (psi) ⁽⁸⁾ | Splitting Tensile Strength (psi) ⁽⁸⁾ |
|------------|--------------|----------------------------|------------------------------|---------------------|---------------------|--|----|----|---------------|-------------|------------------------------------|--|---|----------------------------------|--|--|
| | | | | | | LL | PL | PI | | | | | | | | |
| Stockpiles | Topsoil-01 | Bulk | Sandy Clay | CL | 2.68 | 33 | 10 | 23 | 2.6 | 32.4 | 65.0 | | | | | |
| | Topsoil-02 | Bulk | Sandy Clay | CL | 2.71 | 39 | 12 | 27 | 0.5 | 26.8 | 72.7 | | | | | |
| | TI-SP1-01 | Bulk | Crusher Fines | | | | | | 1.9 | 80.8 | 17.3 | | | | | |
| | TI-SP2-01A | Bulk | Erosion Protection Gravel | | 2.78 ⁽³⁾ | | | | 93.0 | 6.3 | 0.7 | 5.7 | 8.26 | 1.868 | | |
| | TI-SP2-01C | Bulk | Erosion Protection Gravel | | | | | | 83.3 | 4.9 | 11.8 | | | | | |
| | TI-SP3-01A | Bulk | Road Base (gravel with sand) | | | | | | 67.4 | 24.6 | 8.0 | | | | | |
| | TI-SP4-01A | Bulk | Erosion Protection Gravel | | 2.75 ⁽³⁾ | | | | 98.0 | 1.2 | 0.8 | 6.1 | 10.47 | 2.091 | | |
| | TI-SP6 (56A) | Bulk | 9-inch riprap | | | | | | | | | | | | 20,780 and 23,630 | 1,320 and 1,400 |
| | TI-SP6 (56B) | Bulk | 9-inch riprap | | | | | | | | | | | | 19,100 and 14,440 | 1,530 and 1,720 |

Notes: 1. Bulk = bucket/grab sample

2. USCS = Unified Soil Classification System, material descriptions are based on field observations, and refined with laboratory data, if available.

USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay

3. Average of three bulk saturated surface dry (SSD) specific gravity results for the rock samples (ASTM C127)

4. LL = liquid limit, PL = plastic limit, PI = plasticity index

5. L.A. Abrasion results are percent loss, by mass, for 100 revolutions.

6. Weighted percentage loss for 0.75 to 1.5-inch size range

7. Average of three absorption results

8. Specimens were collected from the 9-inch stockpile and cored for strength testing.

Table 3-6 Geotechnical Test Results

| Sample ID ¹ | Sample Location | Sample Type | Sample Depth Interval | | Gravimetric Water content | Dry Density | Specific gravity | Standard Proctor | |
|------------------------|-----------------|-----------------|-----------------------|-----------------|---------------------------|----------------|----------------------|-------------------------------------|---------------------------|
| | | Units: | top (ft bgs) | bottom (ft bgs) | (% by mass) | (pcf) | (g/cm ³) | max. dry density (pcf) ³ | optimum water content (%) |
| NECR1-CC01 | NECR-1 | Bulk | 10 | 20 | | | 2.68 | 120.7 | 11.9 |
| NECR1-CC17 | | CA ² | 5.5 | 6 | 4.9 | 92.3 | | | |
| NECR1-CC17 | | CA | 10.5 | 11 | 6.2 | 96.5 | | | |
| NECR1-CC17 | | CA | 15.5 | 16 | 2 | 106.7 | | | |
| NECR1-CC17 | | CA | 20.5 | 21 | 19.1 | 95.8 | | | |
| NECR1-CC17 | | Bulk | 0 | 10 | | | | 120.3 | 11.3 |
| NECR1-CC17 | NECR-2 | Bulk | 10 | 20 | | | | 125.1 | 10 |
| NECR2-CC05 | | Bulk | 0 | 10 | | | | 118.8 | 11.9 |
| NECR2-CC07 | | Bulk | 0 | 10 | | | 2.71 | 117.8 | 11.6 |
| NECR2-CC05 | | CA | 2.5 | 3 | 8.1 | 93.7 | | | |
| NECR2-CC05 | | CA | 5 | 5.5 | 10 | D ³ | | | |
| NECR2-CC06 | | CA | 3.5 | 4 | 4.7 | 101.1 | | | |
| NECR2-CC07 | | CA | 6 | 6.5 | 2.7 | 101 | | | |
| NECR2-CC07 | | CA | 5.5 | 6 | 4.5 | 101.3 | | | |
| NECR2-CC07 | | CA | 10 | 10.5 | 4.1 | 97.1 | | | |
| NECR2-CC01 | | CA | 5.5 | 6 | 7.4 | 99.1 | | | |
| NECR2-CC06 | NECR-2 Drainage | CA | 3 | 3.5 | 5 | 103.4 | | | |
| N2D-CC01 | | Bulk | 0 | 10 | | | | 115.6 | 13.4 |
| N2D-CC01 | | CA | 3.5 | 4 | 8.6 | 91.2 | | | |
| N2D-CC01 | | CA | 6 | 6.5 | 4.7 | 87.2 | | | |
| N2D-CC01 | NEMSA | CA | 11 | 11.5 | 4 | 91.8 | | | |
| NMSA-CC02 | | CA | 3 | 3.5 | 8.1 | 110.6 | | | |
| NMSA-CC02 | | CA | 6 | 6.5 | 20 | 97.5 | | | |
| NMSA-CC02 | | CA | 10.5 | 11 | 15 | 86.6 | | | |
| NMSA-CC04 | Pond 2 | Bulk | 0 | 15 | | | 2.66 | 125.2 | 9.8 |
| P2-CC04 | | Bulk | 0 | 3 | | | 2.66 | 102.0 | 20.6 |
| P3-CC07 | Pond 3 | Bulk | 0 | 5 | | | 2.63 | 109.7 | 13.7 |
| SF2-CC01 | Sandfill 2 | Bulk | 0 | 10 | | | 2.65 | 121.5 | 10.5 |
| SF3-CC01 | Sandfill 3 | Bulk | 0 | 10 | | | 2.68 | 121.7 | 11.1 |
| SF3-CC01 | | CA | 3.5 | 4 | 17 | 99.3 | | | |
| SF3-CC01 | | CA | 6 | 6.5 | 10.5 | 96.4 | | | |
| SF3-CC01 | | CA | 11 | 11.5 | 8.2 | 83.5 | | | |
| SP-CC13 | Sediment Pad | CA | 5.5 | 6 | 10.2 | 101.4 | | | |
| SP-CC13 | | CA | 11 | 11.5 | 3.5 | 100.8 | | | |
| SP-CC13 | | CA | 15.5 | 16 | 6.9 | 97.5 | | | |
| SP-CC13 | | Bulk | 0 | 15 | | | 2.62 | 120.6 | 11.5 |




Notes:

pcf=pounds per cubic foot

1. Samples collected October-December 2013 during the Pre-Design Studies
2. CA = 2-inch diameter California sample, Bulk = 5-gallon bucket sample
3. Maximum dry density listed includes rock correction
4. D = Disturbed, moisture content only




ATTACHMENT B

TAILINGS DISPOSAL AREA BOREHOLE LOGS (MWH, 2014A)

| | | | | | | | |
|---|------------------|---|------------|-----------------------------|-----------------|--|-----------------------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B1 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6969.7 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | FINISH: 11/21/2013 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | DEPTH TO BEDROCK (FT): N/A | |
| | | | | | | TOTAL DEPTH (FT): 70.0 | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS |
| | | | | | | | GRAPHIC |
| | | | | | | | WATER CONT. (%) |
| | | | | | | | DRY DENSITY (PCF) |
| | | | | | | | SPECIFIC GRAVITY |
| | | | | | | | ATTERBERG LIMITS (LL/PL/PI) |
| | | | | | | | % GRAVEL |
| | | | | | | | % SAND |
| | | | | | | | % FINES |
| | | | | | | | SAT. HYD. COND. (cm/s) |
| | | | | | | | CONSOLIDATION (Co) |
| | | | | | | | TRIAxIAL (PHI, C (PSF)) |
| 14" | | | | | NA | (0' - 8") SILTY CLAY (FILL) - Light brown, soft, moist silty clay, trace to few very fine to fine sand. | |
| 1 | | | | | | (8" - 12") ROCK - 1/2" to 3" crushed basalt. | |
| 2 | | | | | | (1' - 18.5') SILTY CLAY WITH SAND (FILL) - Dark brown, firm to hard, slightly moist silty clay, little to some very fine to fine sand, occasional coarse sand and gravel (upper ~5' may be compacted radon barrier). | |
| 3 | | | | | | | |
| 4 | | | | | | [0 - 5' Core not retained.] | |
| 5 | 24" | CA 18" | 1C | 8 | | | |
| 6 | | | 1B | 9 | | | |
| 7 | | AC | 2 | 11 | | | |
| 8 | | | | | | | |
| 9 | | | | | | | |
| 10 | 30" | CA 18" | 3C | 10 | | [Below ~10', occasional elevated rad readings indicating possible sand tailings mixed with silty clay fill.] | |
| 11 | | | 3B | 12 | | | |
| 12 | | | 3A | 14 | | (~11' - ~11.5') 1/2" to 1" gravel observed. | |
| 13 | | AC | 4 | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.






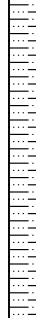
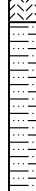
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|--|--|---------|--|--|--|---|--|------------|--|---------------------------|--|--|--|--|--|--|--|--|--|
|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | | | | | LABORATORY TEST DATA | | | | | | | | | |
| MATERIAL DESCRIPTION | | | | | | | | | | | | | | | | | | | |
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LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:

Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B1 | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|--|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|-------|------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | | |
| 28 | 30" | AC | 10 | NA | (29.6' - 34.3') Very fine grained sand tailings, abundant clayey zones. | CL |  | 13.9 | | | | | | | | | | | |
| 29 | | | | | | | | 14.6 | 91.6 | | | | | | | | 3.0E-7 | 0.092 | |
| 30 | 36" | CA 18" | 11C | 5 | | | | 41.6 | 76.5 | 2.69 | 44/17/27 | 0.0 | 30.9 | 69.1 | | | | | 33.3 |
| 31 | | | 11B | 5 | | | | | | | | | | | | | | | |
| | | | 11A | 4 | | | | | | | | | | | | | | | |
| 32 | | AC | 12 | | (34.3' - ~41') CLAYEY SAND - Loose to medium dense, very moist to wet, very fine-grained clayey sand, increasing clay content with depth. | CL |  | 27.8 | | | 33/16/17 | 0.0 | 46.7 | 53.3 | | | | | |
| 33 | | | | | | | | | | | | | | | | | | | |
| 34 | | | | | | | | | | | | | | | | | | | |
| 35 | 40" | CA 18" | 13C | 7 | | | | | | | | | | | | | | | |
| | | | 13B | 20 | (35' - 36.5') High blow counts due to rock in CA shoe. | CL |  | | | | | | | | | | | | |
| 36 | | | 13A | 22 | | | | 21.0 | 97.3 | 2.73 | | 0.0 | 62.5 | 37.5 | 1.7E-6 | 0.059 | | | |
| 37 | | AC | 14B | | | | | | | | | | | | | | | | |
| 38 | | AC | 14A | | | | | | | | | | | | | | | | |
| 39 | | | | | (41' - ~45') SANDY CLAY - Very moist to wet sandy clay, sand is very fine-grained. | CL |  | | | | | | | | | | | | |
| 40 | 42" | CA 18" | 15C | 3 | | | | | | | | | | | | | | | |
| | | | 15B | 5 | | | | | | | | | | | | | | | |
| 41 | | | 15A | 5 | | | | 26.7 | 98.6 | | 31/15/16 | 0.0 | 18.2 | 81.8 | 1.2E-7 | | | | |
| 42 | | | | | | | | | | | | | | | | | | | |




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NOTES:
Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.

Page 3 of 5




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

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | TI-B1 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B1 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | | | | | | | | | | | | | | | | | | | |
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| 59 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 60 | 60" | CA 18" | | 5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | 20B | | 8 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 61 | | 20A | | 12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 65 | 60" | CA 18" | | 5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | 21B | | 7 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 66 | | 21A | | 11 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 68 | | | | | | (68.2' - E.O.B.) SILTY SAND - Brown, silty, moist very fine to fine sand. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 70 | | | | | | E.O.B. at 70.0' | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | | | | | | | | | | | | | | NOTES: Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | | | | | | |
| Page 5 of 5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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|--|--|---------------------------------------|--|--|--|-----------------------------|--|---------------------------|--|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B2 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/20/2013 | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6959.9 | | FINISH: 11/21/2013 | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 33.5 | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 38.7 | | | |
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|  | | CLIENT: | |  | |  | | BORING LOG | | | | BOREHOLE ID: TI-B2 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | | | | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) | | |
| 40" | AC | 7 | | | | tailings, very fine silty sand from 12.8' to 13.2', fine to medium sand from 13.2' to 15'. | | | | |  | 39.6 | | | | 0.0 | 23.1 | 76.9 | | | | | |
| 14 | | | | | | | | | | | | | | | | | | | | | | | |
| 15 | 42" | CA 18" | 8C | 5 | | (15' - 25.7') SILTY SAND - Brown, medium dense, moist silty very fine to fine sand, occasional roots. Appears to be natural "alluvium." Occasional dark brown clay lenses. Rad levels ~ background. | | | | |  | 6.9 | 90.4 | 2.68 | | | | | | | | | |
| 16 | | | 8B | 5 | | | | | | | | | | | | | | | | | | | |
| 16 | | | 8A | 7 | | | | | | | | | | | | | | | | | | | |
| 17 | AC | 9 | | | | | | | | | | | | | | | | | | | | | |
| 18 | AC | 10 | | | | | | | | | | | | | | | | | | | | | |
| 19 | | | | | | | | | | | | | | | | | | | | | | | |
| 20 | 42" | CA 18" | 11C | 4 | | | | | | | | | | | | | | | | | | | |
| 21 | | | 11B | 4 | | | | | | | | | | | | | | | | | | | |
| 21 | | | 11A | 6 | | | | | | |  | 7.0 | 91.4 | 2.74 | | 0.0 | 82.9 | 17.1 | | | | | |
| 22 | AC | 12 | | | | | | | | | | | | | | | | | | | | | |
| 23 | AC | 13 | | | | | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | | | | | | | |
| 25 | 48" | CA 18" | 14C | 5 | | | | | | | | | | | | | | | | | | | |
| 26 | | | 14B | 6 | | | | | | | | | | | | | | | | | | | |
| 26 | | | 14A | 6 | | (25.7' - 33.5') SILTY CLAY - Dark brown, moist, firm to hard, silty clay, trace to few very fine to fine sand, occasional coarse sand. | | | | CL |  | 23.5 | 93.2 | | 34/16/18 | 0.0 | 20.9 | 79.1 | | | | | |
| 27 | | | | | | | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | | | | | | | | | | | | | | | | | NOTES: Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout. At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation as hole was left open overnight). | |
| Page 2 of 3 | | | | | | | | | | | | | | | | | | | | | | | |

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|---|------------------|---------------------------------------|------------|---|-----------------|---|------------|---------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B2 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 28-48" | | | | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | | |
| 30 | 54" | CA 18" | 15C | 6 | | | | | | | | | | | | | | |
| | | | 15B | 11 | | | | | | | | | | | | | | |
| 31 | | | 15A | 12 | | | | | | | | | | | | | | |
| 32 | | | | | | (32' - 33.5') Softer (soft to firm). | | | | | | | | | | | | |
| 33 | | | | | | | | | | | | | | | | | | |
| 34 | | | | | | (33.5' - 38.7') WEATHERED SANDSTONE - Mottled pale yellow and reddish orange, moist, fissile, lightly cemented, very fine to fine sand. | | | | | | | | | | | | |
| 35 | 48" | NR | | 50/1" | | | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | | |
| 39 | | | | 16 | | Bag sample of SS Core. | | | 13.5 | X | | | | | | | | |
| | | | | | | E.O.B. = 38.7' (Practical Auger Refusal) | | | | | | | | | | | | |
| 40 | | | | | | | | | | | | | | | | | | |
| 41 | | | | | | | | | | | | | | | | | | |
| 42 | | | | | | | | | | | | | | | | | | |

LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY





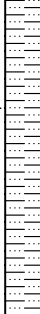
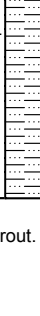
NOTES:
Rad levels measured with Ludlum Model 2 meter. Hole backfilled with cement/bentonite grout.
At 8:30 AM on 11/21/13, water was measured at 38.3' bgs (may be due to overnight precipitation as hole was left open overnight).

Page 3 of 3

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|---|------------------|--|------------|-----------------------------|-----------------|---|--|--|------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | | | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | | | | | | | | | | | | | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6968.6 | | | | | | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): N/A | | | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 70.0 | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | | | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 31" | | | | | | (0' - 0.8') SANDY CLAY FILL - Brown, hard, slightly moist sandy clay, silty, sand is fine-grained. | | | | | | | | | | | | | | |
| 1 | | | | | | (0.8' - 10.8') GRAVELLY SAND FILL - Pale yellow, dense, slightly moist gravelly very fine to medium sand. | | | | | | | | | | | | | | |
| 2 | | | | | | | | | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | | | |
| 5 | 33" | CA 7" | | 18 | | [Sample loose - not retained.] | | | | | | | | | | | | | | |
| 6 | | | | 50/6" | | | | | | | | | | | | | | | | |
| 7 | | | | | | | | | | | | | | | | | | | | |
| 8 | | | | | | | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | | | |
| 10 | 50" | CA 18" | 1C | 30 | | | | | | | | | | | | | | | | |
| 11 | | | 1B | 34 | | | | | | | | | | | | | | | | |
| 11 | | | 1A | 43 | | (10.8' - 16.8') SILTY SAND FILL - Yellow/orange, dense, moist very fine to fine sand, silty, occasional gravel. | | | | | 5.1 | 108.4 | 2.64 | | 5.4 | 74.7 | 19.9 | | | |
| 12 | | | | | | | | | | | | | | | | | | | | |
| 13 | | | | | | | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

| | | | | | | | | | | | | | | | | | | | |
|---|------------------|---------------------------------------|------------|--|-----------------|---|--|----------------------|--|-----------------|-------------------|---------------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |  | |  | | BORING LOG | | | | BOREHOLE ID: TI-B3 | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 50" | | | | | | | (16.8' - 46.5') SANDY CLAY - Dark brown, firm to hard, moist sandy clay, very fine to fine sand. | |  | 4.7 | 105.3 | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | | | |
| 15 | 38" | CA 18" | 2C | 18 | | | | | | | | | | | | | | | |
| 16 | | | 2B | 21 | | | | | | | | | | | | | | | |
| 16 | | | 2A | 28 | | | | | | | | | | | | | | | |
| 17 | | | | | | | | | | | | | | | | | | | |
| 18 | | | | | | | | | | | | | | | | | | | |
| 19 | | | | | | | | | | | | | | | | | | | |
| 20 | 50" | ST 28.5' | 3 | | | | | | | | | | | | | | | | |
| 21 | | | | | | | | CL |  | 16.0 | 111.1 | | 30/12/18 | 0.0 | 32.8 | 67.2 | | | 32.2, 195 |
| 22 | | | | | | | | | | | | | | | | | | | |
| 23 | | | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | | | |
| 25 | 52" | CA 18" | 4C | 10 | | | | | | | | | | | | | | | |
| | | | 4B | 12 | | | | | | | | | | | | | | | |
| 26 | | | 4A | 16 | | | | CL |  | 12.0 | 106.8 | | 25/13/12 | | | | | | |
| 27 | | | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)

ST = SHELBY TUBE (3-INCH OD)

AC = ACRYLIC LINER

HSA = HOLLOW-STEM AUGER

CC = CONTINUOUS CORE

NR = NO RECOVERY

NOTES:

Hole backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 2 of 5




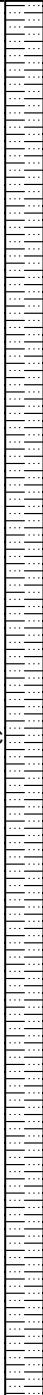
LEGEND:




- CA = CALIFORNIA SAMPLE (2-INCH OD)
- ST = SHELBY TUBE (3-INCH OD)
- AC = ACRYLIC LINER
- HSA = HOLLOW-STEM AUGER
- CC = CONTINUOUS CORE
- NR = NO RECOVERY




NOTES:





Hole backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.

Page 2 of 5

| | | | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|--|----------------------|---|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|--|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | | |
| 28-52" | | | | | | (31.5' - 36') More sand: Sandy/Silty Clay. | SC |  | 16.1 | 108.4 | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | | | | |
| 30 | 58" | CA 18" | 5C | 10 | | | | | | | | | | | | | | | | |
| | | | 5B | 10 | | | | | | | | | | | | | | | | |
| 31 | | | 5A | 19 | | | | | | | | | | | | | | | | |
| 32 | | | | | | | | | | | | | | | | | | | | |
| 33 | | | | | | | | | | | | | | | | | | | | |
| 34 | | | | | | | | | | | | | | | | | | | | |
| 35 | 48" | ST 28" | 6 | | | | | | 10.5 | | | | | | | | | | | |
| 36 | | | | | | | | | 14.7 | 102.2 | 2.67 | 23/14/9 | 2.1 | 50.2 | 47.7 | | | 33.7, 135 | | |
| 37 | | | | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | | | | |
| 40 | 57" | CA 18" | 7C | 3 | | (~40') Becomes very moist to wet. | | | | | | | | | | | | | | |
| | | | 7B | 7 | | | | | | | | | | | | | | | | |
| 41 | | | 7A | 6 | | | | | 21.5 | 90.6 | | | 0.0 | 33.8 | 66.2 | | | | | |
| 42 | | | | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY CORE</div> <div>NOTES: Hole backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.</div> | | | | | | | | | | | | | | | | | | | | |
| Page 3 of 5 | | | | | | | | | | | | | | | | | | | | |

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|---|------------------|---------------------------------------|------------|--|---|------------|----------------------|---------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B3 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 57" | | | | | | | | | | | | | | | | | |
| 43- | | | | | | | | | | | | | | | | | |
| 44- | | | | | | | | | | | | | | | | | |
| 45- | 48" | CA 17" | 6 | | | | | | | | | | | | | | |
| | | 8B | 7 | | | | | 17.0 | 110.1 | | | | | | | | |
| 46- | | 8A | 12 | | | | | 18.0 | 104.8 | | 28/13/15 | | | | | | 29.3, 293 |
| 47- | | | | | (46.5' - ~55') SILTY/CLAYEY SAND - Brown, loose, very moist to wet, silty/clayey very fine sand. | | | | | | | | | | | | |
| 48- | | | | | | | | | | | | | | | | | |
| 49- | | | | | | | | | | | | | | | | | |
| 50- | 27" | CA 17" | 2 | | ["B" and "C" samples are best.] | | | | | | | | | | | | |
| | | 9B | 3 | | | | | | | | | | | | | | |
| 51- | | 9A | 6 | | | | | | | | | | | | | | |
| 52- | | | | | | | | | | | | | | | | | |
| 53- | | | | | | | | | | | | | | | | | |
| 54- | | | | | | | | | | | | | | | | | |
| 55- | 30" | ST 24" | 10 | | (~55' - 57.3') SILTY CLAY - Dark brown, firm to hard, wet silty clay, few to little very fine sand. | CL | | 22.1 | 105.3 | 2.72 | 43/14/29 | 0.0 | 11.7 | 88.3 | | | 22.2, 494 |
| 56- | | | | | | | | | | | | | | | | | |
| 57- | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout. At 7:45 AM on 11/20/14, water was measured at 65.8' bgs.</div> | | | | | | | | | | | | | | | | | |
| Page 4 of 5 | | | | | | | | | | | | | | | | | |




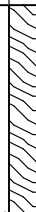

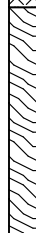


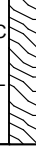
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B3 | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | |
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|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | | DRILL RIG INFORMATION | | | BOREHOLE INFORMATION | | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 12/3/2013 | | | | | | | | | | |
| DRILLER: M. CAIN | | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6976.1 | | FINISH: 12/4/2013 | | | | | | | | | | |
| DRILLER'S HELPER: L. ALDAZ | | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 60.5 | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 65.5 | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | | | | | | | LABORATORY TEST DATA | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C (PSF)) | |
| | | | | | | | | | | | | | | | | | | | |
| 1 2 3 4 5 6 7 8 9 10 11 12 13 | | | | | | (0' - ~7') SANDY CLAY - Dark brown, slightly moist sandy clay, silty, sand is very fine to fine-grained, occasional coarse sand and fine gravel. (0' - 20' No sampling. Material descriptions based on cuttings and should be considered approximate.) | |  | | | | | | | | | | | |
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LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:
Hole backfilled with cement/bentonite grout.

Page 1 of 5

|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | | | |
|--|------------------|--|------------|----------------------|---|---------------------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28 | 54" | AC | 3A | | (28.8' - 31') Pale gray, no sand. | CH |  | 61.8 | 62.7 | | 74/25/49 | 0.0 | 9.2 | 90.8 | | | 0.43 |
| 29 | | | | | | | | | | | | | | | | | |
| 30 | 24" | ST 23" | 4 | | (~31' - ~32.5') SAND TAILINGS - Pale yellowish brown, medium dense, moist, fine to medium sand, trace silt. | |  | 41.4 | | | | | | | | | |
| 31 | | | | | | | | | | | | | | | | | |
| 32 | | | | | (~32.5' - 35') FINE TAILINGS WITH SAND - Pale gray, soft, moist, very fine to fine sand. | |  | | | | | | | | | | |
| 33 | | AC | 5 | | | | | | | | | | | | | | |
| 34 | | | | | (35' - 38.6') CLAYEY/SILTY SAND TAILINGS - Pale yellowish gray, soft, moist, very fine to fine sand. | |  | 14.3 | 90.9 | 2.66 | | | | | 1.6E-5 | | |
| 35 | 30" | ST 28" | 6 | | | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | |
| 38 | | AC | 7 | | (38.6' - 44.5') FINE TAILINGS - Pale gray, firm, moist, trace to few very fine sand. | |  | 16.5 | 89.6 | 2.67 | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | |
| 40 | 37" | ST 27" | 8 | | | | | | | | | | | | | | |
| 41 | | | | | | | | | | | | | | | | | |
| 42 | | | | | | SC / CL |  | 39.7 | 80.4 | 2.63 | | | | | | | |
| | | | | | | | | 34.3 | 83.6 | | 35/16/19 | 0.0 | 51.2 | 48.8 | 1.3E-7 | 0.262 | |




LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:
Hole backfilled with cement/bentonite grout.




Page 3 of 5




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
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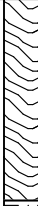

NOTES:
 Hole backfilled with cement/bentonite grout.






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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B8 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | |
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|  PROJ. LOC.: GALLUP, NM | | CLIENT:  NECR - PRE DESIGN STUDY INVESTIGATION | |  F.S. 824-001 Gallup, New Mexico 87301-0011 | | BORING LOG | | BOREHOLE ID: TI-B8 | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 58 | | | | | (58.7' - 59.5') Silty clay with sand. | | | | | | | | | | | | |
| 59 | | | | | (59.5' - 60') Reddish brown, fine to medium sand. | | | | | | | | | | | | |
| 60 | 48" | CA 18" | 13C | 16 | | | | | | | | | | | | | |
| | | | 13B | 22 | (60.5' - 61') COAL - sandy. | | | | | | | | | | | | |
| 61 | | | 13A | 50/ 4" | (61' - E.O.B.) SHALE - Dark grayish brown, hard to very hard, moist, silty, trace very fine sand. | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | |
| | | | | 14 | (bagged core) | | | | | | | | | | | | |
| 64 | 12" | | | | At 64' - becomes fissile, very hard, brittle, more sand (few to little). | | | | | | | | | | | | |
| 65 | | CA 2" | 15 | 50/ 2" | 65.2' E.O.B. (Practical Auger Refusal at 65.0') | | | | | | | | | | | | |
| 66 | | | | | | | | | | | | | | | | | |
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| 70 | | | | | | | | | | | | | | | | | |
| 71 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | | | BOREHOLE INFORMATION | | | | | | | | | | | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | | START: 11/26/2013 | | | | | | | | | | | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6973.3 | | FINISH: 11/27/2013 | | | | | | | | | | | |
| DRILLER'S HELPER: J. RAMIREZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 105.0 | | | | | | | | | | | | | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 108.2 | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 21" | | | | | | (0' - 0.6') SANDY CLAY - Light brown, soft, very moist sandy clay, very fine sand, some roots, silty. | | | | | | | | | | | | | |
| 1 | | | | | | (0.6' - 0.9') ROCK - 1/2" to 3" crushed basalt. | | | | | | | | | | | | | |
| 2 | | | | | | (0.9' - 6.8') SANDY CLAY - Dark brown, hard, slightly moist to moist sandy clay, very fine to fine sand, occasional coarse sand to fine gravel, silty. | | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | | | | | | | | |
| 5 | 45" | CA 17" | | 12 | | | | | | | | | | | | | | | |
| 6 | | 1B | | 13 | | | | | | | | | | | | | | | |
| 7 | | 1A | | 19 | | | | | | | | | | | | | | | |
| 8 | | | | | | | | | | | | | | | | | | | |
| 9 | | | | | | | | | | | | | | | | | | | |
| 10 | 33" | ST 27.5" | 2 | | | (6.8' - 18.9') SILTY SAND TAILINGS - Pale yellowish gray, loose to medium dense, moist, fine to medium, silty sand tailings, occasional more clayey/silty zones. | | | | 9.7 | 110.0 | 2.63 | | | | | | | |
| 11 | | | | | | | | | | 9.0 | 96.8 | | | 0.2 | 71.9 | 27.9 | 4.3E-4 | 0.094 | |
| 12 | | | | | | | | | | | | | | | | | | | |
| 13 | | AC | 3 | | | | | | | 6.7 | | 2.61 | | | | | | | |
| | | | | | | | | | | 7.5 | 99.1 | | | 0.7 | 71.5 | 27.8 | 6.7E-5 | | |
| LEGEND: | | | | | | NOTES: | | | | | | | | | | | | | |
| CA = CALIFORNIA SAMPLE (2-INCH OD) | | | | | | Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | |
| ST = SHELBY TUBE (3-INCH OD) | | | | | | | | | | | | | | | | | | | |
| AC = ACRYLIC LINER | | | | | | | | | | | | | | | | | | | |
| HSA = HOLLOW-STEM AUGER | | | | | | | | | | | | | | | | | | | |
| CC = CONTINUOUS CORE | | | | | | | | | | | | | | | | | | | |
| NR = NO RECOVERY | | | | | | | | | | | | | | | | | | | |
| Page 1 of 8 | | | | | | | | | | | | | | | | | | | |

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|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B10 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) | |
| 43 | 30" | ST 30" AC | 14 15 | | | (44.3' - 44.6') Appears finer grained (clayey), lighter gray, more moist. (44.6' - 85.5') SILTY SAND - Light brown, medium dense, moist, silty very fine to fine sand, occasional coarse sand and fine gravel. | |  | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | | | |
| 45 | 42" | CA 17" | 12 | 16B 12 | | | | | | 9.9 | 95.4 | 2.74 | | 0.0 | 65.8 | 34.2 | | | |
| 46 | | | | 16A 14 | | | | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | | | |
| 50 | 48" | CA 18" | 10 | 17B 11 | | | | | | | | | | | | | | | |
| 51 | | | | 17A 14 | | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | | | |
| 55 | 42" | ST 17" | 18 | | | (Shelby Tube refusal at 56.5') | |  | 14.1 | 100.8 | | | | | | 2.4E-5 | 0.139 | | |
| 56 | | | | | | | | | | | | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | |
| Page 4 of 8 | | | | | | | | | | | | | | | | | | | |

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|--|------------------|---------------------------------------|------------|--|--|---|---|----------------------|-------------------|----------------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | (62.5' - 65.2') Weathered Sandstone (?) - Hard, moist, gravelly. | SM / ML |  | 13.8 | 94.5 | NP | 0.0 | 50.1 | 49.9 | | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | | | | | | | | | | | | | |
| 60" | 39" | CA 18" | 11 | | | | | | | | | | | | | | |
| | | 19B | 11 | | | | | | | | | | | | | | |
| 61" | | 19A | 14 | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 48" | CA 18" | 14 | | | | | | | | | | | | | | |
| | | 20B | 14 | | | | | | | | | | | | | | |
| 66" | | 20A | 15 | | | | | | | | | | | | | | |
| 67" | | | | | (70.5' - 71.5') Moist to very moist, increased clay. | |  | 18.1 | 100.8 | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | 30" | CA 18" | 4 | | | | | | | | | | | | | | |
| | | 21B | 6 | | | | | | | | | | | | | | |
| 71" | | 21A | 10 | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:




Hole backfilled with cement/bentonite grout.




Page 5 of 8

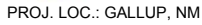
LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|---------|-------------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 72-30" | | | | | | | | | | | | | | | | | |
| 73- | | | | | | | | | | | | | | | | | |
| 74- | | | | | | | | | | | | | | | | | |
| 75-42" | CA 18" | | 5 | | | | | | | | | | | | | | |
| | | 22B | 7 | | | | | | | | | | | | | | |
| 76- | | 22A | 11 | | | | | | | | | | | | | | |
| 77- | | | | | | | | | | | | | | | | | |
| 78- | | | | | | | | | | | | | | | | | |
| 79- | | | | | | | | | | | | | | | | | |
| 80-36" | CA 18" | | 9 | | (80' - 82') Gravelly (sandstone fragments) | | | | | | | | | | | | |
| | | 23B | 14 | | | | | | | | | | | | | | |
| 81- | | 23A | 17 | | | | | | | | | | | | | | |
| 82- | | | | | (82' - 85.5') Weathered Sandstone - Mottled red/gray/brown, moist, fine to medium weathered sandstone. | | | | | | | | | | | | |
| 83- | | | | | | | | | | | | | | | | | |
| 84-NA | 3" | 24 | 50/3" | | | | | | | | | | | | | | |
| 85-50" | | | | | | | | | | | | | | | | | |
| 86- | | | | | (85.5' - 105') CLAYEY SAND - Dark brown, firm, very moist to wet, fine to medium clayey sand, occasional sandstone fragments. | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| Page 6 of 8 | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | |
|--|------------------|---|------------|-----------------|---|-------------------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  PROJ. LOC.: GALLUP, NM | | CLIENT:   NECR - PRE DESIGN STUDY INVESTIGATION | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 50" | | | | | | | | | | | | | | | | | |
| 87 | | | | | | | | | | | | | | | | | |
| 88 | | | | | | | | | | | | | | | | | |
| 89 | | | | | | | | | | | | | | | | | |
| 90 | 40" | CA 18" | 7 | | [CA sampler wet 11/26/13.] [Water measured at approximately 90.2' bgs at 9:30 11/27/13.] | | | | | | | | | | | | |
| | | 25B | 12 | | | | | | | | | | | | | | |
| 91 | | 25A | 10 | | | | | 18.6 | 105.6 | 2.66 | | | | | | | |
| 92 | | | | | | | | | | | | | | | | | |
| 93 | | | | | [Core barrel wet 11/27/13.] | | | | | | | | | | | | |
| 94 | | | | | | | | | | | | | | | | | |
| 95 | 52" | NR | 1 | | | | | | | | | | | | | | |
| | | | 5 | | | | | | | | | | | | | | |
| 96 | | | 8 | | | | | | | | | | | | | | |
| 97 | | | | | | | | | | | | | | | | | |
| 98 | | | | | [Core barrel wet.] | | | | | | | | | | | | |
| 99 | | | | | | | | | | | | | | | | | |
| 100 | 44" | | | | | | | | | | | | | | | | |
| 101 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|--|----------------------|-------------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B10 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 44" | | | | | | | | | | | | | | | | | | |
| 102 | | | | | | [Core barrel wet.] | | | | | | | | | | | | |
| 103 | | | | | | | | | | | | | | | | | | |
| 104 | | | | | | | | | | | | | | | | | | |
| 105 | 36" | | | | | (105' - E.O.B.) WEATHERED SANDSTONE - Pale yellowish brown, very dense, very moist, very fine to fine sandstone, some cemented zones. | | | | | | | | | | | | |
| 106 | | | | | | [Core barrel wet.] | | | | | | | | | | | | |
| 107 | | | | | 26 | (106.9' - 107.3') Bagged core sample. | | | 14.2 | 109.1 | | | | | | 1.4E-7 | | |
| 108 | 1" | | | 50/ 1.5" | 27 | (107.9' - 108') Bagged core sample. (108') CA sample not retained. E.O.B. = 108.2 ft at 9:00 on 11/27/13 (practical auger refusal) | | | | | | | | | | | | |
| 109 | | | | | | | | | | | | | | | | | | |
| 110 | | | | | | | | | | | | | | | | | | |
| 111 | | | | | | | | | | | | | | | | | | |
| 112 | | | | | | | | | | | | | | | | | | |
| 113 | | | | | | | | | | | | | | | | | | |
| 114 | | | | | | | | | | | | | | | | | | |
| 115 | | | | | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | |
| Page 8 of 8 | | | | | | | | | | | | | | | | | | |



BORING LOG

TI-B11

FIELD SAMPLE RECOVERY DATA

LABORATORY TEST DATA

USCS CLASS

GRAPHIC

WATER CONT. (%)

DRY DENSITY (PCF)

SPECIFIC GRAVITY

ATTERBERG LIMITS

(11/PL/PI)

| % OAK | % GRASS |
|-------|---------|
| 0 | 100 |
| 10 | 90 |
| 20 | 80 |
| 30 | 70 |
| 40 | 60 |
| 50 | 50 |
| 60 | 40 |
| 70 | 30 |
| 80 | 20 |
| 90 | 10 |
| 100 | 0 |




| | % FINES |
|------------|---------|
| 0-75 μm | 86.9 |
| 75-150 μm | 10.1 |
| 150-300 μm | 2.0 |
| 300-600 μm | 0.6 |
| >600 μm | 0.4 |






SAT HYD COND (cm/s)




| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 | 91 | 92 | 93 | 94 | 95 | 96 | 97 | 98 | 99 | 100 | 101 | 102 | 103 | 104 | 105 | 106 | 107 | 108 | 109 | 110 | 111 | 112 | 113 | 114 | 115 | 116 | 117 | 118 | 119 | 120 | 121 | 122 | 123 | 124 | 125 | 126 | 127 | 128 | 129 | 130 | 131 | 132 | 133 | 134 | 135 | 136 | 137 | 138 | 139 | 140 | 141 | 142 | 143 | 144 | 145 | 146 | 147 | 148 | 149 | 150 | 151 | 152 | 153 | 154 | 155 | 156 | 157 | 158 | 159 | 160 | 161 | 162 | 163 | 164 | 165 | 166 | 167 | 168 | 169 | 170 | 171 | 172 | 173 | 174 | 175 | 176 | 177 | 178 | 179 | 180 | 181 | 182 | 183 | 184 | 185 | 186 | 187 | 188 | 189 | 190 | 191 | 192 | 193 | 194 | 195 | 196 | 197 | 198 | 199 | 200 | 201 | 202 | 203 | 204 | 205 | 206 | 207 | 208 | 209 | 210 | 211 | 212 | 213 | 214 | 215 | 216 | 217 | 218 | 219 | 220 | 221 | 222 | 223 | 224 | 225 | 226 | 227 | 228 | 229 | 230 | 231 | 232 | 233 | 234 | 235 | 236 | 237 | 238 | 239 | 240 | 241 | 242 | 243 | 244 | 245 | 246 | 247 | 248 | 249 | 250 | 251 | 252 | 253 | 254 | 255 | 256 | 257 | 258 | 259 | 260 | 261 | 262 | 263 | 264 | 265 | 266 | 267 | 268 | 269 | 270 | 271 | 272 | 273 | 274 | 275 | 276 | 277 | 278 | 279 | 280 | 281 | 282 | 283 | 284 | 285 | 286 | 287 | 288 | 289 | 290 | 291 | 292 | 293 | 294 | 295 | 296 | 297 | 298 | 299 | 300 | 301 | 302 | 303 | 304 | 305 | 306 | 307 | 308 | 309 | 310 | 311 | 312 | 313 | 314 | 315 | 316 | 317 | 318 | 319 | 320 | 321 | 322 | 323 | 324 | 325 | 326 | 327 | 328 | 329 | 330 | 331 | 332 | 333 | 334 | 335 | 336 | 337 | 338 | 339 | 340 | 341 | 342 | 343 | 344 | 345 | 346 | 347 | 348 | 349 | 350 | 351 | 352 | 353 | 354 | 355 | 356 | 357 | 358 | 359 | 360 | 361 | 362 | 363 | 364 | 365 | 366 | 367 | 368 | 369 | 370 | 371 | 372 | 373 | 374 | 375 | 376 | 377 | 378 | 379 | 380 | 381 | 382 | 383 | 384 | 385 | 386 | 387 | 388 | 389 | 390 | 391 | 392 | 393 | 394 | 395 | 396 | 397 | 398 | 399 | 400 | 401 | 402 | 403 | 404 | 405 | 406 | 407 | 408 | 409 | 410 | 411 | 412 | 413 | 414 | 415 | 416 | 417 | 418 | 419 | 420 | 421 | 422 | 423 | 424 | 425 | 426 | 427 | 428 | 429 | 430 | 431 | 432 | 433 | 434 | 435 | 436 | 437 | 438 | 439 | 440 | 441 | 442 | 443 | 444 | 445 | 446 | 447 | 448 | 449 | 450 | 451 | 452 | 453 | 454 | 455 | 456 | 457 | 458 | 459 | 460 | 461 | 462 | 463 | 464 | 465 | 466 |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

LEGEND:
CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:
Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|---|----------------------|---------|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | | | | | | | | | | | | |
| 14 | | | | | | | | | | | | | | | | | |
| 15 | 45" | ST 13" | 3 | | (15' - 18') CLAYEY SAND - Light yellowish brown, medium dense, slightly moist, fine to medium clayey sand, occasional gravel up to 1". | | | 8.2 | 110.4 | 2.67 | | 3.9 | 57.6 | 38.5 | 2.5E-5 | 0.09 | |
| 16 | | | | | | | | | | | | | | | | | |
| 17 | | | | | | | | | | | | | | | | | |
| 18 | | | | | (18' - 32.9') SANDY CLAY - Predominantly dark brown, hard, slightly moist sandy clay, silty, very fine to medium sand, few to little coarse sand and gravel up to ~1" size. | | | | | | | | | | | | |
| 19 | | | | | (19.2' - 19.4') Sand, very fine to fine. | | | | | | | | | | | | |
| 20 | 48" | CA 18" | 4C | 4 | | | | | | | | | | | | | |
| | | | 4B | 7 | | | | | | | | | | | | | |
| 21 | | | 4A | 10 | | | | 12.3 | 107.6 | | | | | | | | |
| 22 | | | | | | | | | | | | | | | | | |
| 23 | | | | | | | | | | | | | | | | | |
| 24 | | | | | | | | | | | | | | | | | |
| 25 | 56" | CA 18" | 5C | 7 | | | | | | | | | | | | | |
| | | | 5B | 8 | | | | | | | | | | | | | |
| 26 | | | 5A | 13 | | | | | | | | | | | | | |
| 27 | | | | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 2 of 8 | | | | | | | | | | | | | | | | | |

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|--|------------------|---------------------------------------|------------|---|-----------------|---|------------|---|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 28.56" | | | | | | | | | | | | | | | | | | |
| 29 | | | | | | | | | | | | | | | | | | |
| 30.47" | ST 21" | 6 | | | | | CL |  | 13.7 | 112.4 | | 30/13/17 | 7.1 | 41.3 | 51.6 | 9.0E-7 | 0.06 | |
| 31 | | | | | | | | | | | | | | | | | | |
| 32 | | | | | | | | | | | | | | | | | | |
| 33 | | | | | | (32.9' - 34') SAND (TAILINGS?) - Pale yellowish gray, slightly moist, fine to medium sand. | |  | | | | | | | | | | |
| 34 | NA | | | | | (34' - 45.5') SANDY CLAY WITH GRAVEL - Dark brown, firm to hard, moist sandy clay with very fine to coarse sand and gravel up to ~3", some metallic and fibrous debris. | | | | | | | | | | | | |
| 35 | | | | | | [34' - 38' Drilling through metallic debris (appears to be metal siding). Center bit required to penetrate debris. No core collected. CA sample attempted at 34' and 35' - no penetration or recovery.] | | | | | | | | | | | | |
| 36 | | | | | | | | | | | | | | | | | | |
| 37 | | | | | | | | | | | | | | | | | | |
| 38 | | | | | | | | | | | | | | | | | | |
| 39 | | | | | | | | | | | | | | | | | | |
| 40.51" | CA 3" | | | 25 | | [Metallic debris in CA shoe - no sample.] | | | | | | | | | | | | |
| 41 | | | | 27 | | | | | | | | | | | | | | |
| 42 | | | | 22 | | | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |
| Page 3 of 8 | | | | | | | | | | | | | | | | | | |

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|--|------------------|---------------------------------------|------------|--|--|----------------------|---------|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 51" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 60" | 7C | 7 | | (Photo 310 at 46'.) | | | | | | | | | | | | |
| | | 7B | 7 | | (45.5' - 53.9') FINE TAILINGS - Mottled orange and dark greenish gray (to 50'), pale yellowish gray (50' - 53.9'), firm, moist tailings. | | | 88.7 | | | | | | | | | |
| 46 | | 7A | 8 | | | | | | | | | | | | | | |
| | | | | | [Photo 311 at 46.5'.] | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 43" | ST 28" | 8 | | | | | | | | | | | | | | |
| 51 | | | | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | |
| | | AC | 9 | | [Photo 312 at 52.5'] | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | (53.9' - 55') SILTY CLAY - Dark brown, hard, moist silty clay, trace very fine sand. | | | | | | | | | | | | |
| 55 | 48" | ST 25" | 10 | | (55' - 77.5') SILTY SAND - Yellowish brown, medium dense, slightly moist to moist, silty, very fine to fine sand. | | | | | | | | | | | | |
| 56 | | | | | | SM | | 16.2 | 77.9 | 2.64 | NP | 0.0 | 60.4 | 39.6 | 5.6E-4 | 0.129 | |
| 57 | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:

Hole backfilled with cement/bentonite grout.

Page 4 of 8




LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
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NOTES:
 Hole backfilled with cement/bentonite grout.




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|--|----------------------------|---------------------------------------|------------|--|-----------------------------|------------|----------------------|-------------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| DEPTH (FT) | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | |
| | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 48" | AC | 11 | | | (61.1' - 62.1') Sandy clay. | | | 16.0 | 95.4 | | | 0.0 | 38.7 | 61.3 | | | |
| 58" | | | | | | | | | | | | | | | | | |
| 59" | | | | | (63.1' - 64') Sandy clay. | | | | | | | | | | | | |
| 60" | 48" | CA 17" | 9 | | | | | | | | | | | | | | |
| | | 12B | 11 | | | | | | | | | | | | | | |
| 61" | | 12A | 12 | | | | | | | | | | | | | | |
| 62" | | | | | | | | | | | | | | | | | |
| 63" | | | | | | | | | | | | | | | | | |
| 64" | | | | | | | | | | | | | | | | | |
| 65" | 49" | CA 18" | 13C | 7 | | | | | | | | | | | | | |
| | | 13B | 7 | | | | | | | | | | | | | | |
| 66" | | 13A | 12 | | | | | 14.2 | 96.2 | | | | | | | | |
| 67" | | | | | | | | | | | | | | | | | |
| 68" | | | | | | | | | | | | | | | | | |
| 69" | | | | | | | | | | | | | | | | | |
| 70" | 44" | CA 18" | 14C | 7 | | | | | | | | | | | | | |
| | | 14B | 9 | | | | | | | | | | | | | | |
| 71" | | 14A | 10 | | | | | | | | | | | | | | |
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| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 5 of 8 | | | | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

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|--|------------------|---------------------------------------|-------------------|--|-----------------|---|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) |
| 72-74 | 44" | | | | | (71.5' - 73.5') Abundant clayey sand zones. | | | | | | | | | | | | |
| 75-76 | 38" | CA 18" | 15C 15B 15A | 7 8 11 | | | | | | | | | | | | | | |
| 77-78 | | | | | 16 | (77.5' - 78') WEATHERED SANDSTONE - Rusty red, moist, fine to medium grained. (Sample #16 is bagged core.) | | | | | | | | | | | | |
| 79-80 | | | | | | (78' - 96.9') GRAVELLY SAND - Mottled rusty red/brown/yellow, dense, moist fine to medium sand, silty throughout, some clayey zones, abundant coarse material from coarse sand up to 3" gravel comprised of cemented sandstone. | | | | | | | | | | | | |
| 81-82 | 42" | CA 18" | 17C 17B 17A | 16 21 21 | | | | | 11.0 | 107.6 | 2.76 | | 12.9 | 65.6 | 21.5 | | | |
| 83-84 | | | | | | | | | | | | | | | | | | |
| 85-86 | 36" | CA 17" | 18C 18B 18A | 18 21 19 | | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | | | | | | | | | | | | | |
| NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | | | | | | |

Page 6 of 8

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|---|------------------|---|------------|----------------------|-----------------|---|------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B11 | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 102 | | | | | | (102.5' - 103') Reddish brown, strongly cemented sandstone. | | | | | | | | | | | | |
| 103 | | | | | | E.O.B. at 103.0' at 10:00 (practical auger refusal) | | | | | | | | | | | | |
| 104 | | | | | | | | | | | | | | | | | | |
| 105 | | | | | | | | | | | | | | | | | | |
| 106 | | | | | | | | | | | | | | | | | | |
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LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.



CLIENT:

P.O. BOX 2077
Dallas, Texas 75201-2077

NECR - PRE DESIGN STUDY INVESTIGATION

BORING LOG

BOREHOLE ID:

TI-B15

CONTRACTOR INFORMATION

DRILL RIG INFORMATION

BOREHOLE INFORMATION

DRILLING COMPANY: NATIONAL

DRILLING RIG: CME 85 HD

BIT TYPE: N/A

| |
|-------------------|
| CASING DEPTH: N/A |
|-------------------|

| |
|------------------|
| START: 12/5/2013 |
|------------------|

DRILLER: M. CAIN

DRILLING METHOD: HSA/CC

AUGER O.D.: 8.25"

SURFACE ELEV. (FT): 6976.8

| |
|-------------------|
| FINISH: 12/5/2013 |
|-------------------|

DRILLER'S HELPER: L. ALDAZ

HAMMER TYPE: AUTO

HOLE DIAM.: 8.25"

DEPTH TO BEDROCK (FT): N/A

LOGGED BY: R. SCHAUT

HAMMER WT: 140 lb

CORE DIAM.: 3.0"

TOTAL DEPTH (FT): 71.5

FIELD SAMPLE RECOVERY DATA

LABORATORY TEST DATA




[illegible]




LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY

NOTES:

Hole backfilled with cement/bentonite grout.

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|--|--|---------------------------------------|--|--|--|---|--|----------------------|--|----------------------------|--|--|--|
|  | | CLIENT: | |  | |  | | BORING LOG | | BOREHOLE ID: TI-B15 | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | |
| | | MATERIAL DESCRIPTION | | | | | | | | | | | |
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|---|------------------|---------------------------------------|------------|--|---|----------------------|---------|----------------------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAXIAL (PHI, C [PSF]) |
| 48" | | | | | | | | | | | | | | | | | |
| 43 | | | | | | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | |
| 45 | 26" | CA 18" 15C | 13 | | (45' - 50') SANDY SILT - Dark yellowish brown, hard, moist, very fine to fine sand, occasional clayey sand zones. | | | | | | | | | | | | |
| | | 15B | 25 | | | | | | | | | | | | | | |
| 46 | | 15A | 26 | | | | | 25.8 17.3 | 99.3 | 2.81 | NP | 0.0 | 37.0 | 63.0 | | | |
| 47 | | | | | | | | | | | | | | | | | |
| 48 | | | | | | | | | | | | | | | | | |
| 49 | | | | | | | | | | | | | | | | | |
| 50 | 24" | CA 18" 16C | 6 | | (50' - 52') CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine sand, silty. | | | | | | | | | | | | |
| | | 16B | 8 | | | | | | | | | | | | | | |
| 51 | | 16A | 11 | | | | | | | | | | | | | | |
| 52 | | | | | (52' - 65') SILTY CLAY - Dark yellowish brown, firm to hard, moist silty clay, trace to few very fine to fine sand, occasional thin (1-6") clayey sand zones. (~53' - 55') Very hard, very dense clay. | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | |
| 55 | 18" | CA 18" 17C | 10 | | | | | | | | | | | | | | |
| | | 17B | 11 | | | | | | | | | | | | | | |
| 56 | | 17A | 12 | | | | | 11.7 | 104.2 | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | |

LEGEND:

CA = CALIFORNIA SAMPLE (2-INCH OD)
ST = SHELBY TUBE (3-INCH OD)
AC = ACRYLIC LINER
HSA = HOLLOW-STEM AUGER
CC = CONTINUOUS CORE
NR = NO RECOVERY




NOTES:




Hole backfilled with cement/bentonite grout.



















Page 4 of 5





LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|---|------------|----------------------------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B15 | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | |
| 18" | | | | | | | | | | | | | | | | | | | |
| 58 | | | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | | | |
| 60 | 60" | CA 18" | 18C | 8 | | | | | | | | | | | | | | | |
| | | | 18B | 11 | | | | | | | | | | | | | | | |
| 61 | | | 18A | 15 | | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | | | |
| 63 | | | | | | | | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | | | |
| 65 | 40" | CA 18" | | 6 | | (65' - E.O.B.) CLAYEY SAND - Yellowish brown, medium dense, moist, very fine to fine clayey sand, silty, occasional 1-3" zones of sandy clay. | | | | | | | | | | | | | |
| | | | 19B | 8 | | | | | | | | | | | | | | | |
| 66 | | | 19A | 10 | | | | | 12.7 | 100.7 | | | | | | | | | |
| 67 | | | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | | | |
| 70 | | CA 18" | | 7 | | | | | | | | | | | | | | | |
| | | | 20B | 6 | | | | | | | | | | | | | | | |
| 71 | | | 20A | 9 | | | | | | | | | | | | | | | |
| | | | | | | E.O.B. 71.5' at 14:30 | | | | | | | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | | | | | | | | | | | | | | | | |
| NOTES: Hole backfilled with cement/bentonite grout. | | | | | | | | | | | | | | | | | | | |
| Page 5 of 5 | | | | | | | | | | | | | | | | | | | |




| | | | | | | | |
|---|------------------|---|------------|---|-----------------|---|------------|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B23 | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | |
| CONTRACTOR INFORMATION | | DRILL RIG INFORMATION | | BOREHOLE INFORMATION | | | |
| DRILLING COMPANY: NATIONAL | | DRILLING RIG: CME 85 HD | | BIT TYPE: N/A | | CASING DEPTH: N/A | |
| DRILLER: M. CAIN | | DRILLING METHOD: HSA/CC | | AUGER O.D.: 8.25" | | SURFACE ELEV. (FT): 6959.3 | |
| DRILLER'S HELPER: L. ALDAZ | | HAMMER TYPE: AUTO | | HOLE DIAM.: 8.25" | | DEPTH TO BEDROCK (FT): 43.0 | |
| LOGGED BY: R. SCHAUT | | HAMMER WT: 140 lb | | CORE DIAM.: 3.0" | | TOTAL DEPTH (FT): 70.5 | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS |
| | | | | | | | GRAPHIC |
| 39" | | | | | | (0' - 0.6') SANDY CLAY - Light brown, soft, moist, very fine to fine sand, roots. | |
| 1 | | | | | | (0.6' - 0.9') ROCK - Crushed basalt, 1/2" - 3", sandy clay in voids. | |
| 2 | | | | | | (0.9' - 5') SANDY CLAY - Firm to hard, slightly moist to moist, sandy clay, very fine to fine sand, occasional coarse sand and very fine gravel. | |
| 3 | | | | | | | |
| 4 | | | | | | | |
| 5 | 44" | CA 18" | 1C | 12 | | (5' - 7') SILTY SAND WITH GRAVEL - Light brown, medium dense, slightly moist to moist, silty very fine to fine sand with little to some gravel to 2". | |
| 6 | | | 1B | 14 | | | |
| 7 | | | 1A | 10 | | | |
| 8 | | | | | | | |
| 9 | | | | | | | |
| 10 | 42" | CA 16" | 2B | 5 | | (7' - 13.4') SANDY CLAY - See 0.9' to 5' above. | |
| 11 | | | 2A | 6 | | | |
| 12 | | | | | | | |
| 13 | | | | | | | |
| LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY | | | | NOTES: Hole backfilled with cement/bentonite grout. | | | |

| | | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|-----------------|--|------------|--|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. | SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) |
| 42" | | | | | | (13.4' - 14.2') FINE TAILINGS WITH SAND - Pale gray, soft to firm, moist with very fine to fine sand tailings. | |  | | | | | | | | | | |
| 14 | | | | | | (14.2' - ~16') SILTY SAND TAILINGS - Pale yellowish gray, loose, moist, fine to medium sand. | |  | | | | | | | | | | |
| 15 | 25" | ST 28" | 3 | | | | |  | 20.7 | 87.7 | 2.77 | | 0.0 | 62.8 | 37.2 | | | |
| 16 | | | | | | | |  | | | | | | | | | | |
| 17 | | | | | | (~16' - 20.3') SANDY CLAY - Dark yellowish brown, firm, moist, very fine to fine sand. | |  | | | | | | | | | | |
| 18 | | | | | | | |  | | | | | | | | | | |
| 19 | | | | | | | |  | | | | | | | | | | |
| 20 | 36" | CA 18" | 5C | 5 | | | |  | | | | | | | | | | |
| 21 | | | 5B | 6 | | (20.3' - 23') SILTY SAND - Yellowish brown, medium dense, moist, silty very fine to fine sand. | |  | | | | | | | | | | |
| 22 | | | 5A | 7 | | | |  | | | | | | | | | | |
| 23 | | | | | | (23' - 38.6') SILTY CLAY - Predominantly dark yellowish brown, firm to hard, moist silty clay with varying amount of sand as shown, occasional coarse sand to very fine gravel throughout. | |  | | | | | | | | | | |
| 24 | | | | | | (23' - 27.5') Trace to few sand. | |  | | | | | | | | | | |
| 25 | 39" | ST 30" | 6 | | | | |  | | | | | | | | | | |
| 26 | | | | | | | CL |  | 21.6 | 101.7 | 2.73 | 49/18/31 | 0.0 | 8.8 | 91.2 | | 0.05 | |
| 27 | | | | | | (27.5' - 30') Little to some sand. | |  | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | | |
| Page 2 of 5 | | | | | | | | | | | | | | | | | | |

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|---|------------------|---|------------|----------------------|---|---|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|--|
|  | | CLIENT:   | | BORING LOG | | BOREHOLE ID: TI-B23 | | | | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | | | | | | | | | | | |
| FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, C [PSF]) | |
| 25" | | | | | | | | | | | | | | | | | | |
| 43 | | | | | (43' - 65.5') SANDSTONE - Mostly very pale yellowish gray, moist, mostly non-or weakly cemented very fine to fine sand, some very hard, strongly cemented, fissile zones as shown, some clay zones as shown ("Zone 3"?). (43' - 43.6') Strongly cemented, fissile. (43.6' - 44') Clay - yellowish brown, firm to hard, moist, slightly silty. |  | | | | | | | | | | | | |
| 44 | | | | | | | | | | | | | | | | | | |
| 45 | 32" | CA 8" | 10A | 13 | | | | | | | | | | | | | | |
| 46 | | | | 50/ 3" | | | | | | | | | | | | | | |
| 47 | | | | | | | | | | | | | | | | | | |
| 48 | 16" | | | | (~47' - ~48') Very hard, strongly cemented, fissile. | | | | | | | | | | | | | |
| 49 | | CA NR | | 50/ 4" | | | | | | | | | | | | | | |
| 50 | 29" | | | | | | | | | | | | | | | | | |
| 51 | | | | | | | | | | | | | | | | | | |
| 52 | | | | | | | | | | | | | | | | | | |
| 53 | | | | | | | | | | | | | | | | | | |
| 54 | | | | | | | | | | | | | | | | | | |
| 55 | 33" | CA 3" | 11A | 50/ 5" | (~55' - 63') Coarser (fine to medium). | | | | | | | | | | | | | |
| 56 | | | | | (~56' - 56.8') Color is reddish yellow. | | | | | | | | | | | | | |
| 57 | | | | | | | | | | | | | | | | | | |

LEGEND:
 CA = CALIFORNIA SAMPLE (2-INCH OD)
 ST = SHELBY TUBE (3-INCH OD)
 AC = ACRYLIC LINER
 HSA = HOLLOW-STEM AUGER
 CC = CONTINUOUS CORE
 NR = NO RECOVERY

NOTES:
 Hole backfilled with cement/bentonite grout.

| | | | | | | | | | | | | | | | | | |
|--|------------------|---------------------------------------|------------|--|--|----------------------|---------|-----------------|-------------------|------------------|-----------------------------|----------|--------|---------|------------------------|--------------------|-------------------------|
|  | | CLIENT: | |   | | BORING LOG | | BOREHOLE ID: | | | | | | | | | |
| PROJ. LOC.: GALLUP, NM | | NECR - PRE DESIGN STUDY INVESTIGATION | | | | | | TI-B23 | | | | | | | | | |
| | | FIELD SAMPLE RECOVERY DATA | | | | LABORATORY TEST DATA | | | | | | | | | | | |
| DEPTH (FT) | CORE RECOV. (IN) | SAMPLES & RECOV. SAMPLE NO. | BLOW COUNT | BULK SAMPLE NO. | MATERIAL DESCRIPTION | USCS CLASS | GRAPHIC | WATER CONT. (%) | DRY DENSITY (PCF) | SPECIFIC GRAVITY | ATTERBERG LIMITS (LL/PL/PI) | % GRAVEL | % SAND | % FINES | SAT. HYD. COND. (cm/s) | CONSOLIDATION (Cc) | TRIAxIAL (PHI, c [PSF]) |
| 33" | | | | | | | | | | | | | | | | | |
| 58 | | | | | | | | | | | | | | | | | |
| 59 | | | | | | | | | | | | | | | | | |
| 60 | 24" | CA 3" | 12A | 50/ 4" | | | | | | | | | | | | | |
| 61 | | | | | | | | | | | | | | | | | |
| 62 | | | | | | | | | | | | | | | | | |
| 63 | | | | | (~63 - 65.5') COAL - Black, hard, dry to slightly moist, fissile. | | | | | | | | | | | | |
| 64 | | | | | | | | | | | | | | | | | |
| 65 | 30" | CA 13" | 13B | 24 | | | | | | | | | | | | | |
| 66 | | | 13A | 50/ 4.5" | (65.5' - E.O.B.) SHALE - Gray, very hard, slightly moist shale, trace silt, non- to weakly-cemented ("Zone 2"?). | | | 10.2 | 103.0 | | | | | | 9.7E-8 | | |
| 67 | | | | | | | | | | | | | | | | | |
| 68 | | | | | | | | | | | | | | | | | |
| 69 | | | | | | | | | | | | | | | | | |
| 70 | | CA 4" | 14 | 50/ 5" | | | | | | | | | | | | | |
| 71 | | | | | E.O.B. 70.5' @ 13:50 | | | | | | | | | | | | |
| <div>LEGEND: CA = CALIFORNIA SAMPLE (2-INCH OD) ST = SHELBY TUBE (3-INCH OD) AC = ACRYLIC LINER HSA = HOLLOW-STEM AUGER CC = CONTINUOUS CORE NR = NO RECOVERY</div> <div>NOTES: Hole backfilled with cement/bentonite grout.</div> | | | | | | | | | | | | | | | | | |
| Page 5 of 5 | | | | | | | | | | | | | | | | | |

ATTACHMENT C

TAILINGS DISPOSAL AREA CONE PENETRATION TEST RESULTS (MWH, 2014A)



MWH Americas

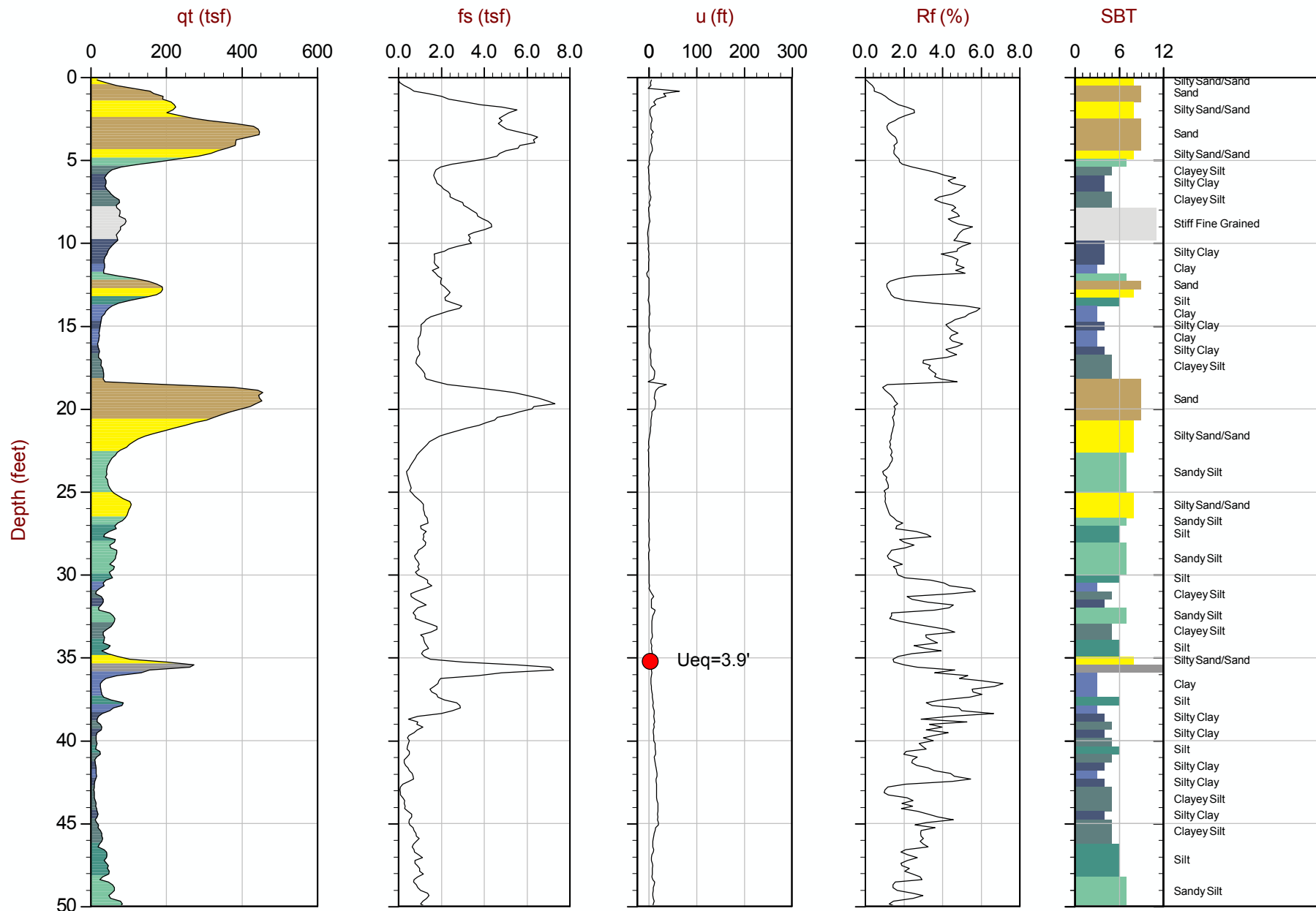
Job No: 13-52118

Date: 11:07:13 15:36

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-01

Cone: 155:T1500F15U500



Max Depth: 26.950 m / 88.42 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP01.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.649117 Long: -108.501667
● Equilibrium Pore Pressure from Dissipation



MWH Americas

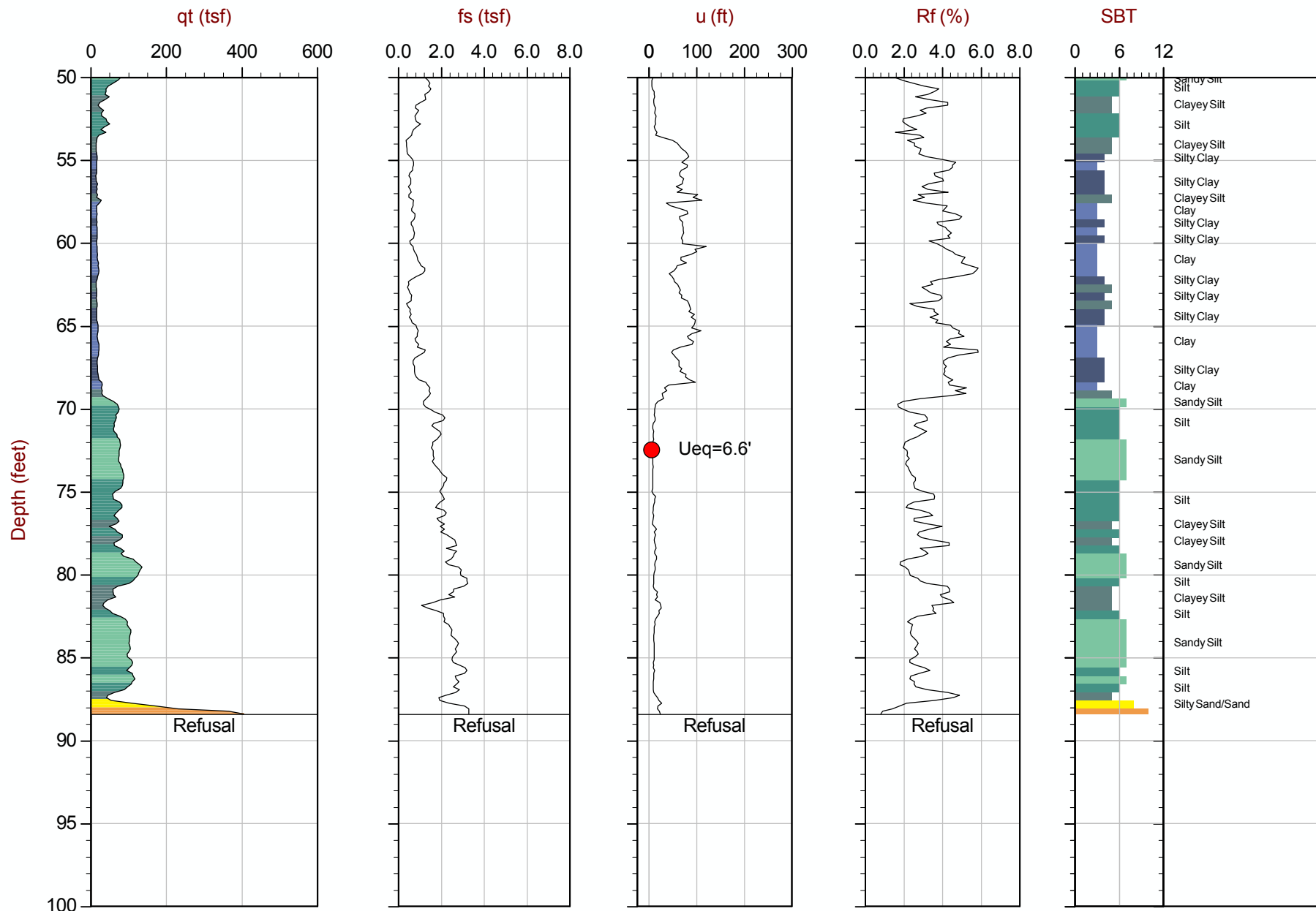
Job No: 13-52118

Date: 11:07:13 15:36

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-01

Cone: 155:T1500F15U500



Max Depth: 26.950 m / 88.42 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP01.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.649117 Long: -108.501667
● Equilibrium Pore Pressure from Dissipation



MWH Americas

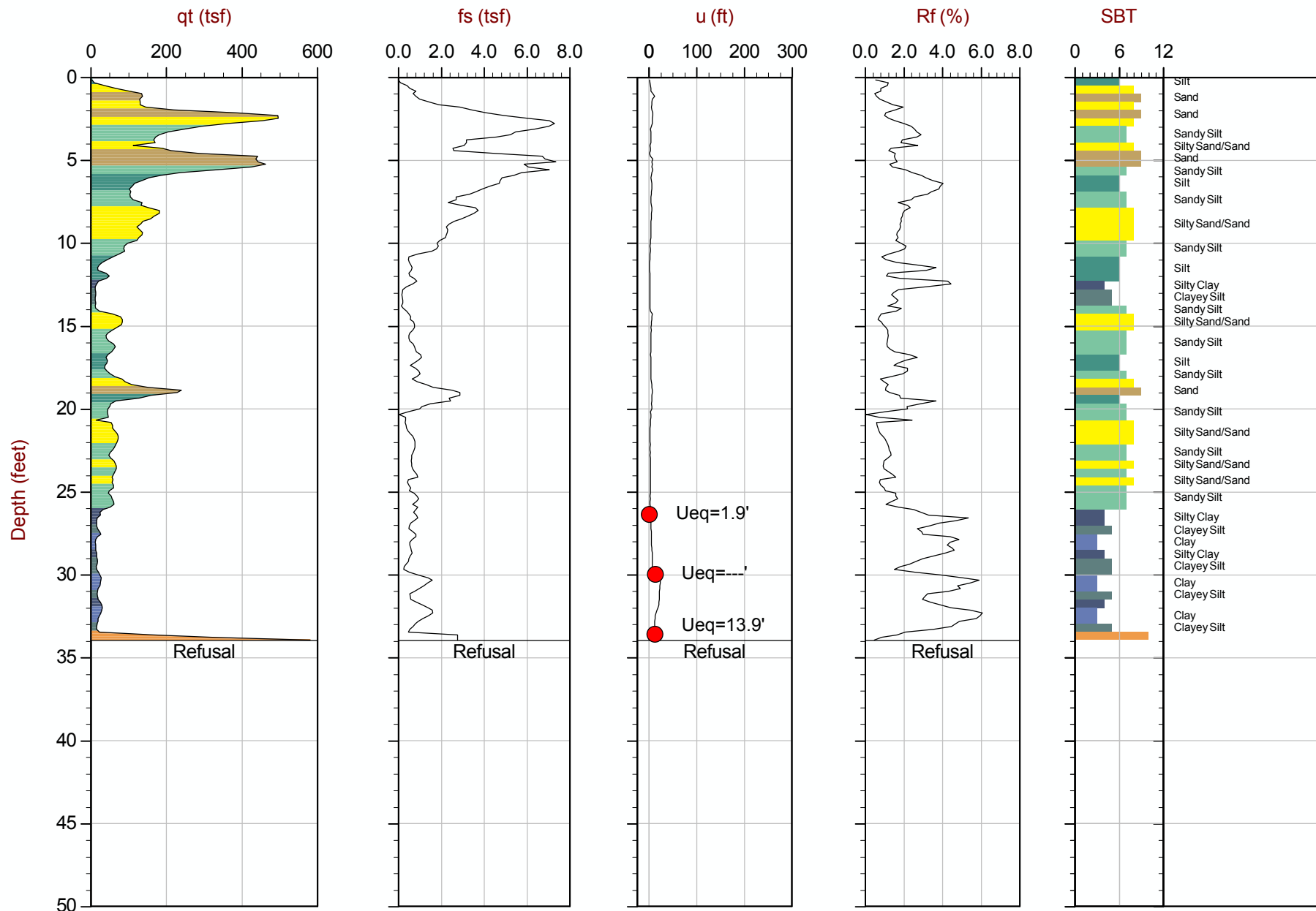
Job No: 13-52118

Date: 11:05:13 13:37

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-02

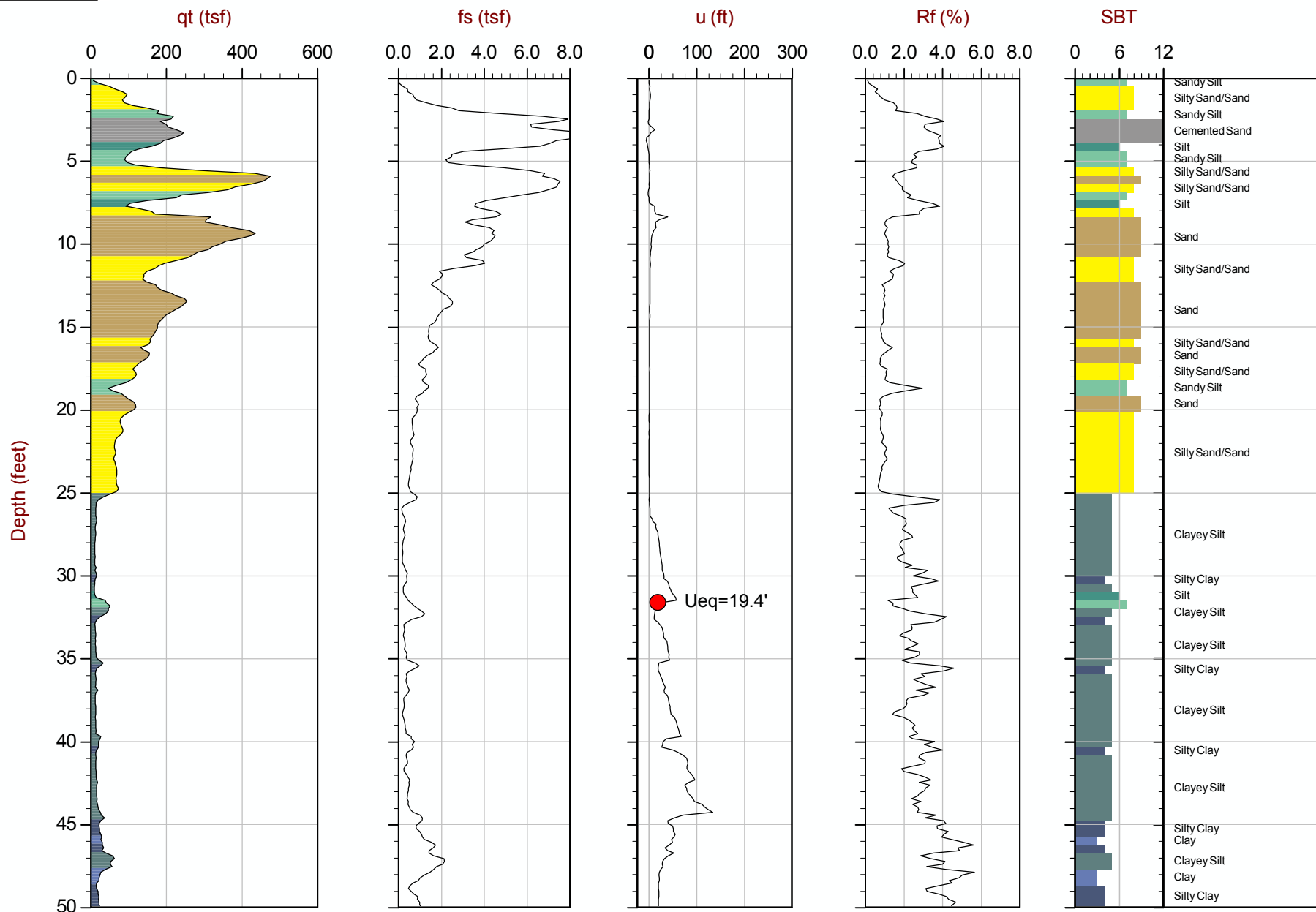
Cone: 155:T1500F15U500



Max Depth: 10.350 m / 33.96 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP02.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.650200 Long: -108.499750
● Equilibrium Pore Pressure from Dissipation



Max Depth: 18.550 m / 60.86 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP08.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
 Coords: Lat: 35.647250 Long: -108.497250
 ● Equilibrium Pore Pressure from Dissipation



MWH Americas

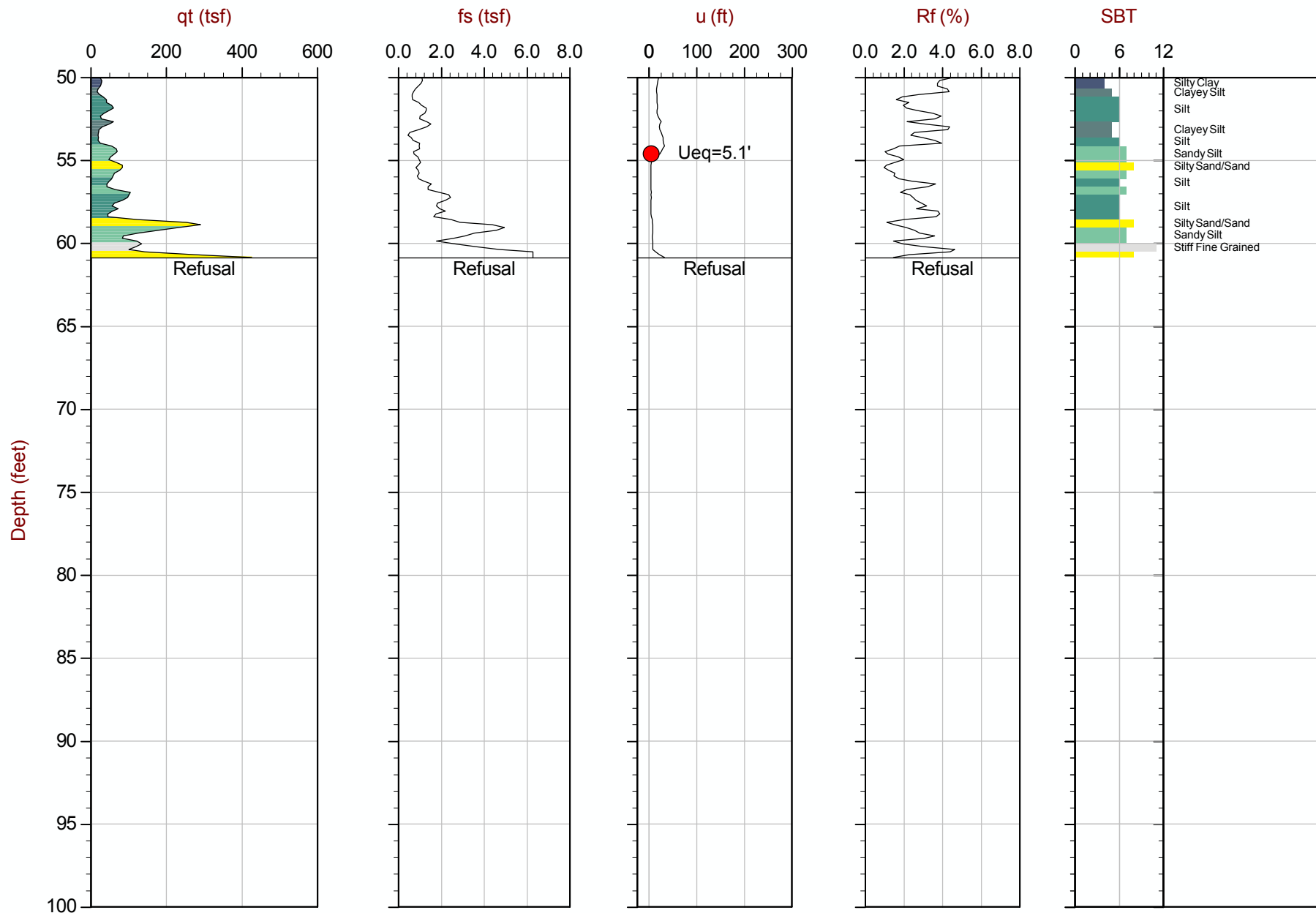
Job No: 13-52118

Date: 11:07:13 08:21

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-08

Cone: 155:T1500F15U500



Max Depth: 18.550 m / 60.86 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP08.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647250 Long: -108.497250
● Equilibrium Pore Pressure from Dissipation



MWH Americas

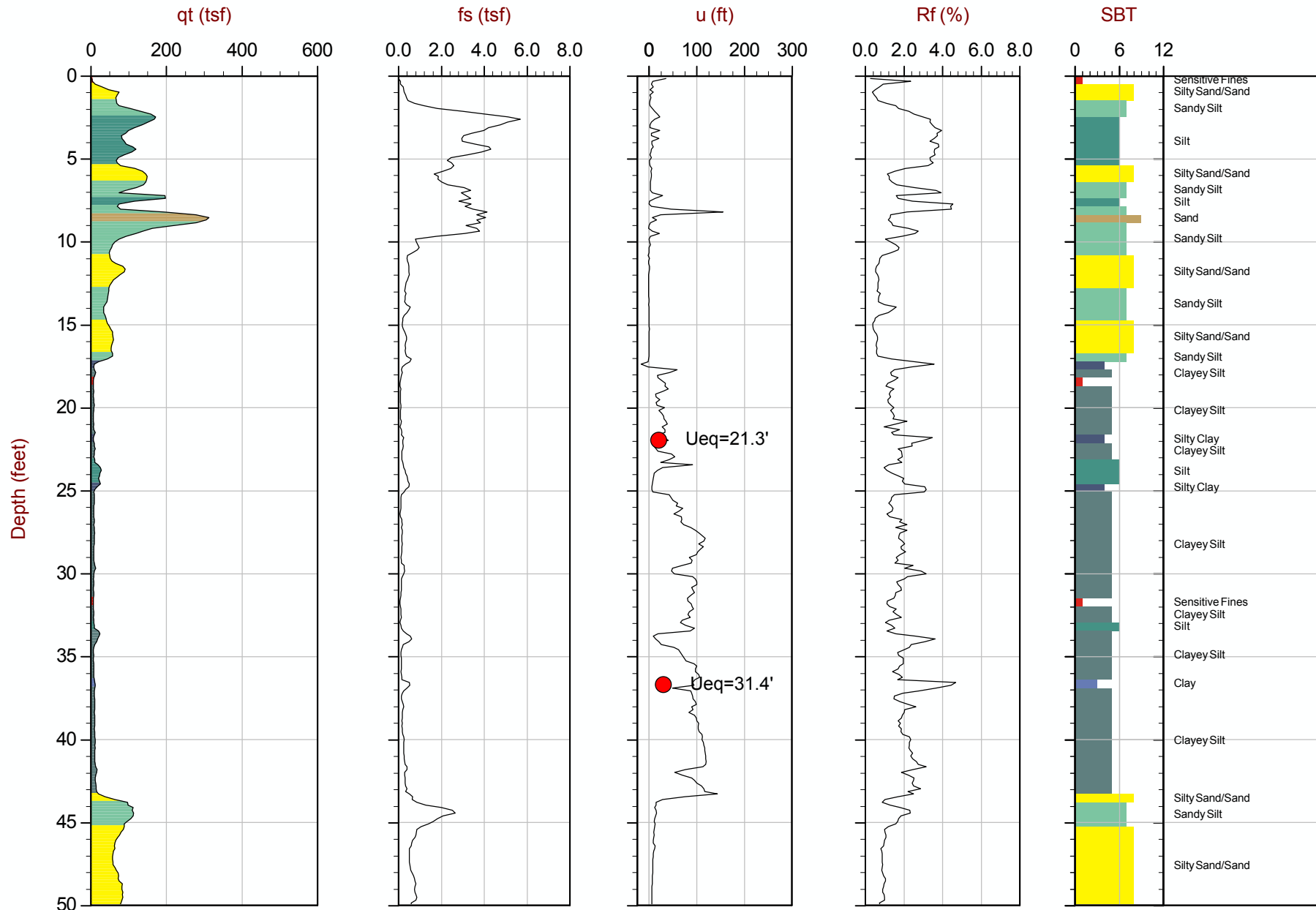
Job No: 13-52118

Date: 11:06:13 10:23

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-10

Cone: 155:T1500F15U500



Max Depth: 19.250 m / 63.16 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP10.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647833 Long: -108.497217
● Equilibrium Pore Pressure from Dissipation



MWH Americas

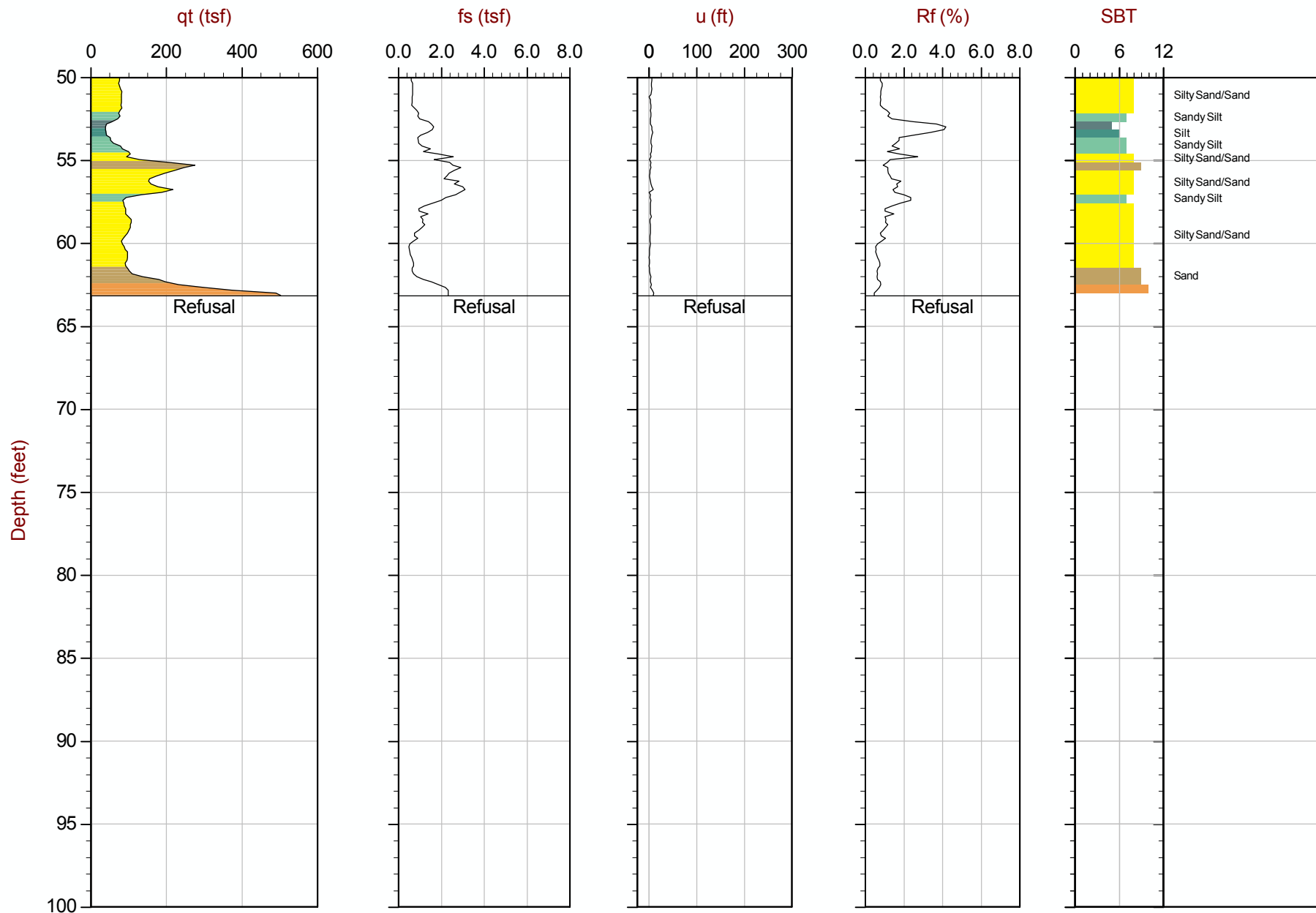
Job No: 13-52118

Date: 11:06:13 10:23

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-10

Cone: 155:T1500F15U500



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● Equilibrium Pore Pressure from Dissipation



MWH Americas

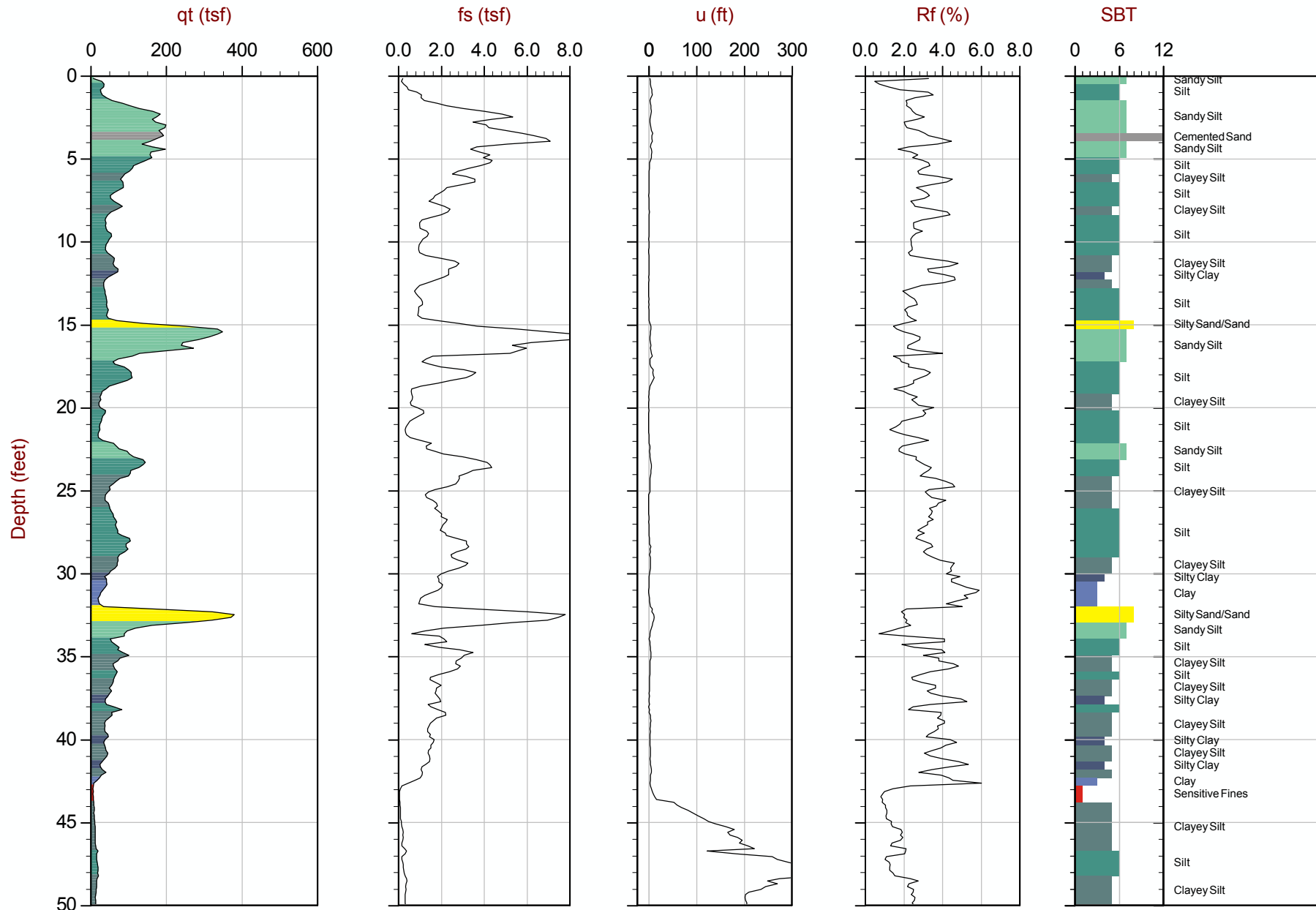
Job No: 13-52118

Date: 11:07:13 12:13

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-11

Cone: 155:T1500F15U500



Max Depth: 29.500 m / 96.78 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP11.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647650 Long: -108.495850
● Equilibrium Pore Pressure from Dissipation



MWH Americas

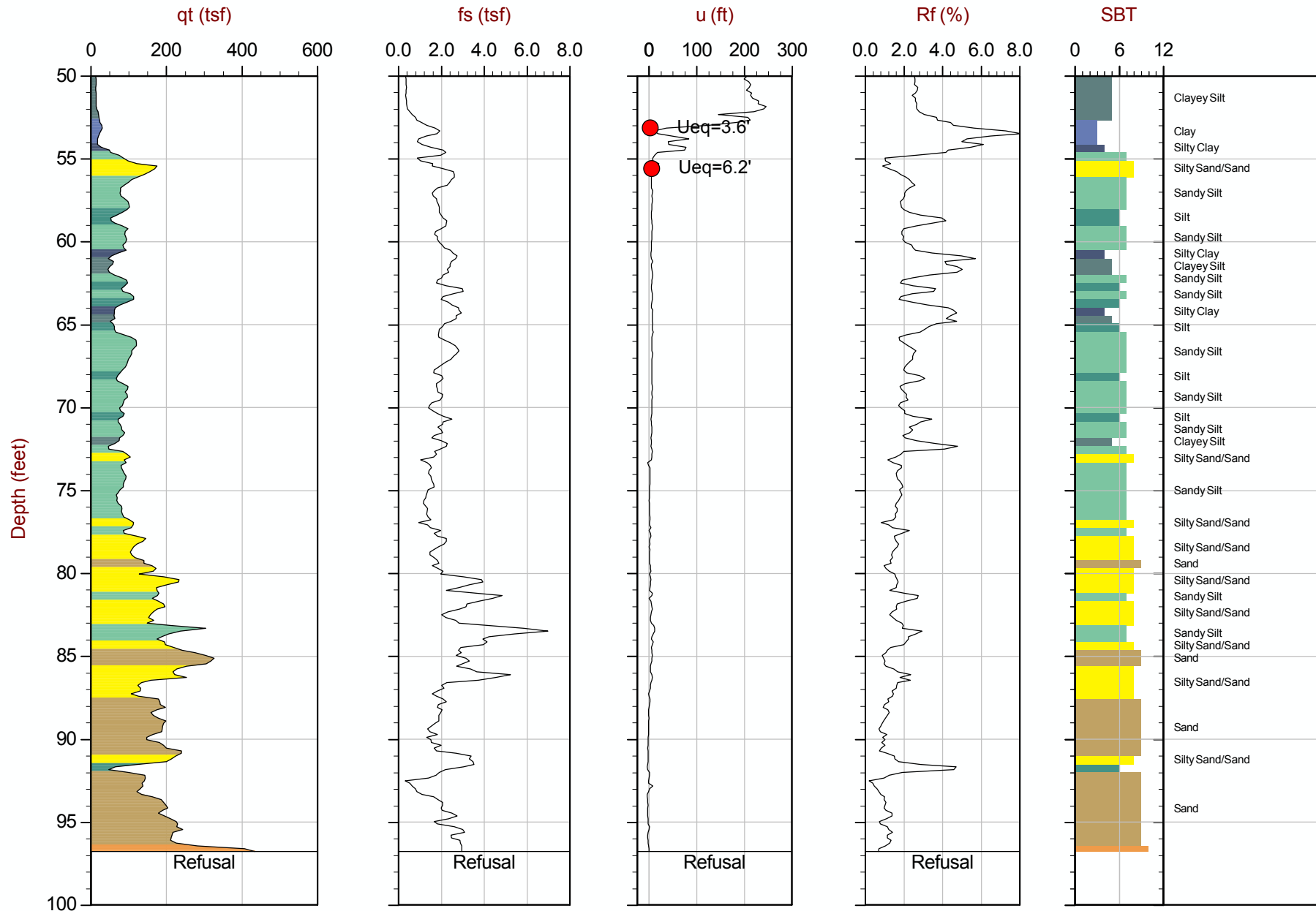
Job No: 13-52118

Date: 11:07:13 12:13

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-11

Cone: 155:T1500F15U500



Max Depth: 29.500 m / 96.78 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP11.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647650 Long: -108.495850
● Equilibrium Pore Pressure from Dissipation



MWH Americas

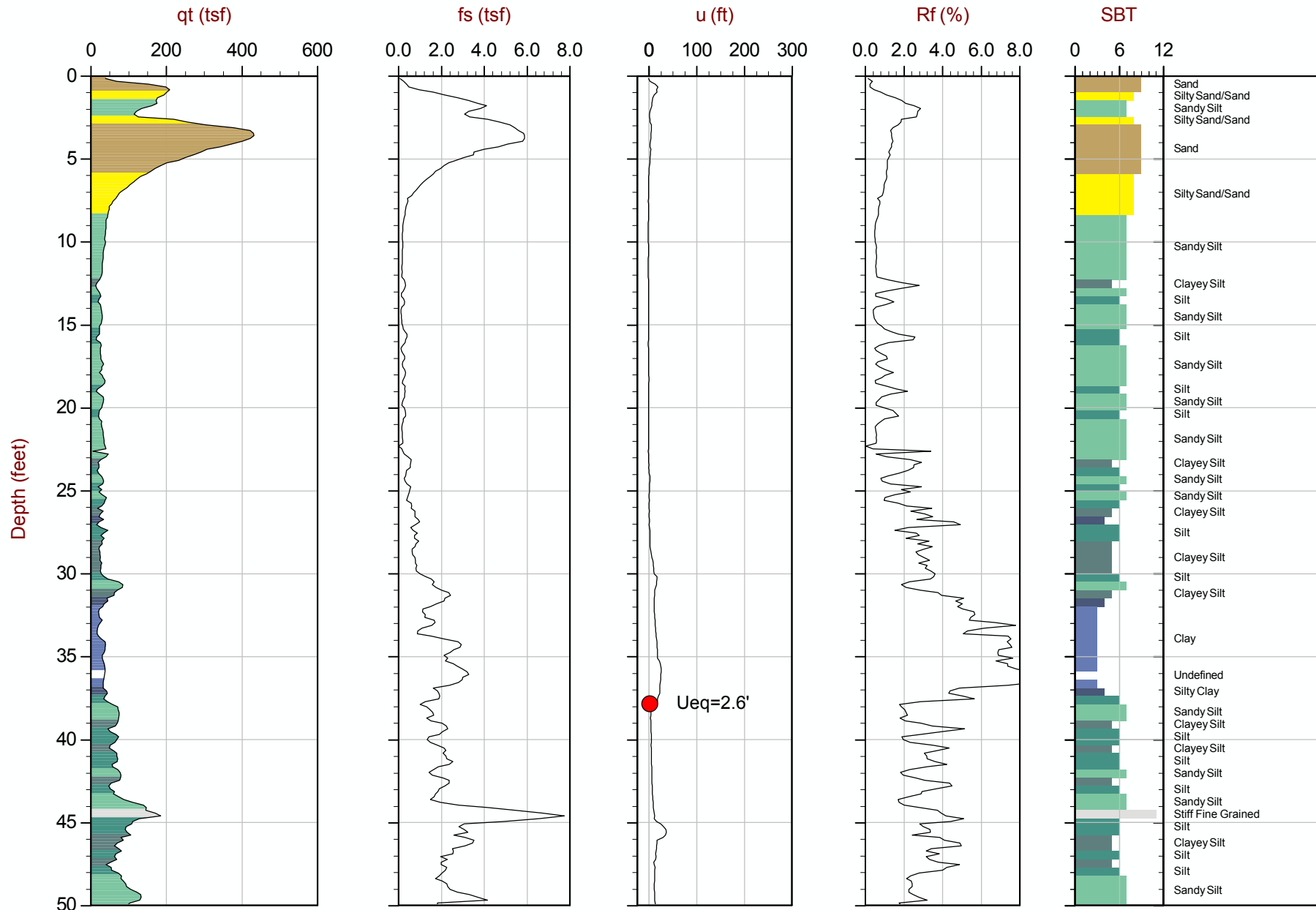
Job No: 13-52118

Date: 11:06:13 16:32

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-15

Cone: 155:T1500F15U500



Max Depth: 16.800 m / 55.12 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP15.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647583 Long: -108.499800
● Equilibrium Pore Pressure from Dissipation



MWH Americas

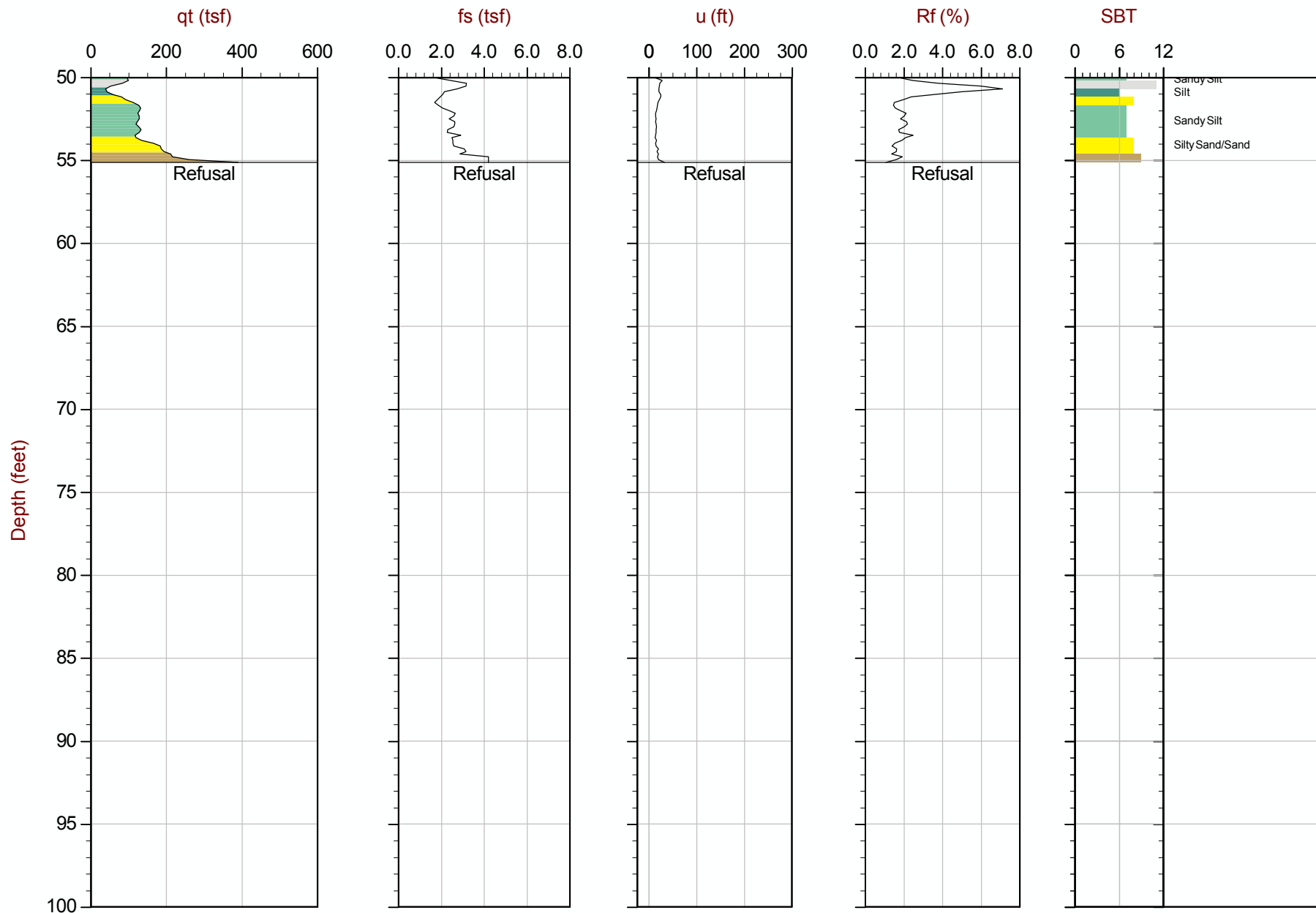
Job No: 13-52118

Date: 11:06:13 16:32

Site: CHURCH ROCK MILL SITE TSF

Sounding: RCPT-15

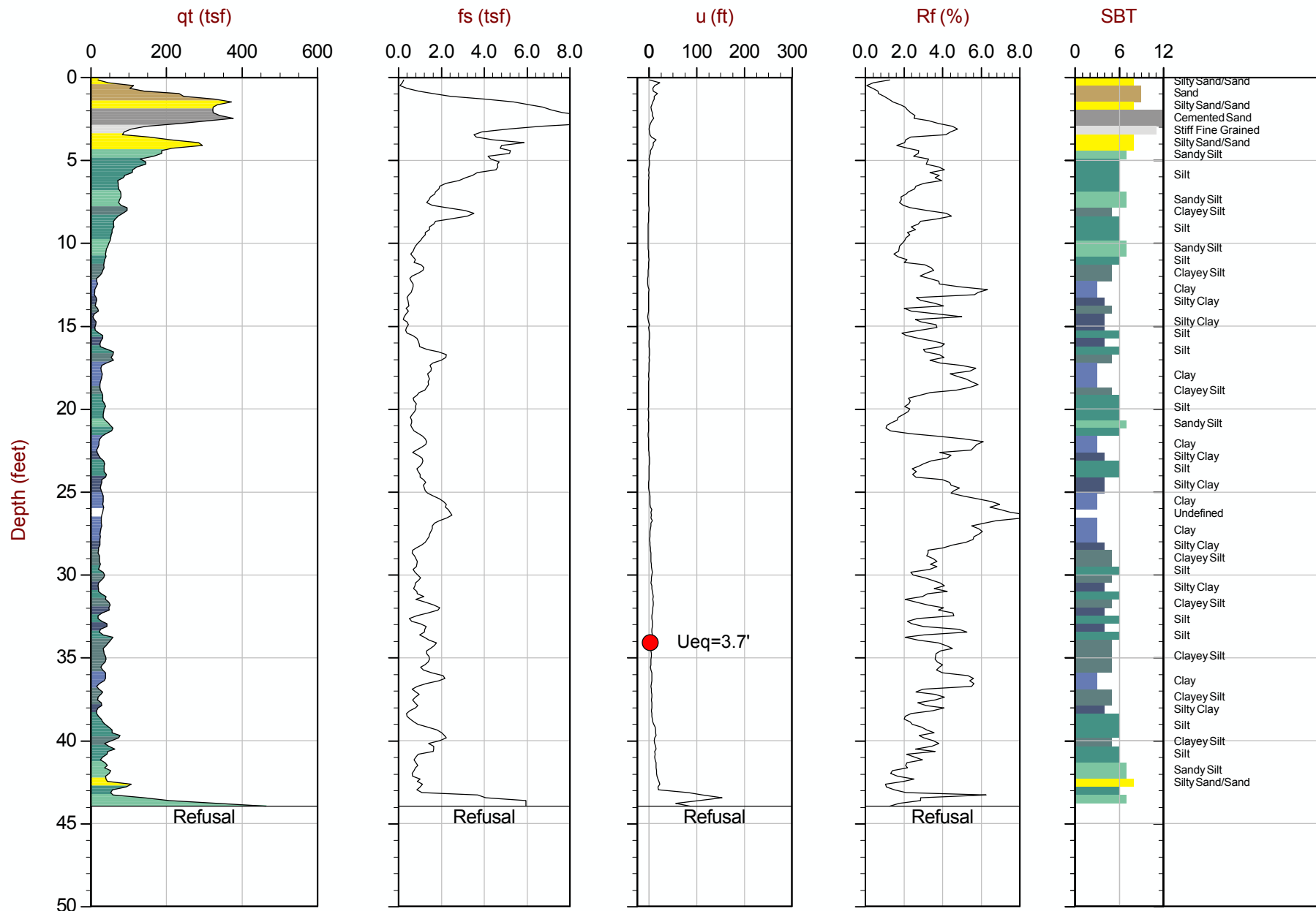
Cone: 155:T1500F15U500



Max Depth: 16.800 m / 55.12 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP15.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
Coords: Lat: 35.647583 Long: -108.499800
● Equilibrium Pore Pressure from Dissipation



Max Depth: 13.400 m / 43.96 ft
Depth Inc: 0.050 m / 0.164 ft
Avg Int: 0.150 m

File: 13-52118_RP23.COR
Unit Wt: SBT Chart Soil Zones

SBT: Lunne, Robertson and Powell, 1997
 Coords: Lat: 35.650833 Long: -108.497700
 ● Equilibrium Pore Pressure from Dissipation

ATTACHMENT D

RECORDED WATER LEVELS AT THE MILL SITE (CHESTER ENGINEERS, 2016)

| Well ID | Measurement Date | Measurement Time | Historical Reference Elev | Water Level Depth | Water Level Elev |
|---------|------------------|------------------|---------------------------|-------------------|------------------|
| 0509 D | 1/4/2016 | 8:37 | 6949.44 | 82.89 | 6866.55 |
| EPA 23 | 1/4/2016 | 9:30 | 6926.31 | 59.52 | 6866.79 |
| GW 1 | 1/4/2016 | 14:20 | 6916.46 | 65.01 | 6851.45 |
| GW 2 | 7/6/2015 | 14:25 | 6912.88 | 58.96 | 6853.92 |
| GW 3 | 7/7/2015 | 10:50 | 6910.04 | 56 | 6854.04 |
| 632 | 1/4/2016 | 12:35 | 6903.49 | 48.05 | 6855.44 |
| EPA 25 | 1/5/2016 | 10:35 | 6903.38 | 56.62 | 6846.76 |
| EPA 27 | 1/12/1999 | | 6910.95 | 55.45 | 6855.5 |
| EPA 28 | 1/4/2016 | 15:20 | 6917.86 | 65.83 | 6852.03 |
| 624 | 1/4/2016 | 16:35 | 6898.57 | 53.61 | 6844.96 |

Note: Water levels provided by email from Chester Engineers, on April 20, 2016.

ATTACHMENT E

LIQUEFACTION TRIGGERING ANALYSIS CALCULATIONS

[illegible]

[illegible]

[illegible]

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------|---------|------|-------|------|------|-------|-------|----------------|-------|-------|------|-------|------|---|------|--------|------|-------|----|-------|-----|-----|------|-------|------|---|---|------|------|------|------|------|-------|-------|-------|-------|-----|------|------|------|------|--------|-------|-------|--------|-------|-----|-----|-----|--|--|------|-------|------|-------|--|--------|------|---|---|---|---|---|---|---|---|---|---|---|---|------|------|---|-------|---|---|
| 68.241 | 6901.72 | 21.3 | 0.970 | 20.8 | 85.3 | 36.97 | 4.55% | Coarse Alkatum | 0.056 | 111.0 | 3.88 | 0.000 | 3.88 | 0 | 0.30 | 6.141 | 0.59 | 7.71 | 4 | 5.57% | 3.4 | 36% | 4.01 | 0.000 | 4.01 | 0 | 0 | 0.50 | 0.04 | 0.93 | 0.10 | #### | 0.000 | 32.46 | 40.17 | 0.069 | N/A | 0.60 | 0.16 | 0.80 | 0.73 | 213.00 | 0.000 | 12.94 | 99.80 | 0.172 | N/A | N/A | N/A | | | 0.30 | 6.30 | 0.60 | 7.71 | | 68.241 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 68.405 | 6901.56 | 29.4 | 1.269 | 28.8 | 97.4 | 42.22 | 4.31% | Coarse Alkatum | 0.056 | 111.0 | 3.89 | 0.000 | 3.89 | 0 | 0.29 | 8.498 | 0.81 | 10.63 | 7 | 4.97% | 3.3 | 36% | 4.02 | 0.000 | 4.02 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.46 | 44.09 | 0.072 | N/A | 0.60 | 0.19 | 0.80 | 0.73 | 214.00 | 0.000 | 10.31 | 109.52 | 0.202 | N/A | N/A | N/A | | | 0.29 | 8.68 | 0.83 | 10.63 | | 68.405 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 68.569 | 6901.39 | 31.0 | 1.356 | 30.7 | 41.3 | 17.91 | 4.38% | Coarse Alkatum | 0.056 | 111.0 | 3.90 | 0.000 | 3.90 | 0 | 0.29 | 9.045 | 0.87 | 11.17 | 7 | 5.01% | 3.3 | 36% | 4.03 | 0.000 | 4.03 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.65 | 44.82 | 0.073 | N/A | 0.60 | 0.19 | 0.80 | 0.73 | 215.00 | 0.000 | 10.04 | 112.12 | 0.211 | N/A | N/A | N/A | | | 0.29 | 9.12 | 0.87 | 11.17 | | 68.569 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 68.733 | 6901.23 | 28.3 | 1.485 | 28.1 | 32.4 | 14.03 | 5.25% | Coarse Alkatum | 0.056 | 111.0 | 3.91 | 0.000 | 3.91 | 0 | 0.29 | 8.262 | 0.79 | 10.19 | 6 | 6.09% | 3.3 | 36% | 4.04 | 0.000 | 4.04 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.31 | 43.50 | 0.072 | N/A | 0.60 | 0.18 | 0.80 | 0.73 | 216.00 | 0.000 | 11.38 | 115.99 | 0.225 | N/A | N/A | N/A | | | 0.29 | 8.32 | 0.80 | 10.19 | | 68.733 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 68.897 | 6901.06 | 30.7 | 1.435 | 30.5 | 37.7 | 16.35 | 4.67% | Coarse Alkatum | 0.056 | 111.0 | 3.92 | 0.000 | 3.92 | 0 | 0.29 | 8.939 | 0.86 | 11.03 | 7 | 5.36% | 3.3 | 36% | 4.05 | 0.000 | 4.05 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.60 | 44.63 | 0.073 | N/A | 0.60 | 0.19 | 0.80 | 0.73 | 217.00 | 0.000 | 10.38 | 114.47 | 0.219 | N/A | N/A | N/A | | | 0.29 | 9.01 | 0.86 | 11.03 | | 68.897 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.061 | 6900.90 | 28.6 | 1.493 | 28.4 | 27.3 | 11.83 | 5.22% | Coarse Alkatum | 0.056 | 111.0 | 3.93 | 0.000 | 3.93 | 0 | 0.29 | 8.325 | 0.80 | 10.26 | 6 | 6.05% | 3.3 | 36% | 4.06 | 0.000 | 4.06 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.33 | 43.59 | 0.072 | N/A | 0.59 | 0.18 | 0.80 | 0.73 | 218.00 | 0.000 | 11.32 | 116.14 | 0.226 | N/A | N/A | N/A | | | 0.29 | 8.38 | 0.80 | 10.26 | | 69.061 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.225 | 6900.74 | 32.7 | 1.426 | 32.5 | 28.4 | 12.32 | 4.36% | Coarse Alkatum | 0.056 | 111.0 | 3.94 | 0.000 | 3.94 | 0 | 0.29 | 9.503 | 0.91 | 11.70 | 7 | 4.96% | 3.2 | 36% | 4.06 | 0.000 | 4.06 | 0 | 0 | 0.50 | 0.05 | 0.93 | 0.10 | #### | 0.000 | 33.83 | 45.53 | 0.073 | N/A | 0.59 | 0.20 | 0.80 | 0.73 | 219.00 | 0.000 | 9.74 | 114.00 | 0.218 | N/A | N/A | N/A | | | 0.29 | 9.55 | 0.91 | 11.70 | | 69.225 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.389 | 6900.57 | 45.3 | 1.300 | 45.1 | 30.4 | 13.18 | 2.87% | Coarse Alkatum | 0.056 | 111.0 | 3.95 | 0.000 | 3.95 | 0 | 0.29 | 13.148 | 1.26 | 16.17 | 10 | 3.15% | 3.0 | 36% | 4.07 | 0.000 | 4.07 | 0 | 0 | 0.50 | 0.05 | 0.92 | 0.10 | #### | 0.000 | 35.37 | 51.54 | 0.079 | N/A | 0.59 | 0.23 | 0.80 | 0.73 | 220.00 | 0.000 | 6.66 | 107.70 | 0.196 | N/A | N/A | N/A | | | 0.29 | 13.20 | 1.26 | 16.17 | | 69.389 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.553 | 6900.41 | 59.5 | 1.179 | 59.4 | 19.2 | 8.30 | 1.98% | Coarse Alkatum | 0.056 | 111.0 | 3.96 | 0.000 | 3.96 | 0 | 0.29 | 17.357 | 1.66 | 21.30 | 14 | 2.12% | 2.8 | 36% | 4.08 | 0.000 | 4.08 | 0 | 0 | 0.50 | 0.05 | 0.91 | 0.10 | #### | 0.000 | 37.14 | 58.44 | 0.086 | N/A | 0.59 | 0.27 | 0.80 | 0.72 | 221.00 | 0.000 | 4.70 | 100.17 | 0.173 | N/A | N/A | N/A | | | 0.29 | 17.39 | 1.67 | 21.30 | | 69.553 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.717 | 6900.24 | 69.4 | 1.177 | 69.4 | 14.2 | 6.16 | 1.70% | Coarse Alkatum | 0.056 | 111.0 | 3.97 | 0.000 | 3.97 | 0 | 0.31 | 21.220 | 2.03 | 26.02 | 17 | 1.80% | 2.7 | 36% | 4.09 | 0.000 | 4.09 | 0 | 0 | 0.50 | 0.06 | 0.91 | 0.10 | #### | 0.000 | 38.77 | 64.79 | 0.093 | N/A | 0.59 | 0.29 | 0.80 | 0.72 | 222.00 | 0.000 | 3.94 | 102.48 | 0.180 | N/A | N/A | N/A | | | 0.31 | 21.25 | 2.03 | 26.02 | | 69.717 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |
| 69.881 | 6900.08 | 73.4 | 1.272 | 73.3 | 13.3 | 5.75 | 1.73% | Coarse Alkatum | 0.056 | 111.0 | 3.97 | 0.000 | 3.97 | 0 | 0.31 | 22.790 | 2.18 | 27.94 | 17 | 1.83% | 2.7 | 36% | 4.10 | 0.000 | 4.10 | 0 | 0 | 0.49 | 0.06 | 0.91 | 0.10 | #### | 0.000 | 39.43 | 67.37 | 0.096 | N/A | 0.59 | 0.31 | 0.80 | 0.72 | 223.00 | 0.000 | 3.83 | 106.94 | 0.194 | N/A | N/A | N/A | | | 0.31 | 22.82 | 2.18 | 27.94 | | 69.881 | 1.00 | - | - | - | - | - | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - |

| UNC-NECR WASTE REPOSITORY LIQUEFACTION ANALYSIS - CPT-02 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--|----------------------------|---|-------------------------|---|-------------------|---|-----------------------------------|---|--|---|-------|--|--|--|---|----------------------------|--|-------------------------|-------------------|-------------------|---------------------------------|-----------------------------------|-----------------------------------|--|---|--------------|--------------------|--------|--------|-----|-------|-------|-------|-------|------|------|-------|------|------------|--------|--------|-----|-------|-------|-------|-------|------|------|-------|------|-------------|--------|--------|-----|-------|-------|-------|-------|------|------|-------|-------|
| Data File: 13-52118_RP02-BSC-CPT | | Cells Requiring User Input/Manipulation | | Idriss and Boulanger (2008) | | 0.00 Water surface elevation during CPT investigation (ft amsl) | | 6960.16 Ground Surface Elevation at time of CPT (ft amsl) | | <div>Proposed Repository</div> <table><thead><tr><th></th><th>Elev. at Top of Layer (ft)</th><th>Midpoint Elev. at Bottom of Layer (ft)</th><th>Thickness of Layer (ft)</th><th>Unit Weight (pcf)</th><th>Unit Weight (pcf)</th><th>Stress at Bottom of Layer (tsf)</th><th>Stress at Midpoint of Layer (tsf)</th><th>Pressure at Bottom of Layer (tsf)</th><th>Pore Pressure at Bottom of Layer (tsf)</th><th>Effective Stress at Bottom of Layer (tsf)</th><th>Stress Ratio</th></tr></thead><tbody><tr><td>Erosion Protection</td><td>6972.9</td><td>6971.4</td><td>1.5</td><td>0.057</td><td>113.3</td><td>0.262</td><td>0.177</td><td>0.00</td><td>0.00</td><td>0.262</td><td>0.04</td></tr><tr><td>Cover Soil</td><td>6972.9</td><td>6971.4</td><td>1.5</td><td>0.057</td><td>113.3</td><td>0.262</td><td>0.177</td><td>0.00</td><td>0.00</td><td>0.262</td><td>0.04</td></tr><tr><td>Mine Spoils</td><td>6969.9</td><td>6965.1</td><td>4.8</td><td>0.058</td><td>116.4</td><td>0.831</td><td>0.547</td><td>0.00</td><td>0.00</td><td>0.831</td><td>0.547</td></tr></tbody></table> | | | | | | Elev. at Top of Layer (ft) | Midpoint Elev. at Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Stress at Bottom of Layer (tsf) | Stress at Midpoint of Layer (tsf) | Pressure at Bottom of Layer (tsf) | Pore Pressure at Bottom of Layer (tsf) | Effective Stress at Bottom of Layer (tsf) | Stress Ratio | Erosion Protection | 6972.9 | 6971.4 | 1.5 | 0.057 | 113.3 | 0.262 | 0.177 | 0.00 | 0.00 | 0.262 | 0.04 | Cover Soil | 6972.9 | 6971.4 | 1.5 | 0.057 | 113.3 | 0.262 | 0.177 | 0.00 | 0.00 | 0.262 | 0.04 | Mine Spoils | 6969.9 | 6965.1 | 4.8 | 0.058 | 116.4 | 0.831 | 0.547 | 0.00 | 0.00 | 0.831 | 0.547 |
| | Elev. at Top of Layer (ft) | Midpoint Elev. at Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Stress at Bottom of Layer (tsf) | Stress at Midpoint of Layer (tsf) | Pressure at Bottom of Layer (tsf) | Pore Pressure at Bottom of Layer (tsf) | | | | | | Effective Stress at Bottom of Layer (tsf) | Stress Ratio | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Erosion Protection | 6972.9 | 6971.4 | 1.5 | 0.057 | 113.3 | 0.262 | 0.177 | 0.00 | 0.00 | | | | | | 0.262 | 0.04 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Cover Soil | 6972.9 | 6971.4 | 1.5 | 0.057 | 113.3 | 0.262 | 0.177 | 0.00 | 0.00 | 0.262 | 0.04 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Mine Spoils | 6969.9 | 6965.1 | 4.8 | 0.058 | 116.4 | 0.831 | 0.547 | 0.00 | 0.00 | 0.831 | 0.547 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Location: UNC-NECR 2015 Mill Site PDS | | Cells Requiring User Input/Manipulation | | Max. Horiz. Acceleration, A _{max} : 0.3 | | 0.00 Water surface elevation at t ₁ (ft amsl) | | 6974.44 Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| http://projects.mhglobal.com/~13-52118_RP02-BSC-CPT.xls | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 5.5 | | 0.00 Water surface elevation at t ₂ (ft amsl) | | 1.50 Thickness of Erosion Protection Layer (rock mulchpots) Immediately after placement | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Erosion Protection | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.69 | | 0.00 Water surface elevation at t ₃ (ft amsl) | | 3.00 Thickness of Water Storage/Rooting Zone (Cover Soil; ft) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Coarse Protection | | Cells Requiring User Input/Manipulation | | Youd, et al (2001) | | 0.00 Water surface elevation at t ₄ (ft amsl) | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Cover Soil | | Cells Requiring User Input/Manipulation | | Max. Horiz. Acceleration, A _{max} : 0.3 | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 6945.15 Elevation of bottom of tailings (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Mine Spoils | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation during CPT investigation (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Radon Barrier | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| General Fill | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₁₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₁₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₁₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₁₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₁₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₁₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₂₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₂₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₂₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₂₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₂₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₂₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₂₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₂₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₂₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₂₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₃₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₃₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₃₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₃₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₃₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₃₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₃₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₃₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₃₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₃₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₄₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₄₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₄₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₄₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₄₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₄₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₄₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₄₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₄₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₅₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₅₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₅₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₅₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₅₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₅₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₅₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₆₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₆₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₆₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₆₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₆₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₆₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₆₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₆₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₆₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₆₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₇₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₇₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₇₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₇₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₇₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₇₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₇₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₇₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₇₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₇₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₈₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₈₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₈₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₈₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₈₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₈₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₈₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₈₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₈₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₈₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₉₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₉₁ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₉₂ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₉₃ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₉₄ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₉₅ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₉₆ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₉₇ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | 1.44 Scaling Factor for stress ratio, r _{sc} | | 0.83 Additional Stress due to Proposed Repository Construction, Δσ _{res} (psf) | | 0.00 Water surface elevation at t ₉₈ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Earthquake Moment Magnitude, M _t : 6.3 | | 0.47 Volumetric Strain Ratio for Site-Specific Design Earthquake | | 0.00 Water surface elevation at t ₉₉ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | Magnitude Scaling Factor, MSF: 1.59 | | 11.02 Equiv. Number of Uniform Stress Cycles, N | | 0.00 Water surface elevation at t ₁₀₀ (ft amsl) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| | |
|----------|--|
| 69976.04 | Ground Surface Elevation at time of CPT (ft amsl) |
| 69990.26 | Ground Surface Elevation Immediately after Placement of Final Cover (ft amsl) |
| 1.50 | Thickness of Erosion Protection Layer (rock mulch/topsoil) Immediately after placement |
| 3.00 | Thickness of Water Storage/Rooting Zone (Cover Soil; ft) |

| | |
|---------|---|
| 0.83 | Additional Stress due to Reprofit Repository Construction, $\Delta\sigma_{\text{repo}}$ (psf) |
| 6931.50 | Elevation of bottom of tailings (ft amsl) |

| Liquefaction Triggering Analyses Using SPT Data | | | | | | | | | | | | | | | | |
|---|--|--|-----------------------------------|---|---------------------------------------|---|--|-----------------------------|-----|------|--------------------|---|---------------|-----|----------------|--|
| General Data | | | | | | | | Idriss & Boulanger (2008) | | | Youd et al. (2001) | | | | | |
| Depth of SPT (ft) | Field Blow Count, N ₆₀ (blows/ ft) | Root Length Corrected Factor, C _R | N ₆₀ (blows/ ft) | (N _s) _{FS} (blows/ft) | Relative Density D _r | m | SPT Corrected on Factor C _d | (CRR) eq-3 | FoS | α | β | (N _s) _{FS} (blows/ft) | (CRR) eq-4 | FoS | Average FoS | |
| | | | | | | | | $\frac{A(N_s)_{FS}}{(CRR)}$ | | | | | | | | |
| 0.184 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 0.328 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 0.492 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 0.656 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 0.820 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 0.984 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.148 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.312 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.476 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.640 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.804 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 1.968 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.133 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.297 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.461 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.625 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.789 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 2.953 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.117 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.281 | - | 0.75 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.445 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.609 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.773 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 3.937 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.101 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.265 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.429 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.593 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.757 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 4.921 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.085 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.249 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.413 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.577 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.741 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 5.905 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.069 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.234 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.398 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.562 | - | 0.80 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.726 | - | 0.85 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 6.890 | - | 0.85 | - | - | - | - | - | - | - | 5.00 | 1.20 | - | 0.000 | - | - | |
| 7.054 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 7.218 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 7.382 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 7.546 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 7.710 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 7.874 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.038 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.202 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.366 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.530 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.694 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 8.858 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.022 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.186 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.350 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.514 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.678 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 9.842 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.006 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.170 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.335 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.499 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.663 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.827 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 10.991 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.155 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.319 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.483 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.647 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.811 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 11.975 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.139 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.303 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.467 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.631 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.795 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 12.959 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.123 | - | 0.85 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.287 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.451 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.615 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.779 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 13.943 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.107 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.271 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.436 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.600 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.764 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 14.928 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |
| 15.092 | - | 0.95 | - | - | - | - | - | - | - | 3.73 | 1.08 | - | 0.000 | - | - | |

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| UNC-NECR WASTE REPOSITORY LIQUEFACTION ANALYSIS - CPT-10 | | | | | | | | | |
|---|--|---|--|-----------------------------------|--|---------|--|--|--|
| Data File: | | 13-52118_RP10-BSC-CPT | | | | | | | |
| Location: | | UNC-NECR 2013 Mill Site PDS | | | | | | | |
| | | Cells Requiring User Input/Manipulation | | | | | | | |
| http://projects.mhglobal.com/~13-52118_RP10-BSC-CPT.XLS | | | | Idriss and Boulanger (2008) | | 6883.40 | | Water surface elevation during CPT investigation (ft amsl) | |
| | | | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | | Water surface elevation at t, (ft amsl) | |
| | | | | Earthquake Moment Magnitude, M: | | 5.5 | | 6883.40 | |
| | | | | Magnitude Scaling Factor, MSF: | | 1.69 | | Water surface elevation at t, (ft amsl) | |
| | | | | Youd, et al (2001) | | | | | |
| | | | | Max. Horiz. Acceleration, Amax/g: | | 0.3 | | 1.44 | |
| | | | | Earthquake Moment Magnitude, M: | | 6.3 | | 0.47 | |
| | | | | Magnitude Scaling Factor, MSF: | | 1.59 | | 11.02 | |
| | | | | | | | | Equiv. Number of Uniform Stress Cycles, N | |

| | | | | | | | | | | | | |
|---------------------|----------------------------|------------------------|-------------------------------|-------------------------|-------------------|-------------------|---------------------------------|-----------------------------------|-----------------------------------|---------------------------|---|---------------|
| Proposed Repository | Elev. at Top of Layer (ft) | Midpoint of Layer (ft) | Elev. at Bottom of Layer (ft) | Thickness of Layer (ft) | Unit Weight (pcf) | Unit Weight (pcf) | Stress at Bottom of Layer (tsf) | Stress at Midpoint of Layer (tsf) | Pressure at Bottom of Layer (tsf) | Pore Pressure at Midpoint | Effective Stress at Bottom of Layer (tsf) | e at Midpoint |
| | 6994.6 | 6993.9 | 6993.1 | 1.5 | 0.061 | 122.9 | 0.092 | 0.046 | 0.00 | 0.00 | 0.092 | 0.046 |
| | 6993.1 | 6991.6 | 6990.1 | 3.0 | 0.057 | 113.3 | 0.262 | 0.177 | 0.00 | 0.00 | 0.262 | 0.177 |
| | 6990.1 | 6981.9 | 6973.6 | 16.5 | 0.058 | 116.4 | 1.222 | 0.742 | 0.00 | 0.00 | 1.222 | 0.742 |

| Liquefaction Triggering Analyses Using CPT Data | | | | | | | | | |
|---|--|--|--|--|--|--|--|--|--|
| Idriss & Boulanger (2008) | | | | | | | | | |
| Youd et al (2001) | | | | | | | | | |
| Idriss & Boulanger (2008) | | | | | | | | | |
| CN | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1N | | | | | | | | | |
| FoS | | | | | | | | | |
| Average FoS | | | | | | | | | |
| Liquefaction? | | | | | | | | | |
| 1=Yes | | | | | | | | | |
| 0=No | | | | | | | | | |

| 2013 CPT Data from ConeTec | | | | | | | | | |
|------------------------------|--|--|--|--|--|--|--|--|--|
| CPT Data Interpretations | | | | | | | | | |
| Conditions at t ₀ | | | | | | | | | |
| Idriss & Boulanger (2008) | | | | | | | | | |
| Youd et al (2001) | | | | | | | | | |
| Idriss & Boulanger (2008) | | | | | | | | | |
| CN | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1N | | | | | | | | | |
| FoS | | | | | | | | | |
| Average FoS | | | | | | | | | |
| Liquefaction? | | | | | | | | | |
| 1=Yes | | | | | | | | | |
| 0=No | | | | | | | | | |

| General Data | | | | | | | | | |
|---------------------------|--|--|--|--|--|--|--|--|--|
| Idriss & Boulanger (2008) | | | | | | | | | |
| Youd et al (2001) | | | | | | | | | |
| CN | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1 | | | | | | | | | |
| qc1N | | | | | | | | | |
| FoS | | | | | | | | | |
| Average FoS | | | | | | | | | |
| Liquefaction? | | | | | | | | | |
| 1=Yes | | | | | | | | | |
| 0=No | | | | | | | | | |

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|---------|-------|-------|-------|-----|------|--------|-----------------|-------|-------|------|-------|------|---|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|
| 8691.07 | 305.3 | 2.953 | 305.2 | 4.1 | 1.91 | 0.078% | Coarse Alliumin | 0.056 | 111.0 | 4.81 | 0.000 | 4.81 | 0 | 0.58 | 176.51 | 16.98 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 | N/A | N/A | N/A | 0.58 | 176.77 | 16.92 | 216.45 | 63 | 0.99% | 21 | 36% | 5.06 | 0.000 | 5.06 | 0 | 0 | 0.43 | 0.30 | 0.45 | -0.60 | 1.00 | 0.109 | 104.45 | 320.90 | 1.000 | N/A | 0.43 | 0.85 | 0.60 | 0.48 | 1.00 | 0.109 | 140 | 303.52 | 1.000 |
|---------|-------|-------|-------|-----|------|--------|-----------------|-------|-------|------|-------|------|---|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|-----|-----|-----|------|--------|-------|--------|----|-------|----|-----|------|-------|------|---|---|------|------|------|-------|------|-------|--------|--------|-------|-----|------|------|------|------|------|-------|-----|--------|-------|

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ATTACHMENT G.7
Cover System Design Report – Dwyer Engineering, LLC

95% DRAFT

ADDENDUM TO COVER SYSTEM DESIGN REPORT



09/10/2019

Northeast Church Rock Site Closure

Prepared by:

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Prepared for:

United Nuclear Corporation
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ADDENDUM SUMMARY

This document is an addendum to the Northeast Church Rock 95% Design Report, Appendix G, Attachment G.7, 95% Revised Draft Cover Design Report issued in July 2018 (Dwyer 2018).

The reason for the addendum is the addition of the soil to be excavated from the Jetty area to be included as the primary borrow source for cover material. The information presented in this addendum demonstrates that the cover performance requirements consistent with Dwyer (2018) are satisfied with the primary cover soil borrow source being soil excavated from the Jetty area.

The difference in cover profile recommended in this addendum compared with the original submittal (Dwyer 2018) includes:

- Cover profile is now recommended to be 4.5 feet thick instead of the previously recommended 4 feet. The thicker cover profile is due to the inclusion of the soil excavated from the Jetty area as the primary cover soil borrow source. Refer to Sections 3 through 5.
- The largest rock size for the admixture is now 3.5 inches with an admixture depth of 31.5 inches instead of the previously largest size of 3 inches with an admixture depth of 27 inches. The reason for the increased rock size and thus increased admixture depth is due to the potential for higher fines content from the soil to be excavated from the Jetty area to be used as cover material. Refer to Section 2.

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ACRONYMS AND ABBREVIATIONS

| | |
|-----------|--|
| ARAR | Applicable or Relevant and Appropriate Requirement |
| ASTM | American Society for Testing and Materials |
| CFR | Code of Federal Regulations |
| cm | centimeter |
| cy | cubic yards |
| DOE | Department of Energy |
| DRAIN | drainage |
| DWYER | Dwyer Engineering, LLC |
| ET | evapotranspiration |
| EVAP | evaporation |
| Mill Site | Church Rock Mill Site |
| NECR | Northeast Church Rock |
| NOAA | National Oceanic and Atmospheric Administration |
| NRC | Nuclear Regulatory Commission |
| PET | potential evapotranspiration |
| PMP | Probable Maximum Precipitation |
| PODR | Point of Diminishing Returns |
| PRECIP | precipitation |
| RAECOM | Radiation Attenuation Effectiveness and Cover Optimization with Moisture Effects |
| TRANSP | transpiration |
| USEPA | United States Environmental Protection Agency |

1.0 OVERVIEW OF DESIGN CHANGES

Mine waste will be disposed of in a repository designed within the footprint of the existing tailings impoundment at the Mill Site. These materials will then be capped with a final cover system, referred to as an evapotranspiration (ET) cover.

Changes to the final cover design recommended in this addendum compared with the original submittal (Dwyer 2018) include:

- Total cover profile thickness increased from 4 feet to 4.5 feet.
- Surface rock/soil admixture layer: rock size range now 1.5 inches diameter to 3.5 inches diameter (prior range 1.5 inches diameter to 3 inches diameter); and layer thickness range is 14 inches to 31.5 inches (prior range was 14 inches to 27 inches).

1.1 Erosion Protection

The Northeast Church Rock (NECR) site is located in an arid climate and exposed to erosion due to high-intensity precipitation events and significant wind. A key performance criterion for the cover system design is to provide for adequate erosion protection and long-term stability. The design event for evaluation of long-term erosional stability is the Probable Maximum Precipitation (PMP) based on Nuclear Regulatory Commission (NRC) guidelines in NUREG-1623 (NRC, 2002). The designed cover system is capable of withstanding the windy conditions at the site as well as a rainfall intensity defined by the PMP event that is 6.5 inches for the 1-hour precipitation frequency [*Note: the PMP is significantly more conservative than the 2.96 inch value provided for the 1-hour precipitation frequency for a 1000 year period defined in the National Oceanic and Atmospheric Administration (NOAA) Atlas 14, Volume 1 (Bonnin, 2011)*].

The cover design is composed of two layers. The top layer is a uniform mixture of cover soil (67 percent by volume) with rock (33 percent by volume) referred to as a ‘desert pavement’. This layer is designed to mitigate erosion by creating an armored surface with rock large enough to resist the erosive forces created during a PMP event. The bottom layer is composed of cover soil only. The reason for the admixture design change is due to the inclusion of the soil to be excavated from the Jetty area (Stantec 2018) as borrow. This soil has a maximum fines content of 96%. The fines content from the previously approved cover soil borrow sources was 57% (Dwyer 2018).

Section 2 describes the calculations performed to support the design of the ‘desert pavement’ and analysis demonstrating the effectiveness of this layer due to the change in the cover soil borrow source. The admixture design is a function of the climate, soil, and cover surface geometry. The final geometry of the impoundment after placement of the mine spoils and cover system includes slopes generally less than 5 percent with slope lengths as long as 1,021 feet. In the design of the ‘desert pavement’ layer, the smallest rock possible while still meeting the erosion requirements is preferred. Rock has no storage capacity, thus the overall thickness of cover is increased to overcome the loss of storage capacity from the addition of the rock. The rock to be used in the surface admixture layer or ‘desert pavement’ shall be durable to meet the 1,000-year design life requirement. This rock may not be readily available or may be expensive to acquire. Thus, the intent of this design is to maximize use of existing rock at the site meeting applicable durability requirements. Consequently, the thickness of the two layers varies depending on the location

and respective slope length. The depth of rock/soil admixture and rock size was varied along the slope length (refer to Tables 1 to 4). The thickest admixture layer is 31.5 inches with a rock size of 3.5 inches. The previous thickest admixture depth was 27 inches with a maximum rock size of 3 inches (Dwyer 2018).

1.2 Cover Depth Required to Minimize Flux

The overall cover thickness including both layers is a consistent 4.5 feet. The previously recommended cover depth was 4 feet (Dwyer 2018). The reason for the change is the inclusion of the soil excavated from the Jetty area as the primary borrow source for cover material.

A series of computer simulations of the two-layered cover profile were performed utilizing properties from the Jetty area soil to evaluate the myriad of variables including vegetation maturation to which the cover could be exposed during the 1,000-year performance period. The model geometry varied based on the respective rock/soil admixture design and depth described in Section 2. The computer sensitivity analyses evaluated possible climate change scenarios over this period of performance also considering information from the USEPA climate change website (<https://www.epa.gov/climatechange>). The full cover profile depth of 4.5 feet was shown to effectively minimize flux (in most cases reduce it to zero) for the multitude of scenarios modeled. The specifics of the modeling performed are contained in Sections 3 through 5.

1.3 Radon Flux Evaluation

Section 6 provides an overview of the estimated radon release rate through the cover profile. The radon flux through the cover soil was calculated using the Radiation Attenuation Effectiveness and Cover Optimization with Moisture Effects (RAECOM) code, as described in (Rogers 1984a, 1984b). The model is used to perform one-dimensional, steady-state radon diffusion calculations for a multi-layer system. The computed radon flux for the cover profile is 6.23 pCi/m²s (Table 13). This value is less than the maximum allowable value of 20 pCi/m²s per 40 CFR 192.02.

2.0 EROSION

A surface layer composed of a uniform mixture of rock and soil is included in the cover profile to mitigate erosion and provide long-term stability (NRC, 2002). Because the Jetty area cover soil borrow source potentially has higher fines content than the previously approved cover soil borrow sources (Dwyer 2018), the admixture design was adjusted to account for this since soil fines content is a sensitive parameter in this design process.

2.1 DESIGN OF COVER SURFACE LAYER (ROCK/SOIL ADMIXTURE)

The cover surface layer is composed of a mixture of rock and cover soil designed to mitigate the potential for rill or gully formation as well as minimize soil loss. This admixture design varies (rock size and depth) depending on the specific slope and slope length (Tables 1 through 4). For example, the base of a long slope may have a large rock within the admixture and greater admixture thickness compared to the top of the slope. This erosion resistant admixture was designed consistent with guidance summarized in Dwyer et al. (2007) and USEPA (2012). The admixture design was reworked to include the worst case soils from the soils to be excavated from the Jetty area and used as cover material. For the admixture design, the worst case includes soil with the highest fines content because the smaller soil particles are more erodible. The highest fines content for the Jetty area cover soil borrow source is 96% (Stantec 2018).

Because the cover system rock requirement is substantial and the slope lengths are varied, admixture requirements were computed in intervals along the slope length. That is, the top of the slope has less erosive forces because the slope length is shorter than the bottom of the slope. Tables 1 to 4 summarize the analysis performed.

Section 2.1.1 presents calculated results for the rock/soil admixture for the 5 percent slope with total slope length of 1,021 feet. Section 2.1.2 presents calculated results for the rock/soil admixture for the 2 percent slope with total slope length of 1,000 feet.

2.1.1 Admixture Design for Single Slope Length of 1,021 feet at 5 Percent Slope

The following rock/soil admixture design is for the final cover system's top surface where slopes are 5 percent. Table 1 summarizes the admixture calculations using a fines content of 96% consistent with the method and intensity values described and utilized in Dwyer (2018).

The critical particle sizes (D_c) and admixture depth are computed based on the slope length location along the 5 percent slope. For slope lengths 900 feet and longer, a 3.5-inch rock is required with a corresponding admixture depth of 31.5 inches. For a slope length of 525 feet to 900 feet, a 3-inch rock is required with a corresponding admixture depth of 27 inches. For a slope length of 350 feet to 525 feet, a 2-inch rock is required with a corresponding admixture depth of 18 inches. From the top of the slope to a slope length of 350 feet, a 1.5-inch rock is required with a corresponding admixture depth of 14 inches. Layer mixtures will be 33 percent rock to 67 percent soil by volume for the full admixture depth.

Table 1. Admixture Design Summary

| I (in/hr) | S (%) | Slope Length (feet) | A (acres) | Q (cfs) | Qm (cfs) | b (feet) | dH (in) | τ (psf) | Rock Size Dc (in) | use Rock Size Dc (in) | % gravel | Ya (in) | Ys (in) | Admix Depth (in) | Comment |
|--------------|----------|---------------------------|--------------|------------|-------------|-------------|------------|-----------------|-------------------------|--------------------------------|-------------|------------|------------|------------------------|---|
| 28.2 | 5 | 1021 | 5.98 | 50.66 | 5.07 | 13.5 | 5.75 | 1.494 | 3.6 | 3.50 | 33% | 10.5 | 21 | 31.5 | |
| 28.3 | 5 | 1000 | 5.74 | 48.77 | 4.88 | 13.3 | 5.66 | 1.471 | 3.5 | 3.50 | 33% | 10.5 | 21 | 31.5 | |
| 28.8 | 5 | 900 | 4.65 | 40.20 | 4.02 | 12.4 | 5.22 | 1.358 | 3.3 | 3.50 | 33% | 10.5 | 21 | 31.5 | |
| 29.4 | 5 | 800 | 3.67 | 32.38 | 3.24 | 11.4 | 4.78 | 1.242 | 3.0 | 3.00 | 33% | 9.0 | 18 | 27.0 | |
| 30.0 | 5 | 700 | 2.81 | 25.32 | 2.53 | 10.4 | 4.31 | 1.122 | 2.7 | 3.00 | 33% | 9.0 | 18 | 27.0 | |
| 30.7 | 5 | 600 | 2.07 | 19.06 | 1.91 | 9.3 | 3.84 | 0.997 | 2.4 | 3.00 | 33% | 9.0 | 18 | 27.0 | |
| 31.4 | 5 | 525 | 1.58 | 14.90 | 1.49 | 8.5 | 3.46 | 0.900 | 2.2 | 3.00 | 33% | 9.0 | 18 | 27.0 | |
| 31.6 | 5 | 500 | 1.43 | 13.60 | 1.36 | 8.2 | 3.34 | 0.867 | 2.1 | 2.00 | 33% | 6.0 | 12 | 18.0 | |
| 32.6 | 5 | 400 | 0.92 | 8.99 | 0.90 | 7.0 | 2.81 | 0.731 | 1.8 | 2.00 | 33% | 6.0 | 12 | 18.0 | |
| 33.3 | 5 | 350 | 0.70 | 7.02 | 0.70 | 6.4 | 2.54 | 0.660 | 1.6 | 2.00 | 33% | 6.0 | 12 | 18.0 | |
| 34.0 | 5 | 300 | 0.52 | 5.26 | 0.53 | 5.7 | 2.25 | 0.586 | 1.4 | 1.50 | 33% | 4.5 | 9 | 14.0 | |
| 35.8 | 5 | 200 | 0.23 | 2.47 | 0.25 | 4.3 | 1.65 | 0.428 | 1.0 | 1.50 ¹ | 33% | 4.5 | 9 | 14.0 | increased rock size per Section 4.2 (NUREG-1623) ¹ |
| 38.8 | 5 | 100 | 0.06 | 0.67 | 0.07 | 2.6 | 0.96 | 0.249 | 0.6 | 1.50 | 33% | 4.5 | 9 | 14.0 | |

¹ Values highlighted (slope lengths 200-feet) required an increase in Dc to meet NUREG-1623 requirements. Refer to Table 2.

Table 2 summarizes the calculations to verify compliance with the long-term stability of a rocky soil slope per NUREG-1623. The slope lengths utilized in the calculations correspond to those from Table 1. The slope is 5 percent, consequently any calculation that computed a stable slope of greater than 5 percent with the particle size determined in Table 1 was acceptable. However, for the slope length of 200 feet, the Dc was increased to satisfy the long-term stability of a rocky soil slope per NUREG-1623.

Table 2. Long-Term Stability of Rocky Soil 5% Slope (NUREG 1623)

| Rock Size (D75) | t | P | L | Calculated Ss | Comments |
|------------------|-----|---------|------|---------------|--|
| 3.50 | 1.4 | 28.228 | 1021 | 6.9% | The calculated stable slope is greater than 5% (the slope to be built), therefore the Dc for rock size summarized in Table 1 is more conservative |
| 3.50 | 1.4 | 28.3266 | 1000 | 7.0% | The calculated stable slope is greater than 5% (the slope to be built) |
| 3.50 | 1.4 | 28.8265 | 900 | 7.5% | The calculated stable slope is greater than 5% (the slope to be built) |
| 3.00 | 1.2 | 29.3851 | 800 | 6.6% | The calculated stable slope is greater than 5% (the slope to be built) |
| 3.00 | 1.2 | 30.0175 | 700 | 7.2% | The calculated stable slope is greater than 5% (the slope to be built) |
| 3.00 | 1.2 | 30.7456 | 600 | 8.1% | The calculated stable slope is greater than 5% (the slope to be built) |
| 3.00 | 1.2 | 31.3739 | 525 | 8.9% | The calculated stable slope is greater than 5% (the slope to be built) |
| 2.00 | 0.8 | 31.6028 | 500 | 5.2% | The calculated stable slope is greater than 5% (the slope to be built) |
| 2.00 | 0.8 | 32.644 | 400 | 6.1% | The calculated stable slope is greater than 5% (the slope to be built) |
| 2.00 | 0.8 | 33.2619 | 350 | 6.7% | The calculated stable slope is greater than 5% (the slope to be built) |
| 1.5 | 0.6 | 33.9694 | 300 | 5.0% | The calculated stable slope is greater than 5% (the slope to be built) |
| 1.5 ¹ | 0.6 | 35.7948 | 200 | 6.7% | As noted in Table 1, the rock size using this formula from NUREG-1623 governs for this slope length. The calculated stable slope is greater than 5% (the slope to be built), therefore |

| Rock Size (D75) | t | P | L | Calculated Ss | Comments |
|-----------------|-----|---------|-----|---------------|---|
| | | | | | the Dc for rock size developed in Section 4.1 is more conservative ¹ |
| 1.5 | 0.6 | 38.7645 | 100 | 11.4% | The calculated stable slope is greater than 5% (the slope to be built), therefore the Dc for rock size summarized in Table 1 is more conservative |

¹ Values highlighted (slope length 200 feet) required an increase in D₇₅ from the Dc value computed using the methods summarized in Table 1 to meet NUREG-1623 requirements.

2.1.2 Admixture Design for Single Slope Length of 1,000 feet at 2 Percent Slope

The following rock/soil admixture design is for the final cover system's top surface where slopes are 2 percent. Table 3 summarizes the admixture calculations using a fines content of 96% following the method and intensity values described and utilized in Dwyer (2018).

The critical particle sizes (Dc) and admixture depth are computed based on the slope length along the 2 percent slope. All mixtures are 33 percent rock to 67 percent soil by volume for the full admixture depth. All slope lengths required a rock size less than 1.5 inches in diameter. However, the rock was adjusted to the 1.5-inch diameter to utilize the 1.5-inch rock available at the site.

Table 3. Admixture Design Summary for Cover 2% Slope

| I (in/hr) | S (%) | Slope Length (feet) | A (acres) | Q (cfs) | Qm (cfs) | b (feet) | dH (in) | τ (psf) | Dc (in) | use Dc (in) ¹ | % Gravel | Ya (in) | Ys (in) | Admix Depth (in) | Comment ¹ |
|--------------|----------|---------------------------|--------------|------------|-------------|-------------|------------|-----------------|------------|-----------------------------|-------------|------------|------------|------------------------|---|
| 24.0 | 2 | 1000 | 5.739 | 41.35 | 4.13 | 12.53 | 5.28 | 0.550 | 1.3 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 24.5 | 2 | 900 | 4.649 | 34.18 | 3.42 | 11.66 | 4.88 | 0.508 | 1.2 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 25.1 | 2 | 800 | 3.673 | 27.61 | 2.76 | 10.75 | 4.47 | 0.465 | 1.1 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 25.7 | 2 | 700 | 2.812 | 21.67 | 2.17 | 9.80 | 4.04 | 0.421 | 1.0 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 26.4 | 2 | 600 | 2.066 | 16.37 | 1.64 | 8.81 | 3.60 | 0.375 | 0.9 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 27.3 | 2 | 500 | 1.435 | 11.74 | 1.17 | 7.77 | 3.14 | 0.326 | 0.8 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 28.3 | 2 | 400 | 0.918 | 7.80 | 0.78 | 6.65 | 2.65 | 0.276 | 0.7 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 29.7 | 2 | 300 | 0.517 | 4.60 | 0.46 | 5.44 | 2.13 | 0.222 | 0.5 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 31.6 | 2 | 200 | 0.230 | 2.18 | 0.22 | 4.09 | 1.56 | 0.163 | 0.4 | 1.5 | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |
| 34.8 | 2 | 100 | 0.057 | 0.60 | 0.06 | 2.51 | 0.92 | 0.095 | 0.2 | 1.5 ¹ | 33 | 4.5 | 9 | 14.0 | increased rock size to utilize existing 1.5-inch rock on site |

¹ Adjusted the Dc to meet the available 1.5-inch rock on-site

Table 4 summarizes the calculations to verify compliance the long-term stability of a rocky soil slope per NUREG-1623. The slope lengths utilized in the calculations correspond to those from Table 3. For all slope lengths, the Dc was increased in the computations to satisfy the long-term stability of a rocky soil slope per NUREG-1623.

Table 4. Long-Term Stability of Rocky Soil 2% Slope (NUREG-1623)

| D75 ¹ | t | P | L | Calculated Ss | Comments ¹ |
|------------------|-----|---------|------|---------------|--|
| 1.5 | 0.6 | 24.0158 | 1000 | 2.4% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 24.5048 | 900 | 2.6% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 25.0542 | 800 | 2.8% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 25.6803 | 700 | 3.1% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 26.4064 | 600 | 3.4% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 27.2686 | 500 | 3.9% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 28.3266 | 400 | 4.6% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 29.6909 | 300 | 5.6% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 31.6028 | 200 | 7.5% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |
| 1.5 | 0.6 | 34.7971 | 100 | 12.5% | The calculated stable slope (Ss) is greater than the slope to be built of 2%, therefore OK |

¹ Adjusted the Dc to meet the available 1.5-inch rock on-site

3.0 UNSATURATED MODELING OF COVER SYSTEM

Modeling was performed to determine an ET cover profile that minimizes flux given the myriad of input parameters (cover soil and vegetation data) and historical climate data for the site. Unsaturated flow modeling of a cover profile is useful to develop a minimum cover thickness and evaluate the cover profile subjected to a variety of conditions as well as input and boundary sensitivities (Dwyer et al. 2007, USEPA 2012). These specific simulations were performed to evaluate the soil excavated from the Jetty area as the primary cover soil borrow source (Stantec 2018). The method used is consistent with those utilized in Dwyer (2018).

3.1 UNSAT-H INPUT PARAMETERS

The input parameters are consistent with those utilized in Dwyer (2018) other than soil properties. Soil samples taken from the Jetty area to be excavated served as input parameters for these computer simulations (Stantec 2018).

3.2 MODEL GEOMETRY

The model geometry was based on the expected depth of the cover system. The nodal spacing was set at a range narrow enough to accurately represent the modeled cover profile. The erosion analysis and subsequent surface erosion protection layer was performed and optimized whereby there are multiple cover profiles (varied admixture depth) changing from the top of the slope toward the base of the slopes (Tables 1 to 4). The surface admixture depth varies from 14 inches (36 cm) to 31.5 inches (80 cm) (Tables 1 to 4). Modeled profiles contain 33 percent rock to 67 percent soil by volume. Cover soil beneath this erosion protection surface layer is from the respective soil sample from the Jetty area summarized in Table 11. The rock is from engineer-approved on-site stockpiles or engineer-approved vendors meeting cover design durability requirements.

3.3 BOUNDARY CONDITIONS

The boundary conditions are consistent with those utilized in Dwyer (2018). That is ten consecutive typical years were modeled followed by the wettest year on record in two consecutive years. The tenth of ten typical years is that presented as output. The ten consecutive typical years allowed for unbiased antecedent conditions.

The wettest year was modeled two years in a row to add conservatism to the analyses. To add additional conservatism, the precipitation was applied at a rate slow enough to essentially force 100 percent infiltration (reduced runoff to zero or near zero), since much of the precipitation received at the NECR site runs off in high intensity storms.

The lower boundary condition was a unit gradient. With the unit gradient, the calculated drainage flux depends on the hydraulic conductivity of the lower boundary node. The unit gradient corresponded to gravity-induced drainage and was most appropriate because drainage was not impeded. The base of the modeled profile was placed well below transient activity and in relative steady state conditions to ensure that the unit gradient bottom boundary condition used did not affect the output for the cover system.

3.4 VEGETATION DATA

The vegetation input parameters are consistent with those utilized in Dwyer (2018). The input parameters representing vegetation include the leaf area index (LAI), rooting depth and density, root growth rate, suction head values corresponding to the wilting point, head corresponding to the water content below which plant transpiration starts to decrease, and a head value corresponding to the water content above which plants do not transpire because anaerobic conditions were defined.

The following rooting parameters (Table 5) were utilized when vegetation was included in the model (Cedar Creek 2014).

Table 5. Rooting Parameters (Cedar Creek 2014)

| Parameter | Reclaimed Analog | Grass Analog | Shrub Analog |
|-----------|------------------|--------------|--------------|
| a | 556.28 | 0.34 | 0.43 |
| b | 0.0000054 | 0.072 | 0.034 |
| c | -555.92 | 0.14 | 0.078 |

Other vegetation parameters including the LAI, percent bare area utilized, and maximum rooting depths for the respective vegetation used in a computer simulation are summarized in Table 6.

Table 6. Vegetation Parameters (Cedar Creek 2014)

| Parameter | Reclaim Analog | Grass Analog | Shrub Analog |
|--------------------|----------------|--------------|--------------|
| LAI | 0.91 | 0.64 | 0.52 |
| % Bare Area | 52.3% | 64.9% | 75.2% |
| Root Length | 147 cm | 142 cm | 155 cm |

In the modeling simulations that included vegetation, the onset and termination of the growing season for the site were Julian days 63 and 343, respectively. This is determined from the typical climate conditions for the NECR site and the respective growing degree days presented in Figure 17. The growing degree days were computed (Samani and Pessarakki 1986) for the typical year. The LAI was transitioned from 0 to the full LAI starting with Julian day 63 to 170. Day 171 through 266, the full LAI was utilized. The LAI was then transitioned down from the full LAI to 0 from Julian day 267 to 343. This was conservative since it is realistic that plants can transpire longer than indicated at this site.

The UNSAT-H model adjusts the full LAI based on the percent bare area of vegetation. For example, for a shrub vegetation with an LAI of 0.52 and a percent bare area of 75.2 percent, the LAI is reduced to $0.752 * 0.52 = 0.39$.

For computer simulations that do not include vegetation, the transpiration is set at zero. That is, all moisture removed from the profile upward is solely by evaporation.

3.5 SOIL PROPERTIES RELATED TO VEGETATION

The soil properties related to vegetation are consistent with those utilized in Dwyer (2018). The wilting point for these computer simulations was set at 40,000 cm for reclaimed vegetation and grassland and 70,000 for shrubland vegetation (Fayer and Walters 1995). This was conservatively used although some shrubs near the site could remove water from the soil to a suction of 100,000 cm (Hillel 1998). Evaporation from the soil surface can further reduce the soil moisture below the wilting point toward the residual saturation, which is the water content at an infinite matric potential. The suction head corresponding to the water content below which plant transpiration starts to decrease was defined as 32.2 feet (1000 cm) (Fayer and Walters 1995, Fayer 2000). The head value corresponding to the water content above which plants do not transpire because of anaerobic conditions was defined at 4 inches (30 cm) (Fayer and Walters 1995).

3.6 SOIL PROPERTIES

Soil mechanical and hydraulic properties for the Jetty soil to be excavated are summarized in Stantec (2018). The top layer or rock/soil admixture of the cover profile has multiple designs depending on where on the slope it is located (Tables 1 to 4). The thinnest and thickest were modeled to verify which produced the worst case scenario (scenario that requires the thickest cover profile to minimize flux). The upper admixture is composed of the mixture of rock and cover soil. The cover soil directly below the upper rock/soil admixture will be composed of soil from the same borrow source.

The top admixture layer has rock mixed into it at a volumetric ratio of 33 percent rock to 67 percent soil. The mixture of rock into the soil effectively alters its hydraulic properties. Consequently, the hydraulic properties were adjusted for the admixture layer (ASTM, 2007). Equation 1 was used to adjust the saturated hydraulic conductivity based on the addition of rock (Peck and Watson 1979).

$$K_b = [K_s * 2(1 - V_r)] / (2 + V_r) \quad \text{Equation 1}$$

where: K_b = saturated hydraulic conductivity, bulk

K_s = saturated hydraulic conductivity, soil

V_r = volume of rock

For the computer simulations, the calculated bulk saturated hydraulic conductivity was then increased an order of magnitude in the top foot of the modeled cover system (14 inches for the admixture that is 14 inches deep) to account for dynamic processes such as freeze/thaw cycles, wet/dry cycles, and biointrusion. This corresponds with findings from the soil analog studied at the site whereby soils in undisturbed settings have a saturated hydraulic conductivity of about one order of magnitude higher than the lower portions of the soil profile (Dwyer 2014).

The moisture retention data for the cover soil was also adjusted to reflect the addition of the rock in the surface admixture layer. The actual volumetric moisture contents versus soil suction measurements made in the laboratory were utilized as the basis. Each respective measured

volumetric moisture content was lowered per Equation 2 [ASTM 2007 and Bouwer & Rice 1984].

$$\theta_b = (1 - V_r)\theta_s \quad \text{Equation 2}$$

where: θ_b = bulk volumetric moisture content

θ_s = saturated volumetric moisture content

V_r = volume of rock

The Mualem conductivity function was used to describe the unsaturated hydraulic conductivity of the soils (van Genuchten et al. 1991). The van Genuchten 'm' parameter for this function is assumed to be '1-1/n'; 'n' being one of the established van Genuchten parameters. The initial soil conditions were expressed in terms of suction head values that correspond to the average moisture content between each soil layer's field capacity and permanent wilting point determined from each respective soil layer's moisture characteristic curve.

The soil input parameters for the UNSAT-H simulations are summarized in Table 7. The respective borrow soil properties adjusted for the additions of 33 percent by volume rock for the top admixture layer are summarized in Table 8. The van Genuchten parameters were developed from the laboratory soil measurements (soil suction versus moisture content) using the RETC software (van Genuchten et al. 1991).

Table 7. Jetty Soil Laboratory Measured Soil Properties

| Borrow | Sample Depth (feet) | Soil Type | K _s (cm/sec @ 90%) | Van Genuchten parameters | | | |
|-----------|---------------------|-----------|-------------------------------|--------------------------|-----------------------|--------|--------|
| | | | | Θ _s (vol.) | Θ _r (vol.) | alpha | n |
| B5A-9A | 10-10.5 | ML | 9.20E-05 | 0.3935 | 0.0266 | 0.0072 | 1.3012 |
| B6A-19A | 20-20.5 | CH | 7.00E-05 | 0.4553 | 0 | 0.0049 | 1.2175 |
| B7A-0-20 | 0-20 | CL | 8.90E-05 | 0.4029 | 0 | 0.0074 | 1.2353 |
| B7A-40-60 | 40-60 | CL | 5.90E-04 | 0.408 | 0 | 0.0153 | 1.1909 |
| B9-20-35 | 20-35 | CH | 4.10E-05 | 0.459 | 0 | 0.0023 | 1.2148 |
| B10-39A | 40-40.5 | CH | 5.50E-05 | 0.467 | 0 | 0.0046 | 1.204 |
| B10-10-25 | 10-25 | CL | 1.00E-04 | 0.4009 | 0 | 0.0104 | 1.2238 |
| Composite | NA | Clay | 4.80E-07 | 0.3778 | 0 | 0.0008 | 1.2352 |
| Composite | NA | Sand | 2.50E-05 | 0.3502 | 0 | 0.0285 | 1.2264 |

Table 8. Adjusted Borrow Soil Laboratory Measured Soil Properties for 33% Rock by Volume

| Borrow | K _s (cm/sec @ 90%) | Van Genuchten parameters | | | |
|-----------|-------------------------------|--------------------------|-----------------------|--------|--------|
| | | Θ _s (vol.) | Θ _r (vol.) | alpha | n |
| B5A-9A | 5.29E-05 | 0.2636 | 0.0178 | 0.0072 | 1.3012 |
| B6A-19A | 4.03E-05 | 0.3051 | 0 | 0.0049 | 1.2175 |
| B7A-0-20 | 5.12E-05 | 0.2699 | 0 | 0.0074 | 1.2353 |
| B7A-40-60 | 0.000339 | 0.2734 | 0 | 0.0153 | 1.1909 |
| B9-20-35 | 2.36E-05 | 0.3075 | 0 | 0.0023 | 1.2148 |
| B10-39A | 3.16E-05 | 0.3129 | 0 | 0.0046 | 1.204 |
| B10-10-25 | 5.75E-05 | 0.2686 | 0 | 0.0104 | 1.2238 |
| Composite | 2.76E-07 | 0.2531 | 0 | 0.0008 | 1.2352 |
| Composite | 1.44E-05 | 0.2346 | 0 | 0.0285 | 1.2264 |

The subgrade soil used in the profile beneath the ET cover is consistent with Dwyer (2018). Specifically, it was measured from the mine spoils using sample TT-205-GT1 (Table 9) remolded to 90 percent of the maximum dry density per ASTM (2012).

Table 9. Mine Spoils Measured Soil Hydraulic Properties

| Sample | Depth | Van Genuchten parameters | | | | K _s (cm/sec) |
|------------|-------|--------------------------|-----------------------|----------|--------|-------------------------|
| | | Θ _s (vol.) | Θ _r (vol.) | α (1/cm) | n | |
| TT-205-GT1 | all | 0.3774 | 0 | 0.0525 | 1.2338 | 2.2E-04 |

4.0 UNSAT-H SENSITIVITY ANALYSES

Modeling was performed to evaluate ET cover profiles utilizing native soil and vegetation parameters described in Dwyer (2018) as well as variability in climate data for the site also described in Dwyer (2018). The sensitivity analyses were performed to assess the range of soil input parameters and climatic scenarios expected over the long-term performance period of the final cover system and to demonstrate the cover system's ability to meet the performance objectives.

Soil samples extracted from the Jetty area were assessed (Tables 7 and 8). These hydraulic soil parameters were measured from remolded samples in the laboratory (Stantec 2018).

The shrubland vegetation was evaluated in these sensitivity analyses because the simulations described in Dwyer (2018) revealed it is the worst case vegetated scenario.

The cover profile differences (varied admixture depth) were evaluated. These profiles included an admixture top surface consisting of 33 percent rock to 67 percent soil by volume with rock 1.5 inches in diameter mixed to a depth of 14 inches and with rock 3.5 inches in diameter mixed to a depth of 31.5 inches. Directly beneath each admixture is the respective cover soil from the same borrow source without the mixture of rock.

Sensitivity to climate variation was evaluated whereby both typical and extreme conditions were modeled. The typical climate year used to evaluate the cover performance was weather from 1949 with an annual precipitation volume of 11.71 inches (29.74 cm). The Ft. Wingate weather data set also had the most extreme weather with the wettest year on record in 1906 with an annual precipitation volume of 23.8 inches (60.5 cm). Much of that moisture came as snow from January to April and October to December. In this period of the modeling the PET is low and transpiration of moisture through vegetation is minimized or completely ceased.

Table 10 summarizes the simulations performed in the cover profile sensitivity analyses. Refer to Appendix A for specifics of output for each simulation.

Table 10. Summary of Computer Simulations in the Cover Profile Sensitivity Analyses (Jetty Soil)

| Simulation Series | Cover Profile/Model Geometry | Input Parameters utilized in respective Sensitivity Analysis | | | | |
|---------------------------|---|--|--------------------|---|------------------|---|
| | | Rock Size in Surface Admixture (D50) | Soil Borrow Sample | Cover Soil Hydraulic Property Measurement | Vegetation Stage | Climate |
| 1.5-inch Admixture | 14-in surface admixture over cover soil | 1.5-inch | B5A-9A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | B6A-19A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |

| Simulation Series | Cover Profile/Model Geometry | Input Parameters utilized in respective Sensitivity Analysis | | | | |
|---------------------------|---|--|--------------------|---|------------------|---|
| | | Rock Size in Surface Admixture (D50) | Soil Borrow Sample | Cover Soil Hydraulic Property Measurement | Vegetation Stage | Climate |
| | 14-in surface admixture over cover soil | 1.5-inch | B7A-0-20 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | B7A-40-60 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | B9-20-35 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | B10-39A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | B10-10-25 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | Composite - Sand | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 14-in surface admixture over cover soil | 1.5-inch | Composite - Clay | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| 3.5-inch Admixture | 31.5-in surface admixture over cover soil | 3.5-inch | B5A-9A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B6A-19A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B7A-0-20 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |

| Simulation Series | Cover Profile/Model Geometry | Input Parameters utilized in respective Sensitivity Analysis | | | | |
|-------------------|---|--|--------------------|---|------------------|---|
| | | Rock Size in Surface Admixture (D50) | Soil Borrow Sample | Cover Soil Hydraulic Property Measurement | Vegetation Stage | Climate |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B7A-40-60 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B9-20-35 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B10-39A | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | B10-10-25 | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | Composite - Sand | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |
| | 31.5-in surface admixture over cover soil | 3.5-inch | Composite - Clay | Tables 11 and 12 | Shrub | Typical & two consecutive years of wettest year on record |

Although the varied input parameters such as soil and cover profile geometry showed some sensitivity, the most sensitive item was the climatic variation. The Point of Diminishing Returns (PODR) method (Dwyer et al. 2007, USEPA 2011) utilizes modeling to calculate the cover thickness required to effectively minimize flux through the cover. That is, at the PODR, an additional inch of soil will no longer enhance the cover's performance. Based on the PODR method (Dwyer et al. 2007, EPA 2011), the outcome predicted a cover thickness of less than 2.5 feet for typical weather conditions for all soil, vegetation input parameters and cover profiles. For the wettest year on record, the PODR was produced at a depth of about 3.7 feet for all soil, vegetation input parameters and cover profiles. The PODR was about 4.5 feet for a climate scenario that is beyond anything experienced in recorded history with the wettest year on record

(much of the moisture is received in the winter months where PET is at its lowest) occurs in consecutive years.

The input parameters were varied one at a time to evaluate their respective change on the calculated output. The recommended cover thickness is 4.5 feet based on analyses that demonstrated the cover has adequate storage capacity to withstand the worst-case scenarios expected over the 1,000-year performance period combined with some expected soil loss due to erosion (limited due to rock/soil admixture). This is an increase of 6 inches from the 4 feet suggested in Dwyer (2018).

The worst case graphic results from the sensitivity analyses are shown in Figure 1 (Soil Sample B9-20-35, shrubland vegetation). The PODR is achieved within the proposed cover profile. The output from all simulations, including output graphics, is included in Appendix A.

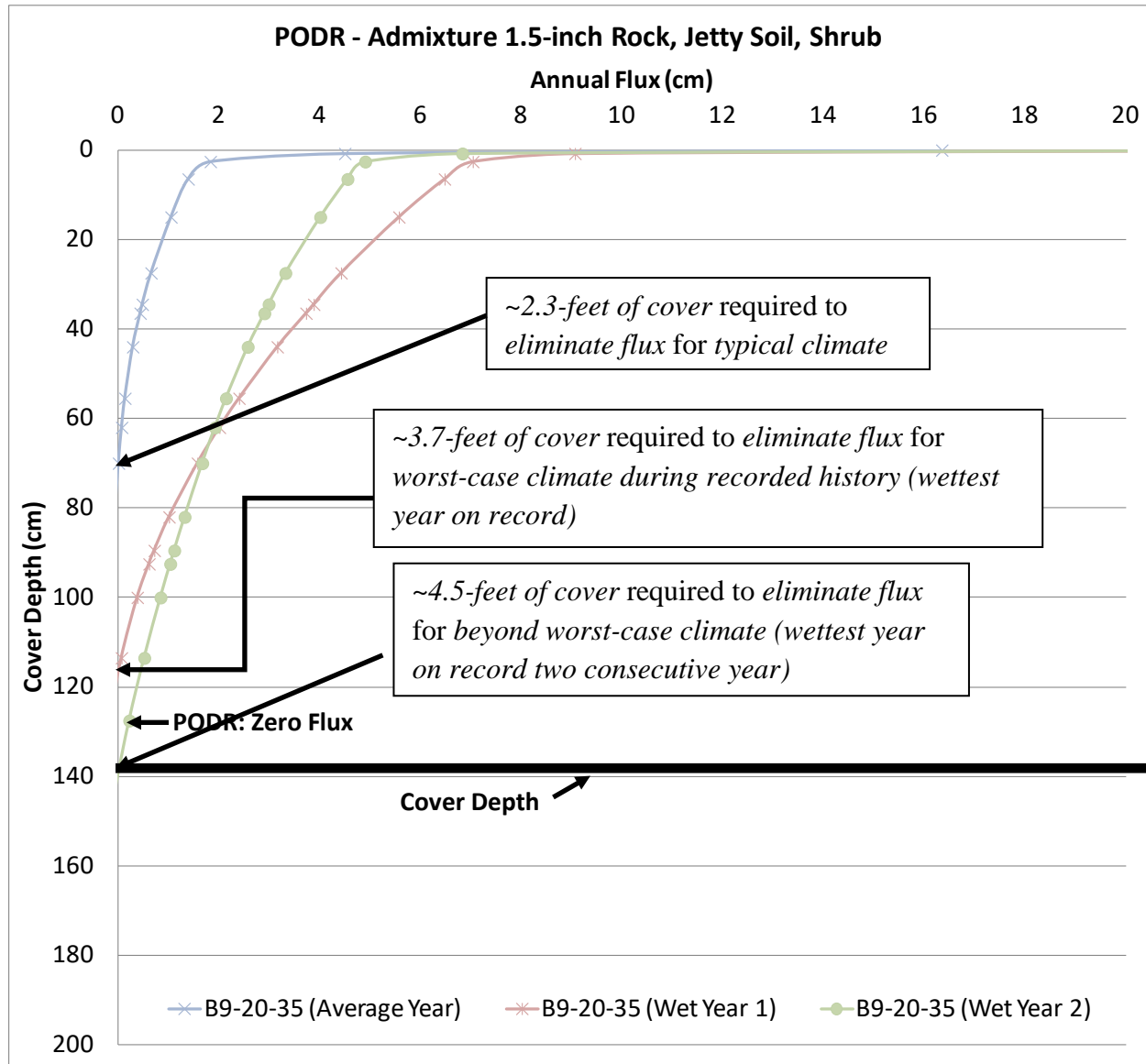


Figure 1. Worst Case Infiltration for Vegetated Cover System from Sensitivity Analyses (Soil Sample B9-20-35, Shrubland Vegetation)

5.0 LONG-TERM SIMULATIONS

After the sensitivity analyses were performed as described in Section 4, another set of computer simulations was performed (utilizing the worst case input parameters that produced output shown in Figure 1) to evaluate the long-term performance of the cover. The method and process used was consistent with that described in Dwyer (2018). The long-term evaluation of the cover profile overlying the mine spoils simulated the potential changes through the 1,000-year performance period varying the vegetation as it evolves. Typical and extreme climate conditions are included.

The UNSAT-H software cannot alter input parameters after initiation of a given simulation. Consequently, the long-term evaluations were calculated in multiple stages, where the output from earlier time steps were used as input for subsequent time steps. That is, for each long-term evaluation, the initial simulation with an initial set of input parameters was performed for a specified period of time and set of climatic conditions. The last day of the last year of that initial simulation output, specifically the matric potential values for each node from the previous simulation, was then used as the initial soil conditions for the subsequent simulation with the altered input parameters (Figure 2). For example, the *final* soil suction values for each node in the model geometry for the 'initial' stage were used as the starting conditions for the next 'short-term' stage.

There was no vegetation included in the 'initial' stage, but vegetation was included in subsequent stages (Cedar Creek 2014). The long-term simulations were performed in four stages: the 'initial' stage, followed by the 'short-term' stage, followed by the 'intermediate' stage, and finally the 'long-term' stage (Figure 2). The initial stage consisted of 3 years of typical or average climate with no vegetation. The short-term stage, intermediate stage, and long-term stage all consisted of 20 years of average to extreme climatic data. Thus the whole long-term simulation is 63 years. The extreme climate data was the wettest year on record run consecutively sandwiched by typical climatic data. There were no dry climatic years included in any of the simulations. All precipitation in the weather files was conservatively set to allow for 100 percent or close to 100 percent infiltration, thus minimizing runoff.

| | | | | |
|----------------------|-------------------------|-------------------------|-------------------------|--------------------------------|
| Time | Initial | Short-Term | Intermediate | Long-Term |
| Vegetation | None | Disturbed (Reclaimed) | Grassland | Shrubland |
| Soil | Remolded (Lab Measured) | Remolded (Lab Measured) | Remolded (Lab Measured) | Undisturbed (In Situ Measured) |
| Climate | Typical | Typical and Extreme | Typical and Extreme | Typical and Extreme |
| No. of Years Modeled | 3 | 20 | 20 | 20 |

Figure 2. Input based on Design Life for Computer Simulations

The first stage (initial time) of the long-term simulations, in the respective series for each admixture design assumed no vegetation from the time of construction completion out three years. The soil to be excavated from the Jetty area (Soil Sample B9-20-35) was used since it produced the worst case cover scenario (Figure 3). No vegetation is assumed for the 'initial' stage for a period of 3 years with average weather conditions. It is highly likely that vegetation will begin to emerge the first year and continue to expand into the second and third years, but to be conservative; no vegetation (and thus no transpiration) is included during the 'initial' model stage. Average weather conditions were assumed for these three years because dry conditions would obviously yield no flux and wet conditions would yield vegetation. The moisture condition (matric potential for each node in the model geometry) at the end of the third year, was used as the initial moisture conditions (matric potential for each node in the new model geometry) for the next 'short-term' stage.

The second stage (short-term time) of the long-term simulations in the respective series for each admixture included vegetation from the reclaimed vegetation analog (Cedar Creek 2014). The reclaimed community of vegetation represents vegetation in a disturbed area and generally considered from shortly after seeding upon construction completion up to about 50 years (Cedar Creek 2014). The soil input parameters and geometries from the first stage of simulations were consistent with this stage of simulations. Typical climate conditions were used for ten consecutive years followed by the wettest year on record two years in a row, followed by eight more years of typical climate conditions. This is conservative given the fact that the wettest year on record appears in two consecutive years every twenty years and that there are no dry years included in the analysis. The wettest years run consecutively is assumed to be 'beyond the worst case' infiltration event the site is likely to see. The moisture condition (matric potential for each node in the model geometry) at the end of the last year of the respective 'short-term' stage for each admixture design, was used as the initial moisture conditions (matric potential for each node in the new model geometry) for the next 'intermediate' stage.

The third stage of the long-term simulations (intermediate time) in the respective series for each admixture included vegetation from the grassland vegetation analog (Cedar Creek 2014). The grassland community represents undisturbed vegetation generally from about 25 to 100 years

after construction (Cedar Creek 2014). The soil input parameters and geometries from the 'short-term' stage were consistent with this 'intermediate' stage. Typical climate conditions were used for ten consecutive years followed by the wettest year on record two years in a row, followed by eight more years of typical climate conditions. The wettest years run consecutively is assumed to be the worst case infiltration events the site is likely to see. The moisture condition (matric potential for each node in the model geometry) at the end of the last year of each respective 'intermediate' stage for each admixture design, was used as the initial moisture conditions (matric potential for each node in the new model geometry) for the next 'long-term' stage.

The fourth stage of the long-term simulations (long-term time) in the respective series for each admixture included vegetation from the shrubland vegetation analog (Cedar Creek 2014). The shrubland community represents undisturbed generally from about 50 to 1,000 years (Cedar Creek 2014). The soil input parameters and geometries from the 'intermediate' stage were consistent with this 'long-term' stage. Typical climate conditions were used for ten consecutive years followed by the wettest year on record two years in a row, followed by eight more years of typical climate conditions. The wettest years run consecutively is assumed to be 'beyond the worst case' infiltration events the site is likely to see.

5.2 Long-Term Simulations Results

The long-term simulations performed were for the cover profile with the 14-inch-deep surface admixture layer. This profile was used because it was the worst case profile requiring the deepest PODR to minimize flux compared to the other admixture depths. The PODR or depth where flux is minimized at just over 2 feet is easily achieved within the recommended cover profile depth. This despite the climate utilized in the long-term simulation included the wettest year on record in consecutive years run every twenty years. That is, during this 63 year simulation, the wettest year on record occurred six times. Figure 4 shows that a drying trend is established and will continue until relative steady state is achieved (Dwyer 2017). Consequently, modeling the profile for any longer would not produce additional useful data.

For years 1, 2, and 3 (no vegetation), there was a de minimis amount of flux estimated but the PODR was reached at a shallow depth (Figure 3). All flux values through the vegetated cover for all subsequent years were zero.

The annual water balance output for each simulation year is contained in Appendix B.

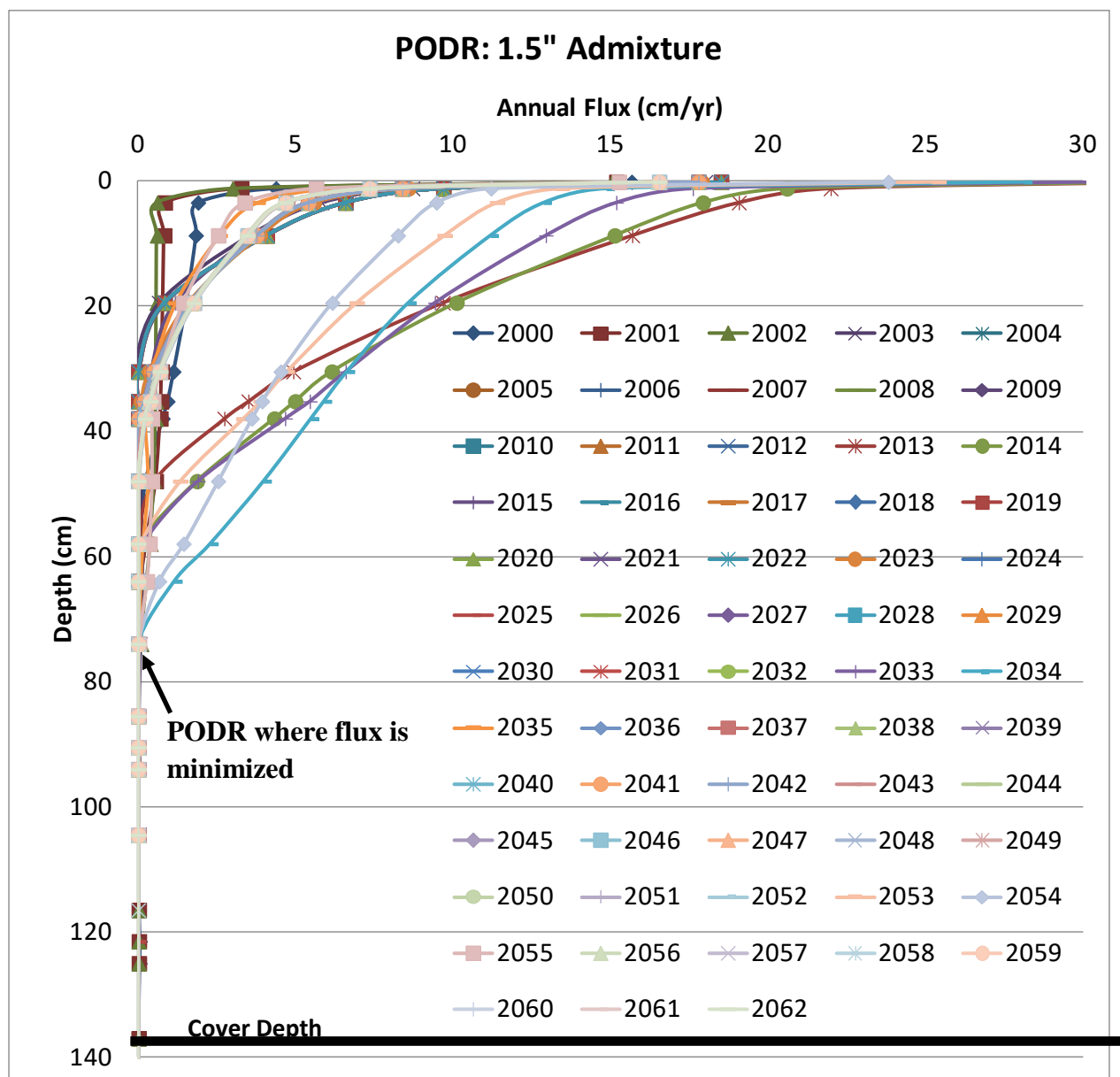


Figure 3. PODR for Long-Term Simulation, 14-inch deep Surface Layer

5.3 Long-Term Simulations Results for Profile B8

Another set of long-term simulations was performed utilizing Profile B8 corresponding to the consolidation and unsaturated flow analysis described in Dwyer (2017) to evaluate the effect of the cover system on the existing tailings and proposed mine spoils. The method and process used was consistent with that described in the Dwyer (2017 and 2018). Profile B8 was modeled because it is the only cross section that included fully saturated tailings due to consolidation resulting from placement of the mine spoils and ET cover on the impoundment (Dwyer 2017).

This set of simulations included placement of the ET cover over the placed mine spoils over the existing profile including the consolidated tailings. This analysis demonstrated that under the most conservative conditions, the cover and underlying mine spoils will not cause moisture build-up on the underlying radon barrier/liner. Details of the original analyses are included in *Tailings Consolidation and Groundwater Evaluation - 95% Design* (Dwyer 2017).

Suction values within the profile modeled that included the installed moisture condition of all of the mine spoils and ET cover at the suction value corresponding to the respective optimum moisture content (ASTM, 2012) are shown in Figure 4. The optimum moisture content is the wettest condition that the materials will be placed. Per the design specifications, any wetter condition will require removal of the material and drying it or reworking the soil to dry it in place. No material will be placed on top of a wet layer of soil until that underlying soil lift meets the specified conditions. Mine spoils are placed directly on the existing cover/radon barrier less the removal of the existing surface riprap that will be utilized elsewhere in the project.

Figure 4 shows that even though the mine spoils initial suction value is very wet, it quickly dries and continues to dry during the full simulation (middle of mine spoils). The base of the mine spoils and adjacent radon barrier suction values move toward a steady state condition (equal suction values) and then eventually all layers show a drying trend for the duration of the long-term simulation. This drying trend will continue until a steady state condition is reached at a greater suction value than the end of this simulation. This is because no net flux will pass through the vegetated cover system; the initial conditions are the wettest conditions and the profile will only dry as time passes.

Figure 4 illustrates that there is no moisture buildup on the existing radon/barrier and thus no potential for future seepage through the barrier.

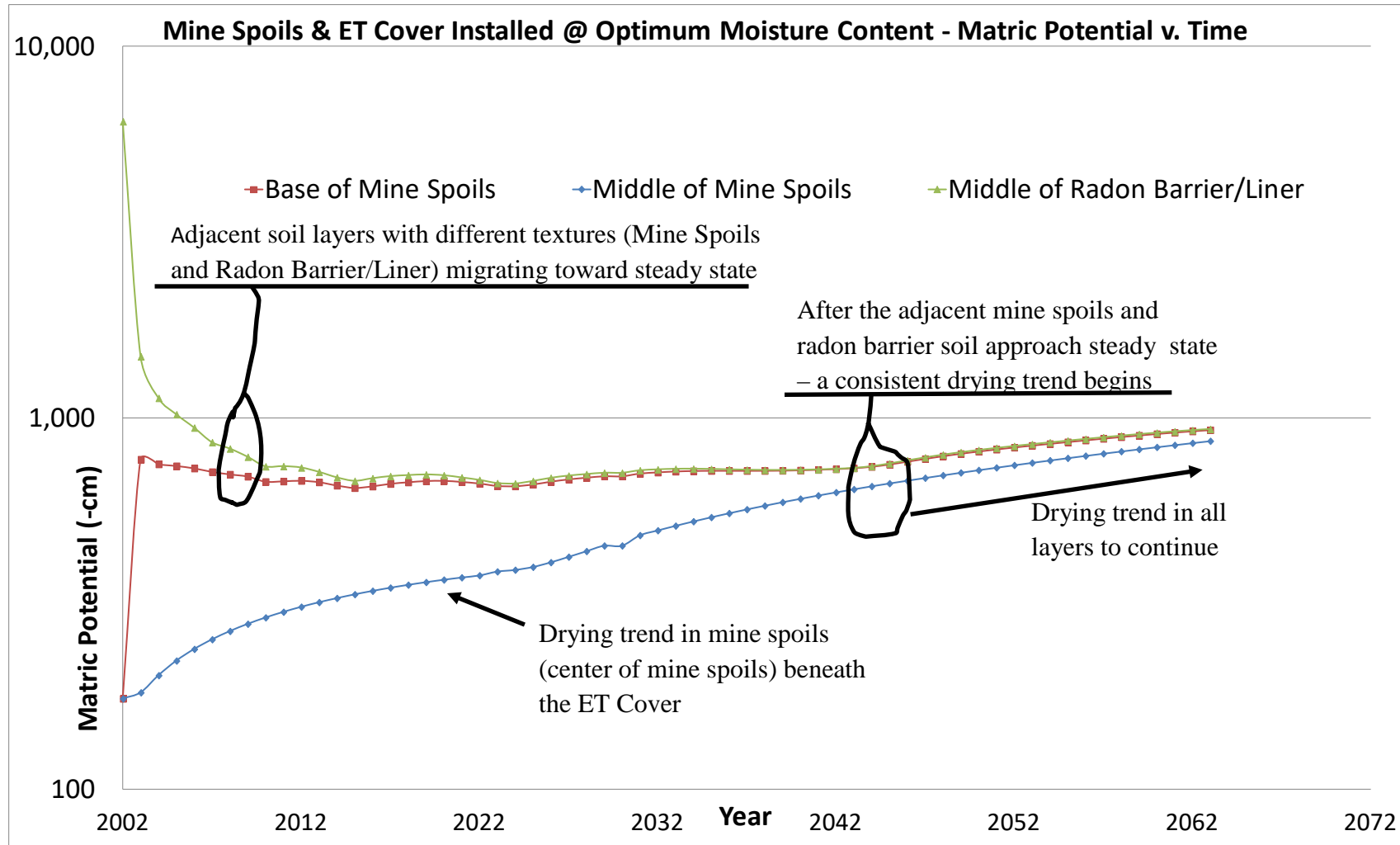


Figure 4. Suction Values for Specified Profile Depth vs. Time

6.0 RADON ATTENUATION

Federal regulations limiting radon releases to the atmosphere are contained in 40 CFR §192.02. The regulations are also typically applied as an ARAR to DOE sites undergoing remediation. These regulations require that release of ^{222}Rn to the atmosphere not exceed: (i) an average release rate of $20 \text{ pCi/m}^2\text{s}$; *or* (ii) increase the annual average concentration of ^{222}Rn in the air at or above any location outside of the disposal site by more than one-half picocurie per liter.

The ET cover soil functions as an effective barrier to gas diffusion; air-filled voids in the soil have to be discontinuous. Gas diffuses slowly through wet soils that contain only occasional, unconnected air bubbles. The ET cover system was evaluated for its ability to limit radon flux (NRC 1989, NRC 2003) consistent with that described in the Northeast Church Rock 95% Design Report, Appendix G, Attachment G.7, 95% Revised Draft Cover Design Report issued in July 2018. The radon flux through the ET cover soil was calculated using the Uranium Mill Tailings Cover Calculator (<http://www.wise-uranium.org/ctc.html>) that is a clone of the RAECOM code, as described in (Rogers 1984a, 1984b). It performs one-dimensional, steady-state radon diffusion calculations for a multi-layer system.

6.1 Input Data for Radon Flux Modeling

- **Layer Data:** The profile was modeled with a bottom layer of mine spoils capped with a two layered ET Cover system. The ET Cover profile is 4.5 feet (1.37 m) thick. The top layer is a 31.5-inch (80 cm) layer of rock and soil mixed at a ratio of 33 percent rock to 67 percent soil by volume. Of the four admixture designs, this is the most conservative given it has the thickest admixture region that has a reduced porosity and fines content. The bottom layer of the cover is all soil 22.5 inches thick (57 cm). The mine spoils were assumed to be 5 m thick (NRC, 1989, Section C 1.1.1).

NUREG Guide 3.64: 'Section C 1.1.1 Layer Thicknesses'

The thickness of the tailings source, x_t , will be determined from the applicant's estimates of total tailings production and areal extent of the pile. Because a tailings thickness greater than about 100-200 cm is effectively equivalent to an infinitely thick radon source, a value of $x_t = 500 \text{ cm}$ represents an equivalent infinitely thick tailings source of radon that may be used in the absence of more specific smaller values.

Thus, 500 cm is the maximum thickness to be used for tailings in the RADON model. Refer to Table 18 for a layer by layer description of parameters.

- **Ra-226 Activity Concentration [pCi/g]:** Activity concentration of Radium-226 in each respective layer. The Ra-226 activity concentration for the mine waste rock was a weighted average of that measured in the field. The Ra-226 concentration used for the entire 5 m depth of waste rock was 29.7 pCi/g (Table 11). A value of zero was assumed for the cover material since it is constructed of clean cover soil for an engineer approved borrow source (NRC, 1989, Section C 1.1.4).

Table 11. Radium-226 Concentrations in Mine Spoils (provided by Stantec)

| | Average Values (pCi/g) | Average of 75th Percentile Values (pCi/g) | Average of 90th Percentile Values (pCi/g) | Total Volumes (CY) |
|---|---------------------------|--|---|--------------------------|
| Area 1 - Vent Holes 3 and 8 | 7.6 | 2.4 | 3.5 | 14,764 |
| Area 2 - Boneyard and NEMSA | 21.0 | 26.7 | 28.5 | 50,535 |
| Area 3 - Road, Sandfill 2, Sandfill 3, NECR-2 | 10.1 | 11.4 | 12.2 | 223,080 |
| Area 4 - North of Pond 3 | 5.8 | 5.2 | 10.0 | 18,148 |
| Area 5 - TPH Stockpile | 9.5 | 9.6 | 23.4 | 30,000 |
| Area 7 - Sandfill 1 | 35.5 | 50.5 | 59.5 | 35,506 |
| Area 6 - Sediment Pad | 74.9 | 86.3 | 91.1 | 56,646 |
| Area 8 - NECR-1 | 21.8 | 24.2 | 28.7 | 361,382 |
| Area 8 - NECR-1 Step out Material | 9.5 | 9.6 | 23.4 | 130,000 |
| Area 11 - TPH Stockpile Area | 7.5 | 9.8 | 11.5 | 16,290 |
| Area 9 - Pond 1 | 60.8 | 85.2 | 94.9 | 44,634 |
| Area 10 - Pond 2 | 8.0 | 10.2 | 11.5 | 18,948 |
| Area 12 - Pond 3 | 6.7 | 7.1 | 7.3 | 22,375 |
| Area 13 - Drainage near Highway | 7.5 | 6.5 | 24.9 | 41,937 |
| Weighted Average by Volume | 20.5 | 24.0 | 29.7 | |

Table 12. Radon Flux Input Parameters

| Layer No. | Thickness [m] | Ra-226 Activity Conc. [pCi/g] | Ra-226 Emanat | Porosity | Moisture [dry wt_%] | Rn-222 Diff. Coeff - <i>calculated</i> [m ² /s] |
|--------------------------------|---------------|-------------------------------|---------------|----------|---------------------|--|
| Mine Spoils | 5 | 29.7 | 0.35 | 0.3774 | 6 | 2.253E-06 |
| Bottom Layer of Cover | 0.57 | 0 | 0.35 | 0.397 | 5.8 | 2.473E-06 |
| Top Layer (Admixture) of Cover | 0.80 | 0 | 0.35 | 0.266 | 4.7 | 1.483E-06 |

- Rn-222 Emanation Fraction** is a fraction of the total amount of radon-222 produced by radium decay that escapes from the soil particles and gets into the pores of the soil. It depends on the soil material and the moisture content. It varies over a range of 0.1 - 0.4 or more; typical values are in the range of 0.2 - 0.3. A value of 0.35 was used [NRC 1989, NRC, 2003]

- Porosity** is the ratio of the pore volume (air- and water-filled) to the total volume of the soil. Refer to Table 12 for the porosity values for each layer. The soil sample (Stantec 2018) with the lowest clay content was Soil Sample B11-39. This sample was taken below acceptable borrow soil and was the top of the sandstone. However, for conservatism this sample was used for the radon analysis. The saturated moisture content is 39.7% and is assumed to be equivalent to the porosity. The rock adjusted value of this moisture content is 26.6%. This reduced saturated moisture content was assumed to be the porosity of the admixture layer. The porosity for Sample TT-205-GT1 considered to be typical for the mine spoils was used (Dwyer 2017).
- Moisture Contents [dry wt_%]** is the percentage of water weight to dry soil weight. The average in situ moisture content for the mine spoils was 8 percent. The average optimum moisture content for the mine spoils is about 12 percent. An initial and long-term moisture content of 6 percent was used for the mine spoils (NRC 1989). NRC (1989) notes that 6 percent represents the lower bound for moisture in western soils and is typically used as a default value for the long term water content of tailings. Using the Rawls and Brakenseik equation as presented in the NRC Regulatory Guide 3.64 (NRC 1989), the long-term moisture content for the cover soil with 6.3% clay content is 5.8%. For the admixture layer, the clay content is reduced by 33%, thus using the Rawls and Brakenseik equation as presented in the NRC Regulatory Guide 3.64 (NRC 1989), the long-term moisture content for the cover admixture soil layer with 4.2% clay content $\{6.3\% \cdot [1 - 0.33]\}$ is 4.7%.
- Rn-222 Effective Diffusion Coefficient [m^2/s]** defined from Fick's equation as the ratio of the diffusive flux density of radon activity across the pore area to the gradient of the radon activity concentration in the pore or interstitial space. This value was calculated in the model based on the assigned input parameters identified above.

6.2 Output for Radon Flux Modeling

The computed radon flux was **6.23 pCi/m²s** (Table 13). This value is less than the maximum allowable of 20 pCi/m²s per 40 CFR 192.02.

Table 13. Radon Flux Calculation Output

| Layer No. | Thickness [m] | Exit Flux [pCi/m ² s] | Exit Conc. [pCi/L] | MIC |
|------------------------------|---------------|----------------------------------|--------------------|-------|
| Mine Spoils | 5.00 | 18.67 | 23.55E+03 | 0.802 |
| Bottom of Cover | 0.53 | 9.27 | 16.26E+03 | 0.824 |
| Top Admixture Layer of Cover | 0.69 | 6.23 | 0 | 0.741 |

7.0 SUMMARY OF RESULTS

The design methods and calculations demonstrate that the recommended cover designs will meet the objectives of performance for 1,000 years to include limiting meteoric flux into the underlying mine waste, minimize erosion, provide a rooting medium for native vegetation, and attenuate emanation of radon-222 from the mine waste. This conclusion is based on erosion computations, moisture flux modeling, and radon emanation calculations.

7.1 Erosion Protection

The cover is composed of two layers. The top layer is a rock/soil admixture referred to as a 'desert pavement'. This layer is designed to mitigate erosion by adding rock to the engineer-approved cover soil. This surface layer satisfies NUREG-1623 (NRC, 2002) for the long-term stability of a rocky soil cover. The overall cover thickness including both layers will be a consistent 4.5 feet (137 cm) while the thickness surface admixture depends on the location and respective slope length (Tables 1 through 4). The bottom layer is composed of cover soil only.

The surface desert pavement is a mixture of 33 percent rock to 67 percent soil by volume. For slope lengths 900 feet and longer, a 3.5-inch rock is required with a corresponding admixture depth of 31.5 inches. For a slope length of 525 feet to 900 feet, a 3-inch rock is required with a corresponding admixture depth of 27 inches. For a slope length of 350 feet to 525 feet, a 2-inch rock is required with a corresponding admixture depth of 18 inches. From the top of the slope to a slope length of 350 feet, a 1.5-inch rock is required with a corresponding admixture depth of 14 inches.

For slopes of 2 percent, the 1.5-inch rock admixture at a depth of 14 inches was adequate for the full 1,000 feet slope length. The resulting surface slopes meet requirements set forth in Dwyer et al. (2007) and NRC (2002).

7.2 Modeling of Cover System

Section 3 summarized the sensitivity analyses evaluating myriad input parameters demonstrating the cover system's effectiveness for the 1,000-year performance period. The modeling output revealed that for typical climatic conditions, a 2.5-feet (76 cm) cover thickness minimized flux due to precipitation. A cover thickness less than 4.5 feet (137 cm) effectively minimized flux even while applying the wettest year on record in two consecutive years. This scenario includes the wettest year where much of this precipitation occurred in the winter and early spring and late fall where PET is low and then doubled it by running the year back-to-back to provide conservatism in the design and analysis. The analyses revealed that the 4.5-feet (137 cm) vegetated ET cover will produce no flux no matter the combination of input parameters possible for the 1,000-year performance period. There was minimal difference in prediction of the PODR for the cover profile from the myriad input parameters modeled including cover soil and vegetation. The most sensitive item was the climate comparing typical to the extreme wet conditions (two consecutive wettest years on record). The 4.5-feet (137 cm) cover depth is the thickness of soil with the required storage capacity needed to minimize flux based on the application of the wettest year on record with close to 100 percent infiltration two years in a row. This climatic scenario is beyond anything seen at the site in recorded history and beyond anything likely to occur at the site. The combination of this climatic data and slow application

rate to force nearly 100 percent infiltration while minimizing runoff created the worst case infiltration design scenario.

7.3 Radon Flux

Section 6 provided an overview of the estimated radon release rate through the cover profile. The radon flux through the cover soil was calculated using the RAECOM code, as described in (Rogers 1984a, 1984b). It performs one-dimensional, steady-state radon diffusion calculations for a multi-layer system. The top layer is a 31.5-inch-thick (80 cm) layer of rock and soil mixed at a ratio of 1 rock to 2 soil by volume. Of the four admixture designs, this is the most conservative given it has the thickest admixture region that has a reduced porosity and fines content. The bottom layer of the cover is a 22.5-inch-thick (57 cm) soil layer. The mine spoils were assumed to be 5 meters thick (NRC, 1989, Section 1.1.1). The computed radon flux was 6.23 pCi/m²s (Table 13). This value is less than the maximum allowable of 20 pCi/m²s per 40 CFR 192.02.

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APPENDIX A

MODELING SENSITIVITY ANALYSES OF COVER SYSTEM

FIGURES

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|---|-----|

The initial modeling performed and summarized in this addendum was intended to evaluate the ET Cover profile utilizing soil to be excavated from the Jetty area as the primary cover soil borrow source. Shrubland vegetation parameters consistent with Dwyer (2018) were utilized because they were shown to produce the worst case vegetated scenario for a cover profile. Two profiles were evaluated: one with a surface admixture consisting of 1.5-inch rock mixed with soil 14 inches deep; the second profile had a surface admixture layer consisting of 3.5-inch rock mixed with soil 31.5 inches deep. The sensitivity analyses also assessed climatic scenarios expected over the full long-term performance period of the final cover system; and demonstrated the cover system's ability to meet the applicable performance objectives (Dwyer 2018).

The first set of computer simulations involved evaluating the borrow area with the remolded samples / laboratory measured soil values without any vegetation and incorporating the 1.5-inch diameter (D50) rock admixture that is 14 inches deep (Table 1 and Figures 1 to 4). Beneath the admixture is cover soil from the same borrow source. The top 14 inches had the saturated hydraulic conductivity increased by an order of magnitude to account for effects such as roots and freeze/thaw action on the soil. This order of magnitude increase to about a foot deep is consistent with soil measurements made in situ (Dwyer 2014).

The second set of computer simulations involved evaluating the borrow area with the remolded samples / laboratory measured soil values without any vegetation and incorporating the 3.5-inch diameter (D50) rock admixture that is 31.5 inches deep. Beneath the admixture is cover soil from the same borrow source (Table 2 and Figures 5 to 8). The top 14 inches had the saturated hydraulic conductivity increased by an order of magnitude to account for effects such as roots and freeze/thaw action on the soil. This order of magnitude increase to about a foot deep is consistent with soil measurements made in situ (Dwyer 2014).

These computer simulations involved running 10 average years in a row to establish appropriate antecedent conditions and mitigate any biases from assumed initial soil conditions. The ten consecutive typical years of climate was then followed by the wettest year on record modeled back to back. Table 1 contains the annual water balance variables for each soil sample modeled for the tenth year of the average consecutive modeled years and the two wettest years on record modeled consecutively. The following water balance variables are summarized for each year: (1) applied precipitation; (2) applied potential evapotranspiration (PET); (3) calculated transpiration; (4) calculated evaporation; (5) calculated runoff; and (6) calculated percolation for a 4-ft-thick cover and 4.5-ft-thick cover. It can be seen in Table 1 that not all percolation was zero for all years for a 4-ft-thick cover but all percolation was zero for a 4.5-ft-thick cover profile. This is the reason for the increase in cover thickness from 4 ft to 4.5 ft recommended in this addendum.

Table 1. Annual Water Balances Output

| Soil Sample from Jetty Area | Climate Year in Simulation Series | Annual Water Balance Output (cm) | | | | | | | | | | | | | |
|-----------------------------|-----------------------------------|---|-------|--------|------|--------|-------------|---------------|---|-------|--------|------|--------|-------------|---------------|
| | | Cover Profile with 14-inch Deep Admixture, Rock size 1.5-in | | | | | | | Cover Profile with 31.5-inch Deep Admixture, Rock size 3.5-in | | | | | | |
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4ft | Drain @ 4.5ft | Precip | PET | Transp | Evap | Runoff | Drain @ 4ft | Drain @ 4.5ft |
| B5A-9A | Average | 29.7 | 211.7 | 4.1 | 26.1 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 3.8 | 26.4 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 6.6 | 49.9 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 6.4 | 50.9 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 7.3 | 52.1 | 0.0 | 0.1 | 0.0 | 60.4 | 215.5 | 7.0 | 52.7 | 0.0 | 0.0 | 0.0 |
| B6A-19A | Average | 29.7 | 211.7 | 3.3 | 27.2 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 3.1 | 27.4 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 5.4 | 51.5 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 5.3 | 52.4 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 6.0 | 53.7 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 5.7 | 54.3 | 0.0 | 0.0 | 0.0 |
| B7A-0-20 | Average | 29.7 | 211.7 | 3.9 | 26.4 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 3.7 | 26.6 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 6.3 | 50.3 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 6.3 | 51.1 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 7.0 | 52.5 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 6.8 | 52.9 | 0.0 | 0.0 | 0.0 |
| B7A-40-60 | Average | 29.7 | 211.7 | 4.0 | 26.3 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 3.7 | 26.6 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 6.6 | 49.9 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 6.4 | 50.8 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 7.4 | 52.1 | 0.0 | 0.3 | 0.0 | 60.4 | 215.5 | 7.1 | 52.6 | 0.0 | 0.2 | 0.0 |
| B9-20-35 | Average | 29.7 | 211.7 | 2.3 | 28.3 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 2.2 | 28.4 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 4.0 | 53.4 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 3.9 | 54.0 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 4.4 | 55.5 | 0.0 | 0.3 | 0.0 | 60.4 | 215.5 | 4.2 | 55.8 | 0.1 | 0.2 | 0.0 |
| B10-39A | Average | 29.7 | 211.7 | 3.4 | 27.2 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 3.2 | 27.4 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 5.5 | 51.3 | 0.1 | 0.0 | 0.0 | 60.4 | 215.5 | 5.3 | 52.3 | 0.1 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 6.0 | 53.6 | 0.1 | 0.0 | 0.0 | 60.4 | 215.5 | 5.8 | 54.2 | 0.1 | 0.0 | 0.0 |
| B10-10-25 | Average | 29.7 | 211.7 | 4.4 | 25.7 | 0.0 | 0.0 | 0.0 | 29.7 | 211.7 | 4.1 | 26.0 | 0.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 7.2 | 49.0 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 7.0 | 50.0 | 0.2 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 8.0 | 51.3 | 0.0 | 0.0 | 0.0 | 60.4 | 215.5 | 7.7 | 52.0 | 0.1 | 0.0 | 0.0 |
| Clay Composite | Average | 29.7 | 211.7 | 2.6 | 21.0 | 7.0 | 0.0 | 0.0 | 29.7 | 211.7 | 2.5 | 21.0 | 7.0 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 3.4 | 31.3 | 24.7 | 0.0 | 0.0 | 60.4 | 215.5 | 3.3 | 31.5 | 24.8 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 3.6 | 32.3 | 24.9 | 0.0 | 0.0 | 60.4 | 215.5 | 3.5 | 32.5 | 24.9 | 0.0 | 0.0 |

| Soil Sample from Jetty Area | Climate Year in Simulation Series | Annual Water Balance Output (cm) | | | | | | | | | | | | | |
|-----------------------------|-----------------------------------|---|-------|--------|------|--------|-------------|---------------|---|-------|--------|------|--------|-------------|---------------|
| | | Cover Profile with 14-inch Deep Admixture, Rock size 1.5-in | | | | | | | Cover Profile with 31.5-inch Deep Admixture, Rock size 3.5-in | | | | | | |
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4ft | Drain @ 4.5ft | Precip | PET | Transp | Evap | Runoff | Drain @ 4ft | Drain @ 4.5ft |
| <u>Sand Composite</u> | Average | 29.7 | 211.7 | 5.2 | 20.5 | 4.1 | 0.0 | 0.0 | 29.7 | 211.7 | 5.2 | 20.5 | 4.1 | 0.0 | 0.0 |
| | Wet Yr 1 | 60.4 | 215.5 | 7.6 | 33.8 | 15.6 | 0.0 | 0.0 | 60.4 | 215.5 | 7.3 | 34.5 | 15.7 | 0.0 | 0.0 |
| | Wet Yr 2 | 60.4 | 215.5 | 8.4 | 35.5 | 15.7 | 0.0 | 0.0 | 60.4 | 215.5 | 8.0 | 36.2 | 15.8 | 0.0 | 0.0 |

Figure 1 presents a graphical summary of computer simulations flux (cm/yr) versus depth (cm) for each year of each simulation for a cover profile with a 14-inch-thick admixture layer (1.5-inch diameter rock). This depth of cover soil where flux is minimized is referred to as the Dwyer Point of Diminishing Returns Method (Dwyer et al 2007). The ‘point of diminishing returns’ (PODR) is defined as the depth at which flux is effectively minimized; that is, the depth at which an additional increment of soil will no longer significantly reduce the flux. For all of these simulations the PODR is where the net annual flux is reduced to zero at a depth less than the 4.5-ft cover thickness. Thus 40 CFR 264.310, 20.3.13.1313 NMAC, and 10 CFR 61.51 are satisfied in that flux through the cover is minimized.

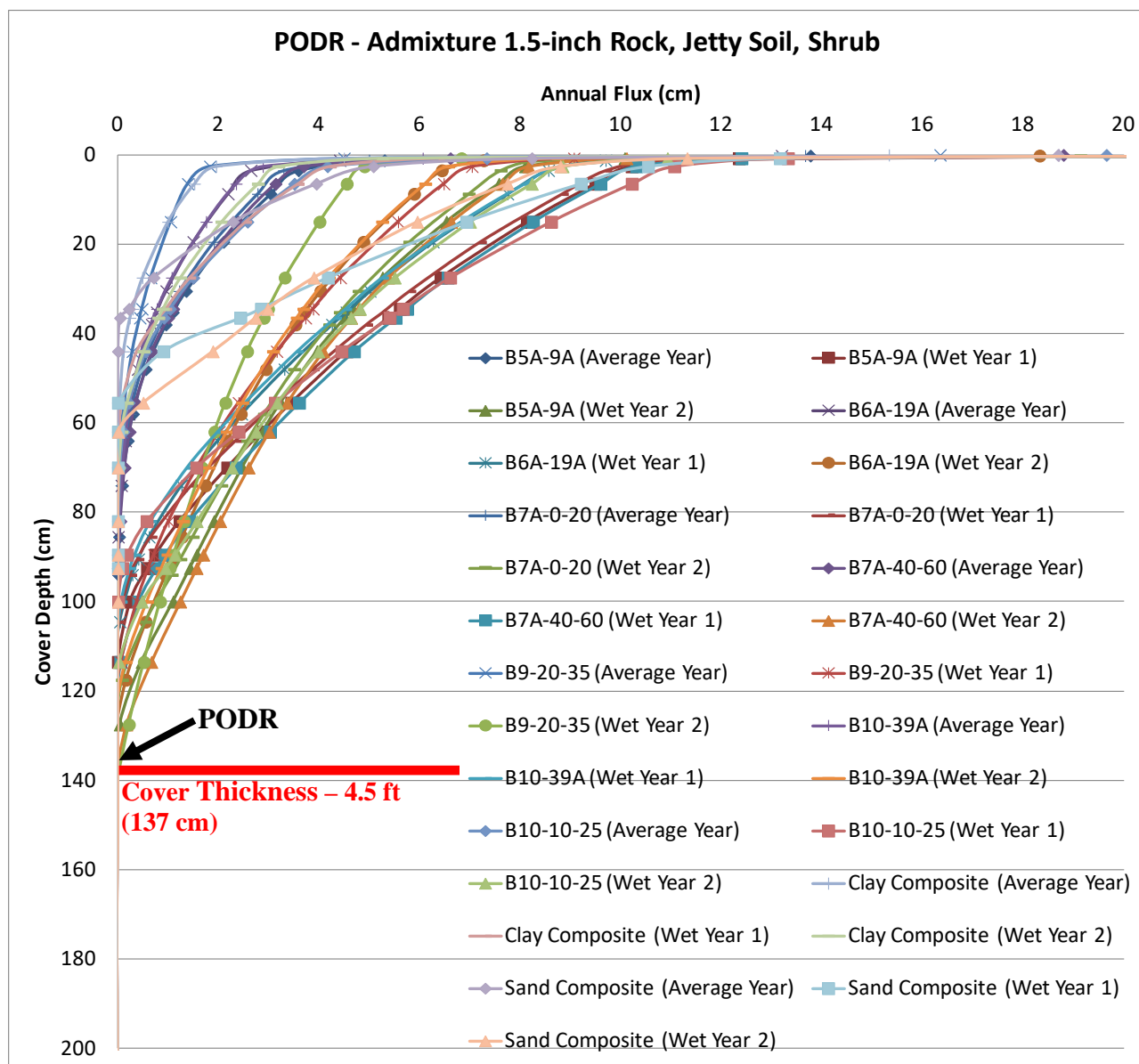


Figure 1. Annual Flux versus Cover Depth – Cover Profile with 14-inch Thick Surface Admixture Layer

Figures 2 to 4 present the data in Figure 1 broken out by respective climate year for clarity. Figure 2 presents a graphical summary of the output from each computer simulations evaluating the available Jetty area soil samples (Stantec 2018) for the typical climate year for a cover profile with a 14-inch-thick surface admixture layer (1.5-inch diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth of about 3 ft (91 cm).

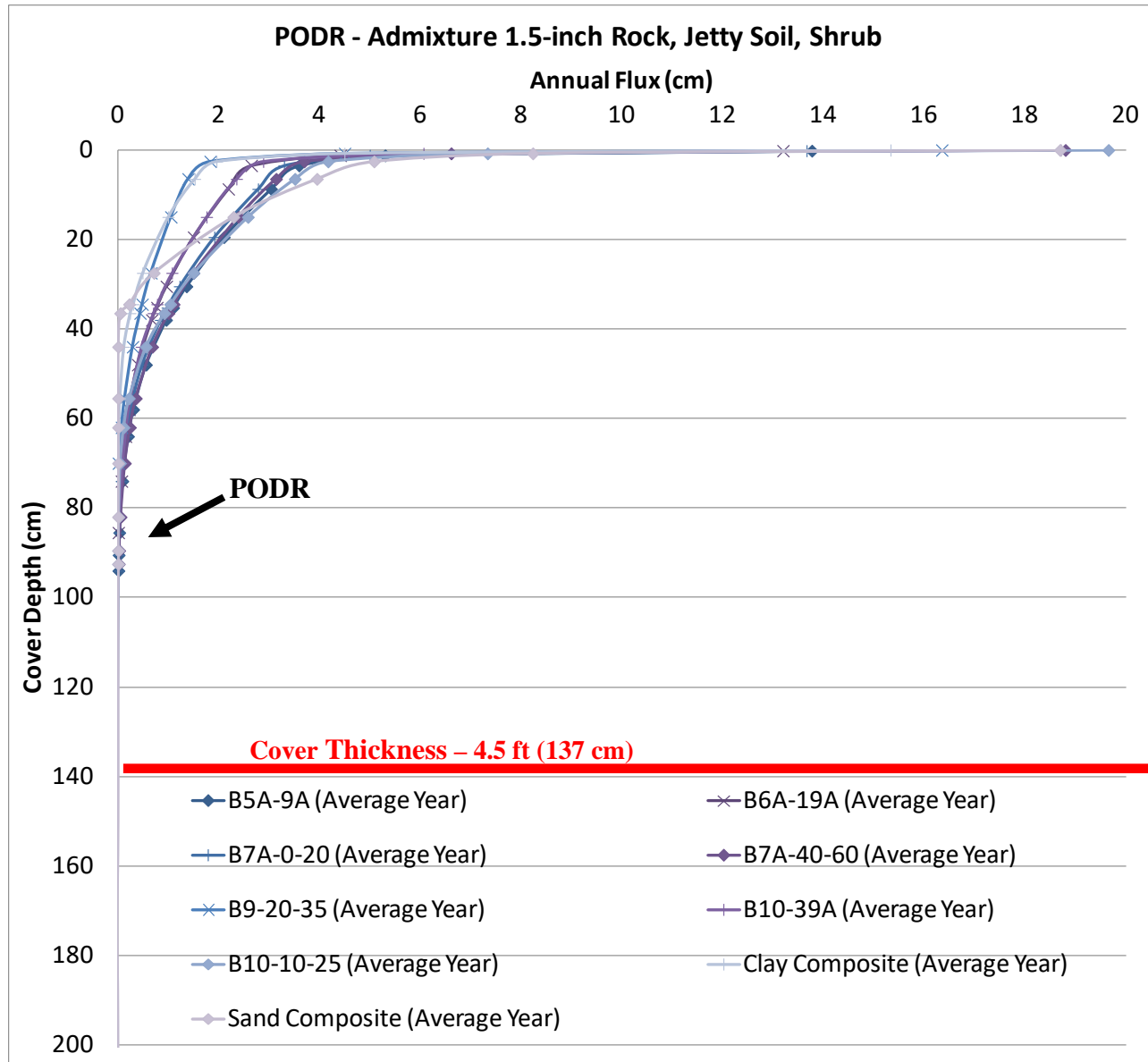


Figure 2. Annual Flux versus Cover Depth for Typical Climate Year for 14-inch (1.5-in dia. Rock) Surface Admixture Layer

Figure 3 presents a graphical summary of the output from each computer simulation representative of the available Jetty area soil samples (Stantec 2018) for the wettest year on record for a cover profile with a 14-inch-thick surface admixture layer (1.5-in diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth less than 4 ft (122 cm).

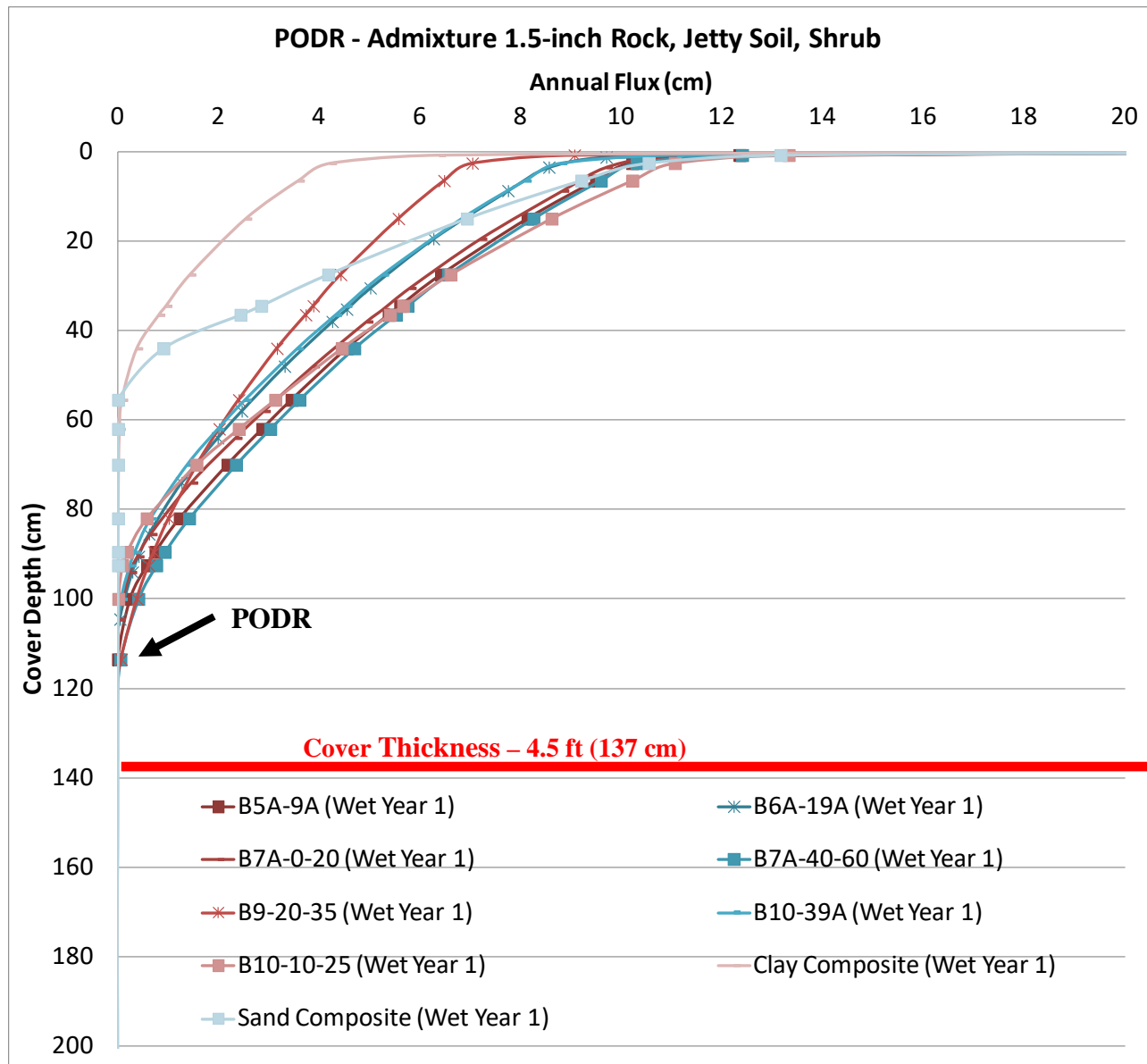


Figure 3. Annual Flux versus Cover Depth for Wettest Year on Record for 14-inch (1.5-in dia. Rock) Surface Admixture Layer

Figure 4 presents a graphical summary of the output from each computer simulation representative of the available Jetty area soil samples (Stantec 2018) for second consecutive wettest year on record for a cover profile with a 14-inch-thick surface admixture layer (1.5-in diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth less than 4.5 ft (137 cm).

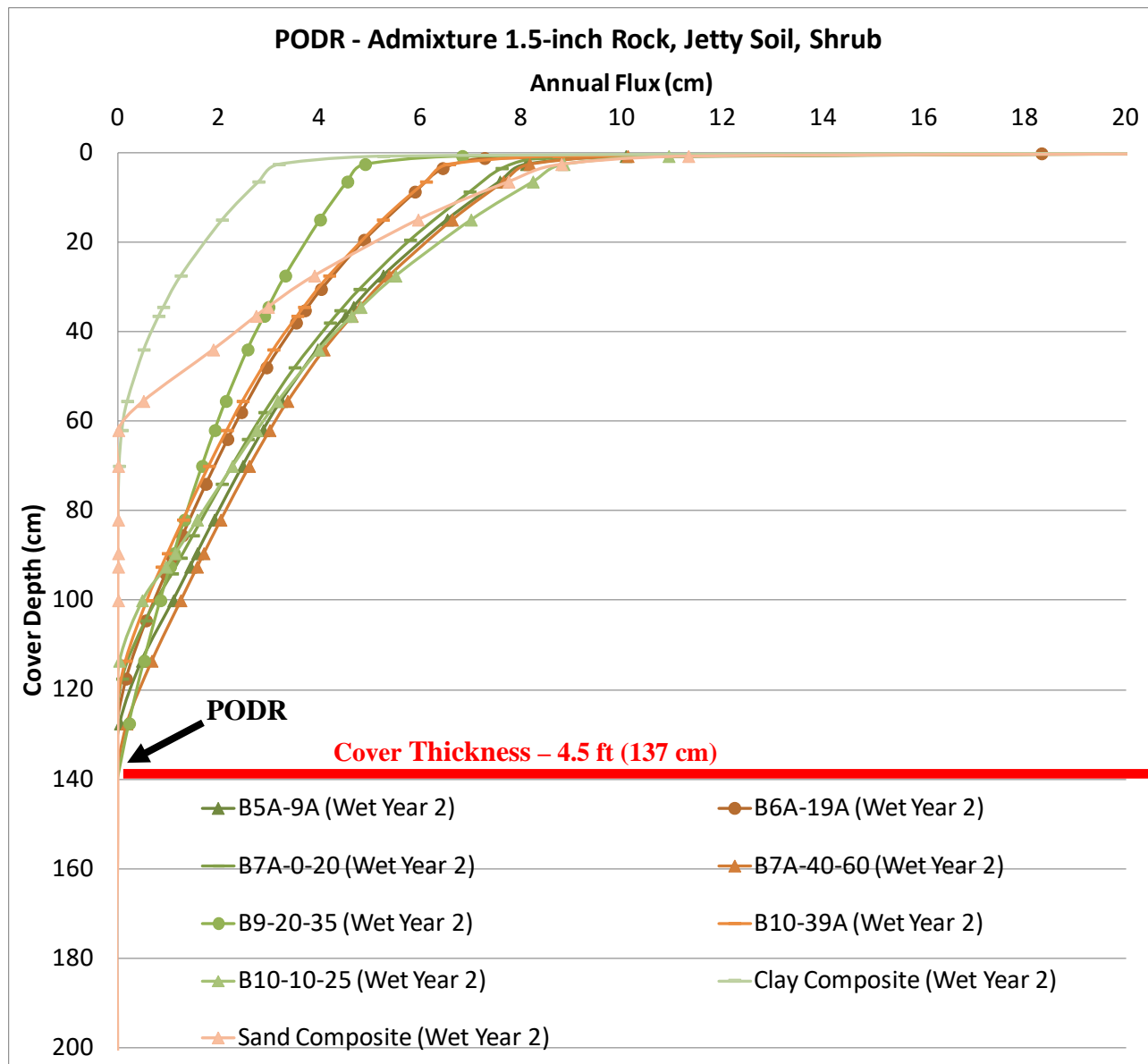


Figure 4. Annual Flux versus Cover Depth for the Second of Consecutive Wettest Years on Record for 14-inch (1.5-in dia. Rock) Surface Admixture Layer

Figure 5 presents a graphical summary of computer simulations flux (cm/yr) versus depth (cm) for each year of each simulation for a cover profile with a 31.5-inch thick admixture layer (3.5-inch diameter rock). For all of these simulations the PODR is where the net annual flux is reduced to zero at a depth less than the 4.5-ft cover thickness. Thus 40 CFR 264.310, 20.3.13.1313 NMAC, and 10 CFR 61.51 are satisfied in that flux through the cover is minimized.

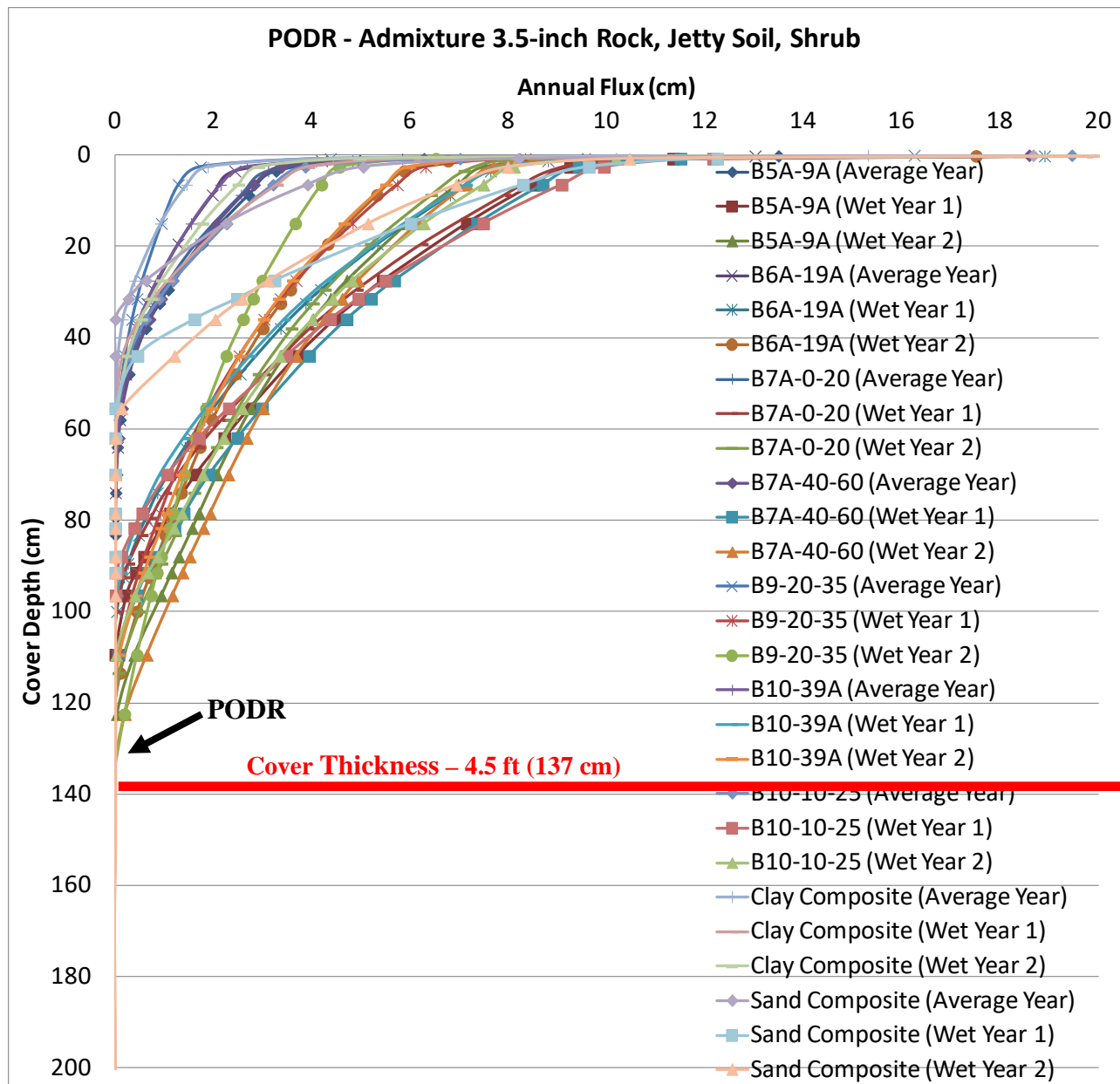


Figure 5. Annual Flux versus Cover Depth – Cover Profile with 31.5-inch Thick Surface Admixture Layer

Figures 6 to 8 present the data in Figure 5 broken out by respective climate year for clarity. Figure 6 presents a graphical summary of the output from each computer simulation representative of the available Jetty area soil samples (Stantec 2018) for the typical climate year for a cover profile with a 31.5-inch thick surface admixture layer (3.5-inch diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth less than 3 ft (91 cm).

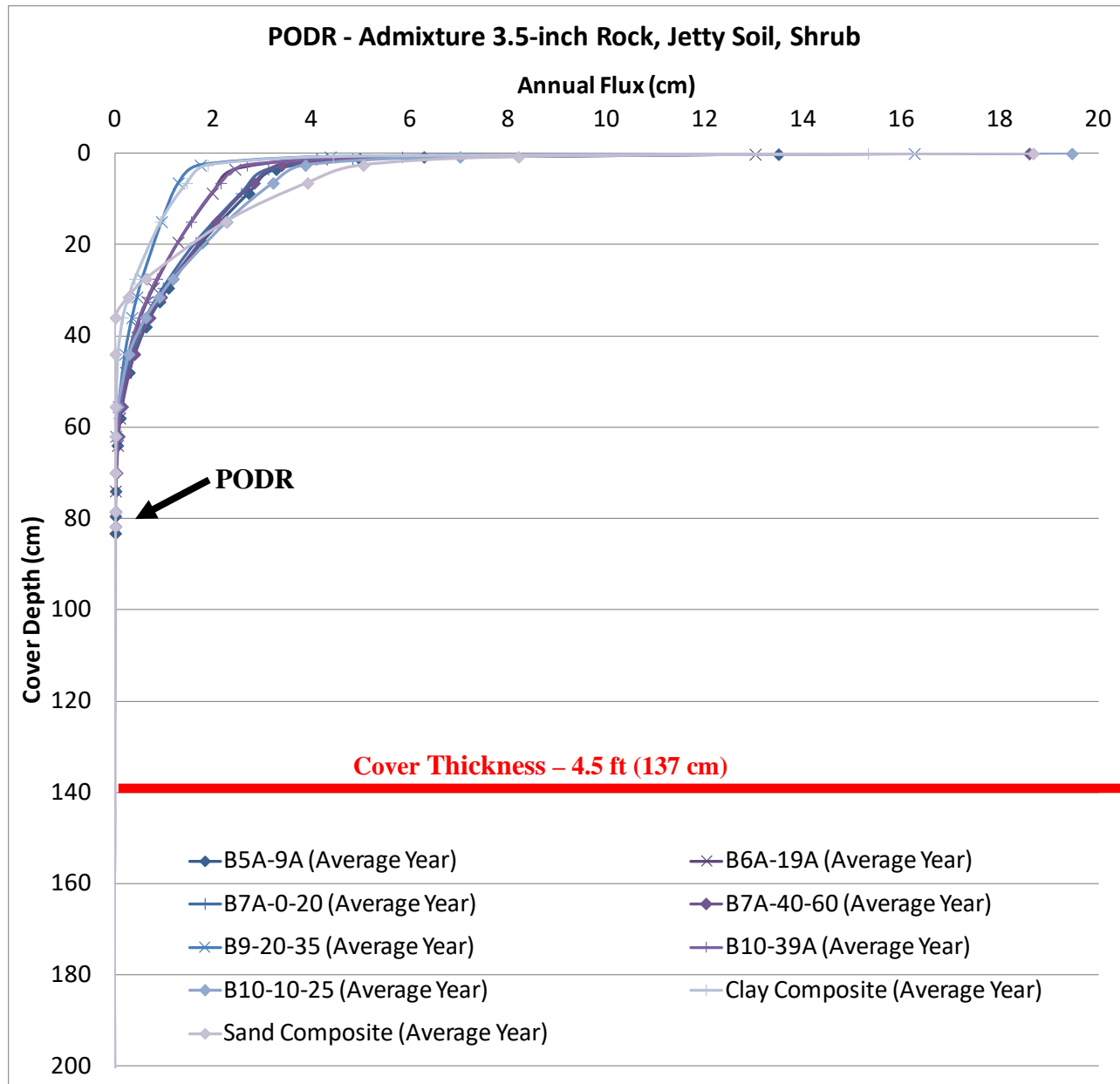


Figure 6. Annual Flux versus Cover Depth for Typical Climate Year for 14-inch (1.5-in dia. Rock) Surface Admixture Layer

Figure 7 presents a graphical summary of the output from each computer simulation representative of the available Jetty area soil samples (Stantec 2018) for the wettest year on record for a cover profile with a 31.5-inch thick surface admixture layer (3.5-inch diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth less than 4 ft (122 cm).

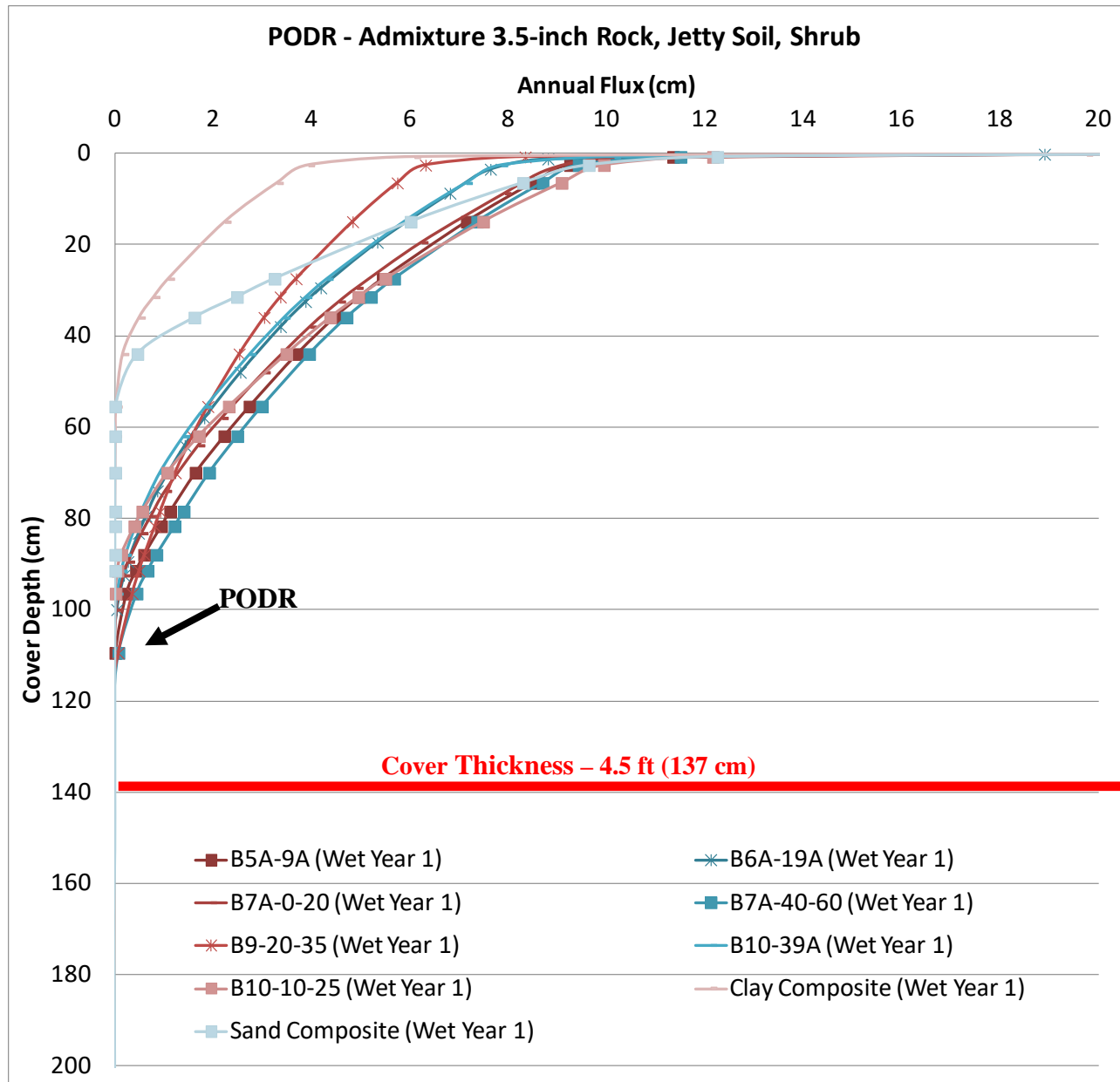


Figure 7. Annual Flux versus Cover Depth for Wettest Year on Record for 31.5-inch (3.5-in dia. Rock) Surface Admixture Layer

Figure 8 presents a graphical summary of the output from each computer simulation representative of the available Jetty area soil samples (Stantec 2018) for second consecutive wettest year on record for a cover profile with a 31.5-inch thick surface admixture layer (3.5-inch diameter rock). It can be seen that the PODR and in these cases net annual zero flux is achieved at a cover depth less than 4.5 ft (137 cm).

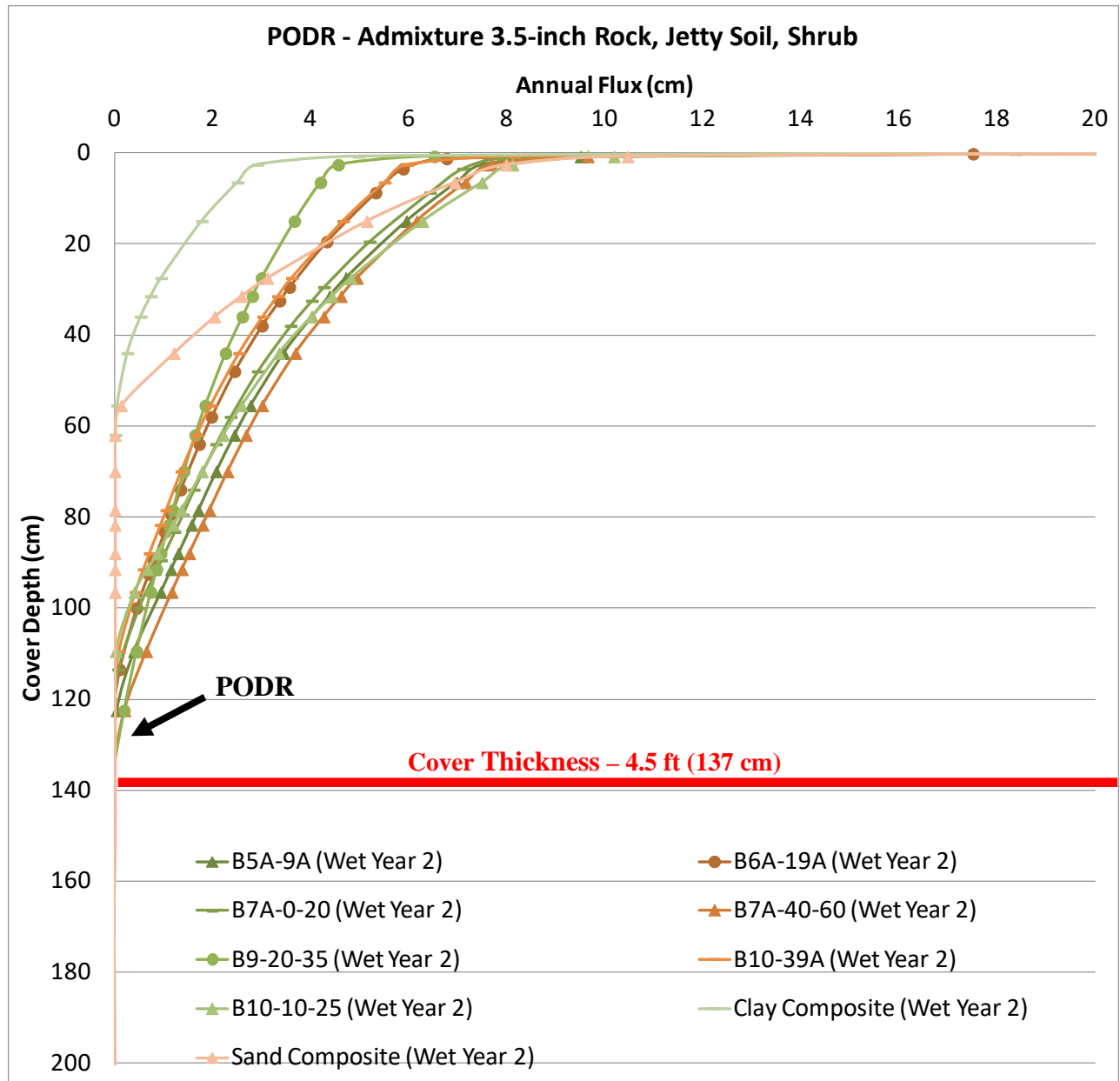


Figure 8. Annual Flux versus Cover Depth for the Second of Consecutive Wettest Years on Record for 31.5-inch (3.5-in dia. Rock) Surface Admixture Layer

APPENDIX B

LONG-TERM MODELING SIMULATIONS OF COVER PROFILES

TABLES

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| Table 1. Annual Water Balance Results for Long-Term Cover Profile Simulation..... | B-3 |
| Table 2. Annual Water Balance Results for Long-Term Simulation over Profile B8..... | B-5 |

The long-term evaluation of the 4.5 ft cover profiles overlying the mine spoils simulated the potential changes through time, varying the vegetation properties as they evolve. Typical and extreme climate conditions are included. The cover profile with a 14-inch-thick surface admixture layer was utilized as it was the profile with Soil Sample B9-20-35 that produced the worst case scenario. That is, the profile that utilized soil hydraulic properties from Jetty area soil sample B9-20-35 with the 14-inch-thick admixture surface layer produced the deepest PODR.

The first set of long-term simulations was performed with the worst case input parameters for the cover profile. The annual water balance output for each year simulated is summarized in Table 1.

Table 1. Annual Water Balance Results for Long-Term Cover Profile Simulation

| Simulation Year | Vegetation | Annual Water Balance Results (cm/yr) | | | | | |
|-----------------|---------------|--------------------------------------|--------|--------|-------|--------|-----------------------|
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4.5ft (137cm) |
| 1 | No Vegetation | 29.74 | 211.74 | 0.00 | 39.75 | 0 | 0 |
| 2 | No Vegetation | 29.74 | 211.74 | 0.00 | 31.48 | 0 | 0 |
| 3 | No Vegetation | 29.74 | 211.74 | 0.00 | 30.60 | 0 | 0 |
| 4 | Reclaimed | 29.74 | 211.74 | 2.55 | 28.11 | 0 | 0 |
| 5 | Reclaimed | 29.74 | 211.74 | 1.18 | 27.60 | 0 | 0 |
| 6 | Reclaimed | 29.74 | 211.74 | 1.21 | 27.57 | 0 | 0 |
| 7 | Reclaimed | 29.74 | 211.74 | 1.39 | 27.58 | 0 | 0 |
| 8 | Reclaimed | 29.74 | 211.74 | 1.53 | 27.59 | 0 | 0 |
| 9 | Reclaimed | 29.74 | 211.74 | 1.61 | 27.60 | 0 | 0 |
| 10 | Reclaimed | 29.74 | 211.74 | 1.66 | 27.61 | 0 | 0 |
| 11 | Reclaimed | 29.74 | 211.74 | 1.70 | 27.61 | 0 | 0 |
| 12 | Reclaimed | 29.74 | 211.74 | 1.72 | 27.62 | 0 | 0 |
| 13 | Reclaimed | 29.74 | 211.74 | 1.75 | 27.62 | 0 | 0 |
| 14 | Reclaimed | 60.35 | 215.46 | 6.77 | 50.43 | 0 | 0 |
| 15 | Reclaimed | 60.35 | 215.46 | 7.44 | 52.25 | 0.004 | 0 |
| 16 | Reclaimed | 29.74 | 211.74 | 2.85 | 29.30 | 0 | 0 |
| 17 | Reclaimed | 29.74 | 211.74 | 2.03 | 27.69 | 0 | 0 |
| 18 | Reclaimed | 29.74 | 211.74 | 1.87 | 27.65 | 0 | 0 |
| 19 | Reclaimed | 29.74 | 211.74 | 1.83 | 27.63 | 0 | 0 |
| 20 | Reclaimed | 29.74 | 211.74 | 1.83 | 27.63 | 0 | 0 |
| 21 | Reclaimed | 29.74 | 211.74 | 1.83 | 27.63 | 0 | 0 |
| 22 | Reclaimed | 29.74 | 211.74 | 1.84 | 27.63 | 0 | 0 |
| 23 | Reclaimed | 29.74 | 211.74 | 1.84 | 27.63 | 0 | 0 |
| 24 | Grassland | 29.74 | 211.74 | 3.92 | 28.57 | 0 | 0 |
| 25 | Grassland | 29.74 | 211.74 | 2.52 | 28.64 | 0 | 0 |
| 26 | Grassland | 29.74 | 211.74 | 1.92 | 28.56 | 0 | 0 |
| 27 | Grassland | 29.74 | 211.74 | 1.66 | 28.52 | 0 | 0 |
| 28 | Grassland | 29.74 | 211.74 | 1.54 | 28.51 | 0 | 0 |

| Simulation Year | Vegetation | Annual Water Balance Results (cm/yr) | | | | | |
|-----------------|------------|--------------------------------------|--------|--------|-------|--------|-----------------------|
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4.5ft (137cm) |
| 29 | Grassland | 29.74 | 211.74 | 1.48 | 28.51 | 0 | 0 |
| 30 | Grassland | 29.74 | 211.74 | 1.44 | 28.50 | 0 | 0 |
| 31 | Grassland | 29.74 | 211.74 | 1.41 | 28.50 | 0 | 0 |
| 32 | Grassland | 29.74 | 211.74 | 1.40 | 28.50 | 0 | 0 |
| 33 | Grassland | 29.74 | 211.74 | 1.39 | 28.50 | 0 | 0 |
| 34 | Grassland | 60.35 | 215.46 | 4.02 | 53.12 | 0.001 | 0 |
| 35 | Grassland | 60.35 | 215.46 | 4.76 | 55.10 | 0.004 | 0 |
| 36 | Grassland | 29.74 | 211.74 | 2.51 | 30.37 | 0 | 0 |
| 37 | Grassland | 29.74 | 211.74 | 1.78 | 28.63 | 0 | 0 |
| 38 | Grassland | 29.74 | 211.74 | 1.52 | 28.55 | 0 | 0 |
| 39 | Grassland | 29.74 | 211.74 | 1.42 | 28.52 | 0 | 0 |
| 40 | Grassland | 29.74 | 211.74 | 1.37 | 28.51 | 0 | 0 |
| 41 | Grassland | 29.74 | 211.74 | 1.35 | 28.50 | 0 | 0 |
| 42 | Grassland | 29.74 | 211.74 | 1.34 | 28.50 | 0 | 0 |
| 43 | Grassland | 29.74 | 211.74 | 1.33 | 28.50 | 0 | 0 |
| 44 | Shrubland | 29.74 | 211.74 | 2.15 | 28.28 | 0 | 0 |
| 45 | Shrubland | 29.74 | 211.74 | 1.98 | 28.21 | 0 | 0 |
| 46 | Shrubland | 29.74 | 211.74 | 1.87 | 28.19 | 0 | 0 |
| 47 | Shrubland | 29.74 | 211.74 | 1.81 | 28.18 | 0 | 0 |
| 48 | Shrubland | 29.74 | 211.74 | 1.76 | 28.17 | 0 | 0 |
| 49 | Shrubland | 29.74 | 211.74 | 1.74 | 28.17 | 0 | 0 |
| 50 | Shrubland | 29.74 | 211.74 | 1.72 | 28.17 | 0 | 0 |
| 51 | Shrubland | 29.74 | 211.74 | 1.70 | 28.17 | 0 | 0 |
| 52 | Shrubland | 29.74 | 211.74 | 1.69 | 28.16 | 0 | 0 |
| 53 | Shrubland | 29.74 | 211.74 | 1.68 | 28.16 | 0 | 0 |
| 54 | Shrubland | 60.35 | 215.46 | 3.47 | 53.11 | 0 | 0 |
| 55 | Shrubland | 60.35 | 215.46 | 3.96 | 55.29 | 0.004 | 0 |
| 56 | Shrubland | 29.74 | 211.74 | 2.59 | 30.22 | 0 | 0 |
| 57 | Shrubland | 29.74 | 211.74 | 2.18 | 28.42 | 0 | 0 |
| 58 | Shrubland | 29.74 | 211.74 | 1.99 | 28.29 | 0 | 0 |
| 59 | Shrubland | 29.74 | 211.74 | 1.88 | 28.23 | 0 | 0 |
| 60 | Shrubland | 29.74 | 211.74 | 1.80 | 28.20 | 0 | 0 |
| 61 | Shrubland | 29.74 | 211.74 | 1.75 | 28.19 | 0 | 0 |
| 62 | Shrubland | 29.74 | 211.74 | 1.72 | 28.18 | 0 | 0 |
| 63 | Shrubland | 29.74 | 211.74 | 1.70 | 28.17 | 0 | 0 |

The second set of long-term simulations was the same cover profile input properties over Profile B8 that included the mine spoils and existing tailings. However, the cover profile was conservatively set at 4 ft deep. This annual water balance output is summarized in Table 2.

Table 2. Annual Water Balance Results for Long-Term Simulation over Profile B8

| Simulation Year | Vegetation | Annual Water Balance Results (cm/yr) | | | | | |
|-----------------|---------------|--------------------------------------|--------|--------|-------|--------|-----------------------|
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4.5ft (137cm) |
| 1 | No Vegetation | 29.74 | 211.74 | 0.00 | 42.61 | 0 | 0 |
| 2 | Reclaimed | 29.74 | 211.74 | 4.67 | 28.89 | 0 | 0 |
| 3 | Reclaimed | 29.74 | 211.74 | 3.09 | 27.47 | 0 | 0 |
| 4 | Reclaimed | 29.74 | 211.74 | 2.74 | 27.36 | 0 | 0 |
| 5 | Reclaimed | 29.74 | 211.74 | 2.62 | 27.33 | 0 | 0 |
| 6 | Reclaimed | 29.74 | 211.74 | 2.57 | 27.32 | 0 | 0 |
| 7 | Reclaimed | 29.74 | 211.74 | 2.54 | 27.31 | 0 | 0 |
| 8 | Reclaimed | 29.74 | 211.74 | 2.52 | 27.31 | 0 | 0 |
| 9 | Reclaimed | 29.74 | 211.74 | 2.51 | 27.31 | 0 | 0 |
| 10 | Reclaimed | 29.74 | 211.74 | 2.50 | 27.31 | 0 | 0 |
| 11 | Reclaimed | 29.74 | 211.74 | 2.49 | 27.30 | 0 | 0 |
| 12 | Reclaimed | 60.35 | 215.46 | 7.48 | 50.04 | 0.003 | 0 |
| 13 | Reclaimed | 60.35 | 215.46 | 8.14 | 51.92 | 0.009 | 0 |
| 14 | Reclaimed | 29.74 | 211.74 | 3.53 | 29.03 | 0 | 0 |
| 15 | Reclaimed | 29.74 | 211.74 | 2.69 | 27.36 | 0 | 0 |
| 16 | Reclaimed | 29.74 | 211.74 | 2.52 | 27.32 | 0 | 0 |
| 17 | Reclaimed | 29.74 | 211.74 | 2.46 | 27.30 | 0 | 0 |
| 18 | Reclaimed | 29.74 | 211.74 | 2.45 | 27.30 | 0 | 0 |
| 19 | Reclaimed | 29.74 | 211.74 | 2.44 | 27.30 | 0 | 0 |
| 20 | Reclaimed | 29.74 | 211.74 | 2.44 | 27.30 | 0 | 0 |
| 21 | Reclaimed | 29.74 | 211.74 | 2.43 | 27.30 | 0 | 0 |
| 22 | Grassland | 29.74 | 211.74 | 4.83 | 28.19 | 0 | 0 |
| 23 | Grassland | 29.74 | 211.74 | 2.96 | 28.25 | 0 | 0 |
| 24 | Grassland | 29.74 | 211.74 | 2.31 | 28.18 | 0 | 0 |
| 25 | Grassland | 29.74 | 211.74 | 2.07 | 28.15 | 0 | 0 |
| 26 | Grassland | 29.74 | 211.74 | 1.97 | 28.14 | 0 | 0 |
| 27 | Grassland | 29.74 | 211.74 | 1.91 | 28.13 | 0 | 0 |
| 28 | Grassland | 29.74 | 211.74 | 1.88 | 28.13 | 0 | 0 |
| 29 | Grassland | 29.74 | 211.74 | 1.87 | 28.13 | 0 | 0 |
| 30 | Grassland | 29.74 | 211.74 | 1.85 | 28.13 | 0 | 0 |
| 31 | Grassland | 29.74 | 211.74 | 1.84 | 28.13 | 0 | 0 |
| 32 | Grassland | 60.35 | 215.46 | 4.89 | 52.39 | 0.004 | 0 |
| 33 | Grassland | 60.35 | 215.46 | 5.70 | 54.35 | 0.009 | 0 |
| 34 | Grassland | 29.74 | 211.74 | 3.02 | 29.97 | 0 | 0 |
| 35 | Grassland | 29.74 | 211.74 | 2.20 | 28.24 | 0 | 0 |
| 36 | Grassland | 29.74 | 211.74 | 1.94 | 28.16 | 0 | 0 |

| Simulation Year | Vegetation | Annual Water Balance Results (cm/yr) | | | | | |
|-----------------|------------|--------------------------------------|--------|--------|-------|--------|-----------------------|
| | | Precip | PET | Transp | Evap | Runoff | Drain @ 4.5ft (137cm) |
| 37 | Grassland | 29.74 | 211.74 | 1.85 | 28.14 | 0 | 0 |
| 38 | Grassland | 29.74 | 211.74 | 1.81 | 28.14 | 0 | 0 |
| 39 | Grassland | 29.74 | 211.74 | 1.79 | 28.13 | 0 | 0 |
| 40 | Grassland | 29.74 | 211.74 | 1.78 | 28.13 | 0 | 0 |
| 41 | Grassland | 29.74 | 211.74 | 1.78 | 28.13 | 0 | 0 |
| 42 | Shrubland | 29.74 | 211.74 | 2.37 | 28.07 | 0 | 0 |
| 43 | Shrubland | 29.74 | 211.74 | 2.20 | 28.03 | 0 | 0 |
| 44 | Shrubland | 29.74 | 211.74 | 2.10 | 28.01 | 0 | 0 |
| 45 | Shrubland | 29.74 | 211.74 | 2.03 | 28.00 | 0 | 0 |
| 46 | Shrubland | 29.74 | 211.74 | 1.99 | 28.00 | 0 | 0 |
| 47 | Shrubland | 29.74 | 211.74 | 1.96 | 27.99 | 0 | 0 |
| 48 | Shrubland | 29.74 | 211.74 | 1.94 | 27.99 | 0 | 0 |
| 49 | Shrubland | 29.74 | 211.74 | 1.93 | 27.99 | 0 | 0 |
| 50 | Shrubland | 29.74 | 211.74 | 1.92 | 27.99 | 0 | 0 |
| 51 | Shrubland | 29.74 | 211.74 | 1.91 | 27.99 | 0 | 0 |
| 52 | Shrubland | 60.35 | 215.46 | 3.68 | 52.93 | 0.003 | 0 |
| 53 | Shrubland | 60.35 | 215.46 | 4.17 | 55.14 | 0.009 | 0 |
| 54 | Shrubland | 29.74 | 211.74 | 2.81 | 30.11 | 0 | 0 |
| 55 | Shrubland | 29.74 | 211.74 | 2.41 | 28.27 | 0 | 0 |
| 56 | Shrubland | 29.74 | 211.74 | 2.21 | 28.13 | 0 | 0 |
| 57 | Shrubland | 29.74 | 211.74 | 2.10 | 28.06 | 0 | 0 |
| 58 | Shrubland | 29.74 | 211.74 | 2.03 | 28.03 | 0 | 0 |
| 59 | Shrubland | 29.74 | 211.74 | 1.98 | 28.01 | 0 | 0 |
| 60 | Shrubland | 29.74 | 211.74 | 1.95 | 28.01 | 0 | 0 |
| 61 | Shrubland | 29.74 | 211.74 | 1.93 | 28.00 | 0 | 0 |

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Appendix H: Borrow Areas

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LIST OF ACRONYMS / ABBREVIATIONS

| | |
|-----------|---|
| AOC | Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery |
| ARAR | Applicable or Relevant and Appropriate Requirement |
| ASTM | American Society for Testing and Materials |
| BMP | best management practice |
| CC | Construction Contractor |
| CY | cubic yard(s) |
| GE/UNC | General Electric/United Nuclear Corporation |
| GSR | Green and Sustainable Remediation |
| Mill Site | Church Rock Mill Site |
| Mine Site | Northeast Church Rock Mine Site |
| NECR | Northeast Church Rock |
| NRC | US Nuclear Regulatory Commission |
| PDS | Pre-Design studies |
| PTW | principal threat waste |
| RA | Removal Action |
| RAL | removal action limit |
| RAO | Remedial Action Objective or Removal Action Objective |
| ROD | Record of Decision |
| SOW | Statement of Work |
| TDA | Tailings Disposal Area |
| USEPA | US Environmental Protection Agency |

H.1 INTRODUCTION

H.1.1 Project Background

The Northeast Church Rock (NECR) Mine Site (Mine Site) Removal Action (RA) consists of excavation and removal of mine waste consisting of soil, rock and debris. This material will be placed within a repository near the former Church Rock Mill Site (Mill Site). Dwyer Engineering, LLC is designing an evapotranspirative soil and rock cover for the repository (see Appendix G). Clean soil and rock fill will be required for final grading and contouring as well as for erosion protection for the soil cover system and associated site reclamation. This appendix discusses sources for clean soil and rock fill materials required for the project. The Northeast Church Rock Mine Site Removal Action Pre-Design Studies (PDS) Report for the Church Rock Mill Site (MWH, 2014) provide geotechnical characterization of the proposed borrow areas. Specifically, this appendix provides the following information:

- Demonstration that the borrow areas providing cover material for the NECR repository meet requirements of the Performance Standards identified in the 2011 Action Memo (USEPA, 2011), Record of Decision (ROD; USEPA, 2013), and the Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery Statement of Work (AOC SOW; USEPA, 2015). US Nuclear Regulatory Commission (NRC) requirements for soil and rock are included in documents listed above.
- Locations and assumptions for estimates of available borrow material at each proposed on-site soil borrow area, including material management strategies, estimated limits of excavations, and identification of temporary stormwater and erosion controls to be used during borrow activities.
- Excavation and final grading plans for each borrow area.
- Proposed reclamation activities at each borrow area.
- Identification of local rock quarry sources and the quality and quantity of available rock, including determination of rock durability and quarry screening capabilities.
- Considerations for Green and Sustainable Remediation (GSR).

H.2 PERFORMANCE STANDARDS

The Performance Standards presented here are defined in the Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Site (2011 Action Memo; USEPA, 2011), the ROD, (USEPA, 2013), and the AOC (USEPA, 2015) including the SOW attached as Appendix D to the AOC, and were developed to define attainment of the Removal Action and Remedial Action Objectives (RAOs) for the Selected Remedy. The Performance Standards include both general and specific standards applicable to the Selected Remedy work elements and associated work components. Table H.2-1 presents performance standards related to the borrow areas and borrow materials for construction and explains how the design accomplishes these standards.

Table H.2-1: Task Specific Performance Standards

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|---|---------------------------------|--|--|
| 95 | 2011 Action Memo, V.A.1, Bullet 8 – Site Restoration | Site Restoration | Restoration activities will include the backfilling and regrading of excavation areas for erosion and stormwater control. These areas will also be re-vegetated with native species | Excavated borrow areas will be regraded to provide stable slopes and enhance control of erosion and stormwater. Disturbed areas will be revegetated in accordance with the revegetation plan. See Sections H.4.3, H.4.4 and Appendix U. |
| 43 | 2013 ROD, Section 2.9.5, Stormwater and Erosion Control | Storm Water and Erosion Control | Disturbed areas will be graded to reduce scouring and erosion potential using gentle slopes, terraces, earthen ridges and catch drains (swales) as necessary. These controls will also be used to minimize the potential for ponded water, reduce the risk of percolation from ponded water, and divert water away from open disposal locations, construction areas, and exposed mine waste. The drainage patterns in the disturbed areas will be integrated with the existing topography and drainage patterns to the extent possible. During construction activities, stormwater controls may include stormwater control channels (header), weirs, spillways, catch basins, check dams, and sediment basins. These controls will be implemented to maintain a safe working environment, to protect human health and the environment, mitigate off-site migration of mine waste, and protect response construction actions. | Borrow areas will be excavated to maintain positive drainage away from the borrow sites. Excavated slopes will not exceed 3H:1V to mitigate against slope instability and erosion. Temporary best management practices (BMPs) for sediment control in disturbed areas will be implemented. Additional stormwater controls are not anticipated to be required due to existing topography at the borrow areas and anticipated storm events. See Sections H.4.2, H.4.3 and H.4.4. |

| Identifying Number* | Location of Performance Standard Requirement | Topic | Performance Standard | Comments |
|---------------------|--|---|--|---|
| 2 | 2015 AOC SOW, Paragraph 17 – Soil Transportation and Management | Soil Transport and Management | <p>In the Design, Respondents shall provide detailed plans and specifications explaining how mine waste from the NECR Site and other materials (including borrow, backfill, and cover materials) will be managed and transported.</p> <p>Respondents shall include details for ensuring that Principal Threat Waste from the NECR Site, as described in the 2011 Action Memo, is not transported to the UNC Site or disposed at the Tailings Disposal Area.</p> | <p>Management and transport of mine waste is addressed in appendices C, D, and M. Borrow materials are anticipated to be directly loaded and transported to the repository on haul roads designated for clean material (e.g. roads not used for transportation of mine waste). Rock will be hauled from offsite and will likely be stockpiled in a clean area near the repository.</p> <p>Management of principal threat waste (PTW) is described in Appendix C. See Section H.4.2, and appendices C and D.</p> |
| 12 | 2015 AOC SOW, Paragraph 27 – Site Restoration | Site Restoration | <p>In the Design, Respondents shall include detailed plans and specifications for restoration of the Tailings Disposal Area and borrow areas on the UNC Site and for restoration of the NECR Site. Respondents shall also include plans and specifications for contouring to promote drainage, and for re-vegetation of the Tailings Disposal Area, borrow pits and NECR Site with native species. Respondents shall include plans and specifications for backfilling and regrading of disturbed (e.g., excavated) areas in the NECR Site and the UNC Site for erosion and storm water control, including re-vegetation of those areas with native species</p> | <p>Design plans and specifications have been prepared with regrading and revegetation plans to promote drainage, control stormwater and erosion. Disturbed areas will be revegetated in accordance with project requirements</p> <p>See Section H.4.4 and Appendix U.</p> |
| 14 | 2015 AOC SOW, Paragraph 29 – Green Remediation Best Management Practices | Green Remediation Best Management Practices | <p>Respondents shall incorporate applicable Best Management Practices for Green Remediation listed in ASTM-E2893-13 consistent with USEPA's policy Superfund Green Remediation Strategy (2010), found at http://www.epa.gov/superfund/greenremediation/sf-gr-strategy.pdf.</p> | <p>Proposed BMPs for green remediation for the borrow areas are described in Section H.5.</p> |

*Refers to identifying numbers listed in Summary of ARARs, Performance Standards and Applicable NRC Design Requirements Table (provided in Attachment 1 to main text of the 95% Design Report)

H.3 ENGINEERING DESIGN DRAWINGS

The engineering design drawings for the borrow areas are contained in Volume II – Design Drawings (Section 8). The complete set of Drawings related to the borrow areas are listed in Table H-3.1 and referenced by sheet number in the text.

Table H.3-1: Engineering Design Drawings

| Drawing No. | Drawing Title |
|-------------|-----------------------------|
| 8-01 | Borrow Areas Location Map |
| 8-02 | Jetty Borrow Area |
| 8-03 | West Borrow Area |
| 8-04 | West Borrow Area Backfilled |
| 8-05 | East Borrow Area |
| 8-06 | South Borrow Area |

H.4 BORROW AREAS

Soil and rock are required for repository construction, including general fill to meet surface design grades, for the cover system and for long-term stormwater controls. As part of the PDS, MWH estimated that approximately 400,000 cubic yards (CY) of soil and rock for the cover system as well as clean general fill will be required for the project (MWH, 2014).

The current repository cover design consists of a 4.5-foot-thick cover system, separated into an upper erosion protection layer and a lower soil layer (with varying thicknesses of layers). The design erosion protection layer is comprised of a soil rock admixture with median rock sizes varying from 1.5 inches to 3.5 inches, depending on the surface grading, and the rock will be mixed with soil at about 33 percent, or 15 percent, by volume. Design criteria and the preliminary design for the soil cover system are discussed in Appendix G. Stantec does not anticipate that additional fill or cover material from the borrow areas will be required for the excavated Mine Site Area, since clean fill is available within the Mine Site Area for final grading and drainage requirements, if needed.

The estimated soil and rock volumes required for repository construction are provided in Table H.4-1 and detailed in the following sections.

Table H.4-1: Project Soil and Rock Material Volume Requirements

| Material Type | Quantity Required (CY) |
|---|------------------------|
| Soil to fill existing cover swales | 11,000 |
| Soil for cover layers | 430,000 |
| Clean soil fill for grading around repository | 12,000 |
| Rock for repository erosion protection | 60,000 |
| Rock for stormwater controls | 102,200 |
| Filter material for stormwater controls | 43,300 |

H.4.1 On-Site Borrow

Four proposed on-site borrow areas were identified during the PDS to meet the repository volume and material property requirements. An additional proposed borrow source, the jetty excavation, was identified during the 95% NECR design. The proposed improvements to the jetty area will require a significant volume of excavated material that can be used for construction. Details on the investigation and development of the five proposed borrow areas are provided in Sections H.4.1.1 through H.4.1.7. Additional on-site material, available for use in other project applications, is discussed in Section H.4.1.9. All five borrow areas are approved by Dwyer Engineering, LLC for cover soil on the Repository.

H.4.1.1 Borrow Area Investigations

Three borrow area investigations have been completed within the proposed borrow areas. The first investigation was completed in 2008 and consisted of 13 test pits excavated within the East Borrow Area and 12 test pits within the West Borrow Area. The 13 test pits within the East Borrow Area were excavated to depths ranging from 8 to 12 feet, while the depths of test pits located in the West Borrow Area ranged from 4 feet to 12 feet (MWH, 2012).

A second borrow area investigation was performed as part of the PDS in November and December 2013. This investigation was completed to further characterize and estimate available material quantities within the borrow areas. Seventeen boreholes were completed within four of the proposed borrow areas, with a minimum of two boreholes in each borrow area. Drilling depths

varied from 10 to 60 feet depending on borehole location within the borrow area and depths to bedrock. Continuous (dry-core) and bulk samples were collected and sent to a laboratory for geotechnical, analytical, and agronomic testing (MWH, 2014).

Analytical radiologic testing was completed on samples from each proposed borrow area to determine if the borrow material was suitable for use in the repository cover system. Test results indicate that radium-226 levels are between 0.8 and 1.7 pCi/g for the borrow materials tested. These test results are included in the Mill Site PDS Report (MWH, 2014).

Geotechnical and agronomic testing at the jetty excavation site was completed in 2016-2017 and during the 2018 investigation. Initial geotechnical test results and interpretation information is included in the Jetty Geotechnical Report (Attachment I.9 of Appendix I of this Design Report). The Jetty report from the 2018 investigation includes further geotechnical testing and detailed comparison of borrow source materials (Stantec, 2019). Attachment H.2 contains supplementary geotechnical testing data. A summary of the geotechnical data from the jetty area is included in Section H.4.1.7. The agronomic data for the jetty soils is included in Appendix P.

H.4.1.2 Borrow Areas Grading Plan Design

With the exception of the jetty excavation borrow, data collected from each borrow area investigation was used to develop preliminary borrow area grading plans to estimate available borrow material in each area. The design basis for the borrow area excavation and final grading plans is provided in Table H.4-2. The individual design basis items comply with regulatory requirements and/or generally accepted engineering practice and meet the overall project design criteria as provided in the Design Work Plan (MWH, 2016). The design basis and determination of borrow volume available for the jetty excavation is included in the description of the jetty design provided in Appendix I of this Design Report.

Table H.4-2: Borrow Excavation and Final Grading Design Basis

| Design Category | Design Basis | Reference |
|--|--|---|
| Borrow Material | Borrow material meets the geotechnical property requirements (fines content) for use in the repository soil cover system. | Repository cover design (Dwyer Engineering, 2017) |
| Borrow Slopes | The borrow area slopes have been designed with a nominal slope of 5H:1V, with no slopes steeper than 3H:1V. Borrow area slope lengths have been shortened where possible. | New Mexico Energy, Minerals and Natural Resources Department, Mining and Minerals Division – Closeout Plan Guidelines for Existing Mines (NMEMND, 1996) |
| Archeological/Cultural Sites | Identified archeological/cultural sites have been accounted for in the borrow areas development designs. | New Mexico Energy, Minerals and Natural Resources Department, Mining and Minerals Division – Closeout Plan Guidelines for Existing Mines (NMEMND, 1996) |
| Radiologic Requirement | Radiologic sampling was completed at each of the proposed borrow locations to confirm that the borrow material to be used for construction will not contain concentrations of RA-226 or Uranium exceeding the RAL. | 2015 AOC (USEPA, 2015) |
| Temporary Stormwater/Erosion Controls Design Storm Event | Design Storm for Temporary Stormwater/Erosion Controls: 2-year, 24-hour storm. | Engineer's experience and judgment |
| Borrow Excavation Depths | Borrow area excavation depths have been developed from review of available information. Borrow area configurations have been designed to maximize suitable available materials, based on known information. | Engineer's experience and judgment based on review of existing site information (MWH, 2014) |

| Design Category | Design Basis | Reference |
|--------------------------------------|---|---|
| Borrow Clearing and Grubbing | Preparation of borrow areas and locations for borrow material stockpiles will include clearing and grubbing to a depth of 12 inches. | Engineer's experience and judgment based on review of existing site information |
| Long-Term Stormwater/Erosion Control | Excavated borrow areas will be graded to provide for long-term slope stability as well as positive drainage from the borrow area to existing drainages. | New Mexico Energy, Minerals and Natural Resources Department, Mining and Minerals Division – Closeout Plan Guidelines for Existing Mines (NMEMND, 1996) |
| Site Reclamation | Disturbed areas in and around the proposed borrow areas will be reclaimed and revegetated. | New Mexico Energy, Minerals and Natural Resources Department, Mining and Minerals Division – Closeout Plan Guidelines for Existing Mines (NMEMND, 1996) |

Estimated available material volumes for each proposed borrow area and materials currently on site are provided in Table H.4-3 and are discussed in the following sections. The proposed grading plans for the borrow areas provide positive surface drainage. Should additional borrow material be required, borrow slopes may be steepened to maximum slopes of 3H:1V, if stable. Additional on-site materials, discussed in Section H.4.1.9, are also included in Table H.4-3.

Table H.4-3: Available On-Site Material Volumes (Soil and Rock)

| Location | Material Type | Estimated Volume (CY) |
|--|-------------------|-----------------------|
| North Borrow Area | Soil | 71,000 |
| South Borrow Area | Soil | 160,000 |
| East Borrow Area | Soil | 55,000 |
| West Borrow Area | Soil | 89,000 |
| Jetty Excavation ¹ | Soil | 486,500 |
| Jetty Excavation ² | Rock | 49,000 |
| Topsoil Stockpile | Soil | 20,000 |
| Stripped Tailings Impoundment Cover | Soil | 24,000 |
| Stripped Tailings Impoundment Cover | Rock ³ | 17,000 |
| Tailings Impoundment Swales ⁴ | Rock | 2,000 |
| Two 1-inch Rock Stockpiles | Rock | 6,000 |
| Various Rock Sizes Stockpiles (6" – 15") | Rock | 1,000 |
| Crusher Fines | - | 1,300 |
| Road Base | - | 700 |

Notes:

1. The Jetty soils can be used as cover soils or in other areas of the project for general fill.
2. The sandstone from the jetty excavation will not meet NRC durability requirements for erosion protection and cannot be used on the TDA except as general fill.
3. Erosion protection layer to be removed from existing Tailings Disposal Area (TDA) within the repository contains median 1.5-inch rock.
4. Existing swales within the repository will be stripped of erosion protection rock and filled.

H.4.1.3 West Borrow Area

The West Borrow Area is located southwest of the proposed repository location. Based on borehole and test pit logs, depth to the underlying bedrock within the proposed borrow areas is greater than 35 feet at its deepest location. The clayey sand and

silty or clayey sand overlying bedrock contains between 37 and 49 percent low plasticity fines. An estimated 89,000 CY of borrow material is available within this borrow area. This borrow area, including borrow excavation depths, is shown in the Section 8 Drawings. The West Borrow Haul Road, which is also shown in the Section 8 Drawings, will be used for borrow area access. Design for the West Borrow Haul Road is discussed in Appendix D.

H.4.1.4 East Borrow Area

The East Borrow Area is adjacent to the southeast corner of the proposed repository. This proposed borrow area consists of sandy clay and clayey sand with low plasticity fines ranging from 45 percent to 74 percent by weight. Investigation borehole logs indicate that depth to underlying bedrock ranges from near surface to greater than 20 feet. Approximately 55,000 CY of borrow material is available in this area. The bottom of borrow excavation contours, along with borrow excavation depths, are provided in the Section 8 Drawings. The East Borrow Haul Road will provide access to the borrow material and the design of the haul road is discussed in Appendix D.

H.4.1.5 South Borrow Area

The South Borrow Area, located northeast of the proposed repository, consists of an estimated 160,000 CY of available repository cover borrow material. Investigation borings show this borrow material reaches a maximum depth of greater than 25 feet and is comprised of sandy clay with low-plasticity fines ranging from 53 percent to 79 percent by weight. The South Borrow Area will be accessed by the South Borrow Haul Road, which is discussed in Appendix D. The South Borrow Area and South Borrow Haul Road are both shown in the Section 8 Drawings. Foundations from previous structures in this area, although not encountered during drilling, may affect borrow volumes.

H.4.1.6 North Borrow Area

The North Borrow Area is located to the northeast of the South Borrow Area. The North Borrow Area is in a narrow valley and is furthest away from the repository. This borrow is considered an auxiliary source of cover material, should suitable volumes of borrow material in the four other borrow areas be exhausted, and is not shown in the Section 8 drawings. Field investigation and laboratory data results from samples obtained within this borrow area indicate that the material consists of sandy or silty clay or clayey sand with low plasticity fines content ranging from 44 percent to 51 percent by weight. Approximately 71,000 CY of borrow material is available in this area, based on information from only two investigation boreholes. These boreholes indicate a depth to bedrock of up to 20 feet. Should construction conditions require use of material from the North Borrow, additional geotechnical investigation should be conducted to confirm available material volumes.

H.4.1.7 Jetty Excavation Borrow Area

The excavation for the proposed jetty improvements will be located southeast of the proposed repository and is expected to require an estimated 547,500 CY of soil and rock removal. This estimate consists of 49,000 CY of rock and 498,500 CY of soil. The 2018 investigation found that approximately 12,000 CY of soil is expected to be impacted material, resulting in an estimated 486,500 bank CY of borrow material available (Stantec, 2019). Investigation borings from the 2016 field program are described in the Jetty Geotechnical Report (Appendix I.9) and in the 2018 Geotechnical Data Report (Stantec, 2019). Excavation on the east side of the arroyo will consist of soil while the west side will be partially in shallow outcropping weathered sandstone. The soil on the east side generally consists of 10 to 15 feet of sand with few fines (SP, SP-SM) over interlayered silty sand (SM) and silty clays (CL, CH) to the depth of the proposed excavation for the jetty. The fines content ranges from about 6 to 18 percent in the upper 15 feet and from about 54 percent to 99 percent by weight below 15 feet. The Jetty Borrow Area will be accessed from access roads on both sides of the arroyo. Table H.4-4 summarizes the laboratory results of the Jetty materials obtained from the investigation borings from the 2016 field program. The results from the 2018 investigation are presented in Stantec (2019).

The boring logs indicate that coarser materials will be encountered and excavated within the jetty area before the deeper finer materials. As such, approximately 16,500 CY of coarser (sandy) material may be used as general fill for the steeper (20%) NE slope of the repository. Fifteen hundred additional cubic yards of the coarser (sandy) excavated material, if screened, may meet the requirements for use as filter material within the site-wide surface water drainage channels.

The Jetty Excavation will require as much as 49,000 CY of rock (sandstone) excavation on the west side of the Arroyo in order to construct the Jetty improvements. This material has been tested for durability and is not suitable as erosion protection rock on the TDA. The sandstone can be used as general site fill, road fill, or will be disposed in the West Borrow. Remaining coarse (sandy) materials, not approved for use as cover fill or filter materials, will be hauled to the West Borrow Area and included in the final reclaimed borrow area grading. Drawing 8-04 includes a conceptual fill plan for the West Borrow Area after the existing borrow soil in that area has been removed. Up to 440,000 CY (soil and rock) can be placed back in the West Borrow, if necessary.

Table H.4-4: Summary of Jetty Geotechnical Laboratory Test Results (2016 and 2018 field investigations)

| Borehole | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (ft) | | USCS ⁽²⁾ | Water Content (by mass, %) | Dry Density (pcf) | Porosity (%) | Specific Gravity | Standard Proctor (max. dd@opt. w.c.), (pcf @ %) | Atterberg Limits (%) ⁽³⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | Pinhole Dispersion ^(3,4E) | Double Hydrometer (% Dispersion) ^(5E) | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽⁶⁾ | SWCC: Saturated Water Content (by vol., %) ⁽⁶⁾ |
|----------|-----------------------------|----------------------------|----------------------------|------|---------------------|----------------------------|-------------------|--------------|------------------|---|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|--------------------------------------|--|---|---|
| | | | | | | | | | | | LL | PL | PI | | | | | | | | | |
| B4A | B4A-4A | CA | 5.0 | 5.5 | CL | 11.2 | 91.7 | 44.7 | 2.66 | | 43 | 20 | 23 | 0.0 | 7.3 | 92.7 | 53.0 | 39.7 | | | | |
| B4A | B4A-14A | CA | 15.0 | 15.5 | CL | | | | | | | | | 0.0 | 11.0 | 89.0 | 64.3 | 24.6 | | | | |
| B4 | B4-16-16.5 ⁽⁷⁾ | CA | 16.0 | 16.5 | | 10.4 | 77.6 | | | | | | | | | | | | | | | |
| B4A | B4A-24A | CA | 25.0 | 25.5 | CL | 13.0 | 91.3 | 45.3 | 2.68 | | 27 | 16 | 11 | 0.0 | 34.8 | 65.2 | 36.9 | 28.3 | | | | |
| B5 | B5-5.5-6 ⁽⁷⁾ | CA | 5.5 | 6.0 | SM | 9.8 | 82.4 | | | | | | | 5.0 | 77.0 | 18.0 | | | | | | |
| B5A | B5A-9A | CA | 10.0 | 10.5 | ML | 7.5 | 81.8 | 50.1 | 2.63 | | NP | | | 0.0 | 41.9 | 58.1 | 39.8 | 18.3 | 0 (NP) | 16 | 9.2E-05 | 39.4 |
| B5 | B5-10.5-11 ⁽⁷⁾ | CA | 10.5 | 11.0 | | 5.2 | 82.9 | | | | | | | | | | | | | | | |
| B5A | B5A-14A | CA | 15.0 | 15.5 | CL | 14.7 | 73.4 | 55.6 | | | | | | 0.0 | 9.9 | 90.1 | 57.0 | 33.1 | | | | |
| B5 | B5-TW-25-27 ⁽⁷⁾ | ST | 25.0 | 27.0 | CH | 21.2 | 98.8 | | | | 61 | 21 | 40 | | | | | | | | | |
| B5 | B5-30.5-31 ⁽⁷⁾ | CA | 30.5 | 31.0 | CL | | | | | | | | | 0.0 | 14.0 | 86.0 | | | | | | |
| B5 | B5-40.5-41 ⁽⁷⁾ | CA | 40.5 | 41.0 | CL | 22.7 | 99.6 | | | | 49 | 20 | 29 | 0.0 | 1.0 | 99.0 | | | | | | |
| B6 | B6-12.8-13.8 ⁽⁷⁾ | CA | 12.8 | 13.5 | SP-SM | | | | | | | | | 5.0 | 89.0 | 6.0 | | | | | | |
| B6 | B6-15.5-16 ⁽⁷⁾ | CA | 15.5 | 16.0 | CL | 10.7 | 93 | | | | 42 | 17 | 25 | 0.0 | 10.0 | 90.0 | | | | | | |
| B6A | B6A-19A | CA | 20.0 | 20.5 | CH | 14.2 | 111.8 | 33.3 | 2.69 | | 51 | 22 | 29 | 0.0 | 13.3 | 86.7 | 44.2 | 42.5 | | 9 | 7.0E-05 | 45.5 |
| B6A | B6A-20-40 (1+2) | Bulk | 20.0 | 40.0 | CL | | | | | 104.9 @ 17.9 | 28 | 17 | 11 | 0.0 | 12.6 | 87.3 | 45.1 | 42.2 | | | | |
| B6 | B6-40.5-41 ⁽⁷⁾ | CA | 40.5 | 41.0 | | 17.9 | 97 | | | | | | | | | | | | | | | |
| B7 | B7-TW-5-6.5 ⁽⁷⁾ | ST | 5.0 | 6.5 | | 6.9 | 94.3 | | | | | | | | | | | | | | | |
| B7 | B7-10.5-11 ⁽⁷⁾ | CA | 10.5 | 11.0 | CL | 7.0 | 99.7 | | | | | | | 0.0 | 10.0 | 90.0 | | | | | | |
| B7 | B7-16-16.5 ⁽⁷⁾ | CA | 16.0 | 16.5 | CL | 17.8 | 99.3 | | | | 35 | 19 | 16 | 1.0 | 45.0 | 54.0 | | | | | | |
| B7 | B7-30.5-31 ⁽⁷⁾ | CA | 30.5 | 31.0 | CL | 16.7 | 102.5 | | | | 40 | 16 | 24 | | | | | | | | | |
| B7A | B7A-39A | CA | 40.0 | 40.5 | ML | 13.6 | 108.2 | 35.3 | | | NP | | | 0.3 | 42.5 | 57.2 | 37.4 | 19.8 | | | | |
| B7A | B7A-0-20 (1+2) | Bulk | 0.0 | 20.0 | CL | | | | 2.67 | 114.2 @ 13.7 | 29 | 15 | 14 | 0.7 | 37.1 | 62.2 | 41.1 | 21.1 | | 17 | 8.9E-05 | 40.3 |
| B7A | B7A-40-60 (1+2) | Bulk | 40.0 | 60.0 | CL | | | | 2.67 | 109.9 @ 15.0 | 37 | 16 | 21 | 0.6 | 28.3 | 71.1 | 40.6 | 30.5 | | 12 | 5.9E-04 | 40.8 |
| B8 | B8-9A | CA | 10.0 | 10.5 | CH | 14.5 | 102.9 | 38.3 | 2.67 | | 54 | 24 | 30 | 0.0 | 3.1 | 96.9 | 48.6 | 48.3 | | | | |
| B8 | B8-24A | CA | 25.0 | 25.5 | CH | 17.8 | 105.8 | 36.0 | | | 67 | 24 | 43 | 0.0 | 0.9 | 99.1 | 41.8 | 57.3 | | | | |
| B8 | B8-34A | CA | 35.0 | 35.5 | SM | 8.3 | 104.9 | 36.9 | 2.67 | | NP | | | 0.2 | 62.4 | 37.4 | 26.9 | 10.5 | | | | |
| B9 | B9-9A | CA | 10.0 | 10.5 | CH | 15.1 | 103.3 | 38.4 | 2.69 | | 52 | 23 | 29 | 0.0 | 3.1 | 96.9 | 49.8 | 47.1 | | | | |
| B9 | B9-39A | CA | 40.0 | 40.5 | ML | 11.6 | 98.6 | 40.8 | 2.67 | | NP | | | 0.0 | 31.4 | 68.6 | 43.7 | 24.9 | | | | |
| B9 | B9-20-35 (1+2) | Bulk | 20.0 | 35.0 | CH | | | | 2.71 | 100.5 @ 20.8 | 55 | 21 | 34 | 0.0 | 3.5 | 96.4 | 42.6 | 53.9 | | 5 | 4.1E-05 | 45.9 |

Table H.4-5: Summary of Jetty Geotechnical Laboratory Test Results (2016 and 2018 field investigations) (cont.)

| Borehole | Sample | Sample Type ⁽¹⁾ | Sample Depth Interval (ft) | | USCS ⁽²⁾ | Water Content (by mass, %) | Dry Density (pcf) | Porosity (%) | Specific Gravity | Standard Proctor (max. dd@opt. w.c.), (pcf @ %) | Atterberg Limits (%) ⁽³⁾ | | | USCS % Gravel | USCS % Sand | % Passing No. 200 Sieve (fines) | % Silt | USDA % Clay (<0.002 mm) | Pinhole Dispersion ^(3,4,6) | Double Hydrometer (% Dispersion) ^(5,6) | Remolded Saturated Hydraulic Conductivity (cm/sec) ⁽⁶⁾ | SWCC: Saturated Water Content (by vol., %) ⁽⁶⁾ |
|-----------|-----------------|----------------------------|----------------------------|------|---------------------|----------------------------|-------------------|--------------|------------------|---|-------------------------------------|----|----|---------------|-------------|---------------------------------|--------|-------------------------|---------------------------------------|---|---|---|
| | | | | | | | | | | | LL | PL | PI | | | | | | | | | |
| B10 | B10-4A | CA | 5.0 | 5.5 | CL | 10.1 | 114.6 | 30.9 | 2.66 | | 31 | 15 | 16 | 0.3 | 41.8 | 57.9 | 38.3 | 19.6 | | | | |
| B10 | B10-39A | CA | 40.0 | 40.5 | CH | | | | 2.68 | | 56 | 21 | 35 | 0.0 | 5.4 | 94.6 | 56.5 | 38.1 | | 15 | 5.5E-05 | 46.7 |
| B10 | B10-10-25 (1+2) | Bulk | 10.0 | 25.0 | CL | | | | 2.68 | 115.5 @ 13.8 | 45 | 16 | 29 | 1.1 | 38.3 | 60.5 | 38.0 | 22.5 | ND2 | 0 | 1.0E-04 | 40.1 |
| B11 | B11-14A | CA | 15.0 | 15.5 | CL | 7.5 | 119.9 | 27.8 | 2.66 | | 27 | 15 | 12 | 1.1 | 46.3 | 52.6 | 35.3 | 17.3 | | | | |
| B11 | B11-29B | CA | 30.0 | 30.5 | CL | 15.7 | 97 | 41.6 | 2.67 | | 31 | 17 | 14 | 0.3 | 33.0 | 66.7 | 44.2 | 22.5 | | | | |
| B11 | B11-39A | CA | 40.0 | 40.5 | SM | 9.5 | 94.5 | 42.9 | 2.66 | | NP | | | 0.0 | 64.6 | 35.4 | 29.1 | 6.3 | 0 (NP) | 0 | 1.0E-03 | 39.7 |
| B11 | B11-0-10 (1+2) | Bulk | 0.0 | 10.0 | CL | | | | 2.66 | 113.6 @ 14.8 | 30 | 16 | 14 | 0.9 | 35.9 | 63.2 | 38.0 | 25.2 | | | | |
| Composite | Clay | Bulk | - | - | CL | 17.6 | 96.2 | | | 106.6 @ 17.7 | 42 | 19 | 24 | 0.0 | 20.0 | 80.0 | | | | | 4.8E-07 | 37.8 |
| Composite | Sand | Bulk | - | - | SM | 14.8 | 102.0 | | | 112.4 @ 14.2 | | | | 0.0 | 65.0 | 35.0 | | | | | 2.5E-05 | 35.0 |

- Notes:
1. Sample Types: CA = California sample, Bulk = bucket/grab sample
 2. USCS = Unified Soil Classification System, material descriptions are based on field observations, and refined with laboratory data, if available. USCS classifications are provided only where sufficient laboratory data are available. CL = low plasticity clay, CH = high plasticity clay, SM = silty sand, ML = silt, SP - SM = poorly graded sand - silty sand
 3. LL = liquid limit, PL = plastic limit, PI = plasticity index, NP = non-plastic
 4. ND2 = nondispersive clay with very slight to no colloidal erosion under 15-inch or 40-inch head (ASTM test method A)
 5. <30% = non-dispersive, 30 to 50% intermediate, >50% dispersive
 6. Specimens remolded to approximately 90% of maximum standard Proctor dry density and between the estimated natural and optimum water contents for the soil. SWCC tests performed with pairs of specimens
 7. 2016 Geotechnical Investigation Sample

H.4.1.8 Proposed Borrow Areas Materials Evaluation

The soil properties from the first four site borrow areas were used by Dwyer Engineering, LLC for the cover modeling and cover calculations described in the Cover System Design Report (Dwyer, 2017), included as Attachment G.7 to Appendix G. The Jetty area soil was evaluated for use as cover material and details are included in an addendum to the Cover System Design Report, addendum to Attachment G.7 of Appendix G. Based on information provided by Dwyer Engineering, the material in any of these borrow areas can be used as needed, and will not require phasing, sequencing, or blending to meet the project cover specifications. Based on 2018 test results for the soil from the Jetty Borrow Area the Repository cover design has been modified to include this material as one of the borrow sources for cover construction. Each of the borrow sources may also be used as general fill, or may be screened and used as filter material for the site surface water drainage channels.

H.4.1.9 Additional On-Site Materials

H.4.1.9.1 Topsoil Stockpile

A topsoil stockpile is located on General Electric/United Nuclear Corporation (GE/UNC) property west of New Mexico Highway 566 (NM 566) and south of the GE/UNC office. Geotechnical investigation information included in the PDS report indicates that approximately 20,000 CY of soil available in this stockpile (MWH, 2014).

H.4.1.9.2 Existing Impoundment Erosion Protection Layer

The existing tailings impoundment cover is constructed with a 6-inch-thick admixture layer of soil and 1.5-inch D_{50} (median diameter) erosion protection rock. Test gradations (eight samples) indicate the existing mixture contains an average of 45 percent rock by mass. Testing, completed during the tailings impoundment cover construction, indicates the 1.5-inch D_{50} rock meets durability requirements for rock included in the repository cover system. Fines contents range from 57 to 73 percent for the existing cover soil (MWH, 2014).

This existing admixture layer will be stripped from the footprint of the new repository, screened to separate rock and soil, and reused for new cover construction. Assuming the mixture is about 42 percent rock by volume, approximately 24,000 CY of soil and 17,000 CY of rock will be available from the stripped cover soil mixture (MWH, 2014). This rock will be used for rock admixture in the repository cover.

H.4.1.9.3 Comparison of Available Borrow Sources

Stantec reviewed the available geotechnical information for the borrow areas and compared it with samples obtained from the existing soil cover on the tailings impoundment as well as the topsoil stockpile on the west side of NM 566, south of the GE/UNC offices (MWH, 2014). This comparison was made to determine if the tailings impoundment cover, anticipated to be stripped prior to placement of mine waste, and the topsoil stockpile material are suitable for use in the repository soil cover system. Specifically, the borrow area soil gradations and Atterberg Limits were compared with the existing TDA cover soil and topsoil stockpile material. As can be seen in Figures H.4-1 and H.4-2, the existing tailings impoundment cover soils and topsoil stockpile materials have similar index properties to the proposed borrow material.

H.4.1.9.4 Rock Material Stockpiles

There are existing material stockpiles at various locations on-site, as shown in the Section 8 Drawings. These materials were characterized during the Mill Site PDS (MWH, 2014). Estimated volumes contained within each of these stockpiles are provided in Table H.4-3. Because the rock in these stockpiles is not of sufficient D_{50} size for the repository cover or channel erosion protection, this material is anticipated to be utilized for temporary surfacing on the proposed haul roads and Support Area facilities.

H.4.1.9.5 Impoundment Surface Swale Rock Removal

Approximately 2,000 CY of 1.5-inch D₅₀ rock will also be removed from the existing swales located within the repository area, on the TDA surface, prior to placement of mine waste. The durability of this rock was tested by Canonie Environmental in 1991, as part of the existing cover swales design, with results showing the rock meets the NRC durability requirements for erosion protection (Canonie, 1991).

H.4.2 Excavation Methods and Procedures

Excavation of the borrow material will be performed using typical earthmoving equipment, including dozers, motor graders, front-end loaders, excavators, rubber-tired backhoes, water trucks, and haul trucks. The following list provides the anticipated excavation procedures for the borrow areas. These procedures align with generally accepted excavation and material borrow practices.

- Establishment of stormwater and erosion control features at borrow area locations.
- Surface vegetation will be stripped from the proposed excavation and borrow material stockpile areas and placed in a topsoil stockpile adjacent to each borrow area. It is estimated that approximately 12 inches of overburden (clearing and grubbing) will be stripped from the surface of each of these areas. The stockpiled topsoil will be reused during borrow area reclamation activities.
- Excavations will include provisions for drainage away from the current borrow area working face to minimize disruption of the borrow activities due to stormwater.
- Sloped excavations will be completed to design grades (to a maximum 3H:1V slope) and design elevations.
- To the extent possible, excavated borrow materials will be loaded directly into haul trucks, transported, and placed within the repository. Exceptions will be 1) stockpiling of borrow material prior to cover material placement to meet project schedule requirements and 2) stockpiling of excavated sandy material from Jetty borrow to be screened and used as filter materials in stormwater control channels site-wide. Quality control for borrow material will be conducted in accordance with the procedures outlined in Appendix J: Technical Specifications.

H.4.2.1 Stockpiling of Material

Although it is anticipated that cover material excavated from the borrow areas will be directly loaded into haul trucks for transportation and placement on the repository, borrow material may be stockpiled temporarily within the borrow areas, depending on construction schedules. These temporary stockpiles will be maintained at stable slope angles with dust and other erosion control measures applied as required. Sandy materials excavated from the Jetty borrow may be stockpiled in locations to be determined, prior to being screened for use as filter materials for site surface drainage channels. Rock imported from offsite quarries is anticipated to be stockpiled on the north or west side of the North Cell of the tailings impoundment.

H.4.2.2 Dust Control

Fugitive dust will be suppressed using the following measures:

- Enforcement of speed limits on haul roads
- Application of water to excavation areas, work areas, and haul roads with water trucks
- Application of approved chemical agents such as calcium chloride or magnesium chloride (or others) to haul roads
- Placement of aggregate wearing course on haul roads to mitigate dust generation in highly trafficked areas

H.4.3 Temporary Construction Stormwater and Erosion Control

The Construction Contractor (CC) will be responsible for implementing temporary construction stormwater and erosion BMPs during the borrow excavation activities to mitigate release of sediment due to erosion into existing drainages, as discussed in Appendix E and shown on the Section 5 Drawings. BMPs could include erosion and sediment controls at the outlets of each borrow area and on exposed slopes within borrow areas. Borrow excavation will be completed to allow for drainage away from the current borrow area working face to minimize disruption of the borrow activities.

H.4.4 Borrow Areas Reclamation

A preliminary post-excavation borrow surface, based on estimated depths of available borrow material, has been developed for each proposed on-site borrow area, with the exception of the jetty excavation. The post-excavation surfaces provide positive drainage away from the borrow areas and into surrounding drainages. Preliminary post-excavation borrow area cut slopes are nominally 5H:1V to provide for long-term erosional stability and revegetation. The post-excavation borrow areas will be reclaimed and revegetated as described in Appendix U. Final, post-excavation grading surfaces for each of the borrow areas are shown in the Section 8 drawings.

H.4.5 Off-Site Borrow Areas

Due to scarcity of competent rock sources within the project area, most rock required for erosion protection in the soil cover system, as well as for erosion protection in stormwater channels, will be obtained from an off-site source (quarry). Rock sizes and quantities required for the project are provided in Table H.4-5. Additional rock in larger sizes will be required for channel construction and the jetty improvements. In addition to size requirements, rock used for the project must meet durability requirements given in the NRC's 2002 Design of Erosion Protection for Long-Term Stabilization (NRC, 2002). Results from specific laboratory durability tests are scored using a weighted evaluation system based on rock type. NRC requires laboratory testing to show that a proposed rock source obtains a score of 80 or greater in order to be approved for use in uranium tailings projects, without additional oversizing of the rock. Granular filter materials, sand and gravel, will also be required for riprap bedding in some locations.

Table H.4-5: Repository Rock Requirements

| Material Specification | Quantity Required (CY) | Location(s) Used |
|----------------------------|------------------------|--|
| Type I Filter | 19,000 | East Repository Drainage Channel, Dilco Hill Channels A and B, Jetty/Pipeline Arroyo |
| Type II Filter | 24,300 | East Repository Drainage Channel, Dilco Hill Channels A and B, Jetty/Pipeline Arroyo |
| D ₅₀ = 1.5 in. | 17,000 | Repository Cover System |
| D ₅₀ = 2.0 in. | 13,500 | Repository Cover System |
| D ₅₀ = 3.5 in. | 29,100 | Repository Cover System |
| D ₅₀ = 3.0 in. | 17,200 | East Repository Drainage Channel, Jetty/Pipeline Arroyo, Erosion Protection for West Apron, 5H:1V Cover Slope Erosion Protection |
| D ₅₀ = 6.0 in. | 700 | Dilco Hill Channels A and B, Mine Site Outlet Channel |
| D ₅₀ = 9.0 in. | 1,700 | East Repository Drainage Channel, Dilco Hill Channels A and B, Mine Site Outlet Channel |
| D ₅₀ = 18.0 in. | 700 | Mine Site Outlet Channel |
| D ₅₀ = 27.0 in. | 87,800 | Jetty/Pipeline Arroyo |

Two off-site quarry sources have been identified for potential use during construction to supplement the available on-site rock. The sources are located near Gallup, NM (approximately 20 miles west of the project site). Each proposed rock source has the capability and capacity to produce sufficient quantities of each required size of rock. A limestone and a granite were tested for specific gravity, absorption, Schmidt hammer, sulfate soundness, and L.A. abrasion. Test results from one of the sources show that the produced rock meets NRC durability requirements without the need for additional oversizing. Test results from the other pit shows that the rock, a limestone, may need to be oversized 5 percent to meet NRC durability requirements. None of the rock from these sources contains radium-226 concentrations above the RAL and rock from any of these pits is suitable for use (with oversizing, as appropriate) in the repository cover system and for erosion protection. Attachment H.1 provides a summary of the analytical test results and the rock durability test results and scores, for the proposed off-site borrow locations. The project specifications require the selected contractor to confirm the durability test results by providing the Construction Supervising Contractor with an additional series of durability test results following selection of the quarry and prior to taking delivery of the rock. Petrographic analyses for the offsite limestone and granite sources indicated that both sources are considered suitable. The complete analyses are included in Appendix I.9.

H.5 GREEN AND SUSTAINABLE REMEDIATION CONSIDERATIONS

USEPA's Superfund Green Remediation Strategy Policy (USEPA, 2010) requires incorporation of BMPs for green remediation as listed in ASTM-E2893-16 (ASTM International, 2016). Specific proposed practices for the borrow areas relate to: (1) construction materials (characteristics, manufacturing and transportation considerations), (2) construction methods, and (3) low impact/sustainability measures during construction. The 'BMP Process', as outlined in the 'Standard for Greener Cleanups' (ASTM, 2016), has been followed to select and prioritize BMPs for implementation during remedial action. The BMPs relating to Borrow Areas are listed below, for a complete description of the BMP Process and list of all GSR BMPs see Section 4 and Appendix A (Section A.5) of the 95% Design Report.

H.5.1 Construction Material Considerations

On-site borrow sources have been utilized whenever possible and haul distances have been minimized in order to maximize fuel efficiency and reduce air emissions. Throughout the project the volume of borrow needed will be re-evaluated to avoid excavation of unneeded material.

H.5.2 Construction Methods

The Technical Specifications will encourage the CC to size equipment correctly for the task to minimize use of heavy equipment for small tasks and decreases fuel use and minimize greenhouse gas and dust emissions. Use of the nearest qualifying sources for borrow materials, or use and reuse of materials already onsite, will reduce fuel emissions associated with increased transportation distances. Over-excavation and unnecessary fuel use and emissions may also be reduced by re-evaluating necessary borrow volumes as the project progresses. Restoration and revegetation will be completed in a timely manner to minimize erosion and prevent growth of invasive species.

H.5.3 Low Impact Development/Sustainability

Key considerations of GSR principles as they apply to borrow pit selection and use focus on minimizing disturbance of previously undistributed areas and minimizing haul distance. Previously used borrow sources will be utilized to minimize disturbance of previously undisturbed areas, and selection and sequencing of borrow sources has been designed to minimize haul distance and associated fuel use and emissions.

H.6 REFERENCES

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- US Environmental Protection Agency (USEPA), 2011. Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Mine Site, McKinley County, New Mexico, Pinedale Chapter of the Navajo Nation. Prepared for U.S. EPA Regions 6 and 9. September 29.
- US Environmental Protection Agency (USEPA), 2013. Record of Decision, United Nuclear Corporation Site, McKinley County, New Mexico, EPA ID: NMD030443303. March 29.
- US Environmental Protection Agency (USEPA), 2015. Administrative Settlement Agreement and Order on Consent for Design and Cost Recovery, United Nuclear Corporation Superfund Site and Northeast Church Rock Mine Removal Site, McKinley County, New Mexico. April 27.

FIGURES

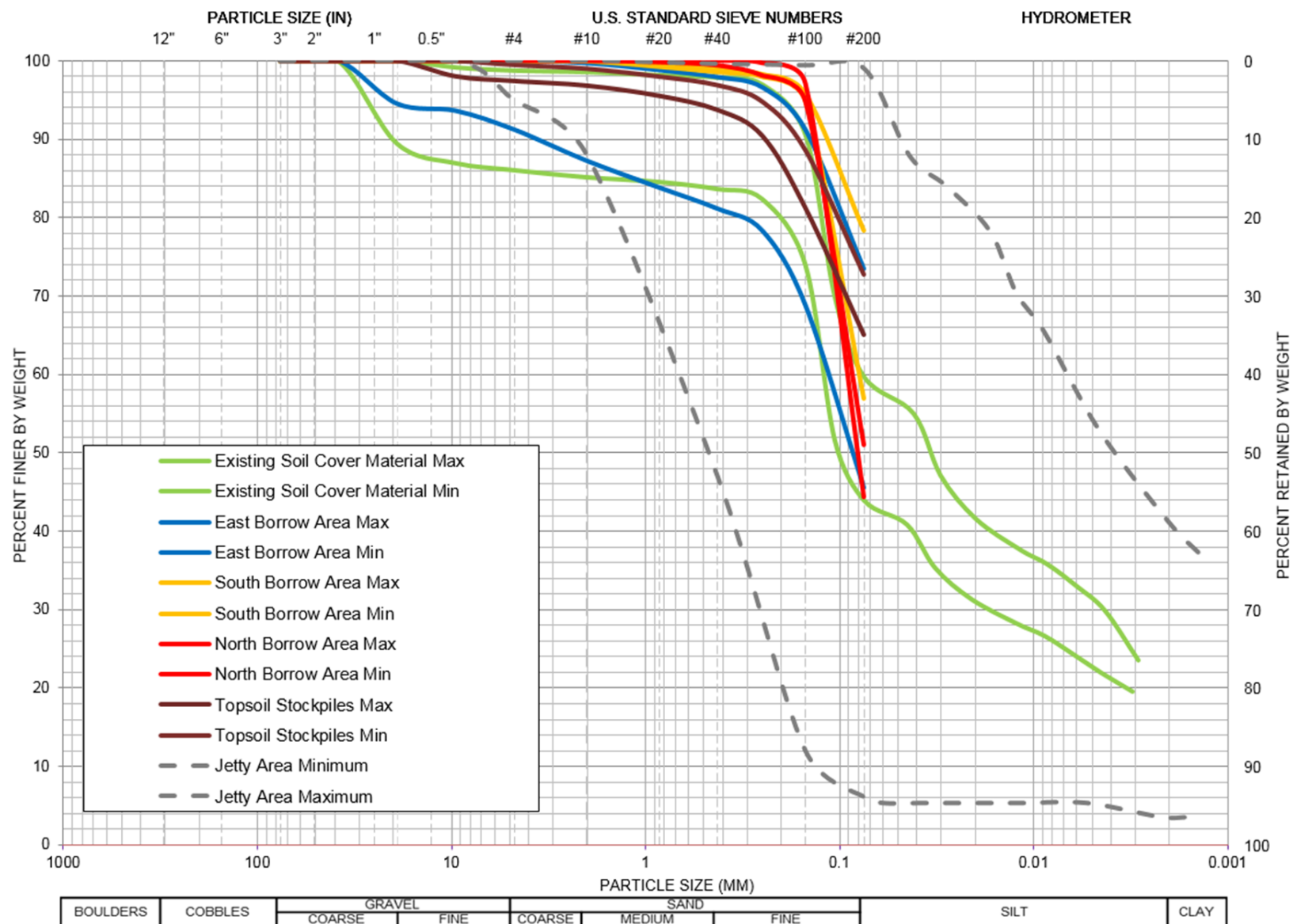


Figure H.4-1: NECR Borrow and Existing Tailings Impoundment Cover Soil Gradations

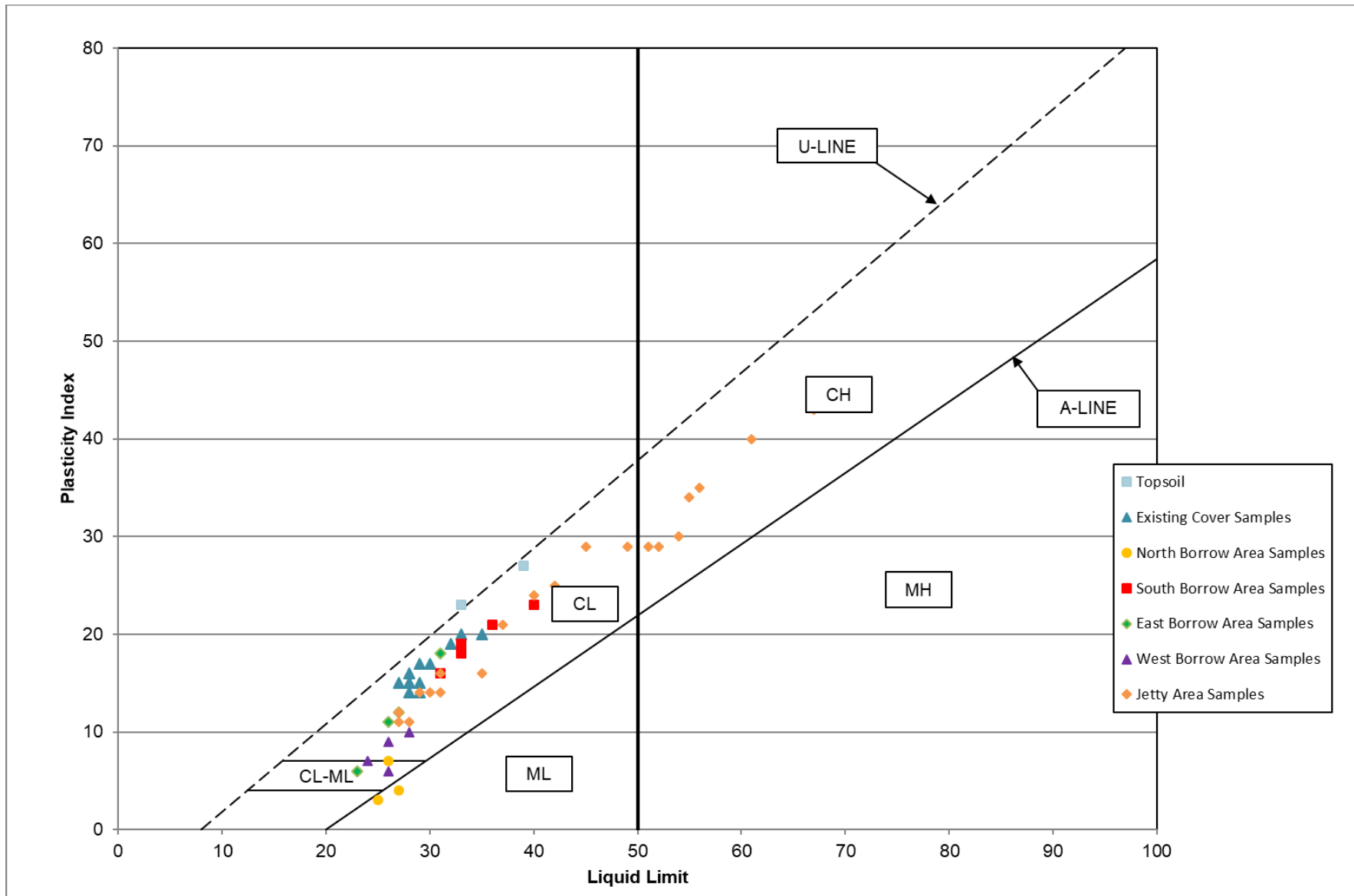


Figure H.4-2: NECR Borrow and Existing Tailings Impoundment Cover Soil Atterberg Limits

ATTACHMENT H.1

Summary of Offsite Rock Durability Testing and Scoring

TO: United Nuclear Corporation

DATE: July 12, 2016

FROM: Michael Witler

REFERENCE: 10505869

SUBJECT: Summary of Durability and Radiologic Testing for Rock Sources (Attachment H.1)

| Revisions | | | | | | |
|-----------|-----------|-------------|-----------|------------|-------------|-----------|
| Revision | Date | Description | Author | Reviewed | Approved | Submitted |
| 0 | 5/18/2016 | First issue | M. Witler | J. Cumbers | C. Strachan | 7/15/206 |
| | | | | | | |
| | | | | | | |

1.0 INTRODUCTION

This memorandum summarizes the results of the durability and radiologic testing of rock sources for the Northeast Church Rock (NECR) Removal Action repository soil cover system. This evaluation has been conducted for United Nuclear Corporation (UNC) by MWH Americas Inc. (MWH, now a part of Stantec).

The current design of the NECR repository soil cover system includes an erosion protection layer, comprised of a soil-rock admixture, as well as rock on the steeper 20 percent side slope on the eastern side of the repository, and rock for stormwater perimeter channels. Testing was previously conducted on basalt samples from the currently inactive Prewitt Pit, located near Thoreau, NM for another UNC project. A representative of UNC collected samples from the Page Pit (limestone) and the Tampico Pit (granite), both near Gallup, NM. The results from testing of samples from these three sources are presented in this memorandum.

2.0 DURABILITY TESTING

A summary of the durability testing scores for each rock source is included in Tables 1 through 3. The test results were scored for durability based on the guidance in NUREG-1623, Table D-1 (NRC, 2002). The individual test scores are based on a range from 0 to 10 with weighting factors based on rock types. Durability testing included; specific gravity, absorption, sodium sulfate soundness, L.A. abrasion, and Schmidt hammer. The supporting laboratory data for the scores are attached. The rock scores for the three sources are calculated on a similar suite of testing, with the exception of the Prewitt Pit calculation, which does not include a Schmidt hammer or Compressive Strength test results. The L.A. abrasion test performed on the basalt

sample was conducted per ASTM for 500 revolutions. The result shown in the table below has been adjusted for 100 revolutions per NUREG-1623.

The rock quality scores are 209/230 (or 91 percent for the Prewitt Pit), 211/260 (or 81 percent for the Page Pit), and 308/410 (or 75 percent for the Tampico Pit). Based on NRC guidance for durability of erosion protection material, material from the Prewitt and Page Pits would be acceptable, without oversizing (score 80 to 100). Rock from the Tampico Pit would require oversizing of 5 percent (increase in the design median (D_{50}) rock sizes).

Table 1. Summary of Scoring Criteria – Prewitt Pit Basalt^a

| Laboratory Test | Result | Score | Weighting Factor ^c | Weighted Score | Maximum Score |
|--|--------|-------|-------------------------------|----------------|---------------|
| Bulk Specific Gravity (ASTM C127) | 2.717 | 9 | 9 | 81 | 90 |
| Absorption, % (ASTM C127) | 1.0 | 5 | 2 | 10 | 20 |
| Sodium Sulfate, % (ASTM C88) | 0.2 | 10 | 11 | 110 | 110 |
| LA Abrasion, % (ASTM C535, 100 revs ^b) | 5.4 | 8 | 1 | 8 | 10 |
| | | | Totals | 209 | 230 |
| Durability Score = Weighted Score / Maximum Score | | | | 91% | |

Notes:

- a) Test results provided by Vinyard and Associates, Albuquerque, NM (2012)
- b) 100 revolutions is a deviation from ASTM, per NUREG-1623
- c) Weighting factors from Table D-1 NUREG-1623 for igneous (basalt) rock

Table 2. Summary of Scoring Criteria – Page Pit Granite^a

| Laboratory Test | Result | Score | Weighting Factor ^d | Weighted Score | Maximum Score |
|--|--------|-------|-------------------------------|----------------|---------------|
| Bulk Specific Gravity ^b (ASTM C127) | 2.585 | 7 | 9 | 63 | 90 |
| Absorption, % (ASTM C127) | 0.8 | 6 | 2 | 12 | 20 |
| Sodium Sulfate, % (ASTM C88) | 0.2 | 10 | 11 | 110 | 110 |
| LA Abrasion, % (ASTM C535, 100 revs ^c) | 5.0 | 8 | 1 | 8 | 10 |
| Schmidt Hammer (ASTM D5873) | 49 | 6 | 3 | 18 | 30 |
| | | | Totals | 211 | 260 |
| Durability Score = Weighted Score / Maximum Score | | | | 81% | |

Notes:

- a) Test results provided by Ninyo and Moore, Phoenix, AZ (2016)
- b) Specific gravity taken as saturated surface dry density
- c) 100 revolutions is a deviation from ASTM, per NUREG-1623
- d) Weighting factors from Table D-1 NUREG-1623 for igneous (granite)

Table 3. Summary of Scoring Criteria – Tampico Pit Limestone^a

| Laboratory Test | Result | Score | Weighting Factor ^d | Weighted Score | Maximum Score |
|---|--------|-------|-------------------------------|----------------|---------------|
| Bulk Specific Gravity ^b (ASTM C127) | 2.682 | 8 | 12 | 96 | 120 |
| Absorption, % (ASTM C127) | 0.4 | 8.5 | 13 | 110 | 130 |
| Sodium Sulfate, % (ASTM C88) | 0.5 | 10 | 4 | 40 | 40 |
| LA Abrasion, % (ASTM C535, 100 revs ^c) | 7.0 | 7 | 1 | 7 | 10 |
| Schmidt Hammer (ASTM D5873) | 40 | 5 | 11 | 55 | 110 |
| | | | Totals | 308 | 410 |
| Durability Score = Weighted Score / Maximum Score = | | | | 75% | |

Notes:

- a) Test results provided by Ninyo and Moore, Phoenix, AZ (2016)
- b) Specific gravity taken as saturated surface dry density
- c) 100 revolutions is a deviation from ASTM, per NUREG-1623
- d) Weighting factors from Table D-1 NUREG-1623 for limestone

3.0 RADIOLOGIC TESTING

Representative samples from each of the three proposed rock quarries were also sent to AVM Environmental Services Inc., in Grants, NM, for radiologic testing, namely radium-226 activity concentration. The test results were used for comparison with the project Removal Action (RA) levels (2.24 pCi/g) (USEPA, 2011). Based on results from the laboratory testing (included as Attachment B to this memo), rock from each of the three proposed quarries is below the RA level for radium-226, and suitable for use on the project.

4.0 CONCLUSIONS

Based on the durability and radiologic test results, rock material from each of the three proposed quarries are suitable for use as erosion protection rock for the project, either in the repository cover or for erosion protection in channels. One of the identified quarry sources would require oversizing of the design rock sizes.

5.0 REFERENCES

- U.S. Nuclear Regulatory Commission (NRC), 2002. Design of Erosion Protection for Long-Term Stabilization," NUREG-1623. September.
- U.S. Environmental Protection Agency (USEPA), Region 6 and Region 9, 2011. Action Memorandum: Request for a Non-Time-Critical Removal Action at the Northeast Church Rock Site, McKinley County, New Mexico, Pinedale Chapter of the Navajo Nation. September 29.

ATTACHMENT A
DURABILITY TEST RESULTS

Prewitt Pit Test Results

Project No.: 12-2-125

Sample: Proposed Rip Rap material sampled onsite.

Sample No.: 407

Date Tested: 9/6/2012

Tested By: mn

SOUNDNESS OF AGGREGATE (C88-05 Sodium Sulfate)

| Sieve Size | Number of Pieces | Grading of original sample % | Wt. of test fraction before test, g | Wt. of test fraction after test, g | % Passing designated sieve after test | Weighted % loss | Container Number |
|--------------|------------------|------------------------------|-------------------------------------|------------------------------------|---------------------------------------|-----------------|------------------|
| 3" to 2 1/2" | 12 | 33.2% | 6885.40 | 6868.40 | 0.2% | 0.1% | A |
| 3 1/2" to 3" | 7 | 33.3% | 6912.30 | 6902.10 | 0.1% | 0.0% | B |
| 4" to 3 1/2" | 5 | 33.5% | 6949.50 | 6925.70 | 0.3% | 0.1% | C |
| Totals | | 100.0% | 20747.2 | 20696.2 | | 0.2% | |

Absorption and Bulk Specific Gravity of Dimension Stone C97-02

| Specimen No.: | Wt. of dry aggregate A | Wt of aggregate SSD B | Wt of Agg in water C | Bulk Spec Grav (A/(B-C)) | Absortion % [(B-A)/A] x 100 |
|---------------|------------------------|-----------------------|----------------------|--------------------------|-----------------------------|
| 407 | 10049.5 | 10148.6 | 6450.4 | 2.717 | 1.0 |
| Required | | | | >2.65 | |

Resistance to Degradation of Large-size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine (ASTM C535-09)

Test results

| Sample | Material | Grading | Abrasion Loss |
|--------|----------|------------|---------------|
| 407 | | 1 / 10,000 | 27.0% |

Prewitt Pit Test results

Sample No.: 407

Date Tested: 9/6/2012

Tested By: mn

Project No.: 12-2-125

Sample: Proposed Rip Rap material sampled onsite.

| Lab Test | Result | Score | Weight | Score x Weight | Max Score |
|---------------|--------|-------|--------|----------------|-----------|
| SP. Grav | 2.717 | 9 | 9 | 81 | 90 |
| Absorp., % | 1.0 | 5 | 2 | 10 | 20 |
| Sod. Sulf., % | 0.2 | 10 | 11 | 110 | 110 |
| L.A. Abr., % | 27.0 | 1 | 1 | 1 | 10 |

| | | | | | |
|--------|--|--|--|-----|-----|
| Totals | | | | 202 | 230 |
|--------|--|--|--|-----|-----|

| | | | |
|------------------|-------------------------------------|----|--------------|
| Durability Score | = (Score x weight) / (Max Score) = | 88 | 80 min req'd |
|------------------|-------------------------------------|----|--------------|

NUREG-1623

Table D-1. Scoring criteria for determining rock quality.

| Laboratory Test | Weighting Factor | | | Score | | | | | | | | | | |
|----------------------------|------------------|------------|---------|-------|------|------|------|------|------|------|------|------|------|------|
| | Lime-stone | Sand-stone | Igneous | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1 | 0 |
| | | | | Good | | | Fair | | | Poor | | | | |
| Sp. Gravity | 12 | 6 | 9 | 2.75 | 2.70 | 2.65 | 2.60 | 2.55 | 2.50 | 2.45 | 2.40 | 2.35 | 2.30 | 2.25 |
| Absorption, % | 13 | 5 | 2 | .10 | .30 | .50 | .67 | .83 | 1.0 | 1.5 | 2.0 | 2.5 | 3.0 | 3.5 |
| Sodium Sulfate, % | 4 | 3 | 11 | 1.0 | 3.0 | 5.0 | 6.7 | 8.3 | 10.0 | 12.5 | 15.0 | 20.0 | 25.0 | 30.0 |
| L/A Abrasion (100 revs), % | 1 | 8 | 1 | 1.0 | 3.0 | 5.0 | 6.7 | 8.3 | 10.0 | 12.5 | 15.0 | 20.0 | 25.0 | 30.0 |
| Schmidt Hammer | 11 | 13 | 3 | 70 | 65 | 60 | 54 | 47 | 40 | 32 | 24 | 16 | 8 | 0 |

1. Scores were derived from Tables 6.2, 6.5, and 6.7 of NUREG/CR-2642 - "Long-Term Survivability of Riprap for Armoring Uranium Mill Tailings and Covers: A Literature Review," 1982.
2. Weighting Factors are derived from Table 7 of "Petrographic Investigations of Rock Durability and Comparisons of Various Test Procedures," by G. W. DuPont, Engineering Geology, July 1965. Weighting factors are based on inverses of ranking of test methods for each rock type. Other tests may be used; weighting factors for these tests may be derived using Table 7, by counting upward from the bottom of the table.
3. Test methods should be standardized, if a standard test is available and should be those used in NUREG/CR-2642, so that proper correlations can be made.

D-30

SAMPLE INFORMATION:

LOCATION: PAGE PIT
DESCRIPTION: AGGREGATE DURABILITY TESTING
LAB TECHNICIAN: HJG

| METHOD | | | | METHOD | |
|---------------------------|--------|------------|------|------------------------|----|
| MECHANICAL SIEVE ANALYSIS | | | | LA ABRASION | |
| ASTM C 136 | | | | ASTM C 131 | |
| SIEVE SIZE | | % PASSING | | RESULTS | |
| US STD | METRIC | RESULTS | SPEC | SPEC | |
| 2" | 50 | -- | -- | 100 REVOLUTIONS % LOSS | 5 |
| 1.5" | 38 | -- | -- | 200 REVOLUTIONS % LOSS | 10 |
| 1.25" | 31.5 | -- | -- | | |
| 1" | 25 | -- | -- | | |
| 3/4" | 19 | -- | -- | | |
| 1/2" | 12.5 | -- | -- | | |
| 3/8" | 9.5 | -- | -- | | |
| 1/4" | 6.4 | -- | -- | | |
| No. 4 | 4.8 | -- | -- | | |
| No. 8 | 2.36 | -- | -- | | |
| No. 10 | 2.00 | -- | -- | | |
| No. 16 | 1.18 | -- | -- | | |
| No. 30 | 0.60 | -- | -- | | |
| No. 40 | 0.425 | -- | -- | | |
| No. 50 | 0.3 | -- | -- | | |
| No. 100 | 0.15 | -- | -- | | |
| No. 200 | 0.075 | NOT TESTED | | | |

| COARSE SPECIFIC GRAVITY & ABSORPTION | | ASTM C 127 |
|--------------------------------------|---------|------------|
| | RESULTS | SPEC |
| SPECIFIC GRAVITY, OD | 2.565 | |
| SPECIFIC GRAVITY, SSD | 2.585 | |
| SPECIFIC GRAVITY, APP | 2.617 | |
| % ABSORPTION | 0.8 | |

*INDICATES OUT OF TOLERANCE

| | | | |
|--------------------------|--------------|---|-----------------------------|
| Ninyo & Moore | | SOILS/AGGREGATE DATA SHEET | LAB NO. 47539 |
| PROJECT NO. | DATE SAMPLED | | |
| 604667002 | 4/27/2016 | MWH/MISC LABORATORY TESTING PHOENIX, ARIZONA | |

SAMPLE INFORMATION:

LOCATION: TAMPICO
DESCRIPTION: AGGREGATE DURABILITY TESTING
LAB TECHNICIAN: HJG

| METHOD | | | | METHOD | |
|---------------------------|--------|------------|------|--------------------------|------|
| MECHANICAL SIEVE ANALYSIS | | | | LA ABRASION | |
| ASTM C 136 | | | | ASTM C 131 | |
| SIEVE SIZE | | % PASSING | | RESULTS | |
| US STD | METRIC | RESULTS | SPEC | SPEC | |
| 2" | 50 | -- | -- | 100 REVOLUTIONS % LOSS | 7 |
| 1.5" | 38 | -- | -- | 200 REVOLUTIONS % LOSS | 13 |
| 1.25" | 31.5 | -- | -- | SODIUM SULFATE SOUNDNESS | |
| 1" | 25 | -- | -- | ASTM C 88 | |
| 3/4" | 19 | -- | -- | RESULTS | |
| 1/2" | 12.5 | -- | -- | SPEC | |
| 3/8" | 9.5 | -- | -- | %LOSS - COARSE AGG | 0.5 |
| 1/4" | 6.4 | -- | -- | SCHMIDT HAMMER | |
| No. 4 | 4.8 | -- | -- | ASTM C 805 | |
| No. 8 | 2.36 | -- | -- | RESULTS | |
| No. 10 | 2.00 | -- | -- | SPEC | |
| No. 16 | 1.18 | -- | -- | HARDNESS NUMBER | 40 |
| No. 30 | 0.60 | -- | -- | STRENGTH (PSI) | 6500 |
| No. 40 | 0.425 | -- | -- | | |
| No. 50 | 0.3 | -- | -- | | |
| No. 100 | 0.15 | -- | -- | | |
| No. 200 | 0.075 | NOT TESTED | | | |

| COARSE SPECIFIC GRAVITY & ABSORPTION | | ASTM C 127 |
|--------------------------------------|---------|------------|
| | RESULTS | SPEC |
| SPECIFIC GRAVITY, OD | 2.672 | |
| SPECIFIC GRAVITY, SSD | 2.682 | |
| SPECIFIC GRAVITY, APP | 2.700 | |
| % ABSORPTION | 0.4 | |

*INDICATES OUT OF TOLERANCE

| | | | | |
|--------------------------|---------------------|---|--|----------------|
| Ninyo & Moore | | SOILS/AGGREGATE DATA SHEET | | LAB NO. |
| PROJECT NO. | DATE SAMPLED | MWH/MISC LABORATORY TESTING PHOENIX, ARIZONA | | 47540 |
| 604667002 | 4/27/2016 | | | |

ATTACHMENT B
RADIOLOGIC TESTING RESULTS

AVM Environmental Services, Inc.
Gamma Spectroscopy Run Data

Technician VP

| Standard ID | | | Std & Bkg Count Date & Time | WTst | CNst | CTst | CTb | ROI | PAst | PAb | PAb | | | | |
|---|-----------------------|-------------|--------------------------------|-----------------------------|---------------------------|--------------------------|--------------------------------|------------------------|----------------------------|-------------------------------|------------------------|--------------------|-----|-----|-----|
| | | | | Std wt (gms) | Std Ra-226 (pCi/gm) | Std Count Time (Mins) | Bkg Count time | | Std Peak Area Counts | Bkg Peak Area | Bkg Peak Int Counts | | | | |
| RAS01-1100 Ra-226 Standard, 100.0 pCi/g (Sealed on 08-06-2012) | | | | 9/30/12 12:01 | 1100 | 100.0 | 20 | 20 | ROI 3 (Pb-214 351 kev) | 91,026 | 0 | 660 | | | |
| | | | | | | | | ROI 4 (Bi-214 609 kev) | 75,662 | 0 | 752 | | | | |
| | | | | | | | | | | | | | | | |
| GS Tag Number | Sample ID | Sample Date | Sample Seal Date &Time | Sample Count Date & Time | Ingrowth Period (Days) | WTs | CTs | ROI | PAs | Sample Results, Ra-226 pCi/gm | | | | | |
| | | | | | | Sample wt (gms) | Sample Count time (Mins) | ROIs | Sample Peak Area Counts | Peak Conc | Avg Conc | Uncertainty 95% | | LLD | |
| 2513 | Prewitt Basalt Quarry | 9/29/2012 | 9/29/12 11:59 | 9/30/12 12:25 | 1 | 1178 | 20 | ROI 4 (Bi-214 609 kev) | 272 | 0.3 | 0.3 | 0.1 | 0.1 | 0.2 | 0.2 |

Projected
pCi/g
<0.5

AVM Environmental Services, Inc.

Gamma Spectroscopy Run Data

Technician VP

| Standard ID | | | Std & Bkg Count Date & Time | WTst | CNst | CTst | CTb | ROI | Past | PAb | PAb | | | | | |
|---|------------------------|-------------|--------------------------------|-----------------------------|---------------------------|--------------------------|--------------------------------|------------------------|----------------------------|-------------------------------|------------------------|--------------------|-----|-----|-----|-----|
| | | | | Std wt (gms) | Std Ra-226 (pCi/gm) | Std Count Time (Mins) | Bkg Count time | | Std Peak Area Counts | Bkg Peak Area Counts | Bkg Peak Int Counts | | | | | |
| RAS01-1100 Ra-226 Standard, 100.0 pCi/g (Sealed on 08-06-2012) | | | | 5/6/16 9:15 | 1100 | 100.0 | 20 | 20 | ROI 3 (Pb-214 351 kev) | 91,813 | 35 | | | | | 556 |
| | | | | | | | | | ROI 4 (Bi-214 609 kev) | 75,972 | 30 | | | | | 642 |
| | | | | | | WTs | CTs | ROI | PAs | Sample Results, Ra-226 pCi/gm | | | | | | |
| GS Tag Number | Sample ID | Sample Date | Sample Seal Date &Time | Sample Count Date & Time | Ingrowth Period (Days) | Sample wt (gms) | Sample Count time (Mins) | ROIs | Sample Peak Area Counts | Peak Conc | Avg Conc | Uncertainty 95% | | LLD | | |
| 1 | Page Pit, Granite | 4/26/2016 | 5/2/16 9:00 | 5/6/16 9:30 | 4 | 1100 | 20 | ROI 3 (Pb-214 351 kev) | 526 | 0.5 | 0.8 | 0.1 | 0.1 | 0.2 | 0.2 | |
| | | | | | | 1100 | 20 | ROI 4 (Bi-214 609 kev) | 662 | 0.8 | | 0.1 | | 0.2 | | |
| 2 | Tampico Pit, Limestone | 4/26/2016 | 5/2/16 9:00 | 5/6/16 9:55 | 4 | 1100 | 20 | ROI 3 (Pb-214 351 kev) | 357 | 0.4 | 0.4 | 0.1 | 0.1 | 0.2 | 0.2 | |
| | | | | | | 1100 | 20 | ROI 4 (Bi-214 609 kev) | 286 | 0.3 | | 0.1 | | 0.2 | | |

Projected
pCi/g

1.0

0.5

ATTACHMENT H.2

Supplementary Geotechnical Laboratory Testing

Laboratory Report for Stantec MWH

NECR Jetty Borrow Soil, P.O. # P30109-N

September 27, 2017



Daniel B. Stephens & Associates, Inc.

4400 Alameda Blvd. NE, Suite C • Albuquerque, New Mexico 87113



September 27, 2017

Stephanie Downey
Stantec MWH
2130 Resort Drive, Suite 350
Steamboat Springs, CO 80487
(970) 871-4389

Re: DBS&A Laboratory Report for Stantec MWH, NECR Jetty Borrow Soil, P.O. # P30109-N Project

Dear Ms. Downey:

Enclosed is the report for the Stantec MWH, NECR Jetty Borrow Soil, P.O. # P30109-N project samples. Please review this report and provide any comments as samples will be held for a maximum of 30 days. After 30 days samples will be returned or disposed of in an appropriate manner.

All testing results were evaluated subjectively for consistency and reasonableness, and the results appear to be reasonably representative of the material tested. However, DBS&A does not assume any responsibility for interpretations or analyses based on the data enclosed, nor can we guarantee that these data are fully representative of the undisturbed materials at the field site. We recommend that careful evaluation of these laboratory results be made for your particular application.

The testing utilized to generate the enclosed report employs methods that are standard for the industry. The results do not constitute a professional opinion by DBS&A, nor can the results affect any professional or expert opinions rendered with respect thereto by DBS&A. You have acknowledged that all the testing undertaken by us, and the report provided, constitutes mere test results using standardized methods, and cannot be used to disqualify DBS&A from rendering any professional or expert opinion, having waived any claim of conflict of interest by DBS&A.

We are pleased to provide this service to Stantec MWH and look forward to future laboratory testing on other projects. If you have any questions about the enclosed data, please do not hesitate to call.

Sincerely,

DANIEL B. STEPHENS & ASSOCIATES, INC.
SOIL TESTING & RESEARCH LABORATORY

Joleen Hines
Laboratory Manager

Enclosure

Daniel B. Stephens & Associates, Inc.
Soil Testing & Research Laboratory

4400 Alameda Blvd. NE, Suite C
Albuquerque, NM 87113

505-889-7752
FAX 505-889-0258

Summaries



Summary of Tests Performed

| Laboratory Sample Number | Initial Soil Properties ¹ | | | Saturated Hydraulic Conductivity ² | | | Moisture Characteristics ³ | | | | | | | | Particle Size ⁴ | | | Specific Gravity ⁵ | | Air Perm- eability | Atterberg Limits | Proctor Compaction |
|-----------------------------|---|----|----|---|----|----|--|----|----|-----|----|----|-----|--------------------|-------------------------------|----|---|----------------------------------|---|--------------------------|---------------------|-----------------------|
| | G | VM | VD | CH | FH | FW | HC | PP | FP | DPP | RH | EP | WHC | K _{unsat} | DS | WS | H | F | C | | | |
| CHR-071117-Clay | | | | | | | | | | | | | | | | X | X | | | | X | |
| CHR-071117-Clay (90%) | X | X | | | | X | X | X | | X | X | | | X | | | | | | | | |
| CHR-071117-Sand | | | | | | | | | | | | | | | | X | X | | | | X | |
| CHR-071117-Sand (91%) | X | X | | | | X | X | X | | X | X | | | X | | | | | | | | |

¹ G = Gravimetric Moisture Content, VM = Volume Measurement Method, VD = Volume Displacement Method

² CH = Constant Head Rigid Wall, FH = Falling Head Rigid Wall, FW = Falling Head Rising Tail Flexible Wall

³ HC = Hanging Column, PP = Pressure Plate, FP = Filter Paper, DPP = Dew Point Potentiometer, RH = Relative Humidity Box, EP = Effective Porosity, WHC = Water Holding Capacity, K_{unsat} = Calculated Unsaturated Hydraulic Conductivity

⁴ DS = Dry Sieve, WS = Wet Sieve, H = Hydrometer

⁵ F = Fine (<4.75mm), C = Coarse (>4.75mm)



Notes

Sample Receipt:

Two samples, each in 1-gallon Ziploc bags with custody seals were received on July 14, 2017. The samples were double-bagged, surrounded with paper and delivered in a cardboard box. Both samples arrived in good order.

Sample Preparation and Testing Notes:

A portion of each sample was remolded into a testing ring to target 90% of the respective maximum dry bulk density at the respective optimum moisture content, based on client provided standard proctor compaction testing results. Each of these remolded sub-samples was subjected to initial properties analysis, saturation, and the hanging column and pressure chamber portions of the moisture retention testing. Secondary sub-samples were also prepared, using the same target remold parameters. The secondary sub-samples were then extruded from the testing ring and were subjected to saturated hydraulic conductivity testing via the flexible wall method. The actual percentage of maximum dry bulk density achieved was added to each sub-sample ID.

Separate sub-samples were obtained for the dewpoint potentiometer and relative humidity chamber portions of the moisture retention testing.

Particles larger than 4.75 mm were removed from the bulk material prior to remolding the sub-samples. Oversize correction calculations are not provided since the removed fraction is less than 5% of the bulk sample mass.

Porosity calculations, and the particle diameter calculations in the hydrometer portion of the particle size analysis testing, are based on the use of an assumed specific gravity value of 2.51.

Volumetric water contents were adjusted for changes in volume, where applicable. Due to the irregularities formed on the sample surfaces during swelling, volume measurements obtained after the initial reading should be considered estimates.



Summary of Sample Preparation/Volume Changes

| Sample ID | Proctor Data | | | Target Remold Parameters ¹ | | | | Actual Remold Data | | | | Volume Change Post Saturation ² | | | | Volume Change Post Drying Curve ³ | | | |
|-----------------------|-------------------|----------------------|------------------|---------------------------------------|----------------------|------------------|-------------------|--------------------|----------------------|------------------|-------------------|--|------------------|-----------------|-------------------|--|------------------|-----------------|-------------------|
| | Opt. Moist. Cont. | Max. Dry Density | Max. Dry Density | Moist. Cont. | Dry Bulk Density | Dry Bulk Density | % of Max. Density | Moist. Cont. | Dry Bulk Density | Dry Bulk Density | % of Max. Density | Dry Bulk Density | Dry Bulk Density | % Volume Change | % of Max. Density | Dry Bulk Density | Dry Bulk Density | % Volume Change | % of Max. Density |
| | (%, g/g) | (g/cm ³) | (pcf) | (%, g/g) | (g/cm ³) | (pcf) | (%) | (%, g/g) | (g/cm ³) | (pcf) | (%) | (g/cm ³) | (pcf) | (%) | (%) | (g/cm ³) | (pcf) | (%) | (%) |
| CHR-071117-Clay (90%) | 17.7 | 1.71 | 106.6 | 17.7 | 1.54 | 95.9 | 90% | 17.6 | 1.54 | 96.2 | 90.3% | 1.50 | 93.9 | 2.5% | 88.1% | 1.52 | 94.9 | 1.4% | 89.0% |
| CHR-071117-Sand (91%) | 14.2 | 1.80 | 112.4 | 14.2 | 1.62 | 101.2 | 90% | 14.8 | 1.63 | 102.0 | 90.8% | 1.63 | 102.0 | --- | 90.8% | 1.63 | 102.0 | --- | 90.8% |

¹Target Remold Parameters: Provided by the client: 90% of maximum dry density at optimum moisture content.

²Volume Change Post Saturation: Volume change measurements were obtained after saturated hydraulic conductivity testing.

³Volume Change Post Drying Curve: Volume change measurements were obtained throughout hanging column and pressure plate testing. The 'Volume Change Post Drying Curve' values represent the final sample dimensions after the last pressure plate point.

Notes:

"+" indicates sample swelling, "-" indicates sample settling, and "---" indicates no volume change occurred.



Daniel B. Stephens & Associates, Inc.

**Summary of Initial Moisture Content, Dry Bulk Density
Wet Bulk Density and Calculated Porosity**

| Sample Number | Moisture Content | | | | Dry Bulk Density (g/cm ³) | Wet Bulk Density (g/cm ³) | Calculated Porosity (%) |
|-----------------------|-------------------------|--|-------------------------|--|--|--|-------------------------|
| | As Received | | Remolded | | | | |
| | Gravimetric (%, g/g) | Volumetric (%, cm ³ /cm ³) | Gravimetric (%, g/g) | Volumetric (%, cm ³ /cm ³) | | | |
| CHR-071117-Clay (90%) | NA | NA | 17.6 | 27.1 | 1.54 | 1.81 | 41.8 |
| CHR-071117-Sand (91%) | NA | NA | 14.8 | 23.9 | 1.62 | 1.86 | 39.0 |

NA = Not analyzed

--- = This sample was not remolded



Summary of Saturated Hydraulic Conductivity Tests

| Sample Number | K_{sat} (cm/sec) | Oversize Corrected K_{sat} (cm/sec) | Method of Analysis | |
|-----------------------|-----------------------|--|--------------------------------|-------------------------------|
| | | | Constant Head Flexible Wall | Falling Head Flexible Wall |
| CHR-071117-Clay (90%) | 4.8E-07 | --- | | X |
| CHR-071117-Sand (89%) | 2.5E-05 | --- | | X |

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

NR = Not requested

NA = Not applicable



**Summary of Moisture Characteristics
of the Initial Drainage Curve**

| Sample Number | Pressure Head (-cm water) | Moisture Content (%, cm ³ /cm ³) |
|-----------------------|------------------------------|--|
| CHR-071117-Clay (90%) | 0 | 38.2 ‡ |
| | 57 | 38.3 ‡ |
| | 155 | 36.8 ‡ |
| | 337 | 36.4 ‡ |
| | 1428 | 31.3 ‡ |
| | 13563 | 23.0 ‡ |
| | 49970 | 16.6 ‡ |
| | 130126 | 12.7 ‡ |
| | 440554 | 9.2 ‡ |
| | 851293 | 6.9 ‡ |
| CHR-071117-Sand (91%) | 0 | 34.0 |
| | 12 | 33.8 |
| | 35 | 33.4 |
| | 105 | 23.8 |
| | 337 | 20.5 |
| | 5303 | 12.7 |
| | 28350 | 7.7 |
| | 128189 | 5.3 |
| | 549468 | 3.7 |
| | 851293 | 3.1 |

‡ Volume adjustments are applicable at this matric potential (see data sheet for this sample).



Summary of Calculated Unsaturated Hydraulic Properties

| Sample Number | α (cm ⁻¹) | N (dimensionless) | θ_r (% vol) | θ_s (% vol) | Oversize Corrected | |
|-----------------------|---------------------------------|----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | | | θ_r (% vol) | θ_s (% vol) |
| CHR-071117-Clay (90%) | 0.0008 | 1.2352 | 0.00 | 37.78 | --- | --- |
| CHR-071117-Sand (91%) | 0.0285 | 1.2264 | 0.00 | 35.02 | --- | --- |

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

NR = Not requested

NA = Not applicable



Summary of Particle Size Characteristics

| Sample Number | d ₁₀ (mm) | d ₅₀ (mm) | d ₆₀ (mm) | C _u | C _c | Method | ASTM Classification | USDA Classification | |
|-----------------|-------------------------|-------------------------|-------------------------|----------------|----------------|--------|---------------------------|------------------------|-------|
| CHR-071117-Clay | 0.00016 | 0.0068 | 0.015 | 94 | 0.50 | WS/H | Lean clay with sand (CL)s | Clay Loam | (Est) |
| CHR-071117-Sand | 8.8E-14 | 0.11 | 0.14 | 1.6E+12 | 3.1E+11 | WS/H | Silty sand (SM) | Sandy Loam | (Est) |

d₅₀ = Median particle diameter

Est = Reported values for d₁₀, C_u, C_c, and soil classification are estimates, since extrapolation was required to obtain the d₁₀ diameter

$$C_u = \frac{d_{60}}{d_{10}}$$

$$C_c = \frac{(d_{30})^2}{(d_{10})(d_{60})}$$

DS = Dry sieve

H = Hydrometer

WS = Wet sieve

[†] Greater than 10% of sample is coarse material



Percent Gravel, Sand, Silt and Clay*

| Sample Number | % Gravel (>4.75mm) | % Sand (<4.75mm, >0.075mm) | % Silt (<0.075mm, >0.002mm) | % Clay (<0.002mm) |
|-----------------|-----------------------|-------------------------------|--------------------------------|----------------------|
| CHR-071117-Clay | 0.2 | 20.0 | 44.1 | 35.8 |
| CHR-071117-Sand | 0.0 | 64.5 | 23.9 | 11.6 |

*USCS classification does not classify clay fraction based on particle size. USDA definition of clay (<0.002mm) used in this table.



Summary of Atterberg Tests

| Sample Number | Liquid Limit | Plastic Limit | Plasticity Index | Classification |
|-----------------|--------------|---------------|------------------|----------------|
| CHR-071117-Clay | 43 | 19 | 24 | CL |
| CHR-071117-Sand | --- | --- | --- | ML |

--- = Soil requires visual-manual classification due to non-plasticity

Initial Properties



**Summary of Initial Moisture Content, Dry Bulk Density
Wet Bulk Density and Calculated Porosity**

| Sample Number | Moisture Content | | | | Dry Bulk Density (g/cm ³) | Wet Bulk Density (g/cm ³) | Calculated Porosity (%) |
|-----------------------|-------------------------|--|-------------------------|--|--|--|-------------------------|
| | As Received | | Remolded | | | | |
| | Gravimetric (%, g/g) | Volumetric (%, cm ³ /cm ³) | Gravimetric (%, g/g) | Volumetric (%, cm ³ /cm ³) | | | |
| CHR-071117-Clay (90%) | NA | NA | 17.6 | 27.1 | 1.54 | 1.81 | 41.8 |
| CHR-071117-Sand (91%) | NA | NA | 14.8 | 23.9 | 1.62 | 1.86 | 39.0 |

NA = Not analyzed

--- = This sample was not remolded



Daniel B. Stephens & Associates, Inc.

**Data for Initial Moisture Content,
Bulk Density, Porosity, and Percent Saturation**

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay (90%)
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

| | <u>As Received</u> | <u>Remolded</u> |
|---|--------------------|-----------------|
| <i>Test Date:</i> | NA | 14-Aug-17 |
| <i>Field weight* of sample (g):</i> | | 536.85 |
| <i>Tare weight, ring (g):</i> | | 133.38 |
| <i>Tare weight, pan/plate (g):</i> | | 0.00 |
| <i>Tare weight, other (g):</i> | | 0.00 |
| <i>Dry weight of sample (g):</i> | | 343.08 |
| <i>Sample volume (cm³):</i> | | 222.62 |
| <i>Assumed particle density (g/cm³):</i> | | 2.65 |
| <hr/> | | |
| <i>Gravimetric Moisture Content (% g/g):</i> | | 17.6 |
| <i>Volumetric Moisture Content (% vol):</i> | | 27.1 |
| <i>Dry bulk density (g/cm³):</i> | | 1.54 |
| <i>Wet bulk density (g/cm³):</i> | | 1.81 |
| <i>Calculated Porosity (% vol):</i> | | 41.8 |
| <i>Percent Saturation:</i> | | 64.8 |
| <hr/> | | |
| <i>Laboratory analysis by:</i> | D. O'Dowd | |
| <i>Data entered by:</i> | J. Falance | |
| <i>Checked by:</i> | J. Hines | |

Comments:

* Weight including tares
NA = Not analyzed
--- = This sample was not remolded



**Data for Initial Moisture Content,
Bulk Density, Porosity, and Percent Saturation**

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Sand (91%)
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

| | <u>As Received</u> | <u>Remolded</u> |
|---|--------------------|-----------------|
| <i>Test Date:</i> | NA | 14-Aug-17 |
| <i>Field weight* of sample (g):</i> | | 546.70 |
| <i>Tare weight, ring (g):</i> | | 133.38 |
| <i>Tare weight, pan/plate (g):</i> | | 0.00 |
| <i>Tare weight, other (g):</i> | | 0.00 |
| <i>Dry weight of sample (g):</i> | | 360.15 |
| <i>Sample volume (cm³):</i> | | 222.72 |
| <i>Assumed particle density (g/cm³):</i> | | 2.65 |
| <hr/> | | |
| <i>Gravimetric Moisture Content (% g/g):</i> | | 14.8 |
| <i>Volumetric Moisture Content (% vol):</i> | | 23.9 |
| <i>Dry bulk density (g/cm³):</i> | | 1.62 |
| <i>Wet bulk density (g/cm³):</i> | | 1.86 |
| <i>Calculated Porosity (% vol):</i> | | 39.0 |
| <i>Percent Saturation:</i> | | 61.2 |
| <hr/> | | |
| <i>Laboratory analysis by:</i> | D. O'Dowd | |
| <i>Data entered by:</i> | J. Falance | |
| <i>Checked by:</i> | J. Hines | |

Comments:

* Weight including tares
NA = Not analyzed
--- = This sample was not remolded

Saturated Hydraulic Conductivity



Summary of Saturated Hydraulic Conductivity Tests

| Sample Number | K_{sat} (cm/sec) | Oversize Corrected K_{sat} (cm/sec) | Method of Analysis | |
|-----------------------|-----------------------|--|--------------------------------|-------------------------------|
| | | | Constant Head Flexible Wall | Falling Head Flexible Wall |
| CHR-071117-Clay (90%) | 4.8E-07 | --- | | X |
| CHR-071117-Sand (89%) | 2.5E-05 | --- | | X |

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

NR = Not requested

NA = Not applicable



Saturated Hydraulic Conductivity Flexible Wall Falling Head-Rising Tail Method

Job name: Stantec MWH
Job number: DB17.1177.00
Sample number: CHR-071117-Clay (90%)
Project name: NECR Jetty Borrow Soil
PO Number: P30109-N

Remolded or Initial Sample Properties

Initial Mass (g): 403.21
Diameter (cm): 6.107
Length (cm): 7.592
Area (cm²): 29.29
Volume (cm³): 222.38
Dry Density (g/cm³): 1.54
Dry Density (pcf): 96.3
Water Content (% g/g): 17.6
Water Content (% vol): 27.1
Void Ratio (e): 0.63
Porosity (% vol): 38.5
Saturation (%): 70.2

Post Permeation Sample Properties

Saturated Mass (g): 450.74
Dry Mass (g): 343.01
Diameter (cm): 6.282
Length (cm): 7.619
Deformation (%)**: 0.36
Area (cm²): 30.99
Volume (cm³): 236.15
Dry Density (g/cm³): 1.45
Dry Density (pcf): 90.7
Water Content (% g/g): 31.4
Water Content (% vol): 45.6
Void Ratio(e): 0.73
Porosity (% vol): 42.1
Saturation (%)*: 108.3

Test and Sample Conditions

Permeant liquid used: Tap Water
Sample Preparation: ☐ In situ sample, extruded
☒ Remolded Sample
Number of Lifts: 3
Split: #4
Percent Coarse Material (%): 0.2
Particle Density(g/cm³): 2.51 ☒ Assumed ☐ Measured
Cell pressure (PSI): 80.0
Influent pressure (PSI): 79.5
Effluent pressure (PSI): 78.5
Panel Used: ☐ D ☐ E ☒ F
Reading: ☐ Annulus ☒ Pipette
Date/Time
B-Value (% saturation) prior to test*: 1.00 9/27/17 750
B-Value (% saturation) post to test: 1.00 9/27/17 933

* Per ASTM D5084 percent saturation is ensured (B-Value \geq 95%) prior to testing, as post test saturation values may be exaggerated during depressurizing and sample removal.

**Percent Deformation: based on initial sample length and post permeation sample length.

Laboratory analysis by: D. O'Dowd
Data entered by: D. O'Dowd
Checked by: J. Hines



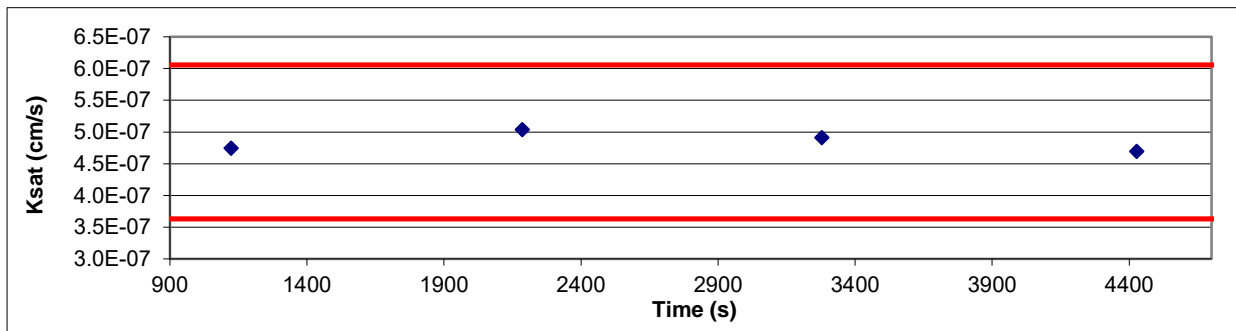
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Saturated Hydraulic Conductivity Flexible Wall Falling Head-Rising Tail Method

Job name: Stantec MWH
Job number: DB17.1177.00
Sample number: CHR-071117-Clay (90%)
Project name: NECR Jetty Borrow Soil
PO Number: P30109-N

| Date | Time | Temp (°C) | Influent Pipette Reading | Effluent Pipette Reading | Gradient ($\Delta H/\Delta L$) | Average Flow (cm ³) | Elapsed Time (s) | Ratio (outflow to inflow) | Change in Head (Not to exceed 25%) | k _{sat} T°C (cm/s) | k _{sat} Corrected (cm/s) |
|-----------|----------|-----------|--------------------------|--------------------------|----------------------------------|---------------------------------|------------------|---------------------------|------------------------------------|-----------------------------|-----------------------------------|
| Test # 1: | | | | | | | | | | | |
| 27-Sep-17 | 08:14:51 | 21.8 | 5.20 | 20.80 | 11.60 | 0.17 | 1124 | 1.00 | 1% | 4.96E-07 | 4.75E-07 |
| 27-Sep-17 | 08:33:35 | 22.0 | 5.40 | 20.60 | 11.54 | 0.17 | 1062 | 1.00 | 1% | 5.28E-07 | 5.03E-07 |
| Test # 2: | | | | | | | | | | | |
| 27-Sep-17 | 08:33:35 | 22.0 | 5.40 | 20.60 | 11.54 | 0.17 | 1062 | 1.00 | 1% | 5.28E-07 | 5.03E-07 |
| 27-Sep-17 | 08:51:17 | 22.1 | 5.60 | 20.40 | 11.48 | 0.17 | 1093 | 1.00 | 1% | 5.16E-07 | 4.91E-07 |
| Test # 3: | | | | | | | | | | | |
| 27-Sep-17 | 08:51:17 | 22.1 | 5.60 | 20.40 | 11.48 | 0.17 | 1093 | 1.00 | 1% | 5.16E-07 | 4.91E-07 |
| 27-Sep-17 | 09:09:30 | 22.1 | 5.80 | 20.20 | 11.41 | 0.17 | 1149 | 1.00 | 1% | 4.93E-07 | 4.69E-07 |
| Test # 4: | | | | | | | | | | | |
| 27-Sep-17 | 09:09:30 | 22.1 | 5.80 | 20.20 | 11.41 | 0.17 | 1149 | 1.00 | 1% | 4.93E-07 | 4.69E-07 |
| 27-Sep-17 | 09:28:39 | 22.1 | 6.00 | 20.00 | 11.35 | 0.17 | | | | | |

Average Ksat (cm/sec): 4.85E-07
Calculated Gravel Corrected Average Ksat (cm/sec): ---



ASTM Required Range (+/- 25%)

Ksat (-25%) (cm/s): 3.63E-07

Ksat (+25%) (cm/s): 6.06E-07



Saturated Hydraulic Conductivity Flexible Wall Falling Head-Rising Tail Method

Job name: Stantec MWH
Job number: DB17.1177.00
Sample number: CHR-071117-Sand (89%)
Project name: NECR Jetty Borrow Soil
PO Number: P30109-N

Remolded or Initial Sample Properties

Initial Mass (g): 410.42
Diameter (cm): 6.095
Length (cm): 7.632
Area (cm²): 29.18
Volume (cm³): 222.68
Dry Density (g/cm³): 1.61
Dry Density (pcf): 100.5
Water Content (% g/g): 14.5
Water Content (% vol): 23.4
Void Ratio (e): 0.56
Porosity (% vol): 35.9
Saturation (%): 65.2

Post Permeation Sample Properties

Saturated Mass (g): 439.33
Dry Mass (g): 358.33
Diameter (cm): 6.081
Length (cm): 7.604
Deformation (%)**: 0.36
Area (cm²): 29.04
Volume (cm³): 220.85
Dry Density (g/cm³): 1.62
Dry Density (pcf): 101.3
Water Content (% g/g): 22.6
Water Content (% vol): 36.7
Void Ratio(e): 0.55
Porosity (% vol): 35.4
Saturation (%)*: 103.7

Test and Sample Conditions

Permeant liquid used: Tap Water
Sample Preparation: ☐ In situ sample, extruded
☒ Remolded Sample
Number of Lifts: 3
Split: #4
Percent Coarse Material (%): 0.0
Particle Density(g/cm³): 2.51 ☒ Assumed ☐ Measured
Cell pressure (PSI): 80.0
Influent pressure (PSI): 79.2
Effluent pressure (PSI): 78.8
Panel Used: ☐ D ☒ E ☐ F
Reading: ☐ Annulus ☒ Pipette
Date/Time
B-Value (% saturation) prior to test*: 1.00 9/27/17 810
B-Value (% saturation) post to test: 1.00 9/27/17 855

* Per ASTM D5084 percent saturation is ensured (B-Value ≥ 95%) prior to testing, as post test saturation values may be exaggerated during depressurizing and sample removal.

**Percent Deformation: based on initial sample length and post permeation sample length.

Laboratory analysis by: D. O'Dowd
Data entered by: D. O'Dowd
Checked by: J. Hines



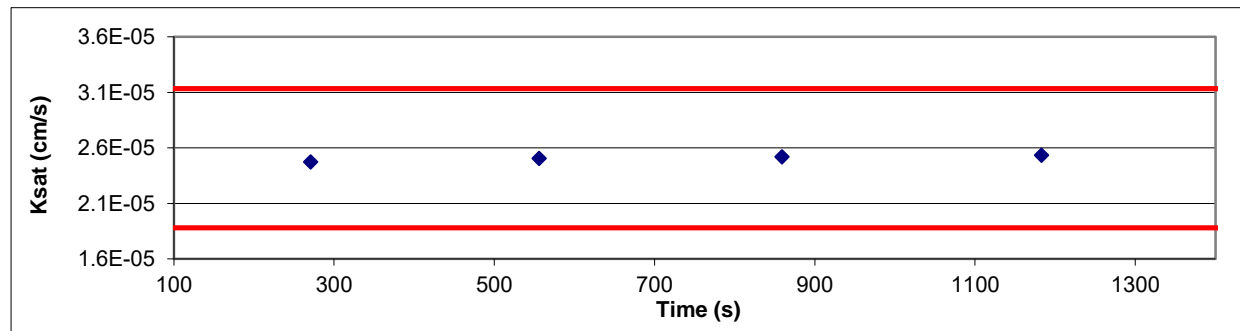
Daniel B. Stephens & Associates, Inc.

Saturated Hydraulic Conductivity Flexible Wall Falling Head-Rising Tail Method

Job name: Stantec MWH
Job number: DB17.1177.00
Sample number: CHR-071117-Sand (89%)
Project name: NECR Jetty Borrow Soil
PO Number: P30109-N

| Date | Time | Temp (°C) | Influent Pipette Reading | Effluent Pipette Reading | Gradient ($\Delta H/\Delta L$) | Average Flow (cm ³) | Elapsed Time (s) | Ratio (outflow to inflow) | Change in Head (Not to exceed 25%) | k _{sat} T°C (cm/s) | k _{sat} Corrected (cm/s) |
|-----------|----------|-----------|--------------------------|--------------------------|----------------------------------|---------------------------------|------------------|---------------------------|------------------------------------|-----------------------------|-----------------------------------|
| Test # 1: | | | | | | | | | | | |
| 27-Sep-17 | 08:23:19 | 21.9 | 9.00 | 18.00 | 5.07 | 0.87 | 271 | 1.00 | 6% | 2.59E-05 | 2.47E-05 |
| 27-Sep-17 | 08:27:50 | 21.9 | 10.00 | 17.00 | 4.76 | 0.87 | 285 | 1.00 | 6% | 2.62E-05 | 2.50E-05 |
| Test # 2: | | | | | | | | | | | |
| 27-Sep-17 | 08:27:50 | 21.9 | 10.00 | 17.00 | 4.76 | 0.87 | 285 | 1.00 | 6% | 2.62E-05 | 2.50E-05 |
| 27-Sep-17 | 08:32:35 | 22.0 | 11.00 | 16.00 | 4.46 | 0.87 | 303 | 1.00 | 7% | 2.64E-05 | 2.52E-05 |
| Test # 3: | | | | | | | | | | | |
| 27-Sep-17 | 08:32:35 | 22.0 | 11.00 | 16.00 | 4.46 | 0.87 | 303 | 1.00 | 7% | 2.64E-05 | 2.52E-05 |
| 27-Sep-17 | 08:37:38 | 22.0 | 12.00 | 15.00 | 4.16 | 0.87 | 324 | 1.00 | 7% | 2.66E-05 | 2.53E-05 |
| Test # 4: | | | | | | | | | | | |
| 27-Sep-17 | 08:37:38 | 22.0 | 12.00 | 15.00 | 4.16 | 0.87 | 324 | 1.00 | 7% | 2.66E-05 | 2.53E-05 |
| 27-Sep-17 | 08:43:02 | 22.0 | 13.00 | 14.00 | 3.85 | 0.87 | | | | | |

Average Ksat (cm/sec): 2.51E-05
Calculated Gravel Corrected Average Ksat (cm/sec): ---



ASTM Required Range (+/- 25%)

Ksat (-25%) (cm/s): 1.88E-05

Ksat (+25%) (cm/s): 3.13E-05

Moisture Retention Characteristics



**Summary of Moisture Characteristics
of the Initial Drainage Curve**

| Sample Number | Pressure Head (-cm water) | Moisture Content (%, cm ³ /cm ³) |
|-----------------------|------------------------------|--|
| CHR-071117-Clay (90%) | 0 | 38.2 ‡ |
| | 57 | 38.3 ‡ |
| | 155 | 36.8 ‡ |
| | 337 | 36.4 ‡ |
| | 1428 | 31.3 ‡ |
| | 13563 | 23.0 ‡ |
| | 49970 | 16.6 ‡ |
| | 130126 | 12.7 ‡ |
| | 440554 | 9.2 ‡ |
| | 851293 | 6.9 ‡ |
| CHR-071117-Sand (91%) | 0 | 34.0 |
| | 12 | 33.8 |
| | 35 | 33.4 |
| | 105 | 23.8 |
| | 337 | 20.5 |
| | 5303 | 12.7 |
| | 28350 | 7.7 |
| | 128189 | 5.3 |
| | 549468 | 3.7 |
| | 851293 | 3.1 |

‡ Volume adjustments are applicable at this matric potential (see data sheet for this sample).



Summary of Calculated Unsaturated Hydraulic Properties

| Sample Number | α (cm ⁻¹) | N (dimensionless) | θ_r (% vol) | θ_s (% vol) | Oversize Corrected | |
|-----------------------|---------------------------------|----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | | | θ_r (% vol) | θ_s (% vol) |
| CHR-071117-Clay (90%) | 0.0008 | 1.2352 | 0.00 | 37.78 | --- | --- |
| CHR-071117-Sand (91%) | 0.0285 | 1.2264 | 0.00 | 35.02 | --- | --- |

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

NR = Not requested

NA = Not applicable



Daniel B. Stephens & Associates, Inc.

Moisture Retention Data Hanging Column / Pressure Plate (Soil-Water Characteristic Curve)

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay (90%)
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Dry wt. of sample (g): 343.08
Tare wt., ring (g): 133.38
Tare wt., screen & clamp (g): 24.24
Initial sample volume (cm³): 222.62
Initial dry bulk density (g/cm³): 1.54
Assumed particle density (g/cm³): 2.65
Initial calculated total porosity (%): 41.84

| | Date | Time | Weight* (g) | Matric Potential (-cm water) | Moisture Content [†] (% vol) | |
|-----------------|-----------|-------|----------------|------------------------------------|---|----|
| Hanging column: | 15-Aug-17 | 8:20 | 587.81 | 0 | 38.19 | ## |
| | 22-Aug-17 | 9:15 | 588.73 | 57.0 | 38.29 | ## |
| | 29-Aug-17 | 16:35 | 585.15 | 155.0 | 36.83 | ## |
| Pressure plate: | 8-Sep-17 | 11:00 | 583.56 | 337 | 36.37 | ## |
| | 20-Sep-17 | 9:10 | 571.30 | 1428 | 31.28 | ## |

Volume Adjusted Data¹

| | Matric Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calculated Porosity (%) |
|-----------------|------------------------------------|--|--|---|---|
| Hanging column: | 0.0 | 228.10 | +2.46% | 1.50 | 43.24 |
| | 57.0 | 229.89 | +3.27% | 1.49 | 43.68 |
| | 155.0 | 229.30 | +3.00% | 1.50 | 43.54 |
| Pressure plate: | 337 | 227.84 | +2.35% | 1.51 | 43.18 |
| | 1428 | 225.72 | +1.40% | 1.52 | 42.65 |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent each of the volume change measurements obtained after saturated hydraulic conductivity testing and throughout hanging column/pressure plate testing. "---" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '---' denotes no volume change occurred.

* Weight including tares

[†] Assumed density of water is 1.0 g/cm³

Volume adjustments are applicable at this matric potential (see comment #1). Changes in volume, if applicable, are estimated based on obtainable measurements of changes in sample length and diameter.

Technician Notes:

Laboratory analysis by: D. O'Dowd
Data entered by: J. Falance
Checked by: J. Hines



Moisture Retention Data

Dew Point Potentiometer / Relative Humidity Box (Soil-Water Characteristic Curve)

Sample Number: CHR-071117-Clay (90%)

Initial sample bulk density (g/cm³): 1.54

Fraction of test sample used (<2.00mm fraction) (%): 99.70

Dry weight* of dew point potentiometer sample (g): 159.53

Tare weight, jar (g): 114.42

| | Date | Time | Weight* (g) | Water Potential (-cm water) | Moisture Content [†] (% vol) | |
|--------------------------|-----------|-------|----------------|--------------------------------|--|----|
| Dew point potentiometer: | 7-Sep-17 | 15:05 | 166.36 | 13563 | 22.95 | ## |
| | 5-Sep-17 | 10:20 | 164.47 | 49970 | 16.58 | ## |
| | 29-Aug-17 | 15:10 | 163.31 | 130126 | 12.69 | ## |
| | 21-Aug-17 | 16:35 | 162.28 | 440554 | 9.25 | ## |

Volume Adjusted Data¹

| | Water Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calc. Porosity (%) |
|--------------------------|-----------------------------------|--|--|---|-----------------------------------|
| Dew point potentiometer: | 13563 | 225.72 | +1.40% | 1.52 | 42.65 |
| | 49970 | 225.72 | +1.40% | 1.52 | 42.65 |
| | 130126 | 225.72 | +1.40% | 1.52 | 42.65 |
| | 440554 | 225.72 | +1.40% | 1.52 | 42.65 |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent the volume change measurements obtained after the last hanging column or pressure plate point. "----" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '---' denotes no volume change occurred.

* Weight including tares

[†] Adjusted for >2.00mm (#10 sieve) material not used in DPP/RH testing. Assumed moisture content of material >2.00mm is zero, and assumed density of water is 1.0 g/cm³.

Volume adjustments are applicable at this matric potential (see comment #1).

Laboratory analysis by: D. O'Dowd/A. Bland

Data entered by: J. Falance

Checked by: J. Hines



Moisture Retention Data

Dew Point Potentiometer / Relative Humidity Box (Soil-Water Characteristic Curve)

Sample Number: CHR-071117-Clay (90%)

Initial sample bulk density (g/cm³): 1.54

Fraction of test sample used (<2.00mm fraction) (%): 99.70

Dry weight* of relative humidity box sample (g): 63.17

Tare weight (g): 40.78

| | Date | Time | Weight* (g) | Water Potential (-cm water) | Moisture Content [†] (% vol) | |
|------------------------|----------|-------|----------------|--------------------------------|--|----|
| Relative humidity box: | 1-Sep-17 | 17:40 | 64.19 | 851293 | 6.91 | †† |

Volume Adjusted Data¹

| | Water Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calc. Porosity (%) |
|------------------------|-----------------------------------|--|--|---|-----------------------------------|
| Relative humidity box: | 851293 | 225.72 | +1.40% | 1.52 | 42.65 |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent the volume change measurements obtained after the last hanging column or pressure plate point. "----" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '----' denotes no volume change occurred.

* Weight including tares

[†] Adjusted for >2.00mm (#10 sieve) material not used in DPP/RH testing. Assumed moisture content of material >2.00mm is zero, and assumed density of water is 1.0 g/cm³.

†† Volume adjustments are applicable at this matric potential (see comment #1). Changes in volume, if applicable, are estimated based on obtainable measurements of changes in sample length and diameter.

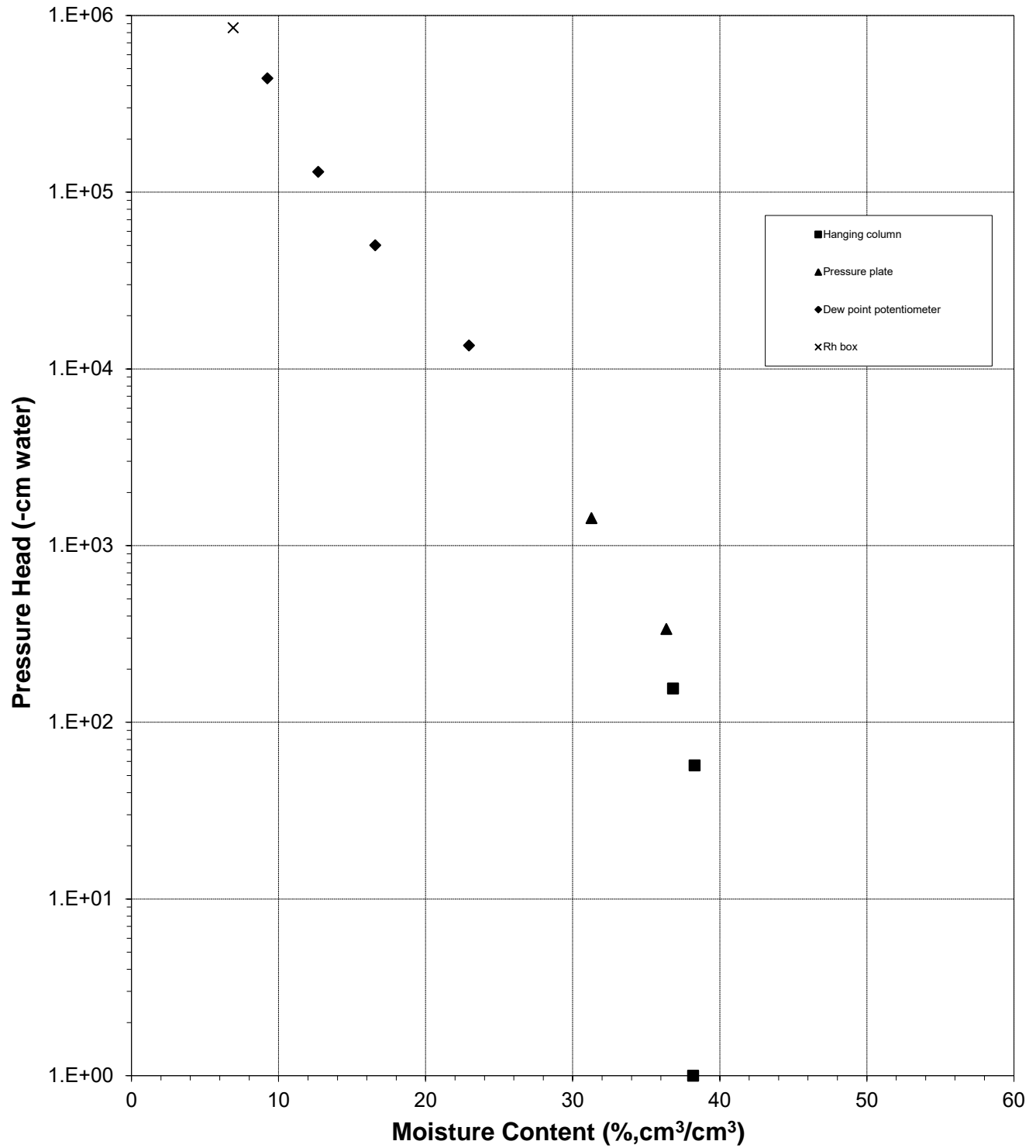
Laboratory analysis by: D. O'Dowd/A. Bland

Data entered by: J. Falance

Checked by: J. Hines



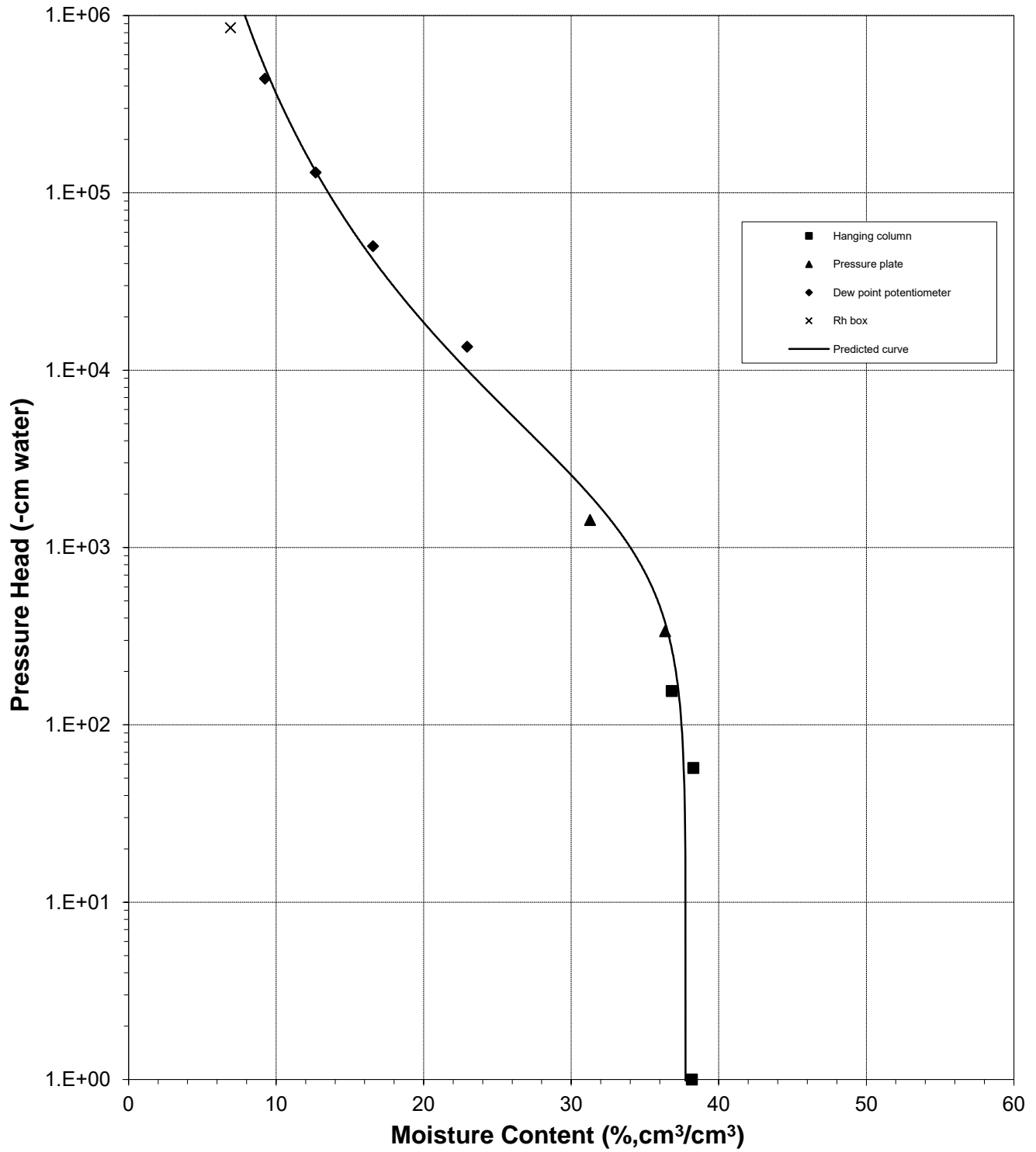
Water Retention Data Points
Sample Number: CHR-071117-Clay (90%)





Predicted Water Retention Curve and Data Points

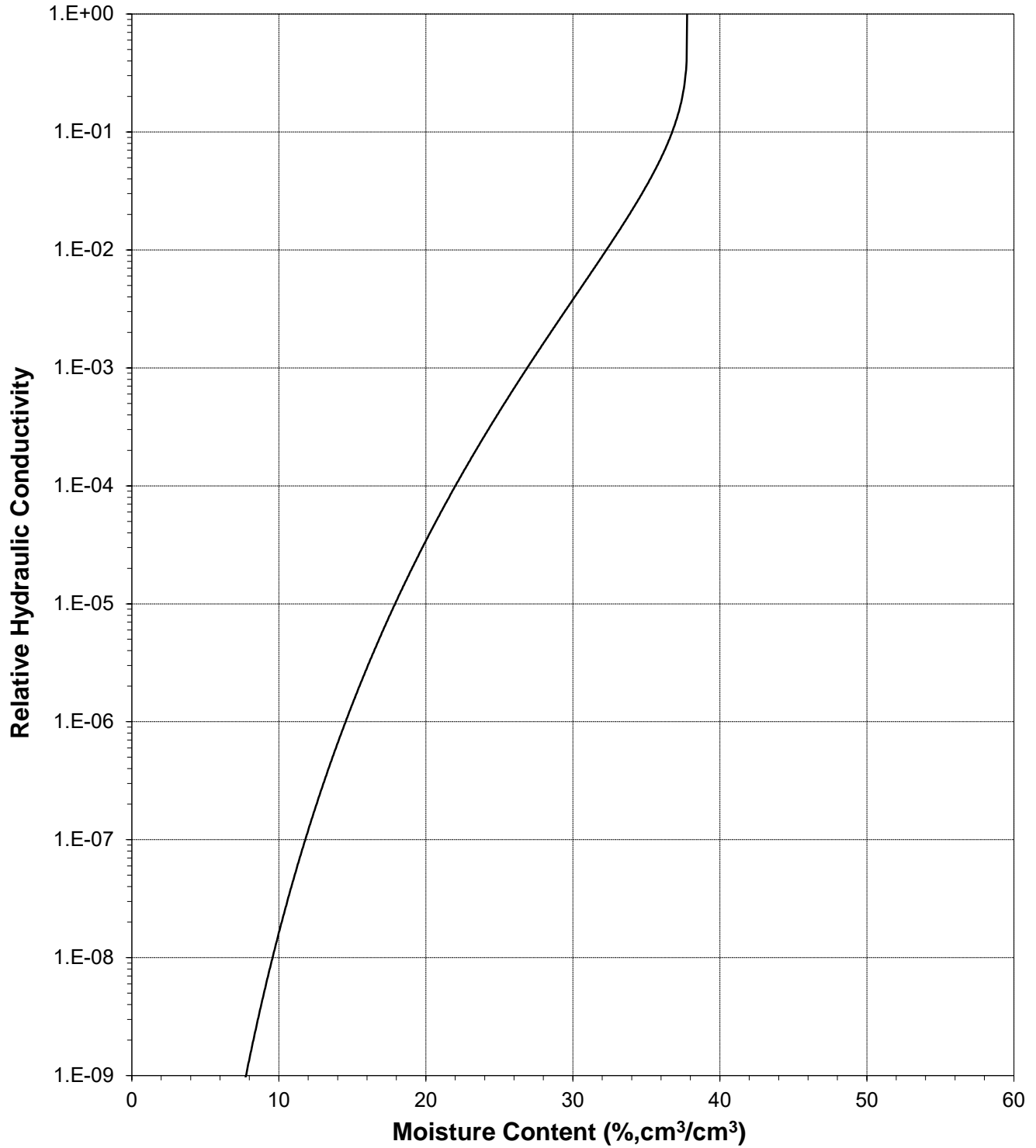
Sample Number: CHR-071117-Clay (90%)





Plot of Relative Hydraulic Conductivity vs Moisture Content

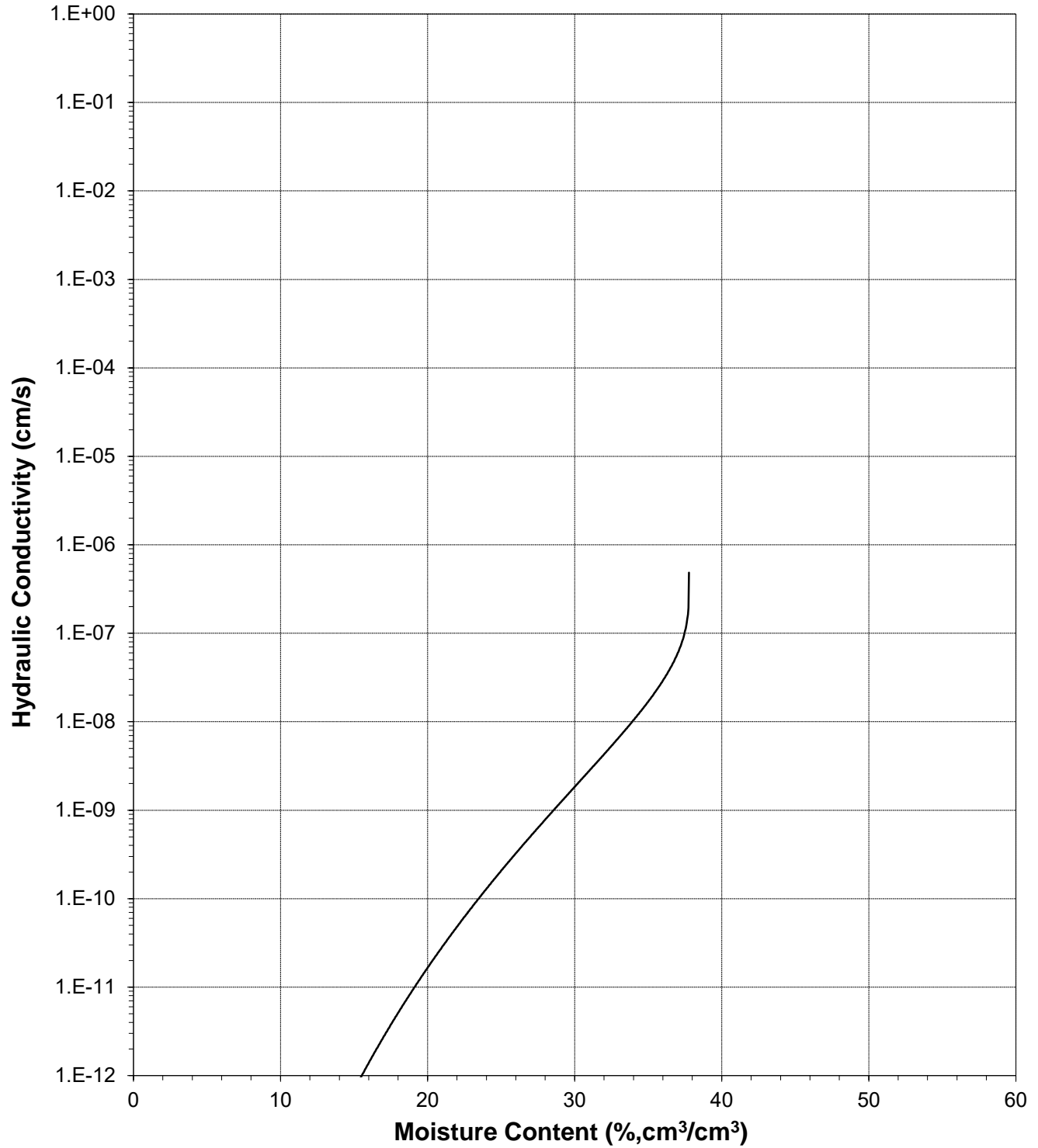
Sample Number: CHR-071117-Clay (90%)





Plot of Hydraulic Conductivity vs Moisture Content

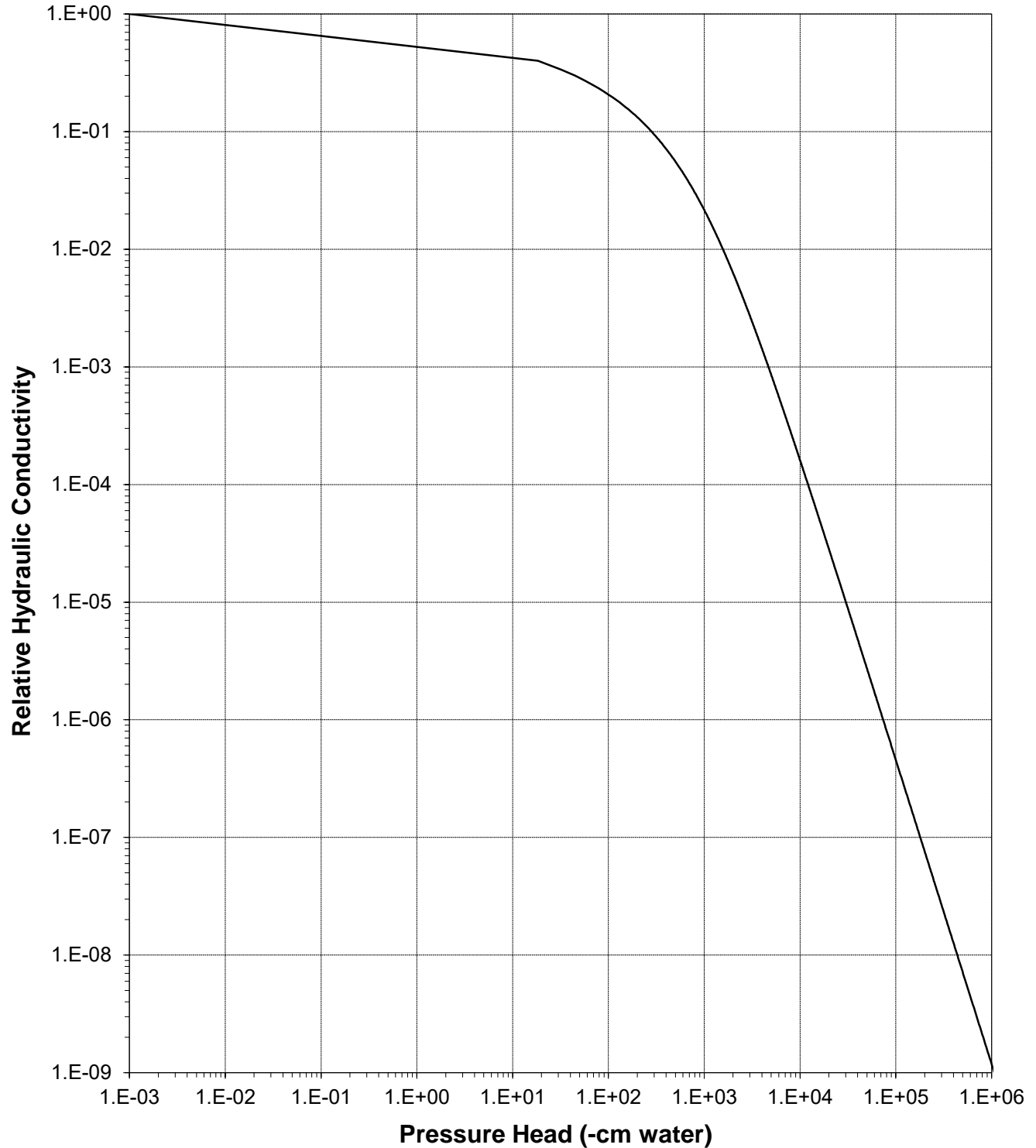
Sample Number: CHR-071117-Clay (90%)





Plot of Relative Hydraulic Conductivity vs Pressure Head

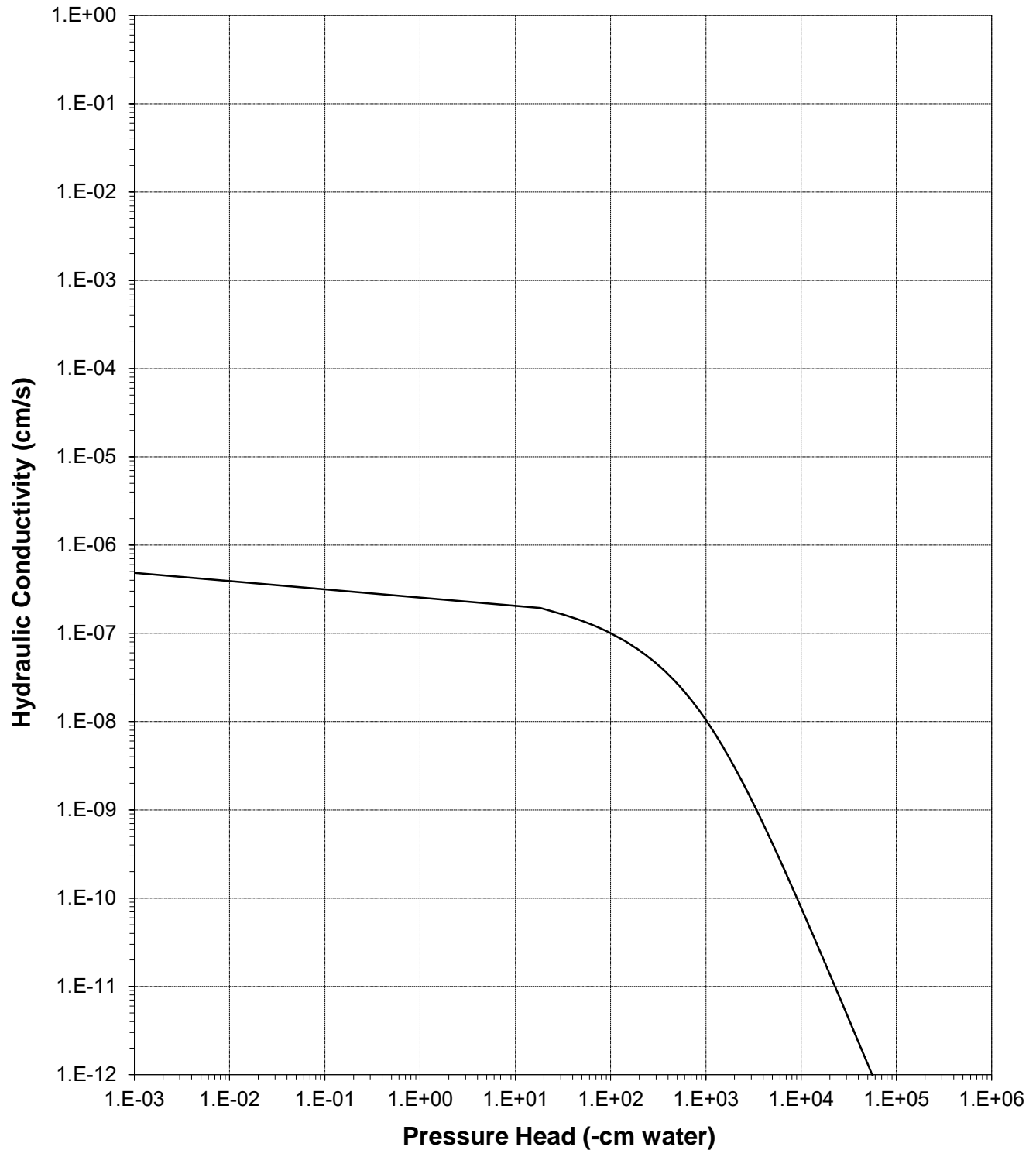
Sample Number: CHR-071117-Clay (90%)





Plot of Hydraulic Conductivity vs Pressure Head

Sample Number: CHR-071117-Clay (90%)





Oversize Correction Data Sheet

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay (90%)
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Split (3/4", 3/8", #4): #4

| | Coarse Fraction* | Fines Fraction** | Composite |
|---|------------------|------------------|-----------|
| Subsample Mass (g): | 0.19 | 99.81 | 100.00 |
| Mass Fraction (%): | 0.19 | 99.81 | 100.00 |
| <i>Initial Sample θ_i</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.54 | 1.54 |
| Calculated Porosity (% vol): | 0.00 | 41.84 | 41.80 |
| Volume of Solids (cm ³): | 0.07 | 37.66 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 27.10 | 27.10 |
| Total Volume (cm ³): | 0.07 | 64.76 | 64.84 |
| Volumetric Fraction (%): | 0.11 | 99.89 | 100.00 |
| Initial Moisture Content (% vol): | 0.00 | 27.13 | --- |
| <i>Saturated Sample θ_s</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.50 | 1.51 |
| Calculated Porosity (% vol): | 0.00 | 43.24 | 43.20 |
| Volume of Solids (cm ³): | 0.07 | 37.66 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 28.70 | 28.70 |
| Total Volume (cm ³): | 0.07 | 66.36 | 66.43 |
| Volumetric Fraction (%): | 0.11 | 99.89 | 100.00 |
| Saturated Moisture Content (% vol): | 0.00 | 37.78 | --- |
| <i>Residual Sample θ_r</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.52 | 1.52 |
| Calculated Porosity (% vol): | 0.00 | 42.65 | 42.60 |
| Volume of Solids (cm ³): | 0.07 | 37.66 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 28.00 | 28.00 |
| Total Volume (cm ³): | 0.07 | 65.67 | 65.74 |
| Volumetric Fraction (%): | 0.11 | 99.89 | 100.00 |
| Residual Moisture Content (% vol): | 0.00 | 0.00 | --- |
| Ksat (cm/sec): | NM | 4.8E-07 | --- |

* = Porosity and moisture content of coarse fraction assumed to be zero.

** = Volume adjusted, if applicable. See notes on Moisture Retention Data pages.

NM = Not measured

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

Laboratory analysis by: D. O'Dowd

Data entered by: J. Falance

Checked by: J. Hines



Moisture Retention Data
Hanging Column / Pressure Plate
 (Soil-Water Characteristic Curve)

Job Name: Stantec MWH
 Job Number: DB17.1177.00
 Sample Number: CHR-071117-Sand (91%)
 Project Name: NECR Jetty Borrow Soil
 PO Number: P30109-N

Dry wt. of sample (g): 360.15
 Tare wt., ring (g): 133.38
 Tare wt., screen & clamp (g): 24.13
 Initial sample volume (cm³): 222.72
 Initial dry bulk density (g/cm³): 1.62
 Assumed particle density (g/cm³): 2.65
 Initial calculated total porosity (%): 38.98

| | Date | Time | Weight* (g) | Matric Potential (-cm water) | Moisture Content [†] (% vol) |
|------------------------|-----------|-------|----------------|------------------------------------|---|
| <i>Hanging column:</i> | 15-Aug-17 | 8:15 | 593.47 | 0 | 34.04 |
| | 22-Aug-17 | 8:20 | 592.96 | 12.0 | 33.81 |
| | 29-Aug-17 | 16:40 | 592.09 | 35.0 | 33.42 |
| | 5-Sep-17 | 16:30 | 570.63 | 105.0 | 23.78 |
| <i>Pressure plate:</i> | 18-Sep-17 | 12:40 | 563.34 | 337 | 20.51 |

Volume Adjusted Data¹

| | Matric Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calculated Porosity (%) |
|------------------------|------------------------------------|--|--|---|---|
| <i>Hanging column:</i> | 0.0 | --- | --- | --- | --- |
| | 12.0 | --- | --- | --- | --- |
| | 35.0 | --- | --- | --- | --- |
| | 105.0 | --- | --- | --- | --- |
| <i>Pressure plate:</i> | 337 | --- | --- | --- | --- |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent each of the volume change measurements obtained after saturated hydraulic conductivity testing and throughout hanging column/pressure plate testing. "---" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '---' denotes no volume change occurred.

* Weight including tares

[†] Assumed density of water is 1.0 g/cm³

[‡] Volume adjustments are applicable at this matric potential (see comment #1). Changes in volume, if applicable, are estimated based on obtainable measurements of changes in sample length and diameter.

Technician Notes:

Laboratory analysis by: D. O'Dowd
Data entered by: J. Falance
Checked by: J. Hines



Moisture Retention Data

Dew Point Potentiometer / Relative Humidity Box (Soil-Water Characteristic Curve)

Sample Number: CHR-071117-Sand (91%)

Initial sample bulk density (g/cm³): 1.62

Fraction of test sample used (<2.00mm fraction) (%): 100.00

Dry weight* of dew point potentiometer sample (g): 169.66

Tare weight, jar (g): 114.22

| | Date | Time | Weight* (g) | Water Potential (-cm water) | Moisture Content [†] (% vol) |
|--------------------------|-----------|-------|----------------|--------------------------------|--|
| Dew point potentiometer: | 11-Sep-17 | 10:25 | 174.01 | 5303 | 12.68 |
| | 6-Sep-17 | 12:05 | 172.30 | 28350 | 7.71 |
| | 30-Aug-17 | 15:55 | 171.48 | 128189 | 5.32 |
| | 28-Aug-17 | 10:50 | 170.93 | 549468 | 3.69 |

Volume Adjusted Data¹

| | Water Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calc. Porosity (%) |
|--------------------------|-----------------------------------|--|--|---|-----------------------------------|
| Dew point potentiometer: | 5303 | --- | --- | --- | --- |
| | 28350 | --- | --- | --- | --- |
| | 128189 | --- | --- | --- | --- |
| | 549468 | --- | --- | --- | --- |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent the volume change measurements obtained after the last hanging column or pressure plate point. "---" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '-' denotes no volume change occurred.

* Weight including tares

[†] Adjusted for >2.00mm (#10 sieve) material not used in DPP/RH testing. Assumed moisture content of material >2.00mm is zero, and assumed density of water is 1.0 g/cm³.

[‡] Volume adjustments are applicable at this matric potential (see comment #1).

Laboratory analysis by: D. O'Dowd/A. Bland

Data entered by: J. Falance

Checked by: J. Hines



Moisture Retention Data

Dew Point Potentiometer / Relative Humidity Box (Soil-Water Characteristic Curve)

Sample Number: CHR-071117-Sand (91%)

Initial sample bulk density (g/cm³): 1.62

Fraction of test sample used (<2.00mm fraction) (%): 100.00

Dry weight* of relative humidity box sample (g): 100.64

Tare weight (g): 85.71

| | Date | Time | Weight* (g) | Water Potential (-cm water) | Moisture Content [†] (% vol) |
|------------------------|----------|-------|----------------|--------------------------------|--|
| Relative humidity box: | 1-Sep-17 | 17:40 | 100.93 | 851293 | 3.12 |

Volume Adjusted Data¹

| | Water Potential (-cm water) | Adjusted Volume (cm ³) | % Volume Change ² (%) | Adjusted Density (g/cm ³) | Adjusted Calc. Porosity (%) |
|------------------------|-----------------------------------|--|--|---|-----------------------------------|
| Relative humidity box: | 851293 | --- | --- | --- | --- |

Comments:

¹ Applicable if the sample experienced volume changes during testing. 'Volume Adjusted' values represent the volume change measurements obtained after the last hanging column or pressure plate point. "----" indicates no volume changes occurred.

² Represents percent volume change from original sample volume. A '+' denotes measured sample swelling, a '-' denotes measured sample settling, and '---' denotes no volume change occurred.

* Weight including tares

[†] Adjusted for >2.00mm (#10 sieve) material not used in DPP/RH testing. Assumed moisture content of material >2.00mm is zero, and assumed density of water is 1.0 g/cm³.

[‡] Volume adjustments are applicable at this matric potential (see comment #1). Changes in volume, if applicable, are estimated based on obtainable measurements of changes in sample length and diameter.

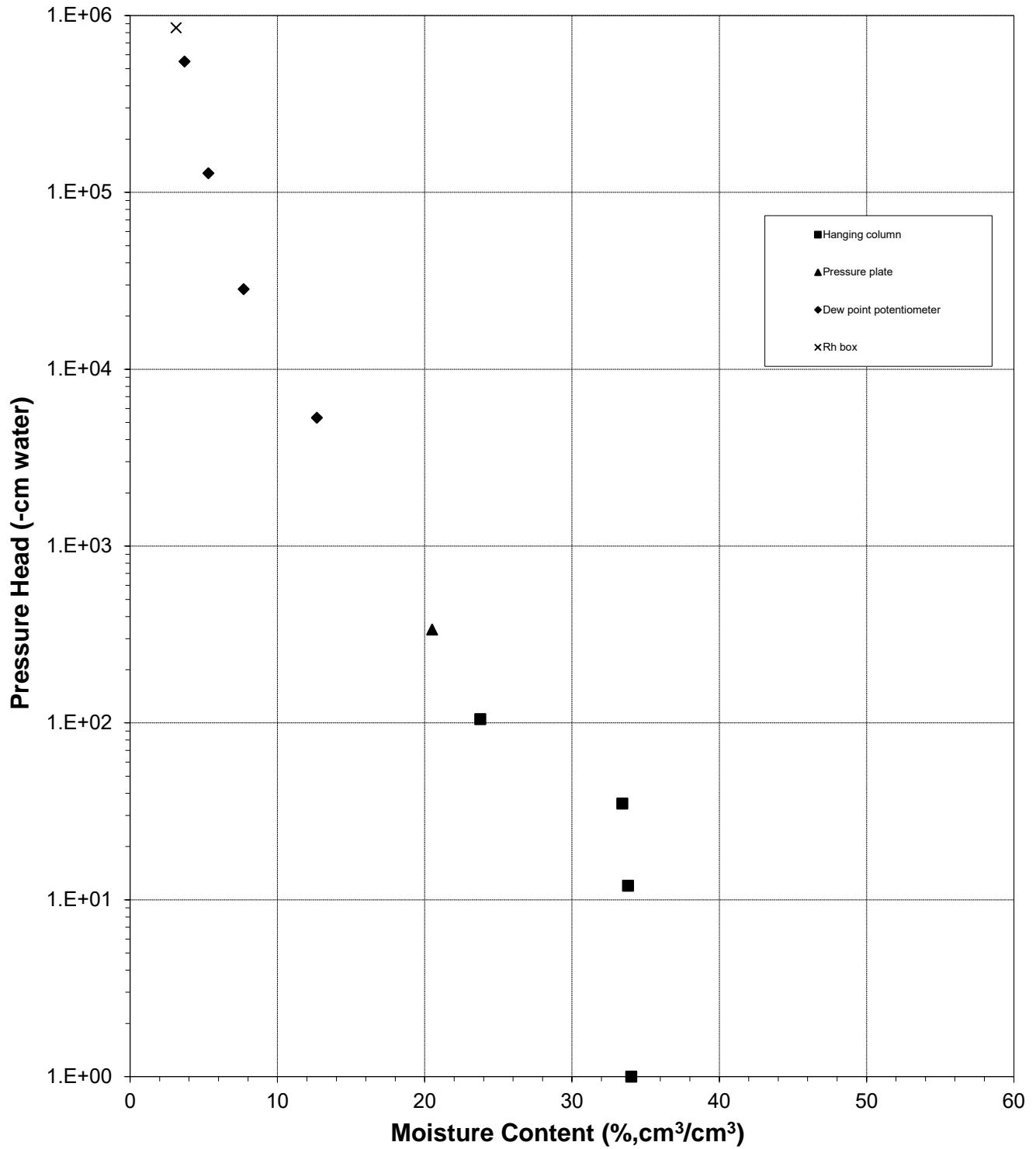
Laboratory analysis by: D. O'Dowd/A. Bland

Data entered by: J. Falance

Checked by: J. Hines



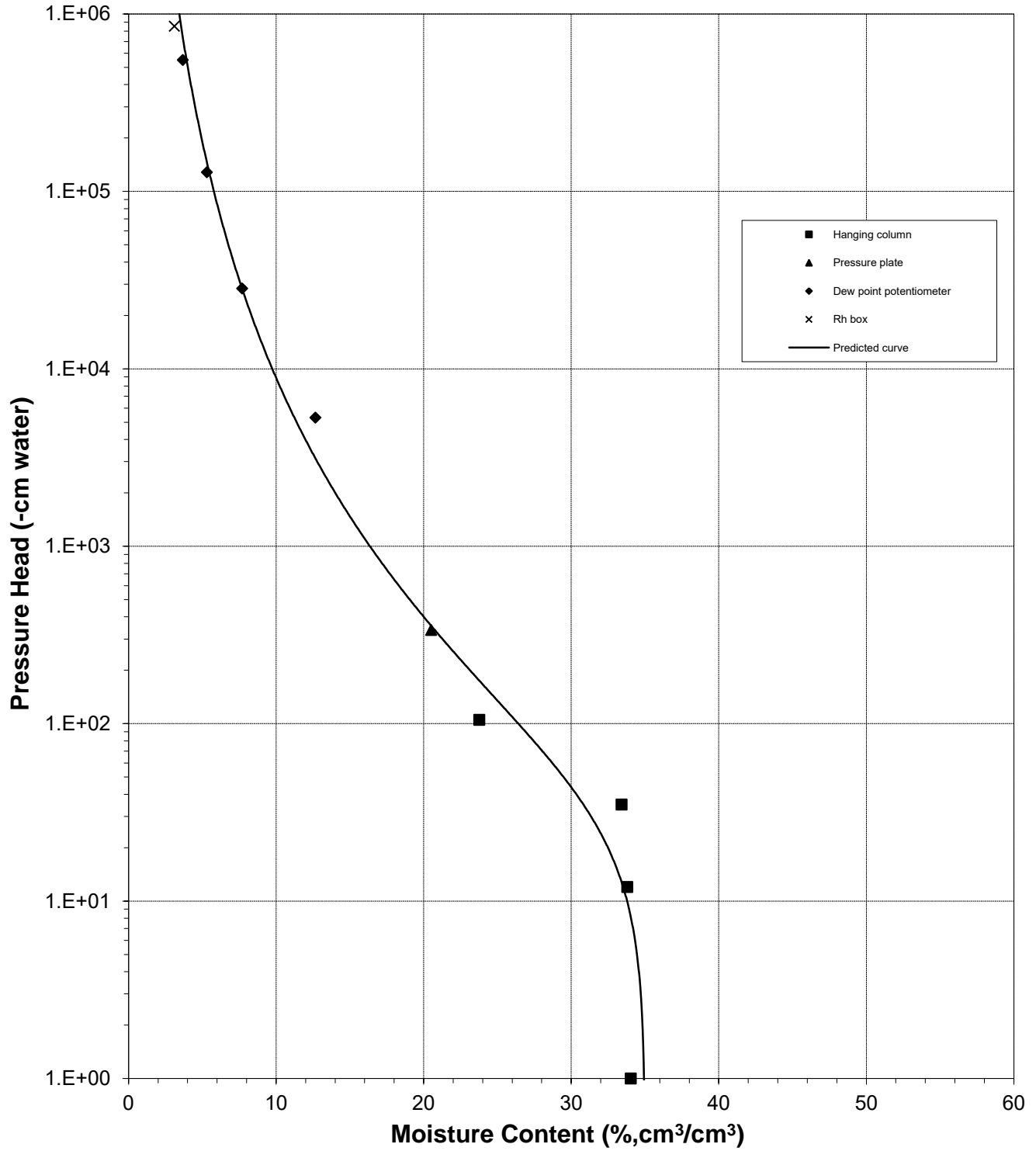
Water Retention Data Points
Sample Number: CHR-071117-Sand (91%)





Predicted Water Retention Curve and Data Points

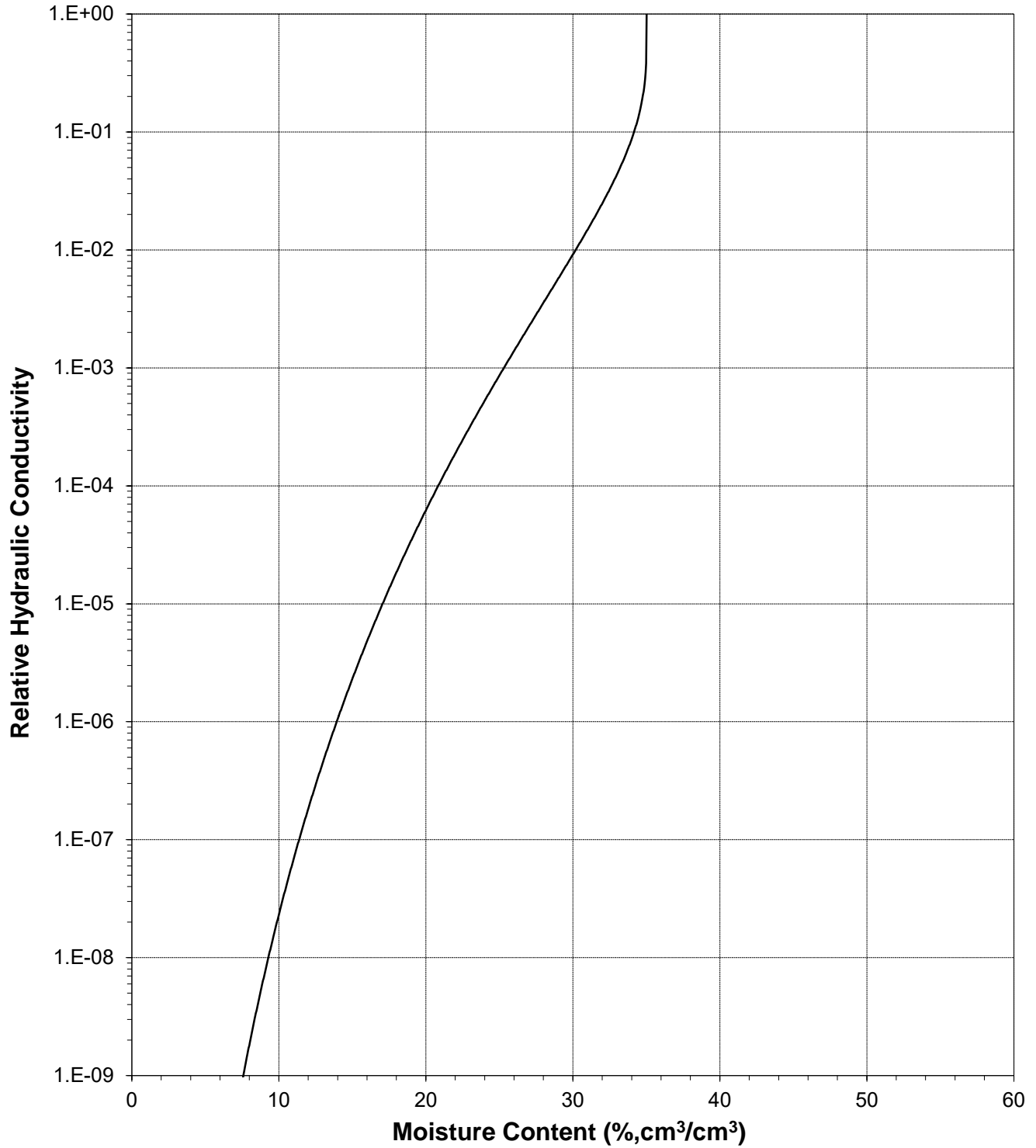
Sample Number: CHR-071117-Sand (91%)





Plot of Relative Hydraulic Conductivity vs Moisture Content

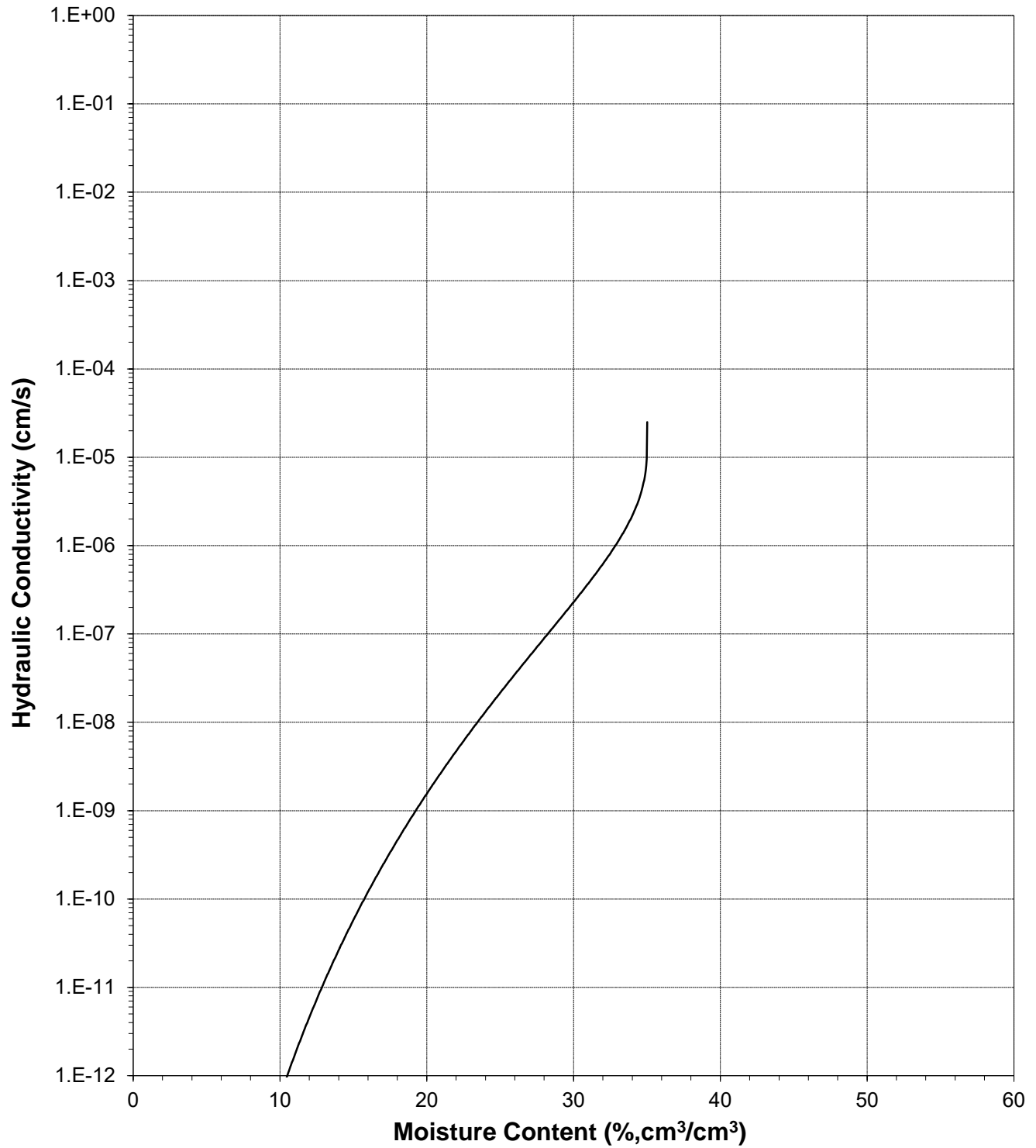
Sample Number: CHR-071117-Sand (91%)





Plot of Hydraulic Conductivity vs Moisture Content

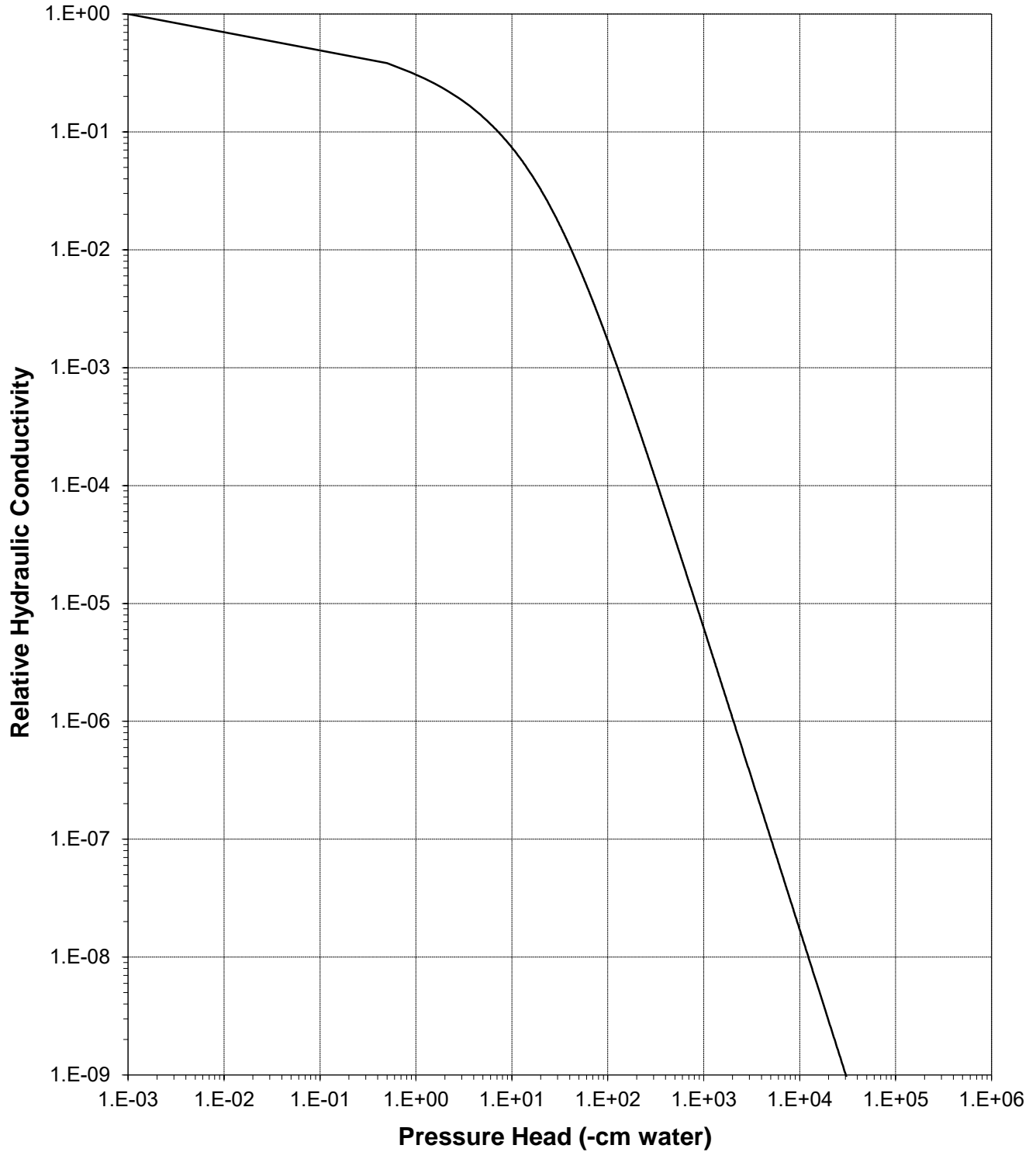
Sample Number: CHR-071117-Sand (91%)





Plot of Relative Hydraulic Conductivity vs Pressure Head

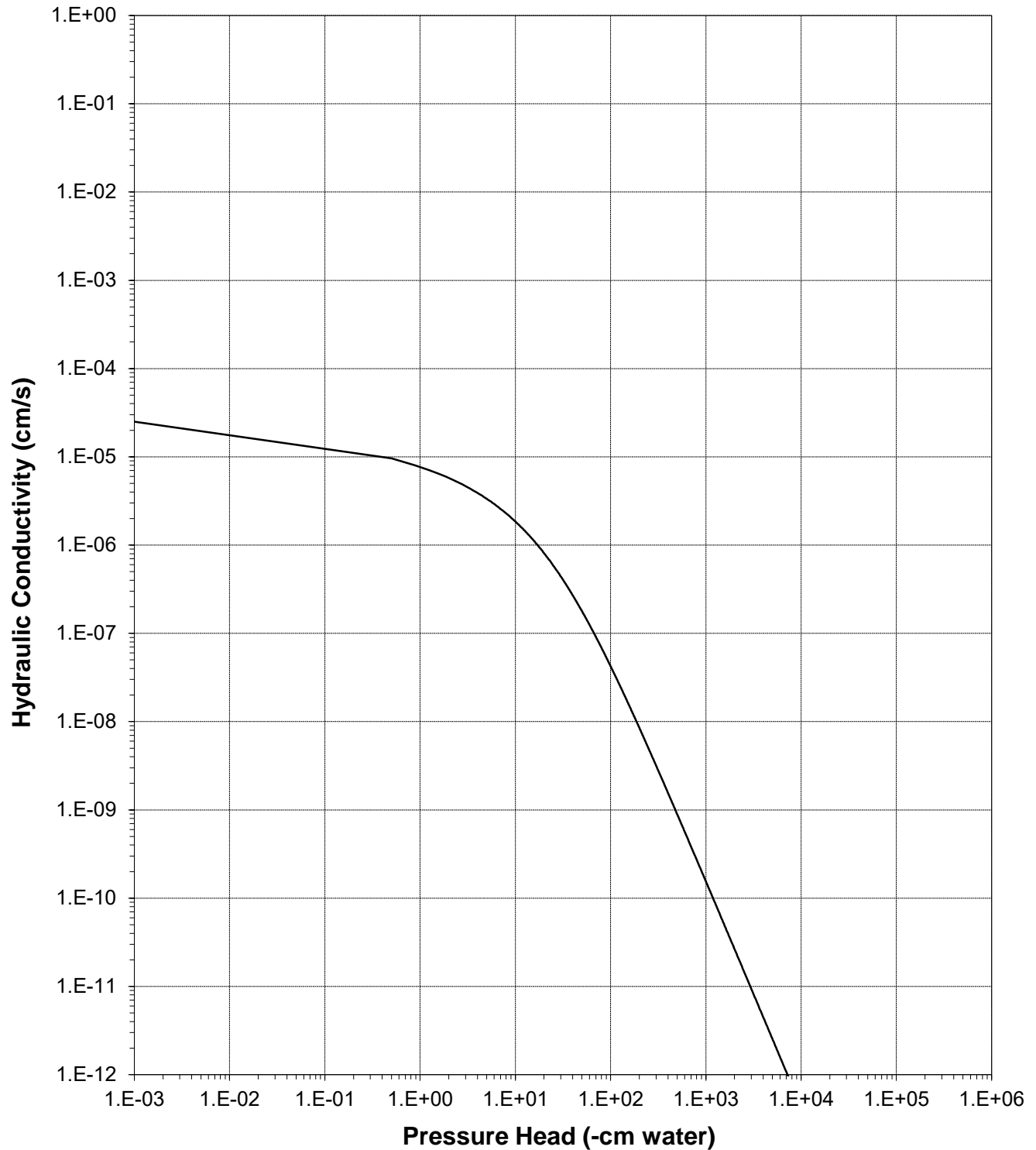
Sample Number: CHR-071117-Sand (91%)





Plot of Hydraulic Conductivity vs Pressure Head

Sample Number: CHR-071117-Sand (91%)





Oversize Correction Data Sheet

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Sand (91%)
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Split (3/4", 3/8", #4): #4

| | Coarse Fraction* | Fines Fraction** | Composite |
|---|------------------|------------------|-----------|
| Subsample Mass (g): | 0.00 | 100.00 | 100.00 |
| Mass Fraction (%): | 0.00 | 100.00 | 100.00 |
| <i>Initial Sample θ_i</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.62 | 1.62 |
| Calculated Porosity (% vol): | 0.00 | 38.98 | 38.98 |
| Volume of Solids (cm ³): | 0.00 | 37.74 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 24.10 | 24.10 |
| Total Volume (cm ³): | 0.00 | 61.84 | 61.84 |
| Volumetric Fraction (%): | 0.00 | 100.00 | 100.00 |
| Initial Moisture Content (% vol): | 0.00 | 23.87 | --- |
| <i>Saturated Sample θ_s</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.62 | 1.62 |
| Calculated Porosity (% vol): | 0.00 | 38.98 | 38.98 |
| Volume of Solids (cm ³): | 0.00 | 37.74 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 24.10 | 24.10 |
| Total Volume (cm ³): | 0.00 | 61.84 | 61.84 |
| Volumetric Fraction (%): | 0.00 | 100.00 | 100.00 |
| Saturated Moisture Content (% vol): | 0.00 | 35.02 | --- |
| <i>Residual Sample θ_r</i> | | | |
| Bulk Density (g/cm ³): | 2.65 | 1.62 | 1.62 |
| Calculated Porosity (% vol): | 0.00 | 38.98 | 38.98 |
| Volume of Solids (cm ³): | 0.00 | 37.74 | 37.74 |
| Volume of Voids (cm ³): | 0.00 | 24.10 | 24.10 |
| Total Volume (cm ³): | 0.00 | 61.84 | 61.84 |
| Volumetric Fraction (%): | 0.00 | 100.00 | 100.00 |
| Residual Moisture Content (% vol): | 0.00 | 0.00 | --- |
| Ksat (cm/sec): | NM | 2.5E-05 | --- |

* = Porosity and moisture content of coarse fraction assumed to be zero.

** = Volume adjusted, if applicable. See notes on Moisture Retention Data pages.

NM = Not measured

--- = Oversize correction is unnecessary since coarse fraction < 5% of composite mass

Laboratory analysis by: D. O'Dowd

Data entered by: J. Falance

Checked by: J. Hines

Particle Size Analysis



Summary of Particle Size Characteristics

| Sample Number | d ₁₀ (mm) | d ₅₀ (mm) | d ₆₀ (mm) | C _u | C _c | Method | ASTM Classification | USDA Classification | |
|-----------------|-------------------------|-------------------------|-------------------------|----------------|----------------|--------|---------------------------|------------------------|-------|
| CHR-071117-Clay | 0.00016 | 0.0068 | 0.015 | 94 | 0.50 | WS/H | Lean clay with sand (CL)s | Clay Loam | (Est) |
| CHR-071117-Sand | 8.8E-14 | 0.11 | 0.14 | 1.6E+12 | 3.1E+11 | WS/H | Silty sand (SM) | Sandy Loam | (Est) |

d₅₀ = Median particle diameter

Est = Reported values for d₁₀, C_u, C_c, and soil classification are estimates, since extrapolation was required to obtain the d₁₀ diameter

$$C_u = \frac{d_{60}}{d_{10}}$$

$$C_c = \frac{(d_{30})^2}{(d_{10})(d_{60})}$$

DS = Dry sieve

H = Hydrometer

WS = Wet sieve

[†] Greater than 10% of sample is coarse material



Percent Gravel, Sand, Silt and Clay*

| Sample Number | % Gravel (>4.75mm) | % Sand (<4.75mm, >0.075mm) | % Silt (<0.075mm, >0.002mm) | % Clay (<0.002mm) |
|-----------------|-----------------------|-------------------------------|--------------------------------|----------------------|
| CHR-071117-Clay | 0.2 | 20.0 | 44.1 | 35.8 |
| CHR-071117-Sand | 0.0 | 64.5 | 23.9 | 11.6 |

*USCS classification does not classify clay fraction based on particle size. USDA definition of clay (<0.002mm) used in this table.



Daniel B. Stephens & Associates, Inc.

Particle Size Analysis Wet Sieve Data (#10 Split)

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Test Date: 23-Aug-17

Initial Dry Weight of Sample (g): 199.63
Weight Passing #10 (g): 199.03
Weight Retained #10 (g): 0.60
Weight of Hydrometer Sample (g): 58.86
Calculated Weight of Sieve Sample (g): 59.04

Shape: Angular
Hardness: Soft

| Test Fraction | Sieve Number | Diameter (mm) | Wt. Retained | Cum Wt. Retained | Wt. Passing | % Passing |
|---------------|---------------------------------|---------------|--------------|------------------|-------------|-----------|
| +10 | 3" | 75 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 2" | 50 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 1.5" | 38.1 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 1" | 25 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 3/4" | 19.0 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 3/8" | 9.5 | 0.00 | 0.00 | 199.63 | 100.00 |
| | 4 | 4.75 | 0.37 | 0.37 | 199.26 | 99.81 |
| | 10 | 2.00 | 0.23 | 0.60 | 199.03 | 99.70 |
| -10 | (Based on calculated sieve wt.) | | | | | |
| | 20 | 0.85 | 0.29 | 0.47 | 58.57 | 99.21 |
| | 40 | 0.425 | 0.37 | 0.84 | 58.20 | 98.58 |
| | 60 | 0.250 | 0.61 | 1.45 | 57.59 | 97.55 |
| | 140 | 0.106 | 6.28 | 7.73 | 51.31 | 86.91 |
| | 200 | 0.075 | 4.17 | 11.90 | 47.14 | 79.85 |
| | dry pan | | 1.15 | 13.05 | 45.99 | |
| | wet pan | | | 45.99 | 0.00 | |

d₁₀ (mm): 0.00016 d₅₀ (mm): 0.0068
d₁₆ (mm): 0.00029 d₆₀ (mm): 0.015
d₃₀ (mm): 0.0011 d₈₄ (mm): 0.092

Median Particle Diameter --d₅₀ (mm): 0.0068
Uniformity Coefficient, Cu --[d₆₀/d₁₀] (mm): 94
Coefficient of Curvature, Cc --[(d₃₀)²/(d₁₀*d₆₀)] (mm): 0.50
Mean Particle Diameter --[(d₁₆+d₅₀+d₈₄)/3] (mm): 0.033

Note: Reported values for d₁₀, C_u, C_c, and soil classification are estimates, since extrapolation was required to obtain the d₁₀ diameter

Classification of fines: CL

ASTM Soil Classification: Lean clay with sand (CL)s
USDA Soil Classification: Clay Loam

Laboratory analysis by: J. Falance/E. Bastien
Data entered by: J. Falance
Checked by: J. Hines



Daniel B. Stephens & Associates, Inc.

Particle Size Analysis Hydrometer Data

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Test Date: 21-Aug-17
Start Time: 9:06

Type of Water Used: DISTILLED
Reaction with H_2O_2 : NA
Dispersant*: $(NaPO_3)_6$
Assumed particle density: 2.51

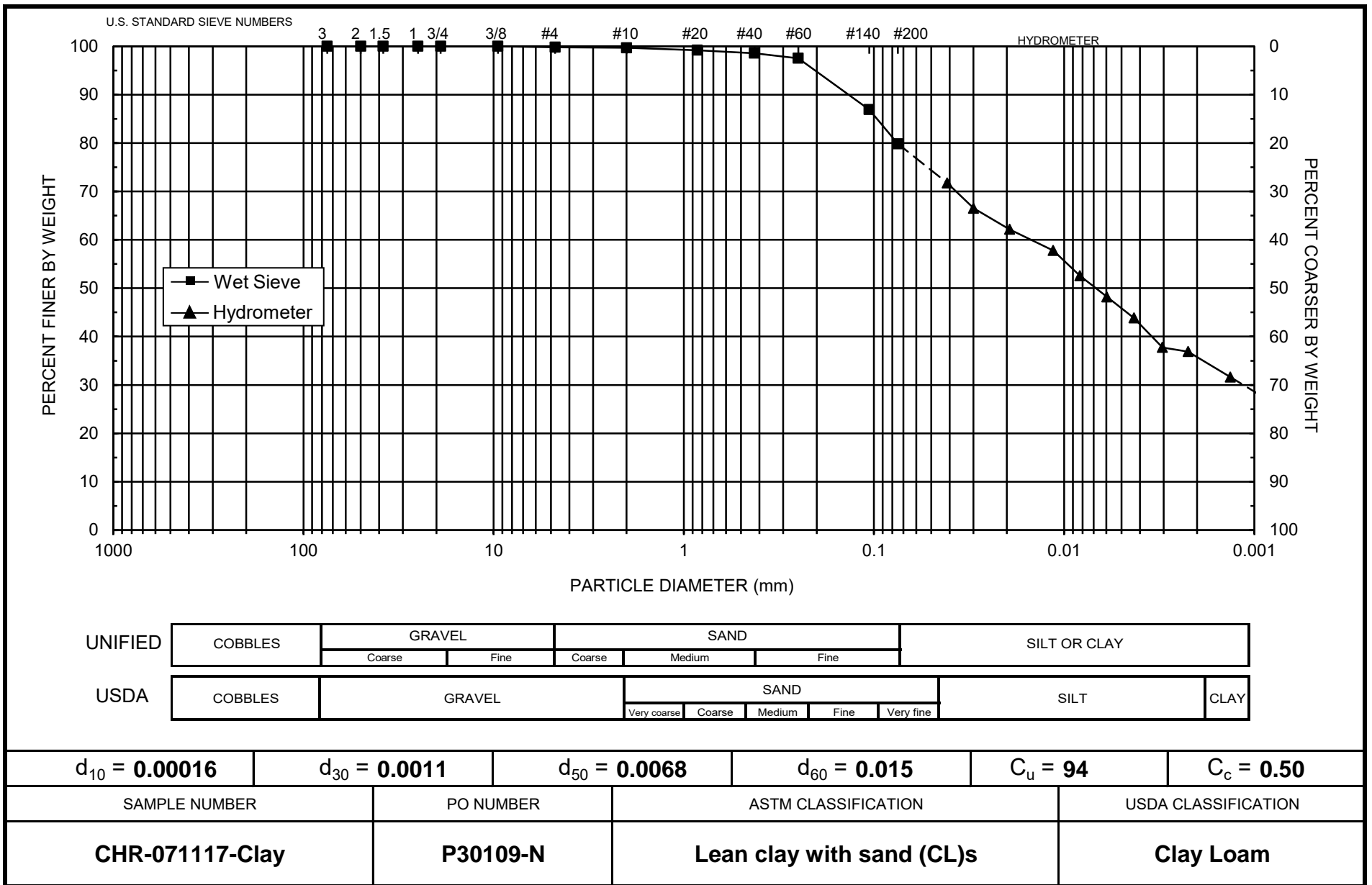
Initial Wt. (g): 58.86
Total Sample Wt. (g): 199.63
Wt. Passing #10 (g): 199.03

| Date | Time (min) | Temp (°C) | R (g/L) | R _L (g/L) | R _{corr} (g/L) | L (cm) | D (mm) | P (%) | % Finer |
|-----------|---------------|--------------|------------|-------------------------|----------------------------|-----------|-----------|----------|---------|
| 21-Aug-17 | 1 | 21.5 | 46.5 | 5.4 | 41.1 | 8.7 | 0.04119 | 72.0 | 71.8 |
| | 2 | 21.5 | 43.5 | 5.4 | 38.1 | 9.2 | 0.02994 | 66.7 | 66.5 |
| | 5 | 21.5 | 41.0 | 5.4 | 35.6 | 9.6 | 0.01935 | 62.3 | 62.2 |
| | 15 | 21.5 | 38.5 | 5.4 | 33.1 | 10.0 | 0.01141 | 58.0 | 57.8 |
| | 30 | 21.5 | 35.5 | 5.4 | 30.1 | 10.5 | 0.00826 | 52.7 | 52.6 |
| | 60 | 21.5 | 33.0 | 5.4 | 27.6 | 10.9 | 0.00596 | 48.4 | 48.2 |
| | 120 | 21.5 | 30.5 | 5.4 | 25.1 | 11.3 | 0.00429 | 44.0 | 43.8 |
| | 250 | 21.6 | 27.0 | 5.4 | 21.6 | 11.9 | 0.00304 | 37.9 | 37.8 |
| | 467 | 21.8 | 26.5 | 5.4 | 21.2 | 12.0 | 0.00223 | 37.0 | 36.9 |
| 22-Aug-17 | 1358 | 21.5 | 23.5 | 5.4 | 18.1 | 12.4 | 0.00134 | 31.7 | 31.6 |

Comments:

* Dispersion device: mechanically operated stirring device

Laboratory analysis by: D. Davis
Data entered by: J. Falance
Checked by: J. Hines



Note: Reported values for d_{10} , C_u , C_c , and ASTM classification are estimates, since extrapolation was required to obtain the d_{10} diameter

Daniel B. Stephens & Associates, Inc.





Particle Size Analysis **Wet Sieve Data (#10 Split)**

Job Name: Stantec MWH
 Job Number: DB17.1177.00
 Sample Number: CHR-071117-Sand
 Project Name: NECR Jetty Borrow Soil
 PO Number: P30109-N

Test Date: 23-Aug-17

Initial Dry Weight of Sample (g): 119.56
 Weight Passing #10 (g): 119.56
 Weight Retained #10 (g): 0.00
 Weight of Hydrometer Sample (g): 59.09
 Calculated Weight of Sieve Sample (g): 59.09

Shape: Angular
 Hardness: Soft

| Test Fraction | Sieve Number | Diameter (mm) | Wt. Retained | Cum Wt. Retained | Wt. Passing | % Passing |
|---------------|---------------------------------|---------------|--------------|------------------|-------------|-----------|
| +10 | 3" | 75 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 2" | 50 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 1.5" | 38.1 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 1" | 25 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 3/4" | 19.0 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 3/8" | 9.5 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 4 | 4.75 | 0.00 | 0.00 | 119.56 | 100.00 |
| | 10 | 2.00 | 0.00 | 0.00 | 119.56 | 100.00 |
| -10 | (Based on calculated sieve wt.) | | | | | |
| | 20 | 0.85 | 0.12 | 0.12 | 58.97 | 99.80 |
| | 40 | 0.425 | 0.68 | 0.80 | 58.29 | 98.65 |
| | 60 | 0.250 | 4.97 | 5.77 | 53.32 | 90.24 |
| | 140 | 0.106 | 25.89 | 31.66 | 27.43 | 46.42 |
| | 200 | 0.075 | 6.48 | 38.14 | 20.95 | 35.45 |
| | dry pan | | 1.83 | 39.97 | 19.12 | |
| | wet pan | | | 19.12 | 0.00 | |

d₁₀ (mm): 8.8E-14 d₅₀ (mm): 0.11
 d₁₆ (mm): 0.0099 d₆₀ (mm): 0.14
 d₃₀ (mm): 0.062 d₈₄ (mm): 0.22

Median Particle Diameter --d₅₀ (mm): 0.11
 Uniformity Coefficient, Cu --[d₆₀/d₁₀] (mm): 1.6E+12
 Coefficient of Curvature, Cc --[(d₃₀)²/(d₁₀*d₆₀)] (mm): 3.1E+11
 Mean Particle Diameter --[(d₁₆+d₅₀+d₈₄)/3] (mm): 0.11

Note: Reported values for d₁₀, C_u, C_c, and soil classification are estimates, since extrapolation was required to obtain the d₁₀ diameter

Classification of fines (visual method): ML

ASTM Soil Classification: Silty sand (SM)
 USDA Soil Classification: Sandy Loam

Laboratory analysis by: J. Falance
 Data entered by: J. Falance
 Checked by: J. Hines



Daniel B. Stephens & Associates, Inc.

Particle Size Analysis Hydrometer Data

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Sand
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Test Date: 21-Aug-17
Start Time: 9:00

Type of Water Used: DISTILLED
Reaction with H_2O_2 : NA
Dispersant*: $(NaPO_3)_6$
Assumed particle density: 2.51

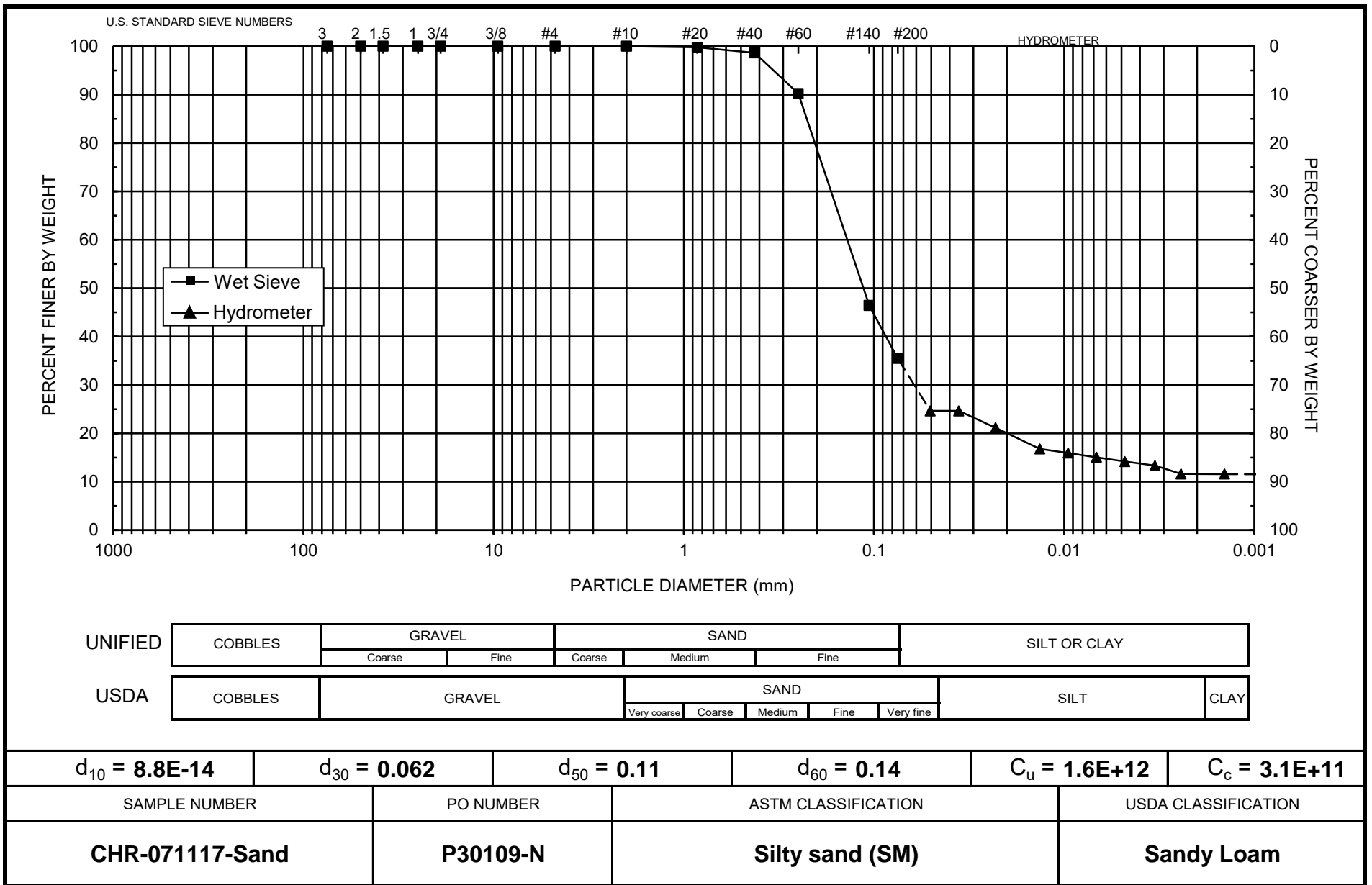
Initial Wt. (g): 59.09
Total Sample Wt. (g): 119.56
Wt. Passing #10 (g): 119.56

| Date | Time (min) | Temp (°C) | R (g/L) | R _L (g/L) | R _{corr} (g/L) | L (cm) | D (mm) | P (%) | % Finer |
|-----------|---------------|--------------|------------|-------------------------|----------------------------|-----------|-----------|----------|---------|
| 21-Aug-17 | 1 | 21.5 | 19.5 | 5.4 | 14.1 | 13.1 | 0.05062 | 24.6 | 24.6 |
| | 2 | 21.5 | 19.5 | 5.4 | 14.1 | 13.1 | 0.03579 | 24.6 | 24.6 |
| | 5 | 21.5 | 17.5 | 5.4 | 12.1 | 13.4 | 0.02292 | 21.1 | 21.1 |
| | 15 | 21.5 | 15.0 | 5.4 | 9.6 | 13.8 | 0.01343 | 16.8 | 16.8 |
| | 30 | 21.5 | 14.5 | 5.4 | 9.1 | 13.9 | 0.00953 | 15.9 | 15.9 |
| | 60 | 21.5 | 14.0 | 5.4 | 8.6 | 14.0 | 0.00676 | 15.0 | 15.0 |
| | 120 | 21.5 | 13.5 | 5.4 | 8.1 | 14.1 | 0.00479 | 14.2 | 14.2 |
| | 250 | 21.6 | 13.0 | 5.4 | 7.6 | 14.2 | 0.00332 | 13.3 | 13.3 |
| | 472 | 21.8 | 12.0 | 5.4 | 6.7 | 14.3 | 0.00243 | 11.6 | 11.6 |
| 22-Aug-17 | 1363 | 21.5 | 12.0 | 5.4 | 6.6 | 14.3 | 0.00143 | 11.6 | 11.6 |

Comments:

* Dispersion device: mechanically operated stirring device

Laboratory analysis by: D. Davis
Data entered by: J. Falance
Checked by: J. Hines



Note: Reported values for d_{10} , C_u , C_c , and ASTM classification are estimates, since extrapolation was required to obtain the d_{10} diameter

Daniel B. Stephens & Associates, Inc.



Atterberg Limits/ Identification of Fines



Summary of Atterberg Tests

| Sample Number | Liquid Limit | Plastic Limit | Plasticity Index | Classification |
|-----------------|--------------|---------------|------------------|----------------|
| CHR-071117-Clay | 43 | 19 | 24 | CL |
| CHR-071117-Sand | --- | --- | --- | ML |

--- = Soil requires visual-manual classification due to non-plasticity



Atterberg Limits

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Clay
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N
Test Date: 17-Aug-17

Liquid Limit

| | Trial 1 | Trial 2 | Trial 3 |
|---------------------------------------|---------|---------|---------|
| Number of drops: | 35 | 25 | 17 |
| Pan number: | LL1 | LL2 | LL3 |
| Weight of pan plus moist soil (g): | 123.94 | 127.24 | 127.73 |
| Weight of pan plus dry soil (g) | 120.73 | 123.40 | 124.19 |
| Weight of pan (g): | 113.17 | 114.47 | 116.10 |
| Gravimetric moisture content (% g/g): | 42.46 | 43.00 | 43.76 |
| Liquid Limit: | 43 | | |

Plastic Limit

| | Trial 1 | Trial 2 |
|---------------------------------------|---------|---------|
| Pan number: | PL1 | PL2 |
| Weight of pan plus moist soil (g): | 121.88 | 120.07 |
| Weight of pan plus dry soil (g) | 120.73 | 118.90 |
| Weight of pan (g): | 114.51 | 112.67 |
| Gravimetric moisture content (% g/g): | 18.49 | 18.78 |
| Plastic Limit: | 19 | |

Results

Percent of Sample Retained on #40 Sieve: See Sieve

Liquid Limit: 43
Plastic Limit: 19
Plasticity Index: 24
Classification: CL

Comments:

- = Soil requires visual-manual classification due to non-plasticity
- * = 1-point method requested by client

Laboratory analysis by: D. O'Dowd
Data entered by: D. O'Dowd
Checked by: J. Hines



Atterberg Limits

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Sand
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N
Test Date: 17-Aug-17

Liquid Limit

| | Trial 1 | Trial 2 | Trial 3 |
|---------------------------------------|---------|---------|---------|
| Number of drops: | | | |
| Pan number: | | | |
| Weight of pan plus moist soil (g): | | | |
| Weight of pan plus dry soil (g) | | | |
| Weight of pan (g): | | | |
| Gravimetric moisture content (% g/g): | --- | --- | --- |
| Liquid Limit: | --- | | |

Plastic Limit

| | Trial 1 | Trial 2 |
|---------------------------------------|---------|---------|
| Pan number: | | |
| Weight of pan plus moist soil (g): | | |
| Weight of pan plus dry soil (g) | | |
| Weight of pan (g): | | |
| Gravimetric moisture content (% g/g): | --- | --- |
| Plastic Limit: | --- | |

Results

Percent of Sample Retained on #40 Sieve: See Sieve

Liquid Limit: ---
Plastic Limit: ---
Plasticity Index: ---
Classification (Visual Method): ML

Comments:

- = Soil requires visual-manual classification due to non-plasticity
- * = 1-point method requested by client

Laboratory analysis by: D. O'Dowd
Data entered by: D. O'Dowd
Checked by: J. Hines



Data for Description and Identification of Fines (Visual-Manual Procedure)

Job Name: Stantec MWH
Job Number: DB17.1177.00
Sample Number: CHR-071117-Sand
Project Name: NECR Jetty Borrow Soil
PO Number: P30109-N

Test Date: 17-Aug-17

Visual-manual classification of material passing the #40 sieve in lieu of
Atterberg analysis due to non-plasticity:

Descriptive Information:

Color of Moist Sample: Dark Brown (10YR 3/3)
Odor: None
Moisture Condition: Moist
HCl Reaction: Weak

Preliminary Identification:

Dry Strength: None
Dilatency: Rapid
Toughness: Low
Plasticity: Non-plastic

Identification of Inorganic Fine Grained Soils:

Silt (ML)

Laboratory analysis by: D. O'Dowd
Data entered by: D. O'Dowd
Checked by: J. Hines

Laboratory Tests and Methods



Tests and Methods

| | |
|--|---|
| Dry Bulk Density: | ASTM D7263 |
| Moisture Content: | ASTM D7263, ASTM D2216 |
| Calculated Porosity: | ASTM D7263 |
| Saturated Hydraulic Conductivity: Falling Head Rising Tail: (Flexible Wall) | ASTM D5084 |
| Hanging Column Method: | ASTM D6836 (modified apparatus) |
| Pressure Plate Method: | ASTM D6836 (modified apparatus) |
| Water Potential (Dewpoint Potentiometer) Method: | ASTM D6836 |
| Relative Humidity (Box) Method: | Campbell, G. and G. Gee. 1986. Water Potential: Miscellaneous Methods. Chp. 25, pp. 631-632, in A. Klute (ed.), Methods of Soil Analysis. Part 1. American Society of Agronomy, Madison, WI; Karathanasis & Hajek. 1982. Quantitative Evaluation of Water Adsorption on Soil Clays. SSA Journal 46:1321-1325 |
| Moisture Retention Characteristics & Calculated Unsaturated Hydraulic Conductivity: | ASTM D6836; van Genuchten, M.T. 1980. A closed-form equation for predicting the hydraulic conductivity of unsaturated soils. SSSAJ 44:892-898; van Genuchten, M.T., F.J. Leij, and S.R. Yates. 1991. The RETC code for quantifying the hydraulic functions of unsaturated soils. Robert S. Kerr Environmental Research Laboratory, Office of Research and Development, U.S. Environmental Protection Agency, Ada, Oklahoma. EPA/600/2091/065. December 1991 |
| Particle Size Analysis: | ASTM D7928, ASTM D6913 |
| USCS (ASTM) Classification: | ASTM D7928, ASTM D6913, ASTM D2487 |
| USDA Classification: | ASTM D7928, ASTM D6913, USDA Soil Textural Triangle |
| Atterberg Limits: | ASTM D4318 |
| Visual-Manual Description: | ASTM D2488 |

| SAMPLE LOCATION | SAMPLE DEPTH (FT) | FIELD CLASS | MOISTURE (%) | DRY DENSITY (pcf) |
|-----------------|----------------------|----------------|-----------------|----------------------|
| B-4 | 16.0-16.5 | -- | 10.4 | 77.6 |
| B-5 | 10.5-11.0 | -- | 5.2 | 82.9 |
| B-5 (TRIAXIAL) | TW 25.0-27.5 | -- | 21.2 | 98.8 |
| B-5 | 40.5-41.0 | -- | 22.7 | 99.6 |
| B-6 | 15.5-16.5 | -- | 10.7 | 93.0 |
| B-6 | 40.5-41.0 | -- | 17.9 | 97.0 |
| B-7 | TW 5.0-6.5 | -- | 6.9 | 94.3 |
| B-7 | 10.5-11.0 | -- | 7.0 | 99.7 |
| B-7 | 30.5-31.0 | -- | 16.7 | 102.5 |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 2937 & ASTM D 2216

| | | | |
|---------------------------------|------|-------------------------------------|--------|
| <i>Ninyo & Moore</i> | | MOISTURE - DENSITY TEST DATA | FIGURE |
| PROJECT NO. | DATE | | |
| 604667003 | 3/17 | | |

WEIGHT OF SAMPLE DISPERSED: 51.2
 PERCENT PASSING #10 SIEVE: 100.0

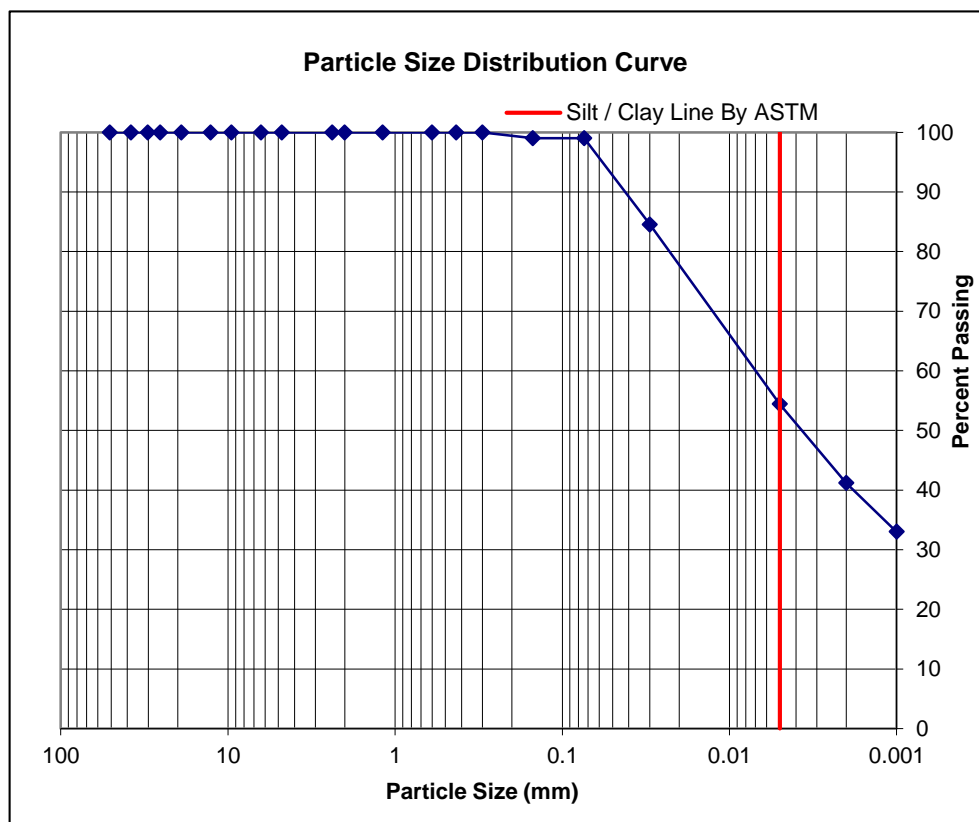
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0438 | 0.0283 | 0.0168 | 0.0123 | 0.0089 | 0.0046 | 0.0020 | 0.0014 |
| PERCENT SAMPLE TESTED | 87.9 | 84.0 | 78.2 | 70.3 | 65.5 | 52.8 | 41.0 | 37.1 |
| PERCENT TOTAL SAMPLE | 87.9 | 84.0 | 78.2 | 70.3 | 65.5 | 52.8 | 41.0 | 37.1 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|-------|-------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 99.0 | 99.4 | 99.6 | 99.6 | 99.8 | 100.0 | 100.0 |

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 100 | |
| # 30 | 100 | |
| # 40 | 100 | |
| # 50 | 100 | |
| # 100 | 99 | |
| # 200 | 99 | |
| 0.03 mm | 84.6 | |
| 0.005 mm | 54.4 | |
| 0.002 mm | 41.2 | |
| 0.001 mm | 33.0 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-5 | 40.5-41.0 | 49 | 20 | 29 | -- | -- | 0.007 | -- | -- | 99.0 | CL |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| | | | |
|--------------------------|------|--|--------|
| Ninyo & Moore | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 100.0

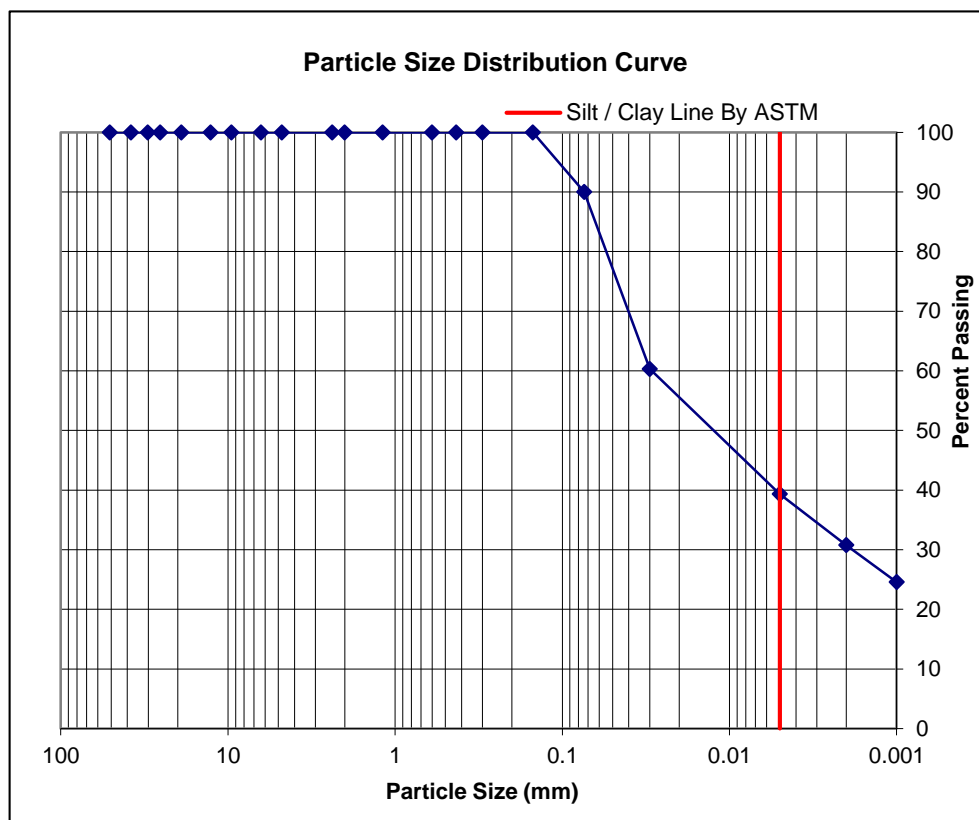
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0485 | 0.0315 | 0.0187 | 0.0134 | 0.0096 | 0.0048 | 0.0021 | 0.0015 |
| PERCENT SAMPLE TESTED | 68.0 | 61.0 | 54.0 | 50.0 | 46.0 | 39.0 | 31.0 | 28.0 |
| PERCENT TOTAL SAMPLE | 68.0 | 61.0 | 54.0 | 50.0 | 46.0 | 39.0 | 31.0 | 28.0 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|-------|-------|-------|-------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 89.8 | 99.6 | 99.8 | 100.0 | 100.0 | 100.0 | 100.0 |

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 100 | |
| # 30 | 100 | |
| # 40 | 100 | |
| # 50 | 100 | |
| # 100 | 100 | |
| # 200 | 90 | |
| 0.03 mm | 60.3 | |
| 0.005 mm | 39.3 | |
| 0.002 mm | 30.8 | |
| 0.001 mm | 24.6 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-6 | 15.0-16.0 | 42 | 17 | 25 | -- | 0.002 | 0.029 | -- | -- | 90.0 | CL |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| | | | |
|---------------------------------|------|--|--------|
| <i>Ninyo & Moore</i> | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 100.0

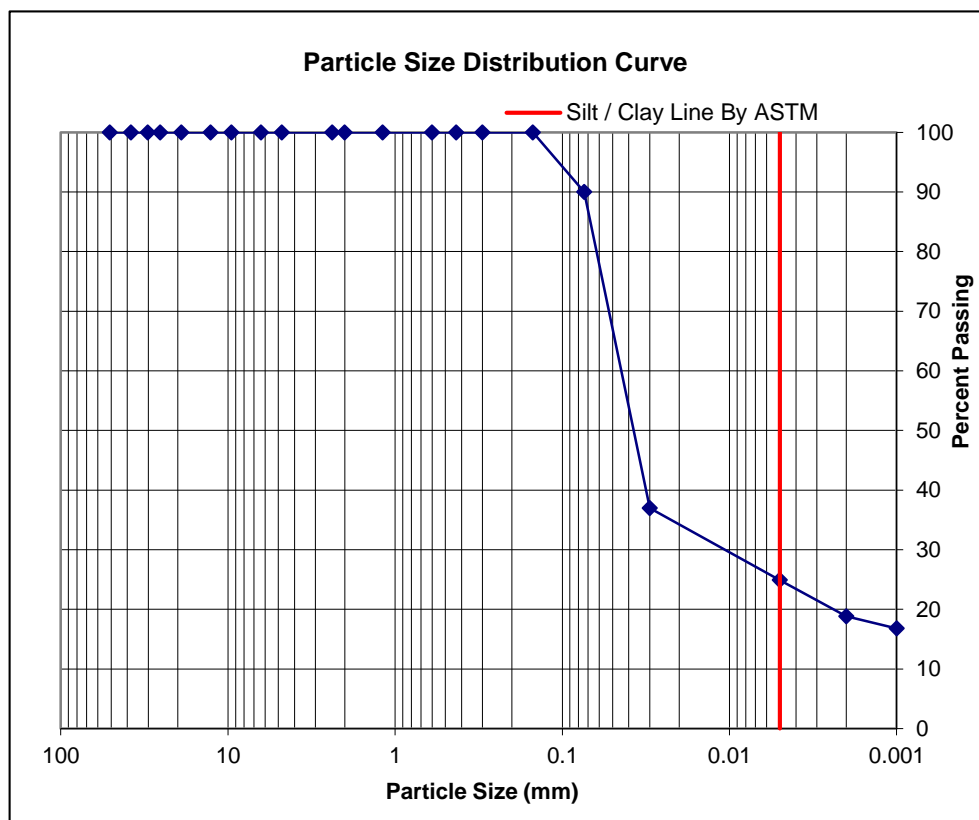
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0535 | 0.0343 | 0.0201 | 0.0143 | 0.0102 | 0.0051 | 0.0021 | 0.0015 |
| PERCENT SAMPLE TESTED | 42.0 | 38.0 | 34.0 | 32.0 | 29.0 | 25.0 | 19.0 | 18.0 |
| PERCENT TOTAL SAMPLE | 42.0 | 38.0 | 34.0 | 32.0 | 29.0 | 25.0 | 19.0 | 18.0 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|-------|-------|-------|-------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 89.8 | 99.6 | 99.8 | 100.0 | 100.0 | 100.0 | 100.0 |

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 100 | |
| # 30 | 100 | |
| # 40 | 100 | |
| # 50 | 100 | |
| # 100 | 100 | |
| # 200 | 90 | |
| 0.03 mm | 37.0 | |
| 0.005 mm | 24.9 | |
| 0.002 mm | 18.8 | |
| 0.001 mm | 16.8 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-7 | 10.5-11.0 | -- | -- | N Test | -- | 0.011 | 0.044 | -- | -- | 90.0 | |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| <i>Ninyo & Moore</i> | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
|--------------------------|------|---|--------|
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

WEIGHT OF SAMPLE DISPERSED: 49.8
 PERCENT PASSING #10 SIEVE: 100.0

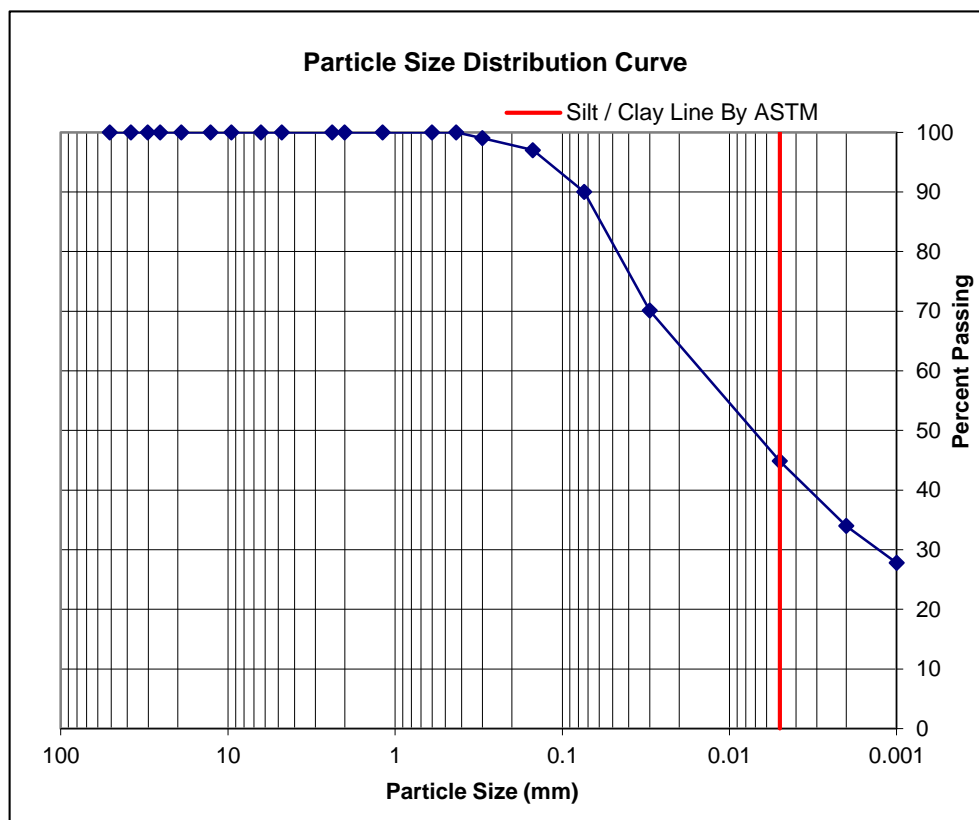
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0470 | 0.0304 | 0.0181 | 0.0131 | 0.0094 | 0.0048 | 0.0020 | 0.0015 |
| PERCENT SAMPLE TESTED | 75.3 | 70.3 | 63.2 | 57.2 | 53.2 | 44.2 | 34.1 | 31.1 |
| PERCENT TOTAL SAMPLE | 75.3 | 70.3 | 63.2 | 57.2 | 53.2 | 44.2 | 34.1 | 31.1 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|-------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 90.0 | 97.0 | 99.4 | 99.6 | 99.6 | 99.8 | 100.0 |

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 100 | |
| # 30 | 100 | |
| # 40 | 100 | |
| # 50 | 99 | |
| # 100 | 97 | |
| # 200 | 90 | |
| 0.03 mm | 70.1 | |
| 0.005 mm | 44.9 | |
| 0.002 mm | 34.0 | |
| 0.001 mm | 27.8 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | TP-1 | BUCKET | -- | -- | N Tested | -- | 0.001 | 0.015 | -- | -- | 90.0 | |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| | | | |
|---------------------------------|------|--|--------|
| <i>Ninyo & Moore</i> | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

WEIGHT OF SAMPLE DISPERSED: **54.5**
 PERCENT PASSING #10 SIEVE: **99.8**

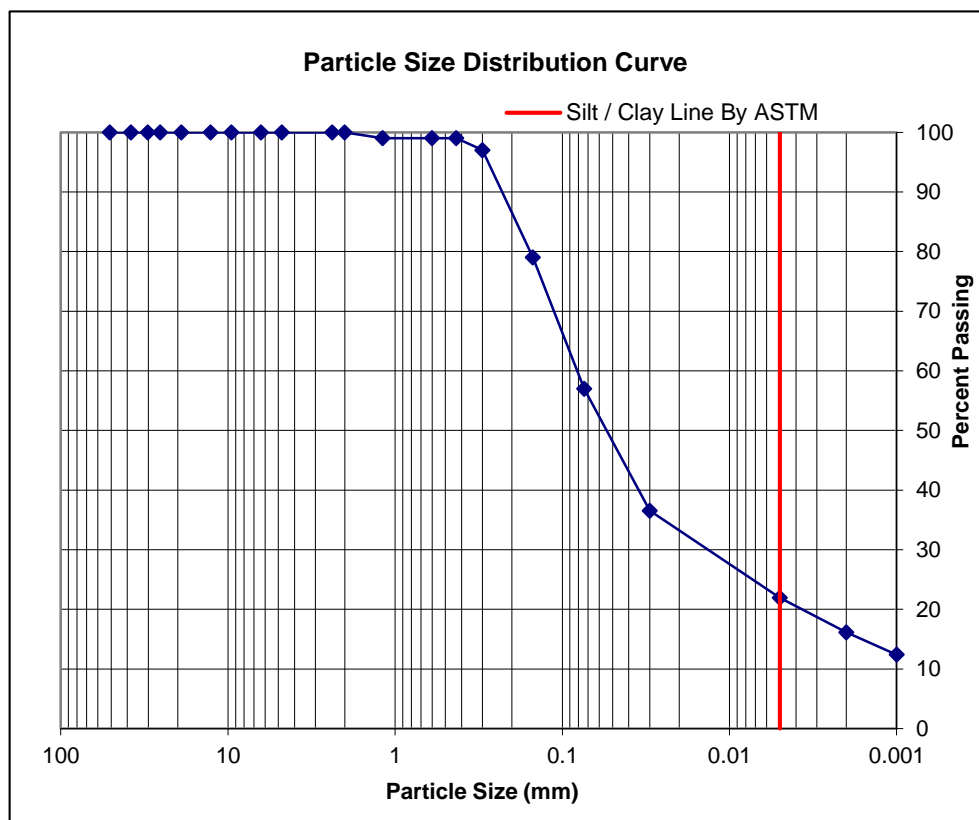
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0531 | 0.0339 | 0.0199 | 0.0142 | 0.0102 | 0.0051 | 0.0021 | 0.0015 |
| PERCENT SAMPLE TESTED | 40.4 | 37.6 | 33.0 | 31.2 | 27.5 | 22.0 | 16.5 | 14.7 |
| PERCENT TOTAL SAMPLE | 40.3 | 37.5 | 33.0 | 31.1 | 27.5 | 22.0 | 16.5 | 14.6 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 56.8 | 79.3 | 96.5 | 98.5 | 99.1 | 99.4 | 99.8 |

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 99 | |
| # 30 | 99 | |
| # 40 | 99 | |
| # 50 | 97 | |
| # 100 | 79 | |
| # 200 | 57 | |
| 0.03 mm | 36.5 | |
| 0.005 mm | 21.9 | |
| 0.002 mm | 16.1 | |
| 0.001 mm | 12.4 | |

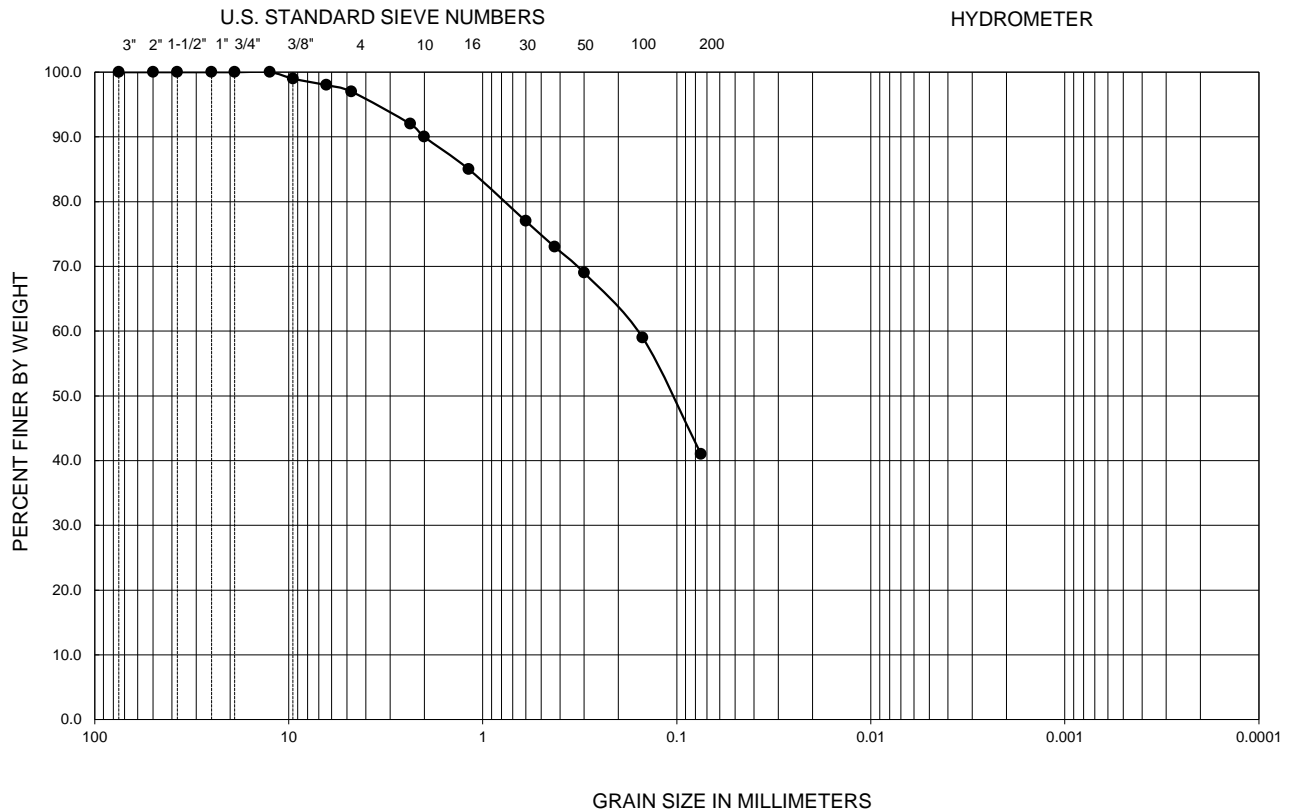


| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | TP-2 | BUCKET | -- | -- | N Tested | -- | 0.013 | 0.081 | -- | -- | 57.0 | |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| | | | |
|---------------------------------|------|--|--------|
| <i>Ninyo & Moore</i> | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

| GRAVEL | | SAND | | | FINES | |
|--------|------|--------|--------|------|-------|------|
| Coarse | Fine | Coarse | Medium | Fine | SILT | CLAY |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| ● | TP-3 | BUCKET | -- | -- | NT | -- | -- | 0.16 | -- | -- | 41.0 | |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM C136 AND C117

| | | | |
|---------------------------------|------|-------------------------------|--------|
| <i>Ninyo & Moore</i> | | GRADATION TEST RESULTS | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING | |
| 604667003 | 3/17 | PHOENIX, ARIZONA | |

WEIGHT OF SAMPLE DISPERSED: **51.8**
 PERCENT PASSING #10 SIEVE: **94.9**

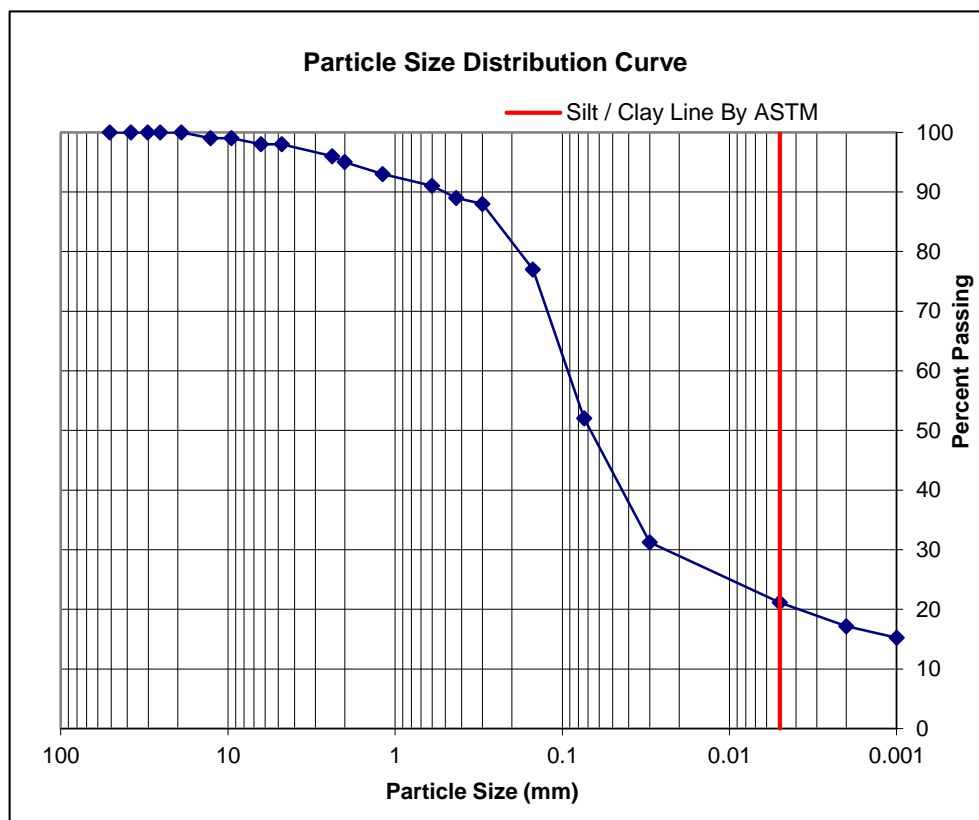
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0544 | 0.0347 | 0.0204 | 0.0145 | 0.0103 | 0.0051 | 0.0022 | 0.0015 |
| PERCENT SAMPLE TESTED | 35.7 | 32.8 | 27.0 | 27.0 | 24.1 | 21.2 | 17.4 | 16.4 |
| PERCENT TOTAL SAMPLE | 33.9 | 31.1 | 25.6 | 25.6 | 22.9 | 20.1 | 16.5 | 15.6 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 51.7 | 76.8 | 87.7 | 89.4 | 91.0 | 92.9 | 94.9 |

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 99 | |
| 3/8 IN | 99 | |
| 1/4 IN | 98 | |
| # 4 | 98 | |
| # 8 | 96 | |
| # 10 | 95 | |
| # 16 | 93 | |
| # 30 | 91 | |
| # 40 | 89 | |
| # 50 | 88 | |
| # 100 | 77 | |
| # 200 | 52 | |
| 0.03 mm | 31.2 | |
| 0.005 mm | 21.1 | |
| 0.002 mm | 17.2 | |
| 0.001 mm | 15.2 | |



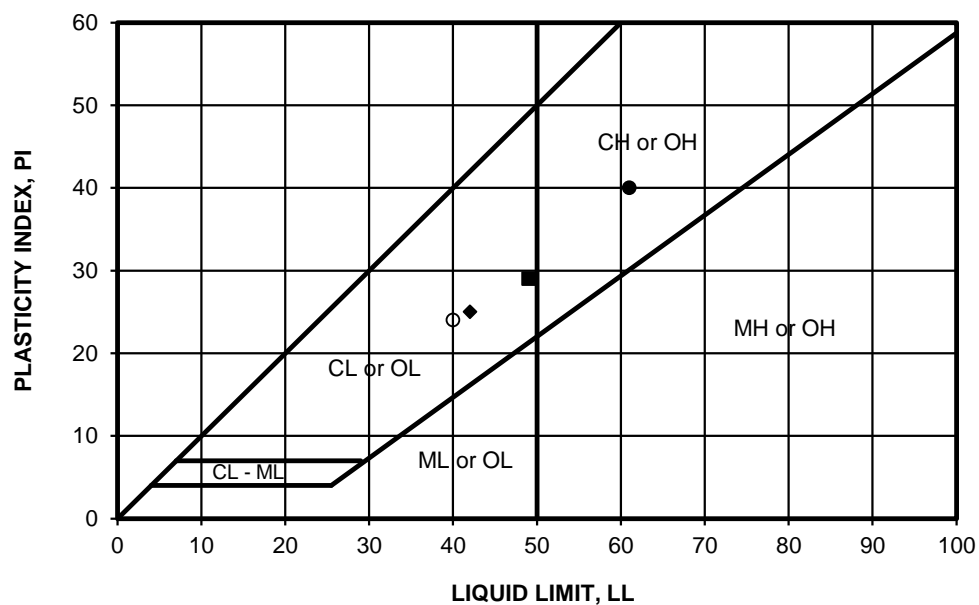
| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | TP-4 | BUCKET | -- | -- | N Tested | -- | 0.024 | 0.093 | -- | -- | 52.0 | |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| <i>Ninyo & Moore</i> | | PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422) | FIGURE |
|--------------------------|------|---|--------|
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

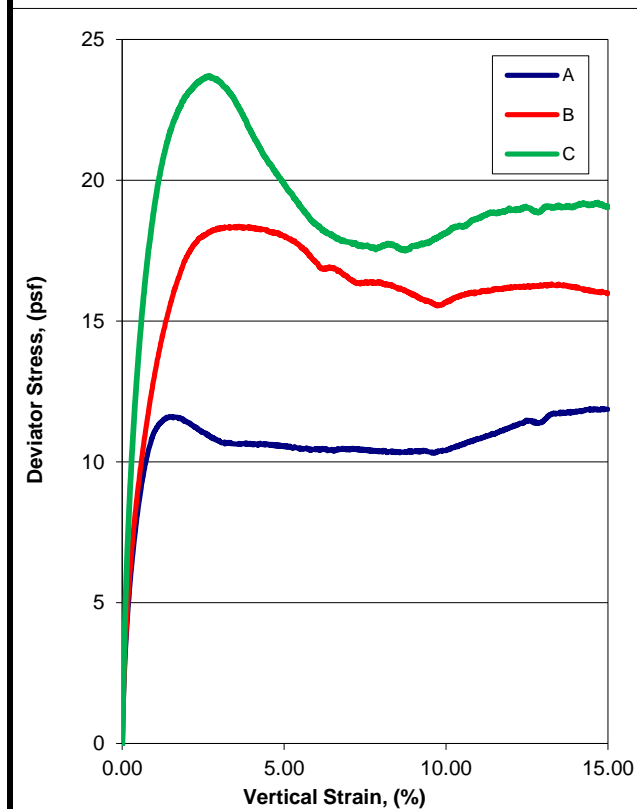
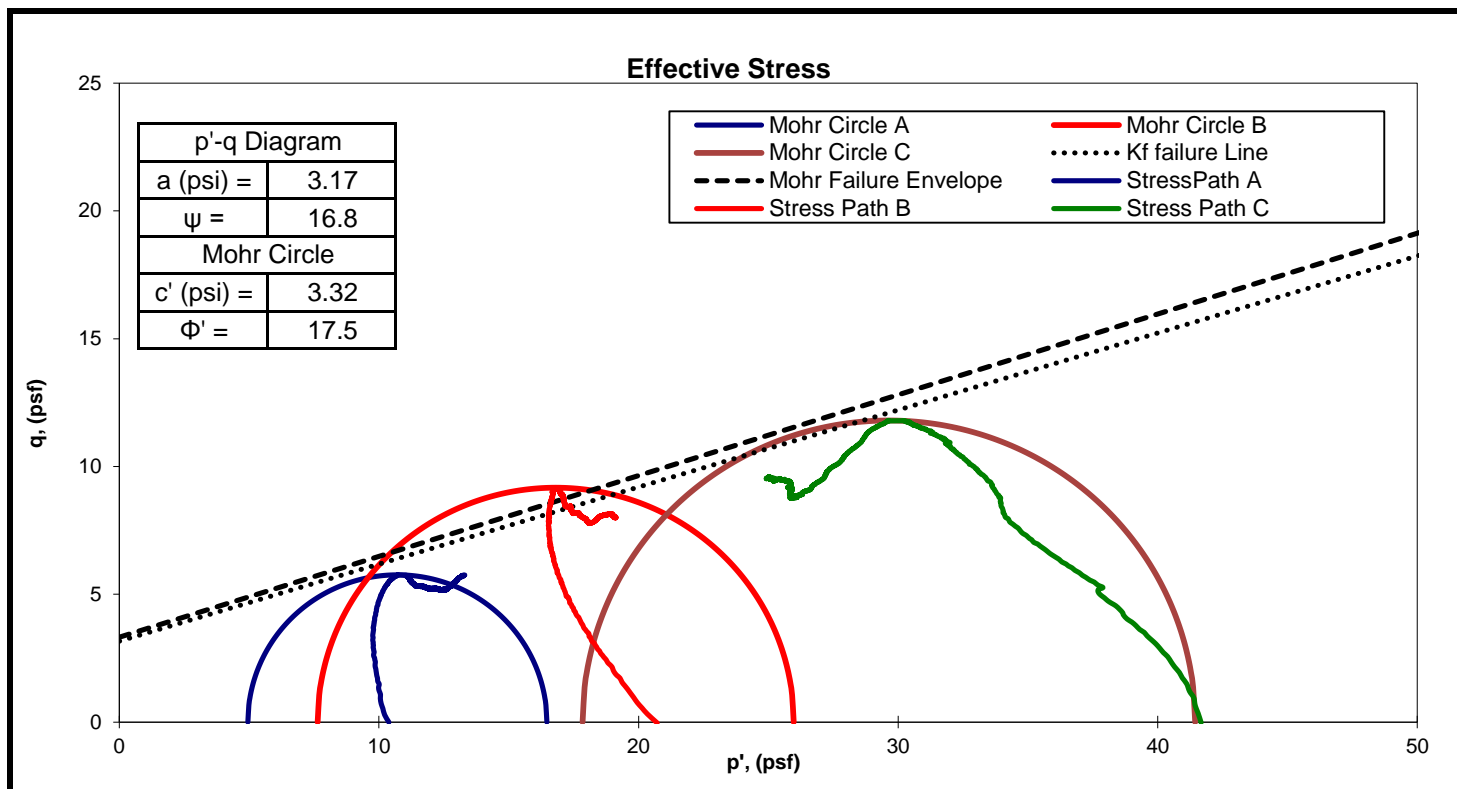
| SYMBOL | LOCATION | DEPTH (FT) | LIQUID LIMIT, LL | PLASTIC LIMIT, PL | PLASTICITY INDEX, PI | USCS CLASSIFICATION (Fraction Finer Than No. 40 Sieve) | USCS (Entire Sample) |
|--------|----------|------------|------------------|-------------------|----------------------|--|----------------------|
| ● | B-5 | TW 25-27 | 61 | 21 | 40 | CH | CH |
| ■ | B-5 | 40.5-41.0 | 49 | 20 | 29 | CL | CL |
| ◆ | B-6 | 15.0-16.0 | 42 | 17 | 25 | CL | CL |
| ○ | B7 | 30.5-31.0 | 40 | 16 | 24 | CL | CL |

NP - INDICATES NON-PLASTIC



PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 4318

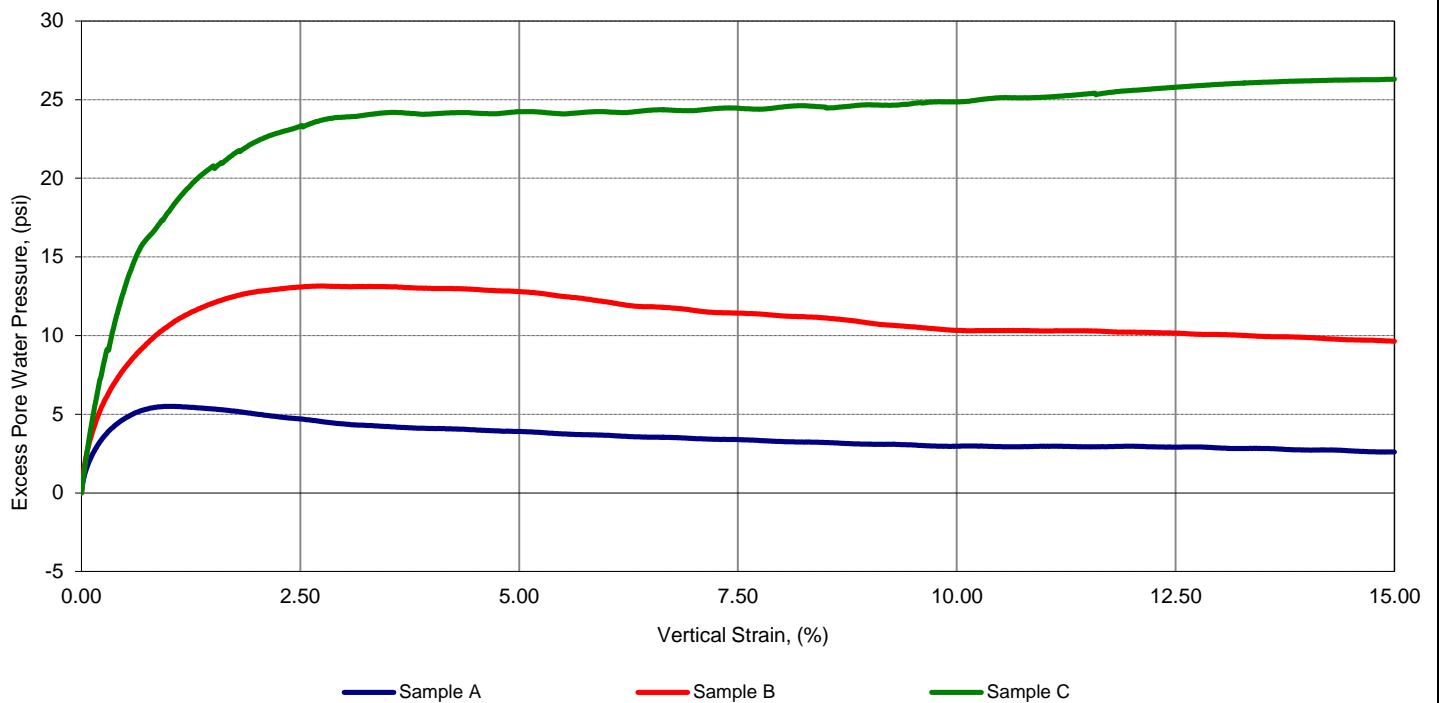
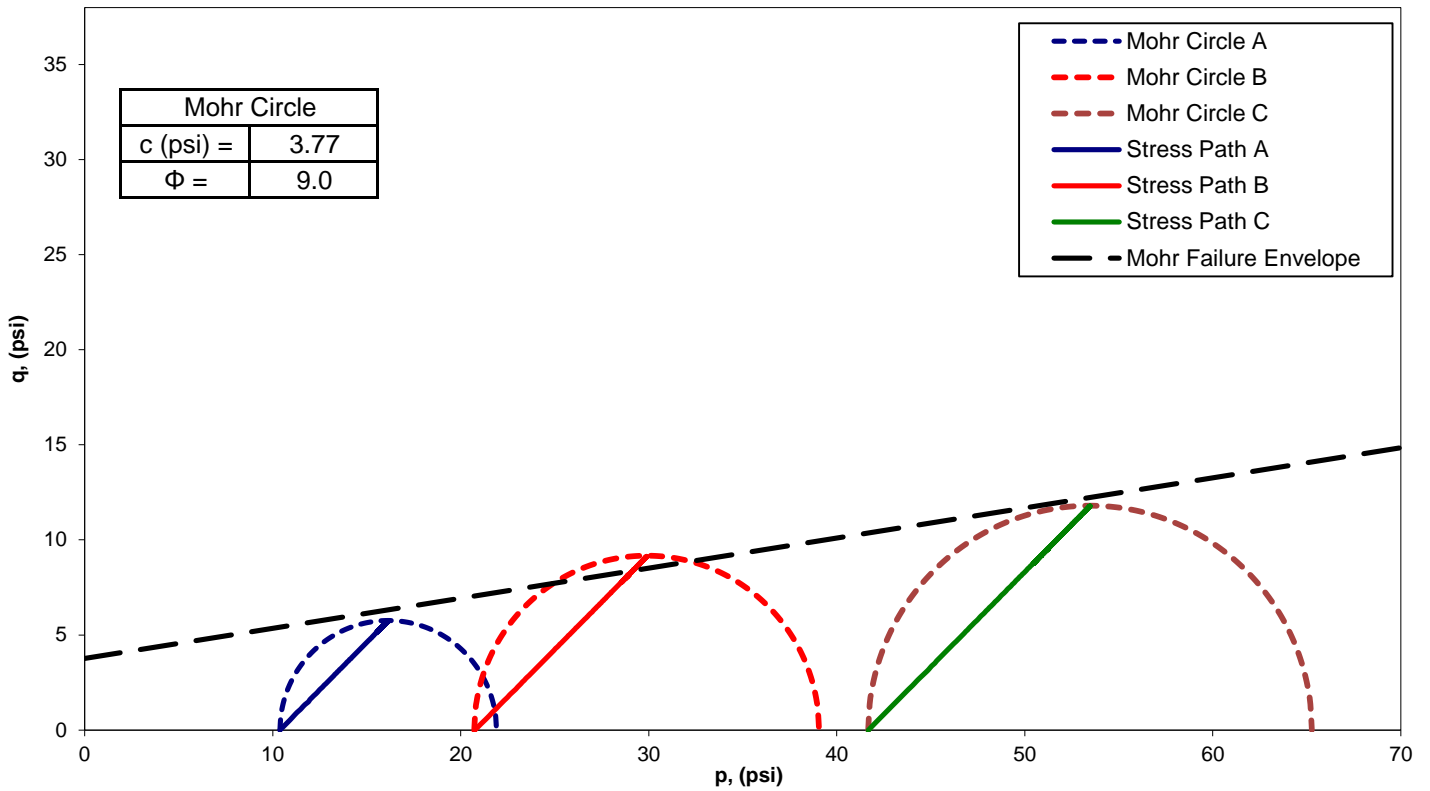
| | | | |
|---------------------------------|------|---|--------|
| <i>Ninyo & Moore</i> | | ATTERBERG LIMITS TEST RESULTS | FIGURE |
| PROJECT NO. | DATE | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |



| | | | | |
|-------------------------------|------------------|--------------|-----------|-----------|
| Location: | | B-5 | | |
| Sample Depth: | | TW 25.0-27.0 | | |
| Lab Technician: | | JCE | | |
| Checked By: | | HJG | | |
| Sample ID | | A | B | C |
| Date Tested | | 2/25/2017 | 2/25/2017 | 2/25/2017 |
| Initial | Diameter, in | 2.88 | 2.88 | 2.87 |
| | Height, in | 5.68 | 5.71 | 5.68 |
| | Water Content % | 21.3% | 21.5% | 20.8% |
| | Dry Density, pcf | 99.1 | 98.3 | 99.0 |
| | Saturation, % | 82.3% | 81.2% | 80.0% |
| Before Shear | Void Ratio | 0.700 | 0.714 | 0.701 |
| | Water Content % | 26.1% | 25.9% | 25.0% |
| | Dry Density, pcf | 96.8 | 98.5 | 98.4 |
| | Saturation, % | 100.0% | 100.0% | 100.0% |
| | Void Ratio | 0.741 | 0.710 | 0.712 |
| Back pressure, psf | | 50 | 50 | 50 |
| Obliquity Effective Failure % | | 1.28 | 3.32 | 2.89 |
| Obliquity Total Failure % | | 1.28 | 3.32 | 2.89 |
| Effective Confinement, psi | | 10.4 | 20.7 | 41.7 |
| B- Value | | 0.95 | 0.95 | 0.95 |
| Strain Rate, %/min | | 0.05 | 0.05 | 0.05 |

| | | | |
|----------------|------|--|--------|
| | | Consolidated Undrained Triaxial Test Data Sheet | Figure |
| | | | |
| Project Number | Date | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| 604667003 | 3/17 | | |

Total Stress



Ninyo & Moore

Location: B-5

Sample Depth (ft): TW 25.0-27.0

Project Number: 604667003

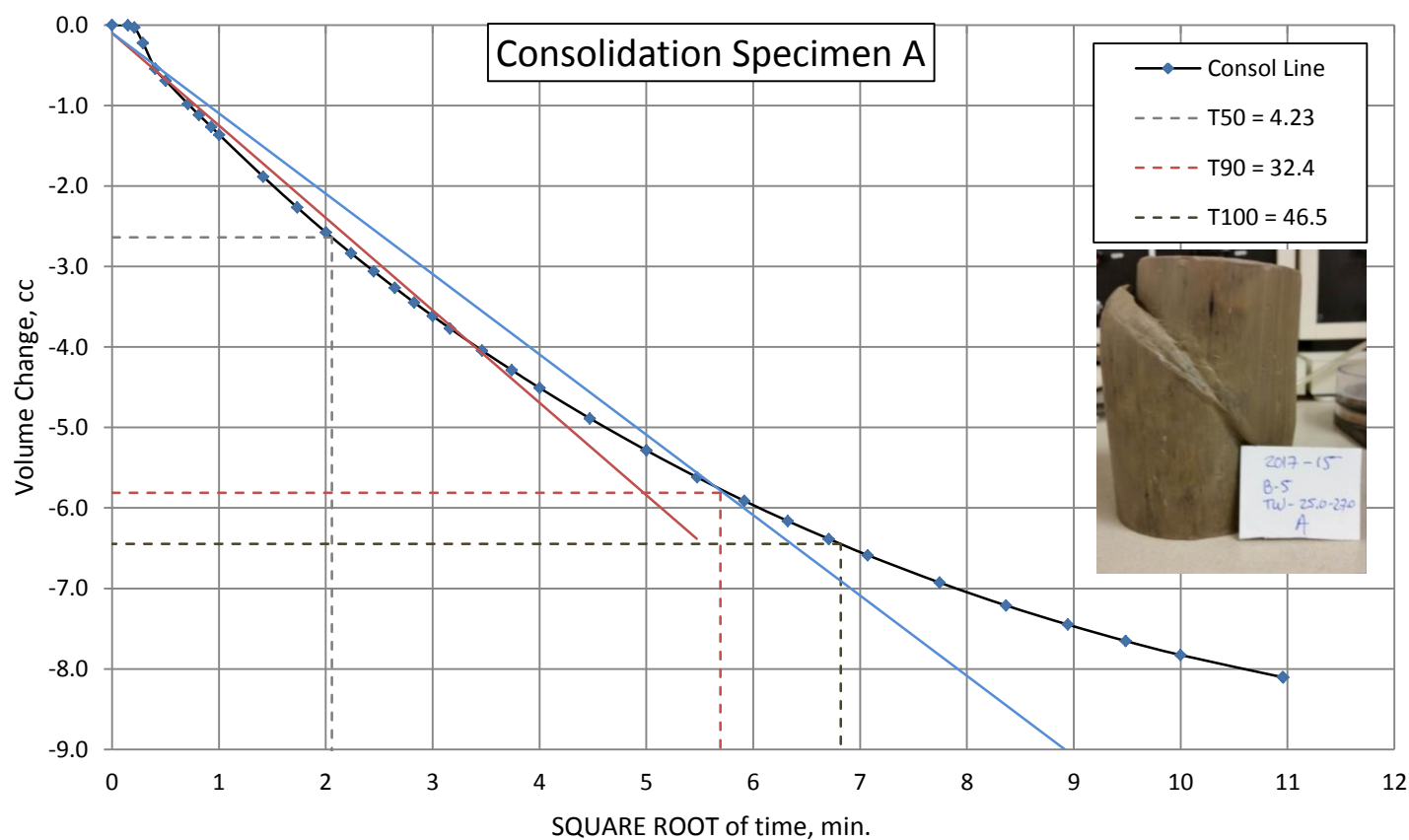
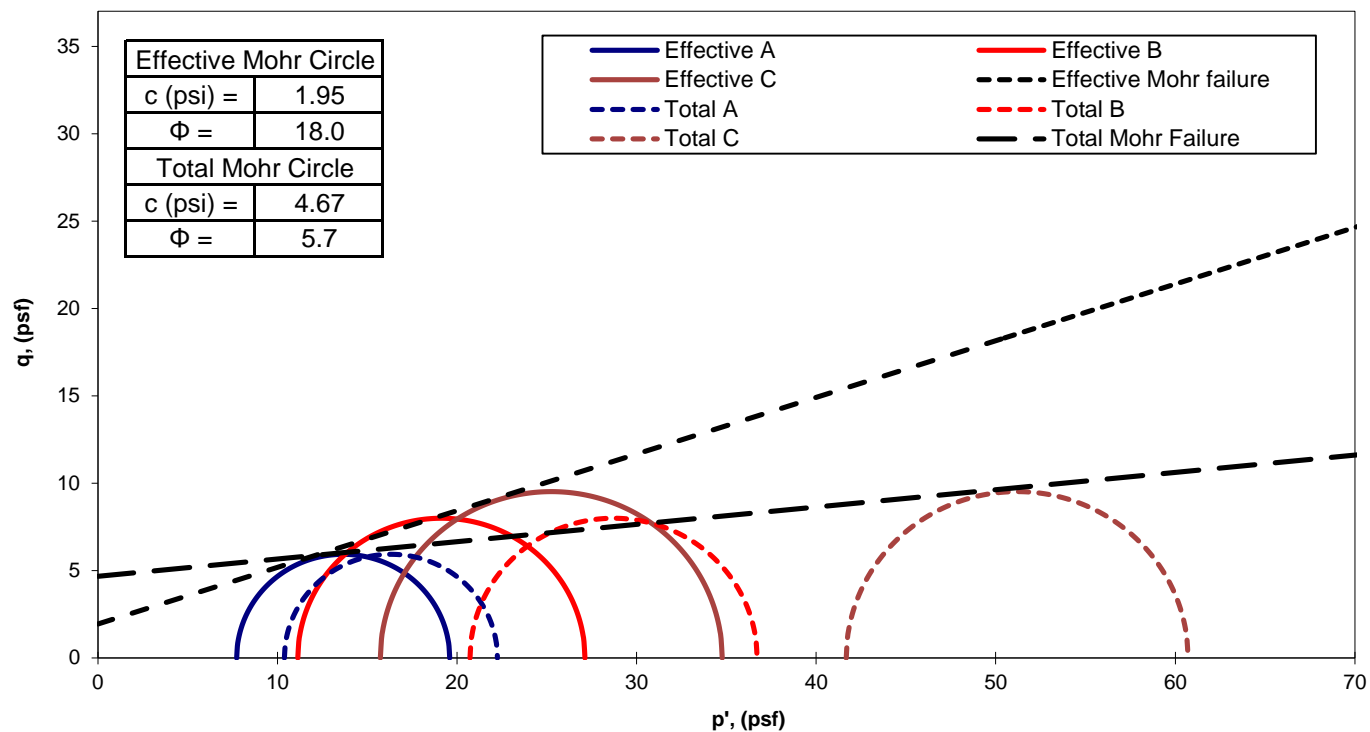
Date: 3/17

Consolidated Undrained Triaxial Test Data Sheet

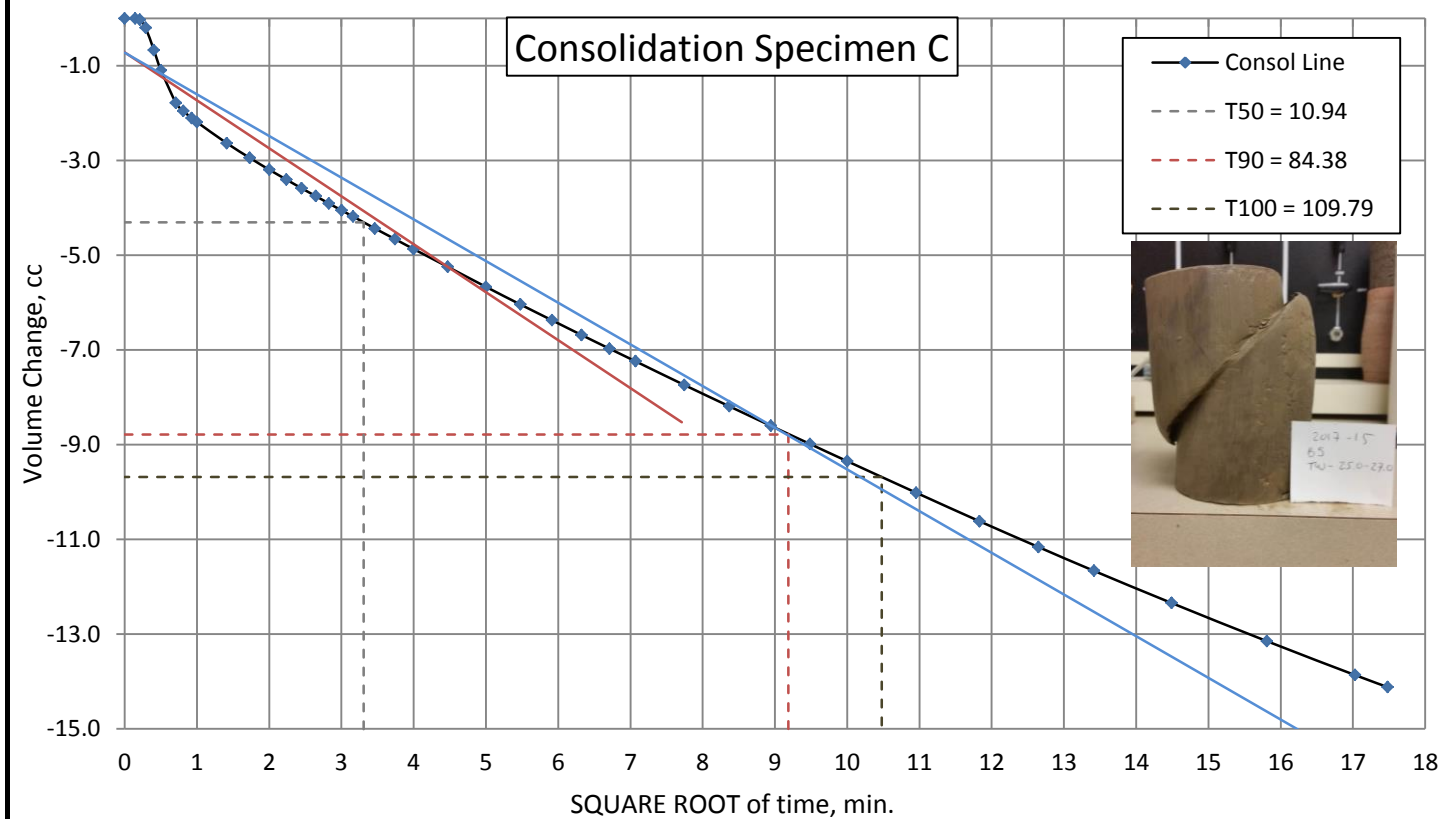
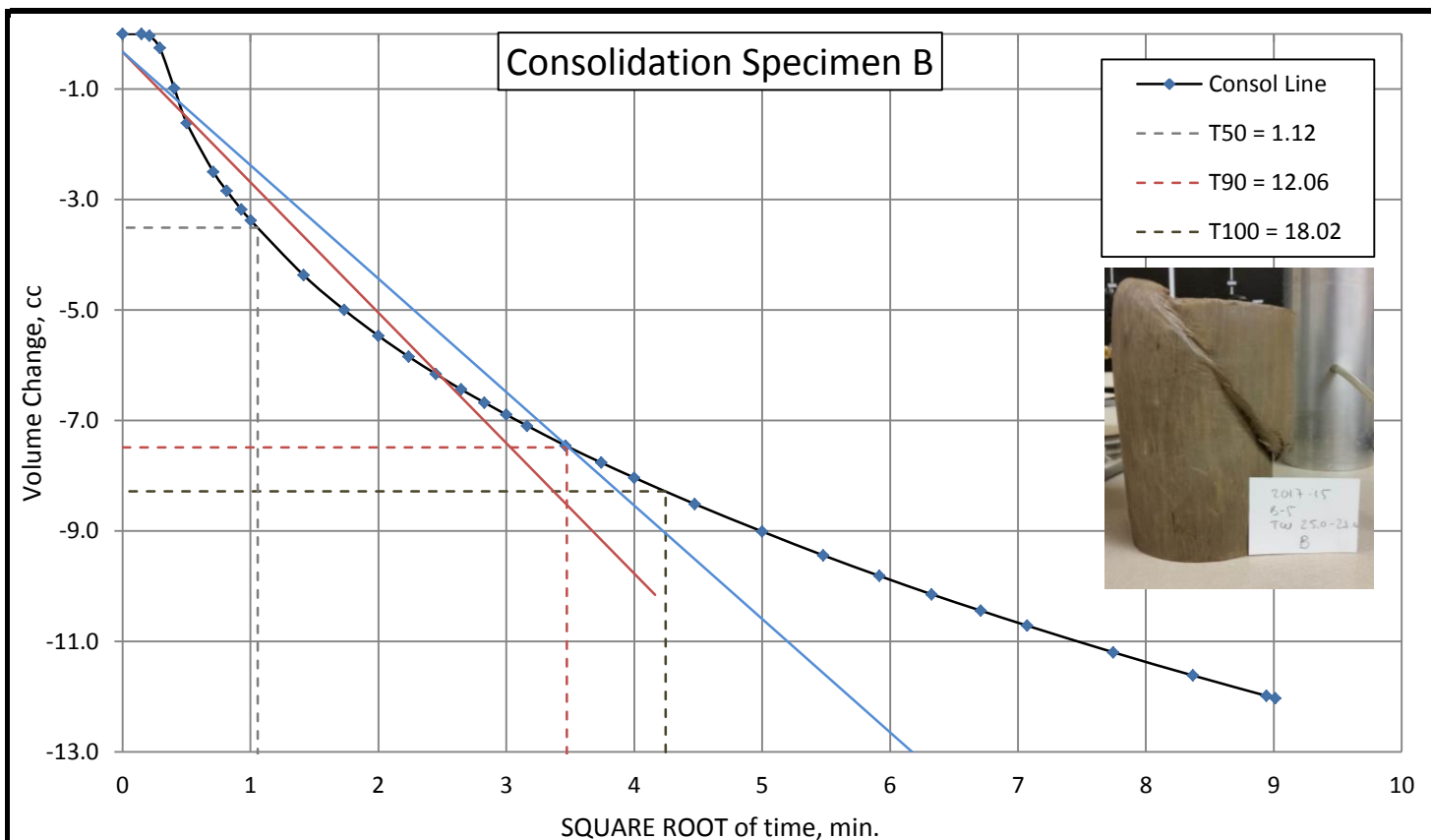
STANTEC/MWH/LAB TESTING
PHOENIX, ARIZONA

Figure

Effective & Total Stress at 15% Shear



| | | | |
|---------------------------|---------------------|--|---------------|
| Ninyo & Moore | | Consolidated Undrained Triaxial Test Data Sheet | Figure |
| Location: | B-5 | | |
| Sample Depth (ft): | TW 25.0-27.0 | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| Project Number: | 604667003 | | |
| Date: | 3/17 | | |



| | | | |
|---------------------------|---------------------|--|---------------|
| Ninyo & Moore | | Consolidated Undrained Triaxial Test Data Sheet | Figure |
| Location: | B-5 | | |
| Sample Depth (ft): | TW 25.0-27.0 | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |
| Project Number: | 604667003 | | |
| Date: | 3/17 | | |

| SAMPLE LOCATION | COMPRESSION STRENGTH Lb/ft2 | VOLUMETRIC DENSITY pcf | SPECIFIC GRAVITY | ABSORPTION % | INITIAL MOISTURE % | SULFATE SOUNDNESS % LOSS |
|--------------------|--------------------------------|---------------------------|------------------|-----------------|-----------------------|-----------------------------|
| B-1 5.3-9.5 | 1210 | 117.4 | 1.871 | 10.8 | 5.6 | 100.0 |
| B-1 17.3-18.0 | 2560 | 135.3 | 2.096 | 7.4 | 2.8 | 81.8 |
| B-2 26.45-27.25 | 2490 | 129.1 | 2.006 | 8.1 | 10.3 | 88.6 |
| B-3 24.2-24.9 | 1390 | 115.6 | 1.937 | 9.3 | 3.9 | 87.7 |
| B-3 24.9-25.6 | 1230 | 117.1 | 1.920 | 9.7 | 3.2 | 100.0 |

| | | | |
|---------------------------------|------|---|--------|
| <i>Ninyo & Moore</i> | | COMPRESSIVE STRENGTH OF SOIL SPECIMENS | FIGURE |
| PROJECT NO. | DATE | | |
| 604667003 | 3/17 | | |
| | | STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA | |

| SAMPLE LOCATION | MOISTURE (%) | DENSITY (pcf) | SPECIFIC GRAVITY |
|-----------------|--------------|---------------|------------------|
| B-5 @ 5.5-6.0 | 9.8 | 82.4 | 2.698 |
| B-5 @ 30.5-31.0 | -- | -- | 2.498 |
| B-6 @ 12.8-13.5 | -- | -- | 2.724 |
| B-6 @ 16.5-20.0 | -- | -- | 2.423 |
| B-7 @ 16.0-16.5 | 17.8 | 99.3 | 2.538 |

WEIGHT OF SAMPLE DISPERSED: **50.0**
 PERCENT PASSING #10 SIEVE: **91.1**

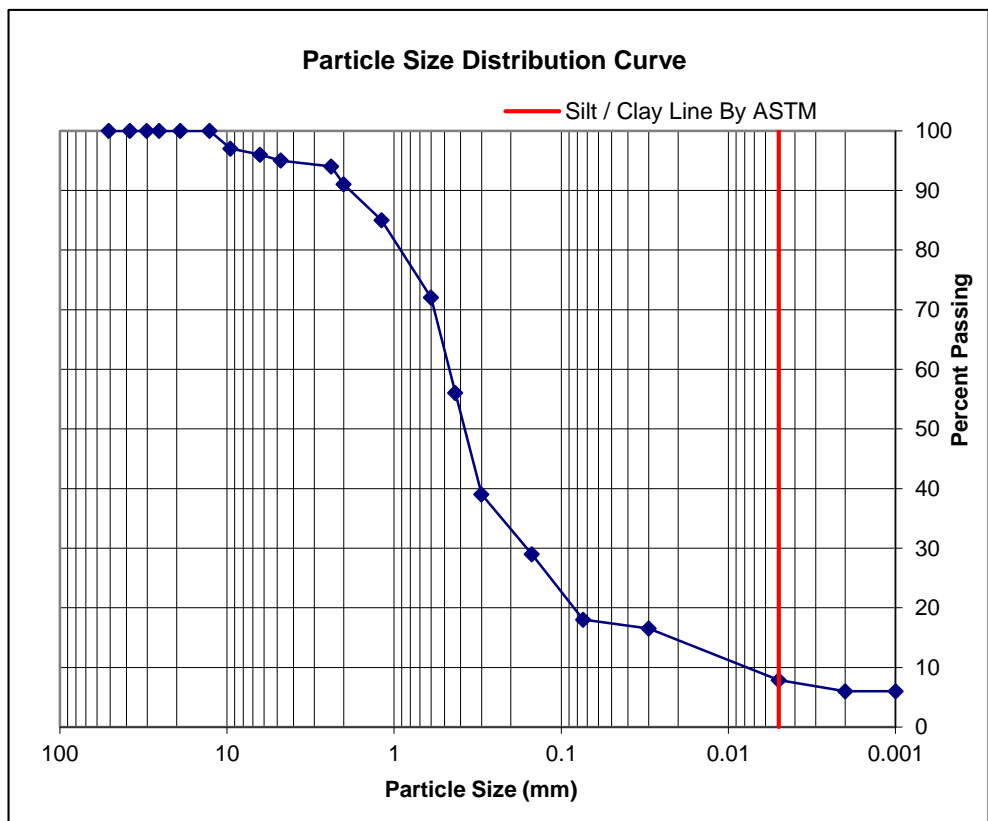
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0567 | 0.0365 | 0.0213 | 0.0152 | 0.0108 | 0.0053 | 0.0022 | 0.0016 |
| PERCENT SAMPLE TESTED | 24.0 | 18.0 | 14.0 | 10.0 | 10.0 | 8.0 | 6.0 | 6.0 |
| PERCENT TOTAL SAMPLE | 21.8 | 16.4 | 12.7 | 9.1 | 9.1 | 7.3 | 5.5 | 5.5 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 17.8 | 28.6 | 39.5 | 55.8 | 72.3 | 84.7 | 91.1 |

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 97 | |
| 1/4 IN | 96 | |
| # 4 | 95 | |
| # 8 | 94 | |
| # 10 | 91 | |
| # 16 | 85 | |
| # 30 | 72 | |
| # 40 | 56 | |
| # 50 | 39 | |
| # 100 | 29 | |
| # 200 | 18 | |
| 0.03 mm | 16.5 | |
| 0.005 mm | 7.9 | |
| 0.002 mm | 6.0 | |
| 0.001 mm | 6.0 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-5 | 5.5-6.0 | -- | -- | NP | 0.008 | 0.161 | 0.467 | 58.4 | 6.9 | 18.0 | SM |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 100.0

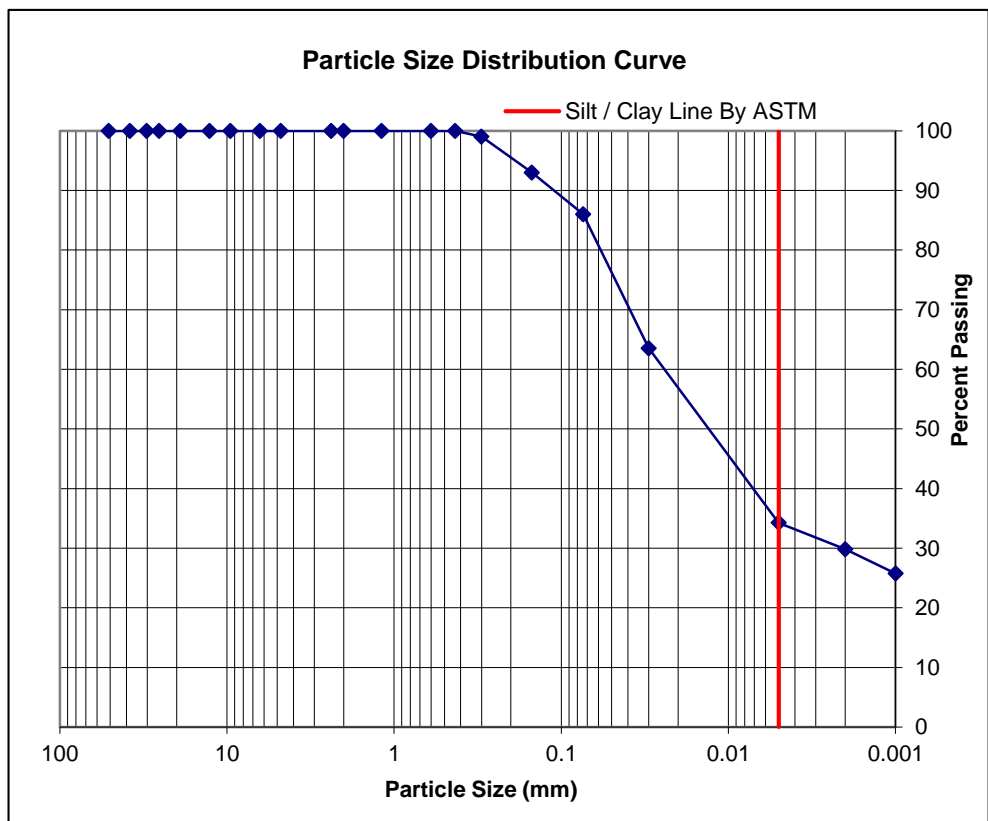
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0477 | 0.0312 | 0.0184 | 0.0135 | 0.0097 | 0.0049 | 0.0021 | 0.0015 |
| PERCENT SAMPLE TESTED | 72.0 | 64.0 | 58.0 | 48.0 | 44.0 | 34.0 | 30.0 | 28.0 |
| PERCENT TOTAL SAMPLE | 72.0 | 64.0 | 58.0 | 48.0 | 44.0 | 34.0 | 30.0 | 28.0 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|-------|-------|-------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 86.0 | 93.1 | 98.5 | 99.6 | 100.0 | 100.0 | 100.0 |

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 100 | |
| # 8 | 100 | |
| # 10 | 100 | |
| # 16 | 100 | |
| # 30 | 100 | |
| # 40 | 100 | |
| # 50 | 99 | |
| # 100 | 93 | |
| # 200 | 86 | |
| 0.03 mm | 63.5 | |
| 0.005 mm | 34.2 | |
| 0.002 mm | 29.8 | |
| 0.001 mm | 25.7 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-5 | 30.5-15.0 | -- | -- | NOT TESTED | -- | 0.002 | 0.024 | -- | -- | 86.0 | CL |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

WEIGHT OF SAMPLE DISPERSED: **50.0**
 PERCENT PASSING #10 SIEVE: **88.4**

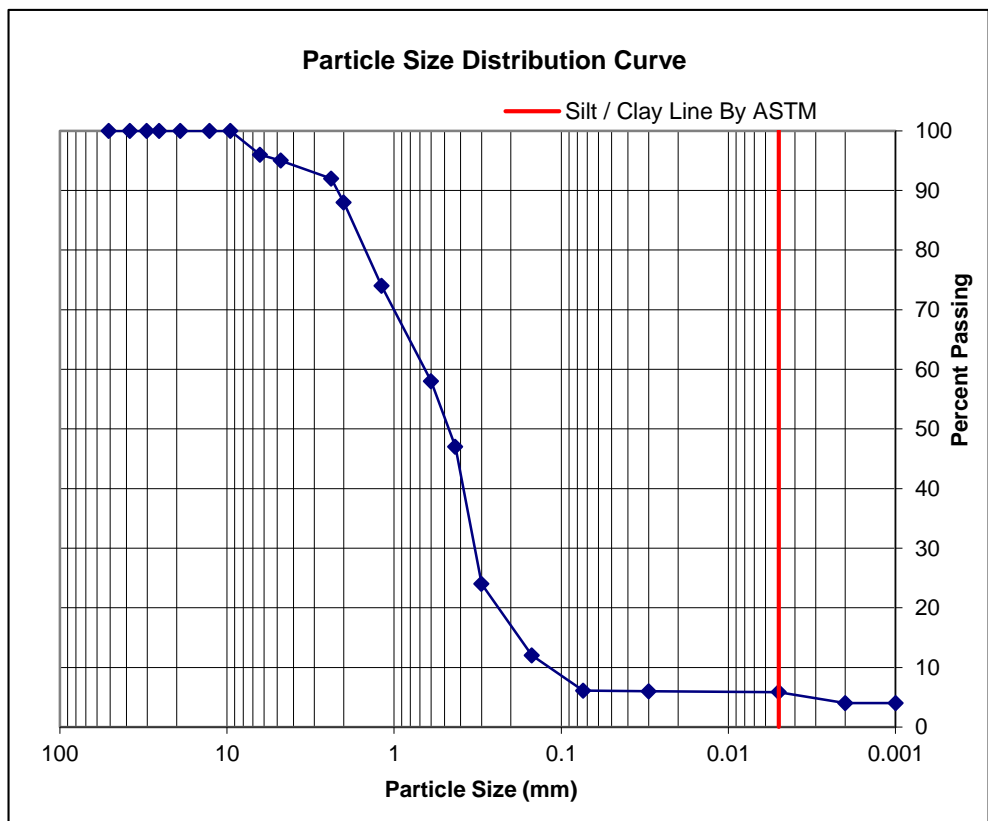
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0597 | 0.0378 | 0.0218 | 0.0154 | 0.0109 | 0.0053 | 0.0022 | 0.0016 |
| PERCENT SAMPLE TESTED | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 4.0 | 4.0 |
| PERCENT TOTAL SAMPLE | 5.3 | 5.3 | 5.3 | 5.3 | 5.3 | 5.3 | 3.5 | 3.5 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 6.1 | 12.0 | 24.4 | 47.0 | 58.4 | 74.5 | 88.4 |

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 96 | |
| # 4 | 95 | |
| # 8 | 92 | |
| # 10 | 88 | |
| # 16 | 74 | |
| # 30 | 58 | |
| # 40 | 47 | |
| # 50 | 24 | |
| # 100 | 12 | |
| # 200 | 6.1 | |
| 0.03 mm | 6.0 | |
| 0.005 mm | 5.8 | |
| 0.002 mm | 4.0 | |
| 0.001 mm | 4.0 | |



| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|-------|
| | B-6 | 12.8-13.5 | -- | -- | NP | 0.118 | 0.330 | 0.654 | 5.5 | 1.4 | 6.1 | SP-SM |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 94.2

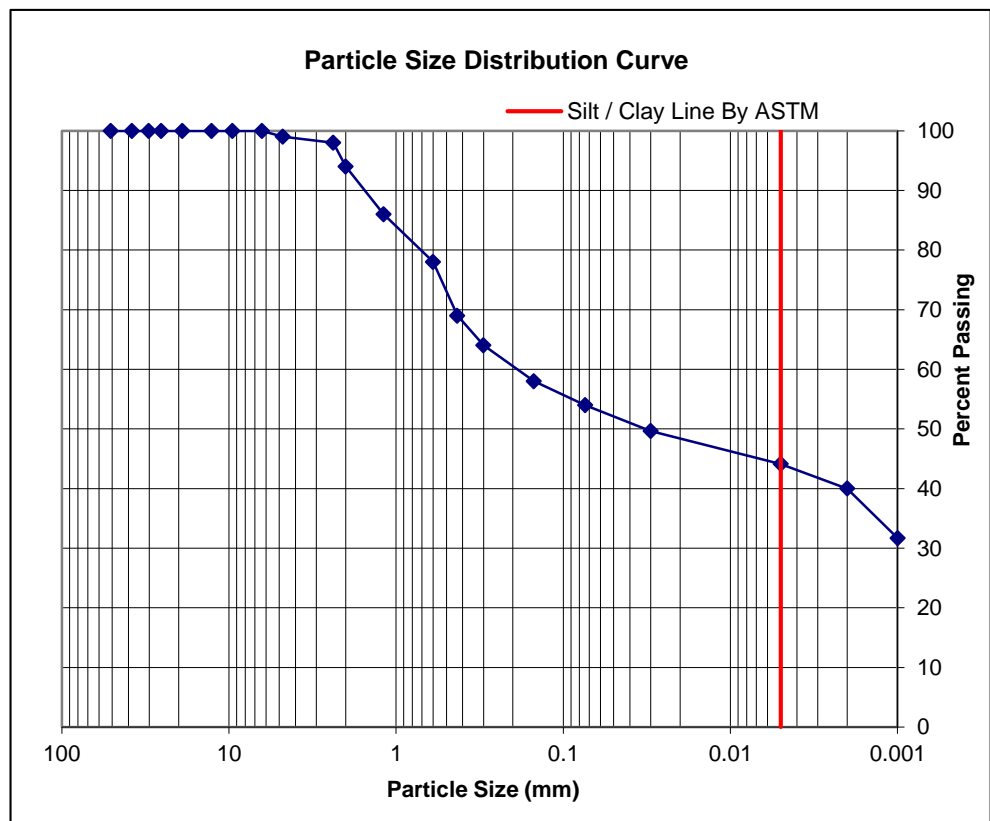
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

| | HYDROMETER RESULTS (% PASSING) | | | | | | | |
|-------------------------|--------------------------------|--------|--------|--------|--------|--------|--------|--------|
| PARTICLE SIZE (DIA. mm) | 0.0512 | 0.0329 | 0.0191 | 0.0136 | 0.0096 | 0.0048 | 0.0020 | 0.0014 |
| PERCENT SAMPLE TESTED | 54.0 | 50.0 | 48.0 | 46.0 | 46.0 | 44.0 | 40.0 | 36.0 |
| PERCENT TOTAL SAMPLE | 50.9 | 47.1 | 45.2 | 43.3 | 43.3 | 41.4 | 37.7 | 33.9 |

| | MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING) | | | | | | |
|----------------------|--|------|------|------|------|------|------|
| SCREEN SIZE | #200 | #100 | #50 | #40 | #30 | #16 | #10 |
| PERCENT TOTAL SAMPLE | 54.0 | 58.0 | 63.6 | 69.3 | 78.3 | 86.1 | 94.2 |

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

| | % Pass | Spec |
|----------|--------|------|
| 2 IN | 100 | |
| 1 1/2 IN | 100 | |
| 1 1/4 IN | 100 | |
| 1 IN | 100 | |
| 3/4 IN | 100 | |
| 1/2 IN | 100 | |
| 3/8 IN | 100 | |
| 1/4 IN | 100 | |
| # 4 | 99 | |
| # 8 | 98 | |
| # 10 | 94 | |
| # 16 | 86 | |
| # 30 | 78 | |
| # 40 | 69 | |
| # 50 | 64 | |
| # 100 | 58 | |
| # 200 | 54 | |
| 0.03 mm | 49.6 | |
| 0.005 mm | 44.1 | |
| 0.002 mm | 40.0 | |
| 0.001 mm | 31.7 | |

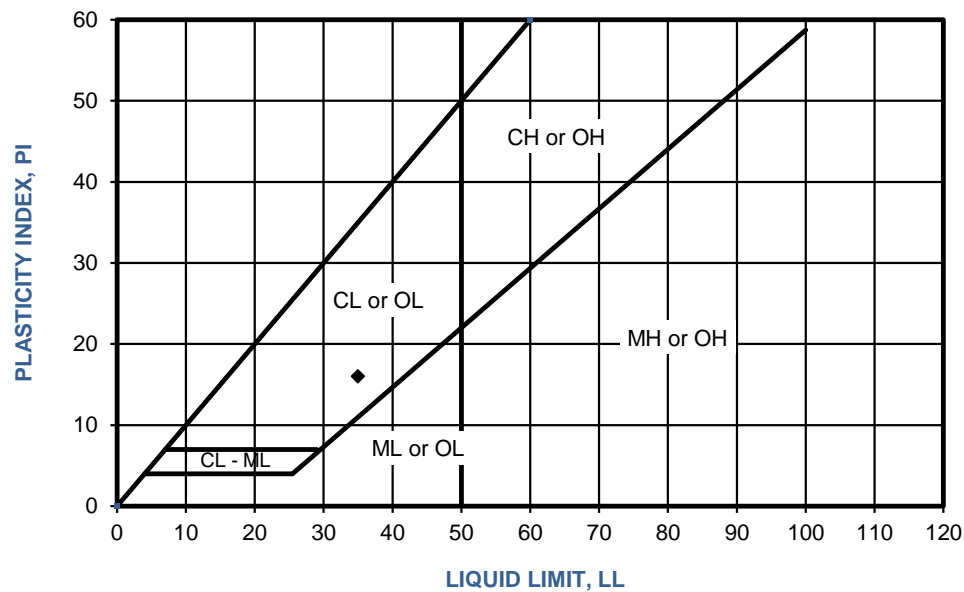


| Symbol | Sample Location | Depth (ft) | Liquid Limit | Plastic Limit | Plasticity Index | D ₁₀ | D ₃₀ | D ₆₀ | C _u | C _c | Passing No. 200 (%) | USCS |
|--------|-----------------|------------|--------------|---------------|------------------|-----------------|-----------------|-----------------|----------------|----------------|---------------------|------|
| | B-7 | 16.0-16.5 | 35 | 19 | 16 | -- | -- | 0.189 | -- | -- | 54.0 | CL |

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

| SYMBOL | LOCATION | DEPTH (ft) | LIQUID LIMIT | PLASTIC LIMIT | PLASTICITY INDEX | USCS CLASSIFICATION (Fraction Finer Than No. 40 Sieve) | USCS |
|--------|----------|------------|--------------|---------------|------------------|--|-------|
| ● | B-5 | 5.5-6.0 | -- | -- | NP | ML | SM |
| ■ | B-6 | 12.8-13.5 | -- | -- | NP | ML | SP-SM |
| ◆ | B-7 | 16.0-16.5 | 35 | 19 | 16 | CL | CL |

NP - INDICATES NON-PLASTIC



PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 4318

FIGURE --

ATTERBERG TEST RESULTS

STANTEC/MWH/LAB TESTING

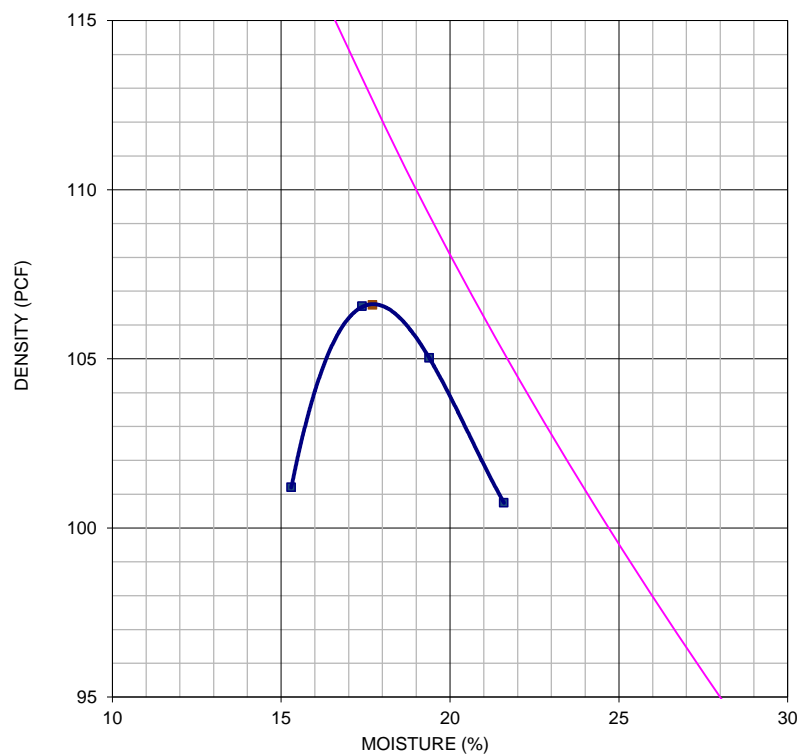
PHOENIX, ARIZONA

604667003 | 8/17

SAMPLE INFORMATION:

DESCRIPTION: CL
LOCATION: CLAY SAMPLES
DEPTH (FT): --
LAB TECHNICIAN: HJG

| PROCTOR ¹ | METHOD |
|-------------------------------|-----------------------|
| MAXIMUM DRY DENSITY (PCF) | ASTM D 698 A 106.6 |
| OPTIMUM MOISTURE CONTENT (%) | 17.7 |
| ROCK CONTENT (%) ² | -- |



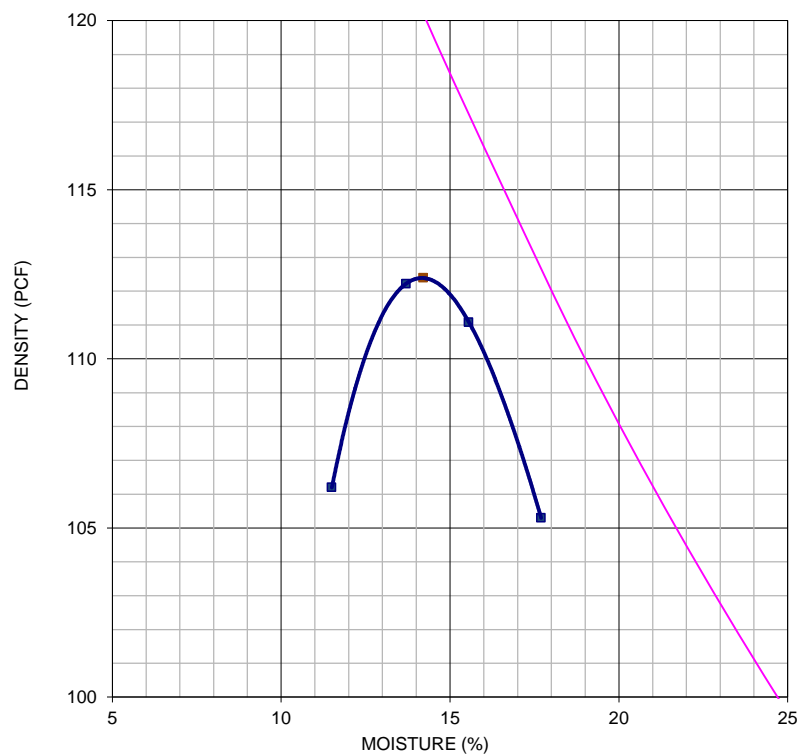
REMARKS: MAXIMUM DENSITY AND OPTIMUM MOISTURE WERE NOT CORRECTED FOR ROCK CONTENT
ROCK CONTENT CALCULATED FROM MATERIAL RETAINED ON NO.4 SCREEN

LAB NUMBER: 2017-72

SAMPLE INFORMATION:

DESCRIPTION: SM
LOCATION: SAND SAMPLES
DEPTH (FT): --
LAB TECHNICIAN: HJG

| PROCTOR ¹ | METHOD |
|-------------------------------|-----------------------|
| MAXIMUM DRY DENSITY (PCF) | ASTM D 698 A 112.4 |
| OPTIMUM MOISTURE CONTENT (%) | 14.2 |
| ROCK CONTENT (%) ² | -- |



REMARKS: MAXIMUM DENSITY AND OPTIMUM MOISTURE WERE NOT CORRECTED FOR ROCK CONTENT
ROCK CONTENT CALCULATED FROM MATERIAL RETAINED ON NO.4 SCREEN

LAB NUMBER: 2017-72