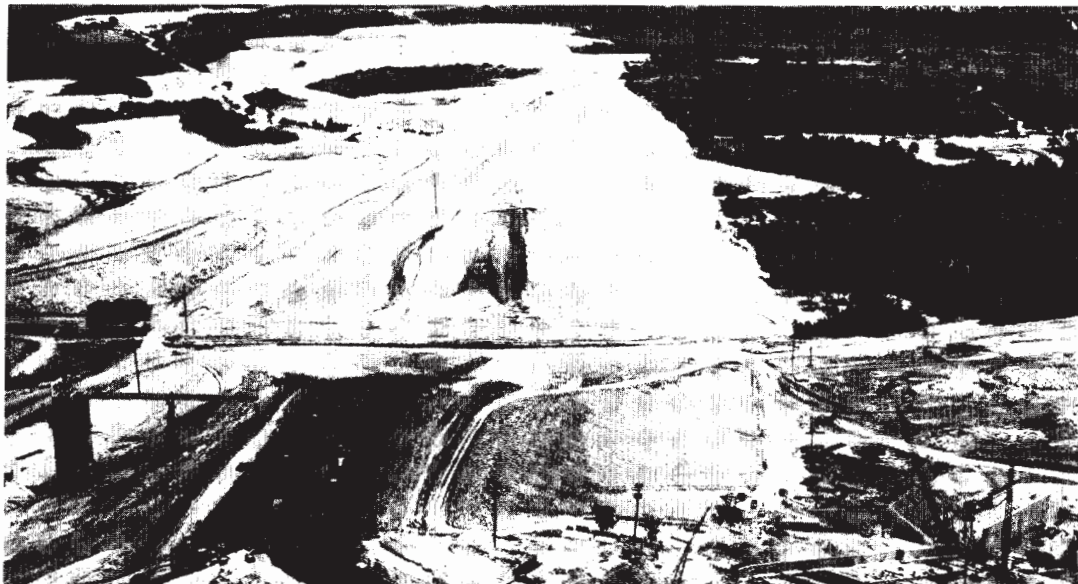


1. SCD Service Spillway Crest and S.W. Abutment Core Trench. View to North (4/26/76).



2. SCD Aerial View of S.W. & N.E. Embankments. View to North (5/19/76).

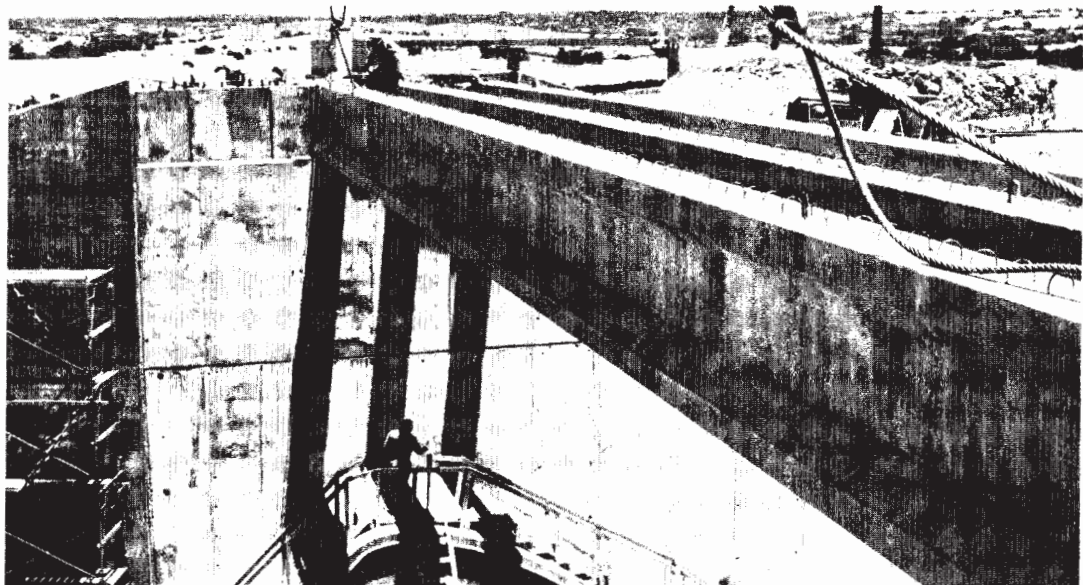
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UNITS 1 and 2

Squaw Creek Dam
Construction Photographs

FIGURE 2.5.6-5G



1. SCD Aerial View of Serv. Spillway. View to West (5/19/76).

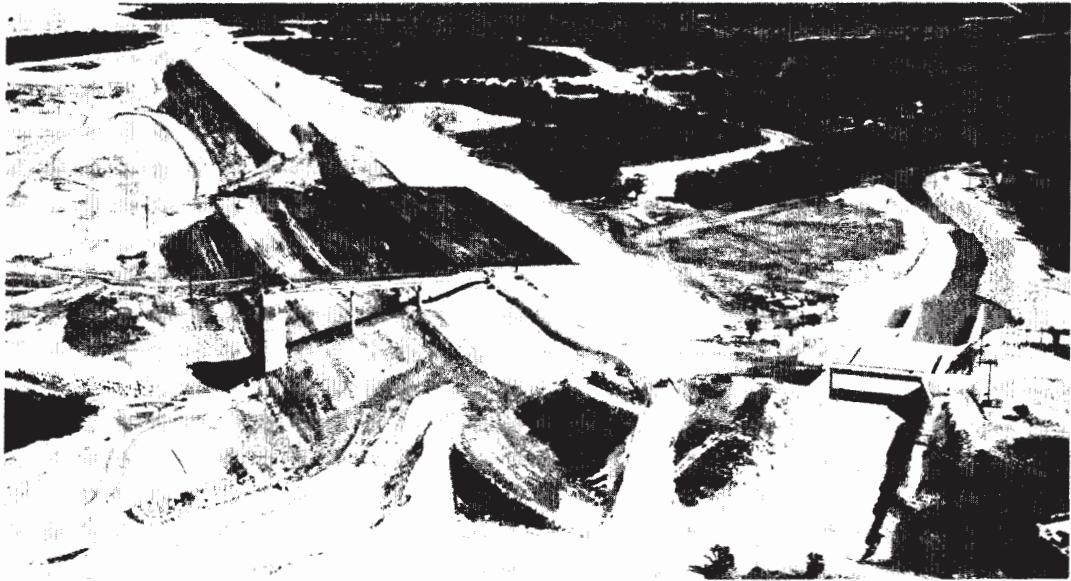


2. SCD Setting Service Spillway Bridge Girders. View to North (6/29/76).

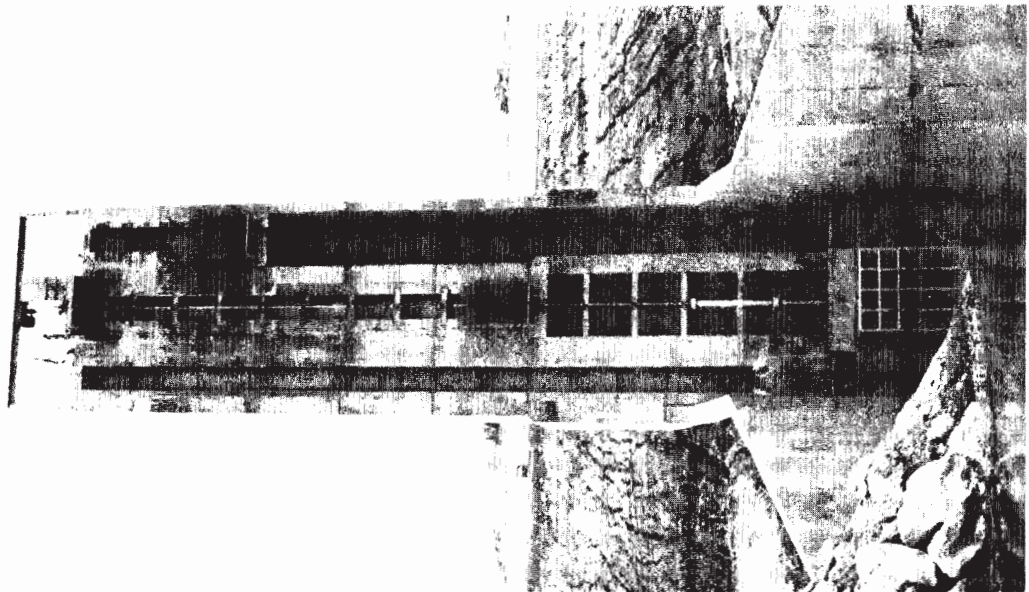
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Construction Photographs

FIGURE 2.5.6-5H



1. SCD Aerial View. View to Northeast (10/20/76).

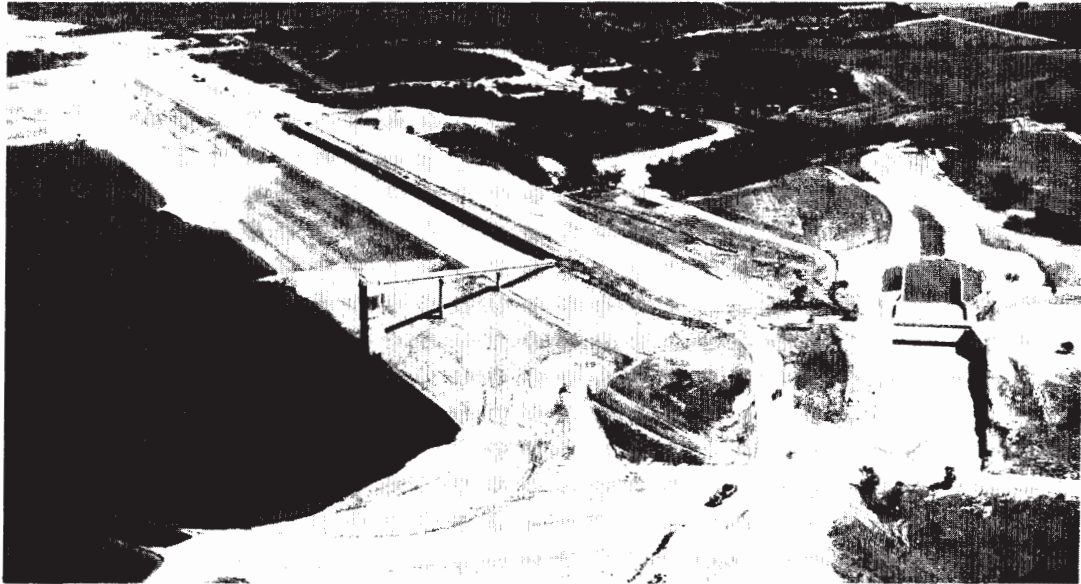


2. SCD Service Outlet Tower on date Water Impoundment Started. View to Southeast (2/15/77).

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Construction Photographs

FIGURE 2.5.6-5I



1. SCD Aerial View. View to East (3/22/77).



2. SCD Aerial View. View to East (4/22/77).

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Squaw Creek Dam.

Construction Photographs

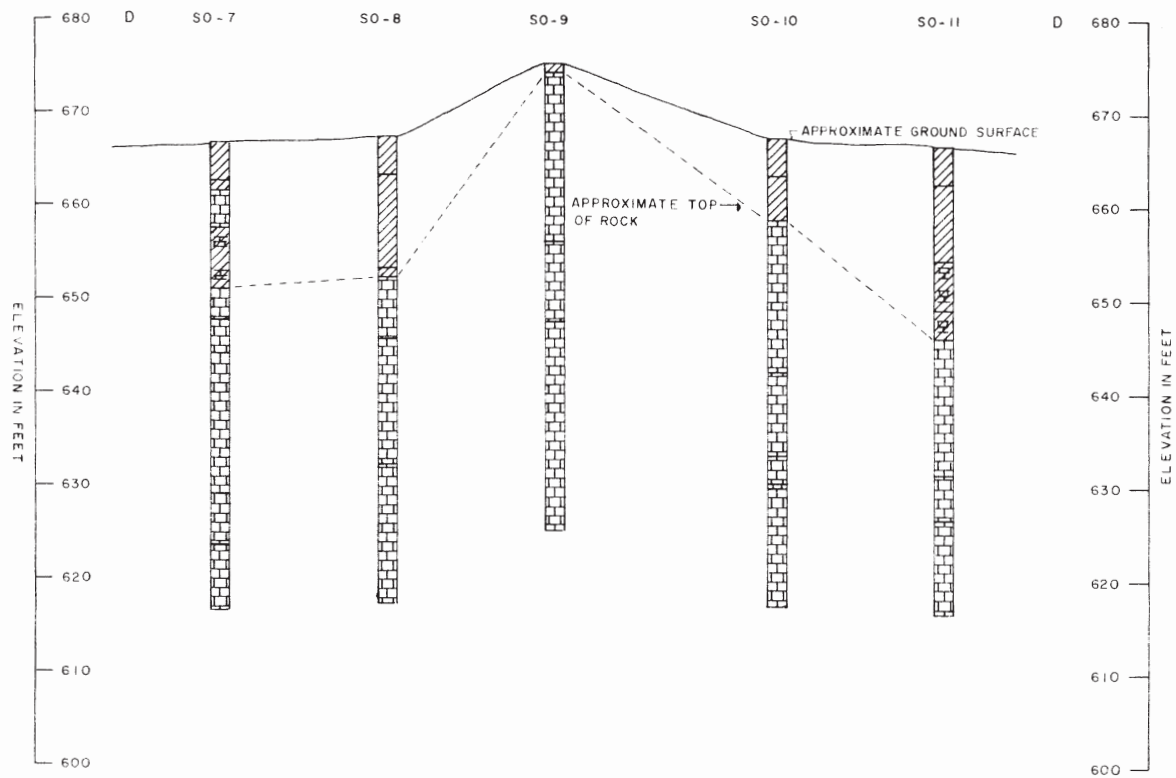
FIGURE 2.5.6-5J



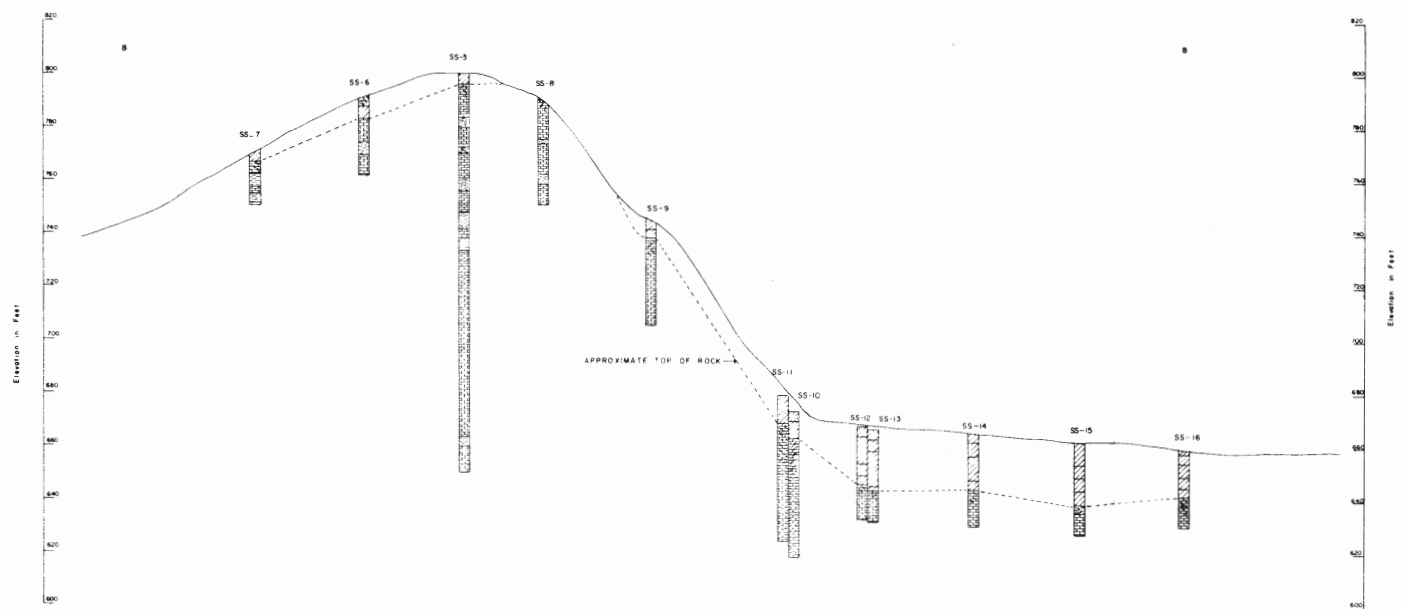
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UNITS 1 and 2

GEOLOGIC PROFILE
CENTERLINE
SQUAW CREEK DAM

FIGURE 2.5.6-7

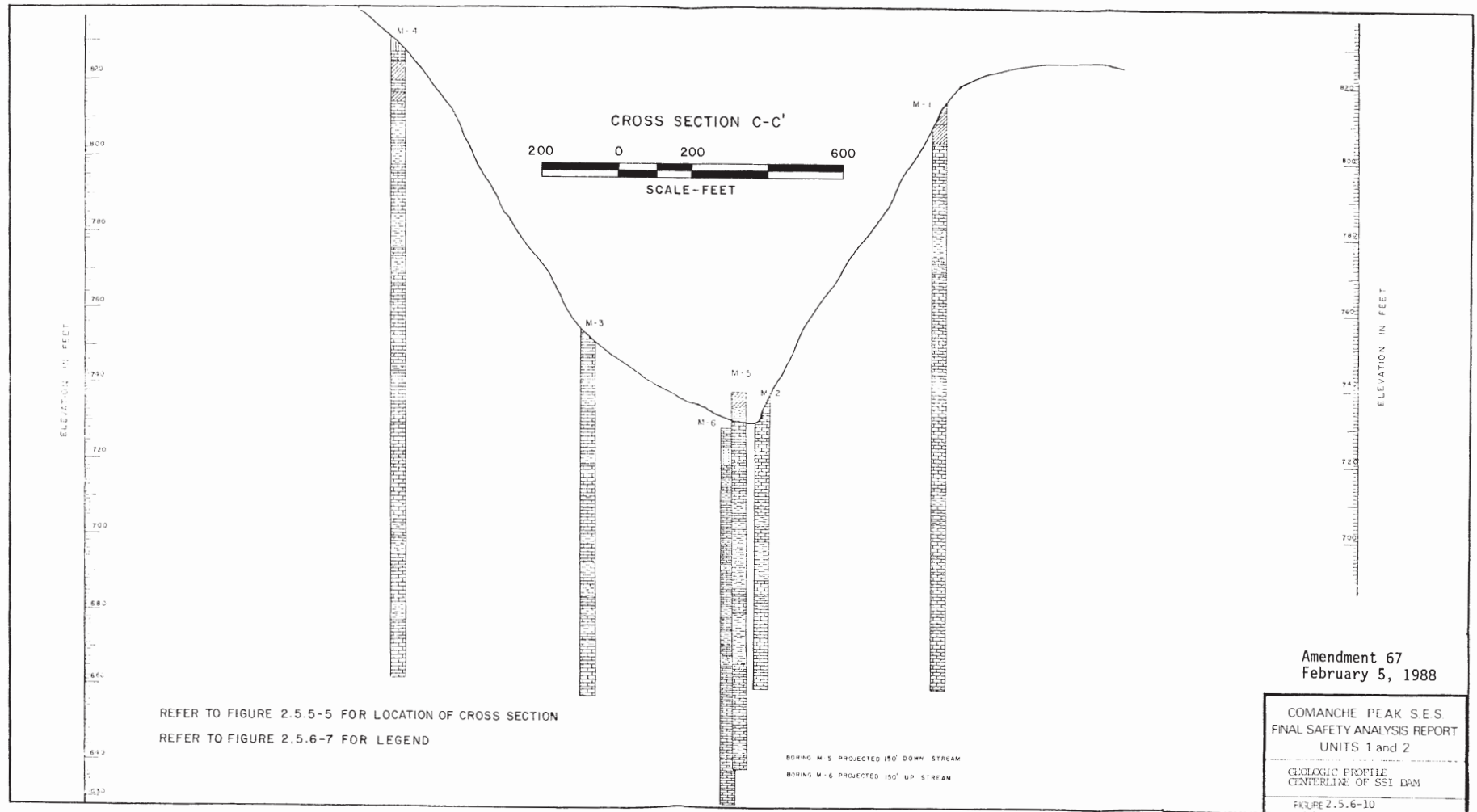


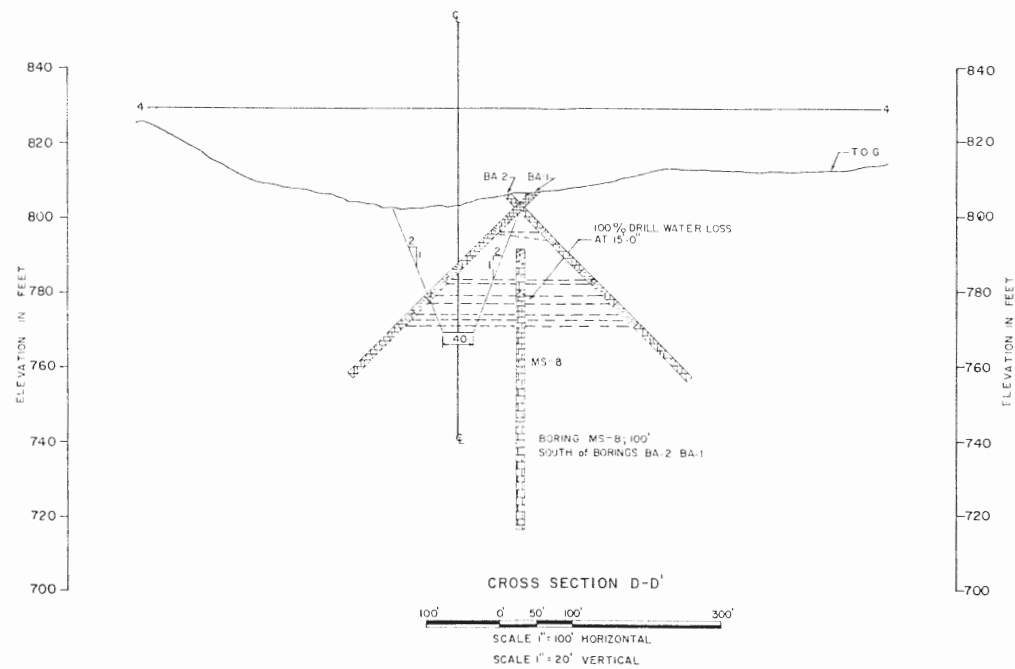
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UNITS 1 and 2
GEOLOGIC PROFILE
SERVICE OUTLET WORKS
FIGURE 2.5.6-8



0' 100' 200' 300' 400'
SCALE

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UNITS 1 and 2
GEOLOGIC PROFILE
SERVICE SPILLWAY
FIGURE 2.5.6-9





REFER TO FIGURE 2.5.5-5 FOR LOCATION OF CROSS SECTION
REFER TO FIGURE 2.5.6-7 FOR LEGEND

Amendment 67
February 5, 1988

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UNITS 1 and 2

GEOLOGIC PROFILE
EQUALIZATION FLOW CHANNEL
FIGURE 2.5.6-11

Sieve Size	Value
36"	914
30"	762
24"	610.0
18"	533.0
15"	487.0
12"	381.0
10"	305.0
8"	254.0
6"	203.2
4"	152.4
3"	101.6
2"	76.20
1 1/2"	50.80
1"	38.10
3/4"	25.40
5/8"	19.05
1/2"	15.88
3/8"	12.70
1/4"	9.525
4"	6.350
2.0"	4.760
1.0"	2.362
0.85"	1.981
0.75"	1.397
0.60"	1.168
0.50"	0.991
0.425"	0.589
0.354"	0.495
0.25"	0.417
0.19"	0.295
0.15"	0.246
0.125"	0.208
0.106"	0.147
0.075"	0.104
0.06"	0.074

D_{50} , mm	MATERIAL
110mm	ROCKFILL
8.0	FILTER-B
0.53	FILTER-A
0.02	IMPERVIOUS CORE

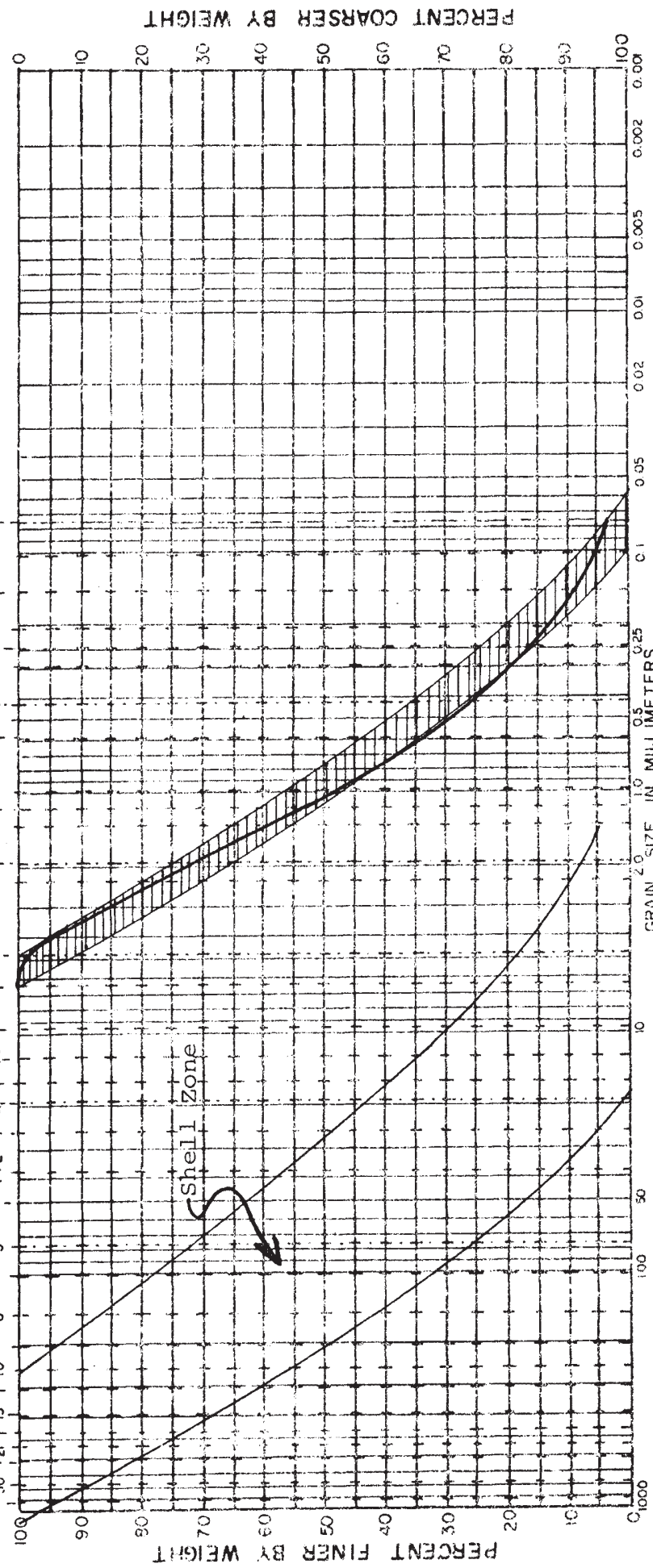
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UNITS 1 and 2

S.S.I. DAM, GRAIN SIZE
DISTRIBUTION OF CORE, FILTERS
& ROCK-FILL.

FIGURE 2.5.6-12

U.S. STANDARD SIEVE SIZES

914	36"	24"	18"	12"	8"	6"	4"	3"	2"	1 1/2"	1"	3/4"	5/8"	3/8"	1/4"	4.760	2.362	1.981	1.397	1.168	0.991	0.589	0.493	0.417	0.295	0.246	0.208	0.147	0.104	0.074
762	30"	24"	18"	12"	8"	6"	4"	3"	2"	1 1/2"	1"	3/4"	5/8"	3/8"	1/4"	4.760	2.362	1.981	1.397	1.168	0.991	0.589	0.493	0.417	0.295	0.246	0.208	0.147	0.104	0.074



COBBLES	GRAVEL			SAND			SILT or CLAY
	Coarse	Fine		Coarse	Medium	Fine	

Hole No.	Depth	Legend	LL	PL	PI	LS	Classification	SUBJECT
								Theoretical
								Gradation Limits of
								Shell Model
								Actual Gradation of
								Triaxial Tests

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

S.S.I. DAM
SHELL MODEL GRADATION

FIGURE 2.5.6-13

STRESS - Pounds/Square Inch

150

100

50

0

50

100

150

200

250

STRESS - Pounds/Square Inch

* When sample was saturated
becomes 37.5°

$$c = 0.0$$

$$\tan \phi = 0.81$$

$$\phi = 39.0^\circ$$

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UNITS 1 and 2

SHELL MODEL MATERIAL
S.S.I. DAM

FIGURE 2.5.6-14

STRESS - Pounds/Square Inch

30

20

10

0

10

20

30

40

50

60

STRESS - Pounds/Square Inch

REMOVED MATERIAL

LL = 30.8%, P.I. = 11
% Compaction = 92.9
M = 21.3% = O.M. + 0.3
40 psi backpressure

$c_u = 5 \text{ psi}$ $c' = 4 \text{ psi}$
 $\tan \phi = 0.27$ $\tan \phi = 0.47$
 $\phi = 15.7^\circ$ $\phi = 25.7^\circ$

$\phi' = 31.1^\circ$

$\phi = 25.7^\circ$

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UNITS 1 and 2

S.S.I. DAM
CLAY CORE

FIGURE 2.5.6-15

ENGINEERS

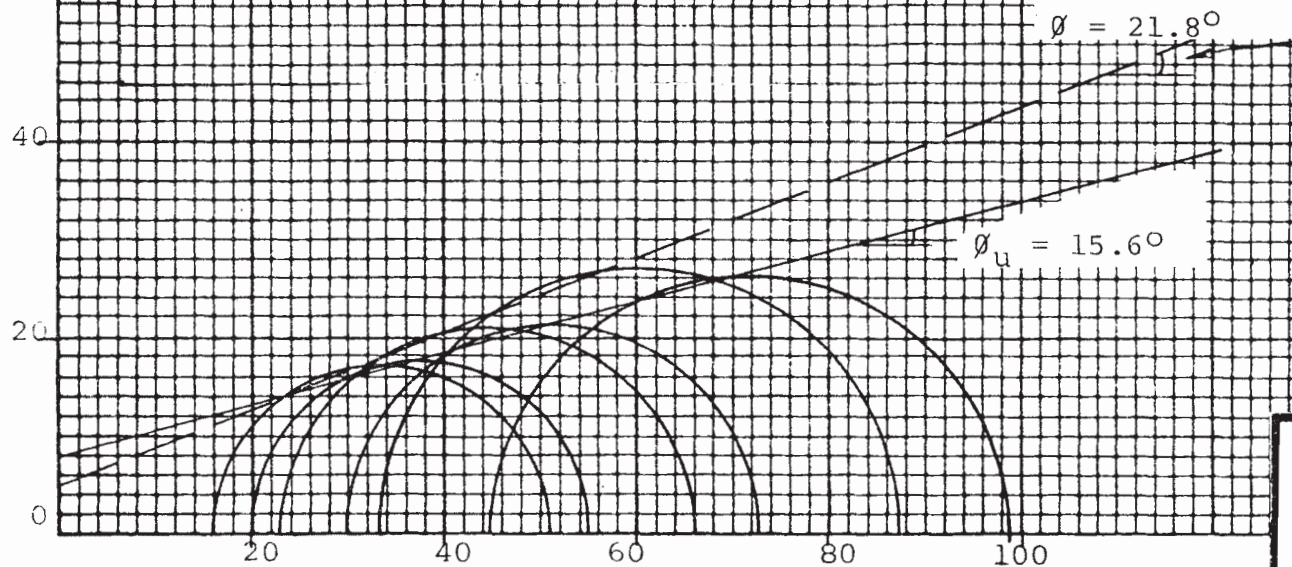
GEOLOGISTS

STRESS - Pounds/Square Inch

REMOLDED MATERIAL

LL = 44% P.I. = 22
 UDW = 98.7 pcf
 % Compaction = 96.3
 M = 20.8% = O.M.+2%
 40 psi backpressure
 Si = 80%
 Sf = 97 - 100%

$c_u = 8$ psi $c = 5.5$ psi
 $\tan \phi = 0.28$ $\tan \phi = 0.4$
 $\phi = 15.6^\circ$ $\phi = 21.8^\circ$



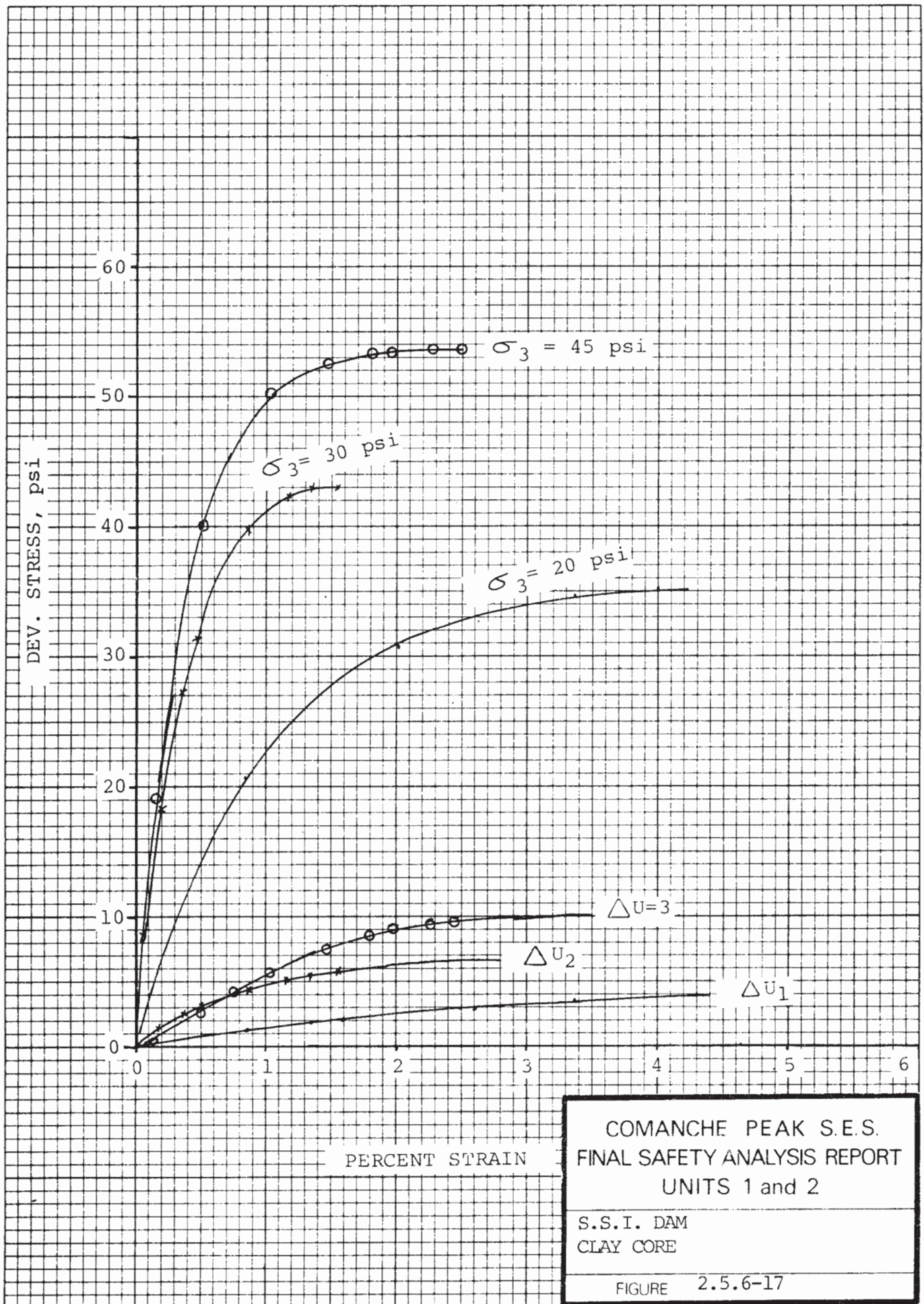
STRESS - Pounds/Square Inch

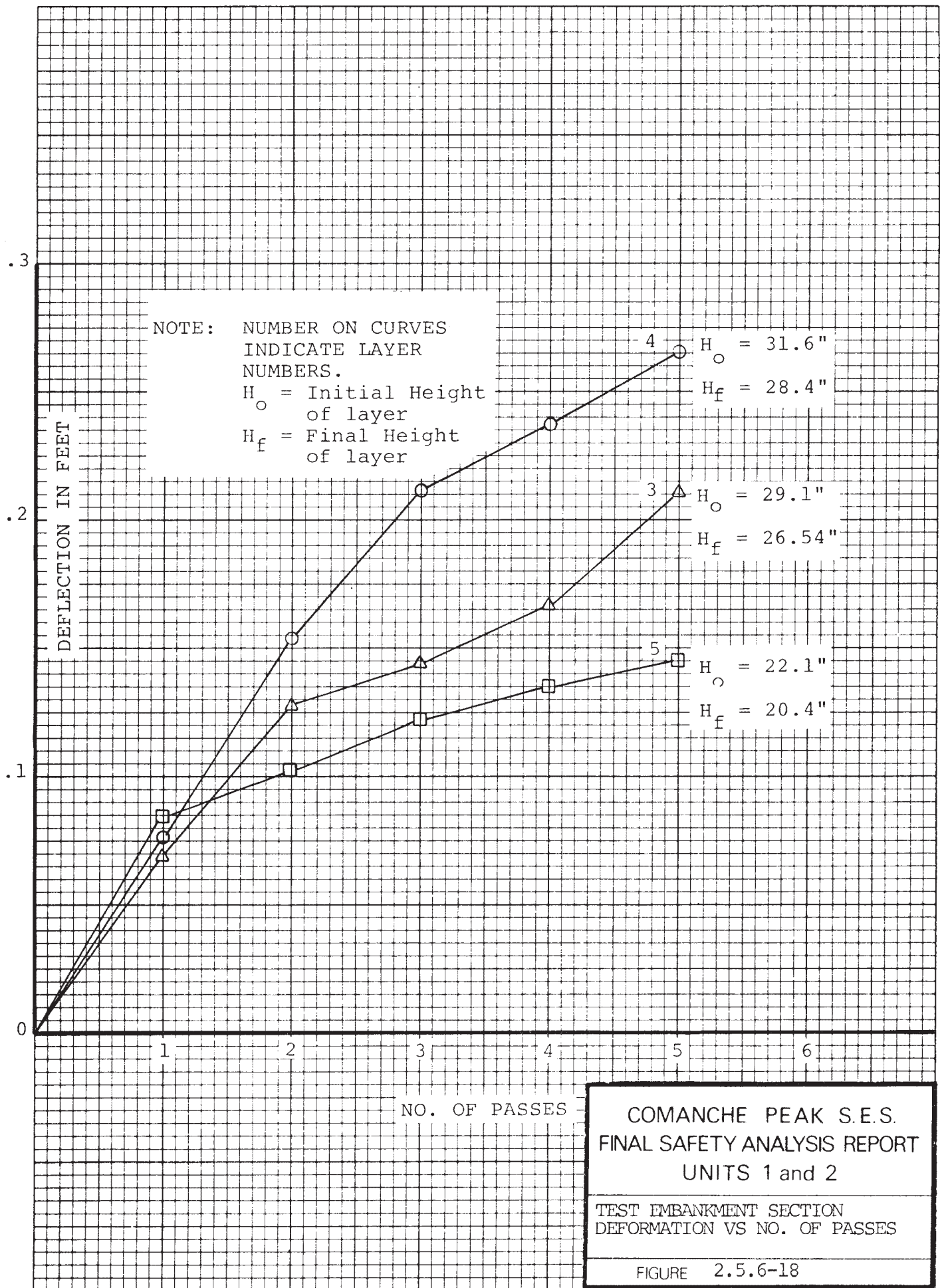
COMANCHE PEAK S.E.S.
 FINAL SAFETY ANALYSIS REPORT
 UNITS 1 and 2

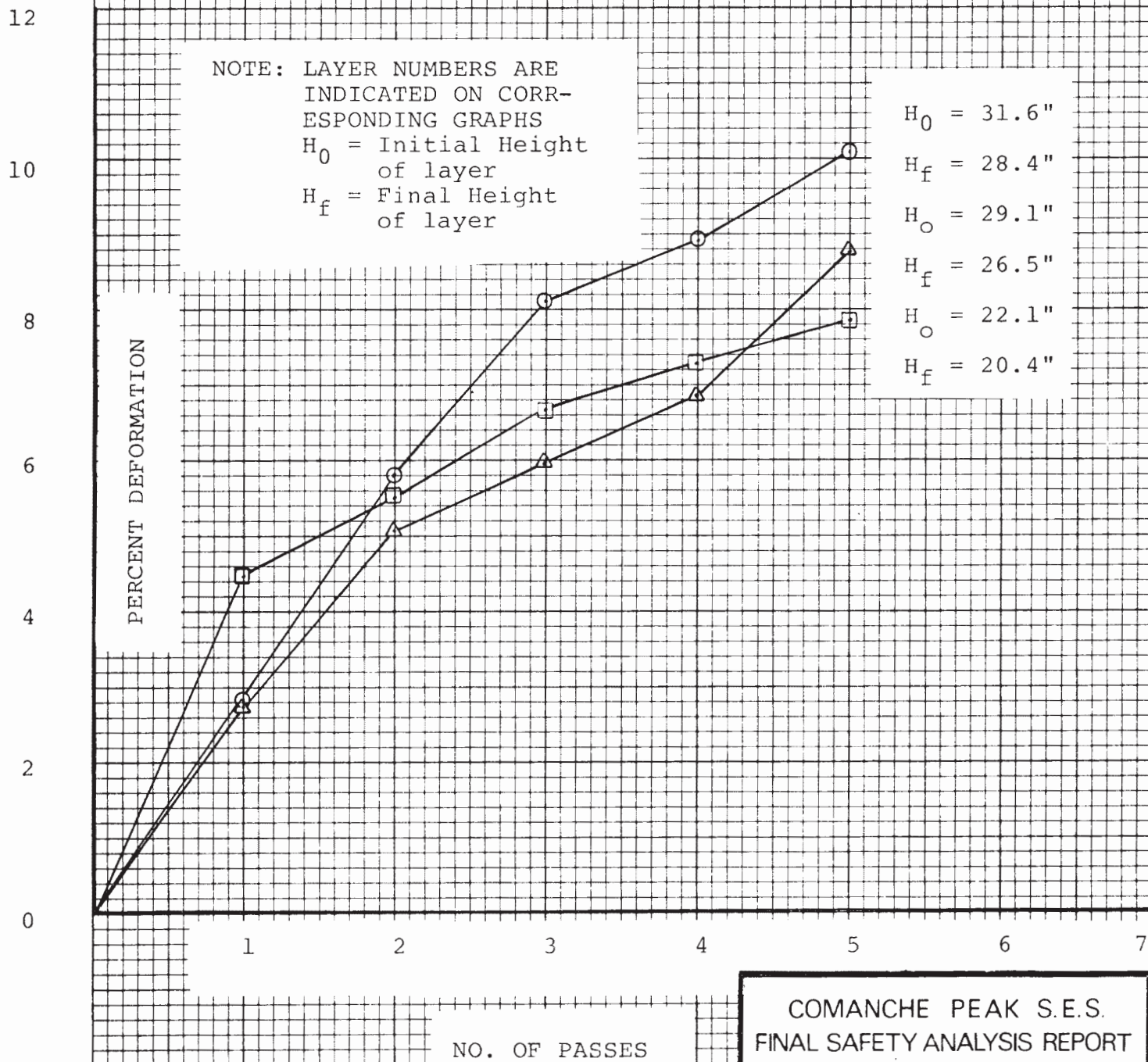
S.S.I. DAM
 CLAY CORE

FIGURE 2.5.6-16

ENGINEERS



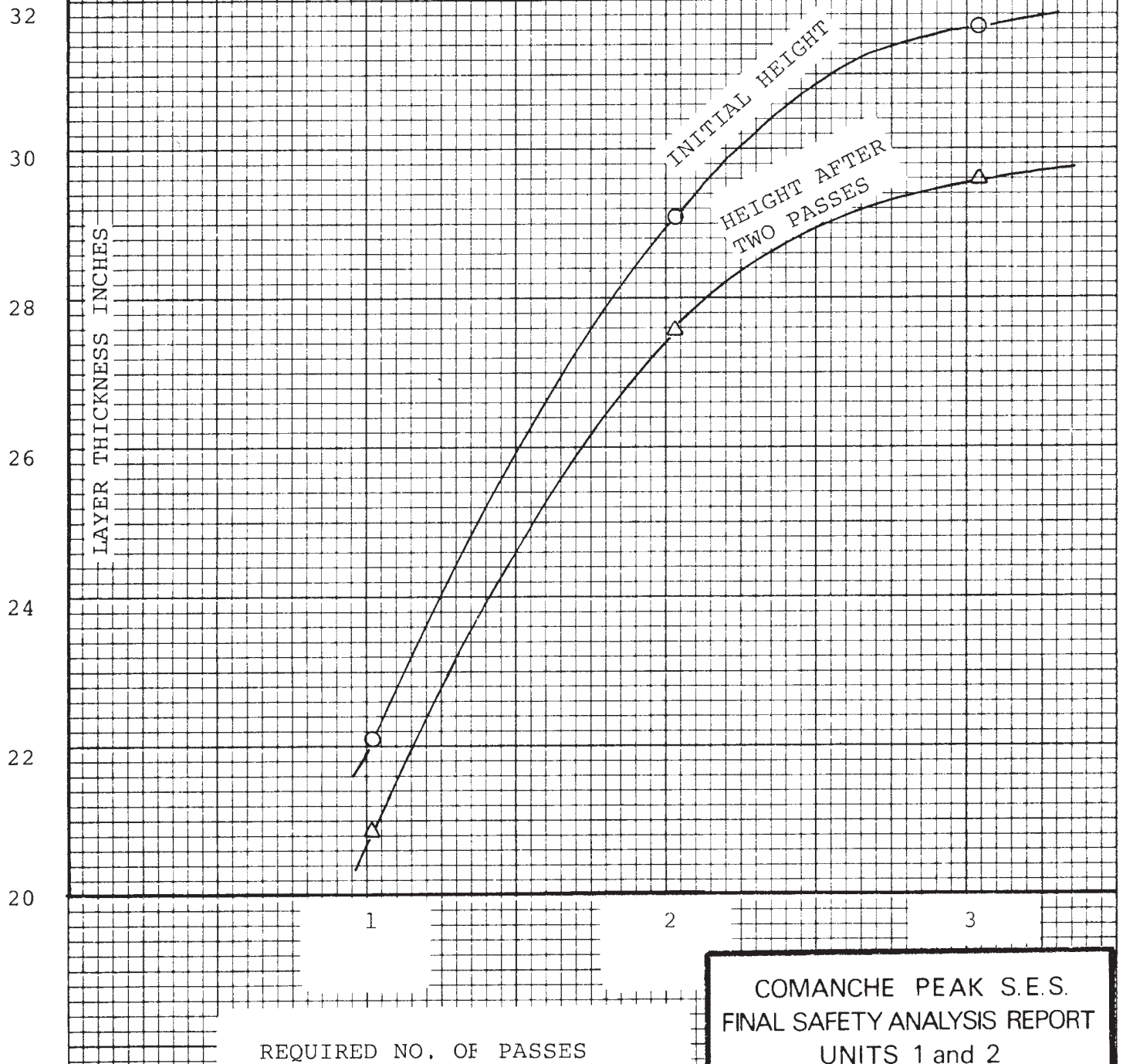




COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

TEST EMBANKMENT SECTION
PERCENT DEFORMATION VS NO.
OF PASSES

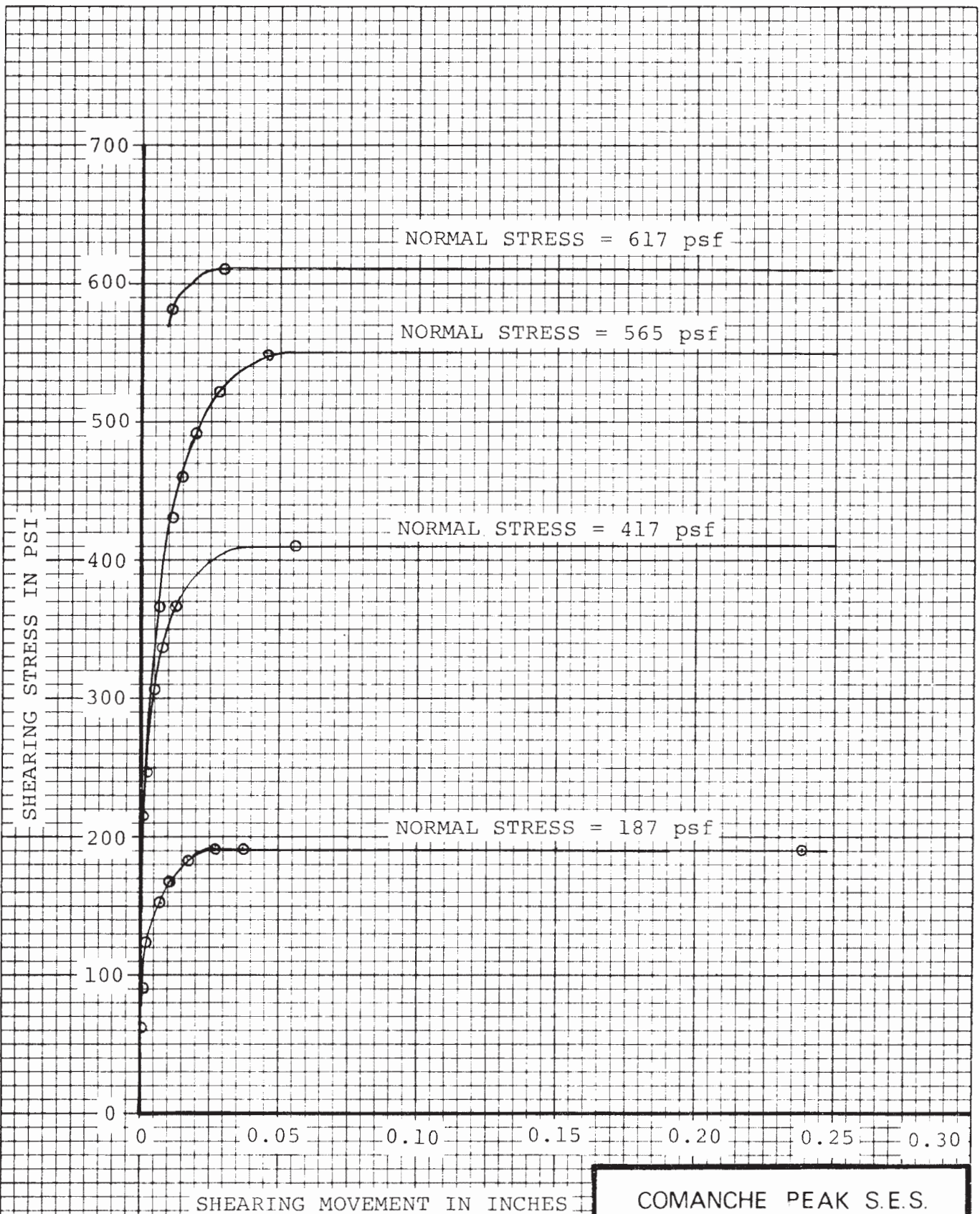
FIGURE 2.5.6-19



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UNITS 1 and 2

TEST EMBANKMENT SECTION
LAYER THICKNESS VS REQUIRED
NO. OF PASSES

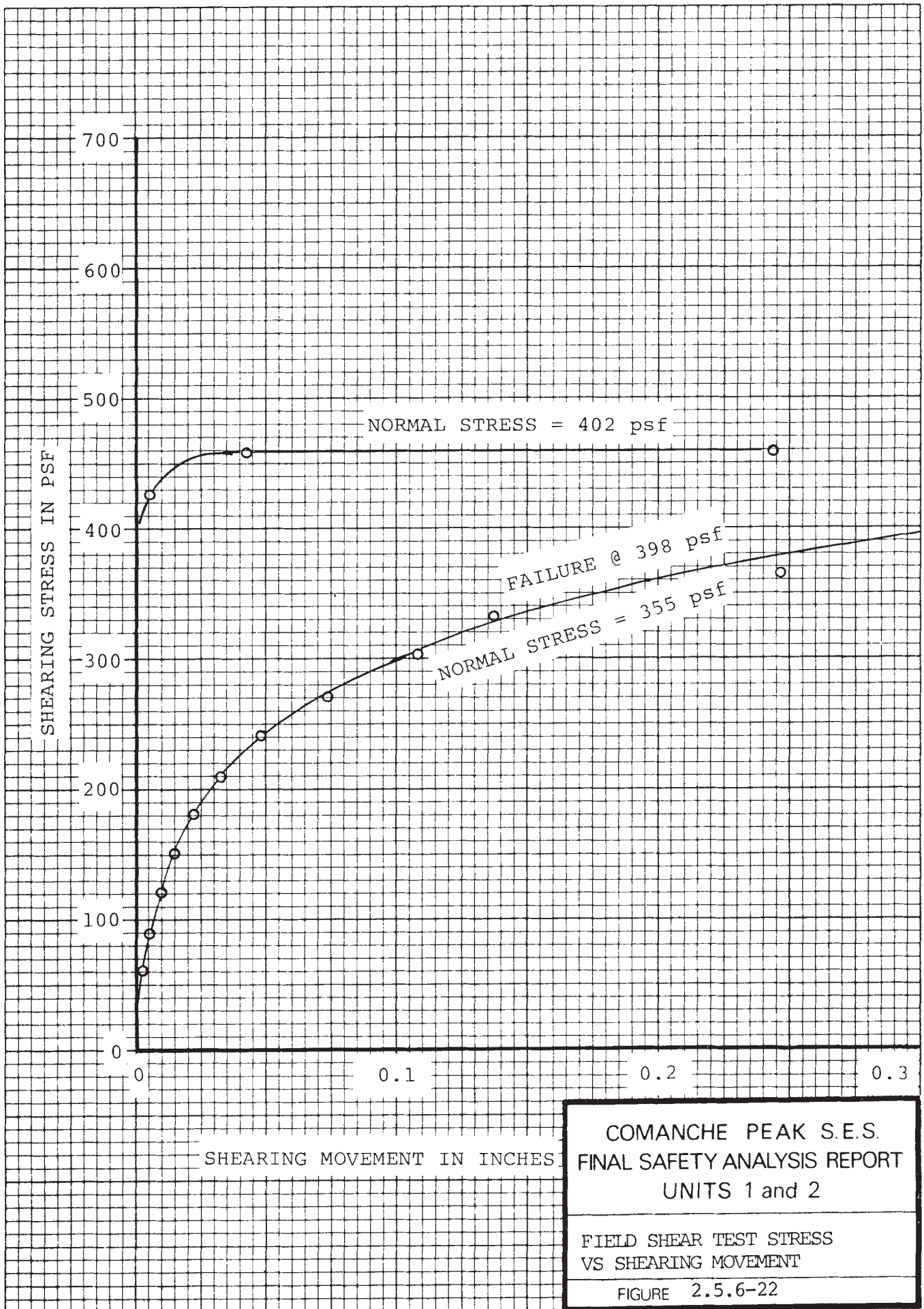
FIGURE 2.5.6-20

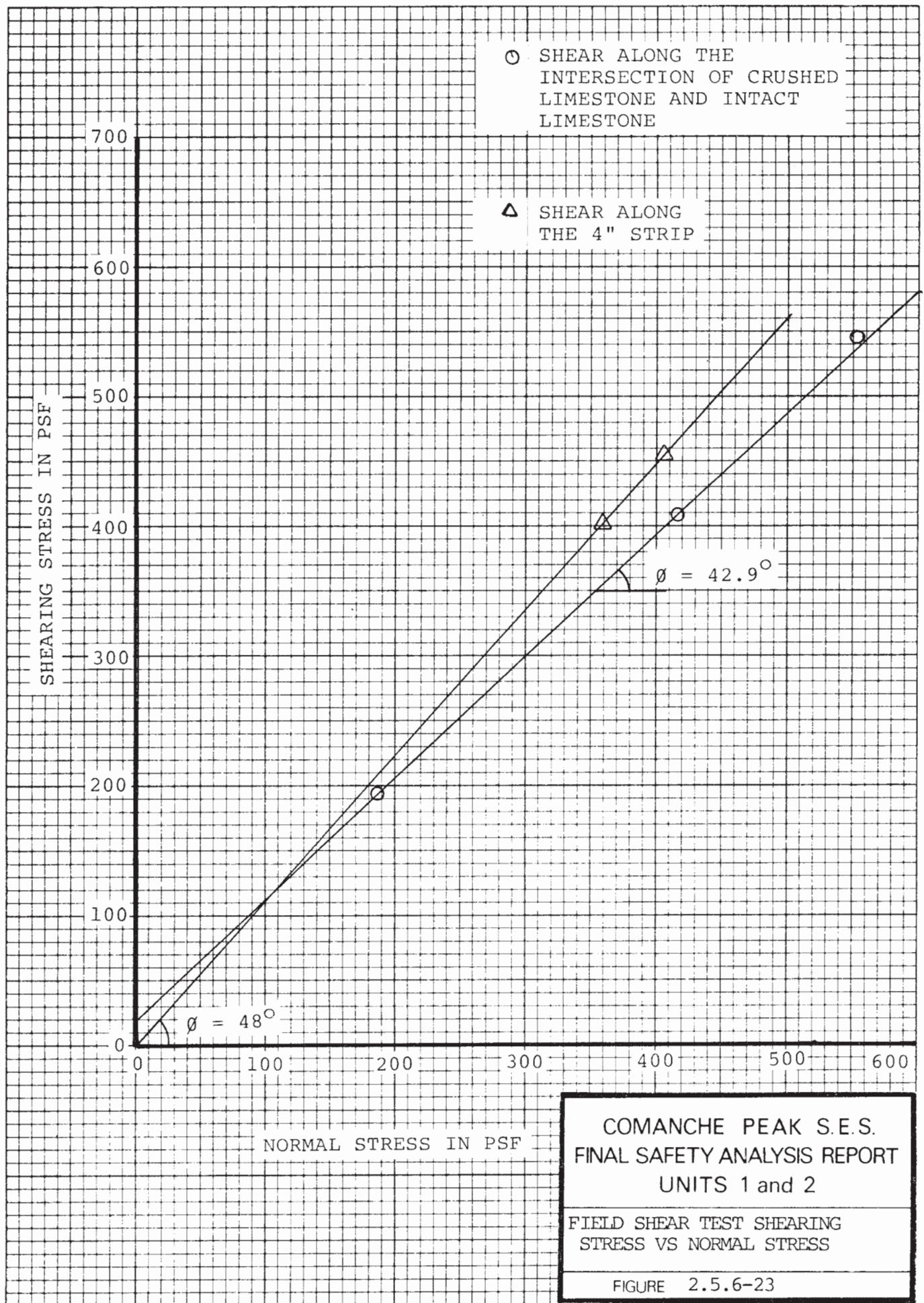


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UNITS 1 and 2

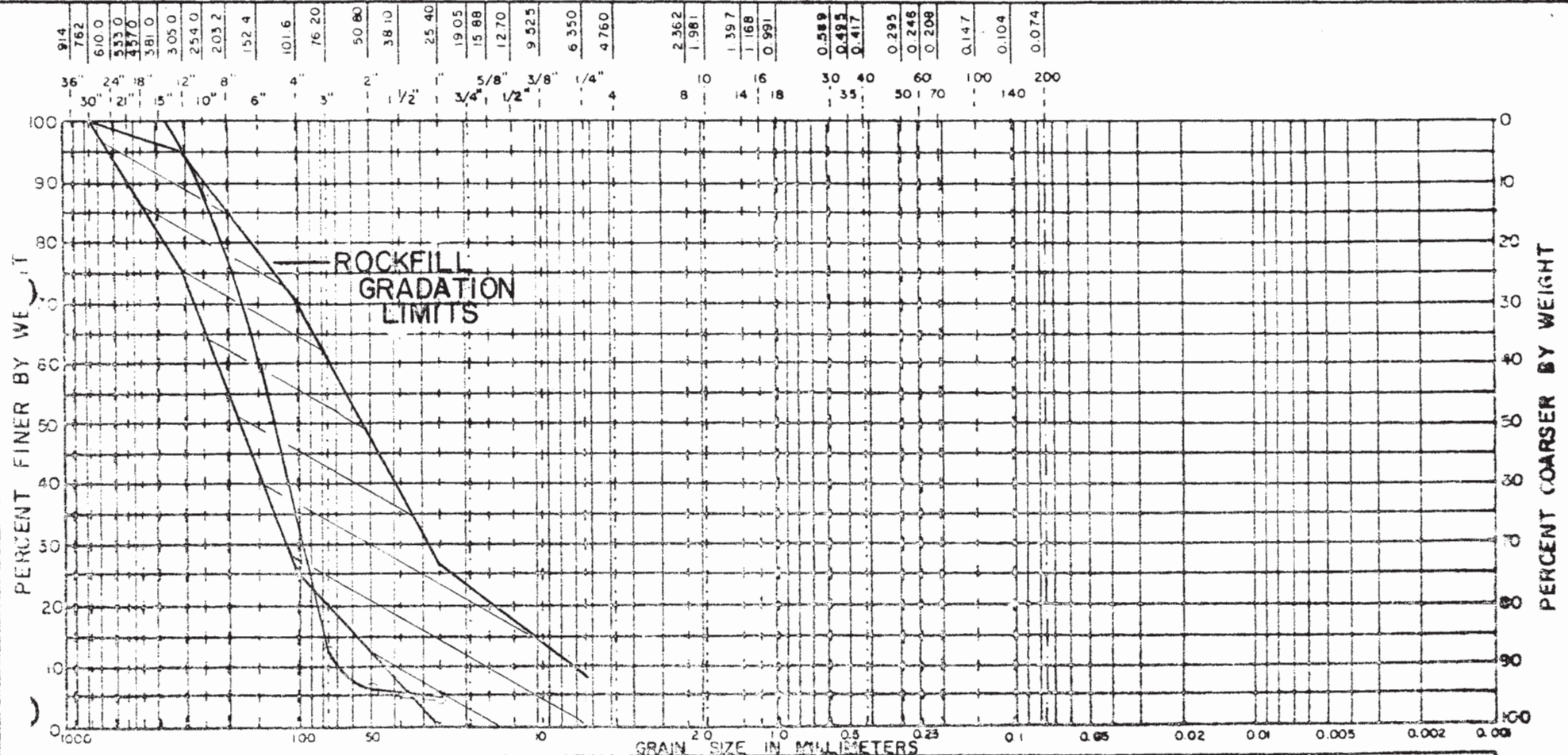
FIELD SHEAR TEST STRESS
VS SHEARING MOVEMENT

FIGURE 2.5.6-21





U. S. STANDARD SIEVE SIZES



COBBLES	GRAVEL		SAND			SILT or CLAY
	Coarse	Fine	Coarse	Medium	Fine	

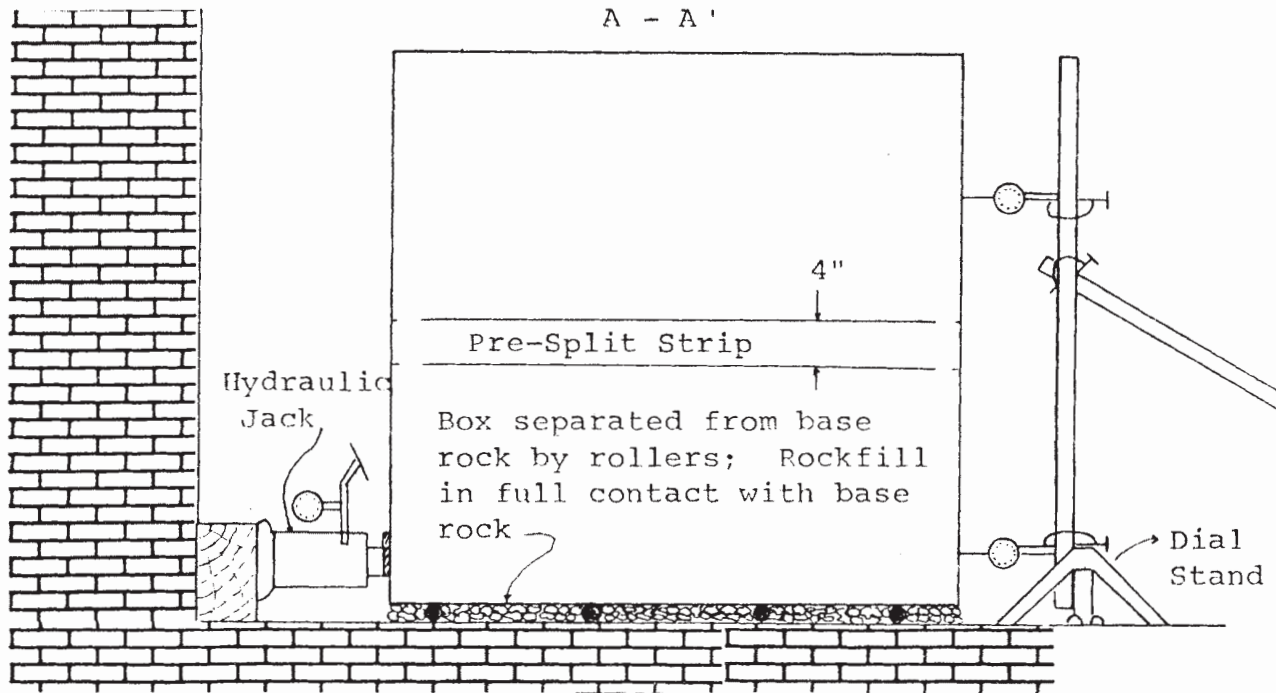
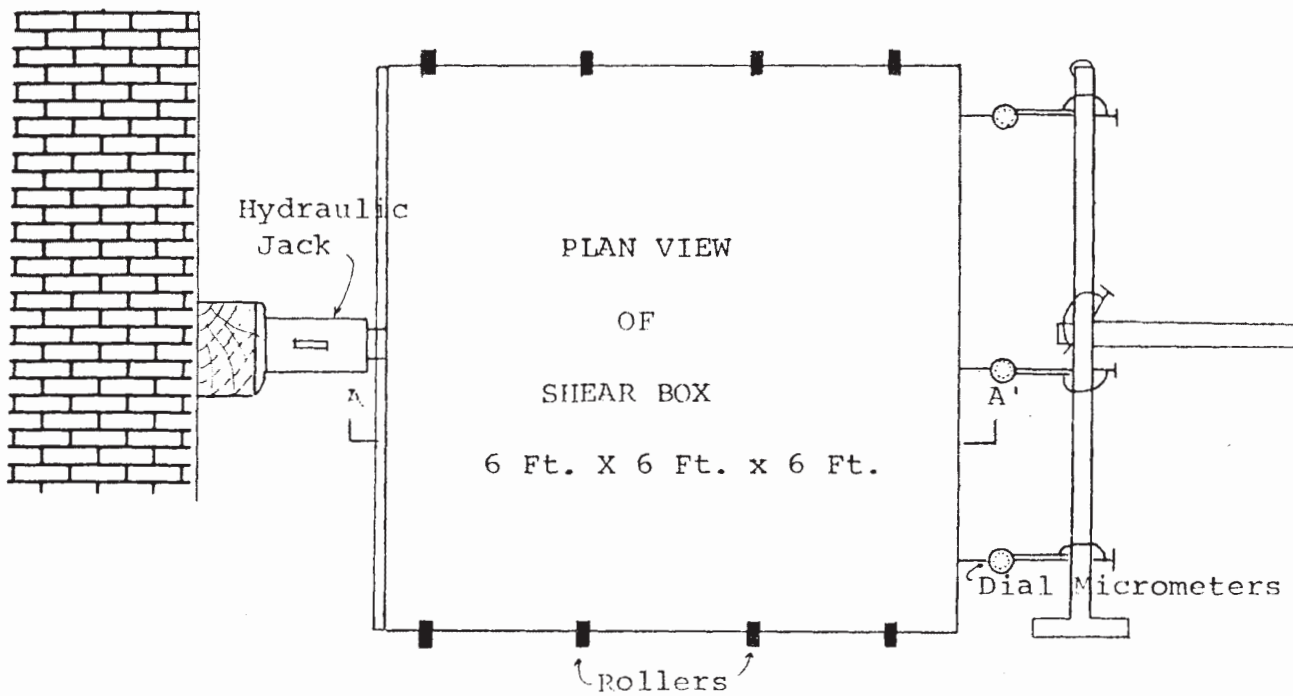
AMENDMENT 2
JULY 27, 1978 AMENDMENT 2

Core No. Splice No.	Depth	Legend	LL	PL	PI	LS	Classification	SUBJECT
1	0-2'						Crushed Limestone	Sieve Analysis on the materials taken from density Pit No. 2

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UNITS 1 and 2

MECHANICAL ANALYSIS

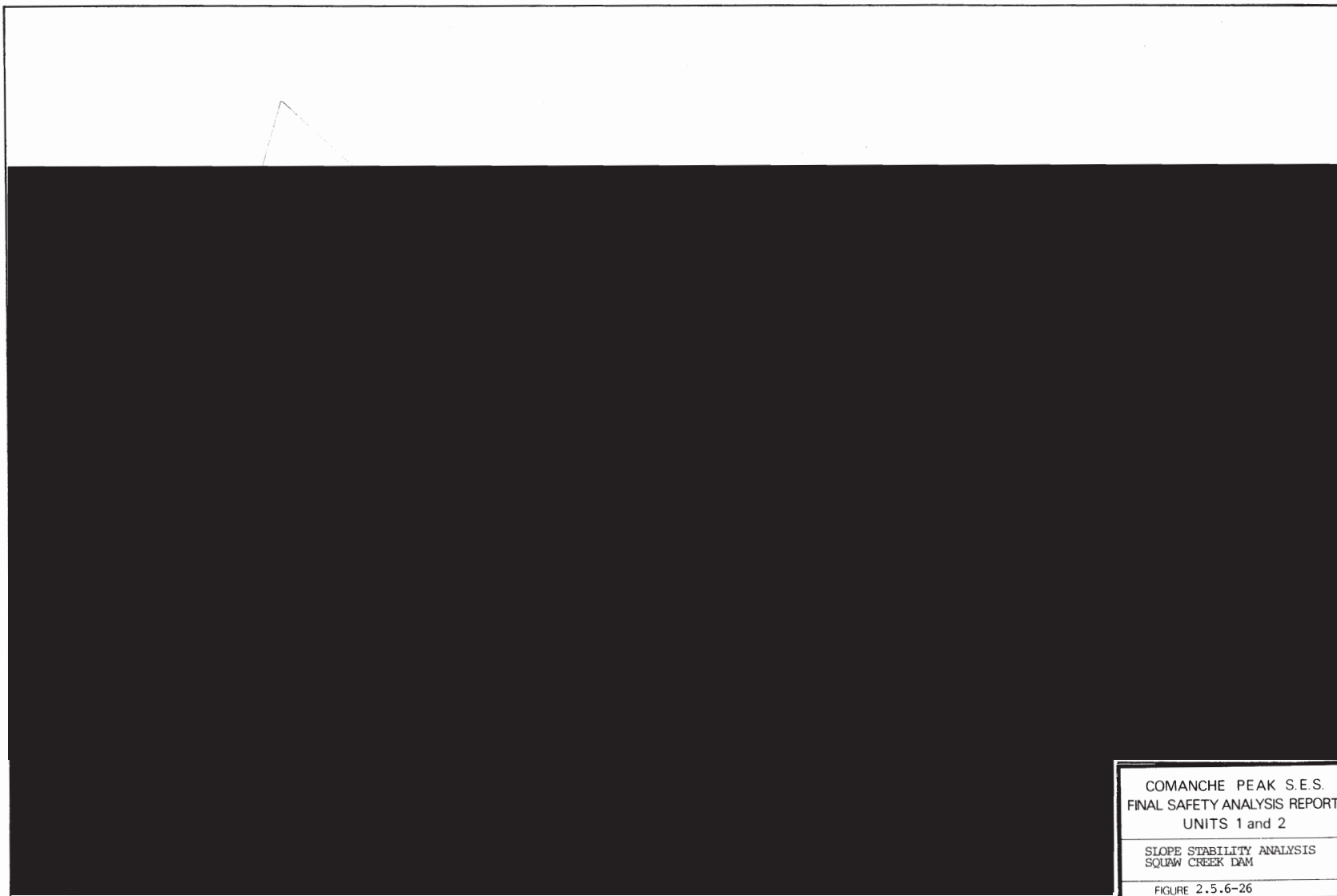
FIGURE 2.5.6-24



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UNITS 1 and 2

FIELD SHEAR TEST

FIGURE 2.5.6-25



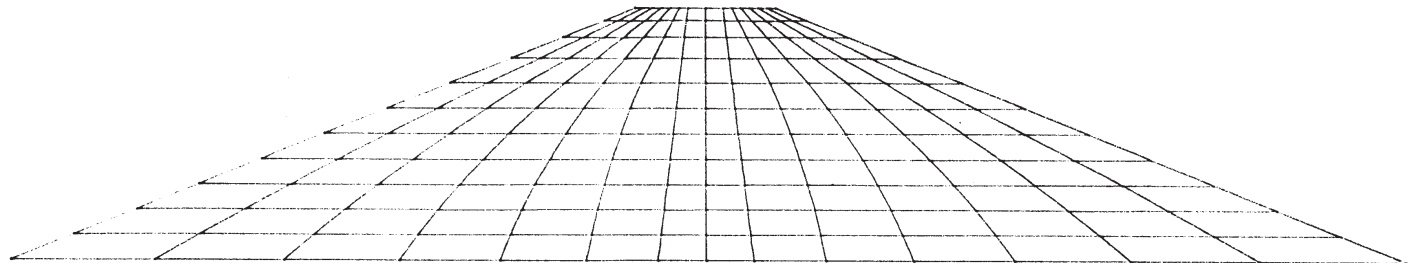
COMANCHE PEAK S.E.S. FINAL SAFETY ANALYSIS REPORT UNITS 1 and 2
SLOPE STABILITY ANALYSIS SQUAW CREEK DAM
FIGURE 2.5.6-26

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

S.S.I. DAM
STATIC ANALYSIS

FIGURE 2.5.6-27

S.5.1. DAM
NODAL POINT CONFIGURATION
SCALE: 1 INCH = 26.33 FEET



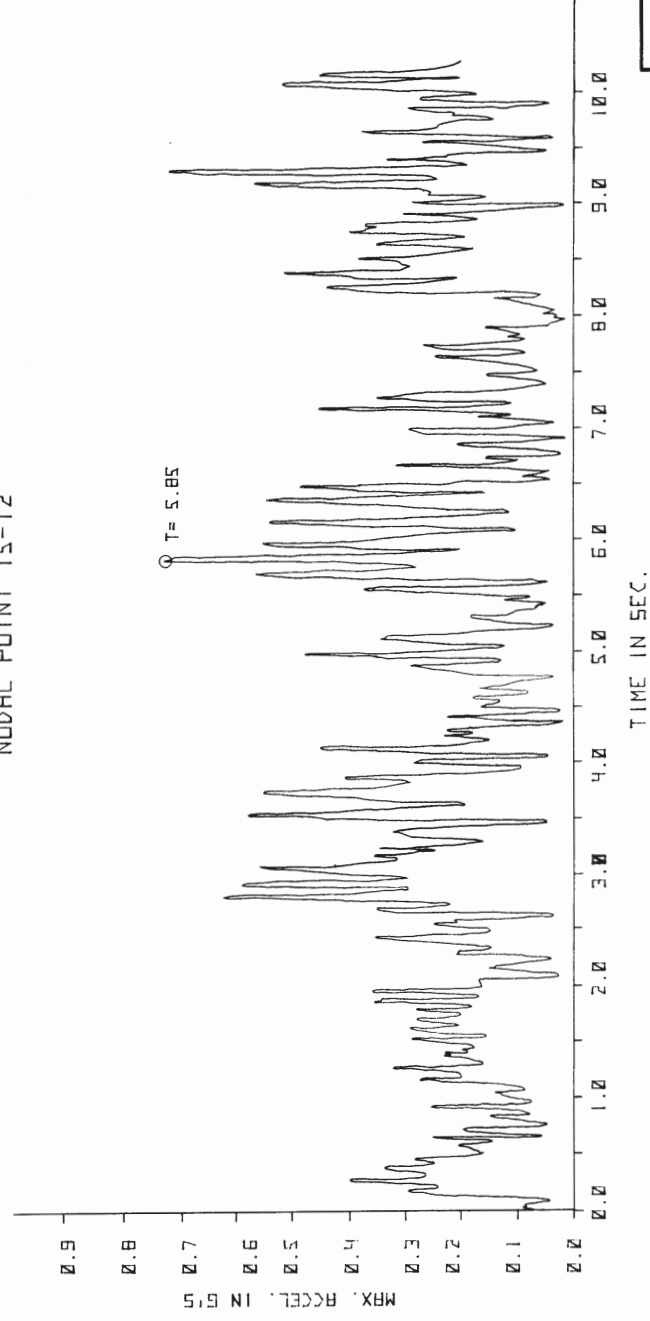
COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-28

S.S.I. DAM

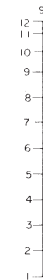
NODAL POINT 15-12



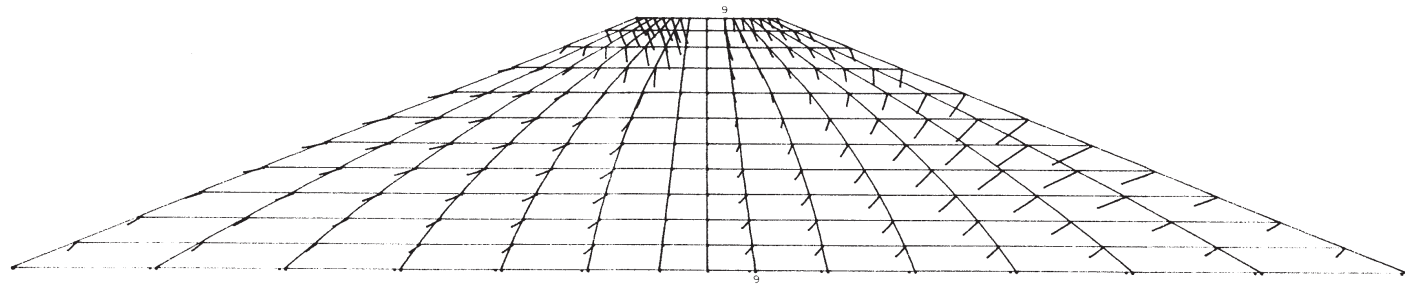
COMANCHE PEAK S.E.S. FINAL SAFETY ANALYSIS REPORT UNITS 1 and 2
FINITE ELEMENT ANALYSIS
FIGURE 2.5.6-29

S.S.I. DAM
NODAL POINT ACCELERATION VECTORS
AT T= 5.61 SEC.
SCALE: 1 INCH = 1.0 G

LINE 9 PROFILE



SCALE: 1" = 10 G

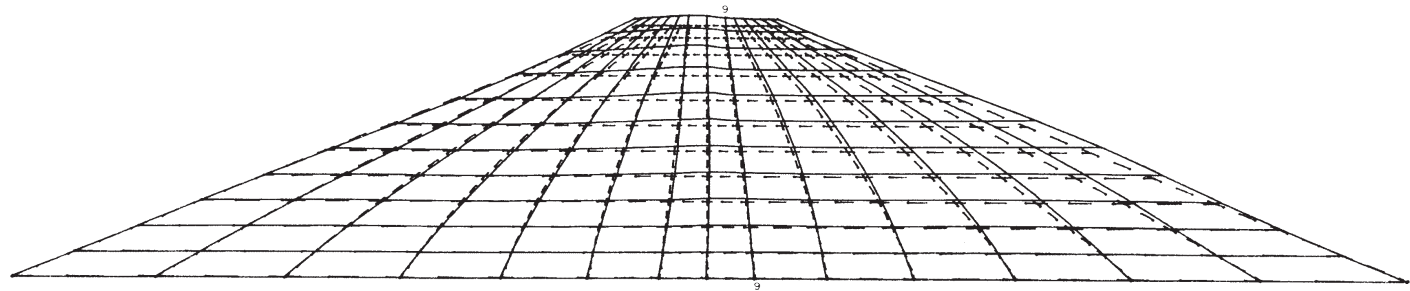
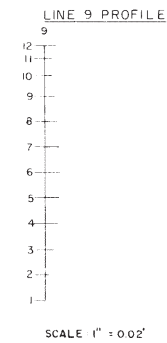


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UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-30

S.S.I. DAM
NODAL POINT DISPLACEMENT
AT T= 5.61 SEC.
DISPLACEMENT SCALE: 1 INCH = 0.1 FEET

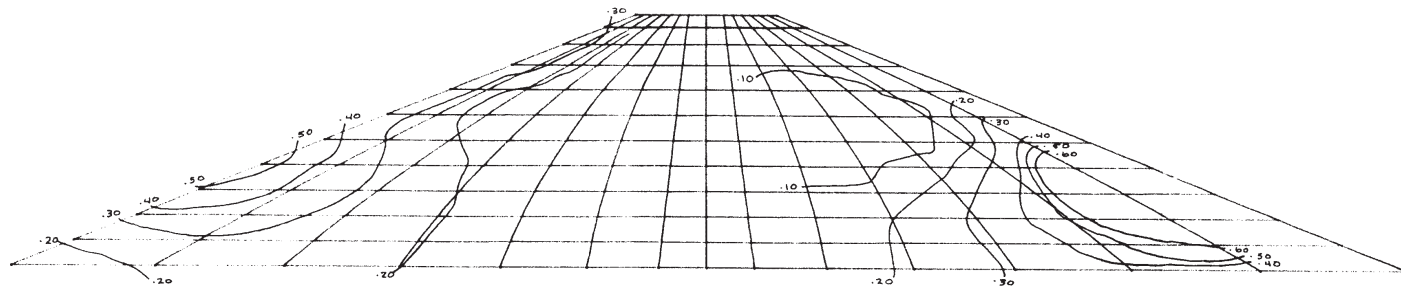


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FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-31

S.S.I. DAM
RATIO OF τ/σ_{VB} STRESS
 $T = 5.61 \text{ SEC.}$

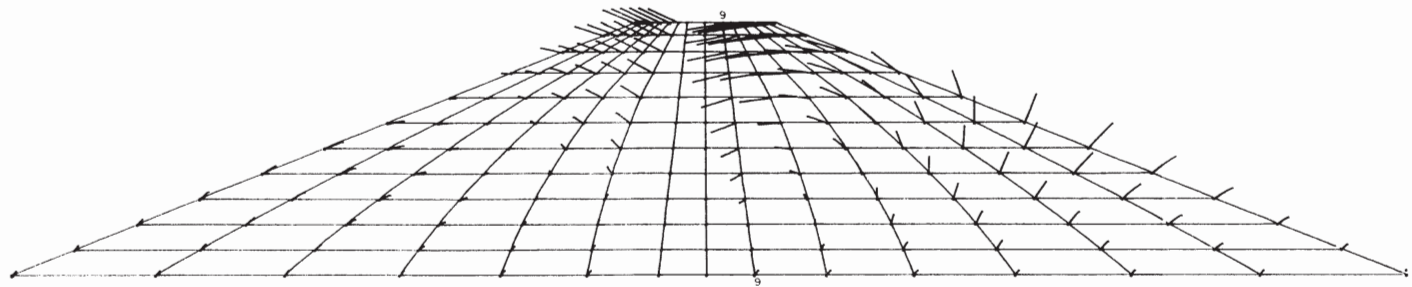
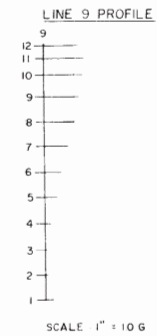


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FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-32

S.S.I. DAM
NODAL POINT ACCELERATION VECTORS
AT T= 5.7 SEC.
SCALE: 1 INCH = 1.0 G

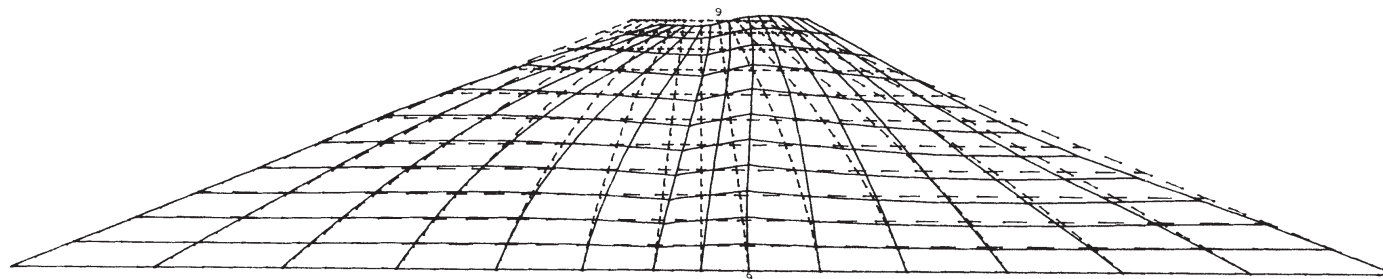
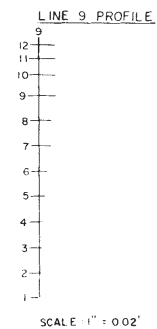


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FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-33

S.S.I. DAM
NODAL POINT DISPLACEMENT
AT T= 5.7 SEC.
DISPLACEMENT SCALE: 1 INCH = 0.1 FEET

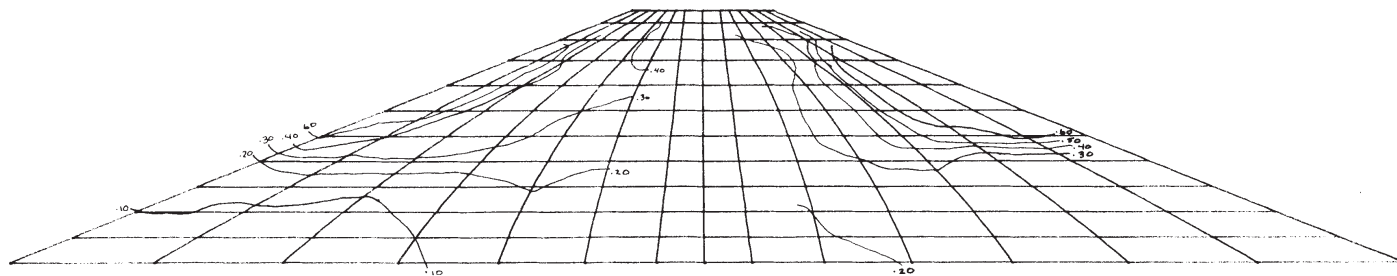


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UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-34

S.5.1. DAM
RATIO OF τ/σ_{VB} STRESS
 $T = 5.70$ SEC.

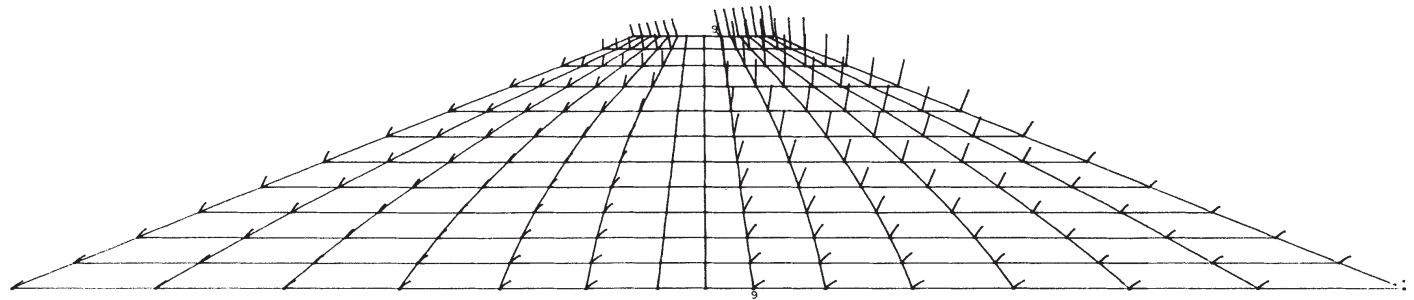
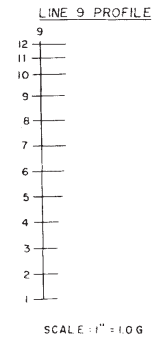


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UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-35

S.S.I. DAM
NODAL POINT ACCELERATION VECTORS
AT T= 5.76 SEC.
SCALE: 1 INCH = 1.0 G

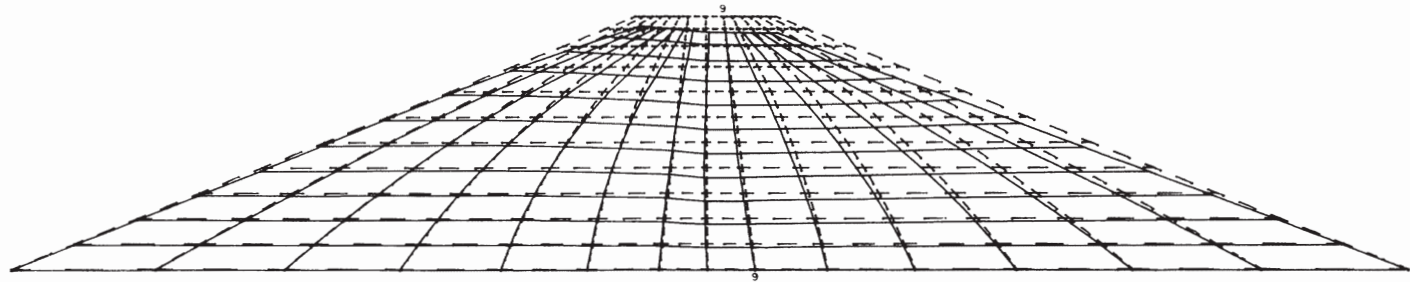
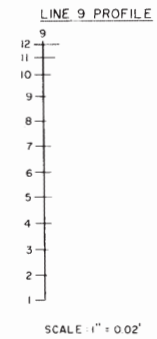


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UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-36

S.S.I. DAM
 NODAL POINT DISPLACEMENT
 AT T= 5.76 SEC.
 DISPLACEMENT SCALE: 1 INCH = 0.1 FEET

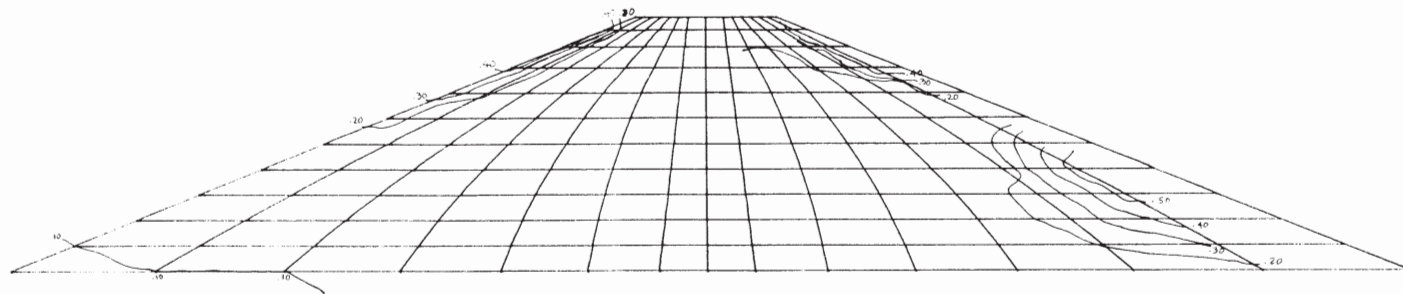


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 FINAL SAFETY ANALYSIS REPORT
 UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-37

S.S.I. DAM
RATIO OF τ/σ_{VB} STRESS
 $T = 5.76 \text{ SEC.}$



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FINITE ELEMENT ANALYSIS

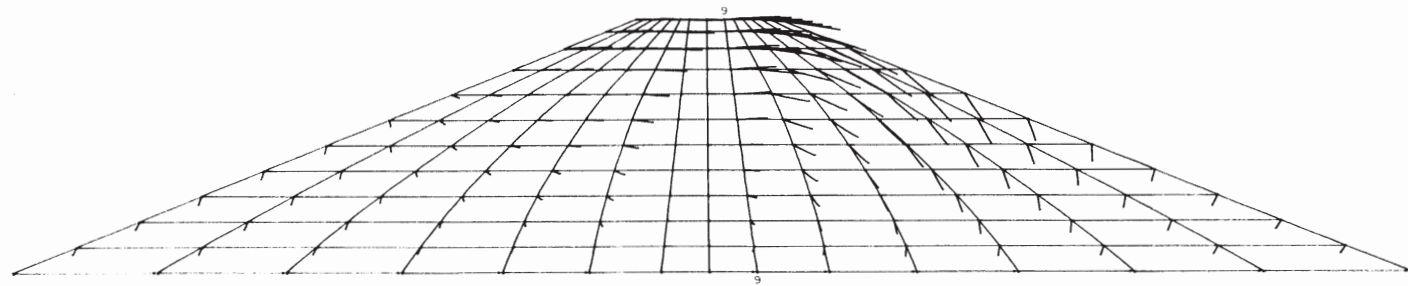
FIGURE 2.5.6-38

S.S.I. DAM
NODAL POINT ACCELERATION VECTORS
AT T= 5.85 SEC.
SCALE: 1 INCH = 1.0 G

LINE 9 PROFILE



SCALE: 1" = 1.0 G

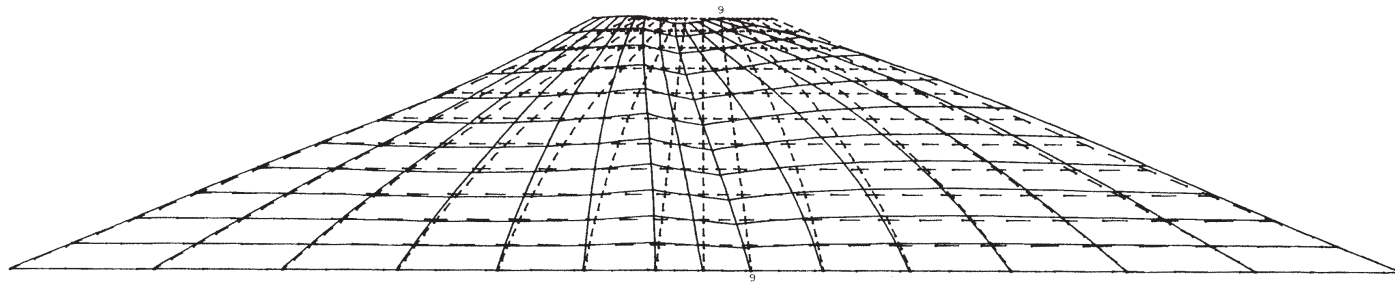
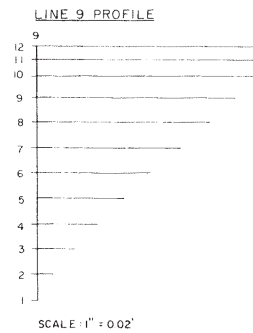


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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-39

S.S.I. DAM
NODAL POINT DISPLACEMENT
AT T= 5.85 SEC.
DISPLACEMENT SCALE: 1 INCH = 0.1 FEET

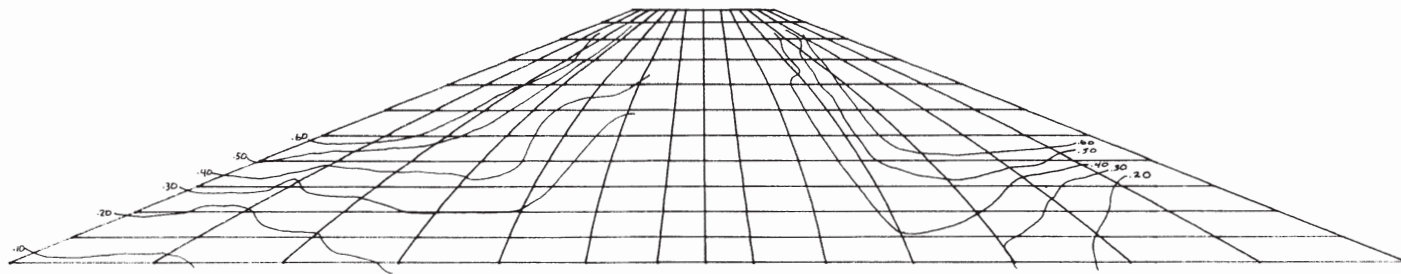


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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-40

S.S.I. DAM
RATIO OF τ/σ_{VB} STRESS
 $T = 5.85 \text{ SEC.}$

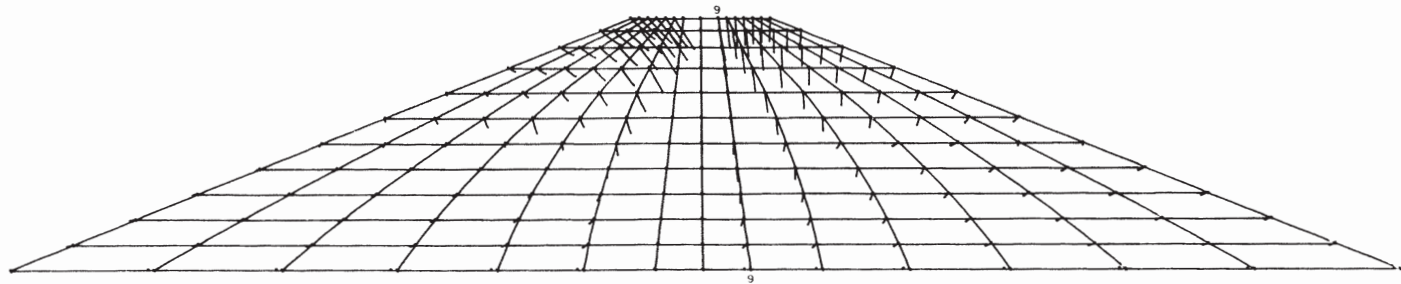
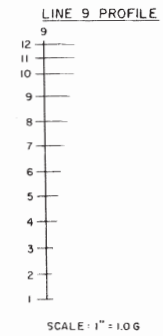


COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-41

S.S.I. DAM
NODAL POINT ACCELERATION VECTORS
AT T= 5.91 SEC.
SCALE: 1 INCH = 1.0 G



COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

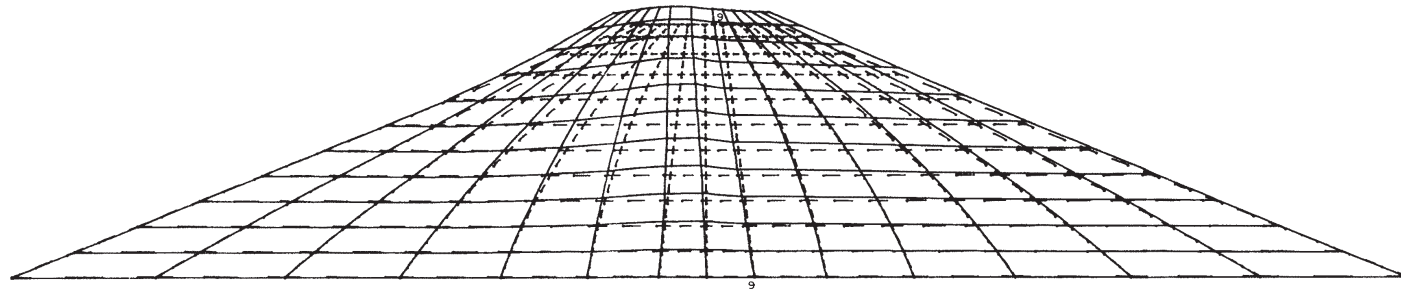
FIGURE 2.5.6-42

S.S.I. DAM
NODAL POINT DISPLACEMENT
AT T= 5.91 SEC.
DISPLACEMENT SCALE: 1 INCH = 0.1 FEET

LINE 9 PROFILE



SCALE: 1" = 0.02'

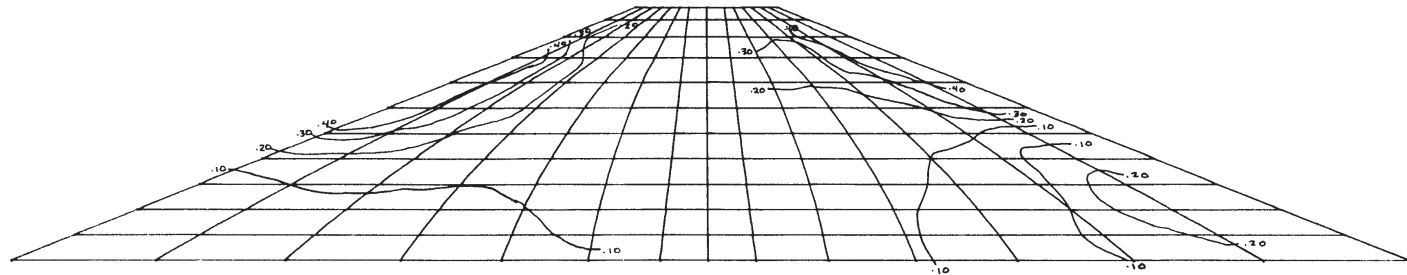


COMANCHE PEAK S.E.S.
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UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-43

S.S.I. DAM
RATIO OF τ/σ_{VB} STRESS
 $T = 5.91 \text{ SEC.}$

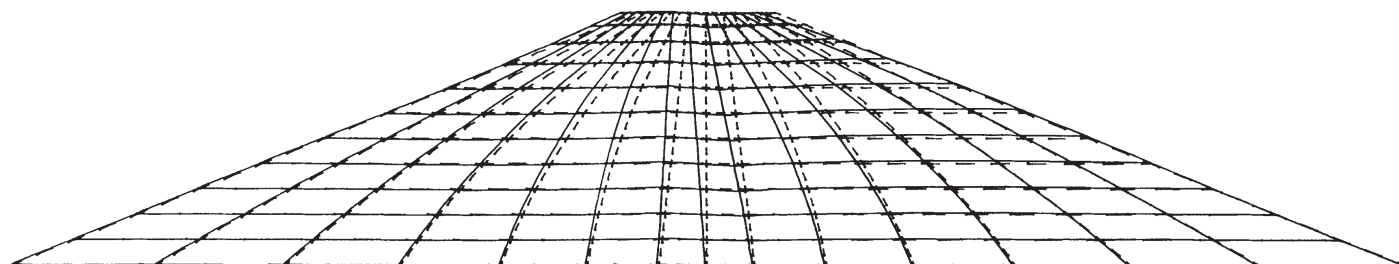


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FIGURE 2.5.6-44

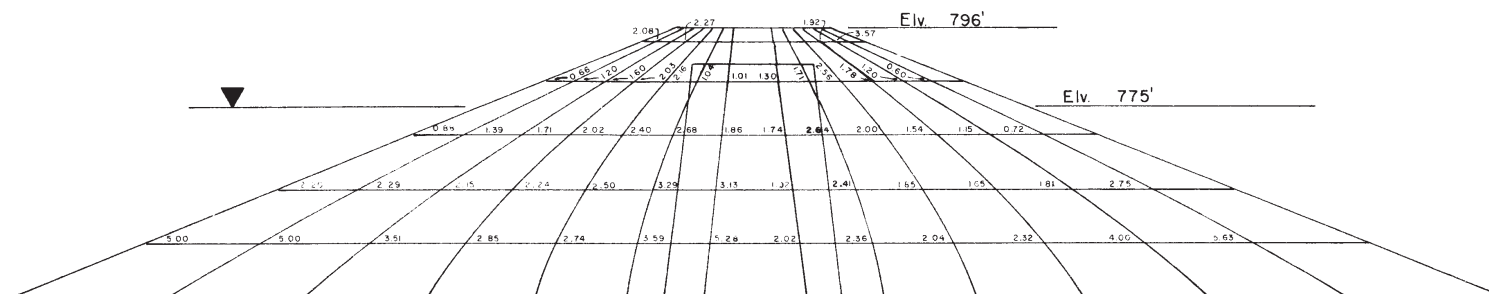
S.S.I. DAM
FINAL NODAL POINT DISPLACEMENT
AT T= 30.00 SEC.
DISPLACEMENT SCALE: 1 INCH = 10.00 FEET



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FIGURE 2.5.6-45



S.S.I. DAM

Plot of τ_d/τ_f Stress Ratio at $T=5.85$ seconds
(Time of maximum acceleration) where:

τ_d = shear stress induced by ground motion

τ_f = shear stress required to cause 5×10^{-2}
strain in 10 cycles

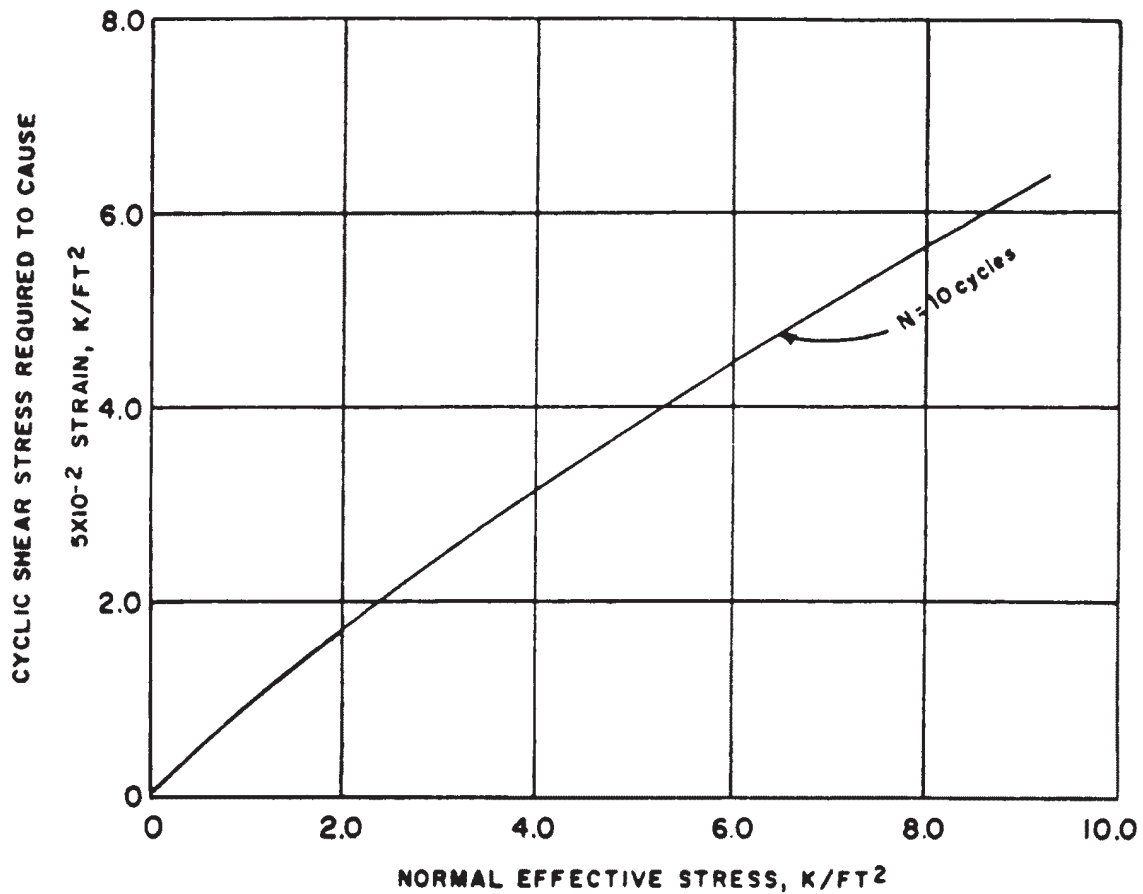
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JULY 27, 1978

COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

FINITE ELEMENT ANALYSIS

FIGURE 2.5.6-46

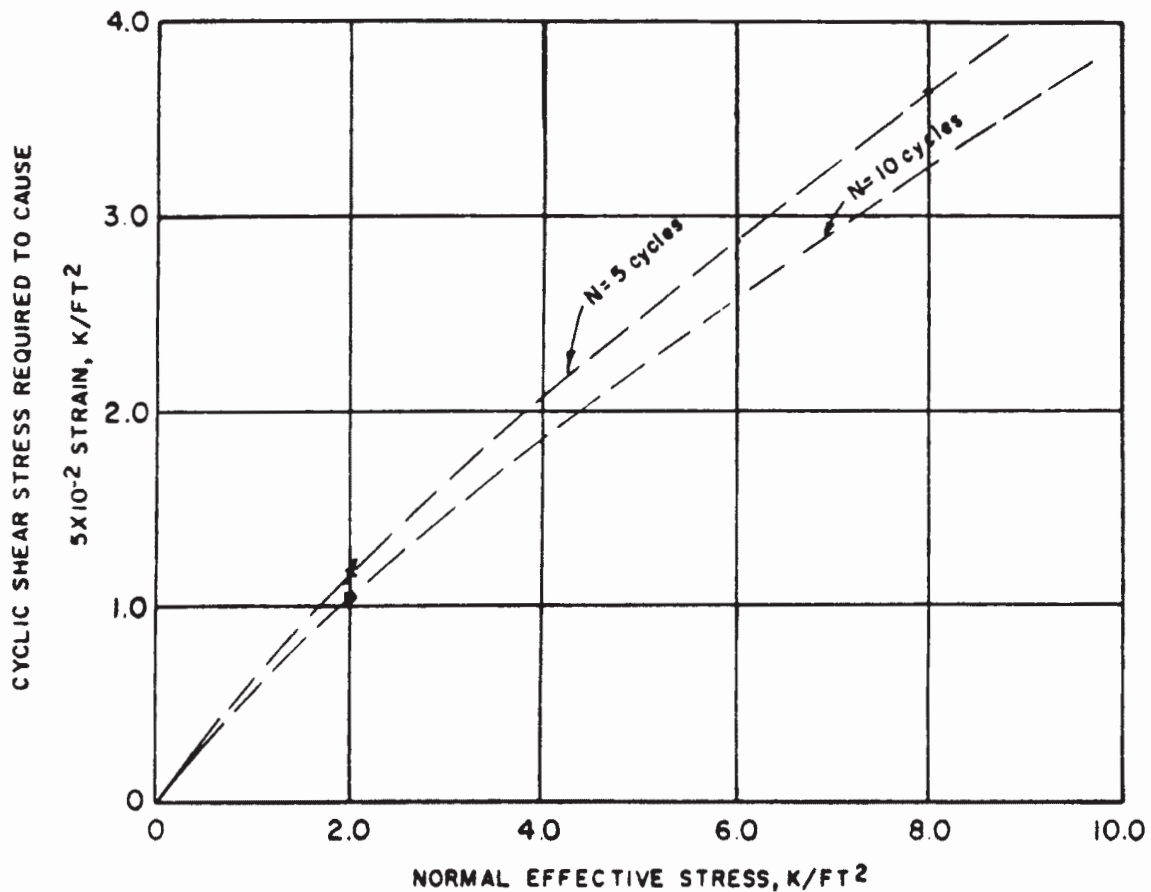
S.S.I. Dam Rockfill; curve based on D₅₀ size of 4 in. or 100 mm and derived from published data from Wong (1970), Lee and Fitton (1969), Lee and Seed (1967).



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February 5, 1988

COMANCHE PEAK S.E.S.
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CYCLIC SHEAR STRENGTH CRITERIA
FOR ROCK FILL
FIGURE 2.5.6-47

S.S.I. Dam Filter Material; curves based on published data from Lee and Seed (1967) for a Relative Density of 80%.



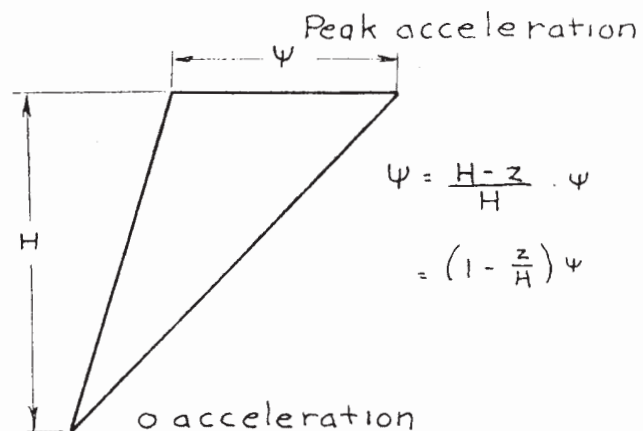
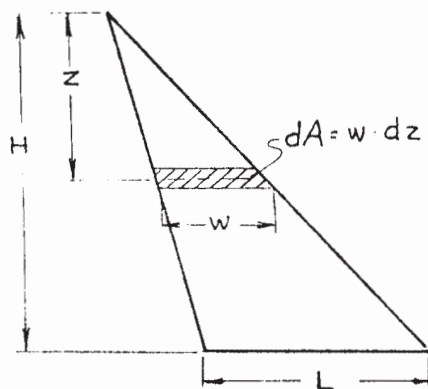
SEE FIG. 2.5A-17 FOR FINAL CYCLIC SHEAR STRENGTH CRITERIA OF FILTER MATERIALS.

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COMANCHE PEAK S.E.S.
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ASSUMED CYCLIC SHEAR STRENGTH
CRITERIA USED FOR FILTER
MATERIALS

FIGURE 2.5.6-48



$$\psi = \frac{H-z}{H} \cdot \psi$$

$$= \left(1 - \frac{z}{H}\right) \psi$$

$$w = \frac{z}{H} \cdot L = \frac{L}{H} \cdot z$$

$$\int dA = \frac{L}{H} \int z dz = \frac{L \cdot z^2}{2 \cdot H}$$

$$\int a \cdot dA = \frac{L}{H} \cdot \psi \int \left(z - \frac{z^2}{H}\right) dz$$

$$= \frac{L}{H} \cdot \psi \left(\frac{z^2}{2} - \frac{z^3}{3H}\right)$$

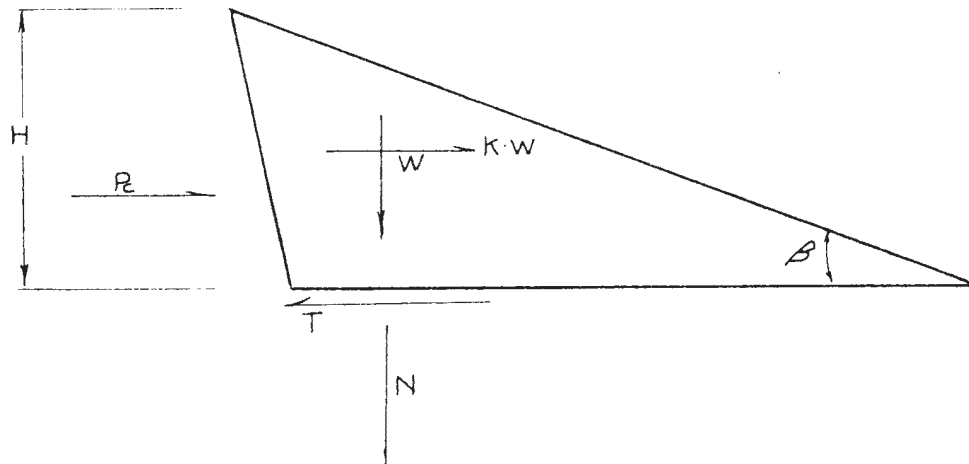
Mean Acceleration = $\bar{a} =$

$$\frac{\int a dA}{\int dA} = \frac{\frac{L}{H} \psi \left(\frac{z^2}{2} - \frac{z^3}{3H}\right)}{\frac{L z^2}{2H}} = \psi \left(1 - \frac{2z}{3H}\right)$$

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DERIVATION OF \bar{a}

FIGURE 2.5.6-49



$$T = K \cdot W + R$$

$$W = \frac{1}{2} \gamma_r \cdot H^2 \cdot \cot \beta$$

$$R = \frac{1}{2} \gamma_c \cdot H^2$$

$$\therefore T = \frac{1}{2} \cdot K \cdot \gamma_r \cdot H^2 \cdot \cot \beta + \frac{1}{2} \gamma_c \cdot H^2$$

$$\text{Shear Resistance, } S = W \cdot \tan \phi = \frac{1}{2} \gamma_r H^2 \cot \beta \cdot \tan \phi$$

$$\text{Factor of Safety: } F = \frac{S}{T} = \frac{\frac{1}{2} \gamma_r H^2 \cot \beta \tan \phi}{\frac{1}{2} K \cdot \gamma_r H^2 \cot \beta + \frac{1}{2} \gamma_c H^2}$$

$$F = \frac{\gamma_r \cdot \cot \beta \cdot \tan \phi}{K \gamma_r \cdot \cot \beta \cdot \tan \phi + \gamma_c}$$

$$\text{For } F=1.0, \quad K = 1 - \frac{\gamma_c}{\gamma_r \cdot \cot \beta \cdot \tan \phi}$$

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DERIVATION OF WEDGE F

FIGURE 2.5.6-50

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UNITS 1 and 2

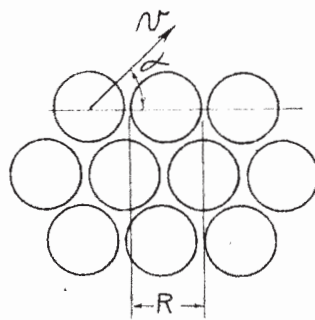
S.S.I. DAM LIQUIFICATION
STUDY

FIGURE 2.5.6-51

Acceleration Impulse Analysis

The velocity needed to be imparted to a particle of rock fill to move it over another neighboring particle is

$$v = \sqrt{\frac{Rg}{\sin 2\alpha}}$$



The rock particle size is 2 feet, thus for an inclination
 $\alpha \approx 15^\circ$

$$v \approx \sqrt{4g} \approx 11 \text{ fps}$$

Assuming a maximum acceleration impulse time $\Delta t = 0.1$ sec.,
the average acceleration required to move one rock
particle over another is

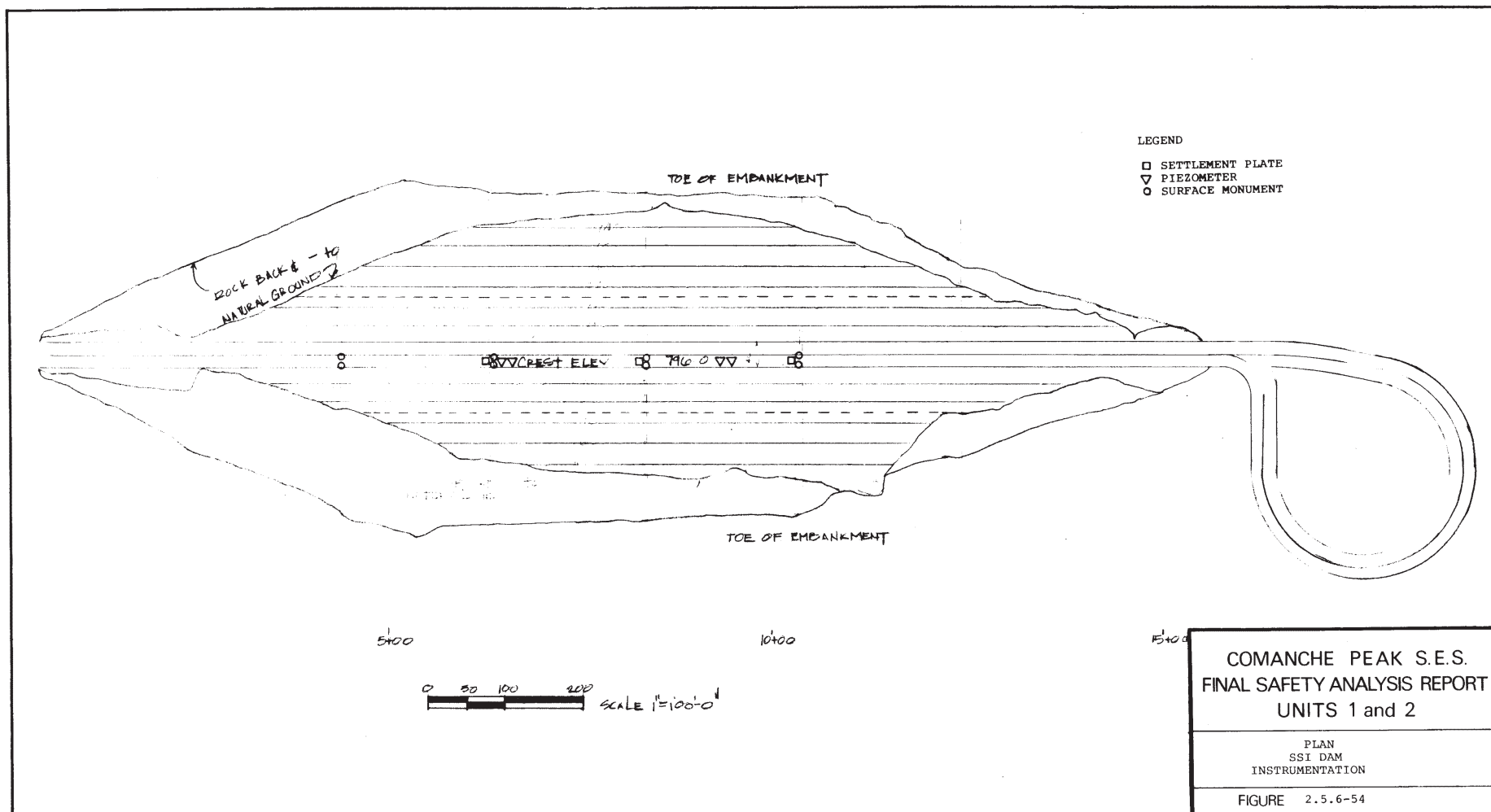
$$\bar{a} \approx \frac{v}{\Delta t} \approx \frac{11}{0.1} = 110 \text{ ft/sec}^2 \approx 4g$$

COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

ROCK ACCELERATION ANALYSIS

FIGURE 2.5.6-52

COMANCHE PEAK S.E.S. FINAL SAFETY ANALYSIS REPORT UNITS 1 and 2	
SECTION AND PROFILE OF	INSTRUMENTATION ON SSI DAM
FIGURE	2.5.6-53





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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2
INSTRUMENTATION
SQUAW CREEK DAM

FIGURE 2.5.6-55

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UNITS 1 and 2

LOG OF BORINGS
SSI-1

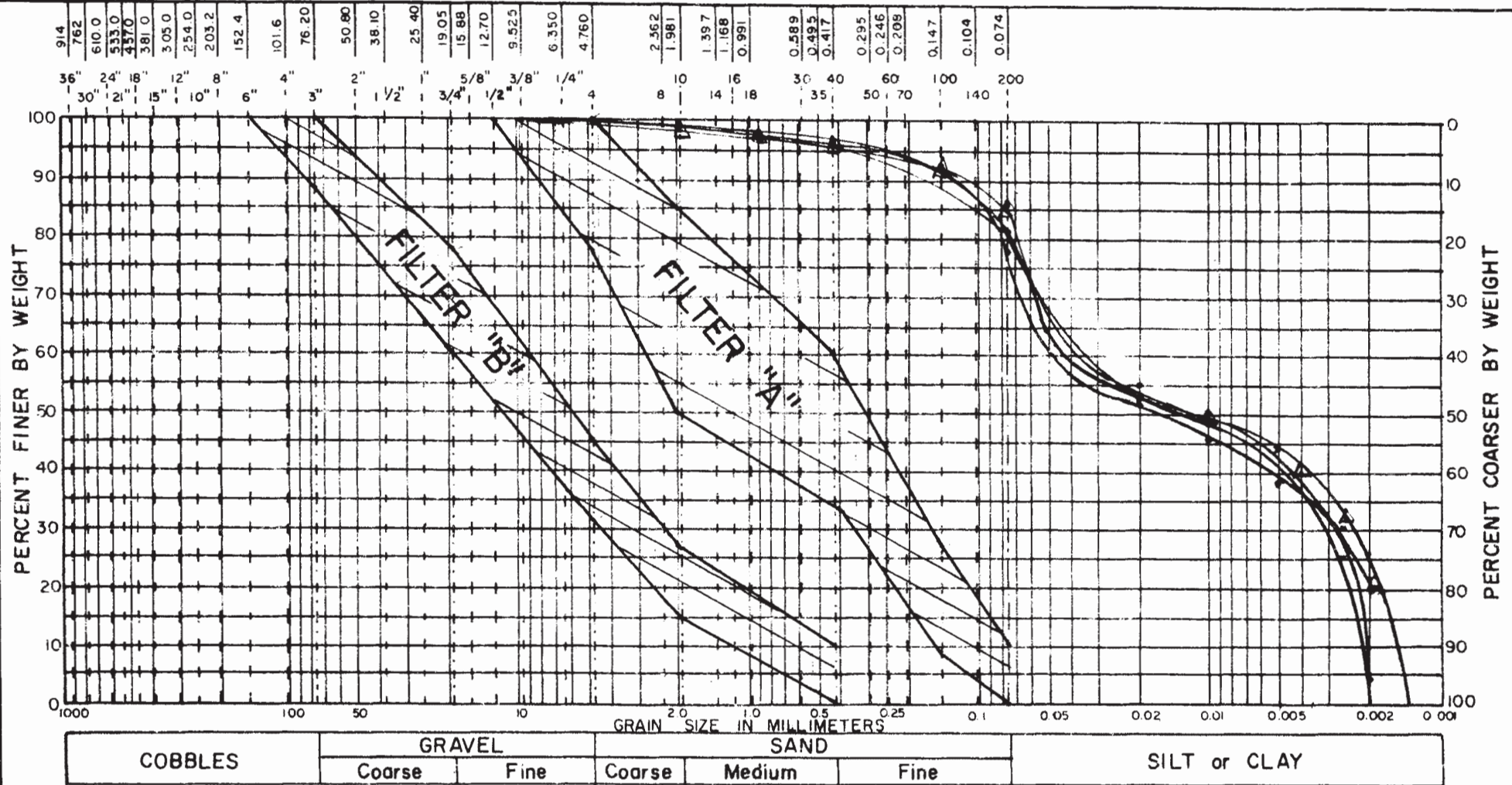
FIGURE 2.5A-1

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

LOG OF BORINGS
SSI-2

FIGURE 2.5A-2

U.S. STANDARD SIEVE SIZES



Hole No. Splice No.	Depth	Legend	LL	PL	PI	LS	Classification	SUBJECT
SSI-1	7.5-10.0		40	20	20		CL	IMPERVIOUS CORE
SSI-1	18.0-19.5		40	22	18		CL	IMPERVIOUS CORE
SSI-2	10.5-12.0		39	19	20		CL	IMPERVIOUS CORE
SSI-2	19.5-21.0		39	20	19		CL	IMPERVIOUS CORE

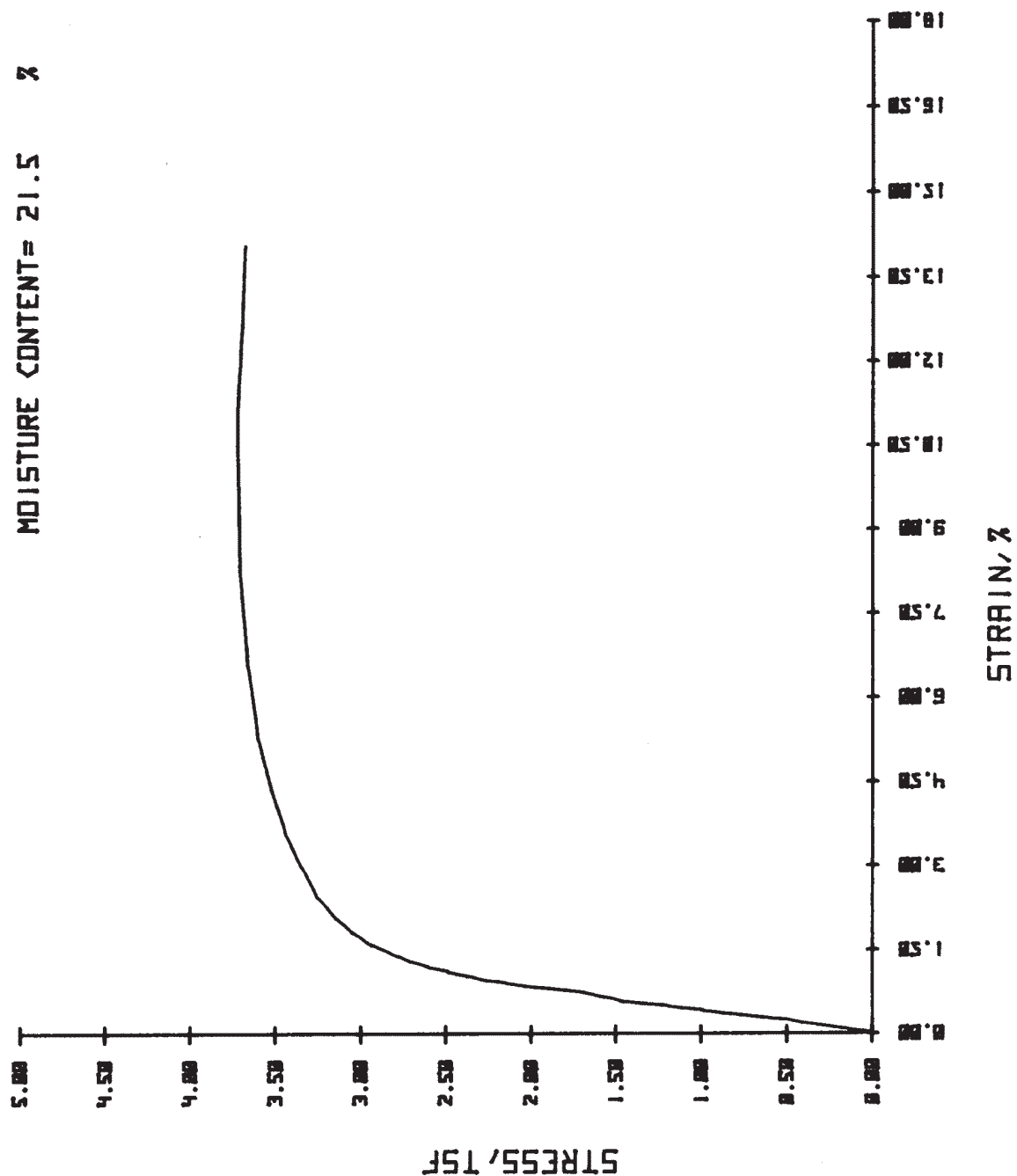
COMANCHE PEAK S.E.S.
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SSI DAM
REPRESENTATIVE GRADATION
OF
IMPERVIOUS CORE

FIGURE 2.5A-3

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BORING NO.: 551-2 SAMPLE: 1.5-3.0
 UNIT DRY WEIGHT= 106.8 LBS/CU.FT.
 MOISTURE CONTENT= 21.5 %

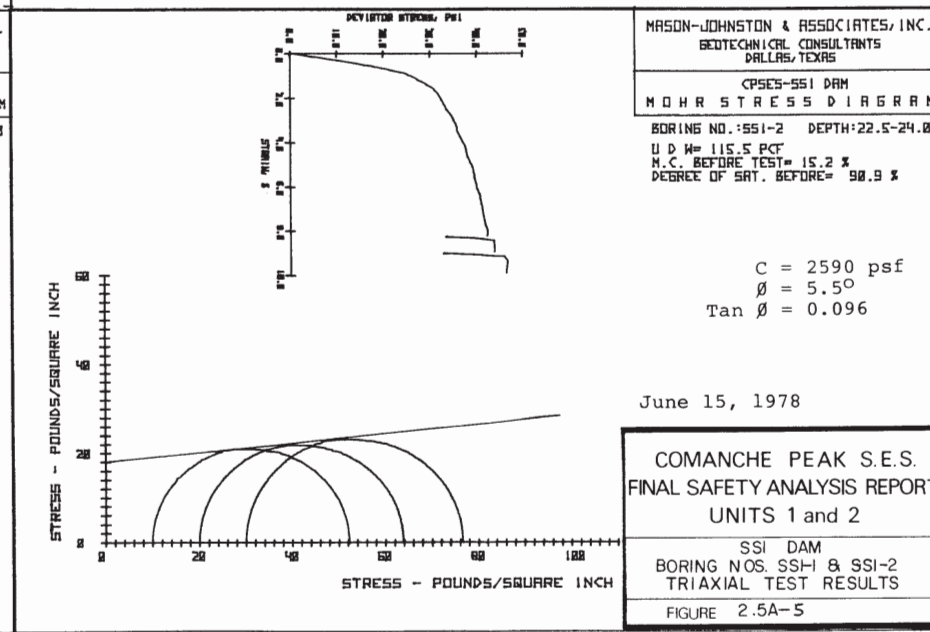
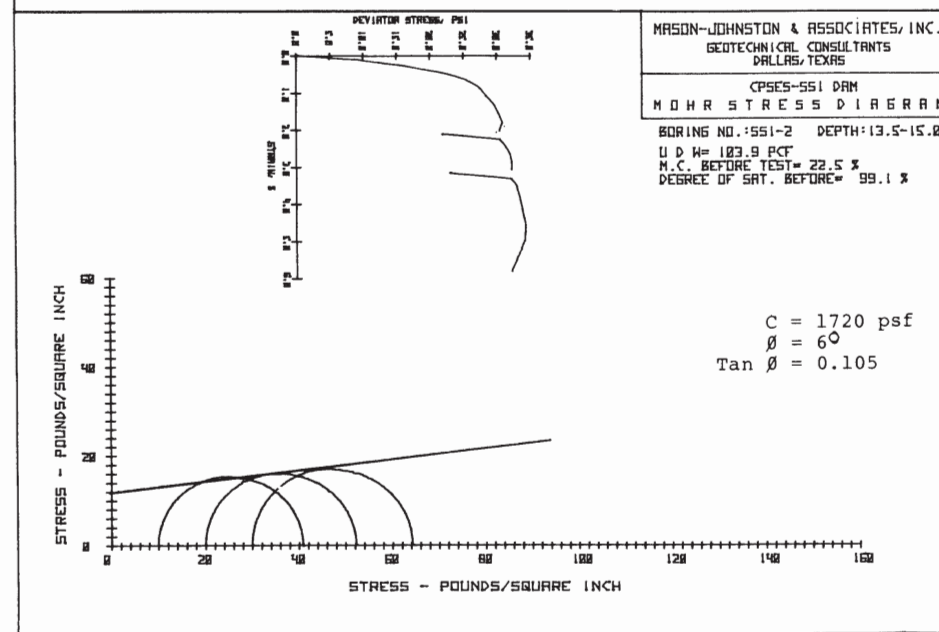
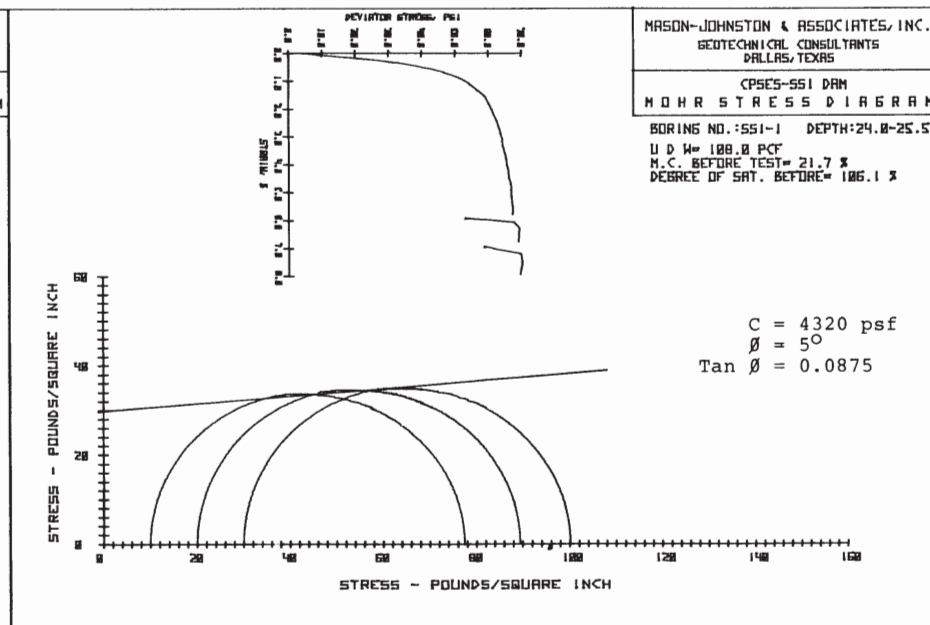
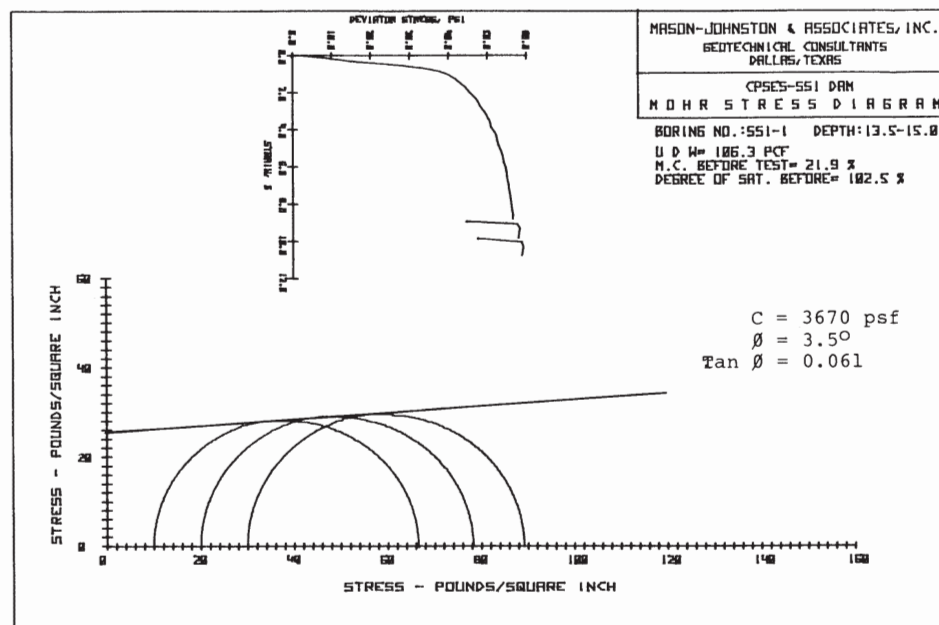


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COMANCHE PEAK S.E.S.
 FINAL SAFETY ANALYSIS REPORT
 UNITS 1 and 2

SSI DAM
 TYPICAL STRESS-STRAIN CURVE
 UNCONFINED COMPRESSION TEST

FIGURE 2.5A-4





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COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

LOG OF BORINGS
IC3

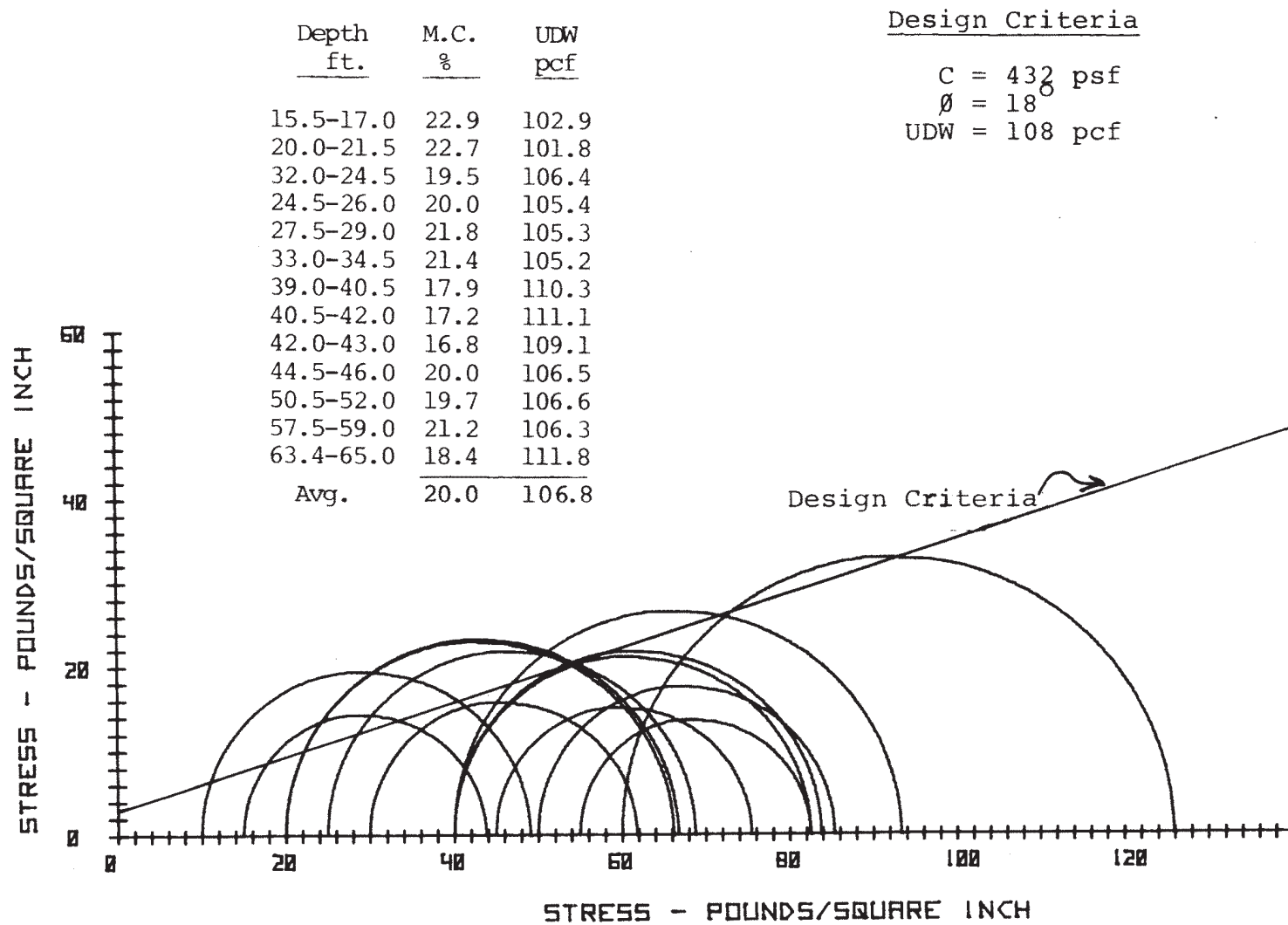
FIGURE 2.5A-6

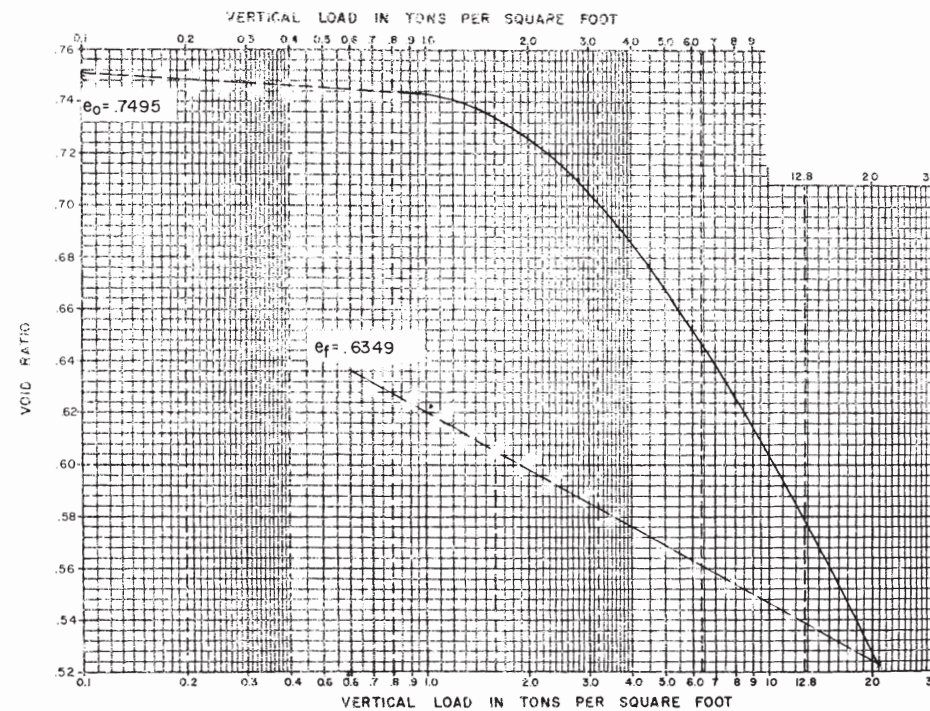
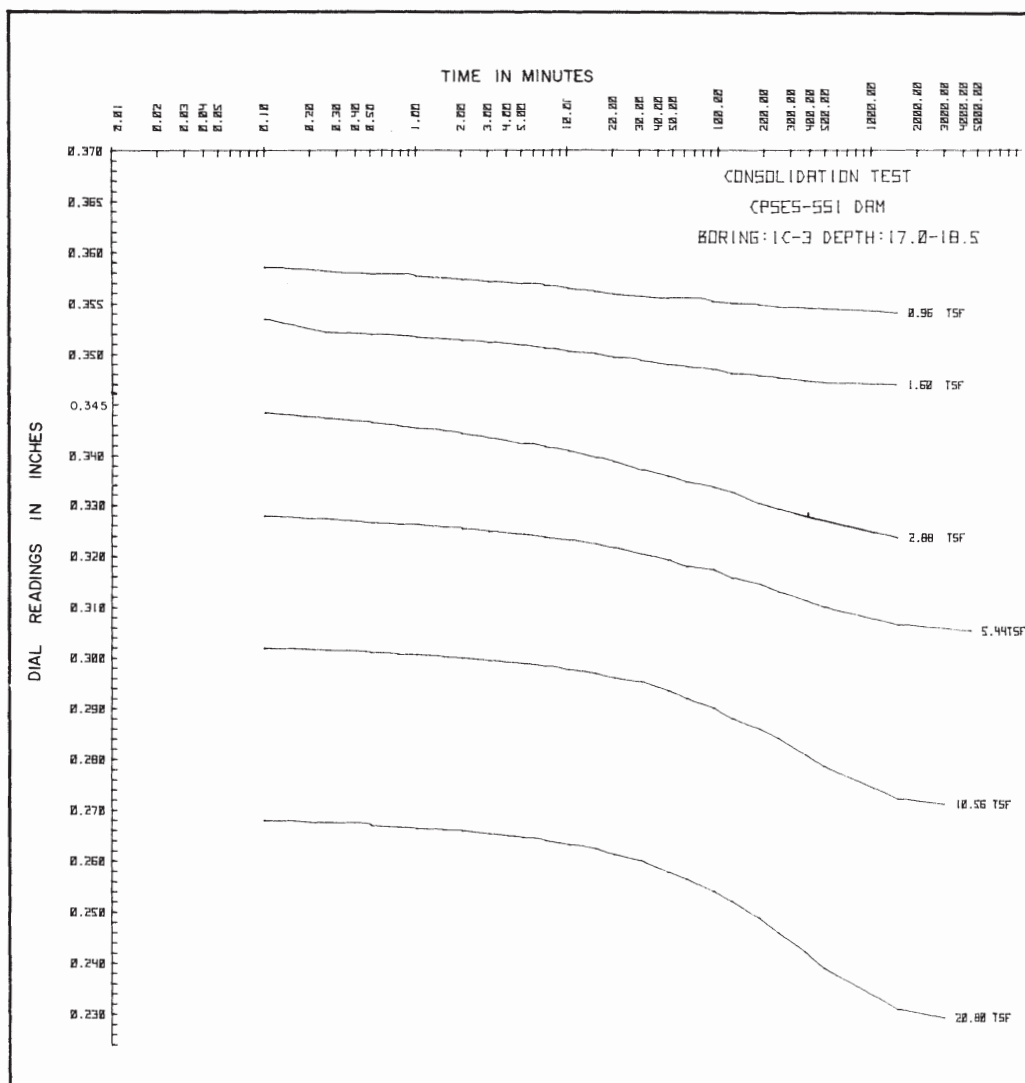
UNITS 1 and 2

SS1 DAM

FIGURE 2.5A-7

JUNE 15, 1978

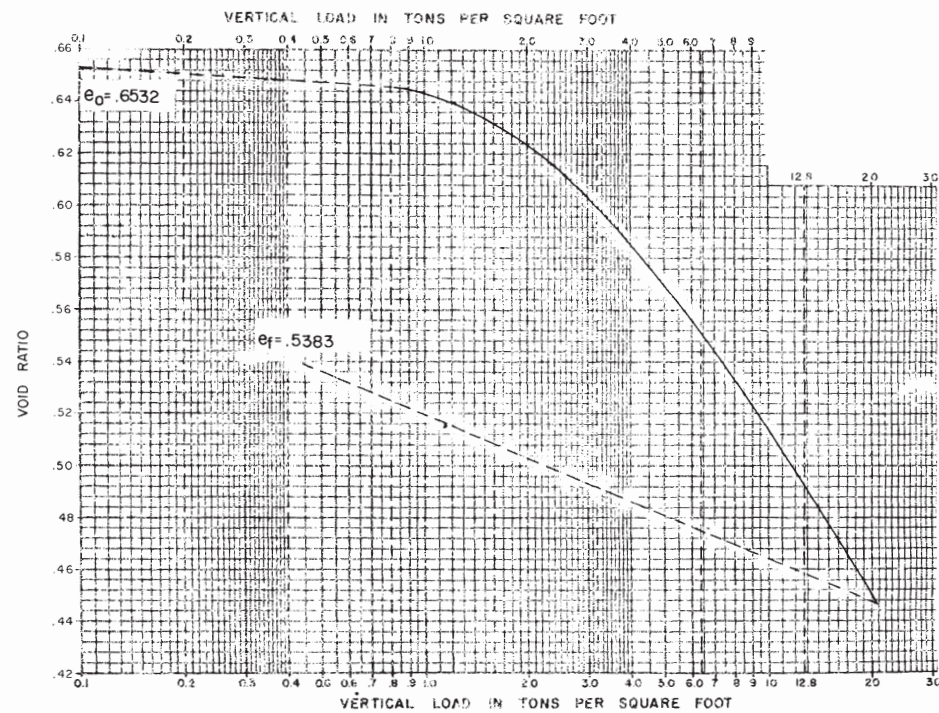
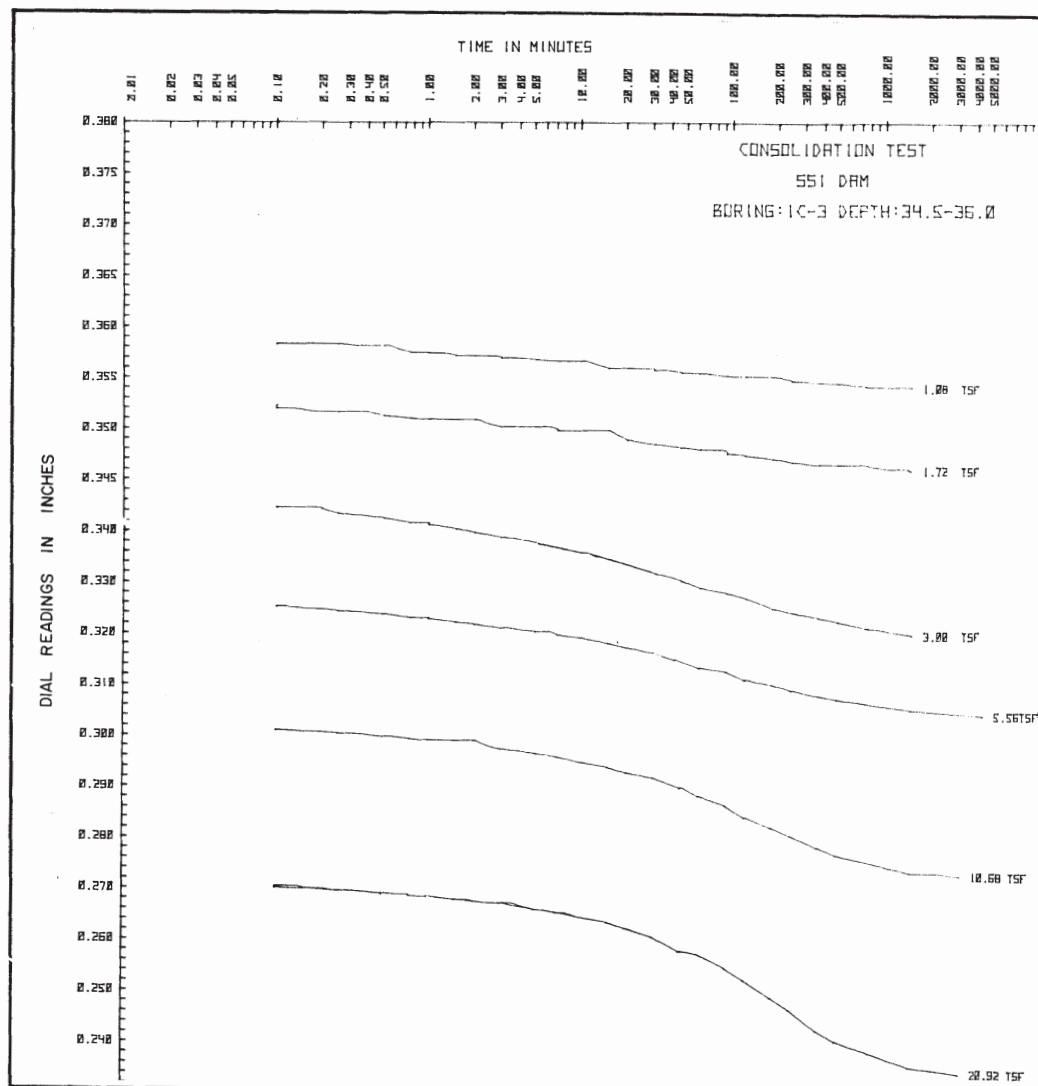




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COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
BORING 1C3
CONSOLIDATION TEST RESULTS
FIGURE 2.5A-8

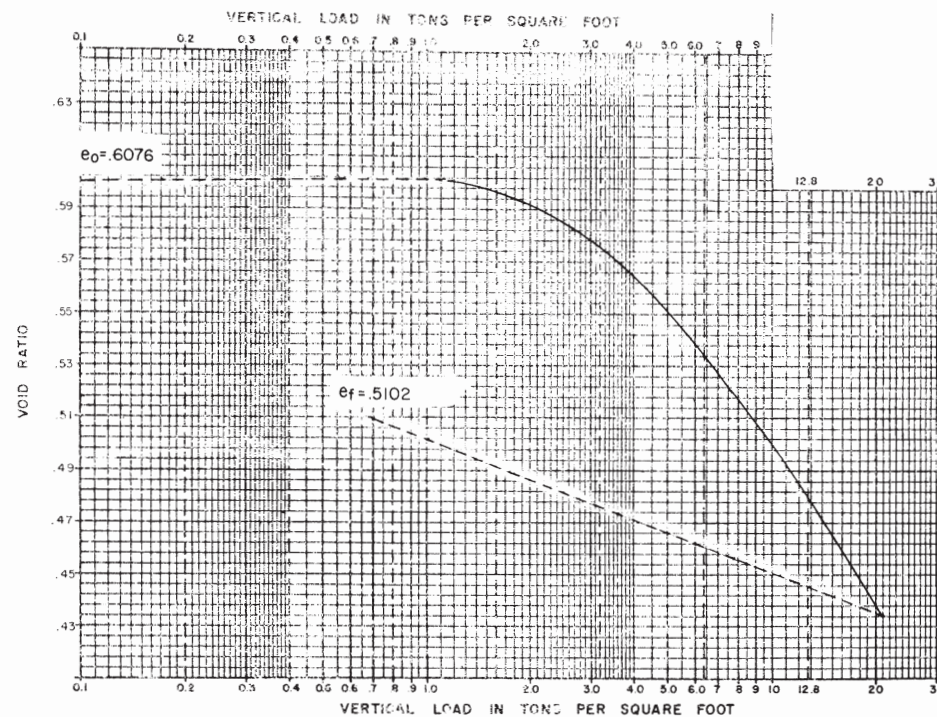
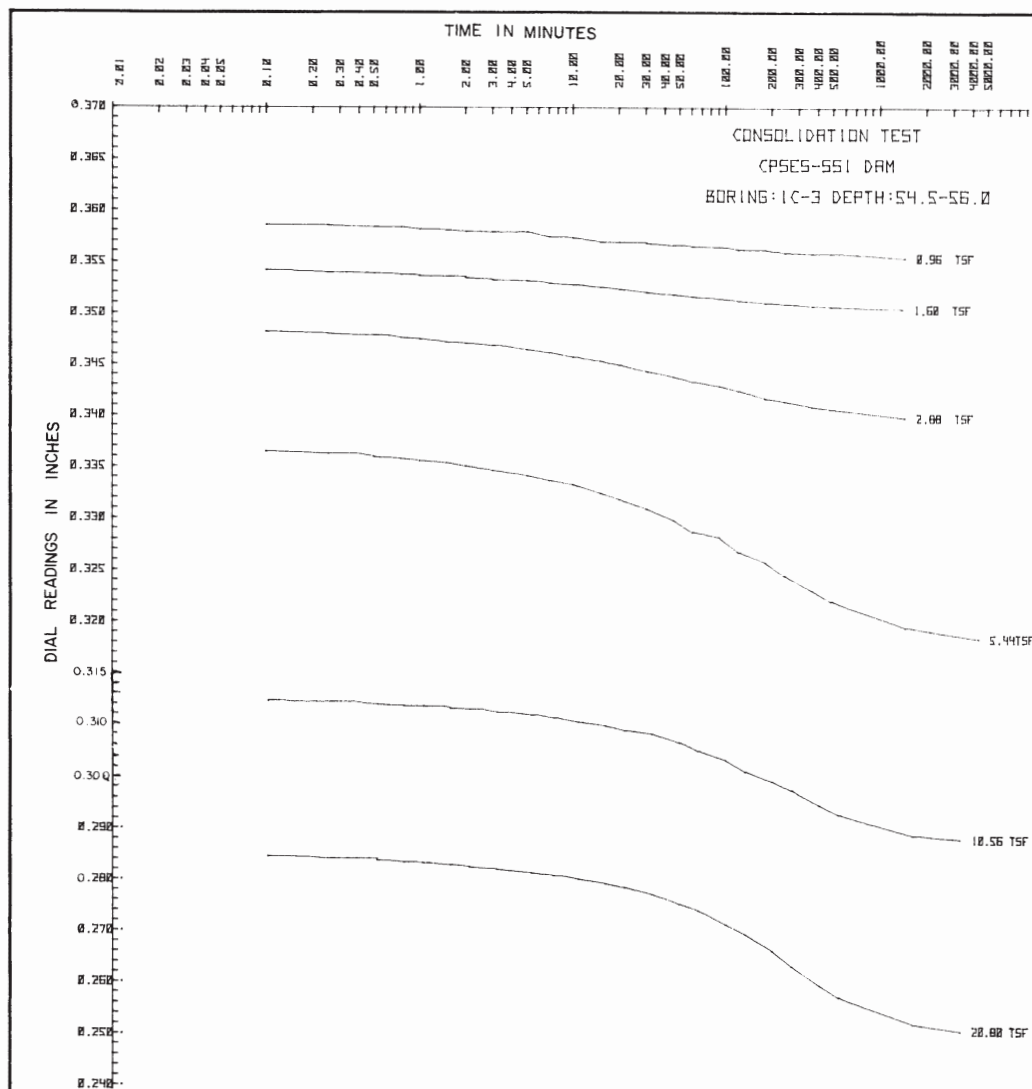


June 15, 1978

COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
BORING IC3
CONSOLIDATION TEST RESULTS

FIGURE 2.5A-9

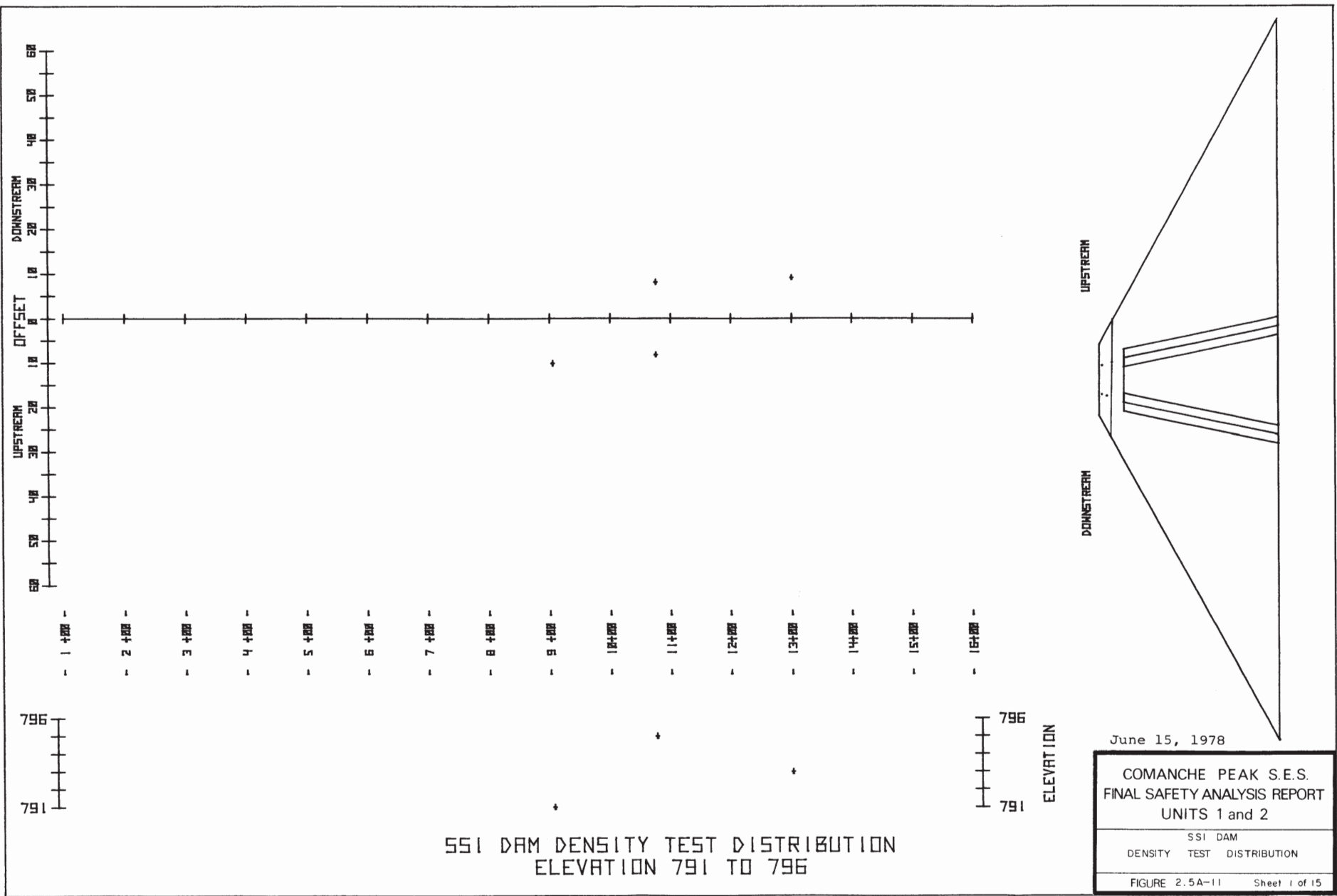


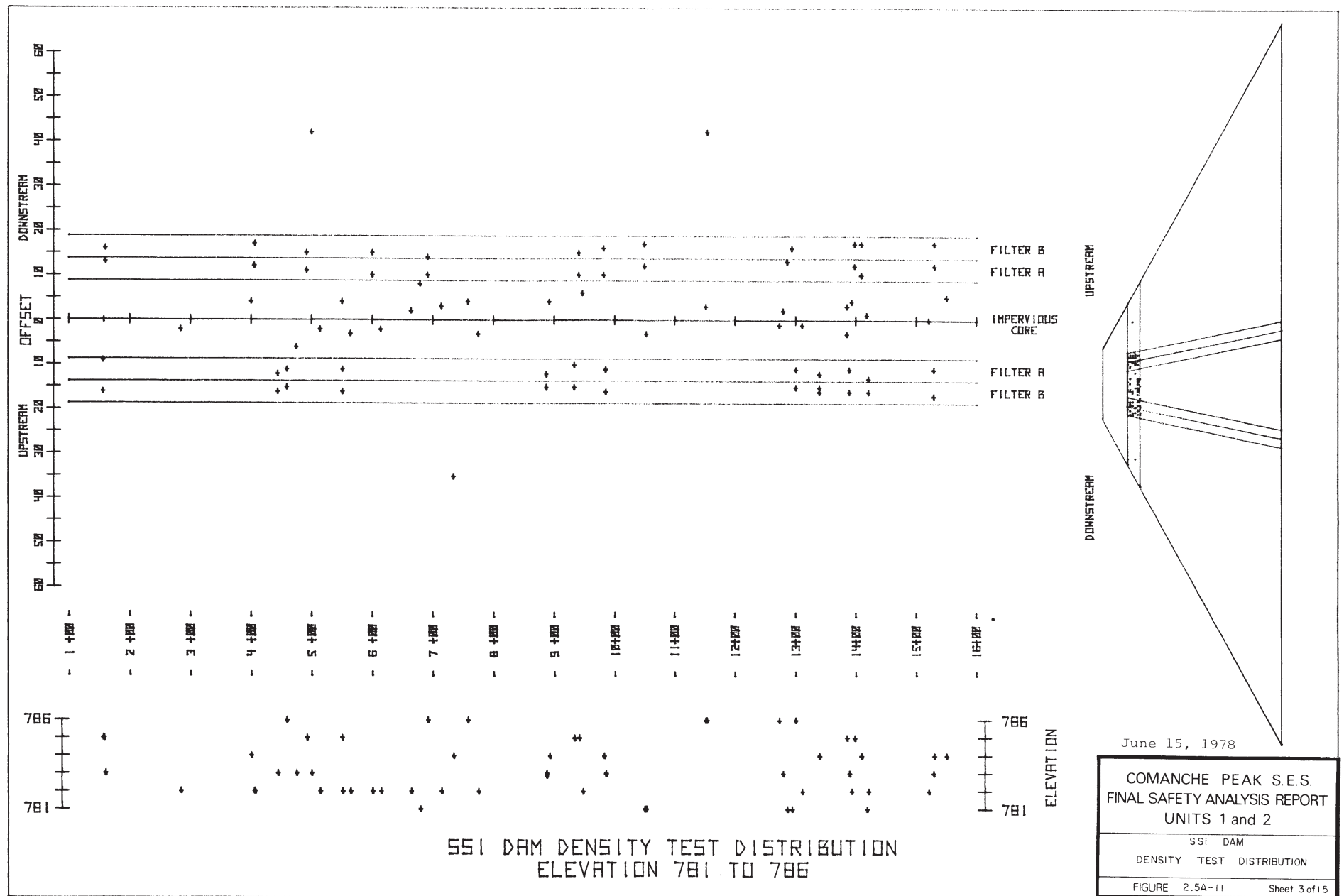
June 15, 1978

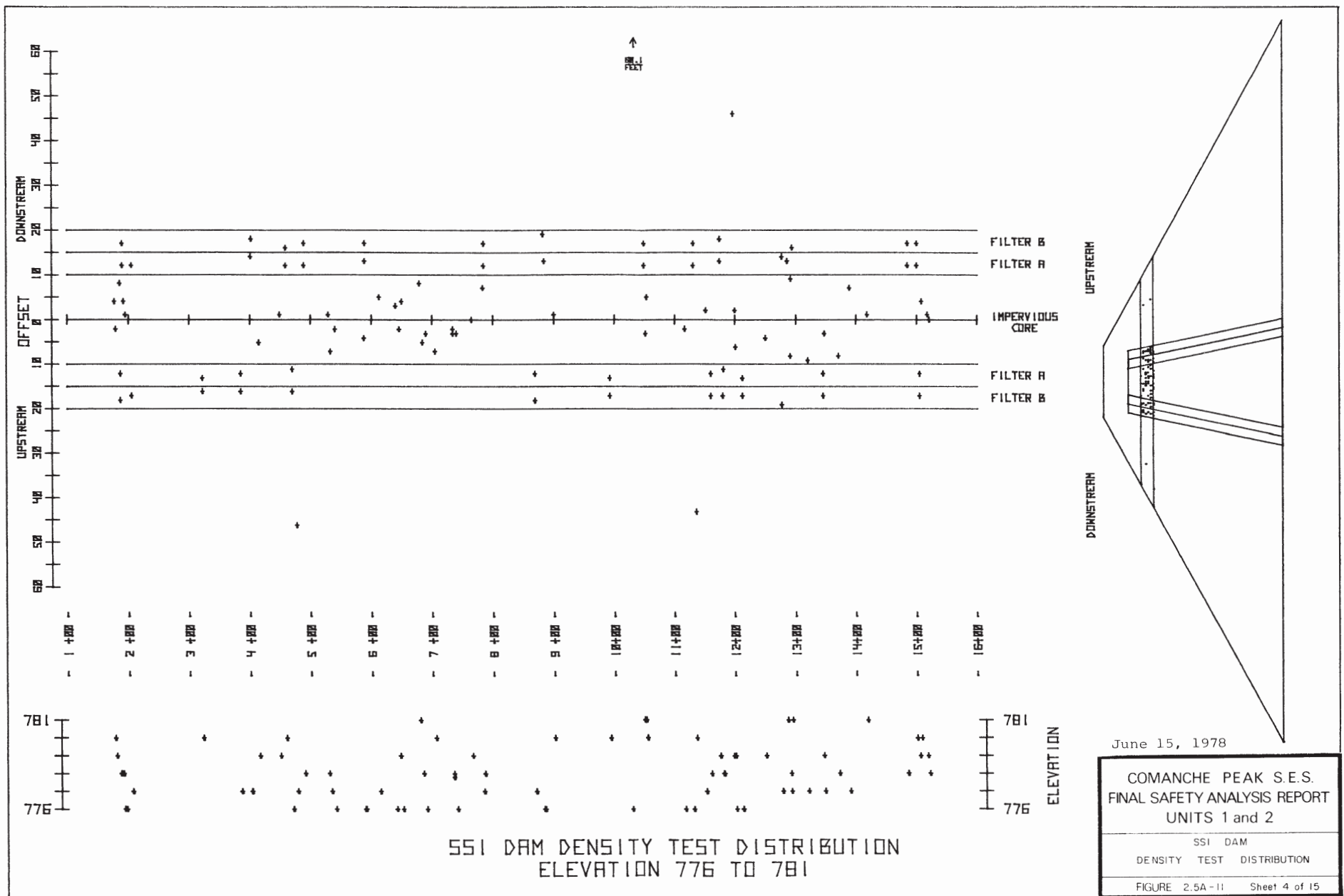
COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

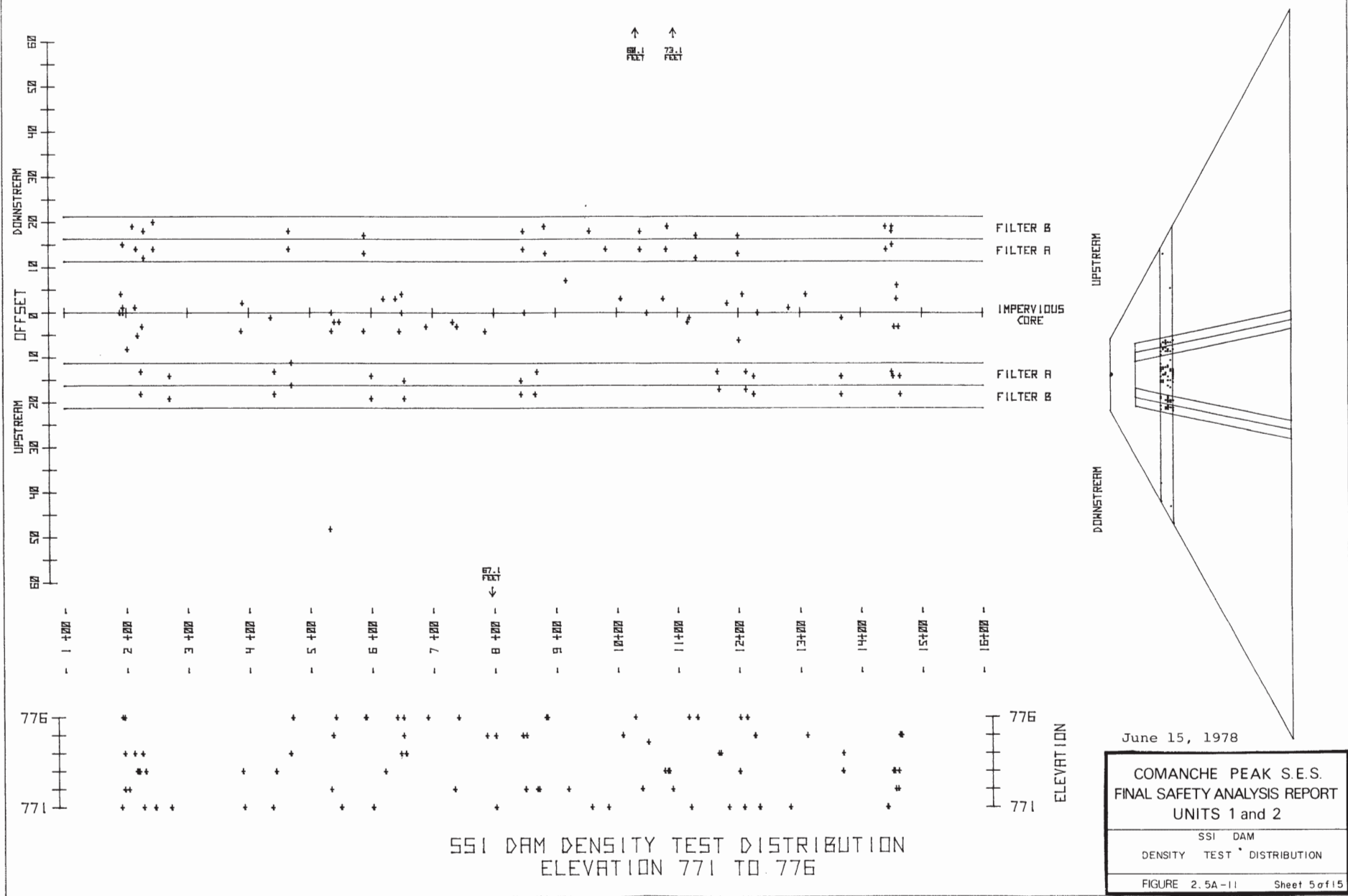
SSI DAM
BORING IC3
CONSOLIDATION TEST RESULTS

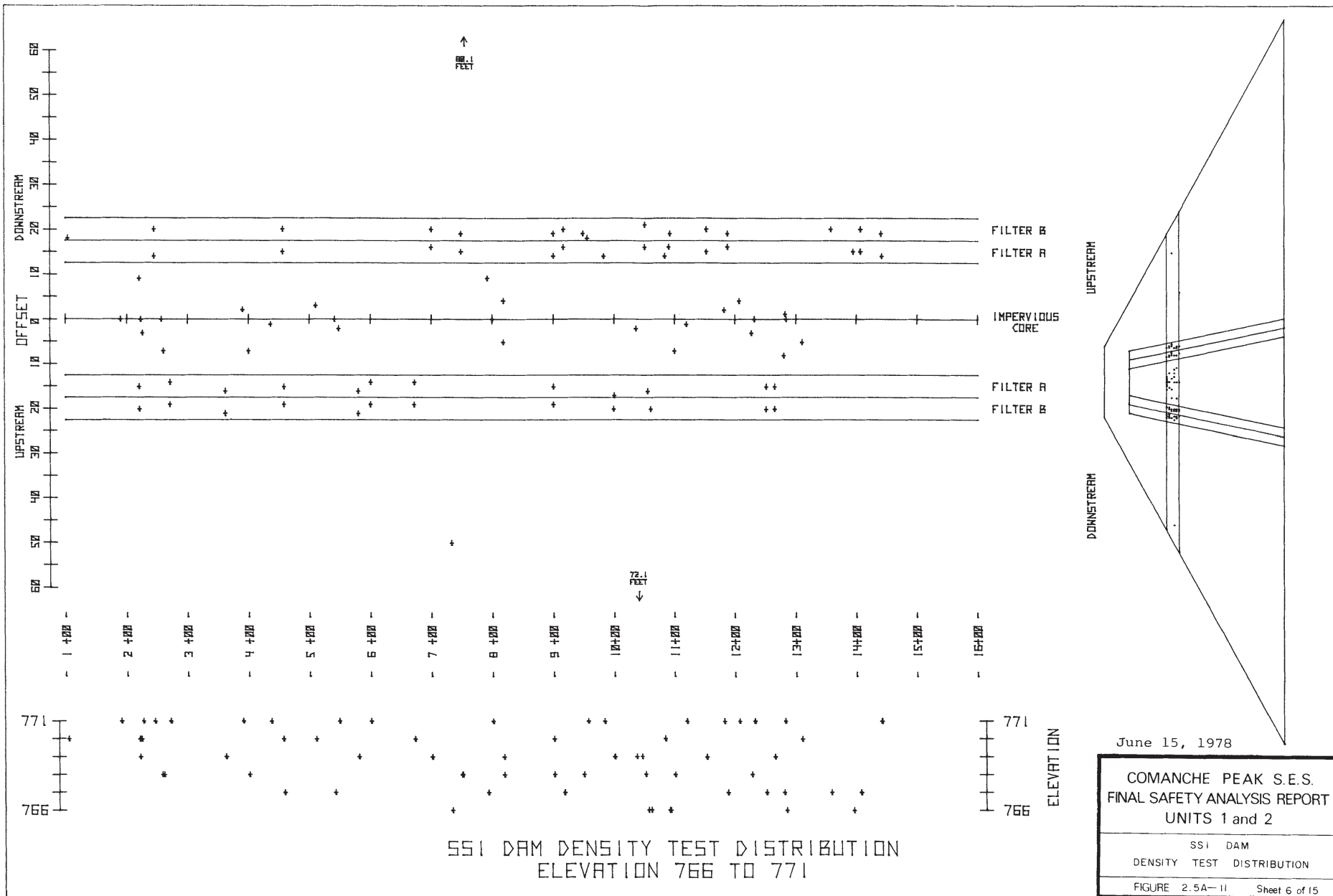
FIGURE 2.5A-10

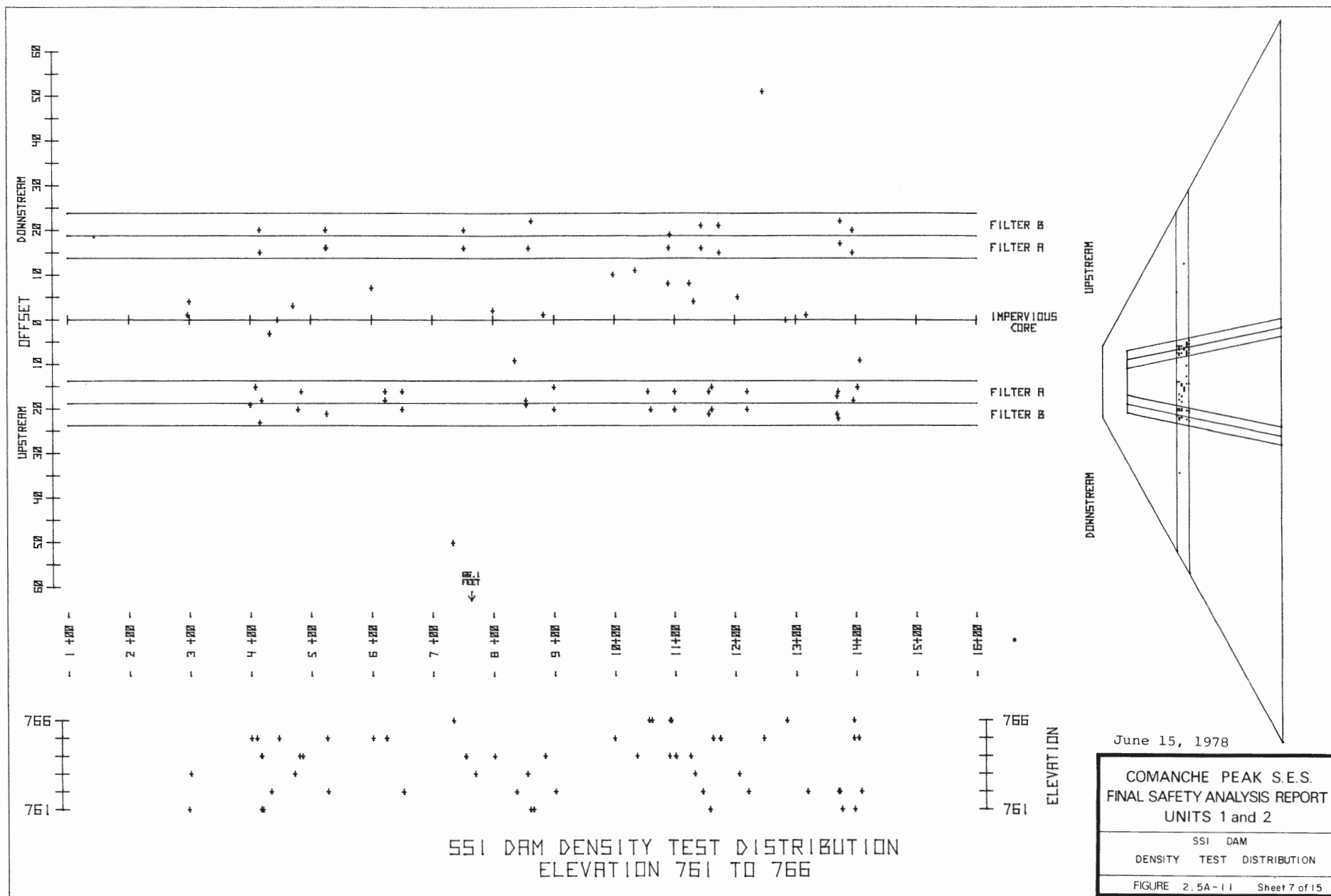


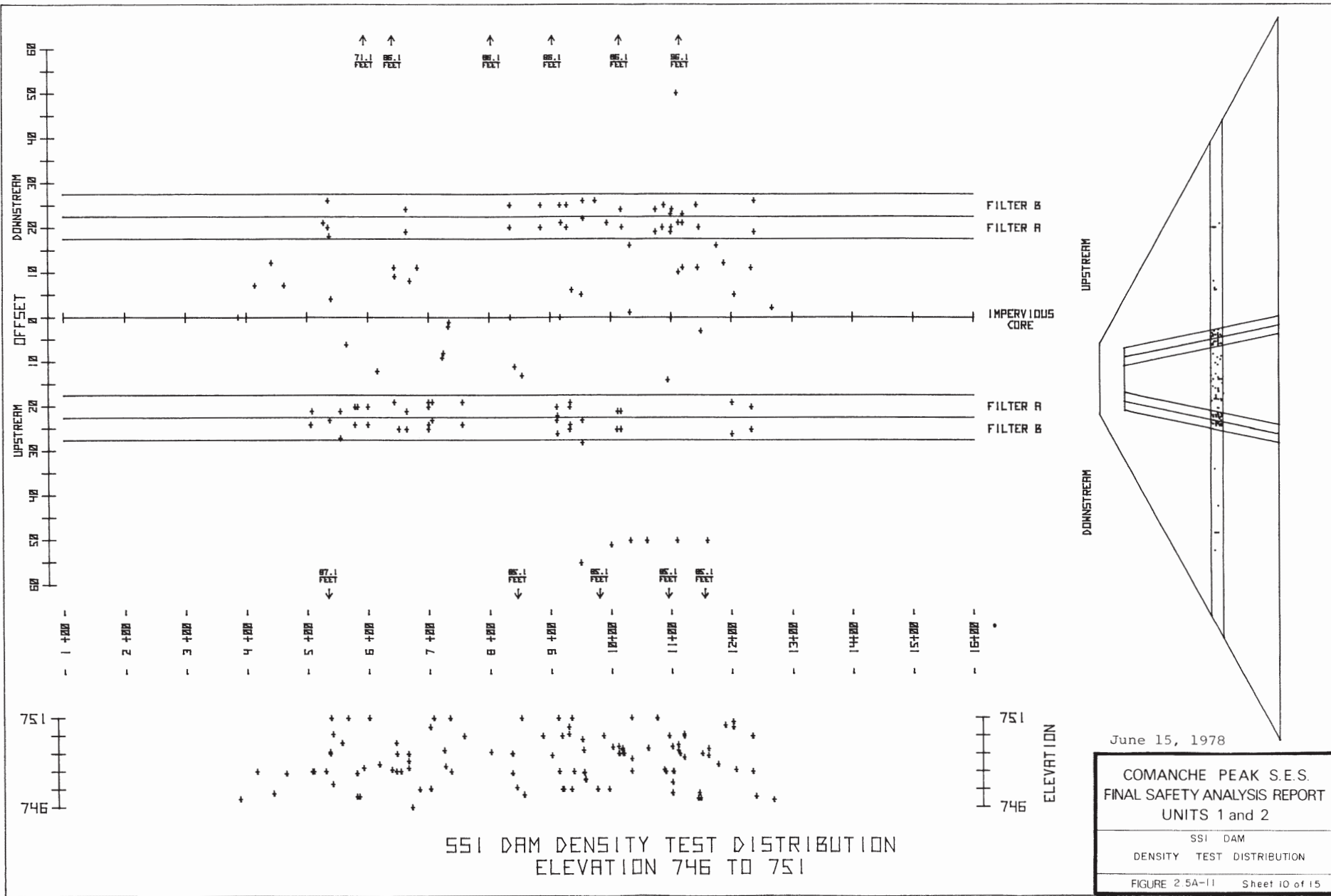


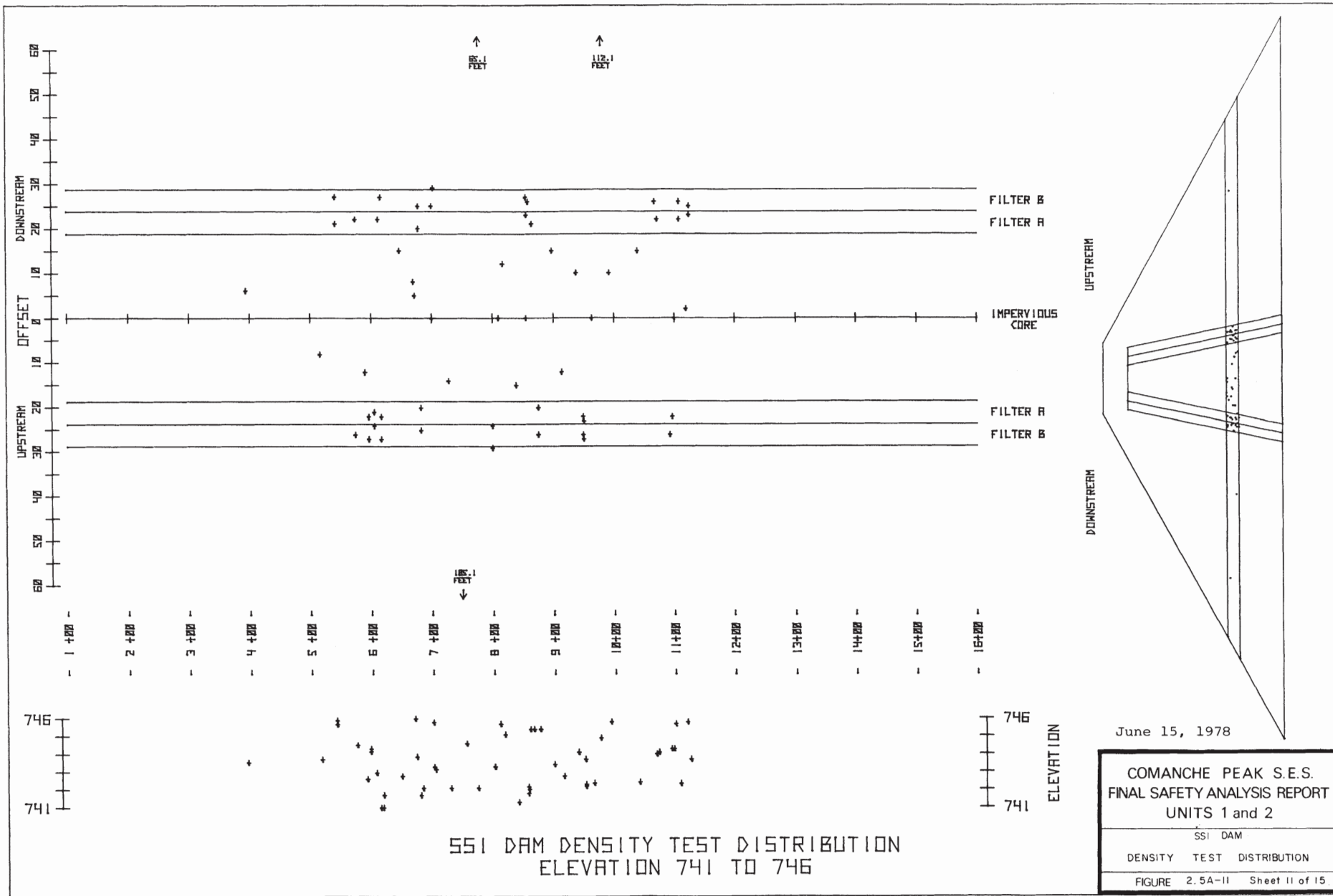


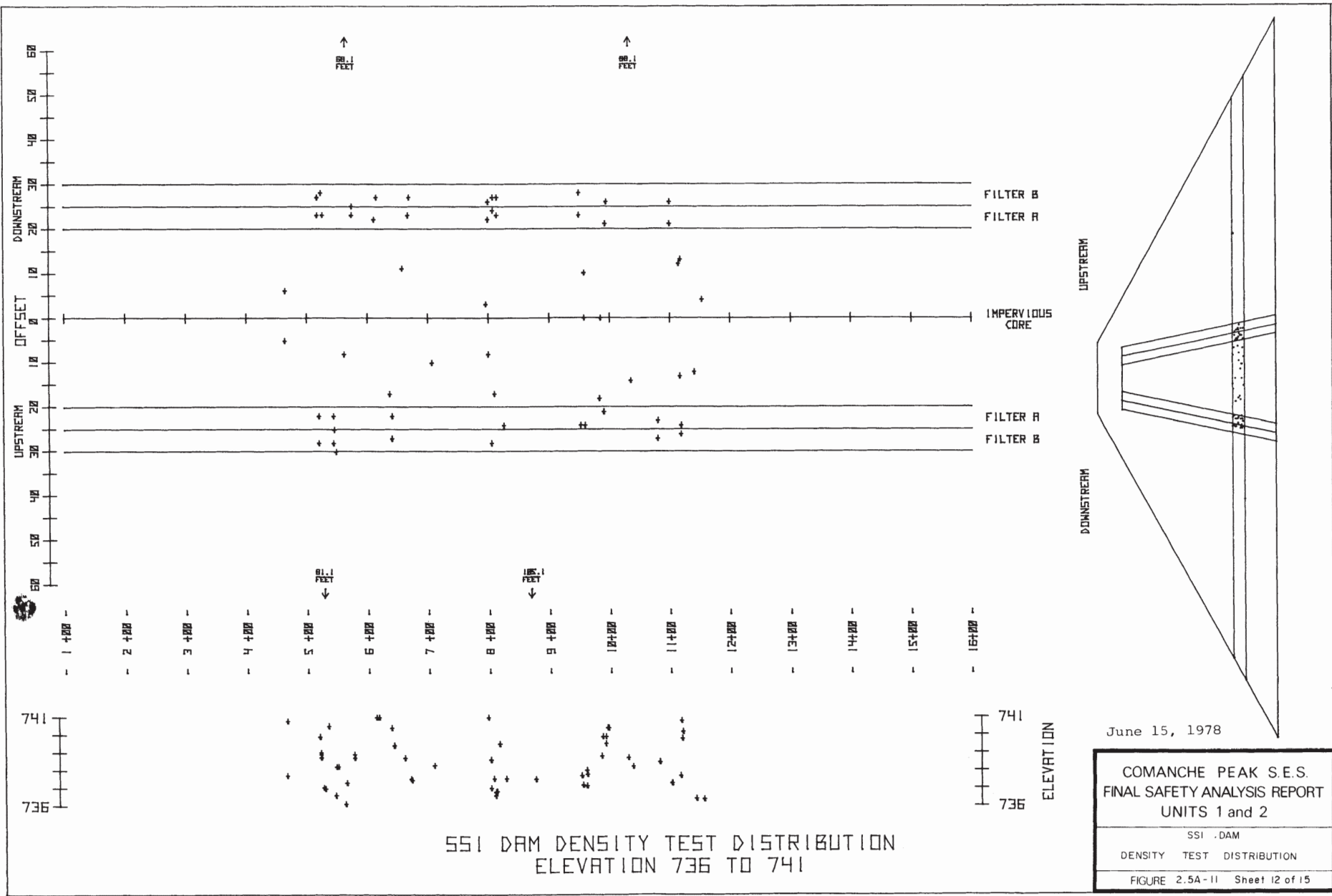


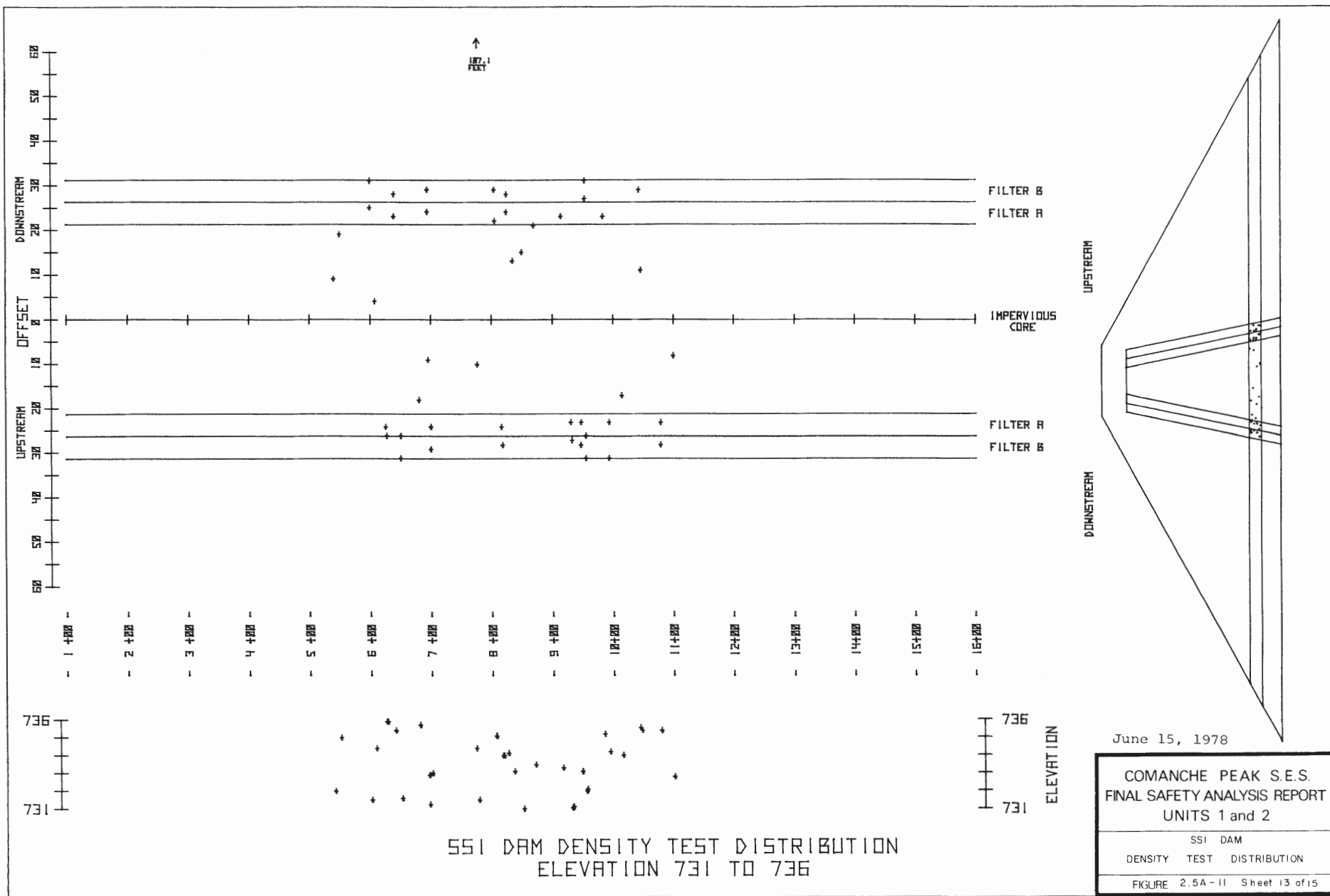


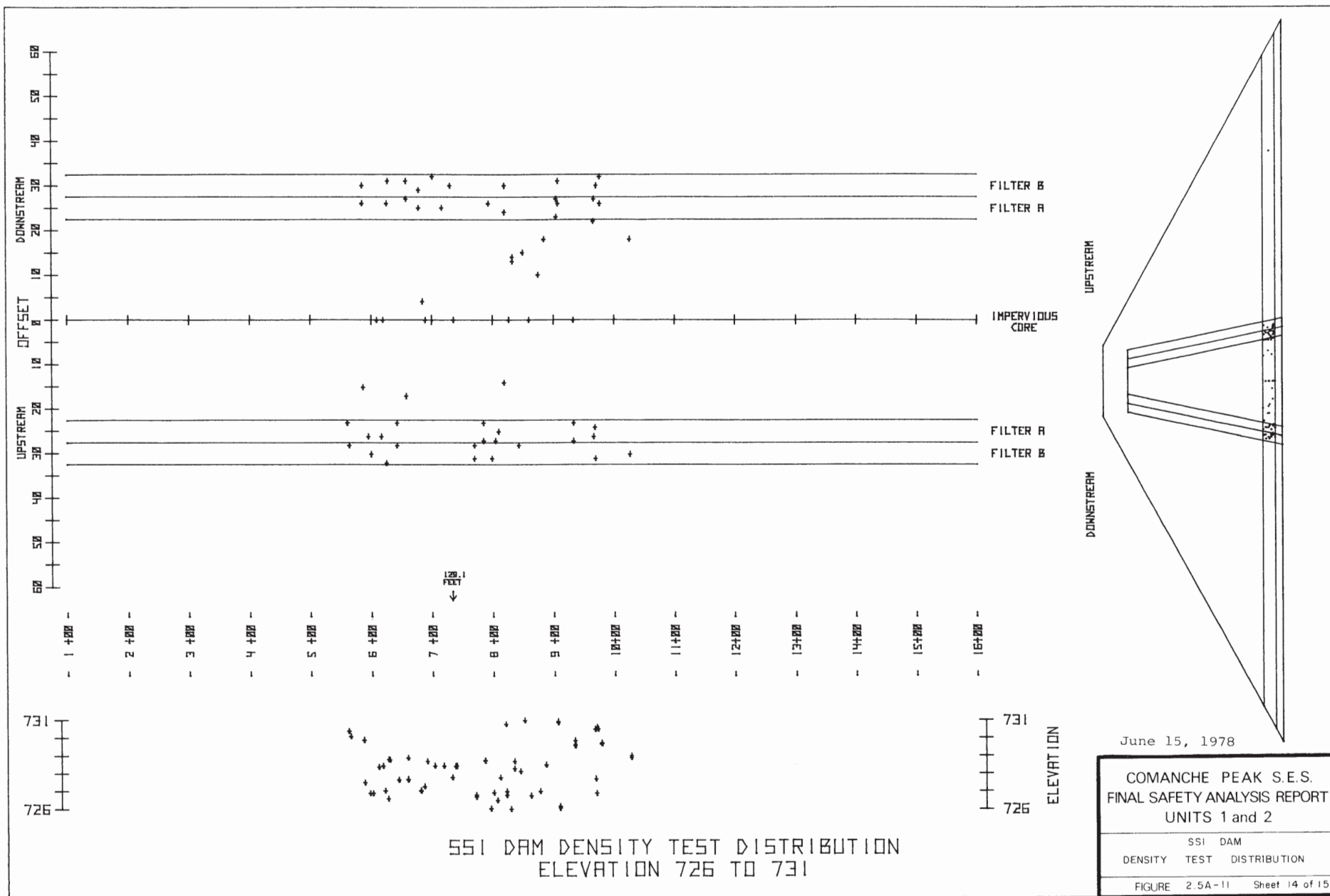


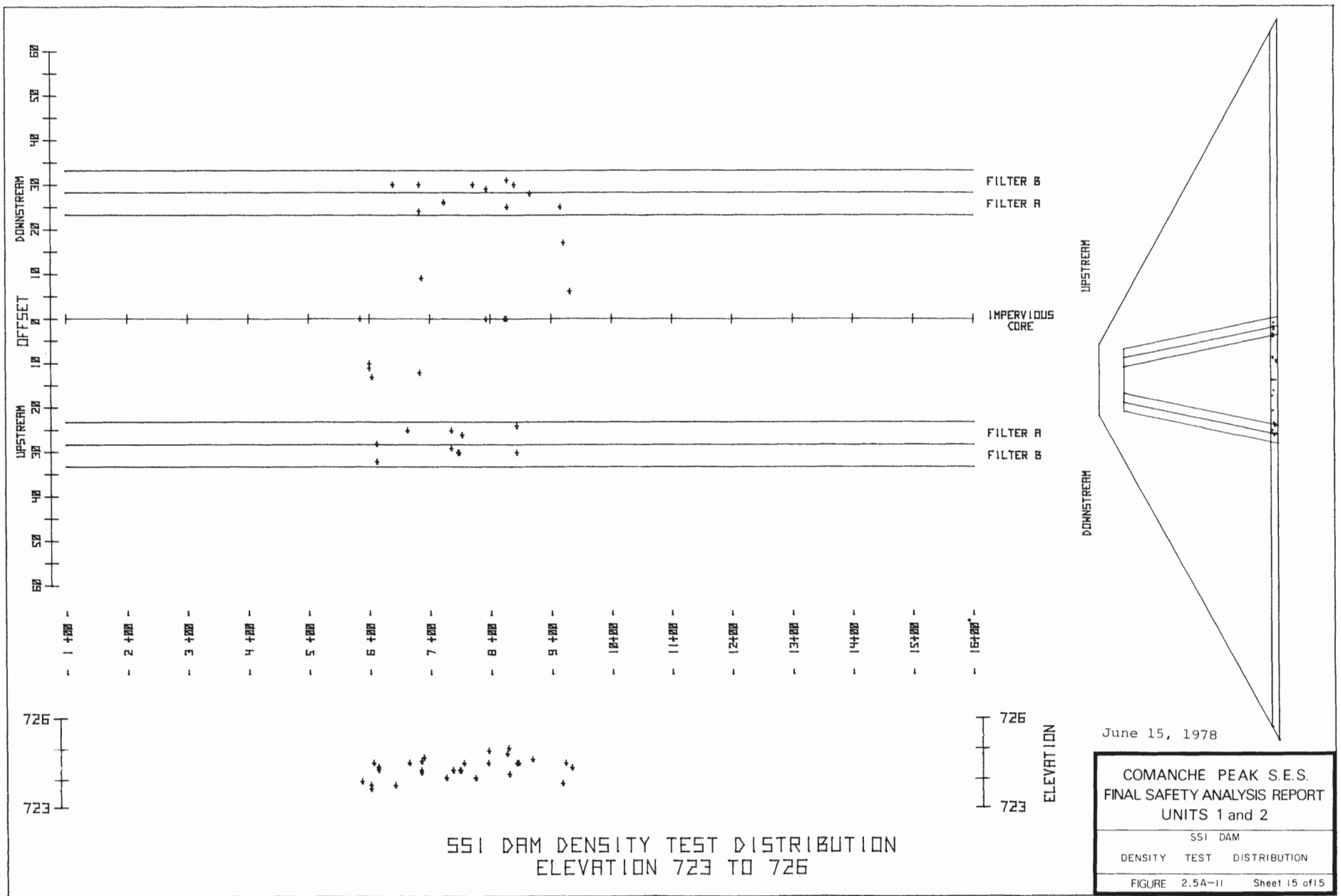


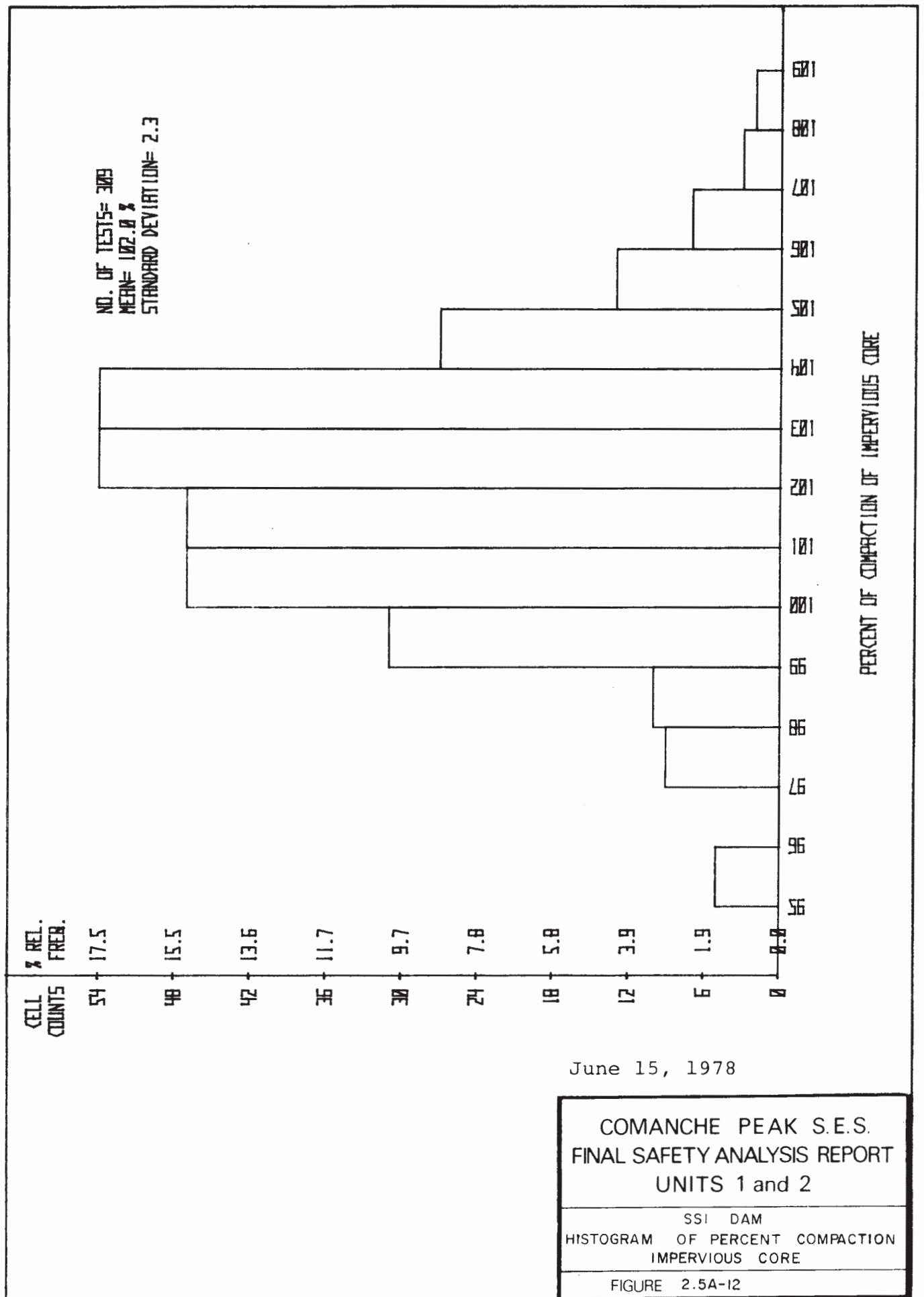


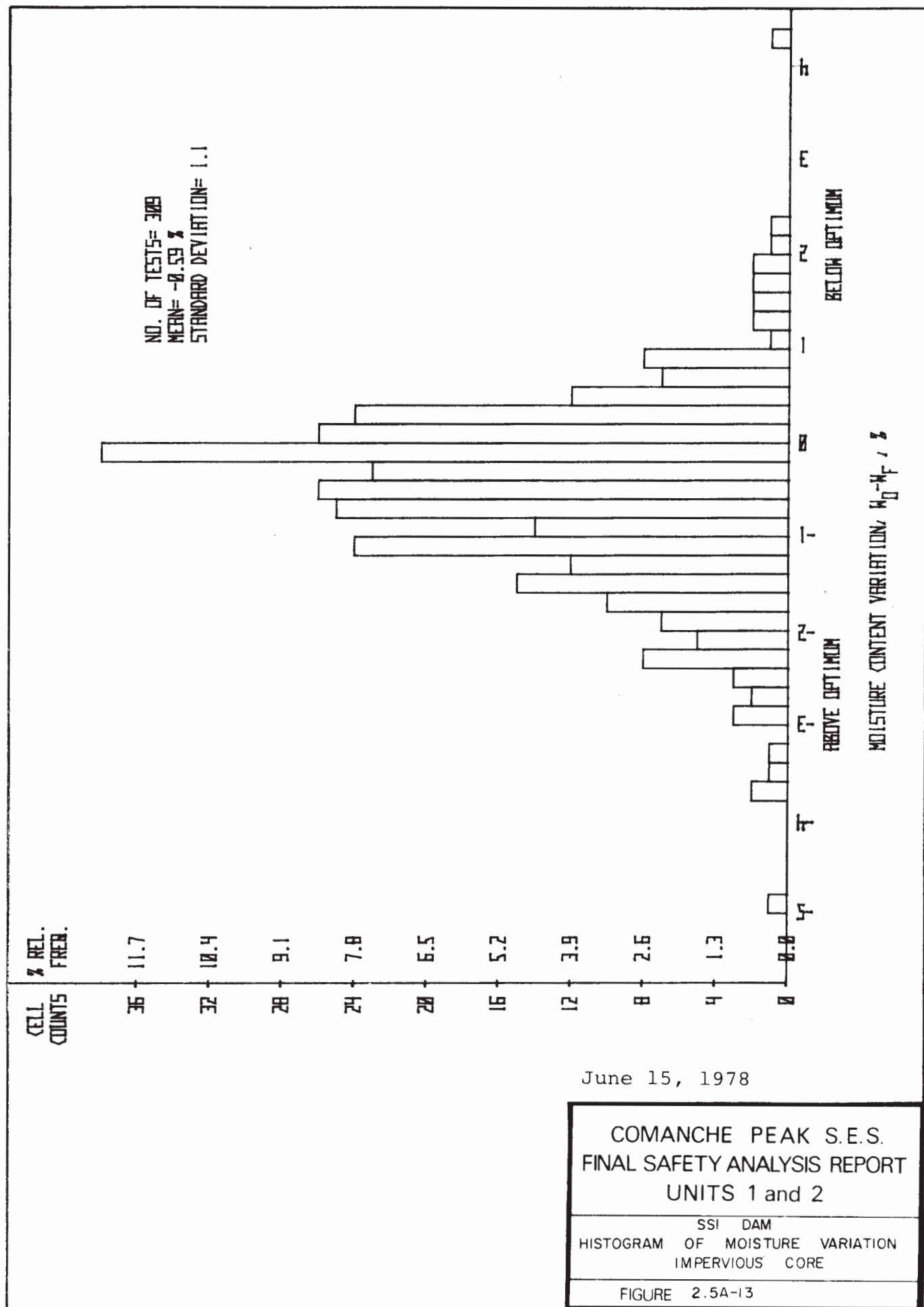


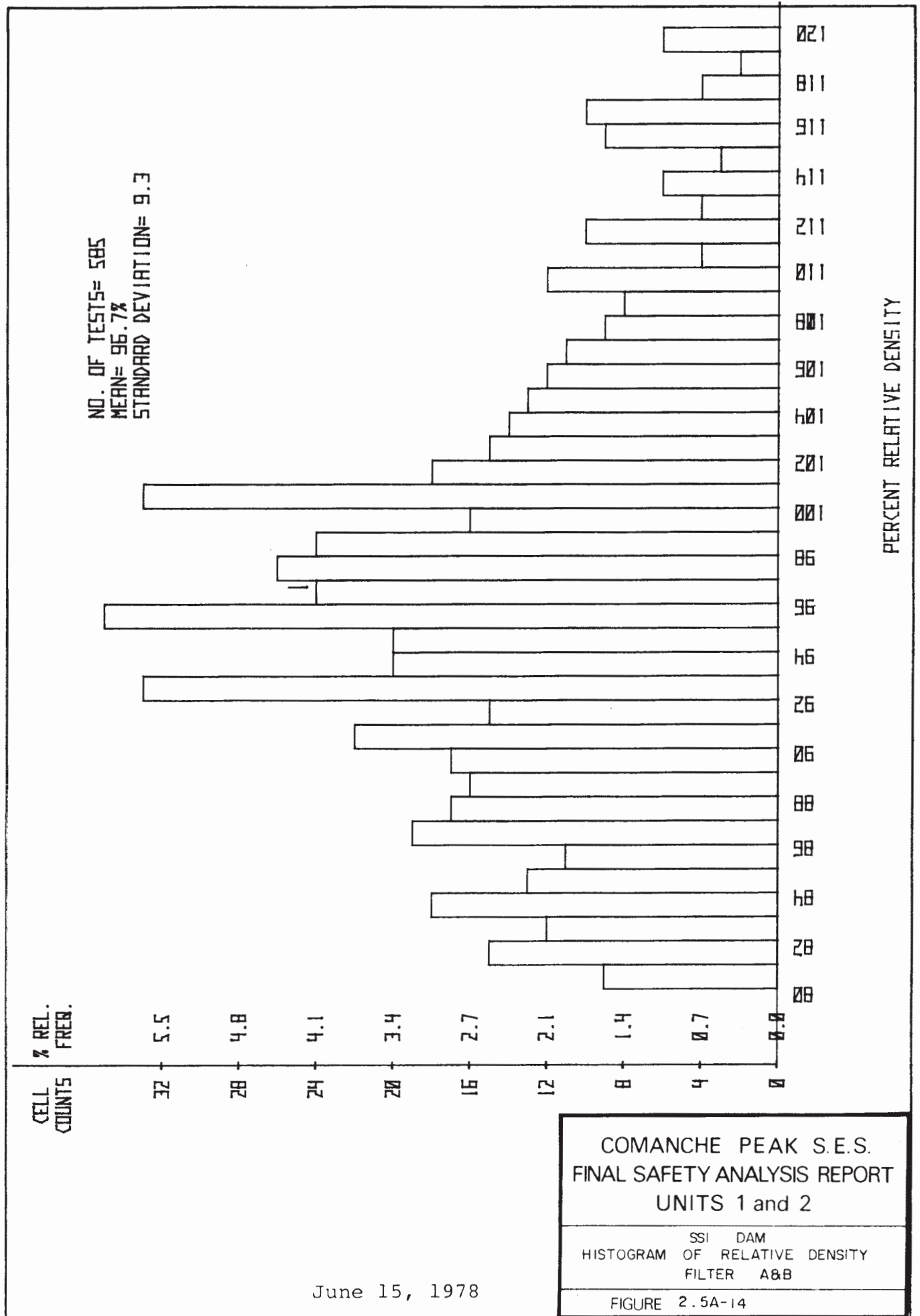




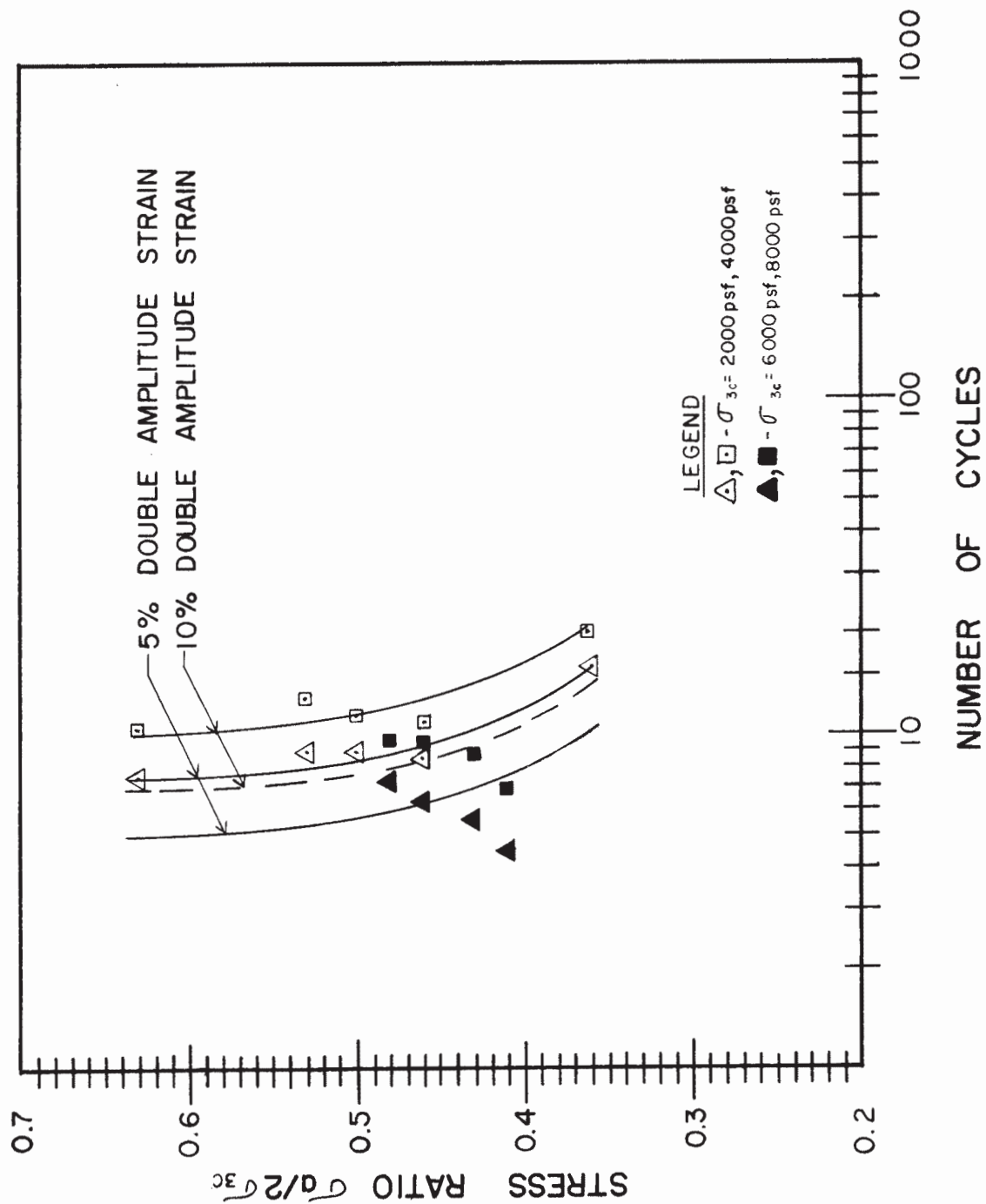








FILTER "A" MATERIAL

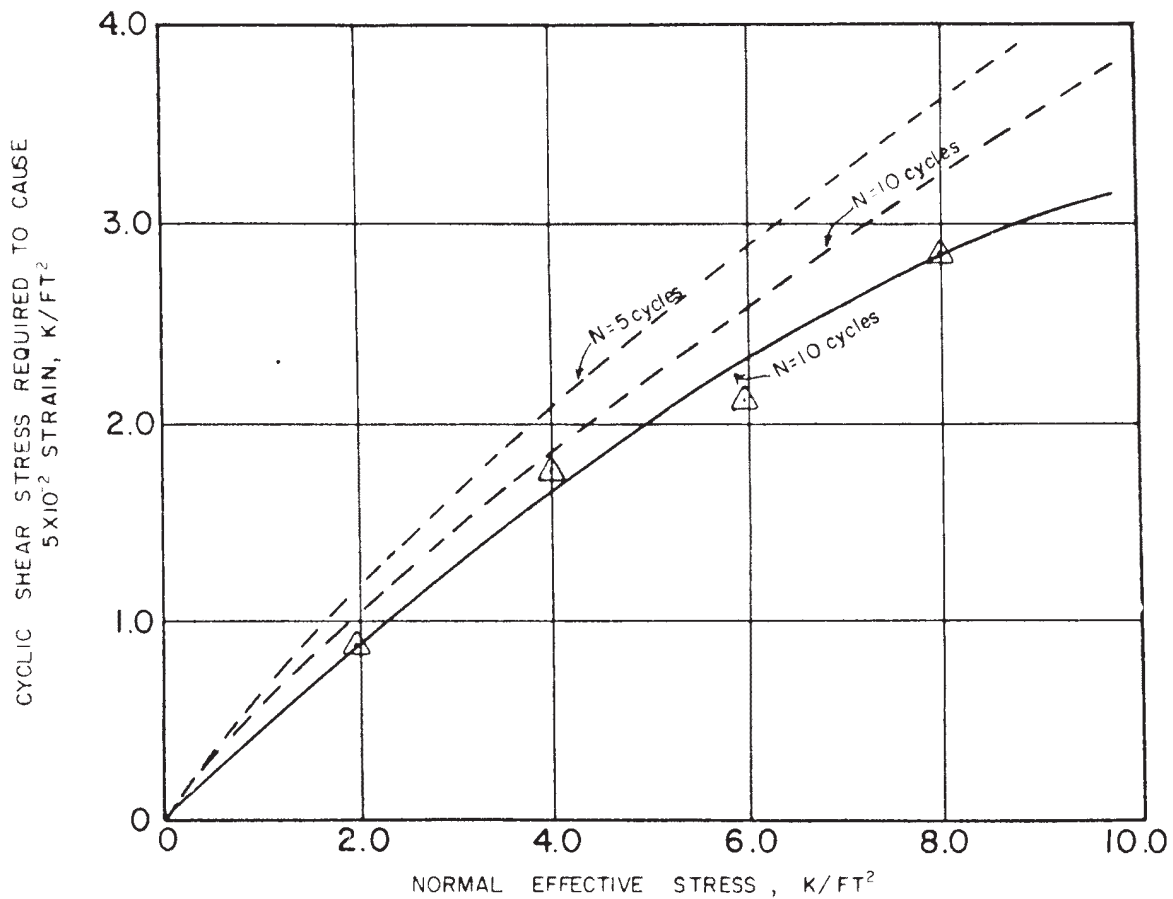


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UNITS 1 and 2

SSI DAM
CYCLIC STRENGTH OF FILTER "A"

FIGURE 2.5A-16

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LEGEND

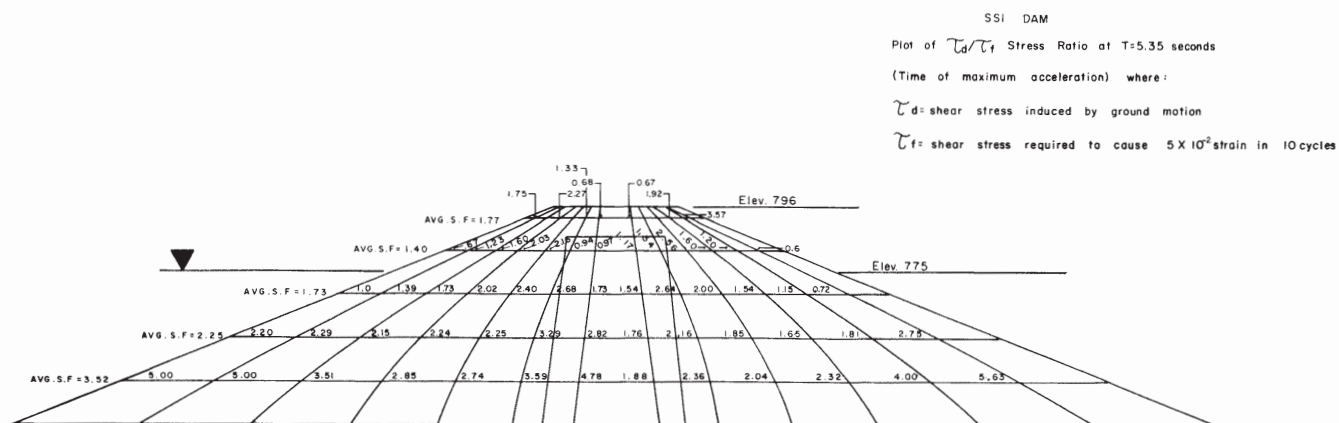
- Curves based on data published from Lee & Seed (1967) for Relative Density of 80%
- Curves based on results of cyclic testing for Relative Density of 80%. See Figure 2.5A-16

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
CYCLIC SHEAR STRESS CRITERIA
FILTER "A" MATERIAL

FIGURE 2.5A-17

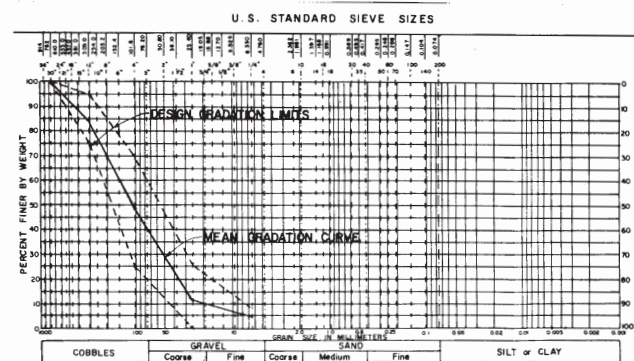
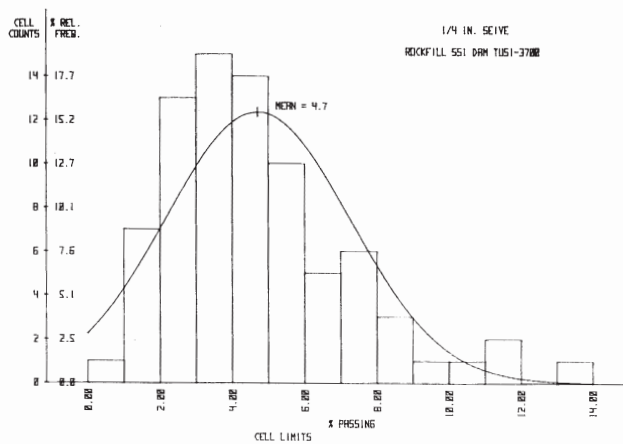
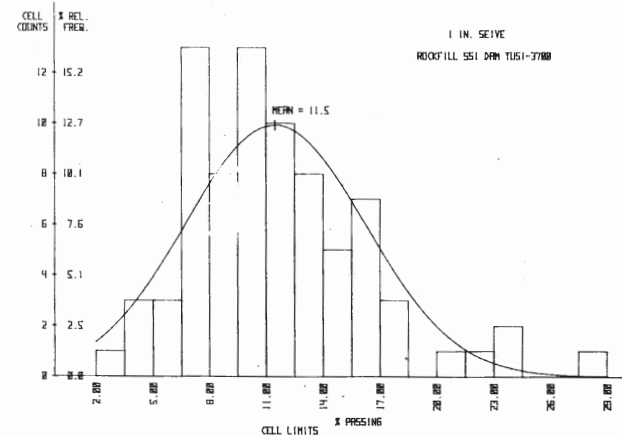
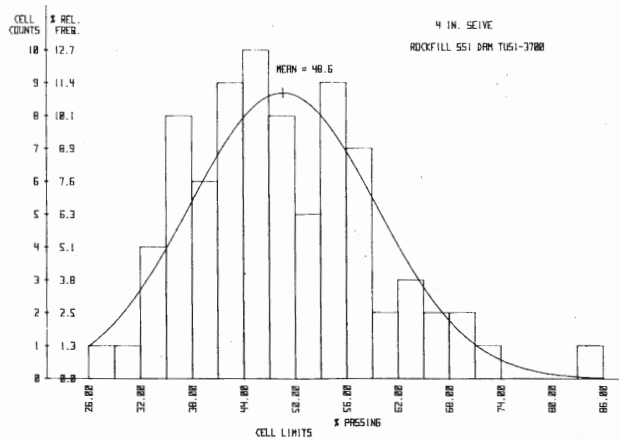
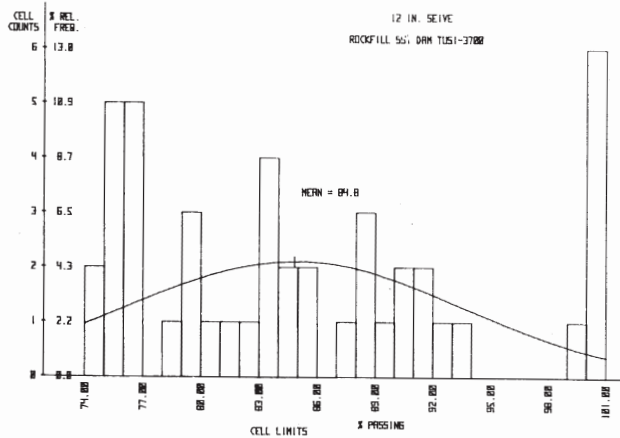


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COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
DYNAMIC STABILITY ANALYSIS
 τ_d / τ_f RATIO

FIGURE 2.5A-18



No. of Tests = 79

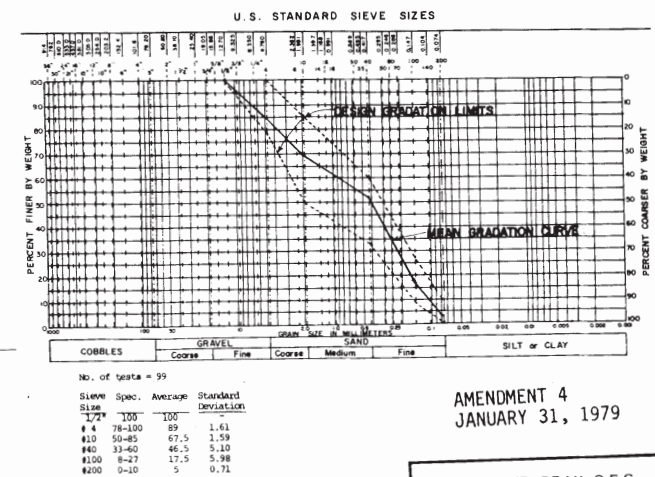
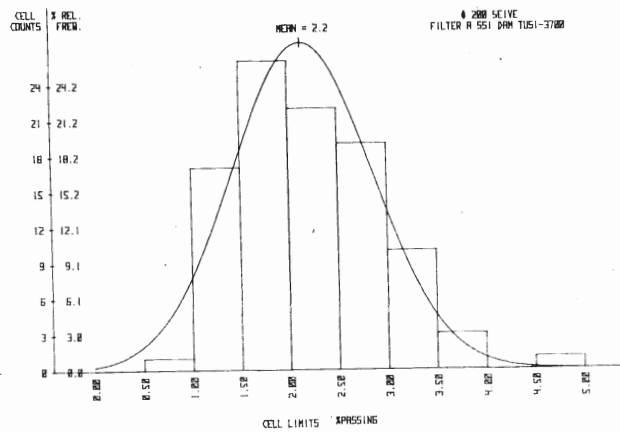
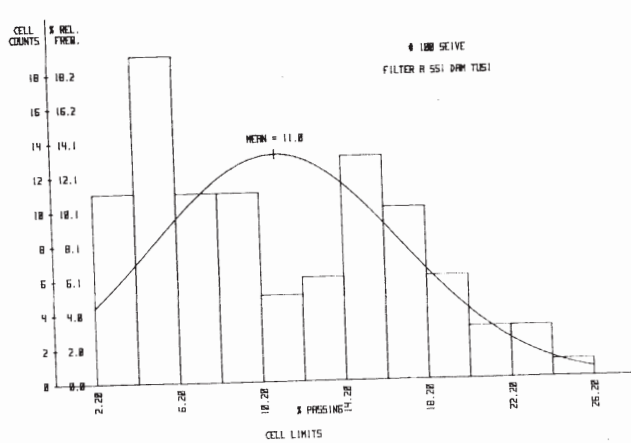
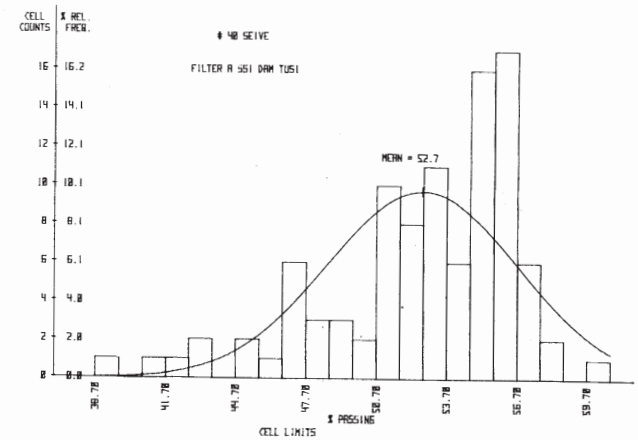
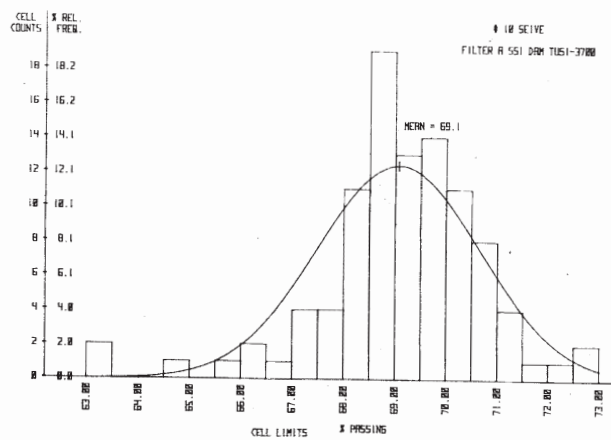
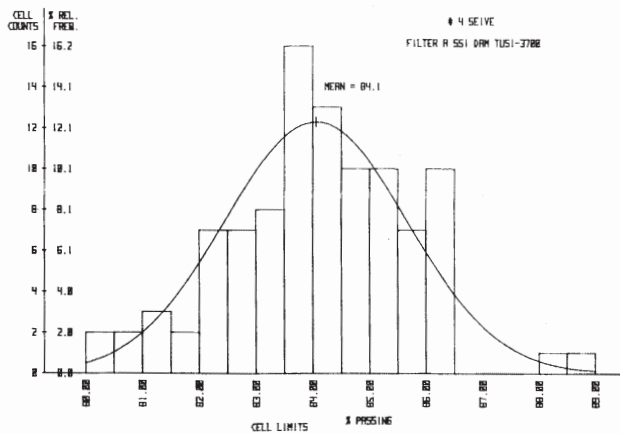
Sieve Size	Spec.	Average	Standard Deviation
30"	100	100	
12"	75-95	84.1	8.68
4"	25-70	49.4	10.86
1"	0-36	11.5	4.76
1/4"	0-8	4.7	2.55

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COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
HISTOGRAM OF GRADATION LIMITS
ROCKFILL

FIGURE 2.5A-19

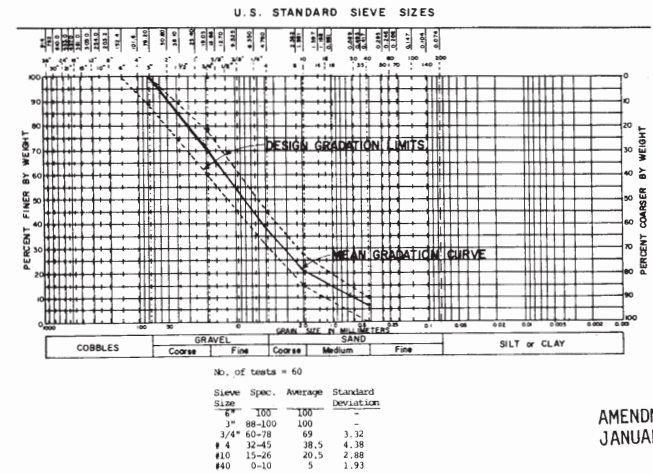
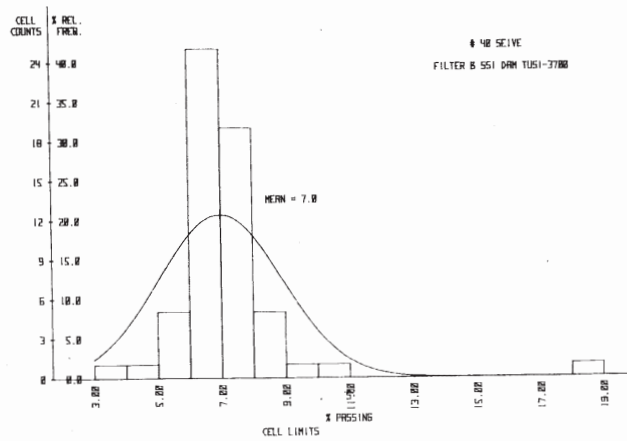
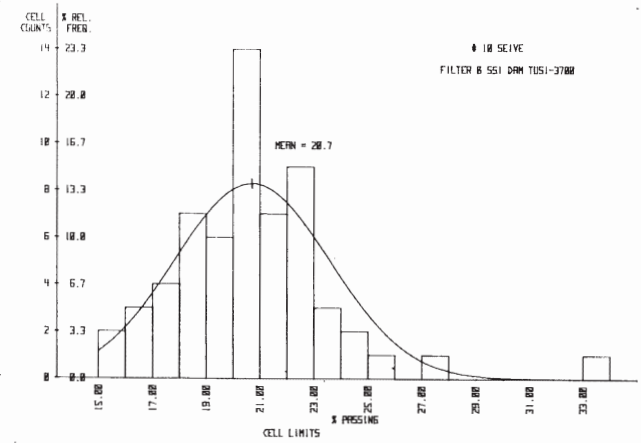
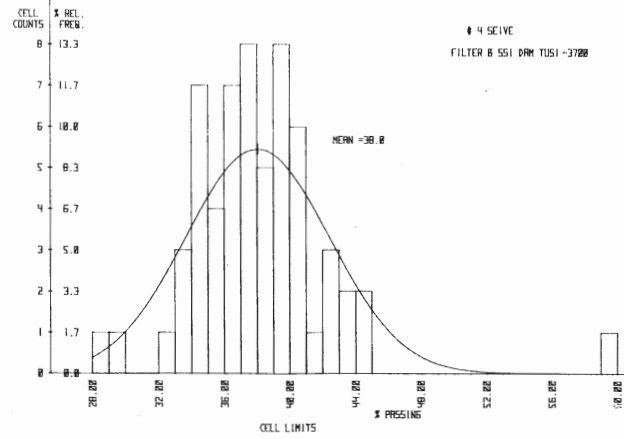
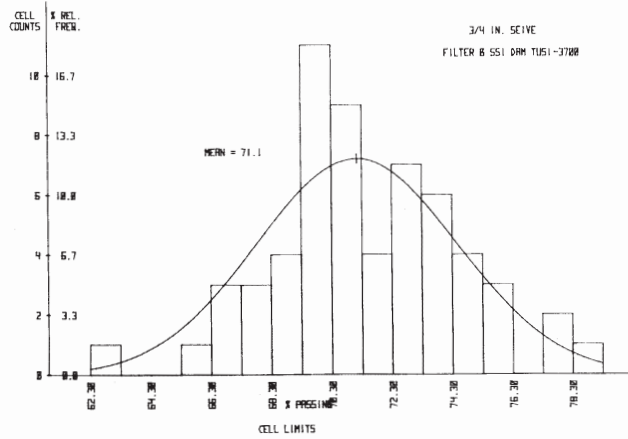


AMENDMENT 4
JANUARY 31, 1979

COMANCHE PEAK S.E.S
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SSI DAM
HISTOGRAM OF GRADATION LIMITS
FILTER A

FIGURE 2.5A-20



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JANUARY 31, 1979

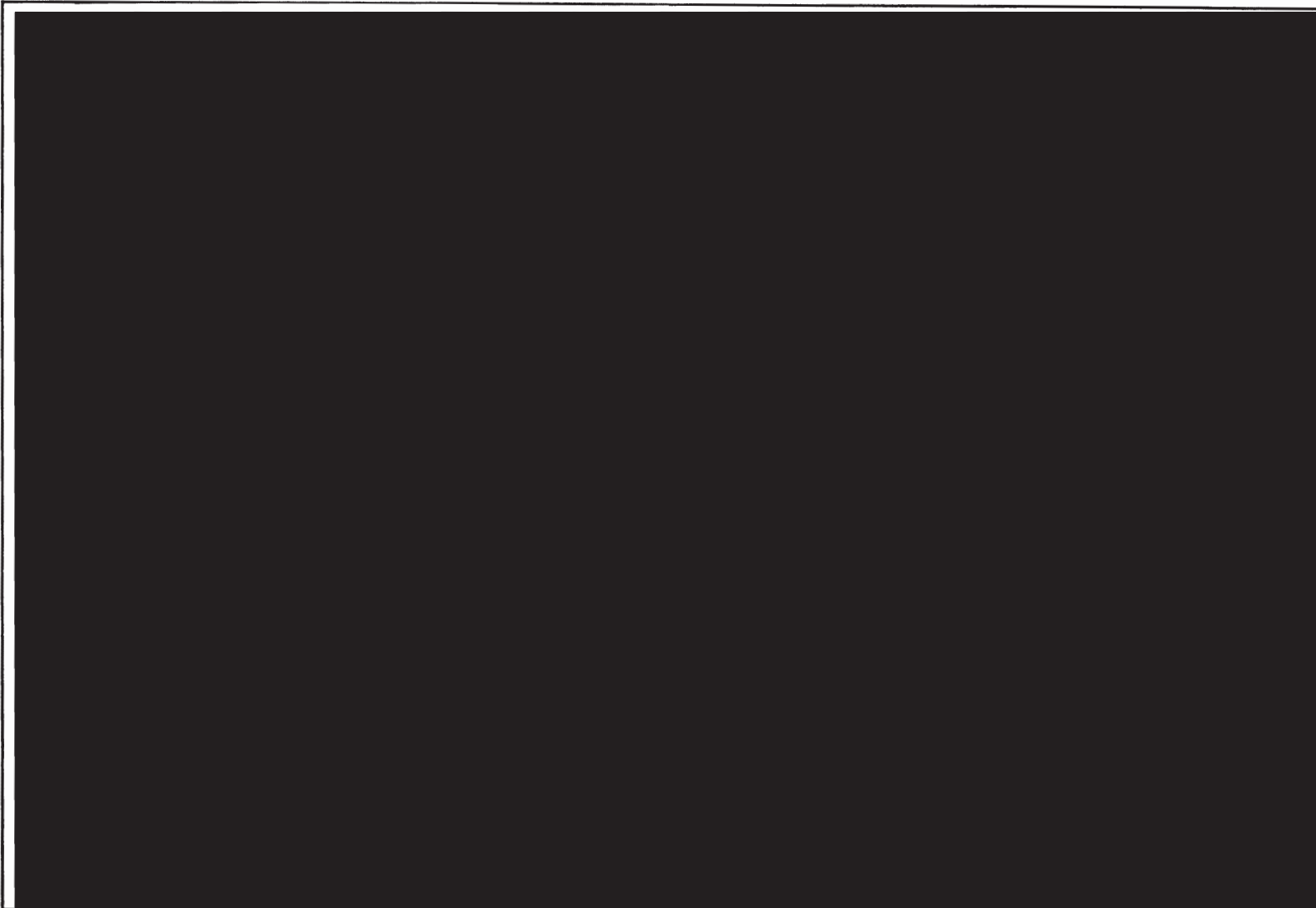
COMANCHE PEAK S.E.S.
FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2
SSI DAM
HISTOGRAM OF GRADATION LIMITS
FILTER B

FIGURE 2.5A-21

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SQUAW CREEK DAM
LOG OF BORING
SCD-1

FIGURE 2.5B-1



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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2


SQUAW CREEK DAM
LOG OF BORING
SCD-2

FIGURE 2.5B-2

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FINAL SAFETY ANALYSIS REPORT
UNITS 1 and 2

SQUAW CREEK DAM
LOG OF BORING
SCD-3

FIGURE 2.5B-3



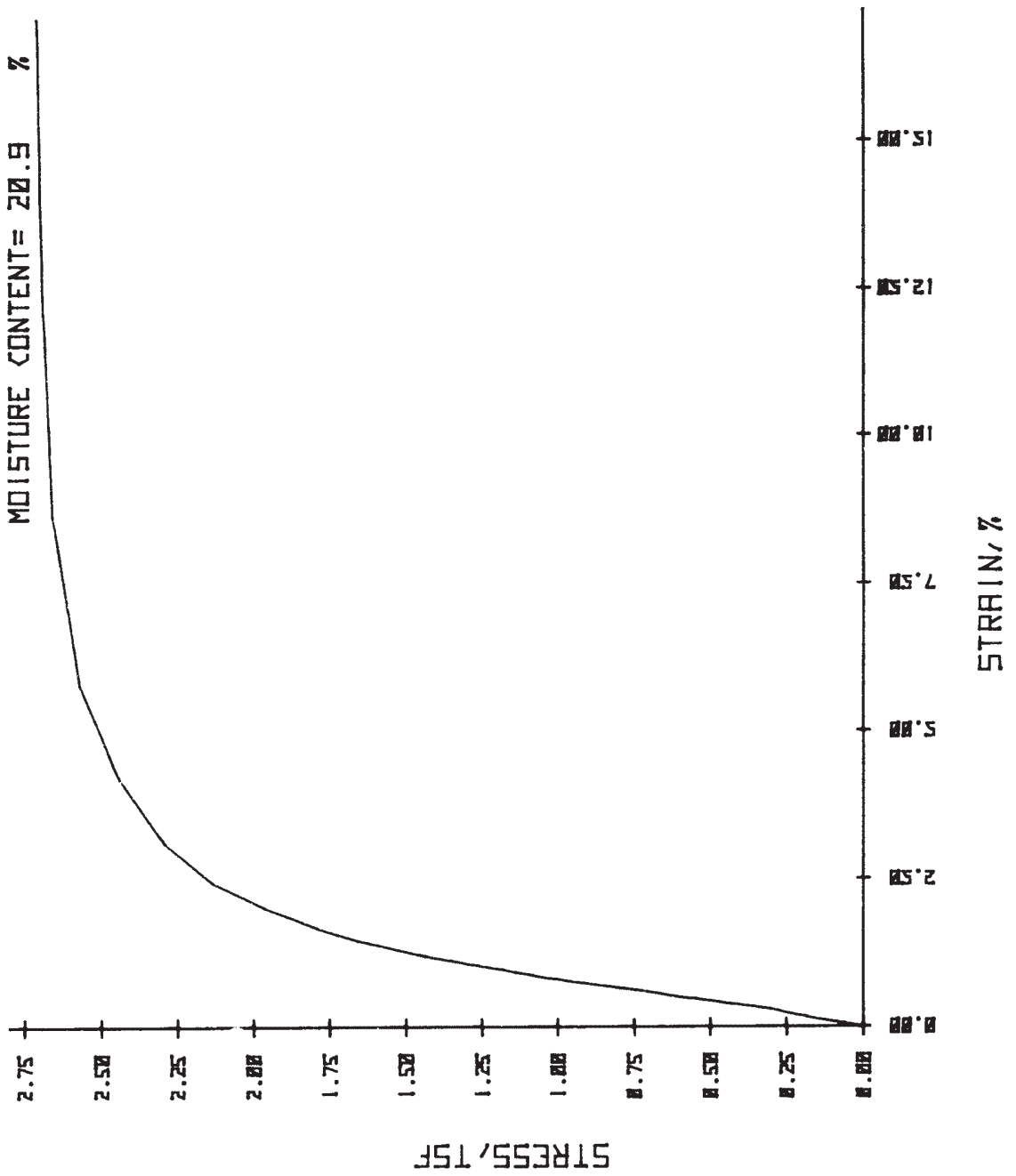
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SQUAW CREEK DAM
LOG OF BORING
SCD-4

FIGURE 2.5B-4

BORING NO.: SCD-4 SAMPLE: 54.0-55.5'
 UNIT DRY WEIGHT= 107.0 LBS/CU.FT.
 MOISTURE CONTENT= 20.9 %



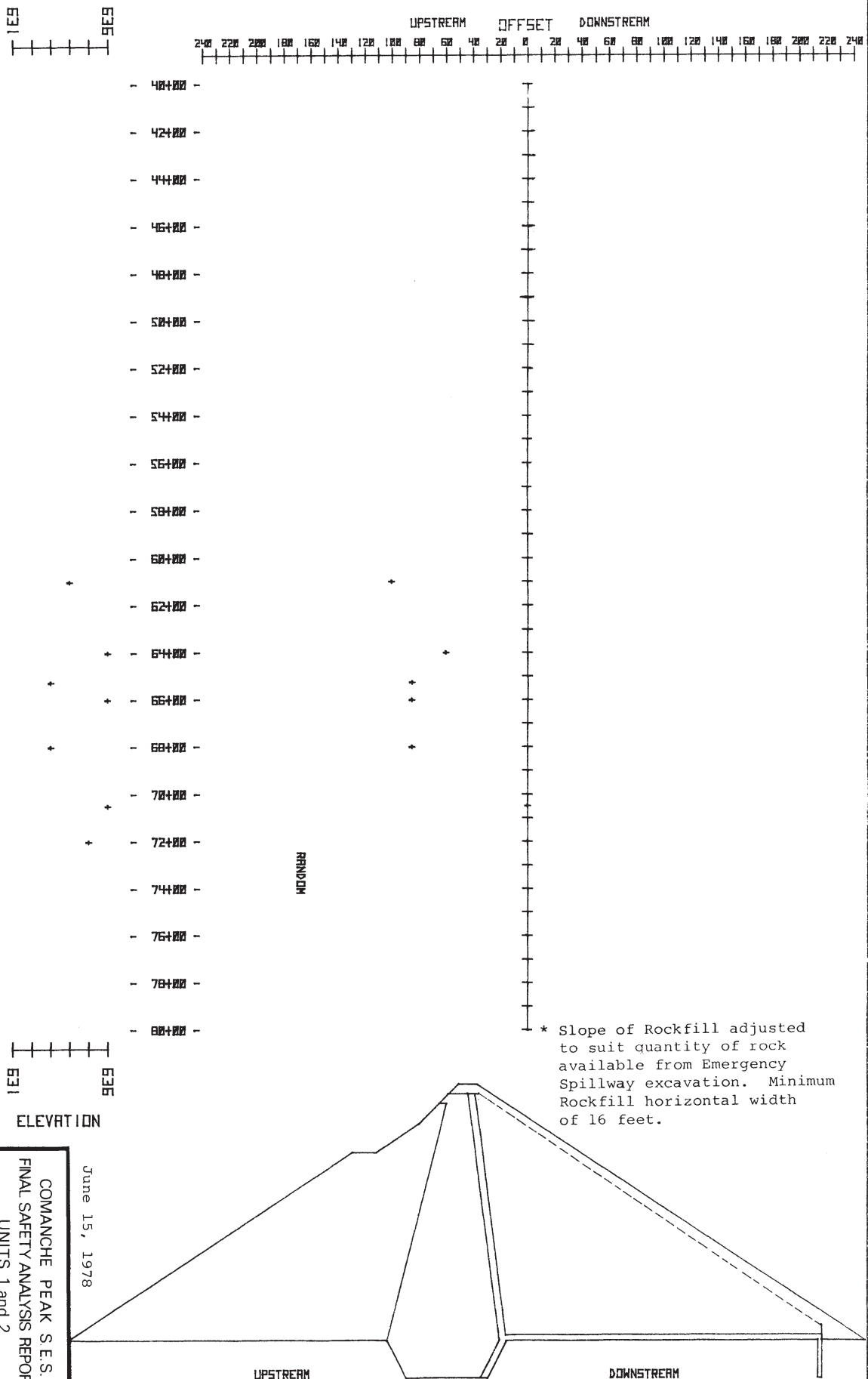
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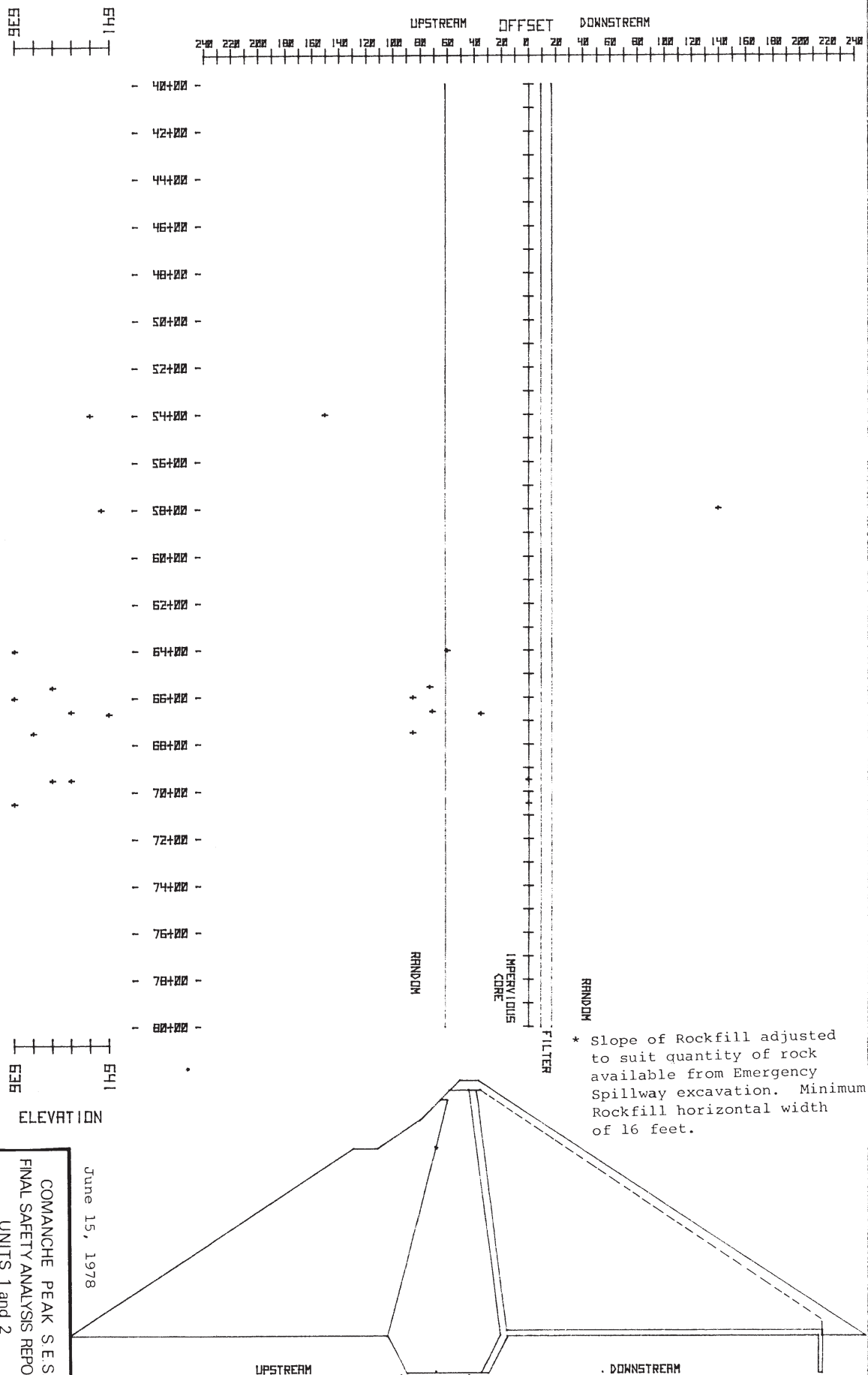
SQUAW CREEK DAM
 STRESS-STRAIN PLOT

FIGURE 2.5B-5

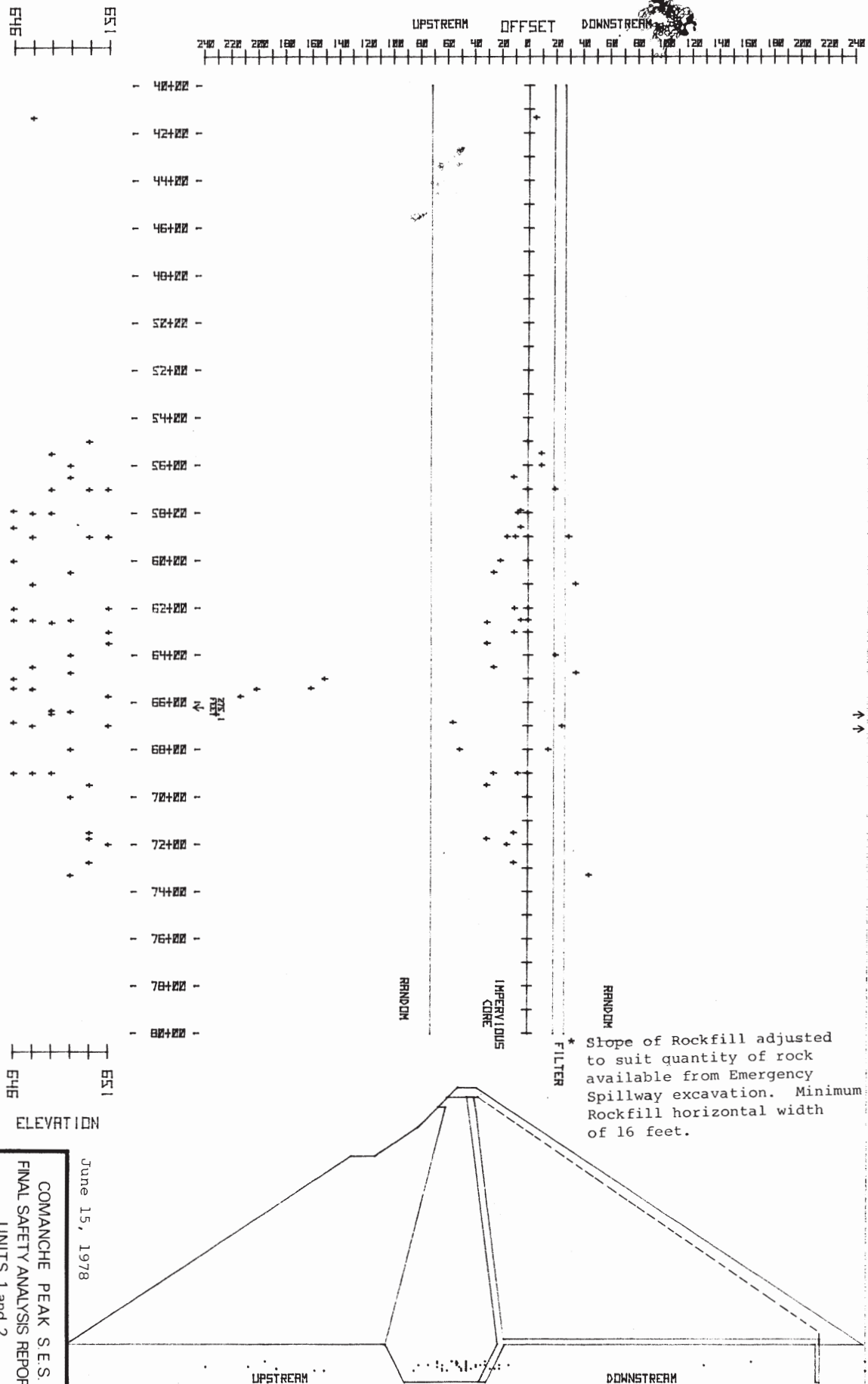
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 631 TO 636



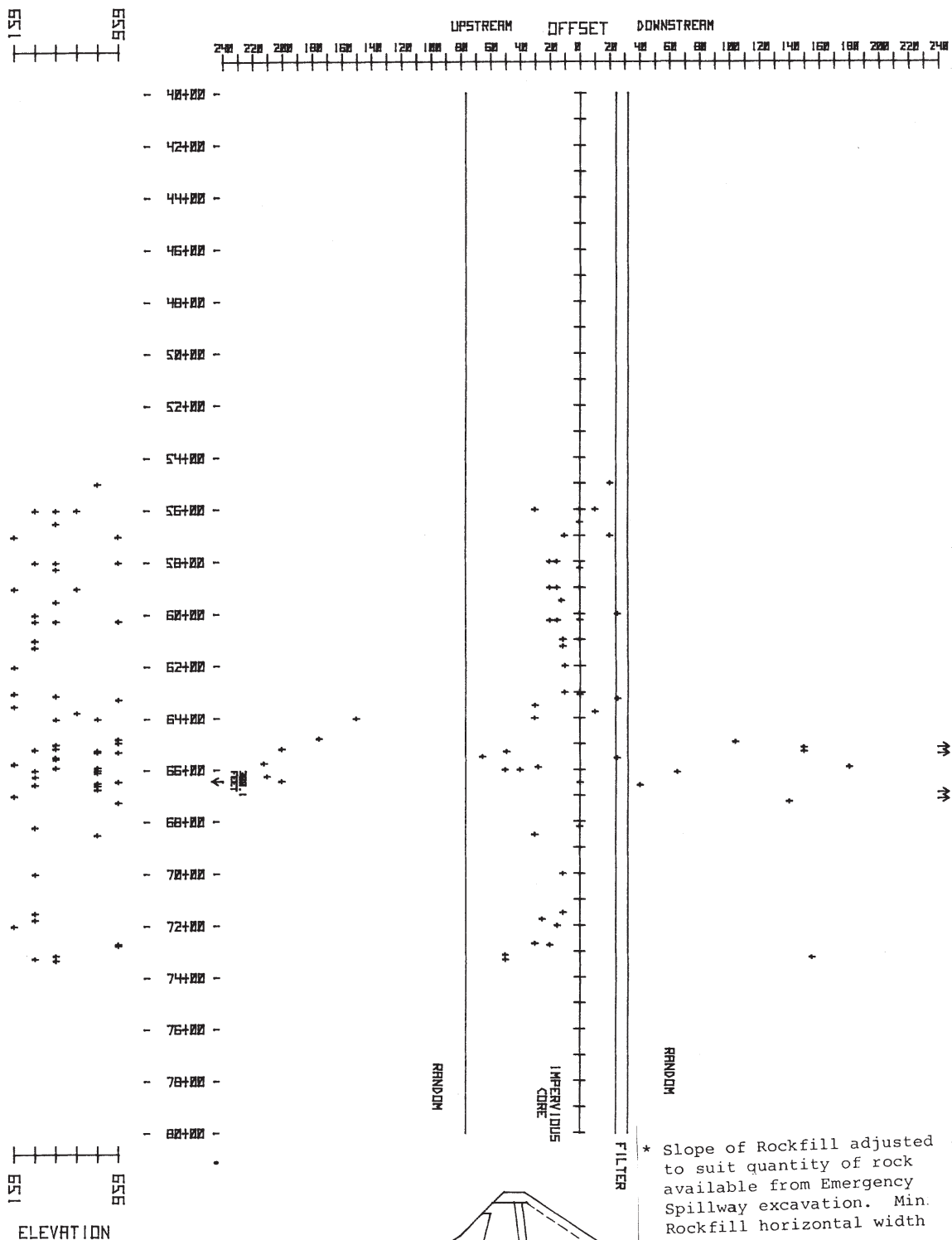
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 536 TO 541



SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 646 TO 651

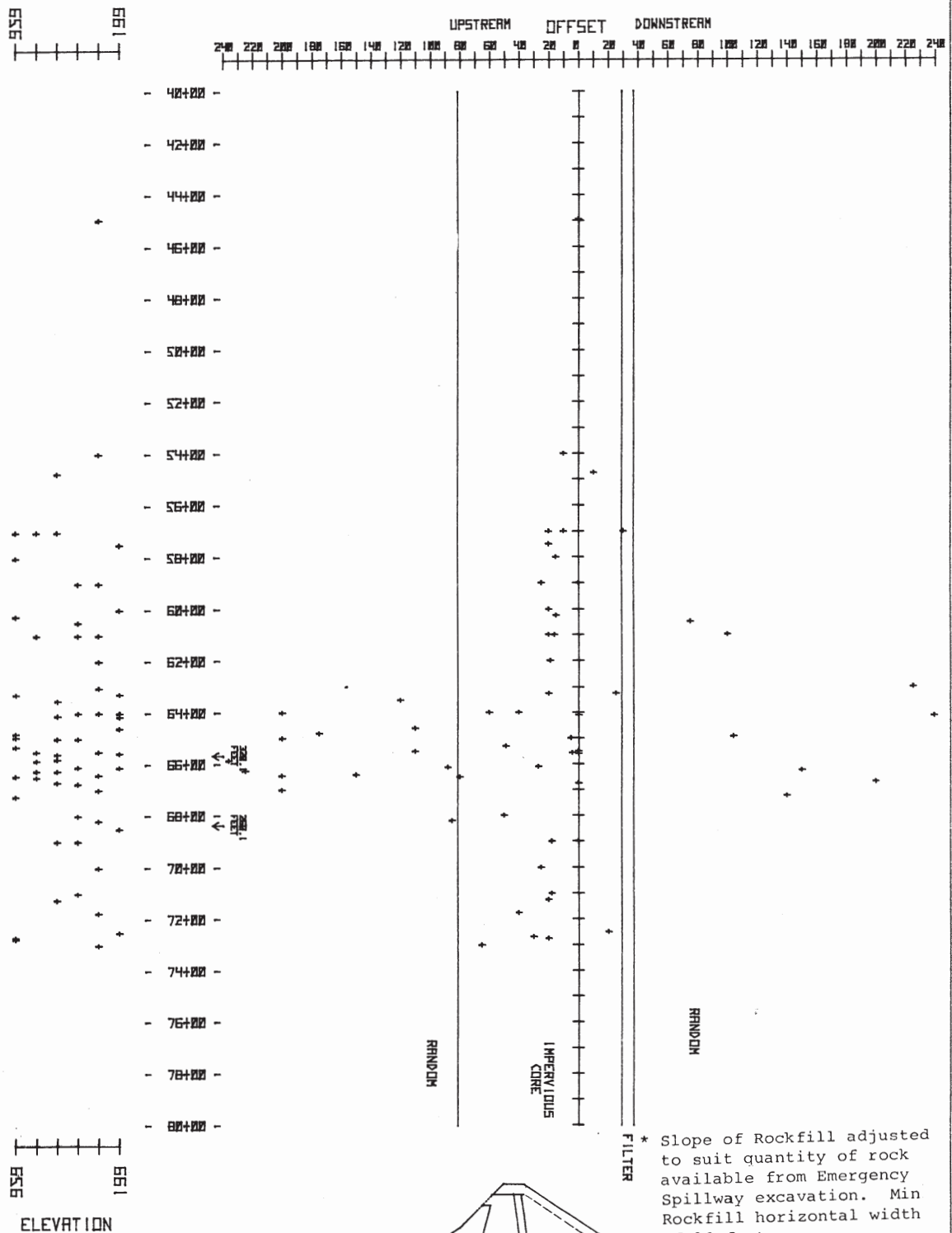


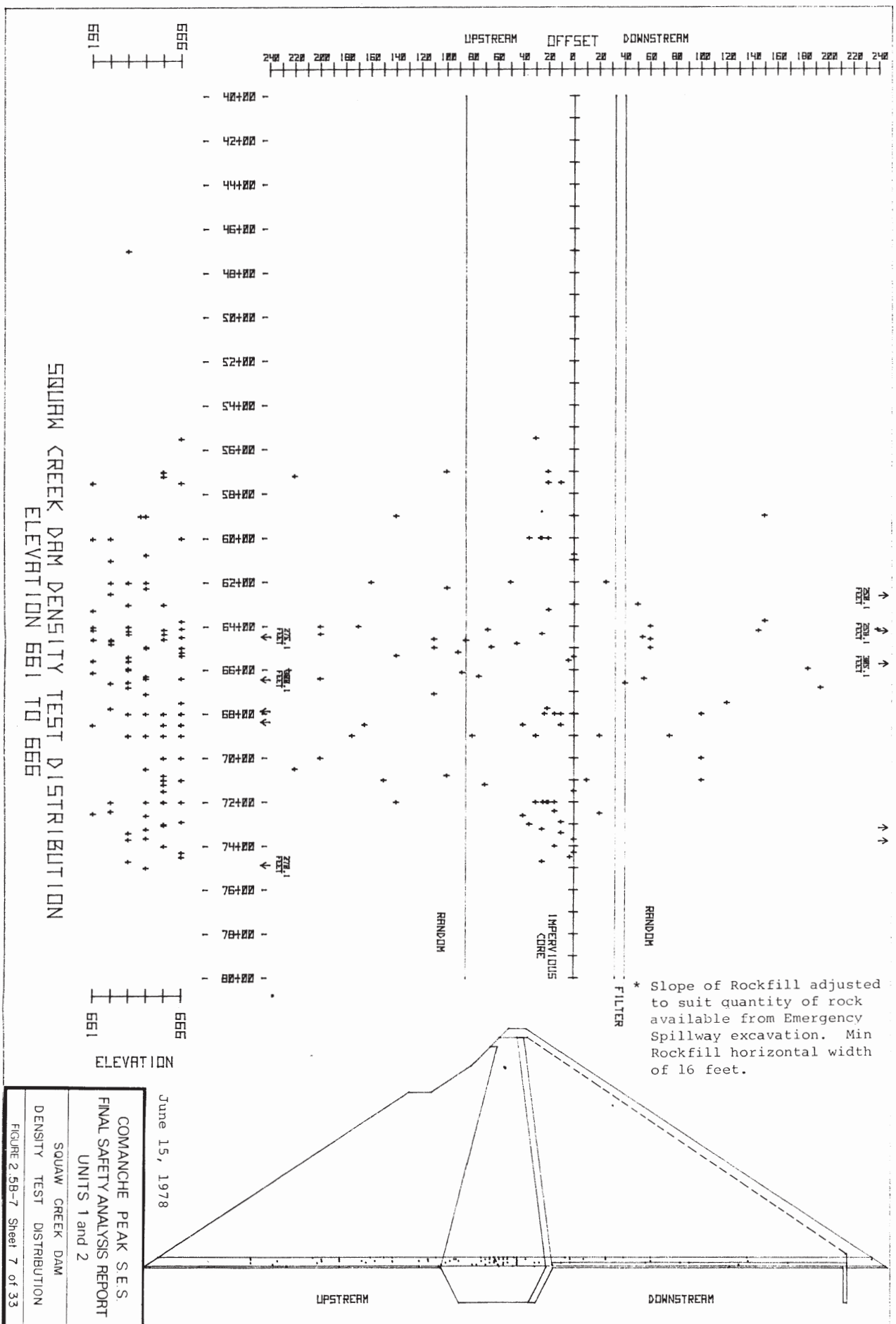
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 651 TO 656

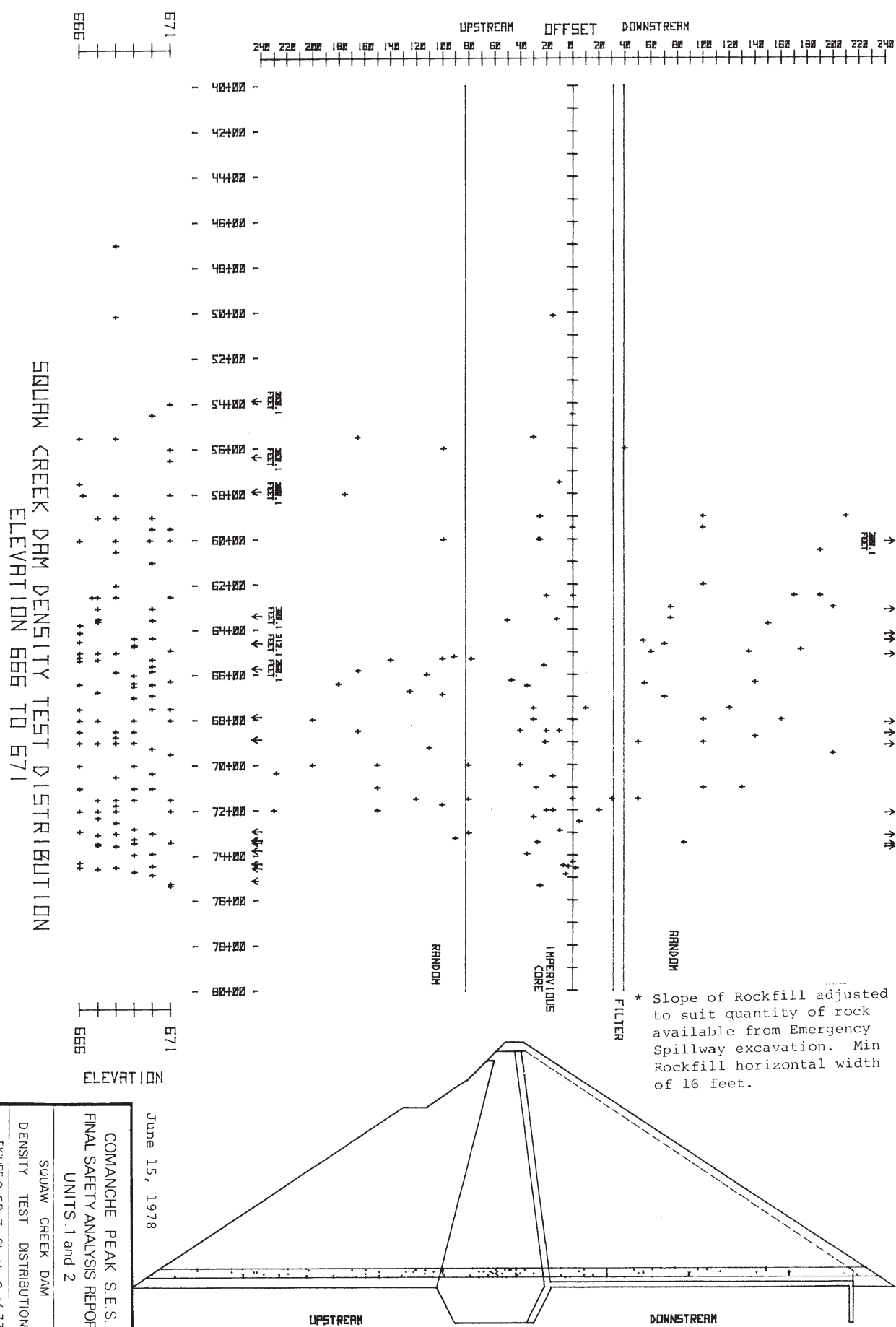


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SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 656 TO 661







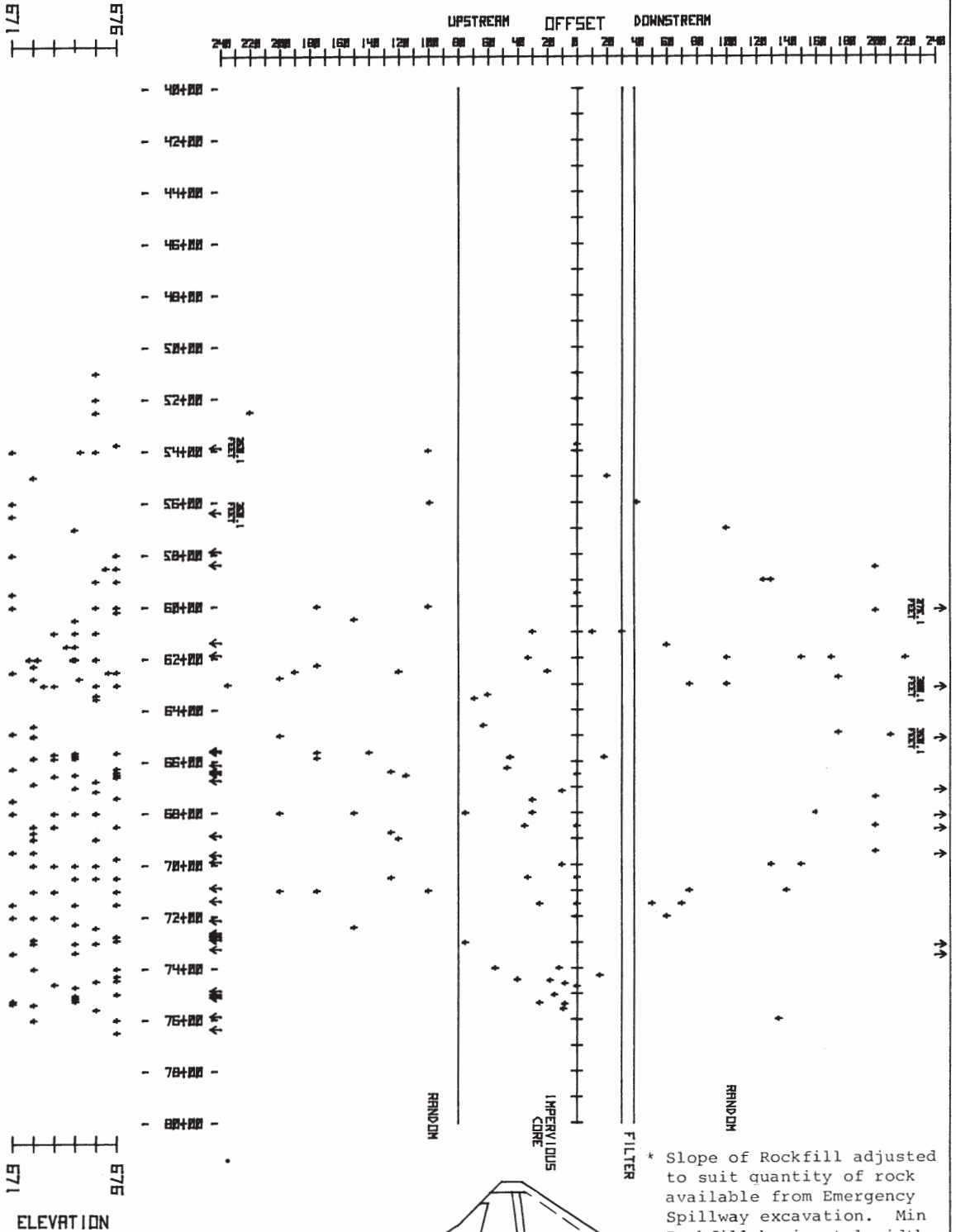
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SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION

FIGURE 2.58-7 Sheet 8 of 33

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SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 671 TO 676



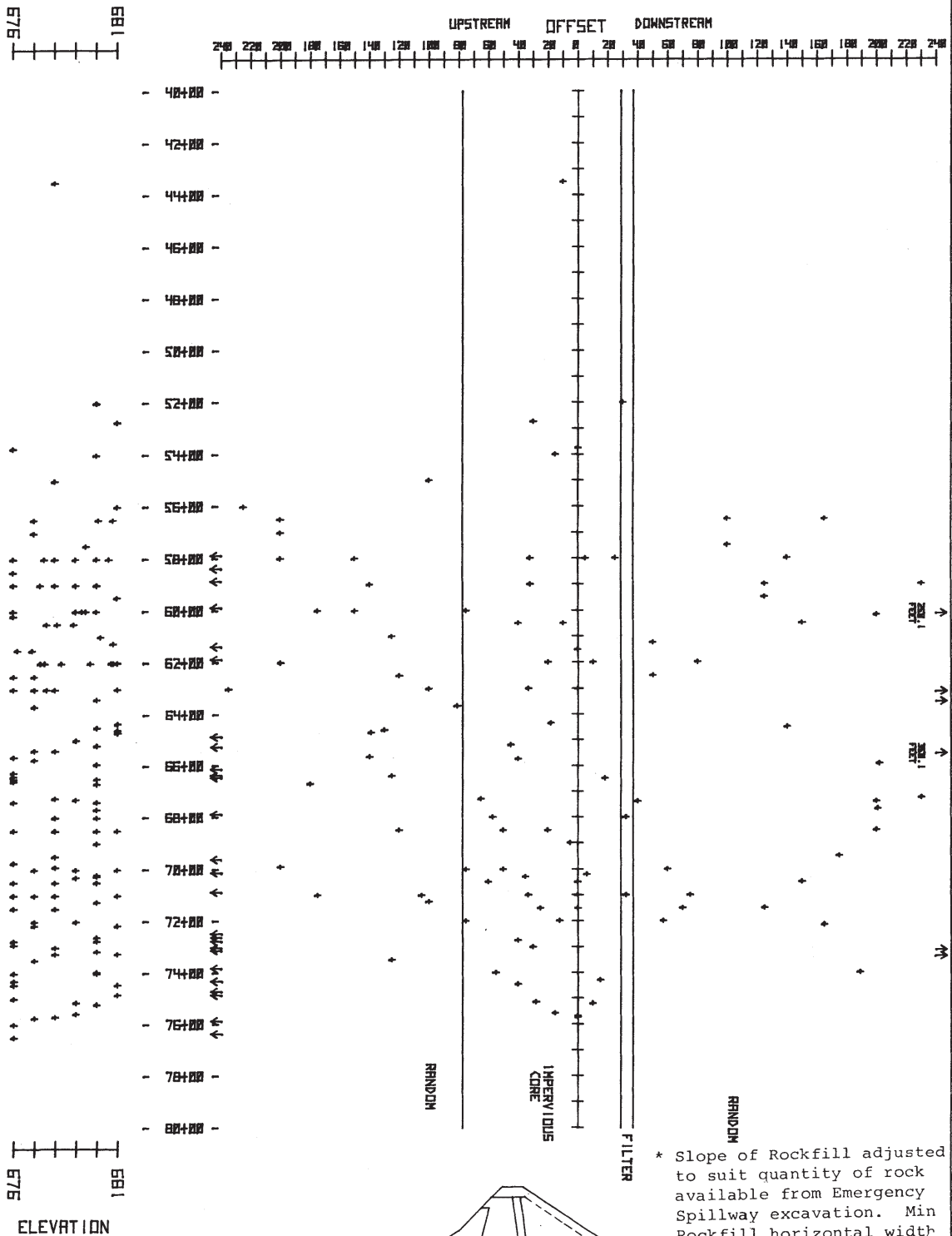
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DENSITY TEST DISTRIBUTION

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SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 676 TO 681

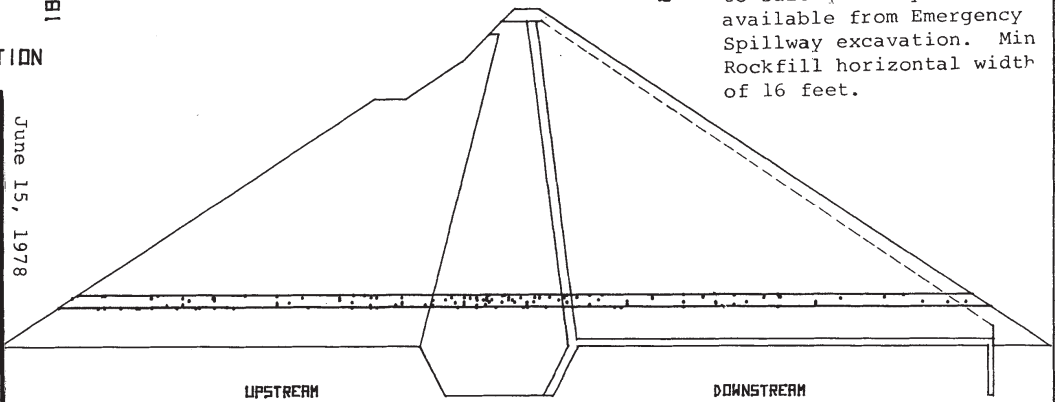


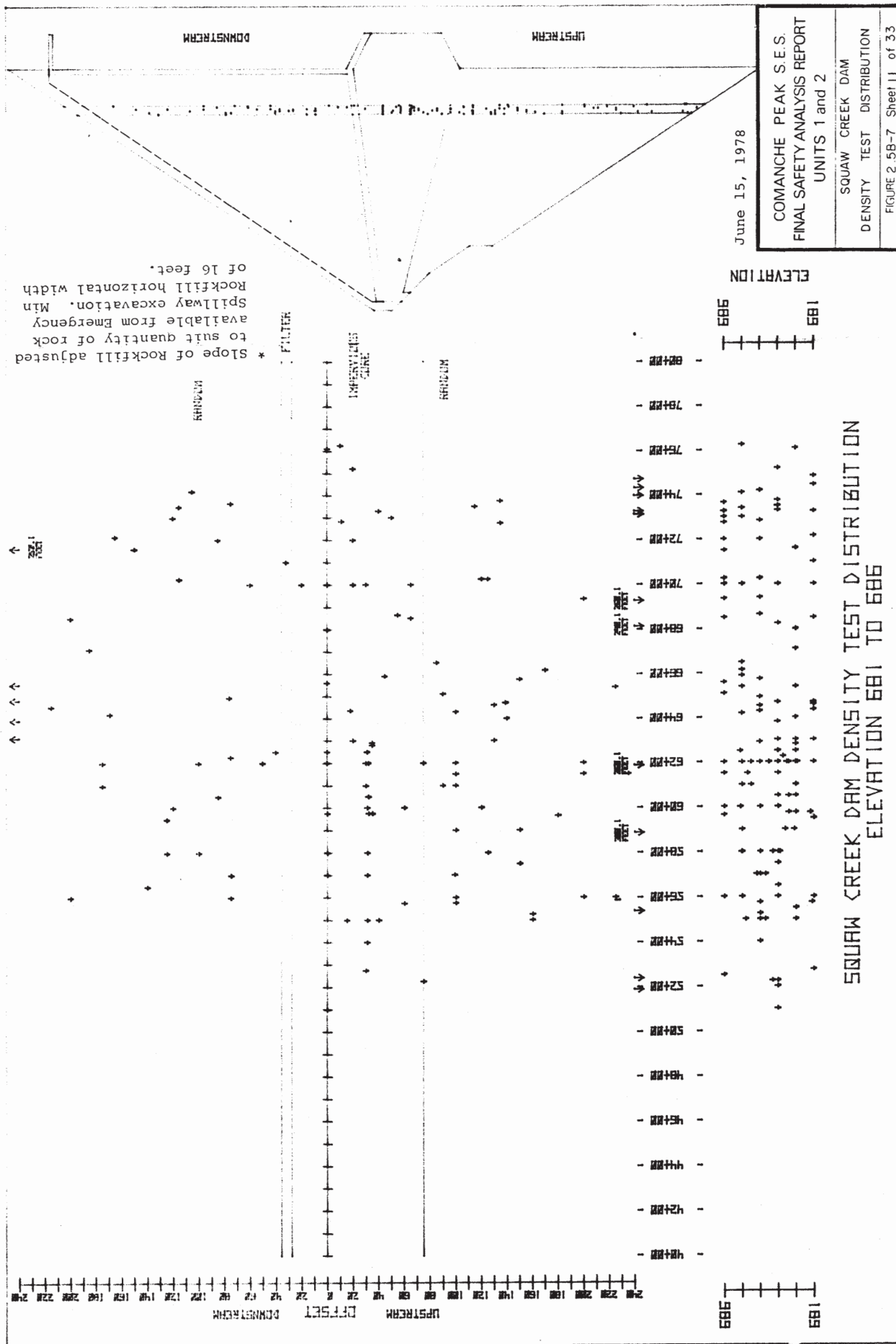
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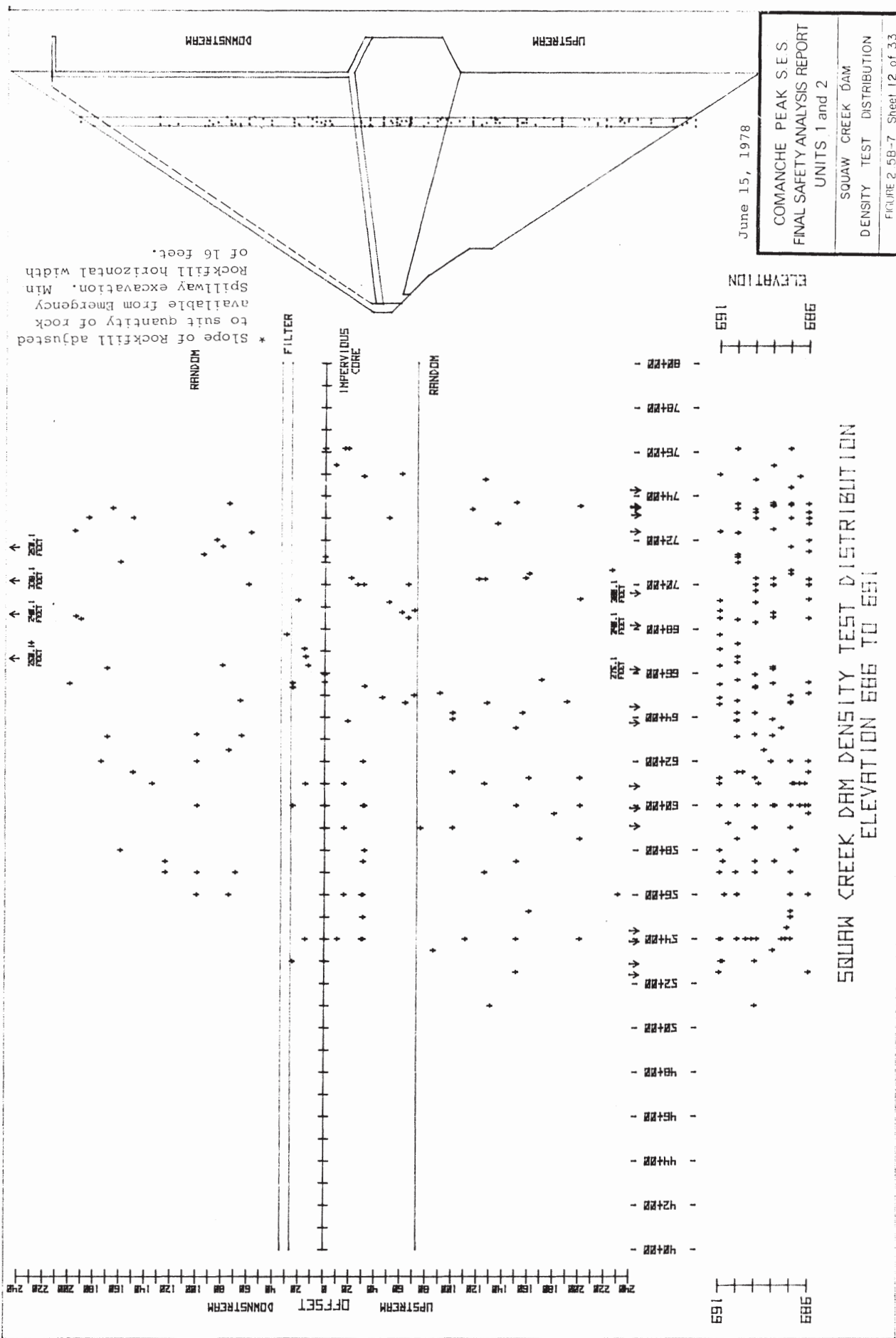
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SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION

FIGURE 2 SB-7 Sheet 10 of 33

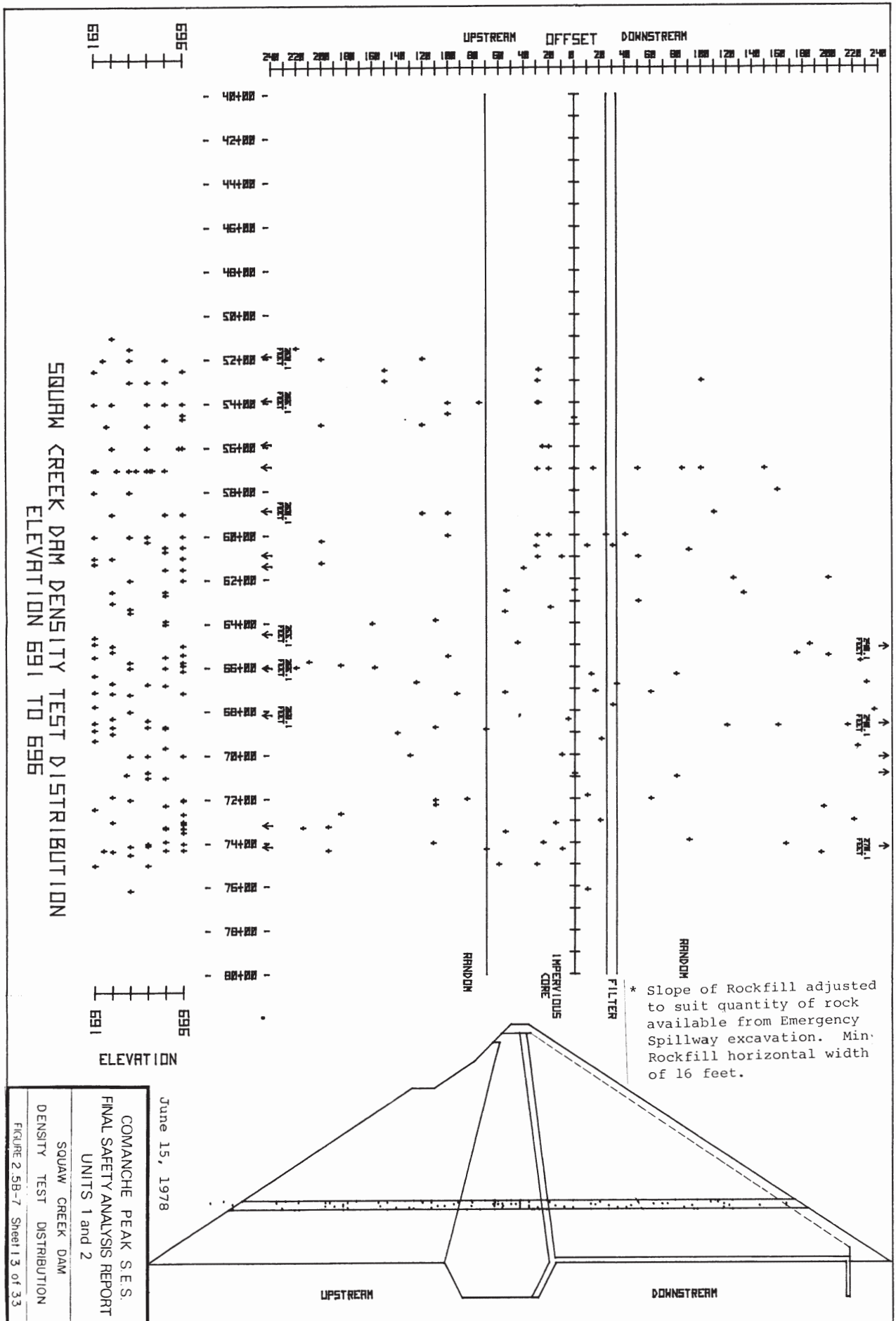






* Slope of Rockfill adjusted to suit quantity of rock available from Emergency Spillway excavation. Min Rockfill horizontal width of 16 feet.

SQUAW CREEK DAM DENSITY TEST DISTRIBUTION
ELEVATION 686 TO 691



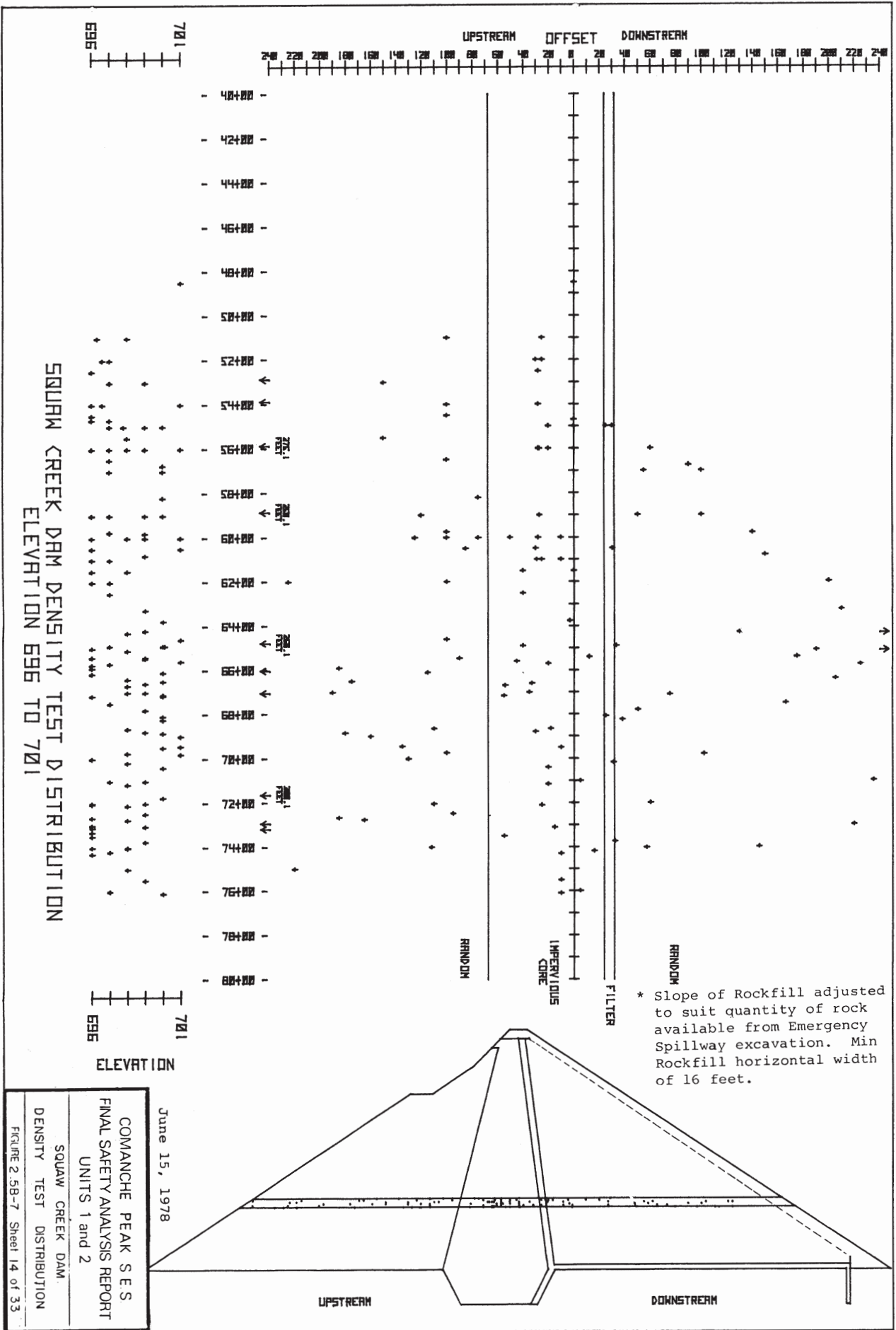
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION
ELEVATION 691 TO 696

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DENSITY TEST DISTRIBUTION

FIGURE 2.5B-7 Sheet 13 of 33



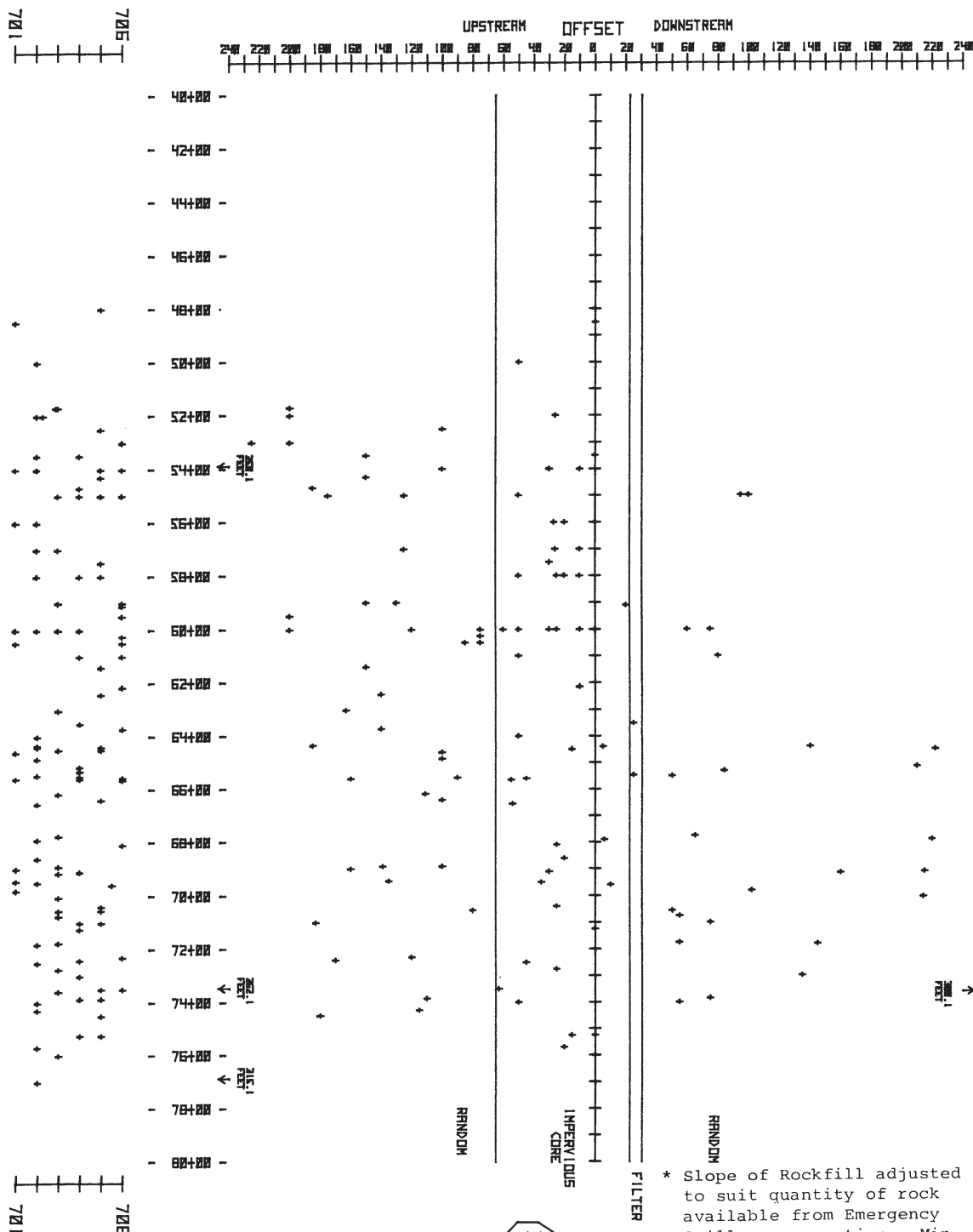
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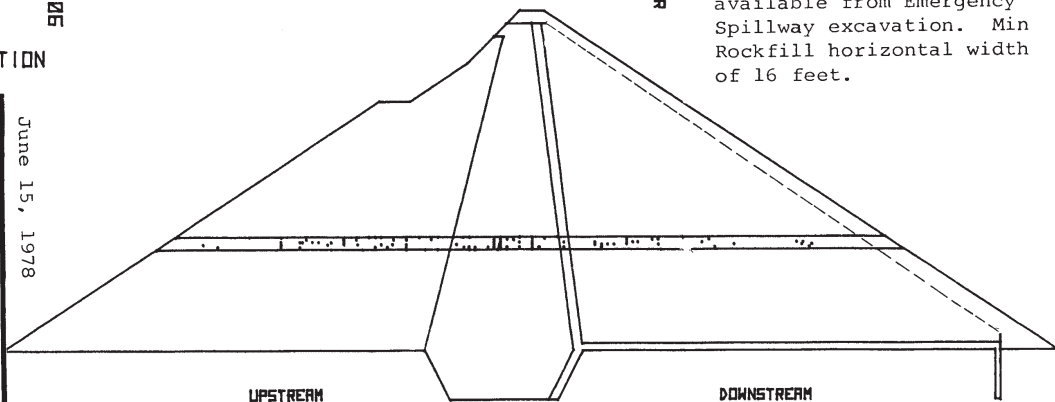
SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION

FIGURE 2.5B-7 Sheet 14 of 33

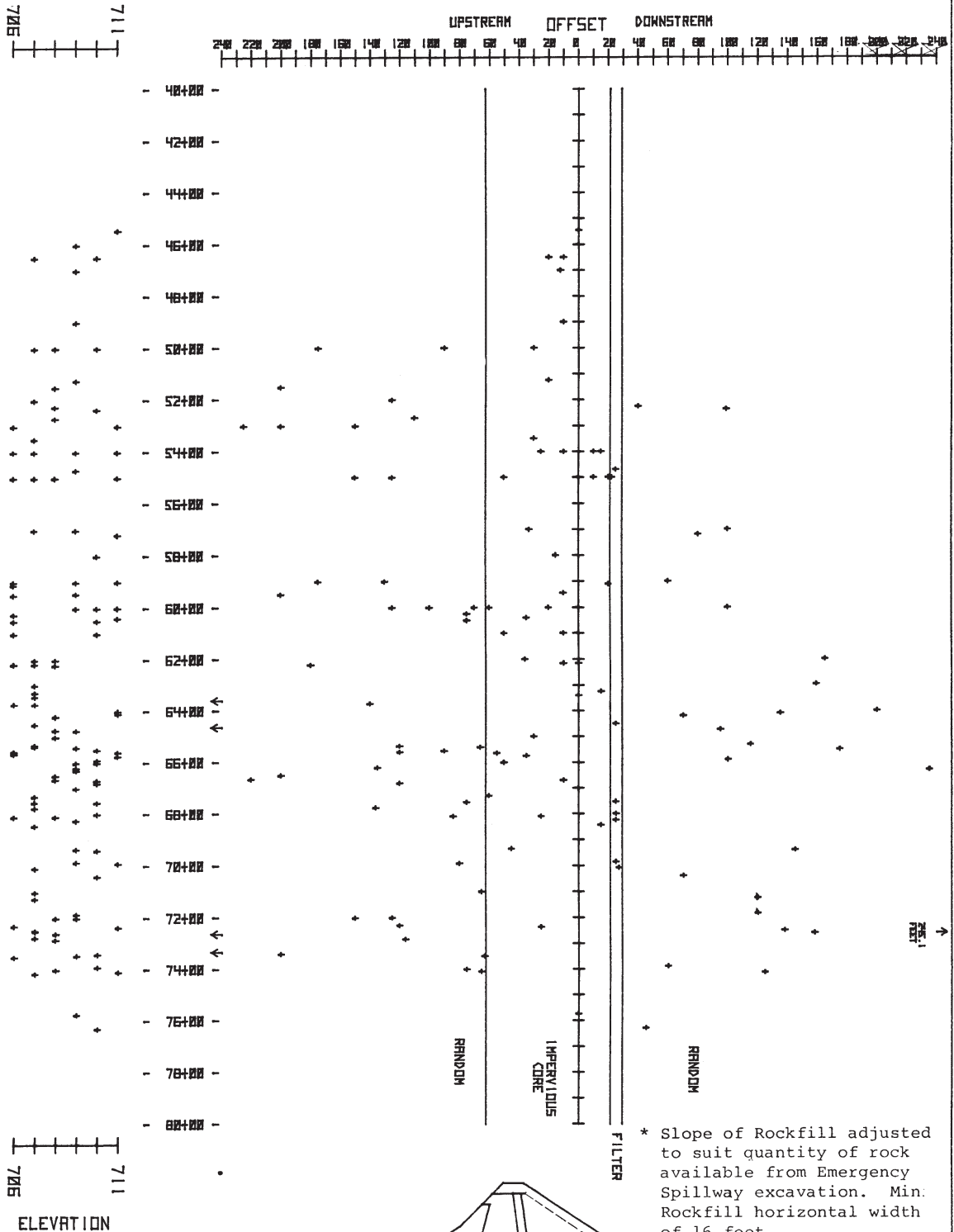
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 701 TO 706



* Slope of Rockfill adjusted to suit quantity of rock available from Emergency Spillway excavation. Min Rockfill horizontal width of 16 feet.



SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 706 TO 711

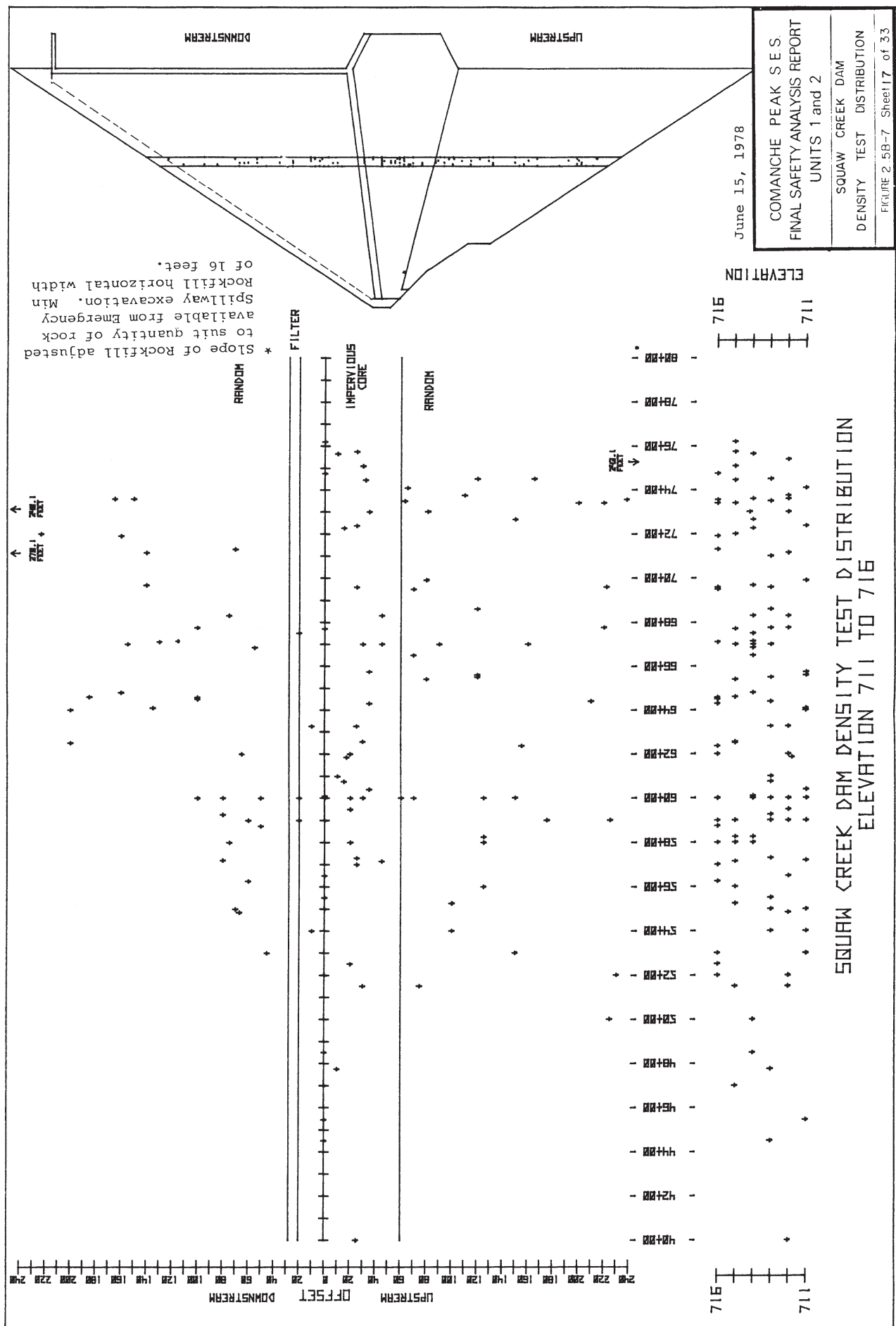


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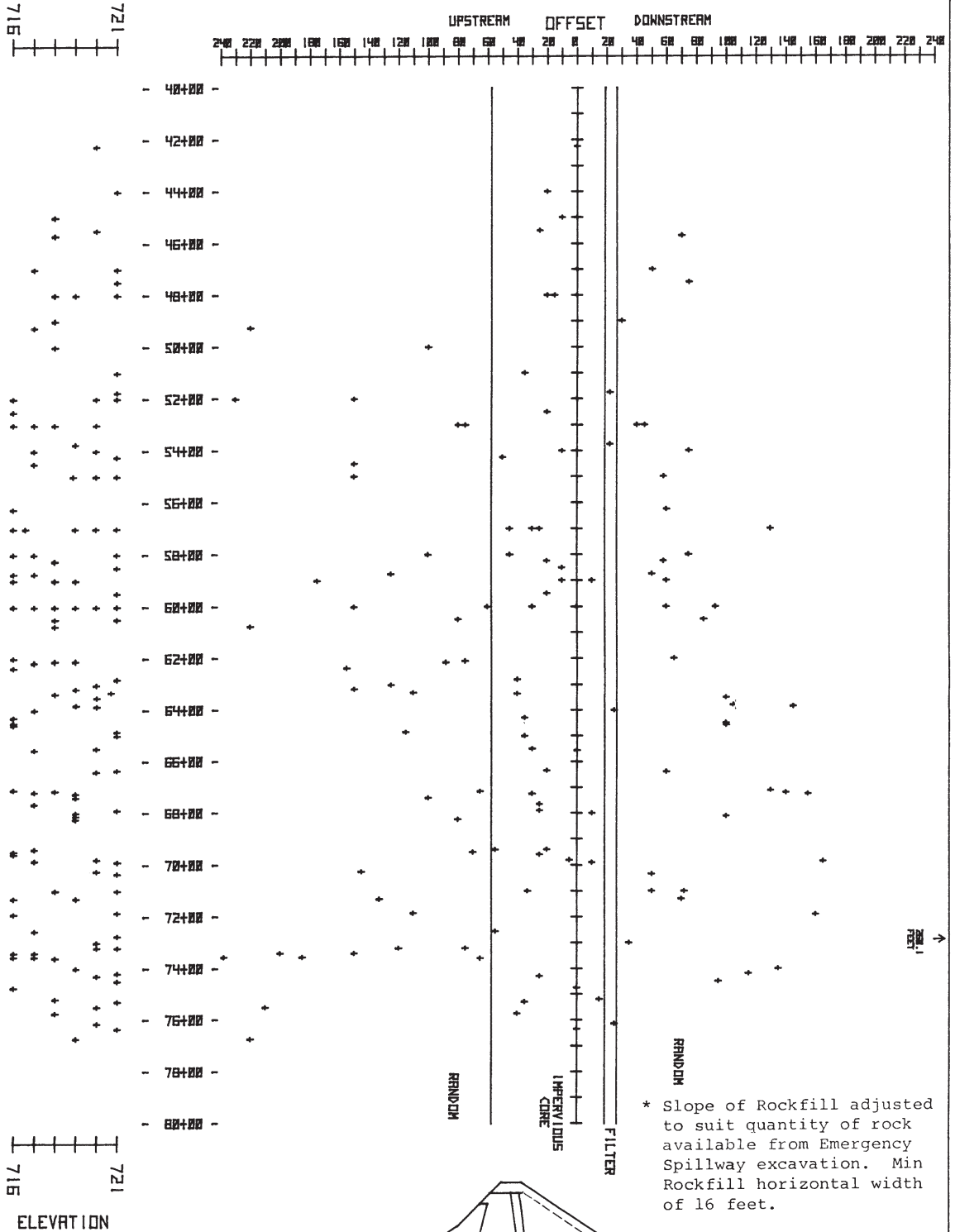
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DENSITY TEST DISTRIBUTION

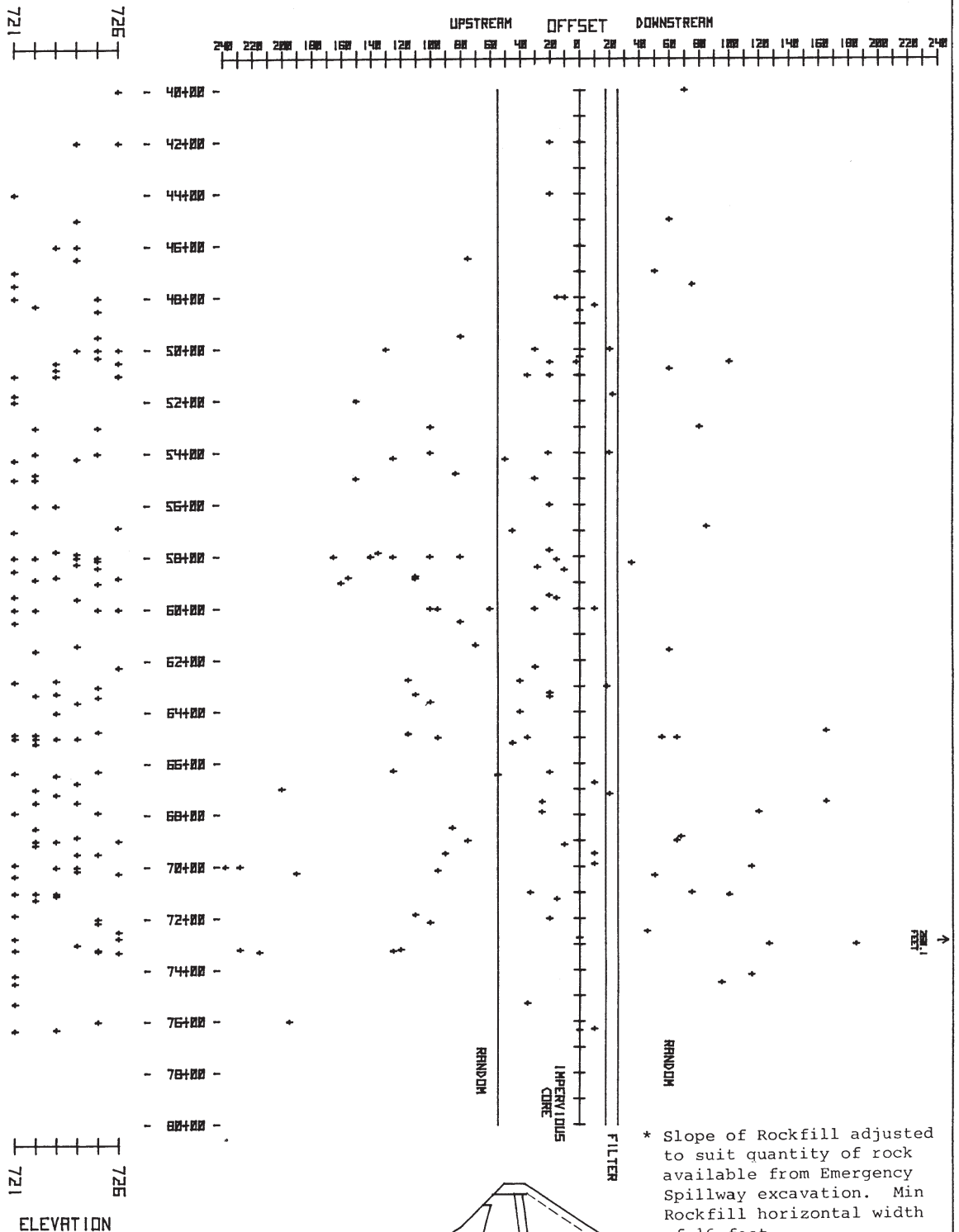
FIGURE 2.5B-7 Sheet 16 of 33



SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 716 TO 721

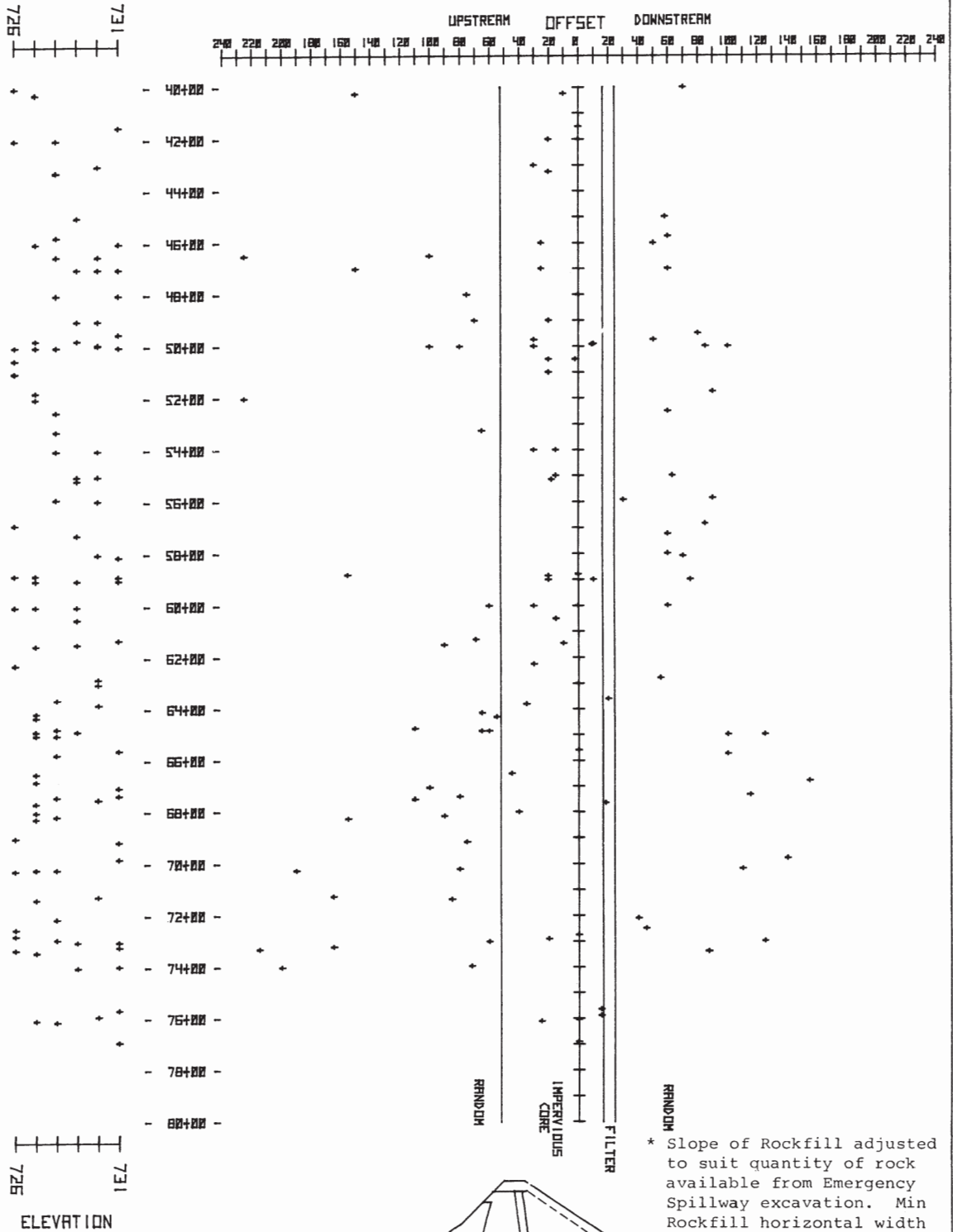


SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 721 TO 726



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SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 726 TO 731

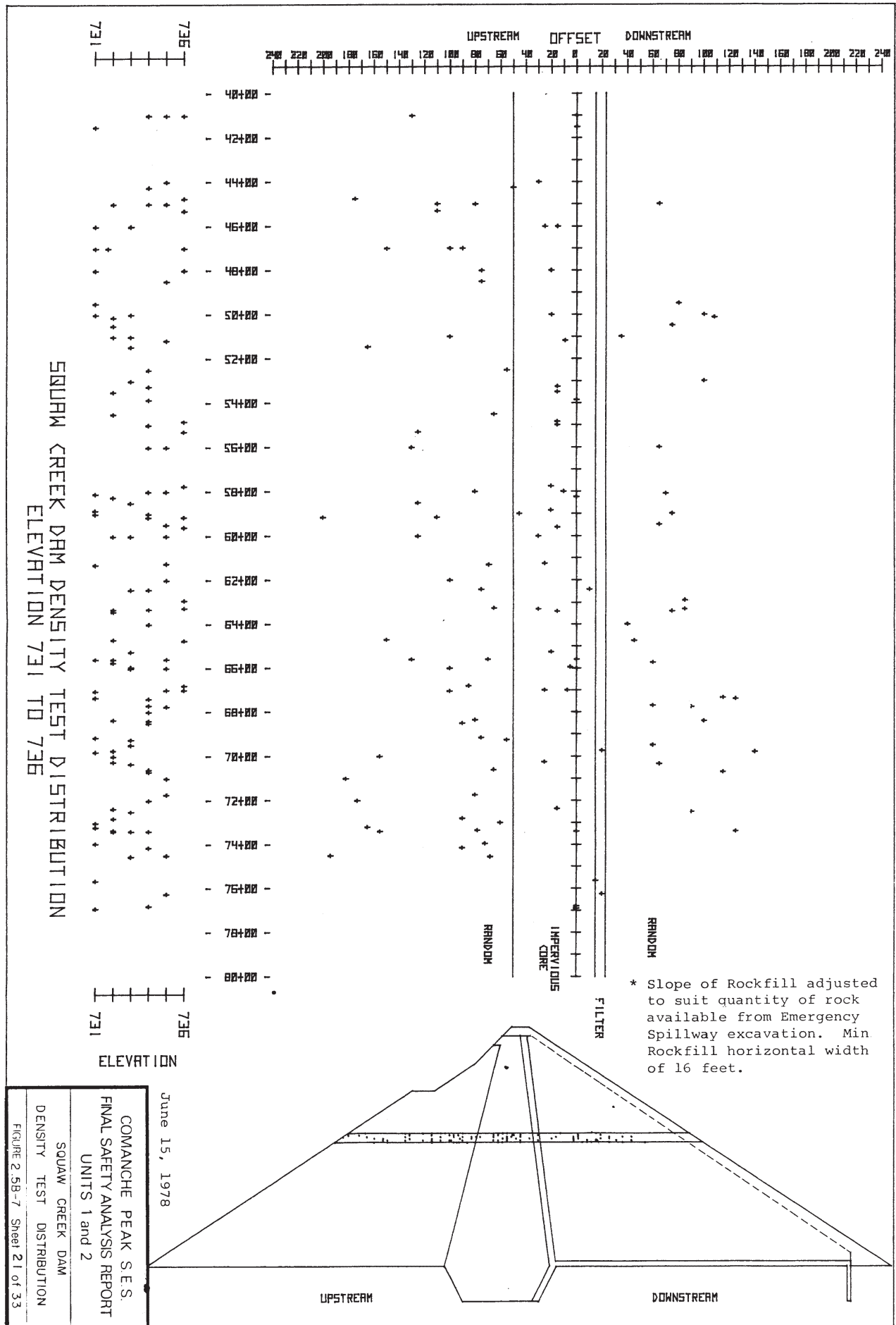


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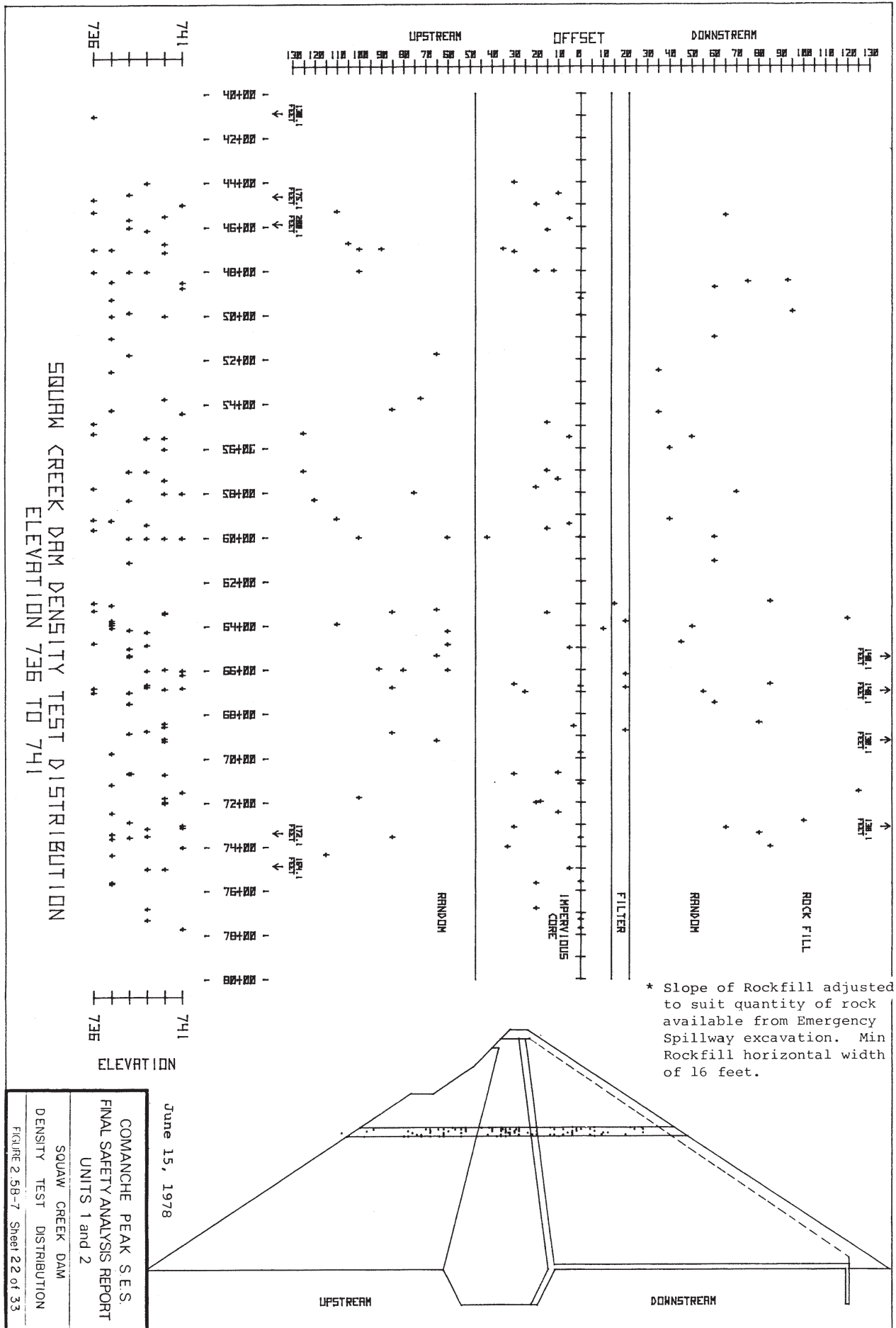
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION
ELEVATION 731 TO 736

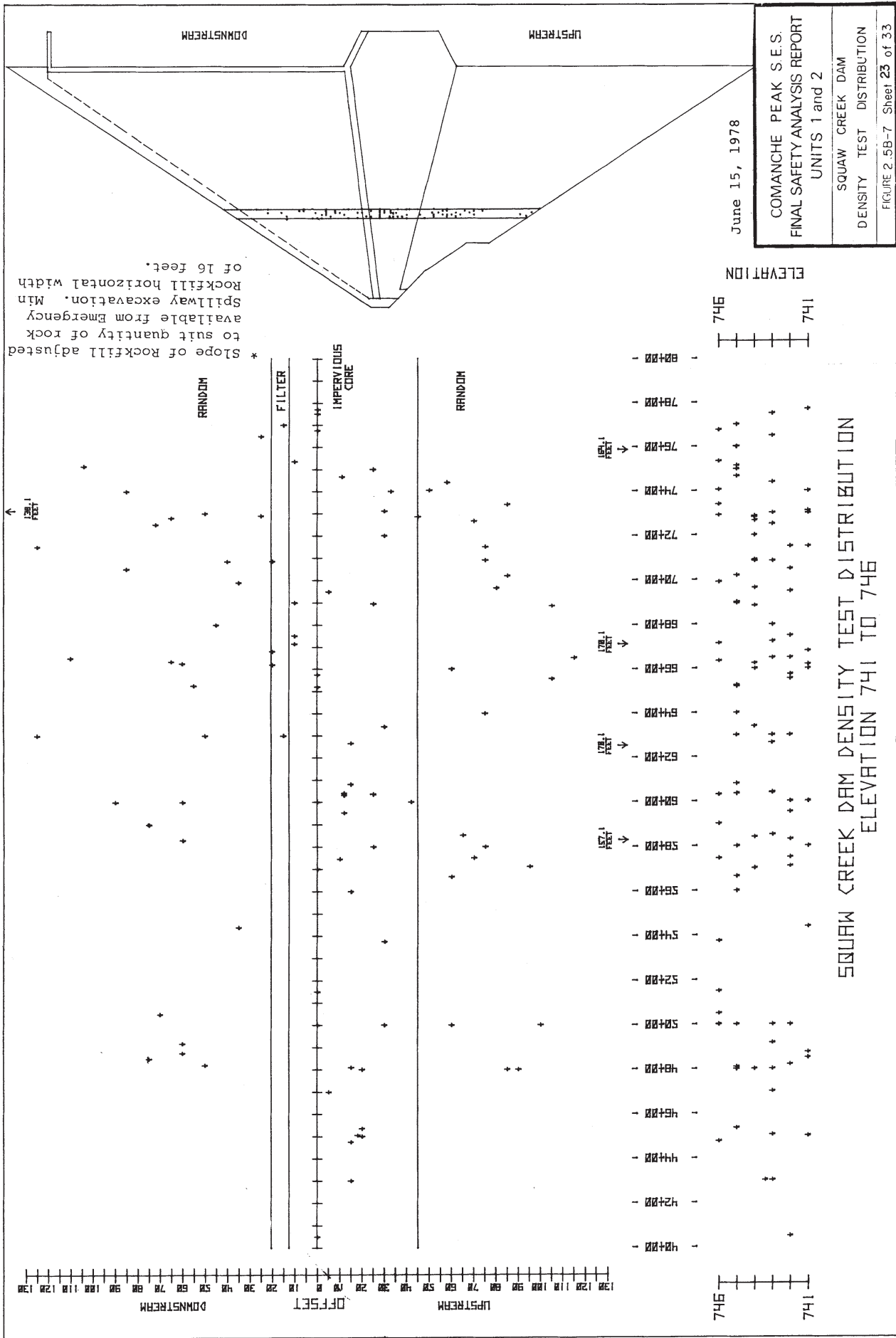
June 15, 1978

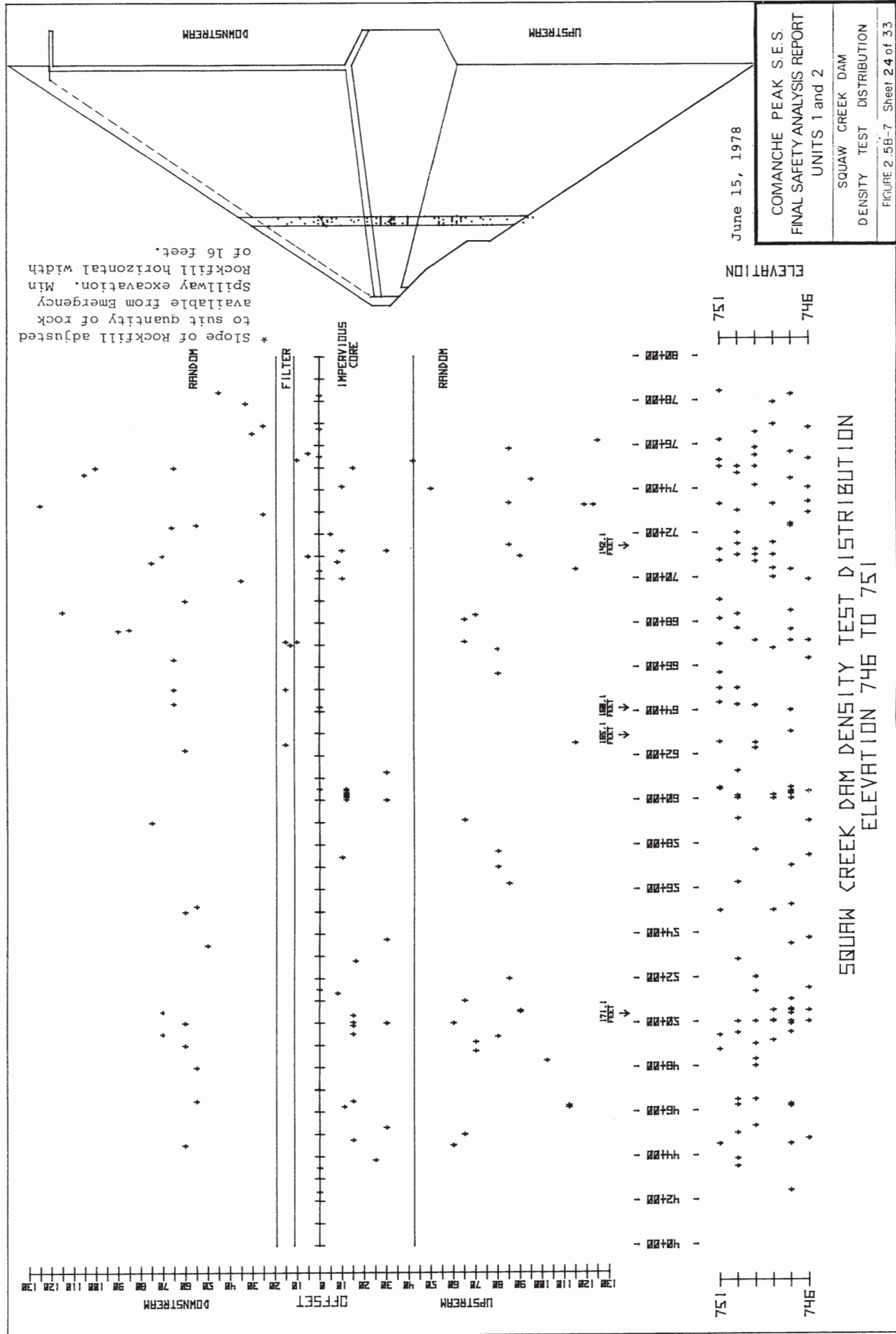
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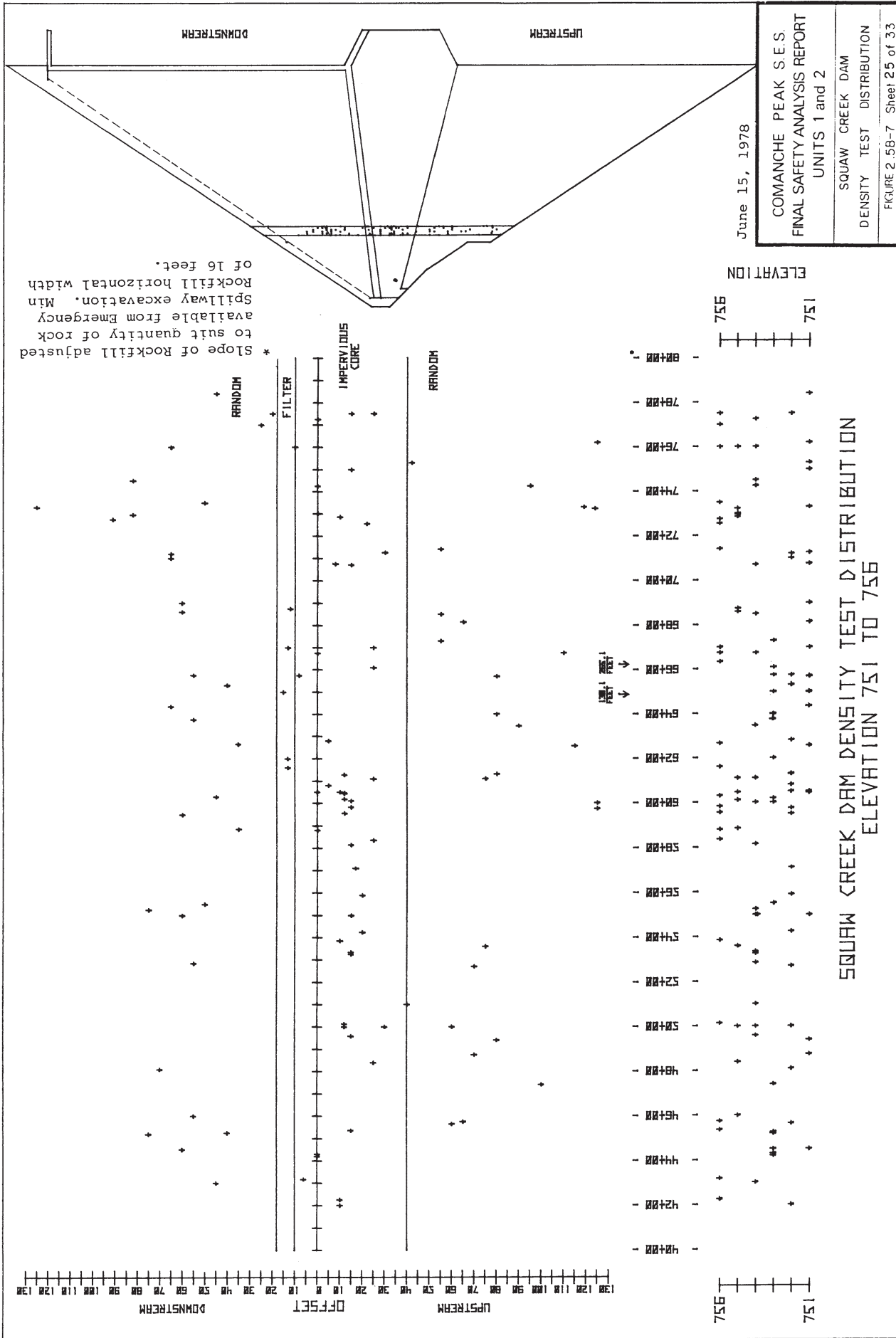
SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION

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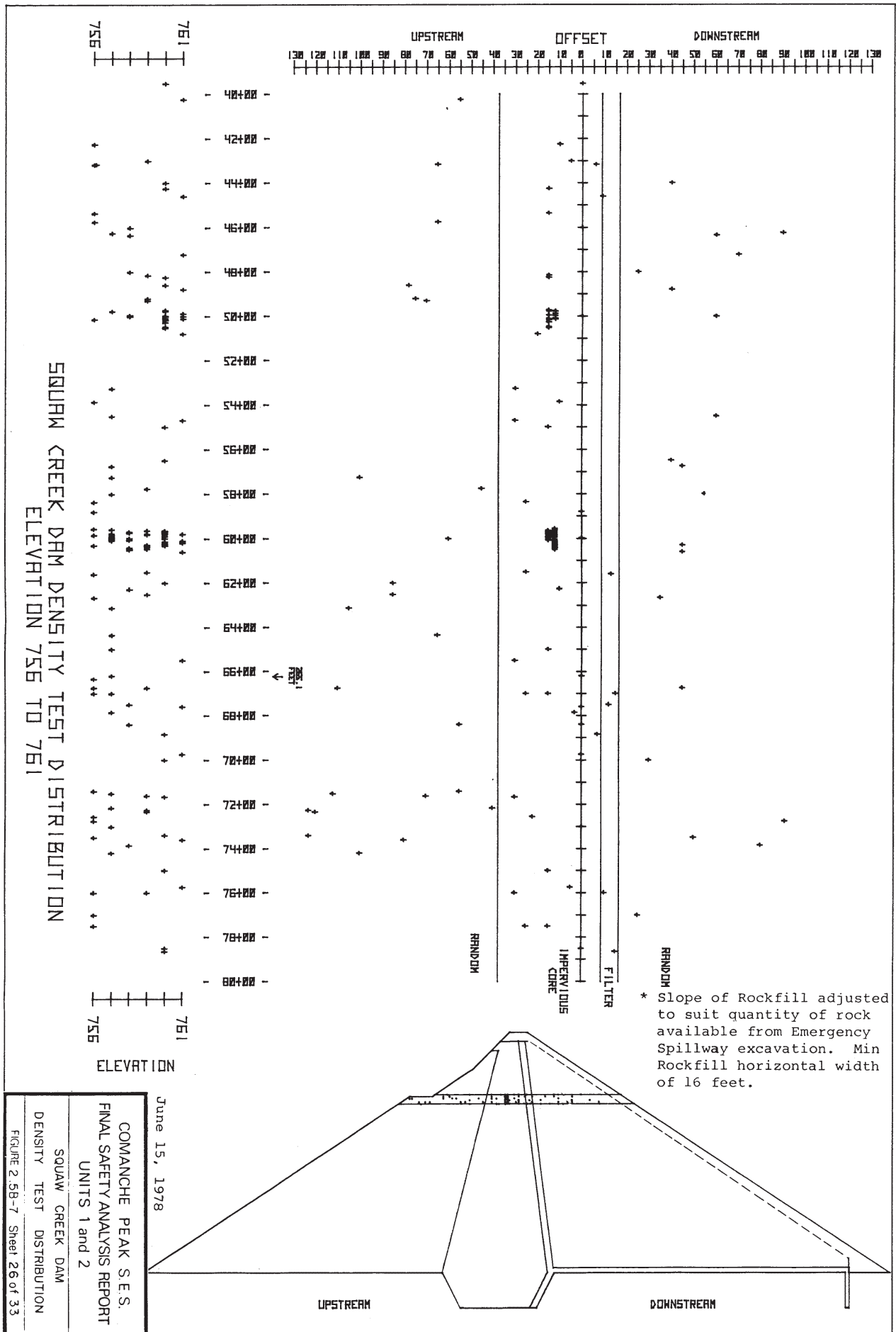
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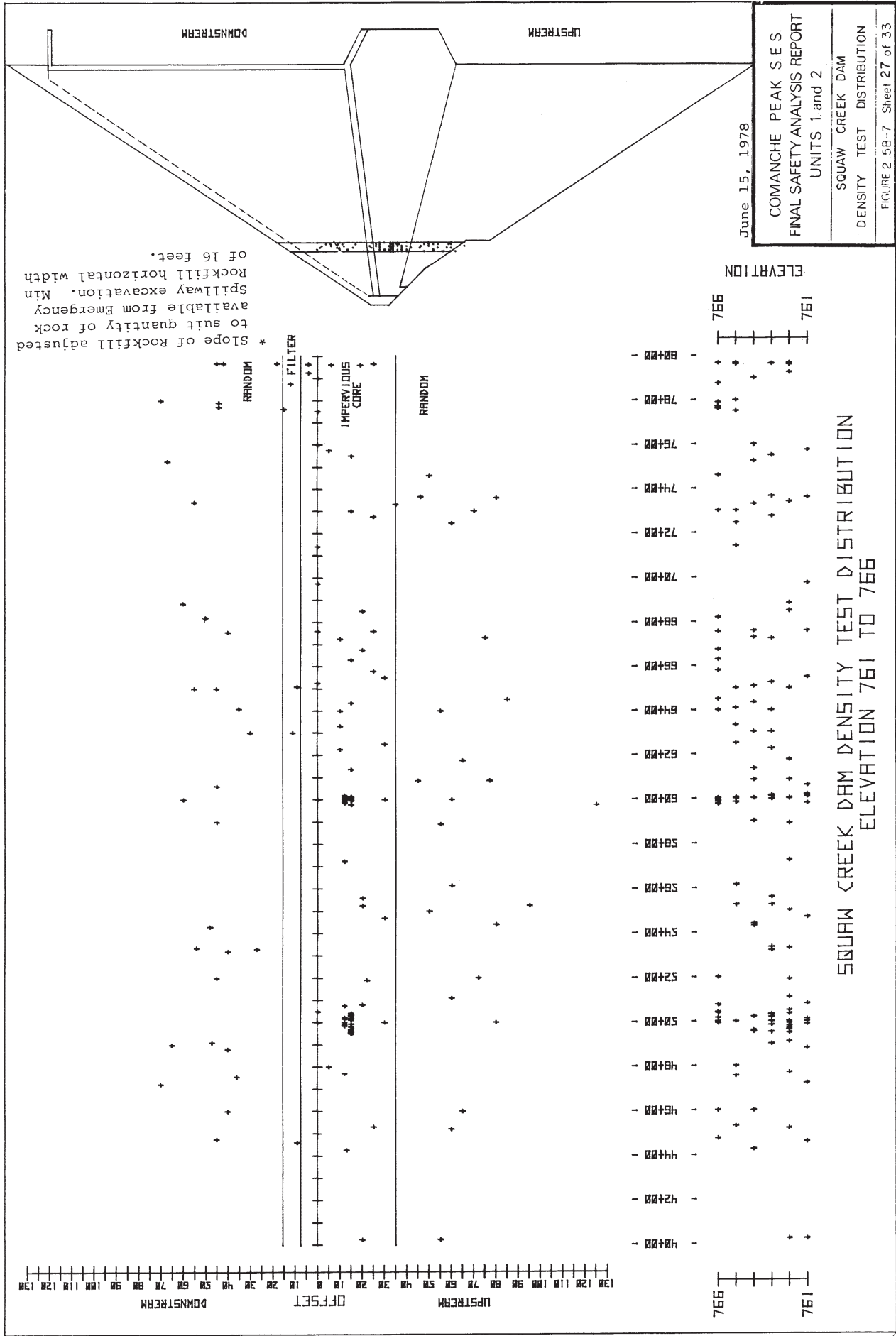
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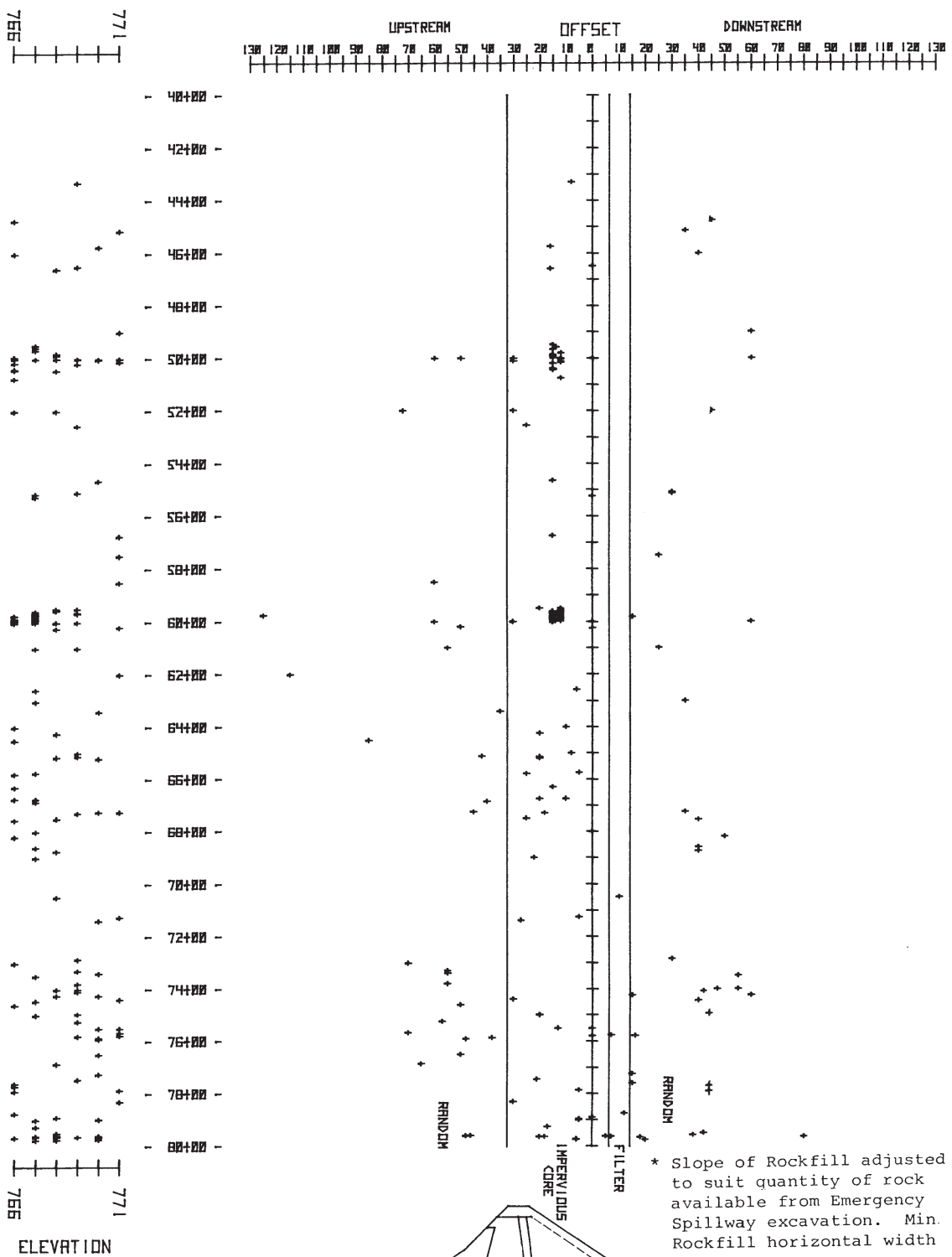
FIGURE 2.58-7 Sheet 25 of 33

SQUAW CREEK DAM DENSITY TEST DISTRIBUTION
ELEVATION 751 TO 756





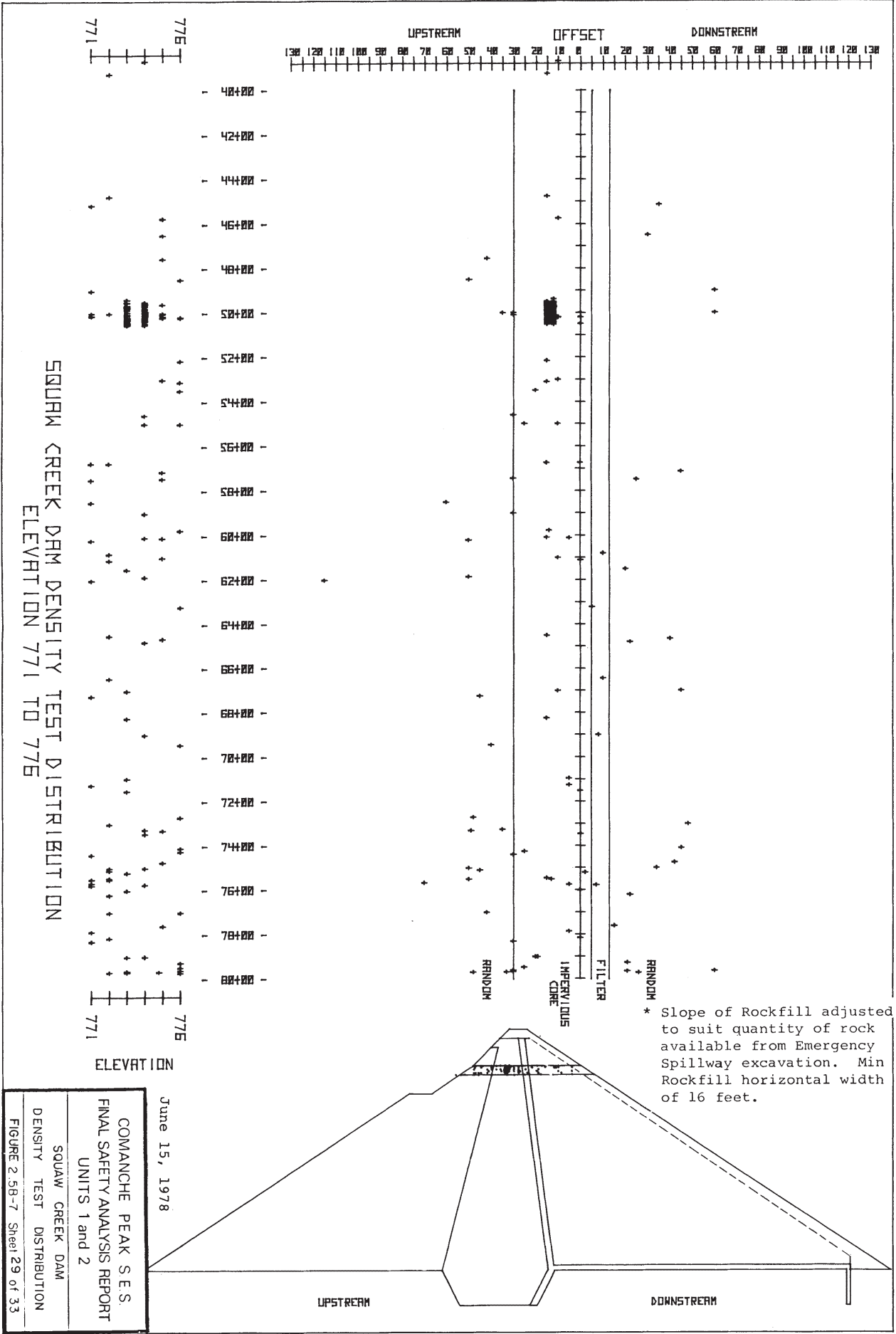
SQUAW CREEK DAM DENSITY TEST DISTRIBUTION ELEVATION 766 TO 771

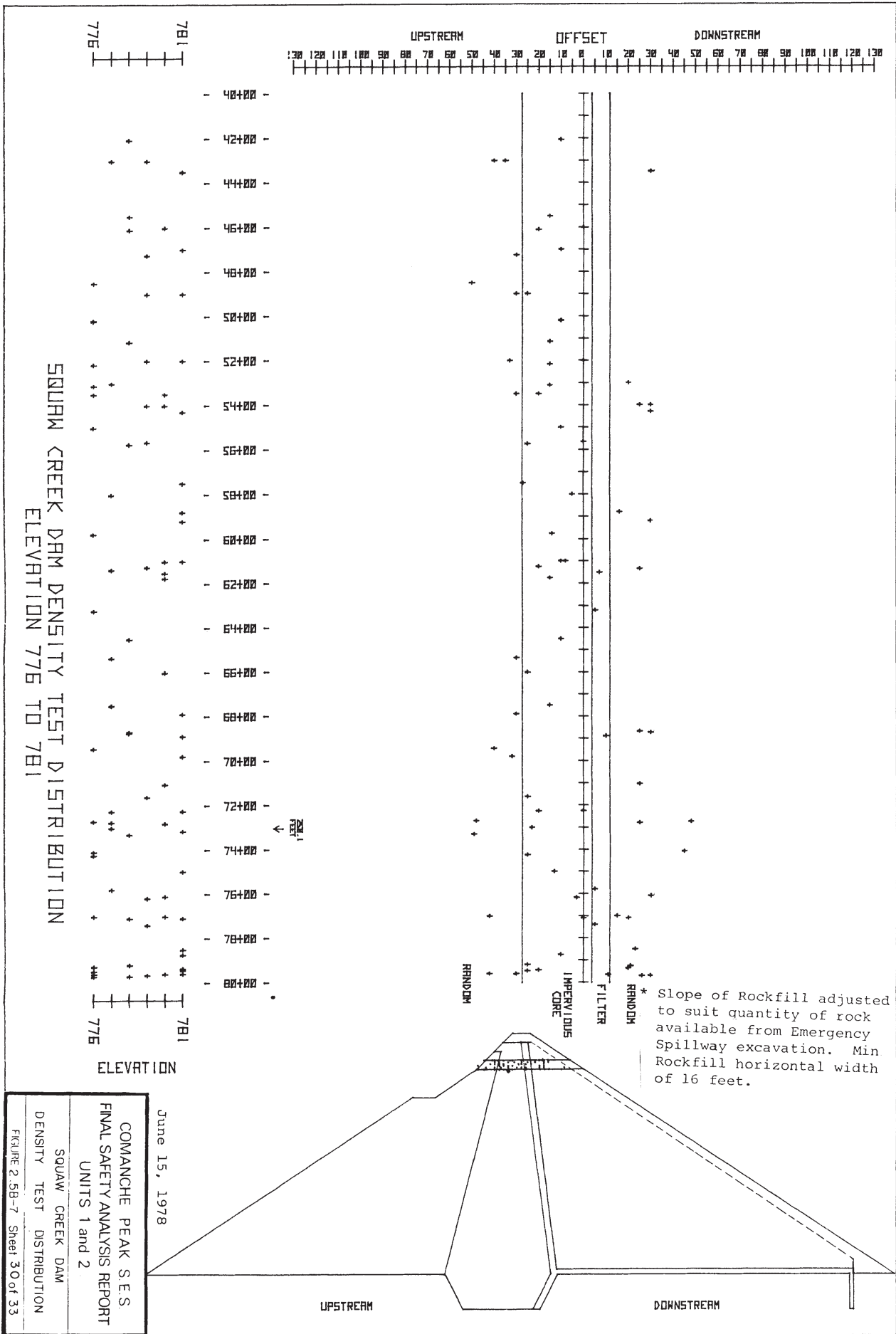


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SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION
FIGURE 2.5B-7 Sheet 28 of 33



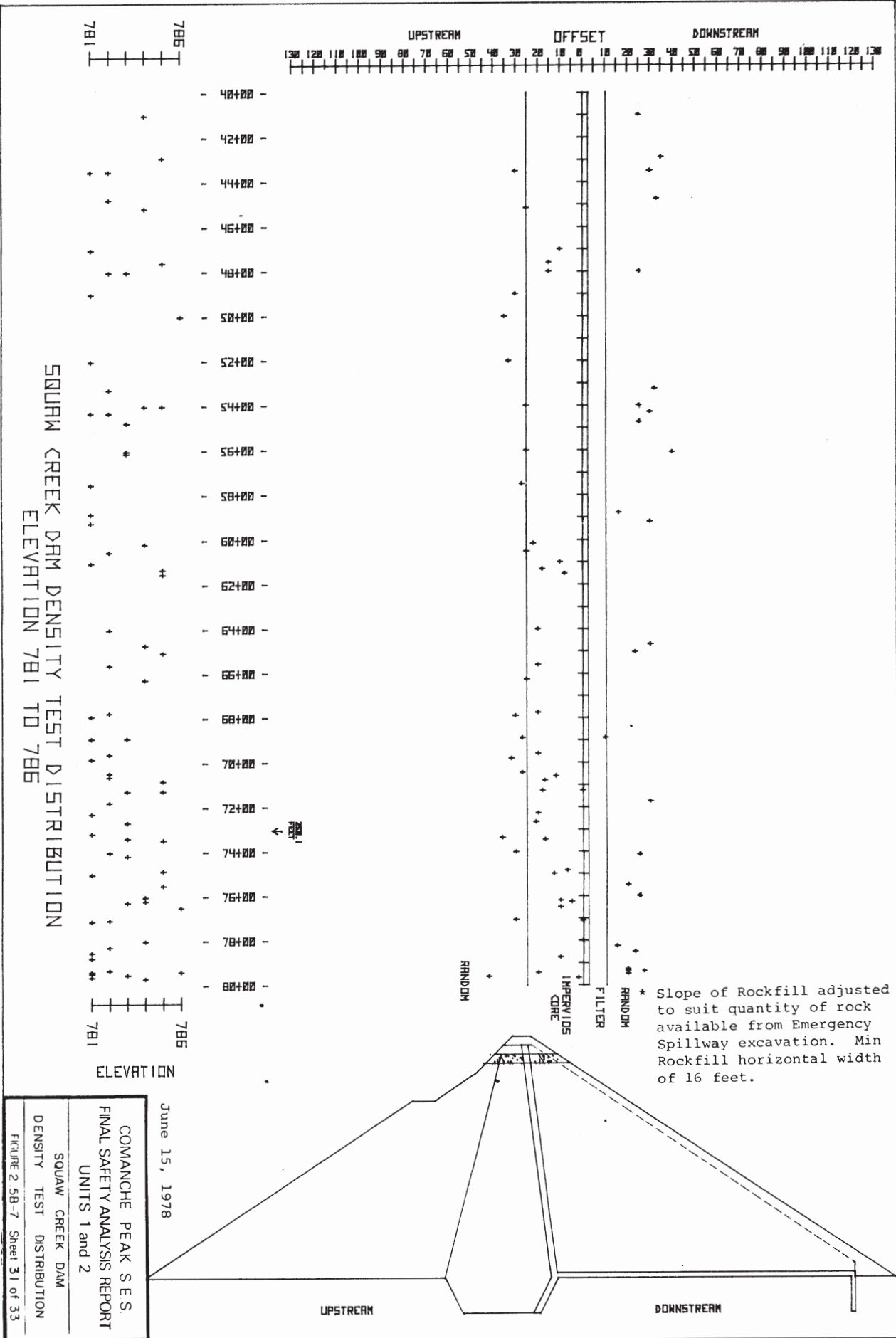


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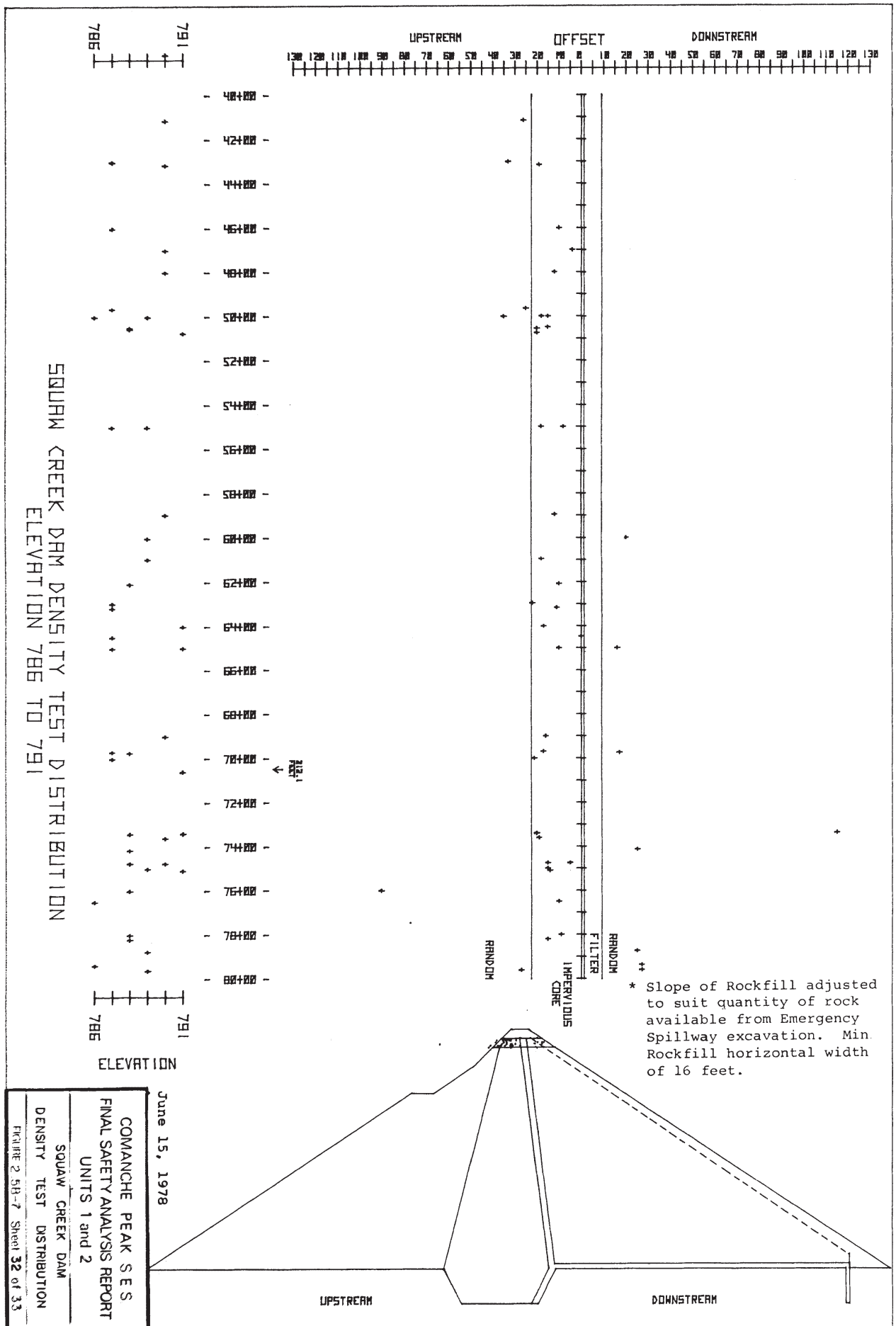


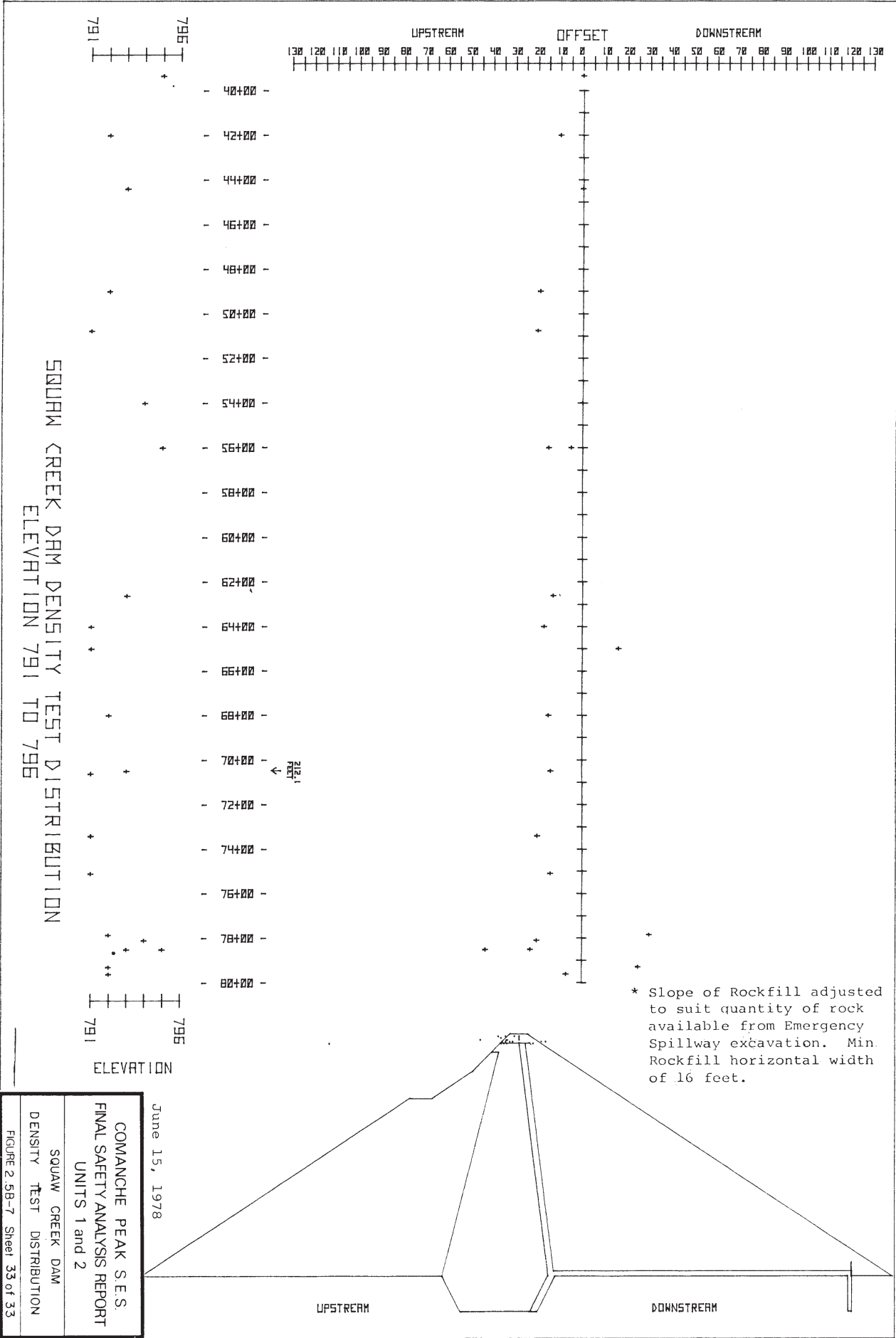
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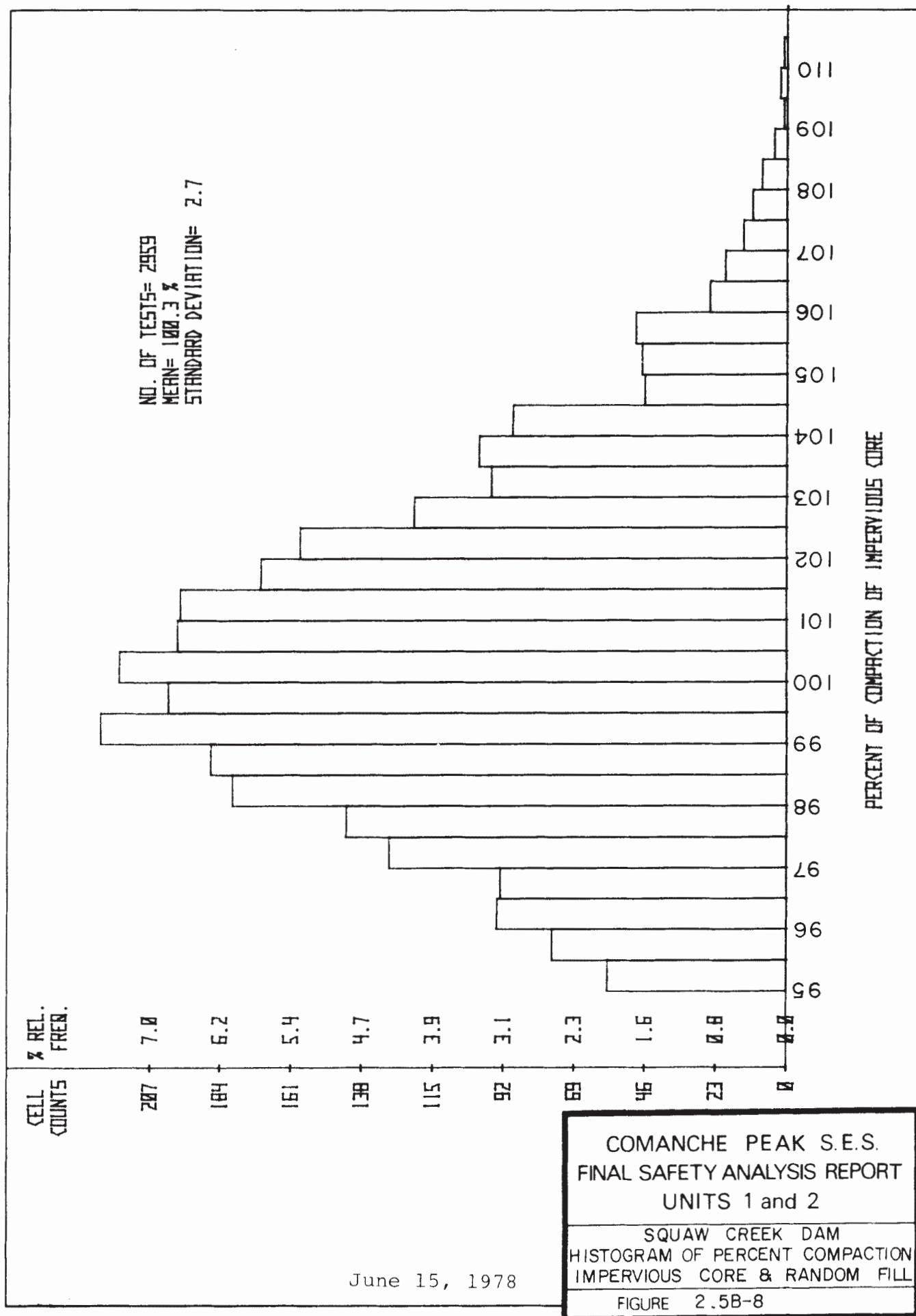
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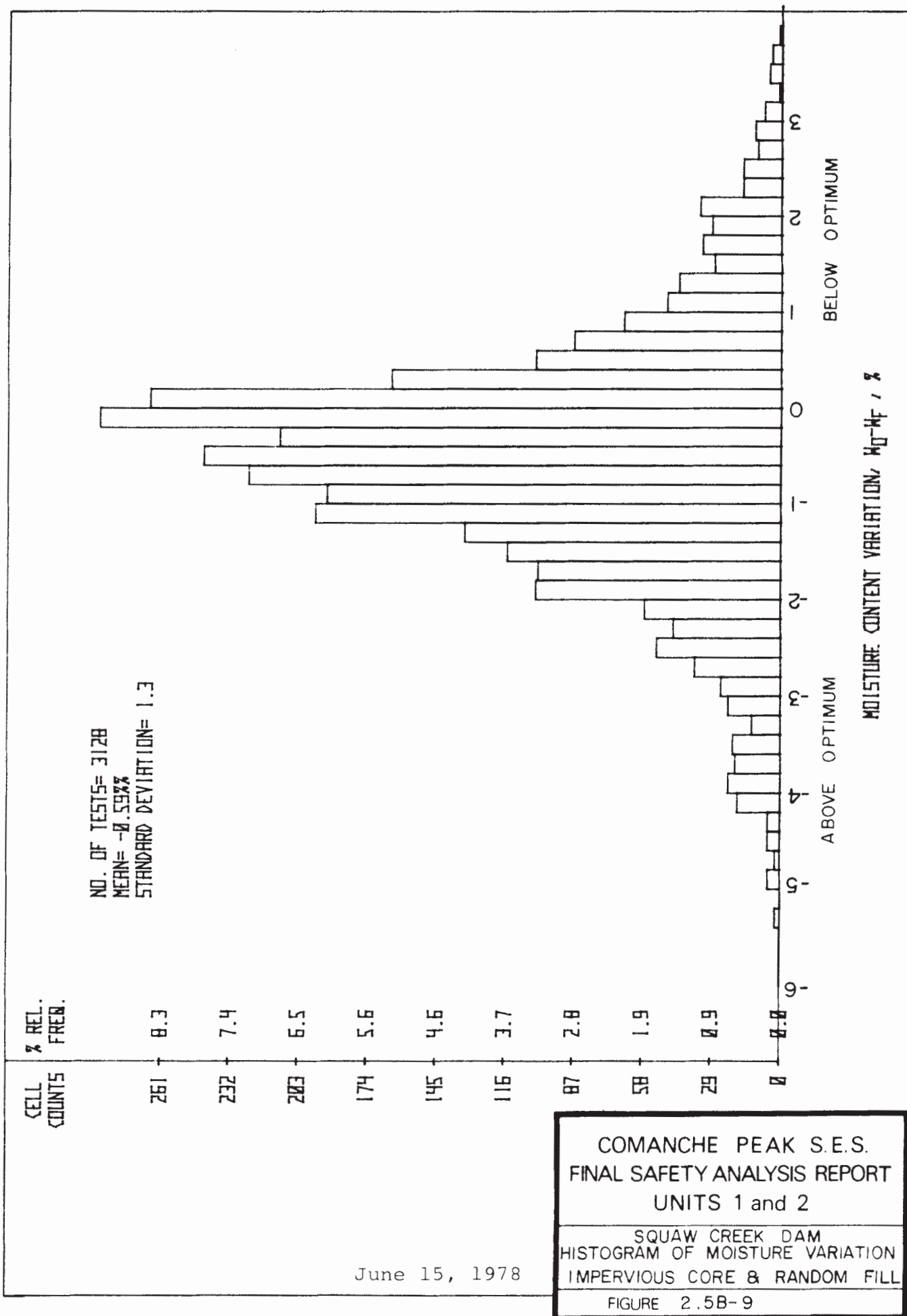
SQUAW CREEK DAM
DENSITY TEST DISTRIBUTION

FIGURE 2 5B-7 Sheet 31 of 33









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SQUAW CREEK DAM
 HISTOGRAM OF MOISTURE VARIATION
 IMPERVIOUS CORE & RANDOM FILL

FIGURE 2.5B-9

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