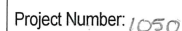


**GEOTECHNICAL EVALUATION
CHURCH ROCK MILL SITE JETTY**

Appendix A Boring Logs and Core Photos
April 24, 2017

DAILY FIELD LOGS



Date: 8-Nov-2016

Field Representative: Jen Van Pelt, S. Downey

Note activities, weather conditions, visitors, variances, safety issues, communications, mechanical issues, scheduling issues, etc.

ON SITE

J. VAN PECT	(MWH)
S. DOWNNEY	(MWH)
J. BROOKES	(EPA)
L. ANDROSS	(EA)
M. CAW	(NEWP)
D. MARTINEZ	(NEWP)
A. CORTEZ	(NEWP)
R. SPITZ	(AMEC FR)

07:00 ON SITE, brief tailgate

07:15 DRILLER MOBE TO B2, finish logging B1

09:15 Begin drilling B2

13:04 Complete dr. fling B2 at 52.1 feed bgs.

Molbe to B3

14:55 Begun drilling B3

15:30 at bed rock duffer switches to

copy

16:30 ⁰drillers ready to core - will start tomorrow

17:00 leave site

[illegible]

Date:



Project: NECR UNR JETTY GEOTECHNICAL

DAILY FIELD
ACTIVITY LOG

Date: 10-Nov-2016

Project Number:

Field Representative: JEN VAN PELT, STEPHANIE DOWNEY

Note activities, weather conditions, visitors,
variances, safety issues, communications,
mechanical issues, scheduling issues, etc.

Binod
CHAUDHURY
(N.M.EPA)

ON SITE:

J. VAN PELT	(MWH)
S. DOWNEY	(MWH)
J. BROOKES	(EPA)
L. ANDROSS	(EA)
M. CAIN	(NEWP)
DAN MARTINEZ	(NEWP)
ART CORTEZ	(NEWP)
RICK SPITZ	(AMEC FW)
Binod CHAUDHURY	(NAVABNTN, EPA)

SAMPLES

B4-5.5-6.0	(SSO)	→ 2" ID
B4-16.0-16.5	(SSO)	
B4-20.5-21.0	(SSO)	
B4-30.5-31.0	(SSO)	
B4-40.5-41.0	(SSO)	
B4-41.0-43.6	(GS)	
B4-45.0-49.0	(GS)	
B4-50.5-51.0	(SSO)	

07:00 MEET AT office, prep for day.
07:15 Move to site, setup for drilling.
07:46 SAFETY MEET.
08:00 PREP FOR DRILLING / CORING B3 at 17.7' bgs
08:15 DRILLING B3 at 15.9' bgs
10:20 coring completed on B3
11:00 drillers packing up gear to move machine
and equipment to other side of jetty
complete logging on B3.
11:50 Pack up gear. Photography of core.
13:00, Break for lunch
13:15 Began move to B4
14:30 drilling B4
16:30 halt core drilling for day. drillers
winterize r.g, containing securing core
gear on trailers
17:25 leave site

Boring	Description / Work Unit	Quantity
B3	core (ft)	
	Boxes (ea)	4
B4	SPT / Mod Cal Sampling	7
	Retained Mod Cal ^{BRASS.}	6
B4	CONT. CORE CME/ HSA. DRILLING	50.0'

Date: 10-Nov-2016

(15.9) 21.0
54' 47' etc.

Field Representative: J. Van Pelt / S. Downey

Note activities, weather conditions, visitors, variances, safety issues, communications, mechanical issues, scheduling issues, etc.

ON SITE:

J. VAN PELT	(MWH)
S. DOWNNEY	(MWH)
L. ANDROSS	(AE)
M. GAIN	(N.EWP)
D. MARTINEZ	(N.EWP)
D.J. MARTINEZ	(N.EWP)
R. SPITZ	(AMELFW)

SAMPLES

B7 - 77.2 - 78.4	(GS)
B7 - 78.4 - 80.0	(GS)
B7 - 80.5 - 81.0	(SSO)
B7 - 81.5 - 84.4	(GS)
B7 - 88.4 - 90	(GS)
B7 - 90.5 - 91.0	(SSO)
B7 - 91.5 - 95	(GS)
B7 - 100.5 - 101.0	(SSO)
B7 - 101.5 - 104	(GS)
B7 - 108.9 - 110.0	(GS)

07:00 on site at Admin. Sign in

DRIVERS prep, head to site

07:30: Retrieve DS : Return to office for safety

induction Re: Radiation

08:05 Tailgate

08:25 Continue drilling. B7 at 71.5'

10:45 At 100' bgs, driller called his PM RE: FOOTAGE
OVER 100

11:45 At 115' bgs, weathered Bedrock

12:00 Drillers break for lunch.

12:30 Prep for trip out of hole.

12:40 Mixing grout - 24 gal/^{50 lb} sack (CETCO) High Solids Bentonite grout

12:50 TREMMIE GROUT.

13:30 Pulled 6 rods/Augers, tremmed again.

178 total gallons of grout - backfill to surface w/ cutting.

14:30 - Mapped Jetty area while drillers

continued w/ cleanup & mops 170 gal (2.70 gal/A)

16:30 driller moved to B5 to begin = 64 feet

drilling on Sunday Morning

17:00 leave site

grout to 51 ft bgs
Back fill to
surface
w/ cuttings

[illegible]

Date: 12-Nov-2016

**GEOTECHNICAL EVALUATION
CHURCH ROCK MILL SITE JETTY**

Appendix B Field Photos
April 24, 2017

APPENDIX B FIELD PHOTOS



1. PIPELINE ARROYO, LOOKING DOWNSTREAM



2. PIPELINE ARROYO, LOOKING UPSTREAM



3. SIDEWALL OF ARROYO, LOOKING WEST-SOUTHWEST



4. EXPOSED COAL LAYER BELOW BORING B1 LOCATION



5. EXISTING JETTY EXPOSED IN THE EROSIONAL FLOW PATHWAY, PARALLEL TO JETTY



6. DRILLING OPERATIONS



7. DRILLING OPERATIONS, GROUTING BOREHOLE B6



8. GENERAL AREA OF DRILL SITE, LOCATION B6



9. TEST PIT 1



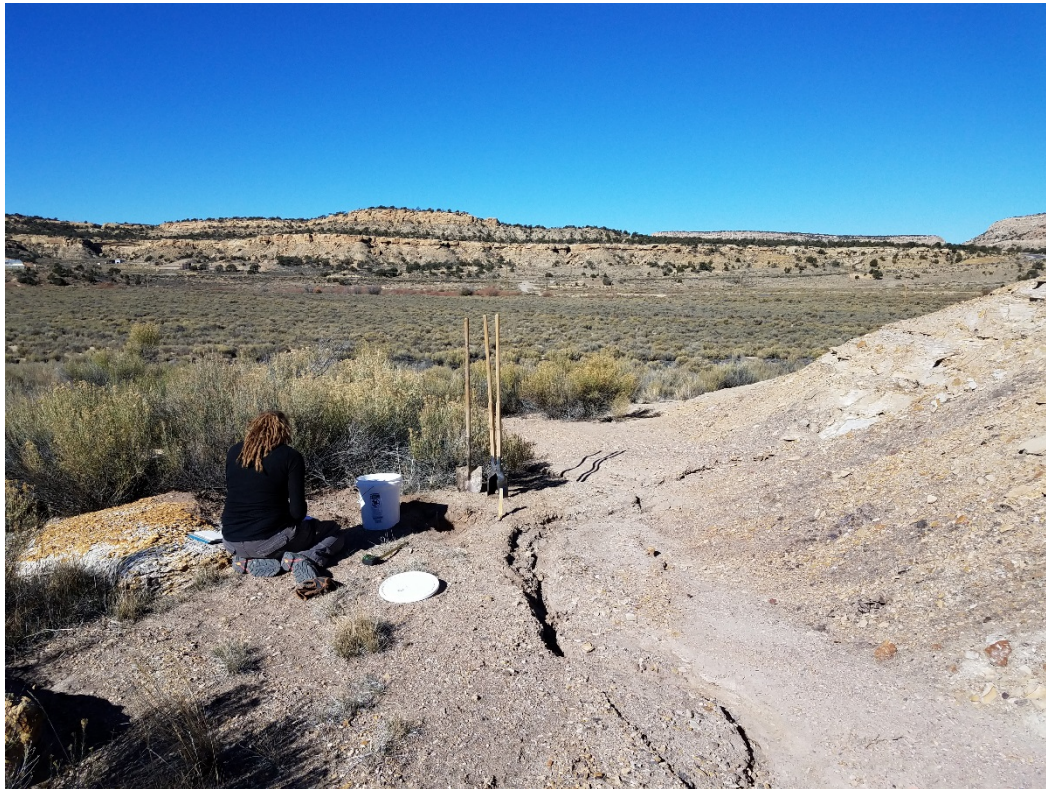
10. TEST PIT 2



11. TEST PIT 3



12. GENERAL LOCATION OF TEST PIT 3, LOOKING SOUTHEAST



13. GENERAL AREA OF TEST PIT 3 NEAR DILCO HILL, LOOKING NORTHWEST



14. TEST PIT 4 NEAR DILCO HILL

**GEOTECHNICAL EVALUATION
CHURCH ROCK MILL SITE JETTY**

Appendix C Laboratory Data
April 24, 2017

APPENDIX C LABORATORY DATA

SAMPLE LOCATION	SAMPLE DEPTH (FT)	FIELD CLASS	MOISTURE (%)	DRY DENSITY (pcf)
B-4	16.0-16.5	--	10.4	77.6
B-5	10.5-11.0	--	5.2	82.9
B-5 (TRIAXIAL)	TW 25.0-27.5	--	21.2	98.8
B-5	40.5-41.0	--	22.7	99.6
B-6	15.5-16.5	--	10.7	93.0
B-6	40.5-41.0	--	17.9	97.0
B-7	TW 5.0-6.5	--	6.9	94.3
B-7	10.5-11.0	--	7.0	99.7
B-7	30.5-31.0	--	16.7	102.5

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 2937 & ASTM D 2216

<i>Ninyo & Moore</i>		MOISTURE - DENSITY TEST DATA	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

WEIGHT OF SAMPLE DISPERSED: 51.2
 PERCENT PASSING #10 SIEVE: 100.0

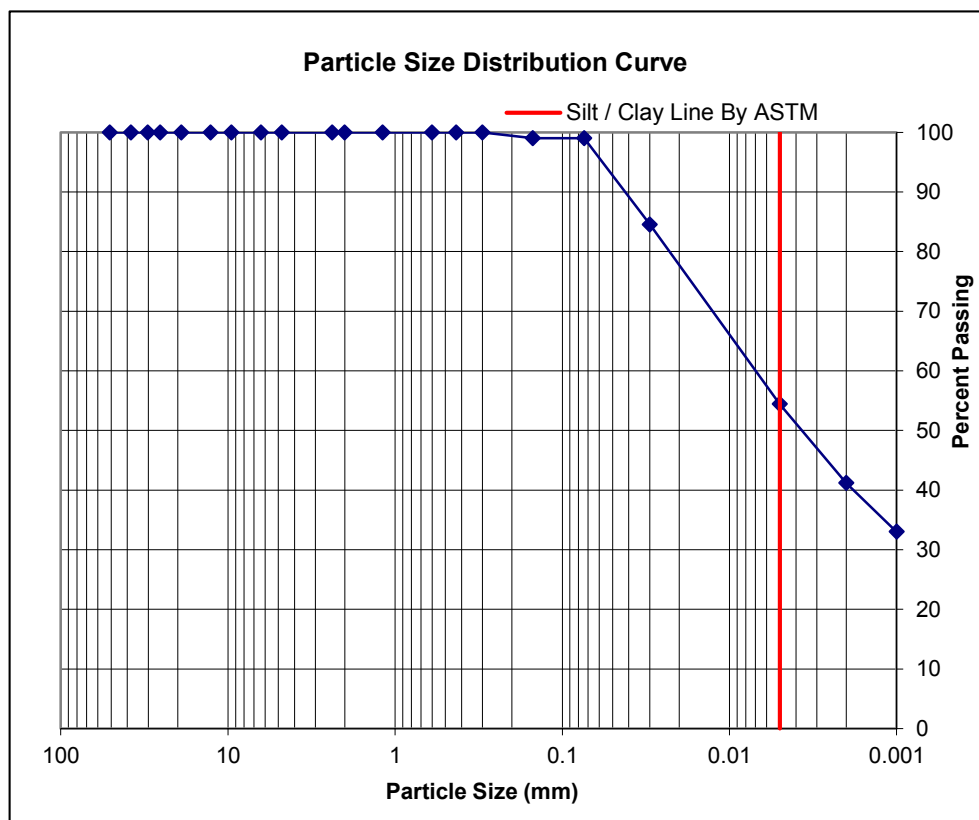
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0438	0.0283	0.0168	0.0123	0.0089	0.0046	0.0020	0.0014
PERCENT SAMPLE TESTED	87.9	84.0	78.2	70.3	65.5	52.8	41.0	37.1
PERCENT TOTAL SAMPLE	87.9	84.0	78.2	70.3	65.5	52.8	41.0	37.1

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	99.0	99.4	99.6	99.6	99.8	100.0	100.0

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	100	
3/8 IN	100	
1/4 IN	100	
# 4	100	
# 8	100	
# 10	100	
# 16	100	
# 30	100	
# 40	100	
# 50	100	
# 100	99	
# 200	99	
0.03 mm	84.6	
0.005 mm	54.4	
0.002 mm	41.2	
0.001 mm	33.0	



Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	B-5	40.5-41.0	49	20	29	--	--	0.007	--	--	99.0	CL

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

Ninyo & Moore		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 100.0

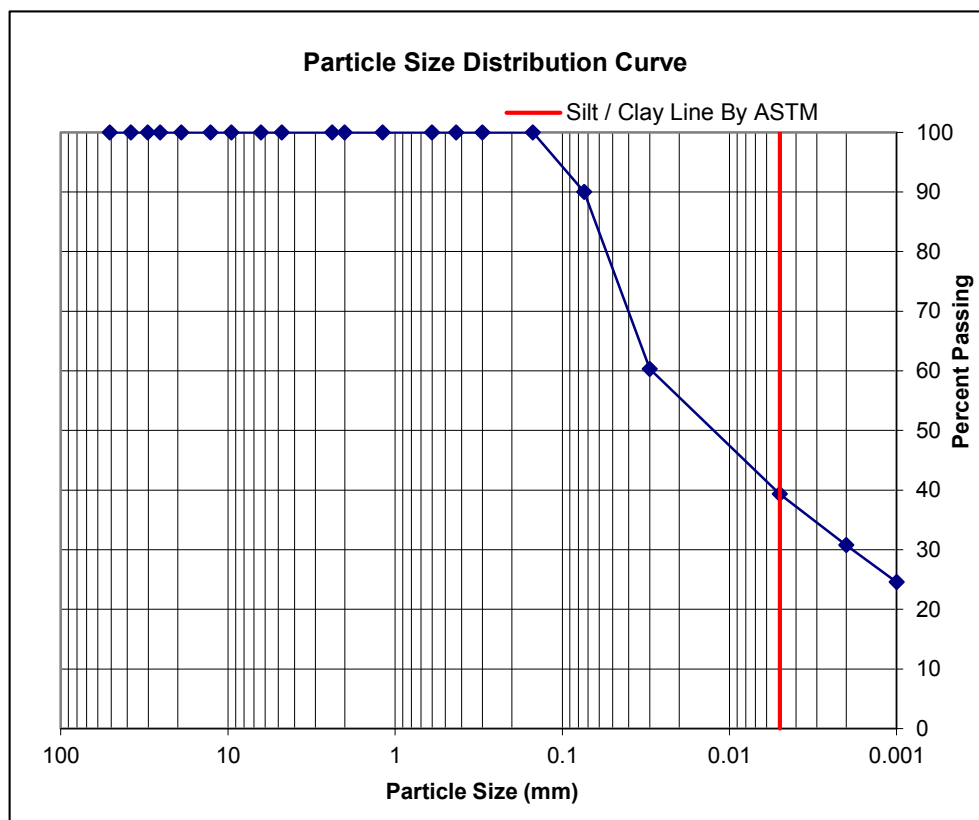
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0485	0.0315	0.0187	0.0134	0.0096	0.0048	0.0021	0.0015
PERCENT SAMPLE TESTED	68.0	61.0	54.0	50.0	46.0	39.0	31.0	28.0
PERCENT TOTAL SAMPLE	68.0	61.0	54.0	50.0	46.0	39.0	31.0	28.0

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	89.8	99.6	99.8	100.0	100.0	100.0	100.0

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	100	
3/8 IN	100	
1/4 IN	100	
# 4	100	
# 8	100	
# 10	100	
# 16	100	
# 30	100	
# 40	100	
# 50	100	
# 100	100	
# 200	90	
0.03 mm	60.3	
0.005 mm	39.3	
0.002 mm	30.8	
0.001 mm	24.6	



Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	B-6	15.0-16.0	42	17	25	--	0.002	0.029	--	--	90.0	CL

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

<i>Ninyo & Moore</i>		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

WEIGHT OF SAMPLE DISPERSED: 50.0
 PERCENT PASSING #10 SIEVE: 100.0

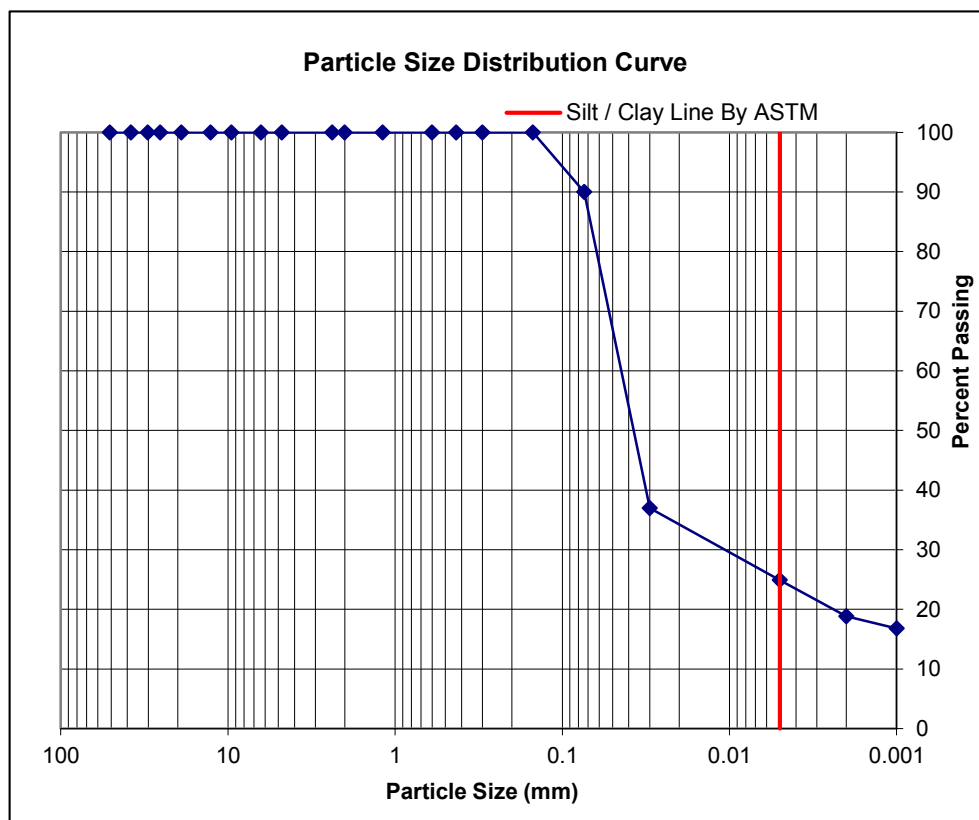
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0535	0.0343	0.0201	0.0143	0.0102	0.0051	0.0021	0.0015
PERCENT SAMPLE TESTED	42.0	38.0	34.0	32.0	29.0	25.0	19.0	18.0
PERCENT TOTAL SAMPLE	42.0	38.0	34.0	32.0	29.0	25.0	19.0	18.0

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	89.8	99.6	99.8	100.0	100.0	100.0	100.0

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	100	
3/8 IN	100	
1/4 IN	100	
# 4	100	
# 8	100	
# 10	100	
# 16	100	
# 30	100	
# 40	100	
# 50	100	
# 100	100	
# 200	90	
0.03 mm	37.0	
0.005 mm	24.9	
0.002 mm	18.8	
0.001 mm	16.8	



Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	B-7	10.5-11.0	--	--	N Test	--	0.011	0.044	--	--	90.0	

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

<i>Ninyo & Moore</i>		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

WEIGHT OF SAMPLE DISPERSED: 49.8
 PERCENT PASSING #10 SIEVE: 100.0

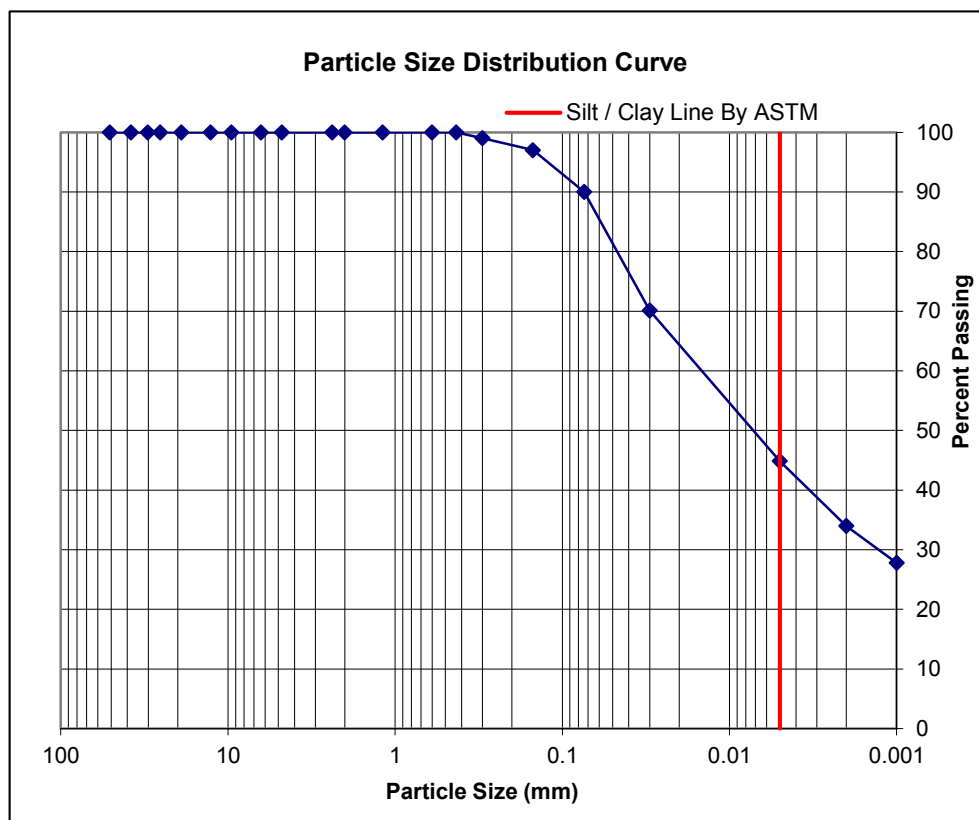
SPECIFIC GRAVITY OF SOLIDS: 2.650 Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0470	0.0304	0.0181	0.0131	0.0094	0.0048	0.0020	0.0015
PERCENT SAMPLE TESTED	75.3	70.3	63.2	57.2	53.2	44.2	34.1	31.1
PERCENT TOTAL SAMPLE	75.3	70.3	63.2	57.2	53.2	44.2	34.1	31.1

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	90.0	97.0	99.4	99.6	99.6	99.8	100.0

**FULL SIEVE ANALYSIS
MECHANICAL SIEVE
& HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	100	
3/8 IN	100	
1/4 IN	100	
# 4	100	
# 8	100	
# 10	100	
# 16	100	
# 30	100	
# 40	100	
# 50	99	
# 100	97	
# 200	90	
0.03 mm	70.1	
0.005 mm	44.9	
0.002 mm	34.0	
0.001 mm	27.8	



Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	TP-1	BUCKET	--	--	N Tested	--	0.001	0.015	--	--	90.0	

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

<i>Ninyo & Moore</i>		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

WEIGHT OF SAMPLE DISPERSED: **54.5**
 PERCENT PASSING #10 SIEVE: **99.8**

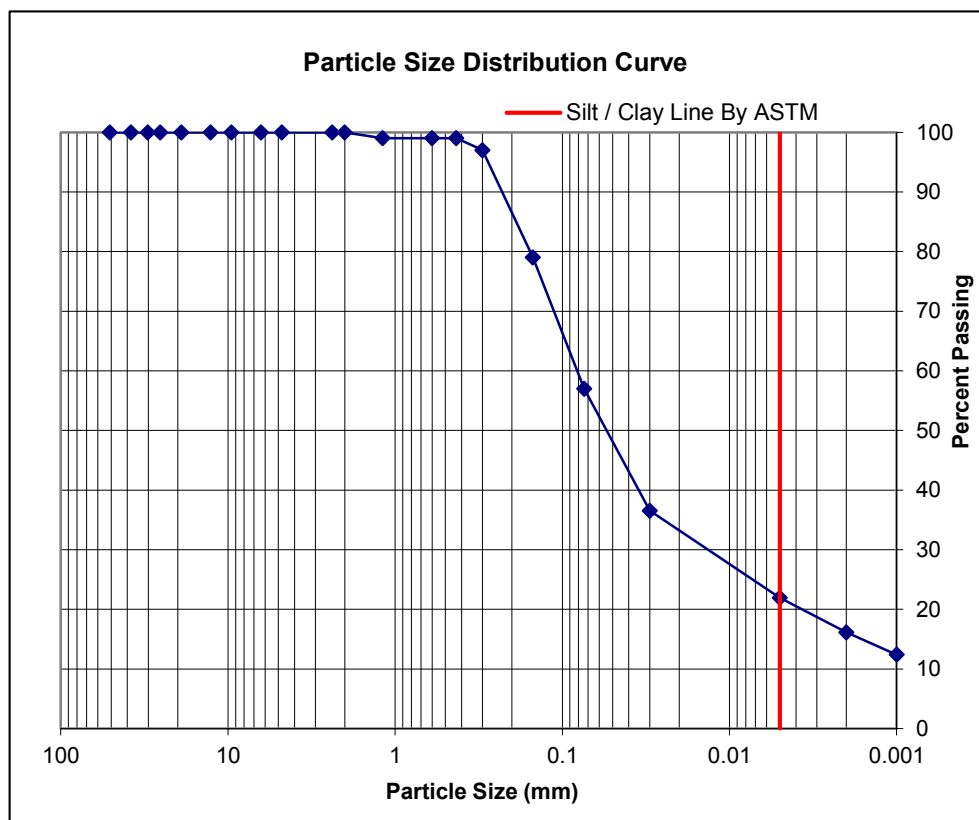
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0531	0.0339	0.0199	0.0142	0.0102	0.0051	0.0021	0.0015
PERCENT SAMPLE TESTED	40.4	37.6	33.0	31.2	27.5	22.0	16.5	14.7
PERCENT TOTAL SAMPLE	40.3	37.5	33.0	31.1	27.5	22.0	16.5	14.6

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	56.8	79.3	96.5	98.5	99.1	99.4	99.8

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	100	
3/8 IN	100	
1/4 IN	100	
# 4	100	
# 8	100	
# 10	100	
# 16	99	
# 30	99	
# 40	99	
# 50	97	
# 100	79	
# 200	57	
0.03 mm	36.5	
0.005 mm	21.9	
0.002 mm	16.1	
0.001 mm	12.4	

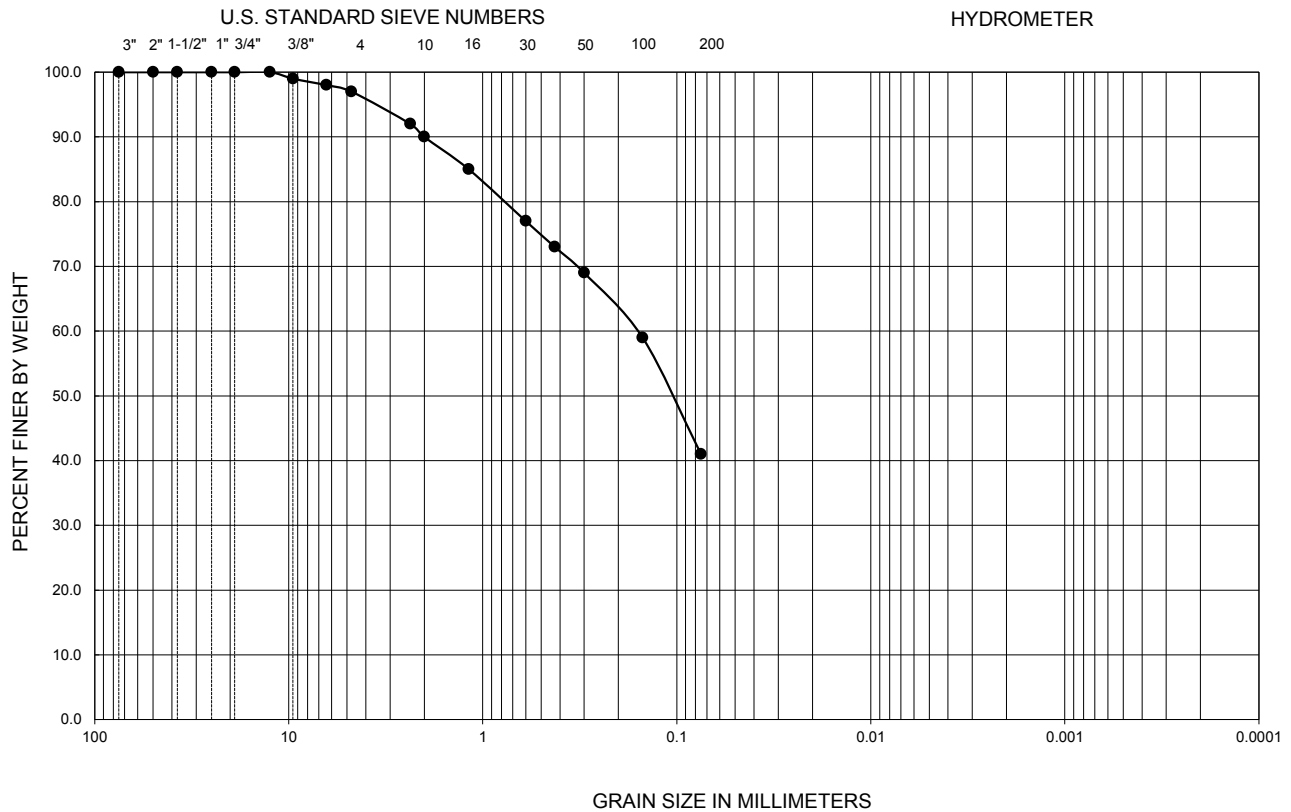


Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	TP-2	BUCKET	--	--	N Tested	--	0.013	0.081	--	--	57.0	

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

<i>Ninyo & Moore</i>		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

GRAVEL		SAND			FINES	
Coarse	Fine	Coarse	Medium	Fine	SILT	CLAY



Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
●	TP-3	BUCKET	--	--	NT	--	--	0.16	--	--	41.0	

PERFORMED IN GENERAL ACCORDANCE WITH ASTM C136 AND C117

<i>Ninyo & Moore</i>		GRADATION TEST RESULTS	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING	
604667003	3/17	PHOENIX, ARIZONA	

WEIGHT OF SAMPLE DISPERSED: **51.8**
 PERCENT PASSING #10 SIEVE: **94.9**

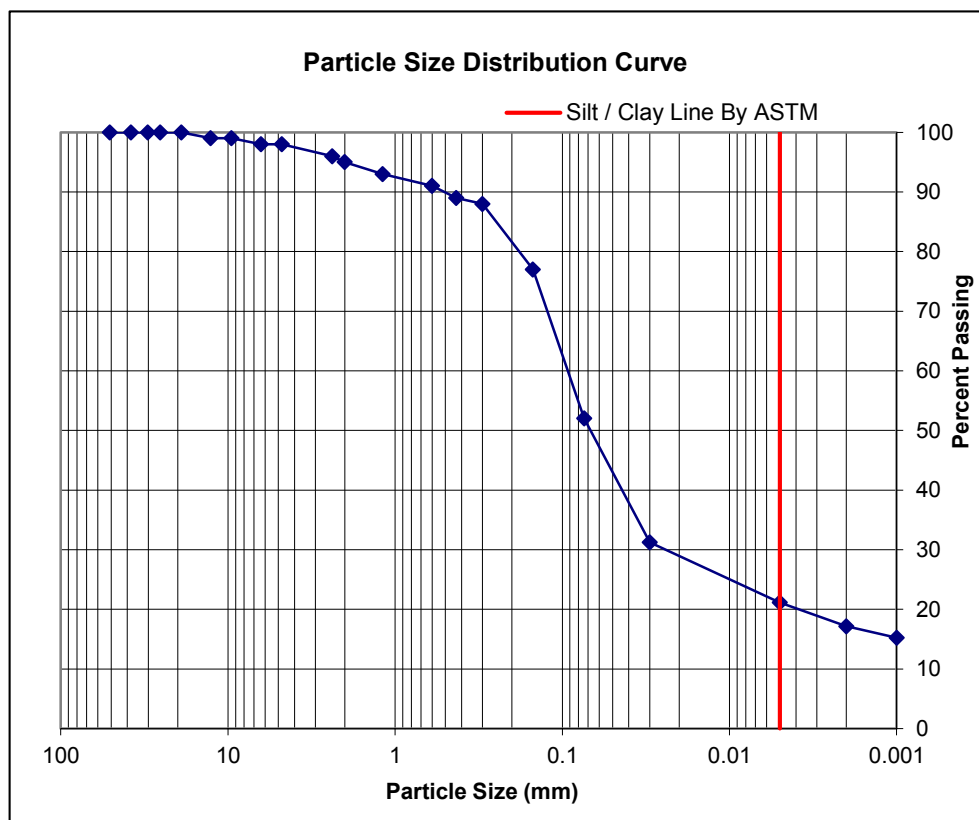
SPECIFIC GRAVITY OF SOLIDS: **2.650** Assumed

	HYDROMETER RESULTS (% PASSING)							
PARTICLE SIZE (DIA. mm)	0.0544	0.0347	0.0204	0.0145	0.0103	0.0051	0.0022	0.0015
PERCENT SAMPLE TESTED	35.7	32.8	27.0	27.0	24.1	21.2	17.4	16.4
PERCENT TOTAL SAMPLE	33.9	31.1	25.6	25.6	22.9	20.1	16.5	15.6

	MECHANICAL SIEVE ANALYSIS AFTER HYDROMETER (% PASSING)						
SCREEN SIZE	#200	#100	#50	#40	#30	#16	#10
PERCENT TOTAL SAMPLE	51.7	76.8	87.7	89.4	91.0	92.9	94.9

**FULL SIEVE ANALYSIS
 MECHANICAL SIEVE
 & HYDROMETER**

	% Pass	Spec
2 IN	100	
1 1/2 IN	100	
1 1/4 IN	100	
1 IN	100	
3/4 IN	100	
1/2 IN	99	
3/8 IN	99	
1/4 IN	98	
# 4	98	
# 8	96	
# 10	95	
# 16	93	
# 30	91	
# 40	89	
# 50	88	
# 100	77	
# 200	52	
0.03 mm	31.2	
0.005 mm	21.1	
0.002 mm	17.2	
0.001 mm	15.2	



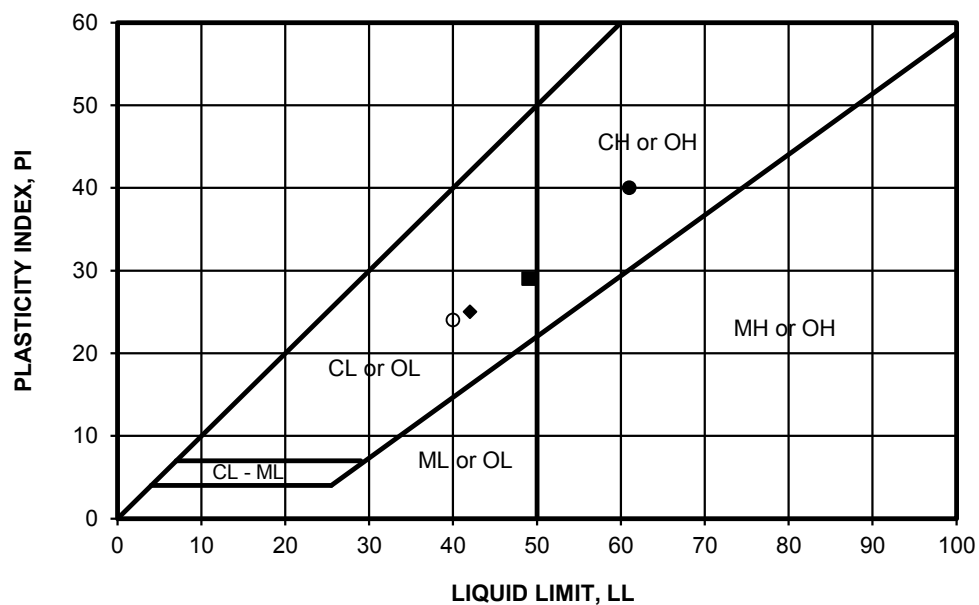
Symbol	Sample Location	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	D ₁₀	D ₃₀	D ₆₀	C _u	C _c	Passing No. 200 (%)	USCS
	TP-4	BUCKET	--	--	N Tested	--	0.024	0.093	--	--	52.0	

PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 422

<i>Ninyo & Moore</i>		PARTICLE-SIZE ANALYSIS OF SOILS (ASTM D422)	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		

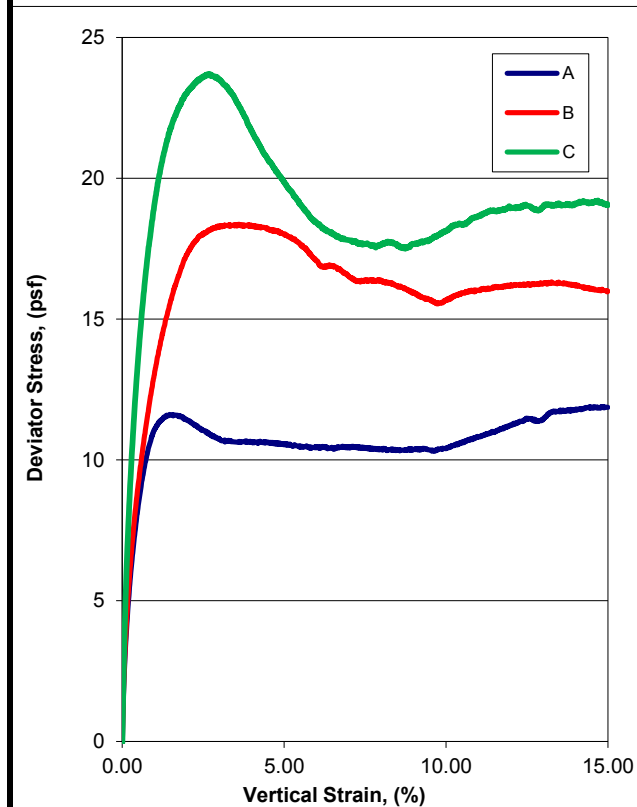
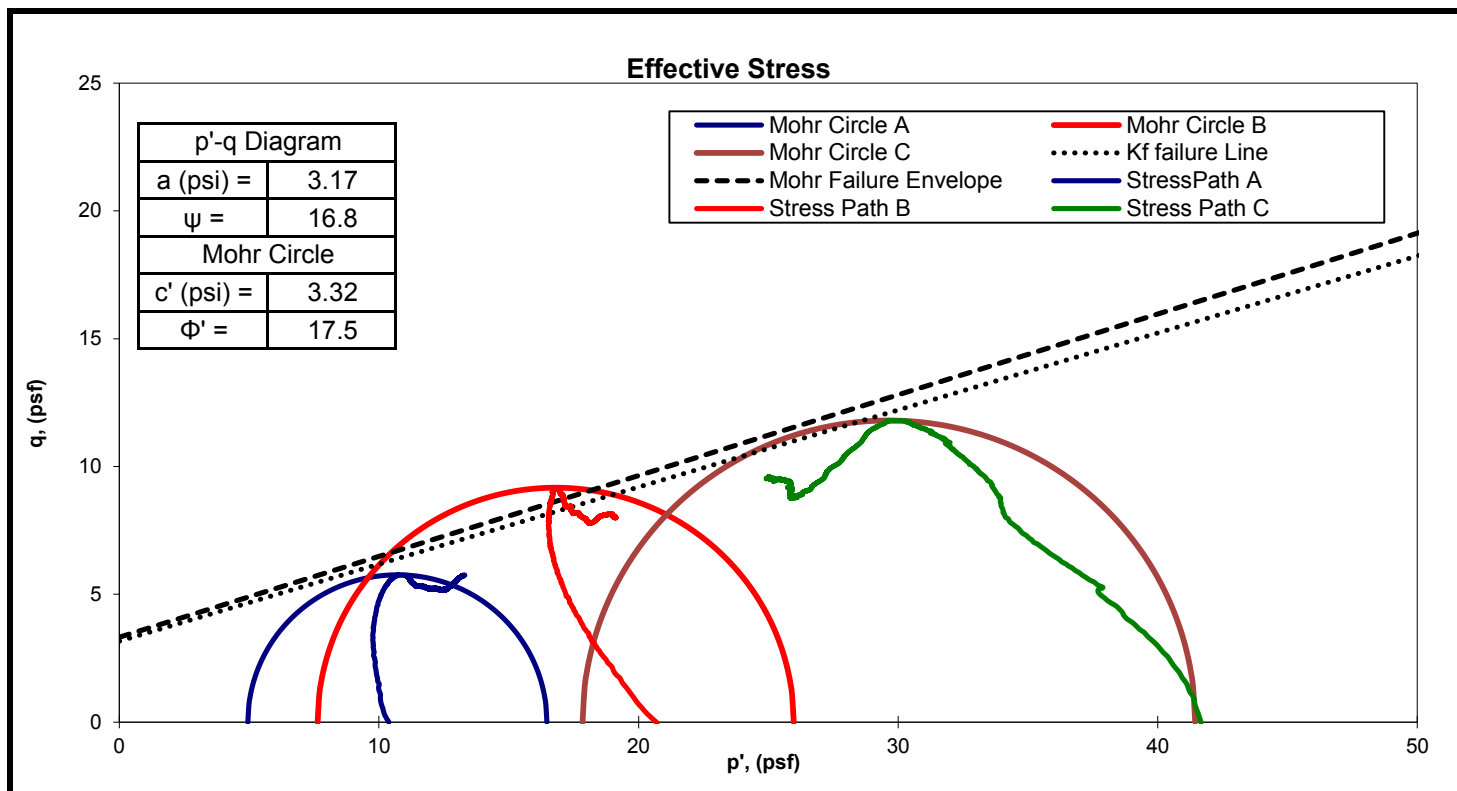
SYMBOL	LOCATION	DEPTH (FT)	LIQUID LIMIT, LL	PLASTIC LIMIT, PL	PLASTICITY INDEX, PI	USCS CLASSIFICATION (Fraction Finer Than No. 40 Sieve)	USCS (Entire Sample)
●	B-5	TW 25-27	61	21	40	CH	CH
■	B-5	40.5-41.0	49	20	29	CL	CL
◆	B-6	15.0-16.0	42	17	25	CL	CL
○	B7	30.5-31.0	40	16	24	CL	CL

NP - INDICATES NON-PLASTIC



PERFORMED IN GENERAL ACCORDANCE WITH ASTM D 4318

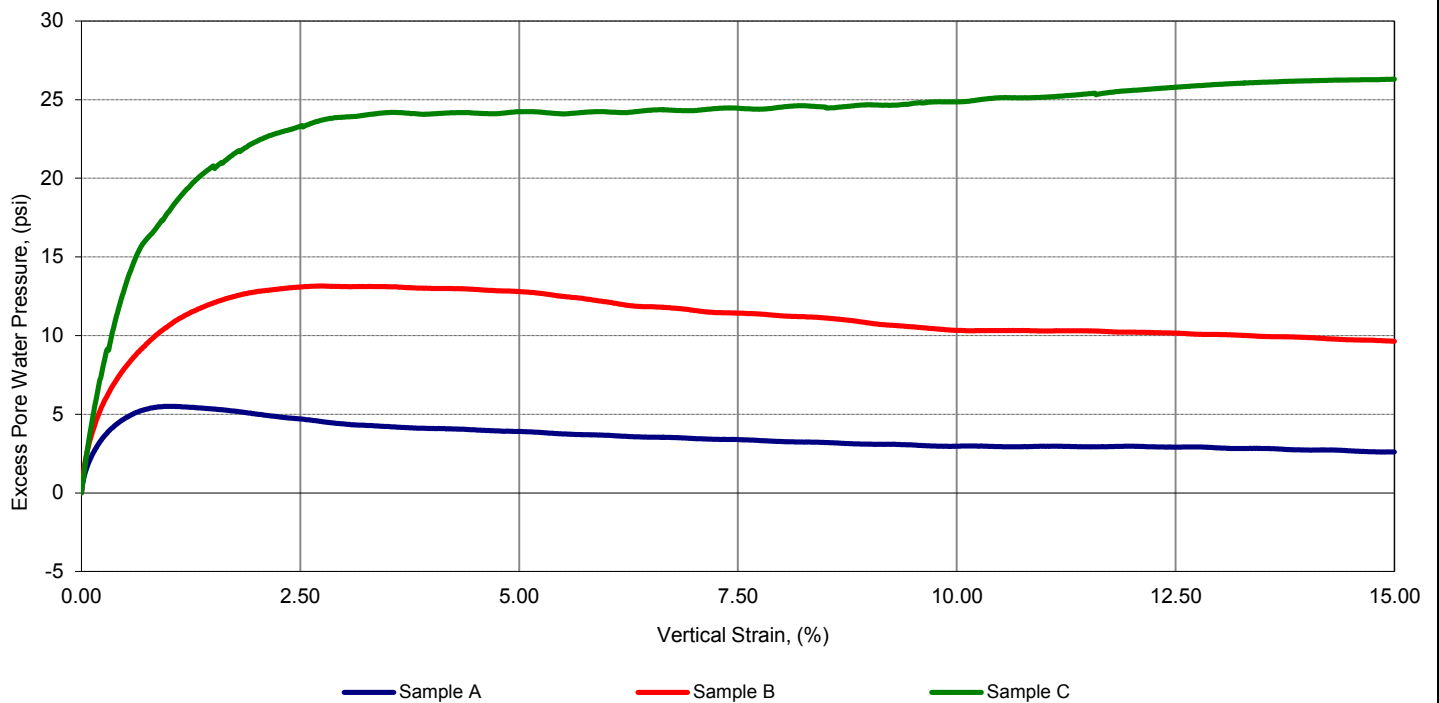
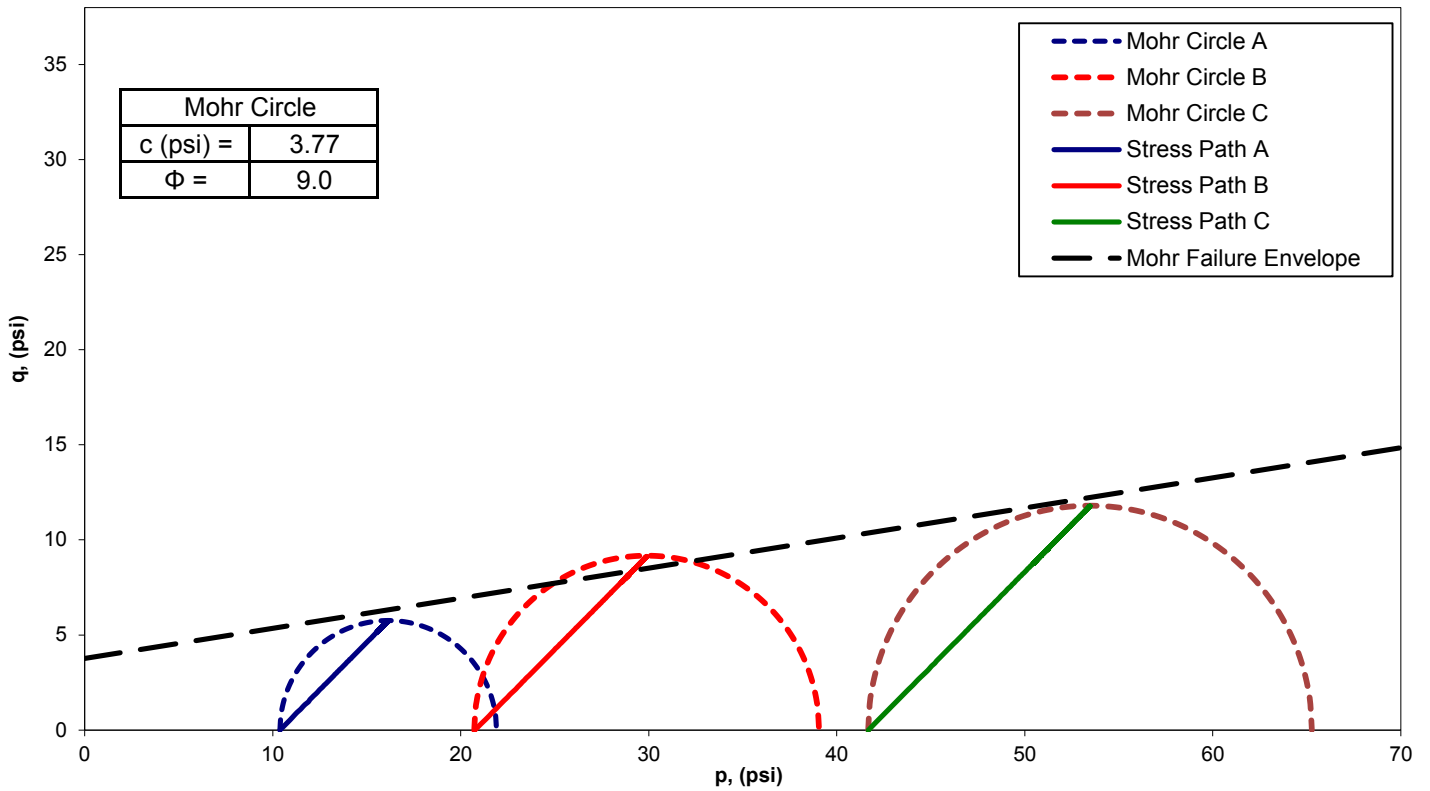
<i>Ninyo & Moore</i>		ATTERBERG LIMITS TEST RESULTS	FIGURE
PROJECT NO.	DATE	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
604667003	3/17		



Location:		B-5		
Sample Depth:		TW 25.0-27.0		
Lab Technician:		JCE		
Checked By:		HJG		
Sample ID		A	B	C
Date Tested		2/25/2017	2/25/2017	2/25/2017
Initial	Diameter, in	2.88	2.88	2.87
	Height, in	5.68	5.71	5.68
	Water Content %	21.3%	21.5%	20.8%
	Dry Density, pcf	99.1	98.3	99.0
	Saturation, %	82.3%	81.2%	80.0%
Before Shear	Void Ratio	0.700	0.714	0.701
	Water Content %	26.1%	25.9%	25.0%
	Dry Density, pcf	96.8	98.5	98.4
	Saturation, %	100.0%	100.0%	100.0%
	Void Ratio	0.741	0.710	0.712
Back pressure, psf		50	50	50
Obliquity Effective Failure %		1.28	3.32	2.89
Obliquity Total Failure %		1.28	3.32	2.89
Effective Confinement, psi		10.4	20.7	41.7
B- Value		0.95	0.95	0.95
Strain Rate, %/min		0.05	0.05	0.05

Ninyo & Moore		Consolidated Undrained Triaxial Test Data Sheet	Figure
Project Number	Date		
604667003	3/17	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	

Total Stress



Ninyo & Moore

Location: B-5

Sample Depth (ft): TW 25.0-27.0

Project Number: 604667003

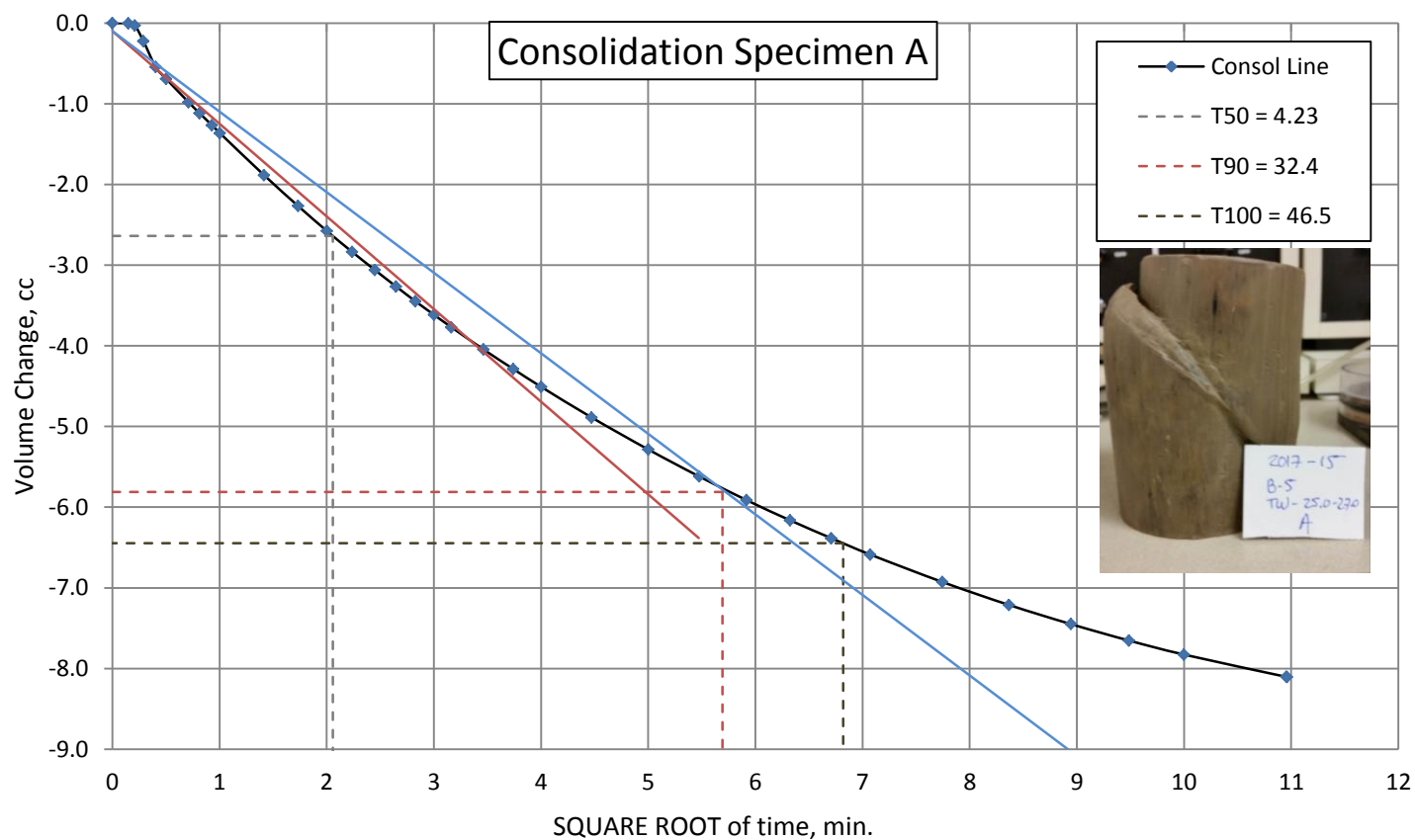
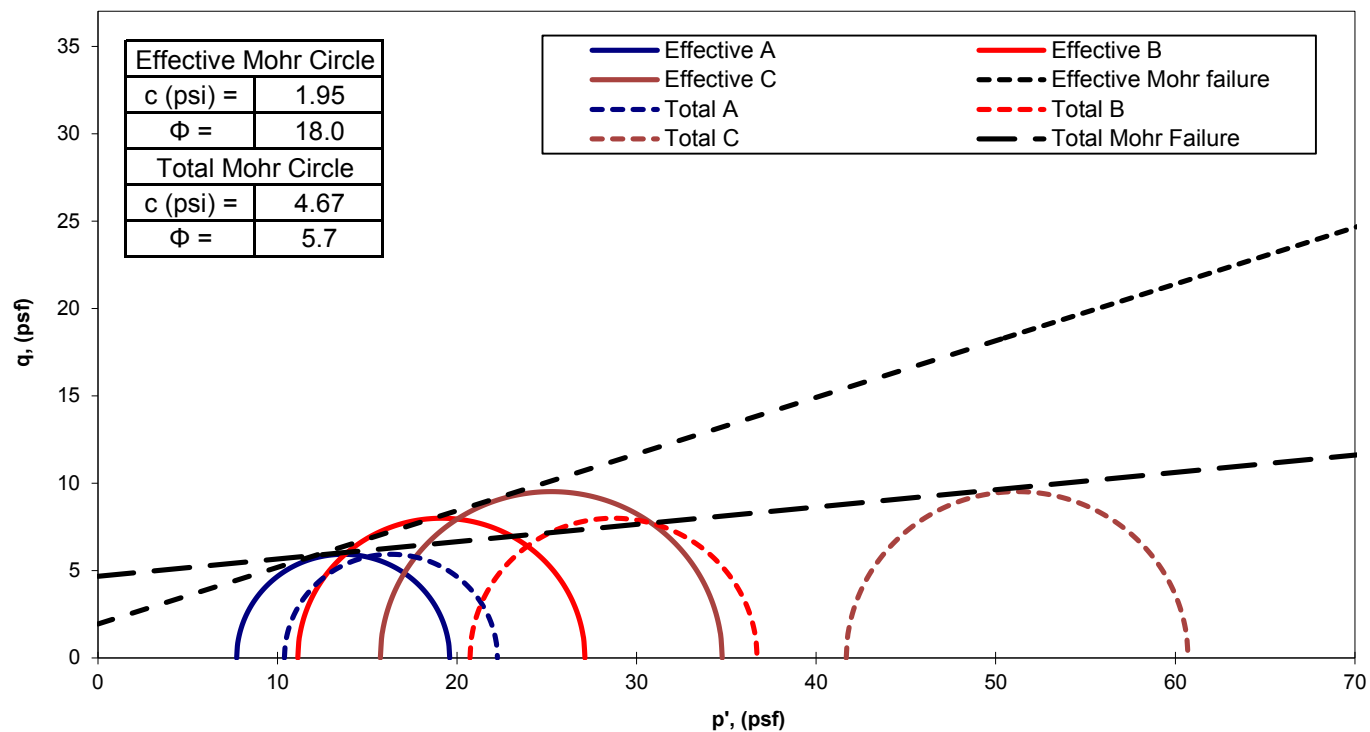
Date: 3/17

Consolidated Undrained Triaxial Test Data Sheet

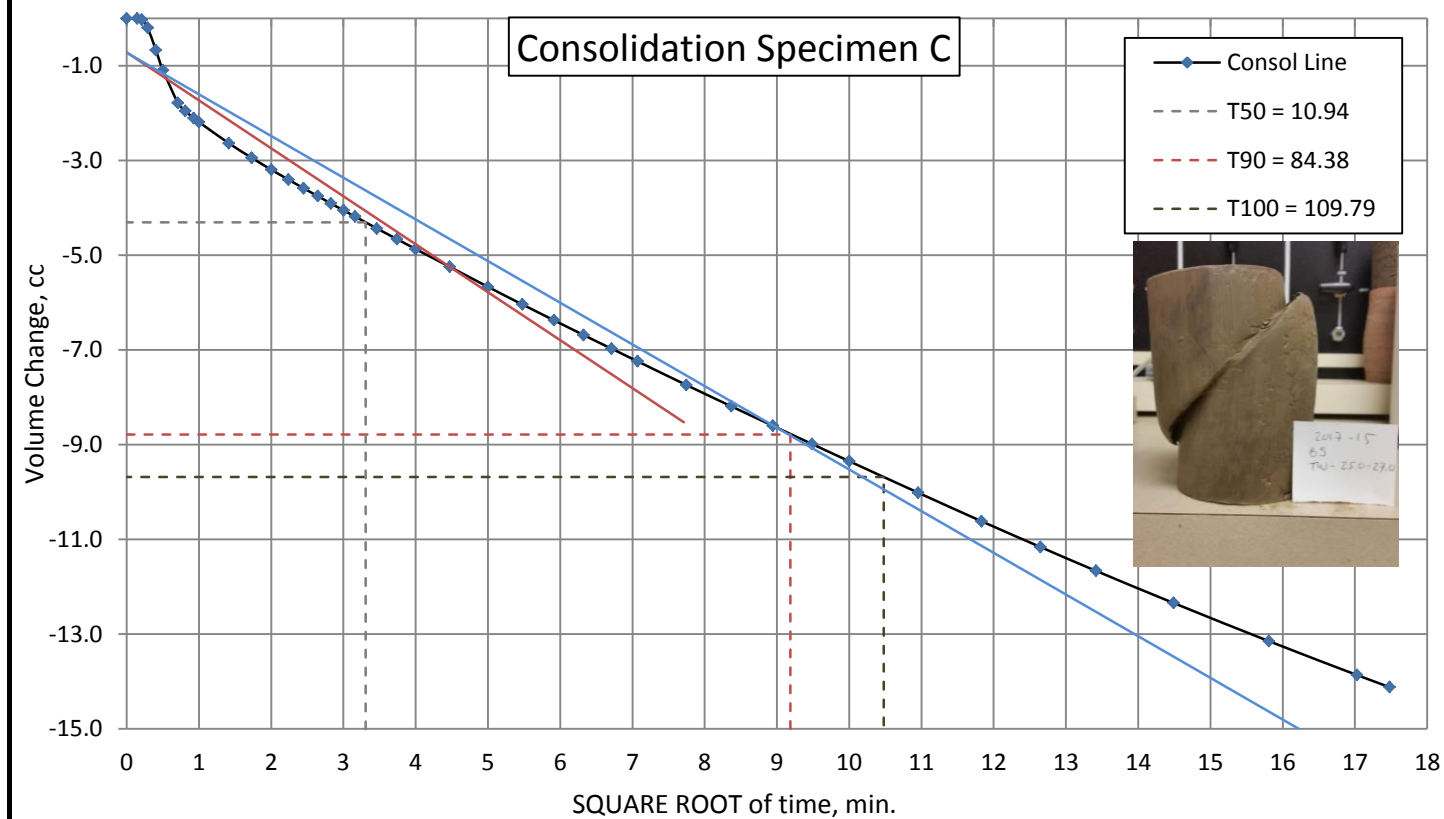
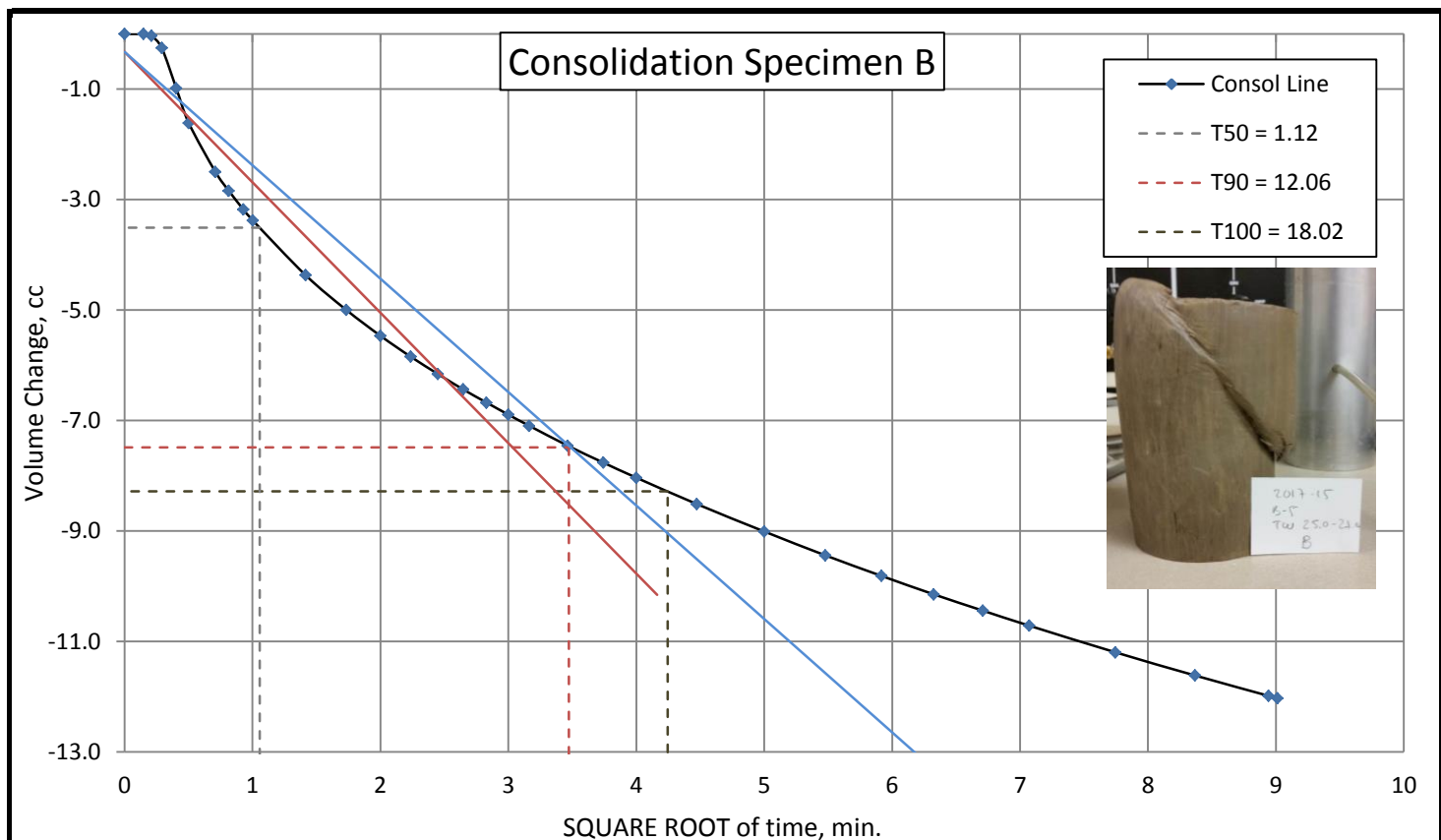
STANTEC/MWH/LAB TESTING
PHOENIX, ARIZONA

Figure

Effective & Total Stress at 15% Shear



<i>Ninyo & Moore</i>		Consolidated Undrained Triaxial Test Data Sheet	Figure
Location:	B-5		
Sample Depth (ft):	TW 25.0-27.0	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
Project Number:	604667003		
Date:	3/17		



Ninyo & Moore		Consolidated Undrained Triaxial Test Data Sheet	Figure
Location:	B-5		
Sample Depth (ft):	TW 25.0-27.0	STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	
Project Number:	604667003		
Date:	3/17		

SAMPLE LOCATION	COMPRESSION STRENGTH Lb/ft2	VOLUMETRIC DENSITY pcf	SPECIFIC GRAVITY	ABSORPTION %	INITIAL MOISTURE %	SULFATE SOUNDNESS % LOSS
B-1 5.3-9.5	1210	117.4	1.871	10.8	5.6	100.0
B-1 17.3-18.0	2560	135.3	2.096	7.4	2.8	81.8
B-2 26.45-27.25	2490	129.1	2.006	8.1	10.3	88.6
B-3 24.2-24.9	1390	115.6	1.937	9.3	3.9	87.7
B-3 24.9-25.6	1230	117.1	1.920	9.7	3.2	100.0

<i>Ninyo & Moore</i>		COMPRESSIVE STRENGTH OF SOIL SPECIMENS	FIGURE
PROJECT NO.	DATE		
604667003	3/17		
		STANTEC/MWH/LAB TESTING PHOENIX, ARIZONA	

**GEOTECHNICAL EVALUATION
CHURCH ROCK MILL SITE JETTY**

Appendix D Petrographic Analysis for rirpap sources
April 24, 2017

APPENDIX D PETROGRAPHIC ANALYSIS FOR RIRPAP SOURCES

To:	UNC/GE	From:	Stantec Consulting Services Inc.
File:	NECR Removal Action 60% Design Petrographic Analysis	Date:	April 24, 2017

Introduction

This memorandum has been prepared to assess the suitability of three rock samples for durability and long-term weathering for the project. The three samples analyzed were an igneous rock, a carbonate, and a sandstone. The igneous rock and the carbonate are from offsite quarries. The sandstone is from a rock core sample collected near the buried rock jetty area. The conclusions presented in this document are the results of hand sample and microscopic examination of thin sections analyses of the three samples. Two pieces of the each rock were chosen for thin sections (2 cm x 4 cm) and studied under the petrographic microscope.

Summary

Sandstone – Sampled from the UNC Mill Site

Modal analyses showed that the sandstone is a quartz arenite according to the classification of Folk (1974). The original sample was collected from 2-inch diameter core. The sandstone is fine (0.1mm) to medium (0.25 mm) grained, moderately well sorted, and loosely packed. The sandstone has a weak structure due to the fact that most quartz grains have poorly developed secondary quartz overgrowths, with additional weak cementation resulting from iron oxide and clay formation in the pore spaces.

The bulk of the sandstone is primarily composed of quartz. Subordinate constituents are iron oxides, clays, and rare calcite minerals. Abundant porosity results from the point-to-point grain contacts throughout the sample. These pores appear to be well connected in the two dimensional framework of the thin section, and are probably commonly interconnected in three dimensions. Unlike typical quartz arenites, secondary quartz overgrowths are rare, resulting in a highly friable rock with little competency.

Carbonate (Tampico Limestone)

The limestone sample contains predominately calcite with notable amounts of dolomite. The original rock fragment was approximately 20 cm in diameter and had a pink to rose color. The sample composed of interlocking very fine (0.05 mm) to medium (0.25 mm) grained carbonate crystals, numerous fractures and vugs are filled with large grained recrystallized calcite. The pink coloration is a result of trace amount of iron oxide within the limestone.

With the exception of trace amounts of iron oxide the sample contained only massively interlocking carbonate (calcite and dolomite) grains. The natural filled fractures do not provide

structural weakness or a natural flowpath. Rare, isolated, small (0.1mm) pore spaces were observed; however no pore spaces were noted to be connecting, suggesting that the rock has very low effective porosity.

Igneous Rock (Page Granite)

Modal mineral analysis of the igneous sample showed that it is a granite. The original rock fragment was approximately 20 cm in diameter and had a dark pink to red color. The sample composed of fine (0.1 mm) to medium (0.25 mm) grained interlocking crystals. In decreasing abundance the granite contained quartz, orthoclase, plagioclase, hornblende, mica, and hematite. The red coloration results from the abundance of orthoclase (which is pink) and trace amount of iron oxide observed staining some of the grain boundaries. Rare, isolated, small (0.1mm) pore spaces were observed; however, no pore spaces were noted to be connecting, suggesting that the rock has very low effective porosity.

Conclusions

Sandstone

The sandstone is friable and not structurally competent from a mineralogical perspective. The sandstone is judged to be poor material for riprap, equal to a clay cemented sandstone. Figure 1 presents a subjective assessment of the suitability of the sandstone for riprap material. This sandstone is considered to be approximately 3 in terms of its suitability for riprap material.

Limestone

With the exception of the potential for the limestone to react (and dissolve) under acidic conditions, the limestone sample would make good rip-rap material, slightly less competent than a quartzite. The absence of fractures or effective porosity suggests that freeze/thaw will not cause fracturing of the rock. Because of its potential reactivity, this limestone is subjectively judged to be approximately 7 in terms of its suitability for riprap material (Figure 1).

Granite

The granite is very competent and is judged to be excellent material for riprap. This granite is considered to be 10 in terms of its suitability for riprap material. It is considered to be a "10" because of the low porosity (both primary and secondary), the interlocking nature of the grains, lack of sulfide minerals (Figure 1).

Methods

The samples to be examined were chosen by the petrographer and then cut with a diamond saw to expose fresh, flat surfaces. A rectangular billet was cut out of each of the samples with a diamond saw and mounted on a glass slide. The mounted rocks were then ground to about 30 microns on a lapidary wheel; final polishing was completed with a fine lapidary lap by an automatic procedure and then by hand. The thin sections were impregnated with blue-epoxy

in a heated laboratory vacuum oven. Following evacuation, atmospheric pressure was used to force the blue epoxy into the pores of the samples.

Thin sections were examined using a Leica Ortholux II petrographic microscope. Color photomicrography was done using a Leica EC3 automatic digital camera mounted on top of the petrographic microscope. Photomicrographs were calibrated by use of an E. Leitz Wetzlar stage micrometer with a 0.01 mm graduated scale. Point counts of the two stained thin sections were made by inserting the thin section into a thin section holder, which was attached to the microscope stage. The holder was adjusted so that the slide would move in increments adjusted to the grain size (approximately one increment per grain) of the samples so that the modal analyses would be representative of the samples. Minerals at the points were recorded on a Lab-Count Denominator.

Point counts consisted of 300 points per thin section. Folk (1974) recommended for purposes of statistical accuracy that at least 300 points are necessary to reflect the mineralogy of a sample in a modal analysis.

Holes in sandstone are called "pores". The pores in these are primary pores created by packing of the grains (in the case of the sandstone) or isolated 'vugs' in the granite or limestone sample. These pores are generally no larger than the size of the original feldspar grain or rock fragment (approximately 0.10 to 0.20 mm). Although Folk (1974) recommends that up to 1,000 points in pores be counted per thin section to most accurately assess the amount of porosity, this amount of point-counting is cost prohibitive. Furthermore, when porosity consists of very small moldic pores formed by grain dissolution as in this study, the percentage of porosity acquired from 300 grain pore counts is close enough to the true porosity to be statistically meaningful.

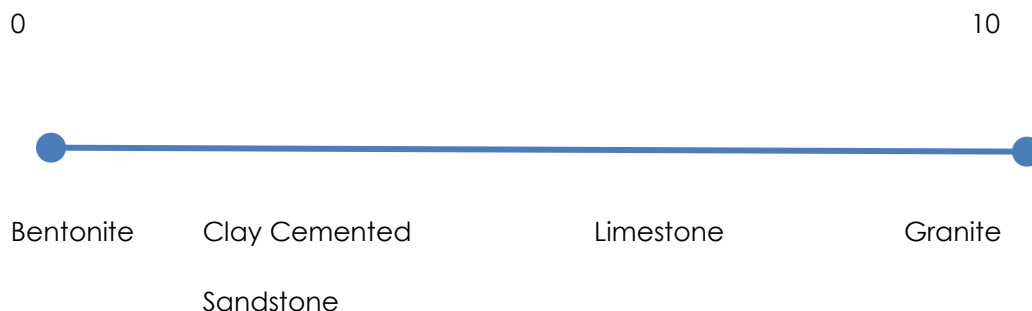


Figure 1. Subjective Estimate of Riprap Suitability

Petrography of the samples

Sample 1. Quartz arenite

Description of sandstone in thin section: Moderately well sorted; poorly-cemented sandstone with predominately 'point-to-point' contacts with cementation dominated by clays and some carbonates. The sample is comprised of mainly quartz with very little altered rock fragments or feldspar; well-developed porosity.

Framework Minerals

Quartz – Fine to medium grained; no quartz overgrowths, long edge to edge contacts rare.

K-feldspar – Angular; very fine grained; one.

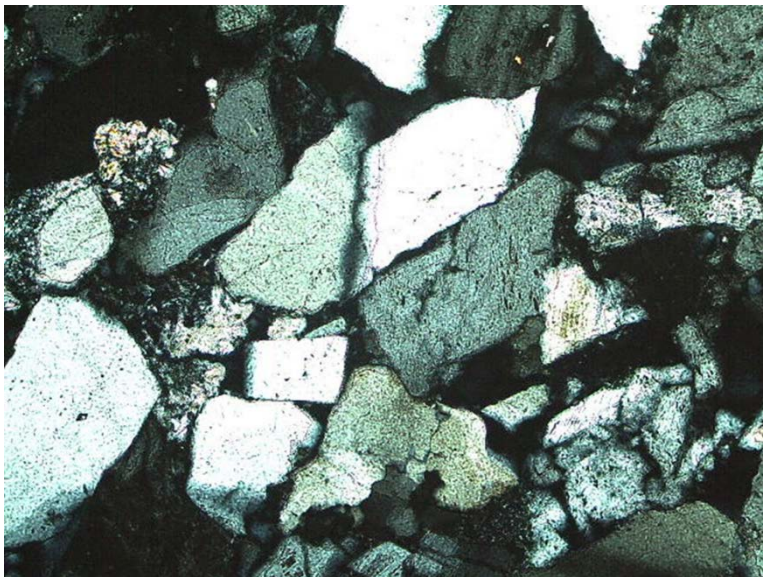
Siltstone rock fragment – Fine grained; subrounded; trace.

Metamorphic rock fragment – Fine grained; mica and quartz; subrounded; trace. Muscovite – 0.1 to 0.5 mm long; rare.

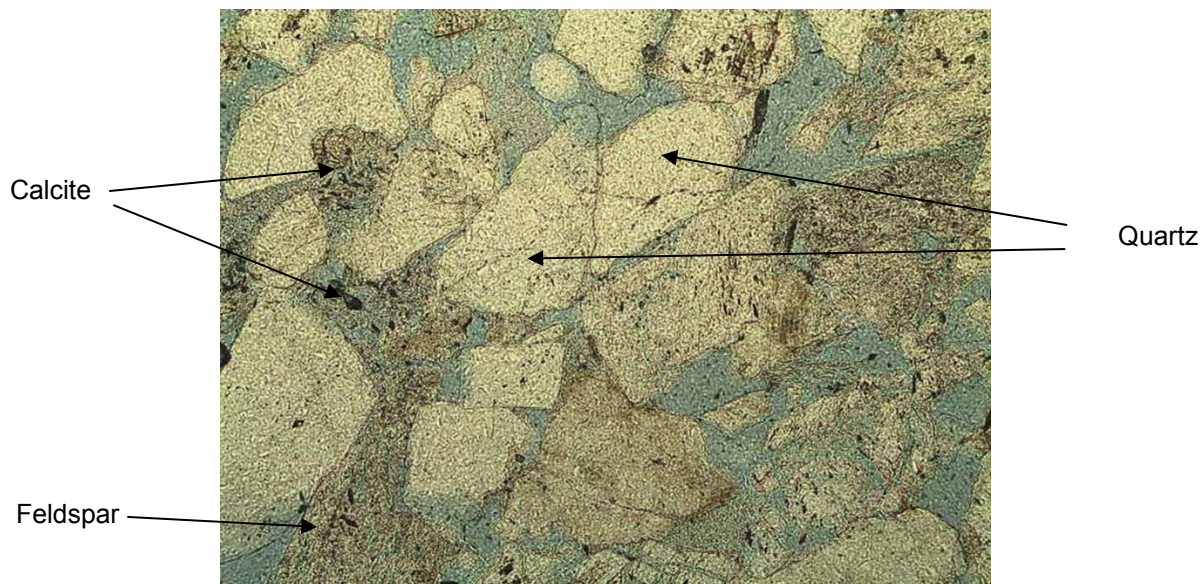
Cement

Clay – Clay minerals unidentifiable in thin-section; iron oxide stained; on edges of vugs and in grains.

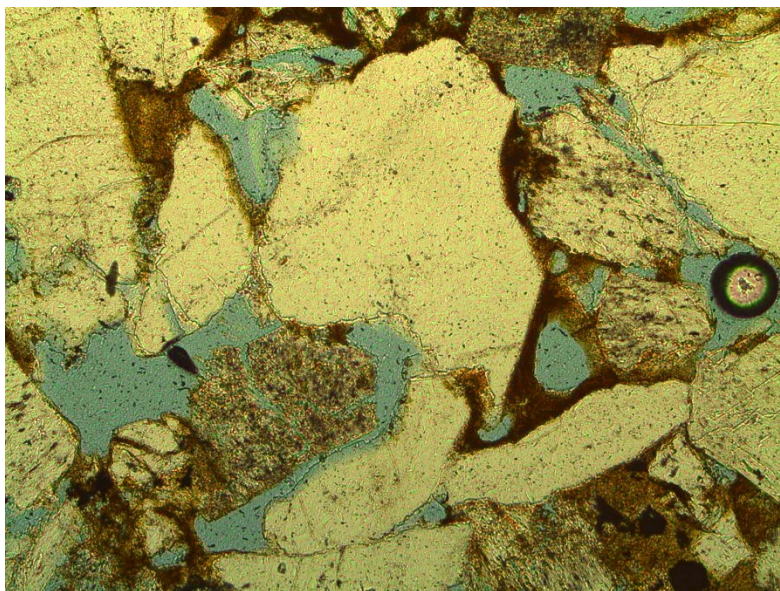
Iron oxides – Orange-red; pore filling; common.



Photograph 1. Grain-supported quartz matrix, feldspar grain and calcite grains. XP, 100x, FL 1.0 mm.



Photograph 2. Grain-supported quartz matrix (same field of view as Figure 1); feldspar grain and calcite grains identified all other grains are quartz. Blue is epoxy showing pore spaces. LP, 100x, FL 1.0 mm.



Photograph 3. Quartz showing abundant porosity and cementation by iron oxides and clays. PL, 100x, FL 1.0 mm.

XP = Crossed polarizers

PL = Polarized light

100x = Magnification
FL = Long dimension of image

Sample 2. Limestone

Description of sandstone in thin section: Very fine to medium grained; angular to subangular; close packed (long highly sutured contacts); carbonate grains are intergrown and locally recrystallized; Rare porosity (<1%).

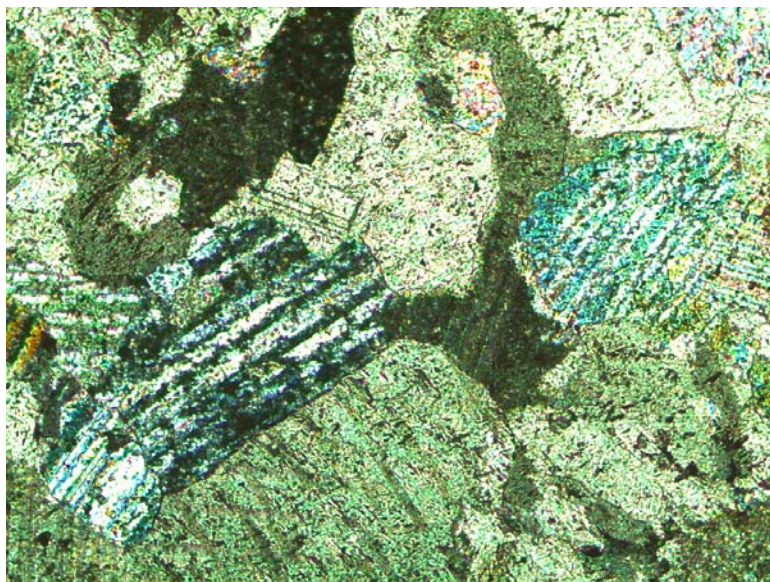
Framework Minerals

Calcite – Occurs in the original rock matrix intergrown with dolomite and recrystallized as fine grained fracture filling.

Dolomite - Occurs in the original rock matrix intergrown with calcite.

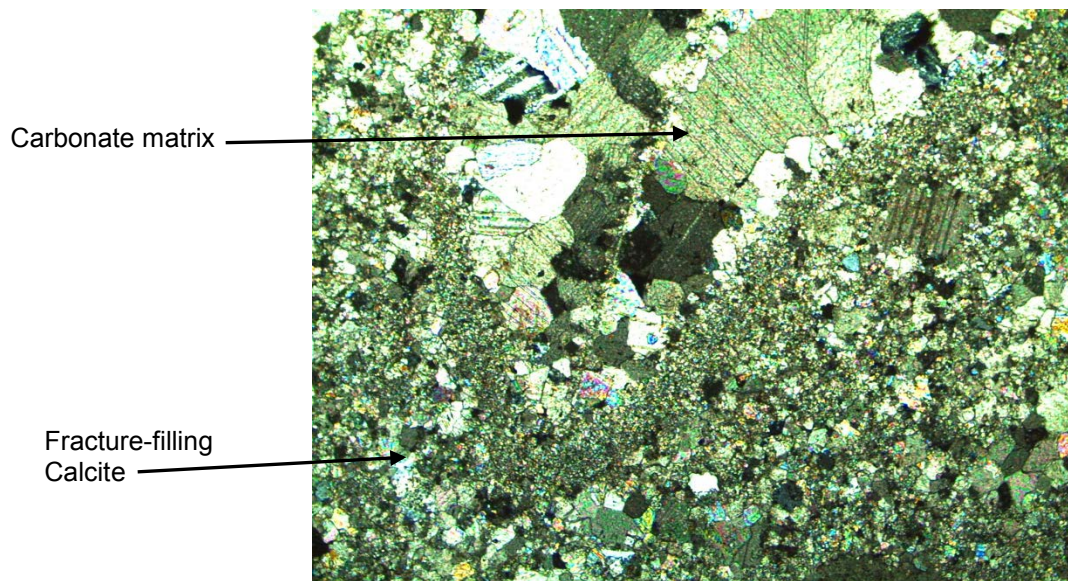
Accessory Minerals

None noted.



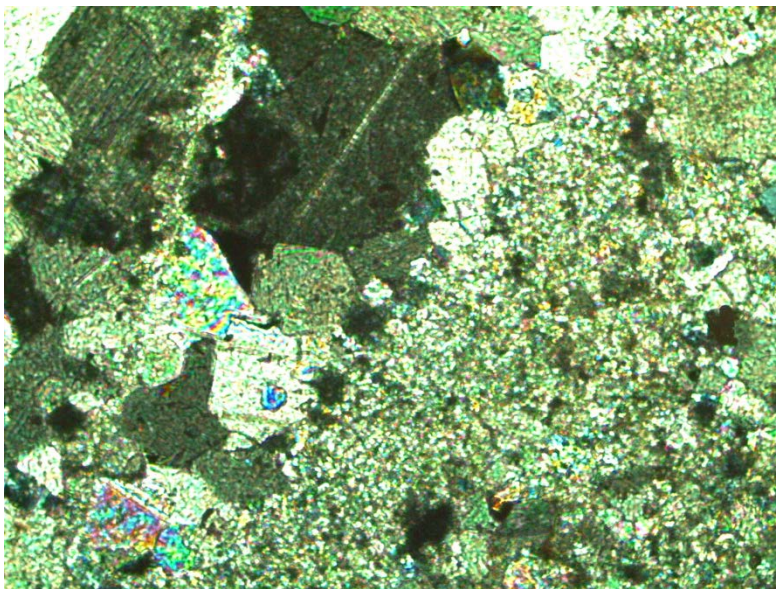
Photograph 4. Interlocking and intergrown carbonate minerals.

XP, 100x, FL 1.0 mm.



Photograph 5. Very fine grained calcite fracture filling.

XP, 40x, FL 2.5 mm.



Photograph 6. Close-up of Figure 5. Note intergrown crystals.

XP, 100x, FL 1.0 mm.

XP = Crossed polarizers

PL = Polarized light

100x = Magnification

FL = Long dimension of image

Sample 3. Granite

Description of sample in thin section: The sample composed of fine (0.1 mm) to medium (0.25 mm) grained interlocking crystals. In decreasing abundance the granite contained quartz, orthoclase, plagioclase, hornblende, mica, and hematite. The red coloration results from the abundance of orthoclase (which is pink) and trace amount of iron oxide observed staining some of the grain boundaries.

Framework Minerals

Quartz – Angular; interlocking with surrounding grains (approximately 60%).

K-feldspar – Untwinned and twinned; angular; fine grained; fresh (approximately 20%).

Plagioclase – Twinned; angular; fine grained; fresh (approximately 15%).

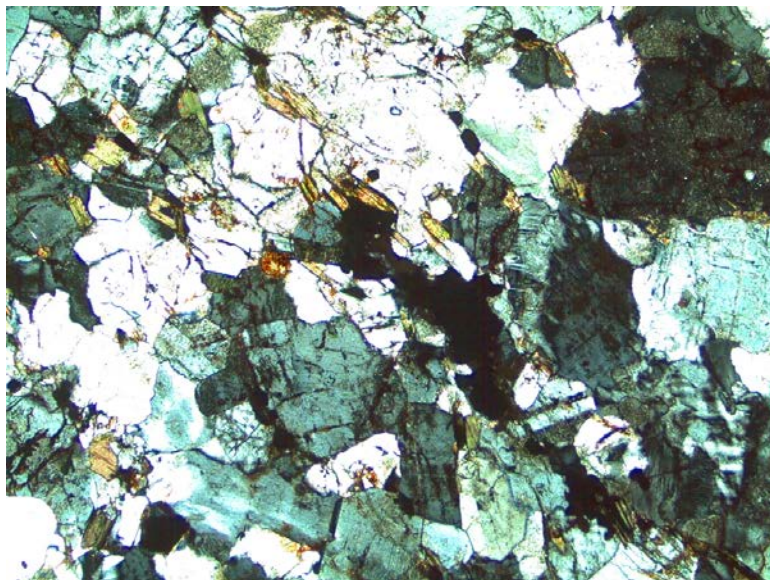
Hornblende – Very fine grained (approximately 3%).

Biotite – some individual grains (approximately 1%).

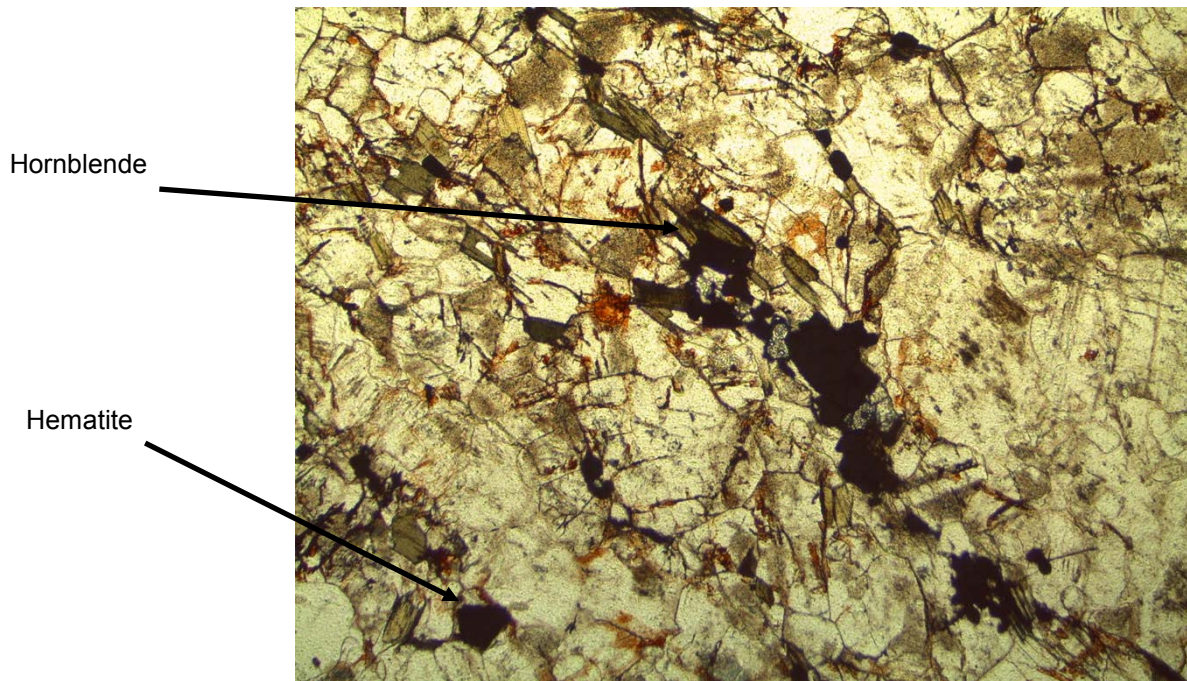
Accessory Minerals

Hematite – Fine-grained (<0.1 mm) opaque angular grains; trace

Iron oxides – Orange-brown; clay size; common.



Photograph 7. Granite, showing interlocking euhedral grains.
XP, 100x, FL 1mm.



Photograph 8. Granite, showing interlocking euhedral grains. Note: Opaque hematite and associated hornblende, PL, 100x, FL 1mm.

XP = Crossed polarizers
PL = Polarized light
100x = Magnification
FL = Long dimension of image

References

Folk, R., L., 1974, Petrology of Sedimentary Rocks. Austin, Texas: Hemphill's Book Store, 182 pp.