

# CROW BUTTE RESOURCES, INC.



## *Industrial Ground Water Permit Amendment*

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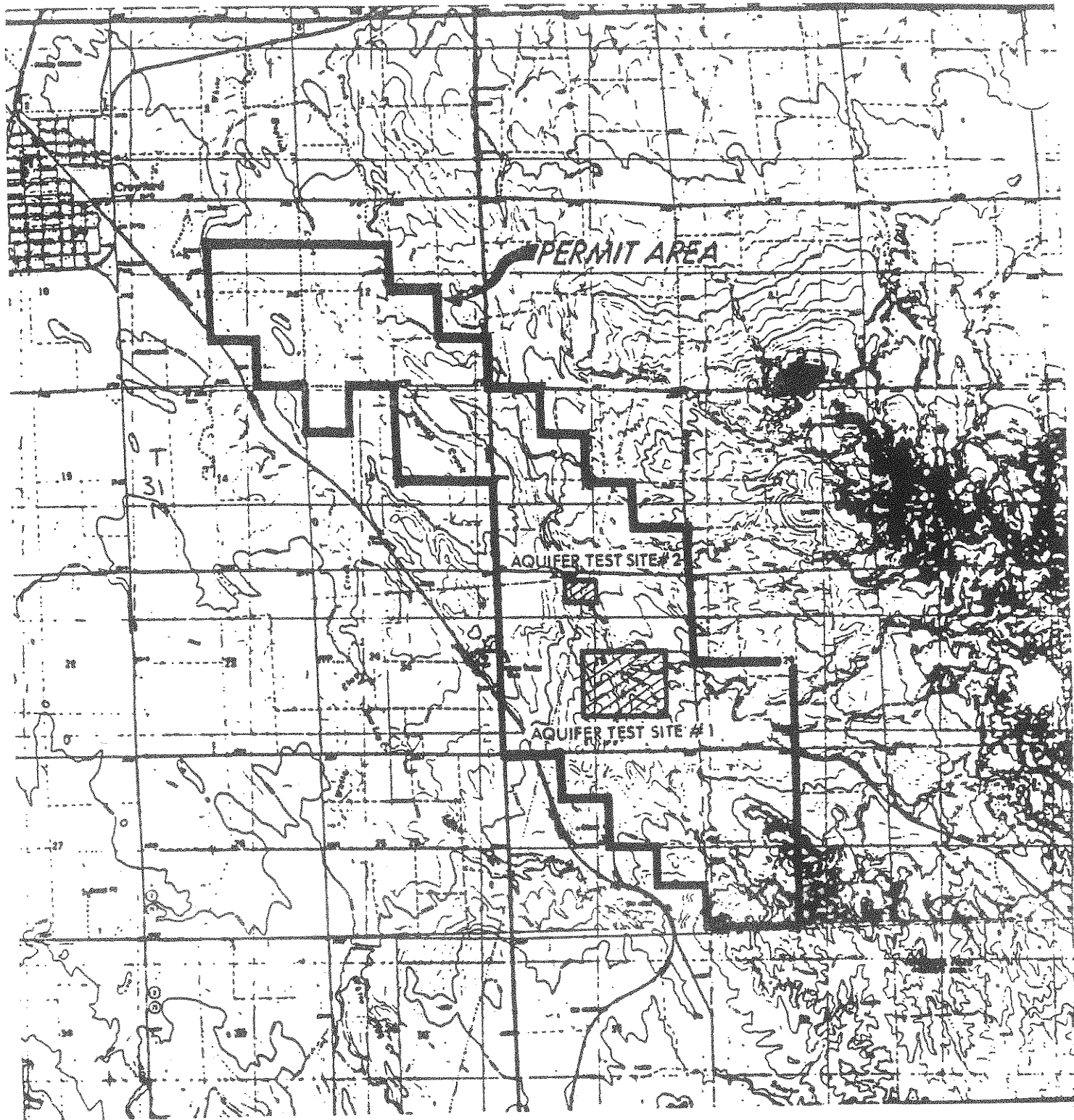
Aquifer Test #2

### Second Aquifer Test:

A second multiple-well aquifer test was performed in the mineralized area near the northern boundary of Section 19. This test was part of a hydrogeologic investigation of the commercial permit area north of the R&D site. This investigation consisted of: (1) a review of existing geologic and hydrogeologic data; (2) design of an appropriate aquifer test; (3) design and construction of an appropriate well array for the aquifer test; (4) laboratory testing of core samples from confining layers; (5) conducting the aquifer test, (6) analyzing the aquifer test data, and (7) interpreting the results. This hydrogeologic investigation was structured to address environmental and operational questions pertinent to ISL uranium mining at the site. Specifically, the requirements outlined by the Nuclear Regulatory Commission (NRC) in Regulatory Guide 3.46, Section 2.7.1 and Draft Staff Technical Position Paper WM-8203, Section 3.1.2. Therefore, this hydrogeologic investigation was oriented toward the characterization of the hydraulic properties of the ore-bearing aquifer, and the hydraulic relationship of the aquifer to the overlying and underlying confining strata and the overlying aquifer. The aquifer test site is located near the north boundary of Section 19, T 31 N, R51 W, Dawes County, Nebraska. This site is approximately 2800 feet north of the R & D site (Figure 2.7-7).

### Site Hydrostratigraphy:

The uranium-bearing aquifer is formed by a coarse-grained arkosic sandstone which is locally known as the Basal Sandstone Member of the Chadron Formation. The Basal Sandstone is believed to be the depositional product of a large, vigorous, braided-stream system which occurred during the early Oligocene age (approximately 36 to 40 million years before present). Regionally, the thickness of the Basal Sandstone ranges from 0 to 350 feet. Exploration drilling in the vicinity of the test site shows that the average thickness of Basal Sandstone is approximately 40 feet. At the test site, the Basal Sandstone is approximately 550 to 600 feet below ground surface. The Chadron Formation lies with marked unconformity on top of the Pierre Shale.



2.7(16) 07/29/87

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DATE	CROW BUTTE PROJECT		
	Dawes County, Nebraska		
	LOCATION MAP		
	PREPARED BY: F.E.N.		
	DWN. BY: JC	DATE: 8/5/87	FIGURE: 2.7-7

The Pierre Shale of late Cretaceous age forms the underlying confining layer for the Basal Chadron Sandstone. The Pierre is a wide-spread dark-gray to black marine shale which is essentially impermeable. Regionally, the Pierre Shale is up to 5000 feet thick. In Dawes County, deep oil test holes have encountered thicknesses of 1200 to 1500 feet of Pierre Shale.

The clays, claystones, and siltstones of the Middle and Upper Members of the Chadron Formation and the Lower Brule Formation form the overlying confining layer for the Basal Chadron Sandstone. At the test site, the overlying confining layer is approximately 315 to 325 feet thick.

Purpose of Investigation:

The purpose of this hydrogeologic investigation was to accurately characterize the hydrogeologic regime of the commercial permit area north of the R&D site as it pertains to ISL uranium mining. The specific objectives of this investigation were to:

- o confirm confinement of the ore-bearing aquifer,
- o determine the transmissivity, hydraulic conductivity, and storativity of the ore-bearing aquifer,
- o determine the azimuth and magnitude of the major and minor axes of transmissivity in the ore-bearing aquifer,
- o use the Neuman-Witherspoon Method to determine the vertical hydraulic conductivity under in situ conditions, of the confining layers which overlie and underlie the ore-bearing aquifer.

In addition to its use in the commercial permit application, the information gathered during this investigation may be used for:

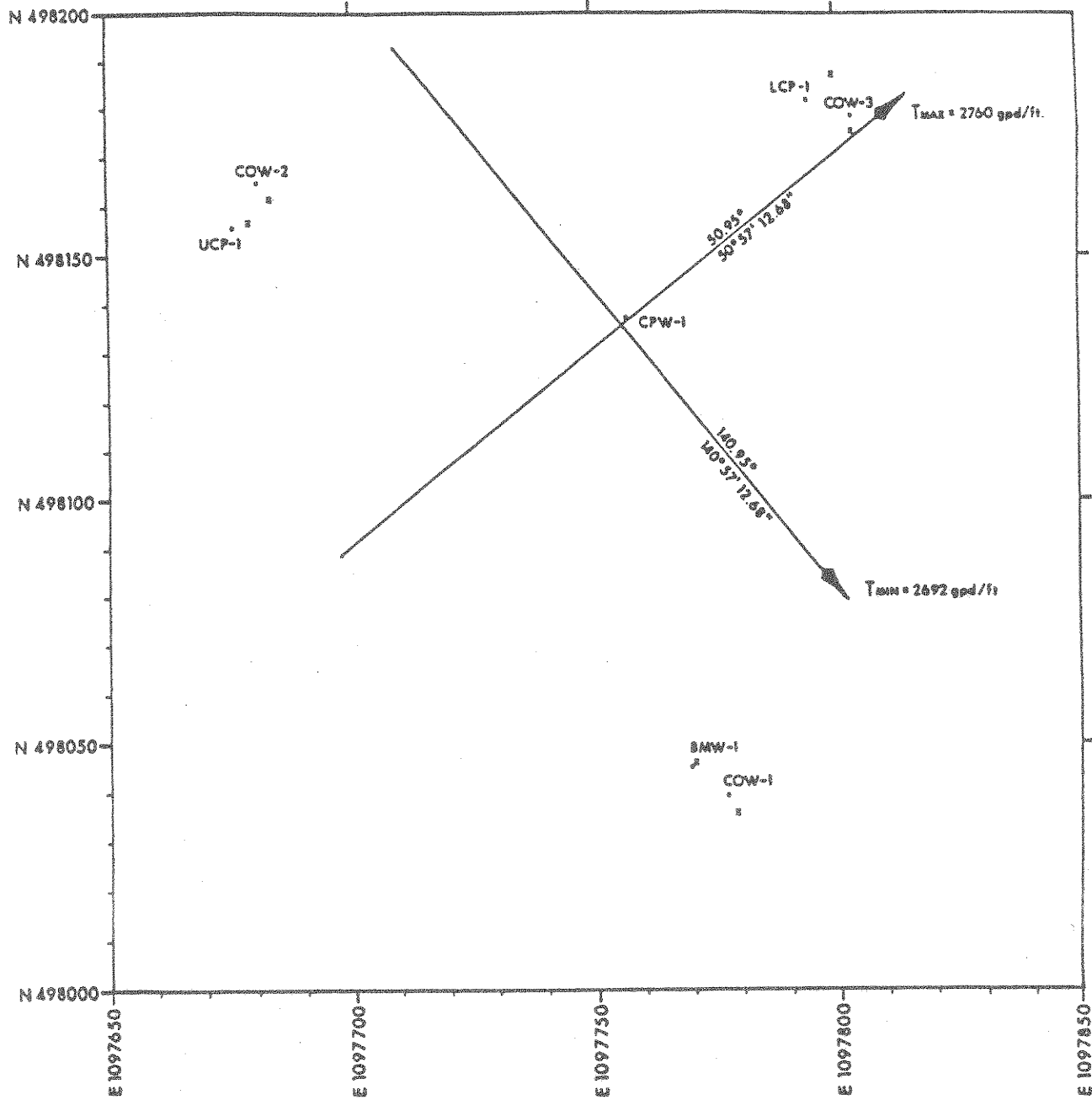
- o design of the commercial wellfield,
- o selection of commercial production parameters,
- o design of the groundwater monitoring system,
- o predictive analysis of the mining and restoration efficiency.

#### AQUIFER TESTING PROGRAM

The aquifer test program was designed to quantify the hydrogeologic parameters recommended by the NRC in Regulatory Guide 3.46, Section 2.7.1, and Draft Staff Technical Position Paper WM-8203. Specifically, this test was designed to allow analysis of the confining layers by the Neuman/Witherspoon Method (1972) which is currently considered by the NRC to be the most applicable to aquifer-aquitard systems commonly associated with uranium deposits.

#### Configuration of Well Array:

The well array used for the aquifer test consisted of five wells and two high-sensitivity piezometers configured as shown in Figure 2.7-8. All of the wells and piezometers used to perform this test were constructed during April and May, 1987 specifically for use in this test. The location and completion details of these wells and piezometers are shown on Tables 2.7-2 and 2.7-3. One pumping well (CPW-1) and three observation wells (COW-1, COW-2, COW-3) were completed in the ore-bearing aquifer (Basal Chadron Sandstone). These wells were screened through the entire thickness of the aquifer (fully penetrating), (Figure 2.7-9). The three observation wells were located in an equiangular arrangement around the central pumping well (Figure 2.7-8). This configuration provided the data needed to define the magnitude and direction of the major and minor axes of transmissivity, the effective transmissivity, the hydraulic conductivity, and the storativity of the ore-bearing aquifer.



**EXPLANATION:**

- SURFACE LOCATION OF WELL
- BOTTOMHOLE LOCATION OF WELL

→ DIRECTION AND MAGNITUDE OF MAJOR AND MINOR AXIS OF TRANSMISSIVITY OF BASAL CHADRON SANDSTONE.

0 50 FEET

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DATE	CROW BUTTE PROJECT		
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	AQUIFER TEST WELL ARRAY		
	PREPARED BY: F.E.N.		
	OWN. BY: J.C.	DATE: 8/5/87	FIGURE: 2.7-8

TABLE 2.7-2

WELL LOCATIONS

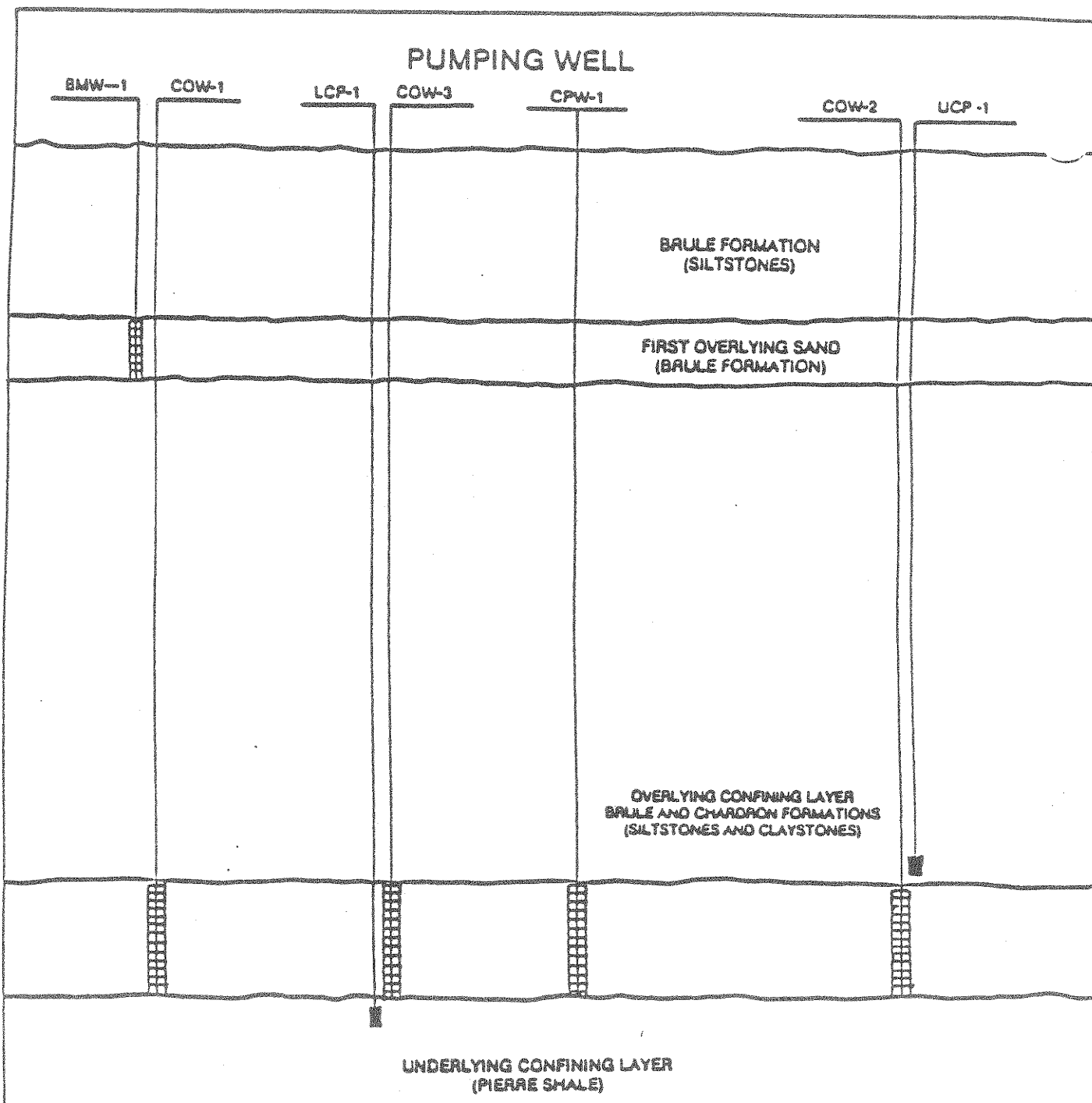
Well	Surface Coordinates (ft)		Deviation (ft)		Bottom-hole Coordinates (ft)		Ground Surface Elevation (ft)		Top of Casing Elevation (ft)	
	E	N	E	N	E	N	E	N	E	N
CPW-1	1,097,757.20	498,137.28	- .64	-1.02	1,097,756.56	498,136.26	3837.55		3838.75	
COW-1	1,097,774.33	498,039.39	+3.02	-2.62	1,097,777.35	498,036.77	3840.21		3842.25	
COW-2	1,097,681.13	498,164.90	+1.89	-2.33	1,097,683.02	498,162.57	3833.61		3835.57	
COW-3	1,097,803.23	498,177.05	- .19	-1.39	1,097,803.04	498,175.66	3840.40		3842.36	
BMW-1	1,097,768.97	498,045.32	+1.63	+ .76	1,097,770.60	498,046.08	3839.85		3841.82	
UCP-1	1,097,676.19	498,156.47	+2.33	+ .58	1,097,678.52	498,157.05	3834.16		3836.82	
LCP-1	1,097,794.73	498,181.79	+4.41	+6.07	1,097,799.14	498,187.86	3840.02		3840.98	

TABLE 2.7-3

## WELL COMPLETION DETAILS

Well	Open Interval Depth (ft)	Completion Stratum	Casing Size I.D. (in)	Total Depth (ft)	From CPW-1 (bottomhole) Distance (ft)	Azimuth	Elevation of Piezometric Surface in ft above MSL (6/28/87)
CPW-1	572-612	Basal Chadron	4.5	617	----	----	3749.3
OCW-1	585-625	Basal Chadron	4.5	630	101.64	168.20°	3749.4
OCW-2	565-610	Basal Chadron	4.5	615	78.10	289.69°	3749.3
OCW-3	575-615	Basal Chadron	4.5	620	60.93	49.71°	3749.4
BMW-1	235-260	Upper Aquifer	4.5	265	91.27	171.15°	3808.0
UCP-1	555-557	Upper Aquiclude	2.0	557	80.76	284.92°	3750.7
LCP-1	618-620	Lower Aquiclude	2.0	620	66.90	39.53	3748.8





NOT TO SCALE

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	Dawes County, Nebraska		
	SCHEMATIC OF WELL		
	COMPLETION INTERVALS		
	PREPARED BY: F. E. N.		
	DWN. BY: JC	DATE: 8/5/87	FIGURE: 2.7-9

One monitor well (BMW-1) was completed in the first overlying sand of the Brule Formation (Figure 2.7-9). Well BMW-1 is also screened through the entire thickness of the aquifer (fully penetrating). This well was used to monitor the water level in the first overlying sand during the aquifer test.

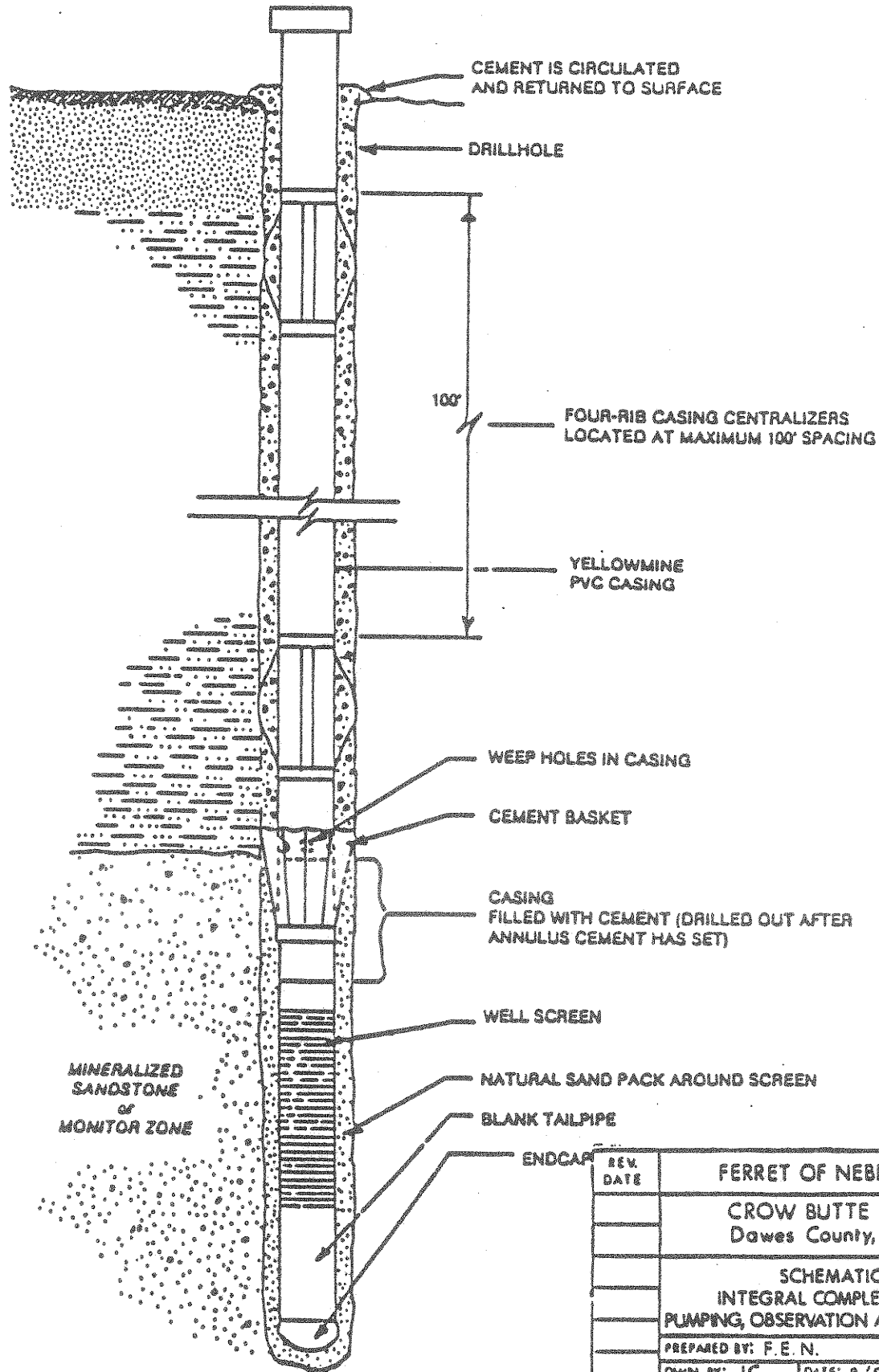
Two small-diameter, high-sensitivity piezometers (UCP-1, LCP-1) were completed in the confining layers which overlie and underlie the ore-bearing aquifer (Figure 2.7-9). These piezometers provided the data to calculate the vertical hydraulic conductivities of these confining layers under in-situ field conditions.

#### Well Construction and Completion Techniques

All well and piezometer boreholes were drilled with a conventional rotary drill rig using a bentonite based drilling fluid. The borehole was drilled to the appropriate depth and was geophysically logged. The log suite consisted of a gamma log, a resistivity log, a neutron log and a deviation survey. The geophysical logs were then used to determine the exact completion interval of each well or piezometer.

The pumping, observation and monitor wells were completed by a single stage or integral completion method. Figure 2.7-10 is a schematic of this completion method. This method consisted of drilling a nominal 8-inch borehole to the desired depth. Next, a string of 4.5-inch diameter Yelomine casing with the desired length of screen attached to the lower end was placed in the hole. A cement basket was attached to the blank casing just above the screen to exclude cement from the screen interval during cementing. The cement was then pumped down the inside of the casing to a plug set just below the cement basket. The cement passed out through weep holes in the casing above the cement basket and was directed by the cement basket back to the surface through the annulus between the casing and the drill hole. After the cement had cured sufficiently, the residual cement and plug were drilled out. The completed wells were then developed by air-lifting. The confining layer piezometers were cased with two-inch I.D. Yelomine casing and a porous stone tip. The porous stone tip was two feet

# WELL COMPLETION METHOD



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		CROW BUTTE PROJECT
		Dawes County, Nebraska
		SCHMATIC OF
		INTEGRAL COMPLETION METHOD FOR
		PUMPING, OBSERVATION AND MONITOR WELL
		PREPARED BY: F. E. N.
		DWN. BY: JC DATE: 8/5/87 FIGURE: 2.7-10



The average hydraulic conductivity of the entire system was found to be almost the same as the hydraulic conductivity of the Red Clay. Furthermore, from the analysis of the aquifer/aquitard interaction from the formation consolidation standpoint, it is apparent that during the period of the pumping test, the water released from the upper aquitard is entirely from the Red Clay. Pore pressure changes at the bottom of the Red Clay did not propagate through the clay into the overlying sandy claystone over the pumping test period. Applying the theory of consolidation (Scott, 1963), the volume of water which could be liberated from the Red Clay under induced drawdown was calculated from the relationship:

(Eq. 6)

$$Q_T = \frac{2 K_v U_i}{\gamma_w \pi C_v} \sqrt{t}$$

Where:

$Q_T$  = volume of water released from the confining bed during the time  $t$   
 $K_v$  = vertical hydraulic conductivity of the confining bed  
 $\gamma_w$  = unit weight of water  
 $U_i$  = induced change in effective overburden pressure, proportional to drawdown ( $s = U/\gamma_w$ )  
 $C_v$  = coefficient of consolidation  
 $t$  = time since drawdown occurred  
 $s$  = drawdown

The analysis showed that Red Clay could release one gallon of water per one foot of drawdown per acre during the 2.09 days (i.e., during the entire pumping test). Using the values of drawdown for a given distance from the pumping well presented in Figure 2.7A-8 and the volumes of water which could be released from confinement, the overall contribution from aquifer upper confinement to the flow produced during the pumping test was calculated. The results of calculations are also illustrated in Figure 2.7A-9. The volume of water released from the Red Clay during the pumping test was thus computed to be about 1,000 gallons. This constitutes approximately 1.4% of the overall flow produced during the pumping test.

The contribution from the Pierre Shale owing to its lower hydraulic conductivity (approximately one order of magnitude less than the upper confinement)(Table 2.7A-6) would be significantly smaller - about 0.06 gallon of water per foot of drawdown per acre - during the entire pumping test. Figure 2.7A-9 illustrates the relationship between volume of inflow

TABLE 2.7-4

RESULTS OF CONSOLIDATION TESTS  
OF CONFINING LAYER CORE SAMPLES

Borehole	Depth (ft)	Lithology	Porosity	Coefficient of Consolidation, $c_v$ ( $\text{cm}^2/\text{sec.}$ )	Compression Index, $C_c$	Coefficient of Compressibility, $a_v$ ( $\text{cm}^2/\text{g}$ )	Vertical Hydraulic Conductivity, $k_v$ <sup>(1)</sup> ( $\text{cm}/\text{sec.}$ )
UCP-1	546.5	red clay	.341	$6.65 \times 10^{-5}$	$2.75 \times 10^{-2}$	$4.46 \times 10^{-7}$	$2.22 \times 10^{-11}$
UCP-1	550.6	red clay	.328	$1.13 \times 10^{-4}$	$2.69 \times 10^{-2}$	$4.37 \times 10^{-7}$	$3.78 \times 10^{-11}$
UCP-1	555.6	red clay	.284	$1.78 \times 10^{-4}$	$1.94 \times 10^{-2}$	$3.15 \times 10^{-7}$	$4.46 \times 10^{-11}$
UCP-1	Average		.318	$1.19 \times 10^{-4}$	$2.46 \times 10^{-2}$	$3.99 \times 10^{-7}$	$3.49 \times 10^{-11}$
LCP-1	617.0	shale	.317	$1.04 \times 10^{-4}$	$2.28 \times 10^{-2}$	$3.70 \times 10^{-7}$	$2.89 \times 10^{-11}$
LCP-1	621.8	shale	.333	$9.10 \times 10^{-5}$	$4.04 \times 10^{-2}$	$6.56 \times 10^{-7}$	$4.36 \times 10^{-11}$
LCP-1	Average		.325	$9.70 \times 10^{-5}$	$3.16 \times 10^{-2}$	$5.13 \times 10^{-7}$	$3.63 \times 10^{-11}$

(1) Calculated for 600 psi effective overburden pressure from consolidation test data.

Haliburton meters which measured both flow rate and volume were installed in the discharge line to measure instantaneous discharge rate and cumulative discharge volume. Only one meter was used at any one time, keeping the second in reserve as a backup.

The discharge line extended about 400 feet from the wellhead to prevent discharged water from leaking downward and recharging the shallow overlying aquifer. The three Chadron observation wells (COW-1, COW-2, and COW-3), the overlying monitor well (BMW-1), and the two confining layer piezometers (UCP-1 and LCP-1) were equipped with electronic pressure transducers. These six pressure transducers were connected to a computer-controlled datalogger which automatically recorded the water levels in each well at specified time intervals. A seventh electronic pressure transducer was used to measure barometric pressure which was also recorded by the datalogger each time the water levels were recorded.

#### Aquifer Test

The pumping phase of the aquifer test began at 12:47 on June 30, 1987 and concluded at approximately 12:47 on July 3, 1987. Thus, the length of the pumping phase of the test was 4322 minutes, or about 72 hours. Just prior to the start of the pumping, static water levels of all the wells were measured and recorded (Table 2.7-5). The recovery phase of the test began at 12:47 on July 3, 1987 and concluded at 13:17 on July 6, 1987, which is a period of 4350 minutes, or 72.5 hours.

The average discharge rate during the pumping phase of the test was 47.74 gpm and the total volume of water discharged was 206,288 gallons. Throughout the pumping phase, the discharge rate was regularly monitored to insure that it remained constant. Tables 2.7-6 and 2.7-7 present the recorded drawdown and recovery data corrected for changes in barometric pressure. The static water level in the pumped well was approximately 484 feet above the top of the aquifer. The calculated maximum drawdown in the pumped well was 36.86 feet, which is approximately 447 feet above the top of the aquifer. Therefore, the aquifer was under confined conditions throughout the test.

TABLE 2.7-5

STATIC WATER LEVELS

<u>Well</u>	<u>Static Water Level 6/30/87</u> <u>(ft. above MSL)</u>
CPW-1	----- *
COW-1	3749.5
COW-2	3749.5
COW-3	3749.5
BMW-1	3808.2
UCP-1	3751.3
LCP-1	3749.4

\* Could not measure water level because pump was in well.



TABLE 2.7-6  
DRAWDOWN DATA

DATE	TIME	RELAXED TIME										ELAPSED TIME									
		MIN	SEC	LOW-1	LOW-2	LOW-3	LLP-1	UCI-1	PMW-1	BAROM	HOURS	MIN.	SEC.	TOTAL MIN.							
1	1247	24.750	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.000	0.00							
2	1247	43.750	0.02	0.06	0.06	0.00	-0.00	0.00	-0.00	0.00	0.0	0.0	9.875	0.16							
3	1247	52.750	0.04	0.18	0.18	-0.02	-0.00	0.00	-0.00	0.00	0.0	0.0	19.750	0.33							
4	1248	1.750	0.09	0.36	0.36	0.05	-0.00	0.00	-0.01	0.00	0.0	0.0	29.625	0.49							
5	1248	10.750	0.17	0.57	0.57	0.06	-0.00	0.00	-0.00	0.00	0.0	0.0	49.500	0.66							
6	1248	19.750	0.27	0.75	0.75	0.10	-0.00	0.00	-0.00	0.00	0.0	0.0	59.375	0.82							
7	1248	28.750	0.38	0.96	0.96	0.14	-0.01	0.00	-0.00	0.00	0.0	0.0	69.250	0.99							
8	1248	39.750	0.53	1.17	1.17	0.15	-0.01	0.00	-0.00	0.00	0.0	0.0	79.125	1.18							
9	1248	50.750	0.68	1.37	1.37	0.22	-0.00	0.00	-0.01	0.00	0.0	0.0	89.000	1.38							
10	1249	1.750	0.84	1.57	1.57	0.32	-0.00	0.00	-0.01	0.00	0.0	0.0	98.875	1.58							
11	1249	12.750	0.99	1.75	1.75	0.44	-0.00	0.00	-0.01	0.00	0.0	0.0	108.750	1.78							
12	1249	23.750	1.14	1.89	1.89	0.51	-0.01	0.00	-0.00	0.00	0.0	0.0	118.625	1.98							
13	1249	34.750	1.28	2.05	2.05	0.59	-0.00	0.00	-0.00	0.00	0.0	0.0	128.500	2.17							
14	1249	45.750	1.41	2.19	2.19	0.64	-0.00	0.00	-0.01	0.00	0.0	0.0	138.375	2.37							
15	1249	56.750	1.54	2.32	2.32	0.76	-0.00	0.00	-0.00	0.00	0.0	0.0	148.250	2.57							
16	1250	7.750	1.67	2.41	2.41	0.78	-0.00	0.00	-0.00	0.00	0.0	0.0	158.125	2.77							
17	1250	18.750	1.79	2.52	2.52	0.89	-0.00	0.00	-0.00	0.00	0.0	0.0	168.000	2.97							
18	1250	32.750	1.93	2.66	2.66	0.98	-0.00	0.00	-0.00	0.00	0.0	0.0	177.875	3.17							
19	1250	46.750	2.07	2.76	2.76	1.15	-0.00	0.00	-0.00	0.00	0.0	0.0	187.750	3.36							
20	1251	0.750	2.20	2.86	2.86	1.20	-0.00	0.00	-0.00	0.00	0.0	0.0	197.625	3.56							
21	1251	14.750	2.31	2.99	2.99	1.30	-0.00	0.00	-0.00	0.00	0.0	0.0	207.500	3.71							
22	1251	28.750	2.42	3.06	3.06	1.48	-0.00	0.00	-0.00	0.00	0.0	0.0	217.375	3.96							
23	1251	42.750	2.52	3.16	3.16	1.54	-0.00	0.00	-0.00	0.00	0.0	0.0	227.250	4.21							
24	1251	56.750	2.61	3.22	3.22	1.66	-0.00	0.00	-0.00	0.00	0.0	0.0	237.125	4.45							
25	1251	10.750	2.71	3.30	3.30	1.76	-0.00	0.00	-0.00	0.00	0.0	0.0	247.000	4.70							
26	1252	29.750	2.82	3.40	3.40	1.95	-0.00	0.00	-0.00	0.00	0.0	0.0	256.875	4.95							
27	1252	48.750	2.93	3.52	3.52	2.01	-0.00	0.00	-0.00	0.00	0.0	0.0	266.750	5.28							
28	1253	7.750	3.03	3.60	3.60	2.24	-0.00	0.00	-0.00	0.00	0.0	0.0	276.625	5.61							
29	1253	26.750	3.13	3.69	3.69	2.36	-0.00	0.00	-0.00	0.00	0.0	0.0	286.500	5.94							
30	1253	45.750	3.22	3.81	3.81	2.42	-0.00	0.00	-0.00	0.00	0.0	0.0	296.375	6.27							
31	1254	4.750	3.32	3.87	3.87	2.58	-0.00	0.00	-0.00	0.00	0.0	0.0	306.250	6.60							
32	1254	23.750	3.42	3.98	3.98	2.77	-0.00	0.00	-0.00	0.00	0.0	0.0	316.125	6.93							
33	1254	42.750	3.49	4.06	4.06	2.81	-0.00	0.00	-0.00	0.00	0.0	0.0	326.000	7.27							
34	1255	1.750	3.57	4.13	4.13	2.97	-0.00	0.00	-0.00	0.00	0.0	0.0	335.875	7.60							
35	1255	20.750	3.64	4.19	4.19	3.09	-0.00	0.00	-0.00	0.00	0.0	0.0	345.750	7.93							
36	1255	39.750	3.72	4.24	4.24	3.18	-0.00	0.00	-0.00	0.00	0.0	0.0	355.625	8.26							
37	1255	58.750	3.80	4.34	4.34	3.32	-0.00	0.00	-0.00	0.00	0.0	0.0	365.500	8.59							
38	1256	17.750	3.86	4.38	4.38	3.48	-0.00	0.00	-0.00	0.00	0.0	0.0	375.375	8.92							
39	1256	36.750	3.92	4.44	4.44	3.55	-0.00	0.00	-0.00	0.00	0.0	0.0	385.250	9.25							
40	1256	55.750	3.99	4.53	4.53	3.71	-0.00	0.00	-0.00	0.00	0.0	0.0	395.125	9.58							
41	1257	94.750	4.16	4.68	4.68	3.94	-0.01	0.00	-0.01	0.00	0.0	0.0	405.000	9.92							
42	1258	53.750	4.33	4.83	4.83	4.23	-0.00	0.00	-0.00	0.00	0.0	0.0	414.875	10.25							
43	1259	54.750	4.47	4.97	4.97	4.37	-0.00	0.00	-0.00	0.00	0.0	0.0	424.750	10.58							
44	1300	51.750	4.60	5.12	4.78	4.78	-0.00	0.00	-0.00	0.00	0.0	0.0	434.625	10.91							
45	1301	50.750	4.72	5.23	4.99	4.99	-0.00	0.00	-0.00	0.00	0.0	0.0	444.500	11.24							
46	1302	49.750	4.85	5.34	5.22	5.22	-0.00	0.00	-0.00	0.00	0.0	0.0	454.375	11.57							
47	1303	48.750	4.95	5.45	5.37	5.37	-0.00	0.00	-0.00	0.00	0.0	0.0	464.250	11.90							
48	1304	47.750	5.05	5.55	5.61	5.61	-0.00	0.00	-0.00	0.00	0.0	0.0	474.125	12.23							
49	1305	46.750	5.15	5.62	5.69	5.69	-0.00	0.00	-0.00	0.00	0.0	0.0	484.000	12.56							
50	1306	45.750	5.25	5.73	5.92	5.92	-0.00	0.00	-0.00	0.00	0.0	0.0	493.875	12.89							
51	1308	44.750	5.41	5.88	6.12	6.12	-0.00	0.00	-0.00	0.00	0.0	0.0	503.750	13.22							
52	1310	43.750	5.56	6.05	6.43	6.43	-0.00	0.00	-0.00	0.00	0.0	0.0	513.625	13.55							

TABLE 2.7-6

53	181	1312	41.750	5.77	6.18	6.61	0.00	0.01	0.01	0.00	0.00	0.00	0.00	0.00	0.00	25.0	53.275	25.84
54	181	1314	41.750	5.85	6.31	6.85	0.01	0.01	0.00	-0.01	0.01	0.00	0.00	0.00	0.00	27.0	53.250	27.84
55	181	1316	40.750	5.97	6.44	6.97	-0.01	-0.00	-0.00	0.00	-0.00	0.00	0.00	0.00	0.00	29.0	53.125	29.84
56	181	1318	39.750	6.11	6.59	7.20	-0.00	0.00	0.00	-0.00	0.00	0.00	0.00	0.00	0.00	31.0	53.000	31.88
57	181	1320	38.750	6.24	6.70	7.32	-0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	33.0	52.875	33.88
58	181	1322	37.750	6.38	6.79	7.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	35.0	52.750	35.88
59	181	1324	36.750	6.43	6.92	7.64	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	37.0	52.625	37.88
60	181	1326	35.750	6.54	7.02	7.73	-0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	39.0	52.500	39.87
61	181	1328	34.750	6.63	7.10	7.85	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	41.0	52.375	41.87
62	181	1330	33.750	6.72	7.20	8.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	43.0	52.250	43.87
63	181	1332	32.750	6.81	7.24	8.11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	45.0	52.125	45.87
64	181	1334	31.750	6.88	7.36	8.11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	47.0	52.000	47.87
65	181	1336	30.750	6.96	7.46	8.26	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	49.0	51.875	49.86
66	181	1339	29.750	7.08	7.57	8.40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	51.0	51.750	51.86
67	181	1342	28.750	7.19	7.69	8.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	53.0	51.625	53.86
68	181	1345	27.750	7.24	7.81	8.64	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	55.0	51.500	55.86
69	181	1348	26.750	7.39	7.90	8.74	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	57.0	51.375	57.86
70	181	1351	25.750	7.48	7.99	8.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	59.0	51.250	59.86
71	181	1354	24.750	7.58	8.11	8.89	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.0	51.125	61.85
72	181	1357	23.750	7.68	8.20	8.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	63.0	51.000	63.85
73	181	1400	22.750	7.77	8.32	9.17	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	65.0	50.875	65.85
74	181	1403	21.750	7.83	8.38	9.26	-0.00	-0.01	-0.01	-0.01	-0.01	-0.01	-0.01	-0.01	-0.01	67.0	50.750	67.85
75	181	1406	20.750	7.92	8.48	9.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	69.0	50.625	69.84
76	181	1411	19.750	8.06	8.60	9.46	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	71.0	50.500	71.84
77	181	1416	18.750	8.19	8.74	9.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	73.0	50.375	73.84
78	181	1421	17.750	8.32	8.86	9.72	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	75.0	50.250	75.84
79	181	1426	16.750	8.42	8.98	9.81	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	77.0	50.125	77.83
80	181	1436	15.750	8.63	9.18	10.10	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	79.0	50.000	79.83
81	181	1446	14.750	8.80	9.36	10.23	-0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	81.0	49.875	81.82
82	181	1456	13.750	8.98	9.55	10.45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	83.0	49.750	83.82
83	181	1506	12.750	9.14	9.70	10.61	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	85.0	49.625	85.82
84	181	1516	11.750	9.29	9.86	10.78	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	87.0	49.500	87.82
85	181	1526	10.750	9.43	9.99	10.95	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	89.0	49.375	89.82
86	181	1536	9.750	9.55	10.11	11.06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	91.0	49.250	91.82
87	181	1546	8.750	9.68	10.24	11.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	93.0	49.125	93.82
88	181	1556	7.750	9.79	10.37	11.33	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	95.0	49.000	95.82
89	181	1606	6.750	9.91	10.48	11.43	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	97.0	48.875	97.81
90	181	1626	5.750	10.11	10.68	11.59	0.01	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	99.0	48.750	99.81
91	181	1646	4.750	10.29	10.85	11.80	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	101.0	48.625	101.81
92	181	1706	3.750	10.47	11.04	11.94	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	103.0	48.500	103.81
93	181	1726	2.750	10.64	11.21	12.17	0.00	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	105.0	48.375	105.81
94	181	1746	1.750	10.80	11.38	12.30	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	107.0	48.250	107.81
95	181	1806	0.750	10.93	11.50	12.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	109.0	48.125	109.80
96	181	1826	59.750	11.05	11.62	12.59	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	111.0	48.000	111.80
97	181	1845	58.750	11.16	11.71	12.67	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	113.0	47.875	113.80
98	181	1900	57.750	11.26	11.83	12.77	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	115.0	47.750	115.80
99	181	1925	56.750	11.30	11.94	12.95	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	117.0	47.625	117.80
100	181	1975	55.750	11.52	12.10	13.05	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	119.0	47.500	119.80
101	181	2025	54.750	11.65	12.22	13.17	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	121.0	47.375	121.80
102	181	2055	53.750	11.76	12.33	13.28	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	123.0	47.250	123.80
103	181	2125	52.750	11.91	12.48	13.47	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	125.0	47.125	125.80
104	181	2185	51.750	12.02	12.60	13.58	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	127.0	47.000	127.80
105	181	2225	50.750	12.13	12.69	13.67	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	129.0	46.875	129.80
106	181	2255	49.750	12.23	12.81	13.73	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	131.0	46.750	131.80
107	181	2275	48.750	12.32	12.91	13.86	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	133.0	46.625	133.80
108	181	2355	47.750	12.41	13.98	13.94	0.01	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	135.0	46.500	135.80
109	182	2355	46.750	12.51	13.08	14.07	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	137.0	46.375	137.80
110	182	2355	45.750	12.67	13.25	14.16	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	139.0	46.250	139.80
111	182	2355	44.750	12.82	13.40	14.34	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	141.0	46.125	141.80
112	182	3225	43.750	12.94	13.50	14.47	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	143.0	46.000	143.80
113	182	405	42.750	13.05	13.61	14.56	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	145.0	45.875	145.80
114	182	525	41.750	13.16	13.72	14.69	0.02	0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	-0.00	147.0	45.750	147.80

TABLE 2.7-6

115	181	625	40.750	13.27	13.02	14.81	0.02	-0.01	-0.07	0.09	12.0	39.0	42.625	1479.75
116	182	725	38.750	13.39	13.94	14.93	0.03	-0.01	-0.07	0.09	11.0	39.0	42.500	1119.75
117	182	825	38.750	13.49	14.06	15.00	0.03	-0.01	-0.07	0.09	19.0	39.0	42.375	1179.75
118	182	925	37.750	13.60	14.16	15.12	0.03	-0.01	-0.06	0.07	20.0	39.0	42.250	1239.75
119	182	1025	36.750	13.66	14.21	15.16	0.02	-0.02	-0.07	0.07	21.0	39.0	42.125	1299.75
120	182	1125	35.750	13.76	14.31	15.30	0.03	-0.01	-0.07	0.07	22.0	39.0	42.000	1359.75
121	182	1225	34.750	13.85	14.41	15.39	0.03	-0.01	-0.07	0.08	23.0	39.0	41.875	1419.75
122	182	1325	33.750	13.93	14.50	15.47	0.03	-0.02	-0.07	0.09	24.0	39.0	41.750	1479.75
123	182	1425	32.750	14.00	14.56	15.51	0.03	-0.02	-0.06	0.10	25.0	39.0	41.625	1539.74
124	182	1525	31.750	14.09	14.64	15.61	0.03	-0.02	-0.06	0.09	26.0	39.0	41.500	1599.74
125	182	1625	30.750	14.16	14.71	15.70	0.03	-0.01	-0.05	0.08	27.0	39.0	41.375	1659.74
126	182	1705	29.750	14.22	14.78	15.74	0.02	-0.02	-0.05	0.08	28.0	19.0	41.250	1719.74
127	182	2025	28.750	14.28	14.94	15.87	0.03	-0.02	-0.07	0.07	31.0	39.0	41.875	1899.75
128	182	2345	27.750	14.39	15.15	16.13	0.03	-0.03	-0.07	0.05	34.0	59.0	42.500	2099.76
129	183	305	26.750	14.72	15.28	16.22	0.04	-0.03	-0.06	0.05	38.0	19.0	42.125	2299.77
130	183	625	25.750	14.82	15.39	16.43	0.04	-0.03	-0.05	0.02	41.0	39.0	42.750	2499.78
131	183	945	24.750	15.11	15.64	16.57	0.04	-0.03	-0.05	0.02	44.0	59.0	47.375	2699.79
132	183	1305	23.750	15.40	15.91	16.85	0.05	-0.03	-0.04	0.02	48.0	19.0	48.000	2899.80
133	183	1625	22.750	15.52	16.02	17.02	0.05	-0.04	-0.06	0.09	51.0	39.0	48.625	3099.81
134	183	1945	21.750	15.61	16.14	17.11	0.05	-0.04	-0.06	0.07	54.0	59.0	49.250	3299.82
135	183	2305	20.750	15.66	16.19	17.13	0.06	-0.04	-0.05	0.03	58.0	19.0	49.875	3499.83
136	184	225	19.750	15.77	16.28	17.19	0.06	-0.05	-0.05	0.03	61.0	39.0	50.500	3699.84
137	184	545	18.750	15.88	16.38	17.31	0.05	-0.06	-0.04	0.03	64.0	59.0	51.125	3899.85
138	184	905	17.750	15.92	16.43	17.39	0.06	-0.05	-0.06	0.08	68.0	19.0	51.750	4099.86
139	184	1225	16.750	16.01	16.51	17.48	0.06	-0.05	-0.06	0.10	71.0	39.0	52.375	4299.87

## LEGEND:

DATE: Julian calendar day of the year.

HH:MM Hours and minutes.

SEC. Seconds

Drawdown units for all wells are feet from the initial water level.

Barometer units are feet of water changes from initial pressure at start of test.

TABLE 2.7-7

## RECOVERY DATA

ELAPSED TIME																TOTAL MIN. 1/1															
DATE	TIME	SLL	LDN-1	LDN-2	LDN-3	LOF-1	BMW-1	BAROM	HOURS	MIN.	SEC	ELAPSED TIME				TOTAL MIN. 1/1															
1	184	1246	48.750	16.01	16.50	17.44	0.06	-0.06	-0.07	0.11	0.0	0.0	0.000	0.00	0.00	0.00	0.00	0.00	0.00												
2	184	1246	57.750	16.00	16.44	17.44	0.06	-0.05	-0.07	0.11	0.0	0.0	9.875	0.16	0.0	0.0	0.0	0.0	0.0												
3	184	1247	6.750	15.98	16.32	17.42	0.06	-0.06	-0.07	0.11	0.0	0.0	19.750	0.33	0.0	0.0	0.0	0.0	0.0												
4	184	1247	15.750	15.93	16.11	17.37	0.06	-0.05	-0.07	0.11	0.0	0.0	29.625	0.49	0.0	0.0	0.0	0.0	0.0												
5	184	1247	24.750	15.85	15.88	17.24	0.06	-0.05	-0.07	0.11	0.0	0.0	39.500	0.66	0.0	0.0	0.0	0.0	0.0												
6	184	1247	33.750	15.75	15.66	17.22	0.06	-0.05	-0.07	0.11	0.0	0.0	49.375	0.82	0.0	0.0	0.0	0.0	0.0												
7	184	1247	42.750	15.62	15.43	17.14	0.06	-0.05	-0.07	0.11	0.0	0.0	59.250	0.99	0.0	0.0	0.0	0.0	0.0												
8	184	1247	51.750	15.46	15.15	17.03	0.06	-0.05	-0.07	0.11	0.0	0.0	69.125	1.18	0.0	0.0	0.0	0.0	0.0												
9	184	1248	4.750	15.28	14.92	16.86	0.06	-0.05	-0.07	0.11	0.0	0.0	79.000	1.38	0.0	0.0	0.0	0.0	0.0												
10	184	1248	15.750	15.11	14.73	16.71	0.06	-0.05	-0.07	0.11	0.0	0.0	88.875	1.58	0.0	0.0	0.0	0.0	0.0												
11	184	1248	26.750	14.95	14.54	16.60	0.06	-0.05	-0.08	0.11	0.0	0.0	98.750	1.78	0.0	0.0	0.0	0.0	0.0												
12	184	1248	37.750	14.79	14.40	16.44	0.06	-0.04	-0.07	0.11	0.0	0.0	108.625	1.98	0.0	0.0	0.0	0.0	0.0												
13	184	1248	48.750	14.64	14.25	16.29	0.06	-0.05	-0.08	0.11	0.0	0.0	118.500	2.17	0.0	0.0	0.0	0.0	0.0												
14	184	1248	59.750	14.50	14.13	16.18	0.06	-0.05	-0.07	0.11	0.0	0.0	128.375	2.37	0.0	0.0	0.0	0.0	0.0												
15	184	1249	10.750	14.37	14.03	16.01	0.06	-0.05	-0.08	0.11	0.0	0.0	138.250	2.57	0.0	0.0	0.0	0.0	0.0												
16	184	1249	21.750	14.25	13.92	15.80	0.06	-0.06	-0.08	0.11	0.0	0.0	148.125	2.77	0.0	0.0	0.0	0.0	0.0												
17	184	1249	32.750	14.14	13.83	15.61	0.06	-0.04	-0.07	0.11	0.0	0.0	158.000	2.97	0.0	0.0	0.0	0.0	0.0												
18	184	1249	43.750	14.00	13.71	15.41	0.06	-0.05	-0.08	0.11	0.0	0.0	167.875	3.21	0.0	0.0	0.0	0.0	0.0												
19	184	1250	54.750	13.88	13.60	15.26	0.06	-0.05	-0.07	0.11	0.0	0.0	177.750	3.46	0.0	0.0	0.0	0.0	0.0												
20	184	1250	6.750	13.76	13.50	15.07	0.06	-0.05	-0.07	0.11	0.0	0.0	187.625	3.71	0.0	0.0	0.0	0.0	0.0												
21	184	1250	17.750	13.63	13.41	14.84	0.06	-0.05	-0.07	0.11	0.0	0.0	197.500	3.96	0.0	0.0	0.0	0.0	0.0												
22	184	1250	28.750	13.53	13.32	14.74	0.06	-0.05	-0.08	0.11	0.0	0.0	207.375	4.21	0.0	0.0	0.0	0.0	0.0												
23	184	1250	39.750	13.45	13.24	14.55	0.06	-0.05	-0.08	0.11	0.0	0.0	217.250	4.45	0.0	0.0	0.0	0.0	0.0												
24	184	1251	50.750	13.36	13.15	14.33	0.06	-0.05	-0.07	0.11	0.0	0.0	227.125	4.70	0.0	0.0	0.0	0.0	0.0												
25	184	1251	6.750	13.27	13.07	14.25	0.06	-0.05	-0.07	0.11	0.0	0.0	237.000	4.95	0.0	0.0	0.0	0.0	0.0												
26	184	1251	17.750	13.15	12.98	13.99	0.06	-0.05	-0.06	0.11	0.0	0.0	246.875	5.20	0.0	0.0	0.0	0.0	0.0												
27	184	1252	28.750	13.05	12.89	13.82	0.06	-0.06	-0.07	0.11	0.0	0.0	256.750	5.41	0.0	0.0	0.0	0.0	0.0												
28	184	1252	39.750	12.94	12.80	13.65	0.06	-0.05	-0.07	0.11	0.0	0.0	266.625	5.64	0.0	0.0	0.0	0.0	0.0												
29	184	1252	50.750	12.85	12.71	13.55	0.06	-0.04	-0.07	0.11	0.0	0.0	276.500	5.87	0.0	0.0	0.0	0.0	0.0												
30	184	1252	6.750	12.76	12.61	13.34	0.06	-0.05	-0.07	0.11	0.0	0.0	286.375	6.11	0.0	0.0	0.0	0.0	0.0												
31	184	1253	17.750	12.67	12.54	13.19	0.06	-0.05	-0.07	0.11	0.0	0.0	296.250	6.34	0.0	0.0	0.0	0.0	0.0												
32	184	1253	28.750	12.59	12.46	13.11	0.06	-0.05	-0.07	0.11	0.0	0.0	306.125	6.57	0.0	0.0	0.0	0.0	0.0												
33	184	1253	39.750	12.51	12.40	12.96	0.06	-0.05	-0.07	0.11	0.0	0.0	316.000	6.80	0.0	0.0	0.0	0.0	0.0												
34	184	1254	50.750	12.44	12.32	12.84	0.06	-0.05	-0.07	0.11	0.0	0.0	325.875	7.03	0.0	0.0	0.0	0.0	0.0												
35	184	1254	6.750	12.37	12.26	12.81	0.06	-0.05	-0.07	0.11	0.0	0.0	335.750	7.26	0.0	0.0	0.0	0.0	0.0												
36	184	1254	17.750	12.31	12.11	12.67	0.06	-0.05	-0.07	0.11	0.0	0.0	345.625	7.49	0.0	0.0	0.0	0.0	0.0												
37	184	1255	28.750	12.23	12.13	12.56	0.06	-0.05	-0.07	0.11	0.0	0.0	355.500	7.72	0.0	0.0	0.0	0.0	0.0												
38	184	1255	39.750	12.18	12.07	12.46	0.06	-0.05	-0.07	0.11	0.0	0.0	365.375	7.95	0.0	0.0	0.0	0.0	0.0												
39	184	1255	50.750	12.11	11.99	12.44	0.06	-0.05	-0.07	0.11	0.0	0.0	375.250	8.18	0.0	0.0	0.0	0.0	0.0												
40	184	1256	6.750	12.05	11.94	12.37	0.06	-0.05	-0.07	0.11	0.0	0.0	385.125	8.41	0.0	0.0	0.0	0.0	0.0												
41	184	1257	17.750	11.98	11.77	12.09	0.06	-0.05	-0.07	0.11	0.0	0.0	395.000	8.64	0.0	0.0	0.0	0.0	0.0												
42	184	1258	28.750	11.73	11.62	11.91	0.06	-0.06	-0.07	0.11	0.0	0.0	404.875	8.87	0.0	0.0	0.0	0.0	0.0												
43	184	1259	39.750	11.58	11.50	11.73	0.06	-0.05	-0.07	0.11	0.0	0.0	414.750	9.10	0.0	0.0	0.0	0.0	0.0												
44	184	1300	50.750	11.45	11.36	11.62	0.06	-0.06	-0.07	0.11	0.0	0.0	424.625	9.33	0.0	0.0	0.0	0.0	0.0												
45	184	1301	6.750	11.32	11.26	11.46	0.06	-0.05	-0.07	0.11	0.0	0.0	434.500	9.56	0.0	0.0	0.0	0.0	0.0												
46	184	1302	17.750	11.21	11.12	11.27	0.06	-0.05	-0.07	0.11	0.0	0.0	444.375	9.79	0.0	0.0	0.0	0.0	0.0												
47	184	1303	28.750	11.10	11.02	11.21	0.06	-0.05	-0.07	0.11	0.0	0.0	454.250	10.02	0.0	0.0	0.0	0.0	0.0												
48	184	1304	39.750	11.00	10.92	11.12	0.06	-0.05	-0.07	0.11	0.0	0.0	464.125	10.25	0.0	0.0	0.0	0.0	0.0												
49	184	1305	50.750	10.91	10.82	11.01	0.06	-0.05	-0.07	0.11	0.0	0.0	474.000	10.48	0.0	0.0	0.0	0.0	0.0												
50	184	1305	6.750	10.82	10.73	10.88	0.06	-0.04	-0.07	0.11	0.0	0.0	483.875	10.71	0.0	0.0	0.0	0.0	0.0												
51	184	1307	17.750	10.64	10.56	10.71	0.06	-0.05	-0.07	0.11	0.0	0.0	493.750	10.94	0.0	0.0	0.0	0.0	0.0												
52	184	1309	28.750	10.48	10.41	10.50	0.06	-0.05	-0.07	0.11	0.0	0.0	503.625	11.17	0.0	0.0	0.0	0.0	0.0												

TABLE 2.7-7

53	184	1311	56.750	10.23	10.29	10.23	0.06	-0.05	-0.07	0.11	0.0	25.0	53.500	25.89	167.90
54	184	1312	56.750	10.19	10.13	10.23	0.06	-0.05	-0.07	0.11	0.0	27.0	53.375	27.89	156.94
55	184	1313	56.750	10.06	10.00	10.06	0.06	-0.05	-0.07	0.11	0.0	29.0	53.250	29.89	145.50
56	184	1314	56.750	9.92	9.86	9.92	0.06	-0.05	-0.07	0.11	0.0	31.0	53.125	31.88	134.52
57	184	1315	56.750	9.82	9.76	9.82	0.06	-0.05	-0.07	0.11	0.0	33.0	53.000	33.88	128.54
58	184	1321	51.750	9.70	9.64	9.70	0.06	-0.05	-0.07	0.12	0.0	35.0	52.875	35.88	121.44
59	184	1325	50.750	9.60	9.53	9.58	0.06	-0.06	-0.07	0.12	0.0	37.0	52.750	37.88	115.08
60	184	1329	49.750	9.50	9.42	9.51	0.06	-0.06	-0.07	0.12	0.0	39.0	52.625	39.88	109.37
61	184	1327	48.750	9.41	9.34	9.36	0.06	-0.05	-0.07	0.12	0.0	41.0	52.500	41.88	104.26
62	184	1329	47.750	9.32	9.24	9.30	0.06	-0.05	-0.07	0.12	0.0	43.0	52.375	43.87	99.20
63	184	1331	46.750	9.23	9.15	9.21	0.06	-0.05	-0.07	0.13	0.0	45.0	52.250	45.87	95.21
64	184	1333	45.750	9.15	9.07	9.10	0.06	-0.05	-0.07	0.13	0.0	47.0	52.125	47.87	91.28
65	184	1335	44.750	9.06	8.98	8.99	0.06	-0.06	-0.07	0.13	0.0	49.0	52.000	49.87	87.66
66	184	1341	42.625	8.84	8.73	8.77	0.06	-0.05	-0.07	0.13	0.0	51.0	51.875	51.95	82.61
67	184	1344	43.625	8.74	8.65	8.67	0.06	-0.05	-0.07	0.13	0.0	53.0	51.750	53.95	78.41
68	184	1347	44.625	8.64	8.54	8.59	0.06	-0.05	-0.07	0.13	0.0	55.0	51.625	55.83	74.48
69	184	1350	46.625	8.56	8.46	8.46	0.06	-0.05	-0.07	0.13	0.0	57.0	51.500	57.81	70.93
70	184	1353	48.625	8.47	8.36	8.35	0.06	-0.05	-0.07	0.13	0.0	59.0	51.375	59.79	67.70
71	184	1356	50.625	8.38	8.27	8.30	0.06	-0.05	-0.07	0.13	0.0	61.0	51.250	61.79	64.79
72	184	1358	52.625	8.29	8.19	8.23	0.06	-0.05	-0.07	0.13	0.0	63.0	51.125	63.78	62.06
73	184	1404	58.625	8.13	8.02	8.09	0.06	-0.06	-0.07	0.13	0.0	65.0	51.000	65.77	59.29
74	184	1408	60.625	8.03	7.91	7.99	0.06	-0.06	-0.07	0.13	0.0	67.0	50.875	67.76	56.51
75	184	1412	62.625	7.93	7.83	7.86	0.06	-0.05	-0.07	0.13	0.0	69.0	50.750	69.76	53.98
76	184	1418	64.625	7.80	7.68	7.67	0.06	-0.05	-0.07	0.13	0.0	71.0	50.625	71.74	51.98
77	184	1422	66.625	7.70	7.57	7.60	0.06	-0.05	-0.07	0.13	0.0	73.0	50.500	73.72	49.61
78	184	1428	68.625	7.58	7.45	7.46	0.06	-0.05	-0.07	0.13	0.0	75.0	50.375	75.72	47.46
79	184	1437	70.625	7.40	7.26	7.31	0.06	-0.05	-0.07	0.13	0.0	77.0	50.250	77.72	45.38
80	184	1446	72.625	7.24	7.11	7.10	0.06	-0.05	-0.07	0.13	0.0	79.0	50.125	79.72	43.86
81	184	1458	74.625	7.04	6.90	6.91	0.06	-0.05	-0.07	0.13	0.0	81.0	50.000	81.71	42.71
82	184	1508	81.625	6.89	6.75	6.74	0.06	-0.05	-0.07	0.13	0.0	83.0	49.875	83.71	41.66
83	184	1518	83.625	6.75	6.61	6.62	0.06	-0.06	-0.07	0.13	0.0	85.0	49.750	85.71	40.71
84	184	1528	85.625	6.63	6.48	6.44	0.06	-0.05	-0.07	0.13	0.0	87.0	49.625	87.71	39.87
85	184	1538	87.625	6.50	6.37	6.34	0.06	-0.05	-0.07	0.13	0.0	89.0	49.500	89.71	39.11
86	184	1548	89.625	6.37	6.25	6.21	0.06	-0.05	-0.07	0.13	0.0	91.0	49.375	91.71	38.41
87	184	1558	91.625	6.25	6.11	6.09	0.06	-0.05	-0.07	0.13	0.0	93.0	49.250	93.71	37.76
88	184	1608	94.625	6.15	6.03	5.95	0.06	-0.05	-0.07	0.13	0.0	95.0	49.125	95.71	37.16
89	184	1628	96.625	6.05	5.82	5.77	0.06	-0.05	-0.07	0.13	0.0	97.0	49.000	97.71	36.61
90	184	1648	98.625	5.95	5.66	5.61	0.06	-0.05	-0.07	0.13	0.0	99.0	48.875	99.71	36.11
91	184	1718	101.625	5.85	5.41	5.35	0.06	-0.05	-0.07	0.13	0.0	101.0	48.750	101.71	35.66
92	184	1738	103.625	5.75	5.42	5.34	0.06	-0.05	-0.07	0.13	0.0	103.0	48.625	103.71	35.26
93	184	1758	105.625	5.65	5.42	5.34	0.06	-0.05	-0.07	0.13	0.0	105.0	48.500	105.71	34.91
94	184	1818	108.625	5.55	5.30	5.28	0.06	-0.05	-0.07	0.13	0.0	107.0	48.375	107.71	34.61
95	184	1838	110.625	5.45	5.15	5.08	0.06	-0.05	-0.07	0.13	0.0	109.0	48.250	109.71	34.36
96	184	1858	112.625	5.35	5.03	4.98	0.06	-0.05	-0.07	0.13	0.0	111.0	48.125	111.71	34.11
97	184	1928	115.625	5.25	4.94	4.85	0.06	-0.05	-0.07	0.13	0.0	113.0	48.000	113.71	33.91
98	184	1958	117.625	5.15	4.74	4.69	0.06	-0.05	-0.07	0.13	0.0	115.0	47.875	115.71	33.76
99	184	2018	119.625	5.05	4.68	4.58	0.06	-0.05	-0.07	0.13	0.0	117.0	47.750	117.71	33.61
100	184	2048	121.625	4.95	4.54	4.45	0.06	-0.05	-0.07	0.13	0.0	119.0	47.625	119.71	33.46
101	184	2118	123.625	4.85	4.39	4.39	0.06	-0.05	-0.07	0.13	0.0	121.0	47.500	121.71	33.31
102	184	2168	125.625	4.75	4.29	4.27	0.06	-0.05	-0.07	0.13	0.0	123.0	47.375	123.71	33.16
103	184	2218	127.625	4.65	4.16	4.10	0.06	-0.05	-0.07	0.13	0.0	125.0	47.250	125.71	33.01
104	184	2268	129.625	4.55	4.05	4.03	0.06	-0.05	-0.07	0.13	0.0	127.0	47.125	127.71	32.86
105	184	2318	131.625	4.45	4.02	4.00	0.06	-0.05	-0.07	0.13	0.0	129.0	47.000	129.71	32.71
106	184	2368	133.625	4.35	3.91	3.89	0.06	-0.05	-0.07	0.13	0.0	131.0	46.875	131.71	32.56
107	184	2418	135.625	4.25	3.85	3.84	0.06	-0.05	-0.07	0.13	0.0	133.0	46.750	133.71	32.41
108	184	2468	137.625	4.15	3.78	3.74	0.06	-0.05	-0.07	0.13	0.0	135.0	46.625	135.71	32.26
109	184	2518	139.625	4.05	3.70	3.63	0.06	-0.05	-0.07	0.13	0.0	137.0	46.500	137.71	32.11
110	184	2568	141.625	3.95	3.63	3.58	0.06	-0.05	-0.07	0.13	0.0	139.0	46.375	139.71	31.96
111	184	2618	143.625	3.85	3.54	3.50	0.06	-0.05	-0.07	0.13	0.0	141.0	46.250	141.71	31.81
112	184	2668	145.625	3.75	3.49	3.40	0.06	-0.05	-0.07	0.13	0.0	143.0	46.125	143.71	31.66
113	184	2718	147.625	3.65	3.39	3.32	0.06	-0.05	-0.07	0.13	0.0	145.0	46.000	145.71	31.51
114	184	2768	149.625	3.55	3.29	3.22	0.06	-0.05	-0.07	0.13	0.0	147.0	45.875	147.71	31.36
115	184	2818	151.625	3.45	3.19	3.14	0.06	-0.05	-0.07	0.13	0.0	149.0	45.750	149.71	31.21
116	184	2868	153.625	3.35	3.10	3.10	0.06	-0.05	-0.07	0.13	0.0	151.0	45.625	151.71	31.06

TABLE 2.7-7

115	185	717	53.625	3.15	3.01	0.21	0.09	0.09	-0.03	18.0	30.0	39.225	1110.64	4.69
116	185	817	52.625	3.06	2.93	0.22	0.11	0.11	-0.03	18.0	30.0	38.250	1170.64	4.69
117	185	917	51.625	2.98	2.81	0.27	0.11	0.11	-0.02	20.0	30.0	38.125	1230.63	4.51
118	185	1017	50.625	2.91	2.76	0.21	0.09	0.09	0.00	21.0	30.0	38.000	1290.63	4.35
119	185	1117	49.625	2.85	2.73	0.21	0.10	0.10	0.00	22.0	30.0	37.875	1350.63	4.20
120	185	1217	48.625	2.79	2.63	0.21	0.10	0.10	0.01	23.0	30.0	37.750	1410.63	4.06
121	185	1317	47.625	2.74	2.59	0.22	0.10	0.11	0.02	24.0	30.0	37.625	1470.63	3.94
122	185	1417	46.625	2.69	2.53	0.23	0.11	0.10	0.04	25.0	30.0	37.500	1530.62	3.82
123	185	1517	45.625	2.65	2.52	0.23	0.12	0.11	0.05	26.0	30.0	37.375	1590.62	3.72
124	185	1617	44.625	2.60	2.47	0.21	0.10	0.10	0.07	27.0	30.0	37.250	1650.62	3.62
125	185	1717	43.625	2.59	2.45	0.21	0.11	0.10	0.08	28.0	30.0	37.125	1710.62	3.53
126	185	1817	42.625	2.57	2.44	0.21	0.10	0.10	0.08	29.0	10.0	37.000	1750.62	3.47
127	185	1917	41.625	2.43	2.24	0.18	0.07	0.06	0.03	32.0	30.0	37.625	1850.63	3.22
128	186	37	40.625	2.32	2.21	0.18	0.07	0.07	0.01	32.0	50.0	38.250	1950.64	3.01
129	186	357	39.625	2.24	2.13	0.18	0.07	0.08	0.01	39.0	10.0	38.875	2050.65	2.84
130	186	717	38.625	2.15	2.00	0.17	0.07	0.08	0.01	43.0	30.0	39.500	2150.66	2.69
131	186	1037	37.625	2.06	1.88	0.16	0.08	0.08	0.01	45.0	50.0	40.125	2250.67	2.57
132	186	1357	36.625	1.97	1.82	0.18	0.08	0.10	0.01	49.0	10.0	40.750	2350.69	2.46
133	186	1717	35.625	1.94	1.82	0.20	0.11	0.13	0.01	52.0	30.0	41.375	2450.69	2.37
134	186	2037	34.625	1.88	1.77	0.19	0.09	0.12	0.02	55.0	50.0	42.000	2550.70	2.29
135	186	2357	33.625	1.82	1.70	0.18	0.09	0.12	0.03	59.0	10.0	42.625	2650.71	2.22
136	187	317	32.625	1.79	1.66	0.21	0.12	0.14	0.04	62.0	30.0	43.250	2750.72	2.15
137	187	637	31.625	1.71	1.58	0.15	0.06	0.10	0.03	65.0	50.0	43.875	2850.73	2.09
138	187	957	30.625	1.62	1.48	0.08	-0.01	0.07	0.05	69.0	10.0	44.500	2950.74	2.04
139	187	1317	29.625	1.56	1.36	0.07	-0.02	0.05	0.06	72.0	30.0	45.125	3050.75	1.99

No equipment failures or interruptions occurred during the aquifer test. However, barometric pressure did vary considerably during the six-day test as the result of the passage of a low pressure system and a cold front with associated thunderstorms and subsequent high pressure.

## ANALYSIS OF DATA

### Analytical Methods

To accomplish the goals of this investigation, the following methods of analysis were used:

- o Theis' Non-Equilibrium Method (Theis, 1935) for analyzing non-equilibrium pumping test data.
- o Theis' Recovery Method (Theis, 1935) for analyzing recovery test data.
- o Jacob's Modified Non-Equilibrium Method (Cooper and Jacob, 1946) for analyzing non-equilibrium pumping test data.
- o Cooper and Jacob's Distance-Drawdown Method (Cooper and Jacob, 1946) for determining radius of influence.
- o Hantush's Method (Hantush, 1966) for determining the magnitude and direction of the major the minor horizontal axes of transmissivity in an anisotropic aquifer.
- o Neuman and Witherspoon's Method (Neuman and Witherspoon, 1972) for determining the hydraulic diffusivity and vertical hydraulic conductivity of confining layers.
- o Darcy's Law (Darcy, 1856) to determine the average pore velocity and the groundwater flux across the aquifer test site.

- o Standard Consolidation Test (ASTM 1985) to determine the coefficient of consolidation, compression index, coefficient of compressibility, and vertical hydraulic conductivity of the confining layer.

From a practical viewpoint, the field conditions at the test site met all the assumptions and conditions necessary for these analytical methods to be applicable and valid.

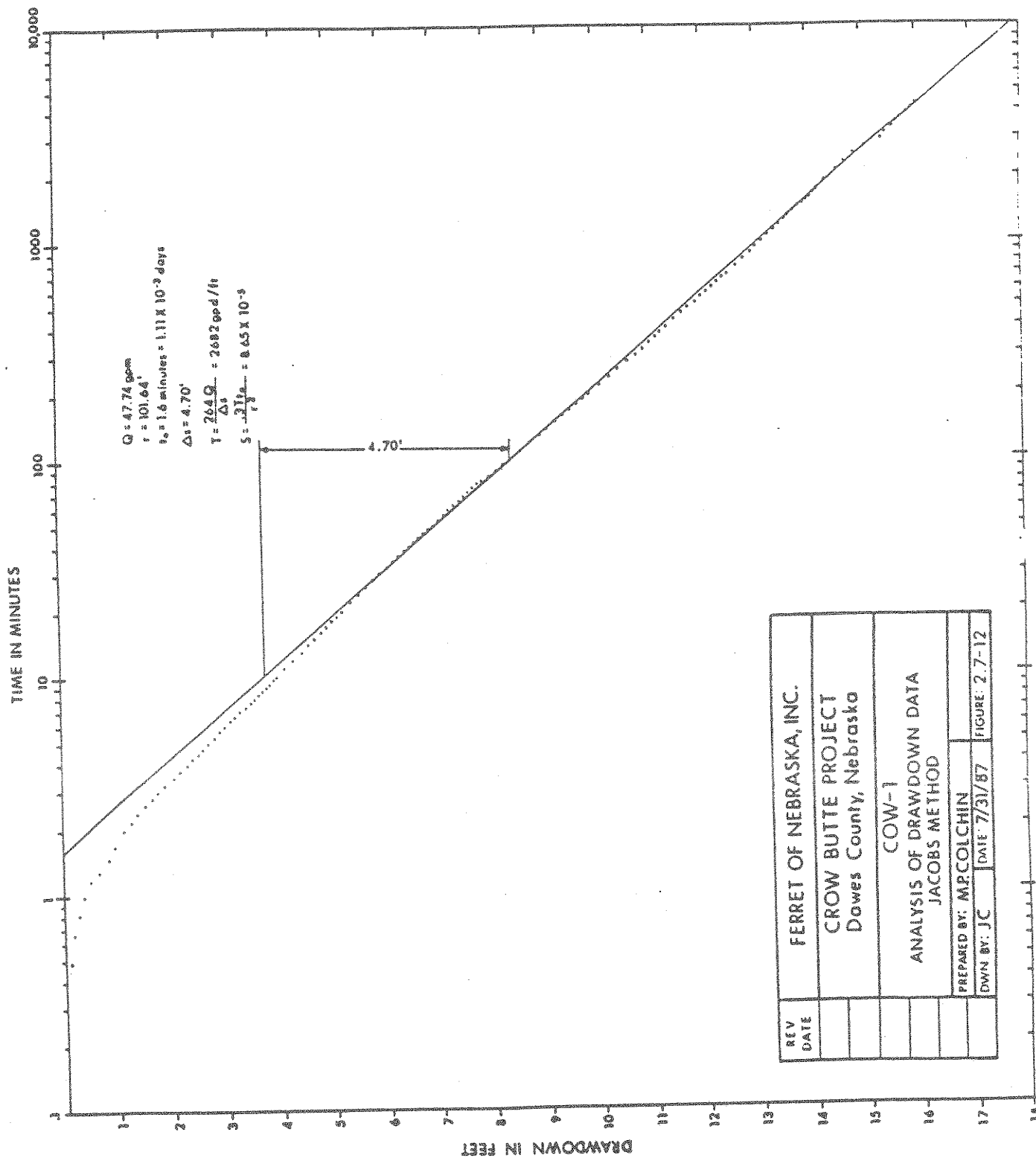
### Results of Analysis

#### Basal Chadron Sandstone

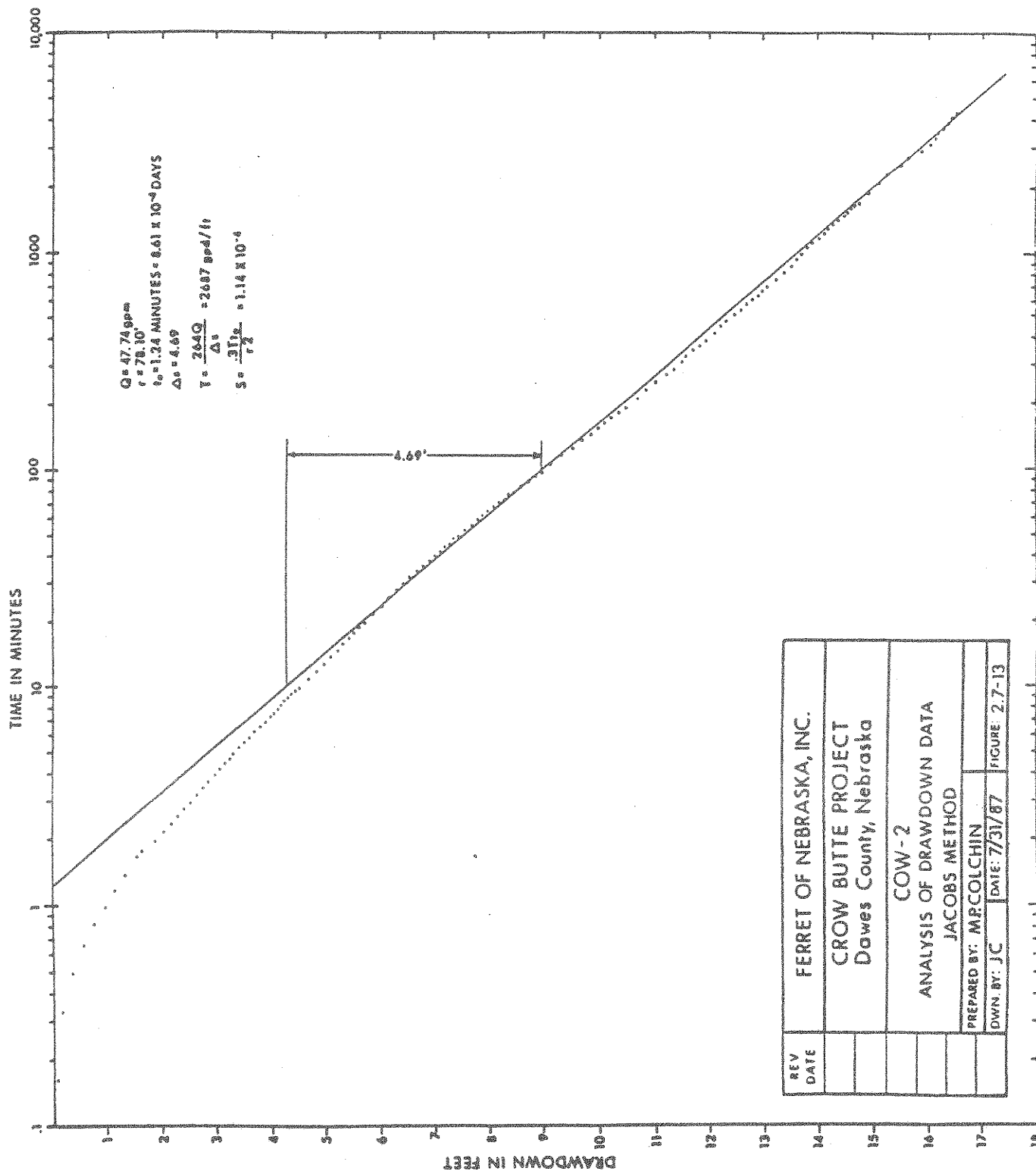
The Jacob Non-Equilibrium Method, the Theis Non-Equilibrium Method and the Theis Recovery Method were used to analyze the aquifer test data from the three Basal Chadron Sandstone wells (Figures 2.7-12 to 2.7-20). A confined non-leaky type of analysis was made because leakage effects were not apparent in the test data and the piezometric surface is well above the top of the aquifer. Inspection of the results of the analyses verifies that these assumptions are valid.

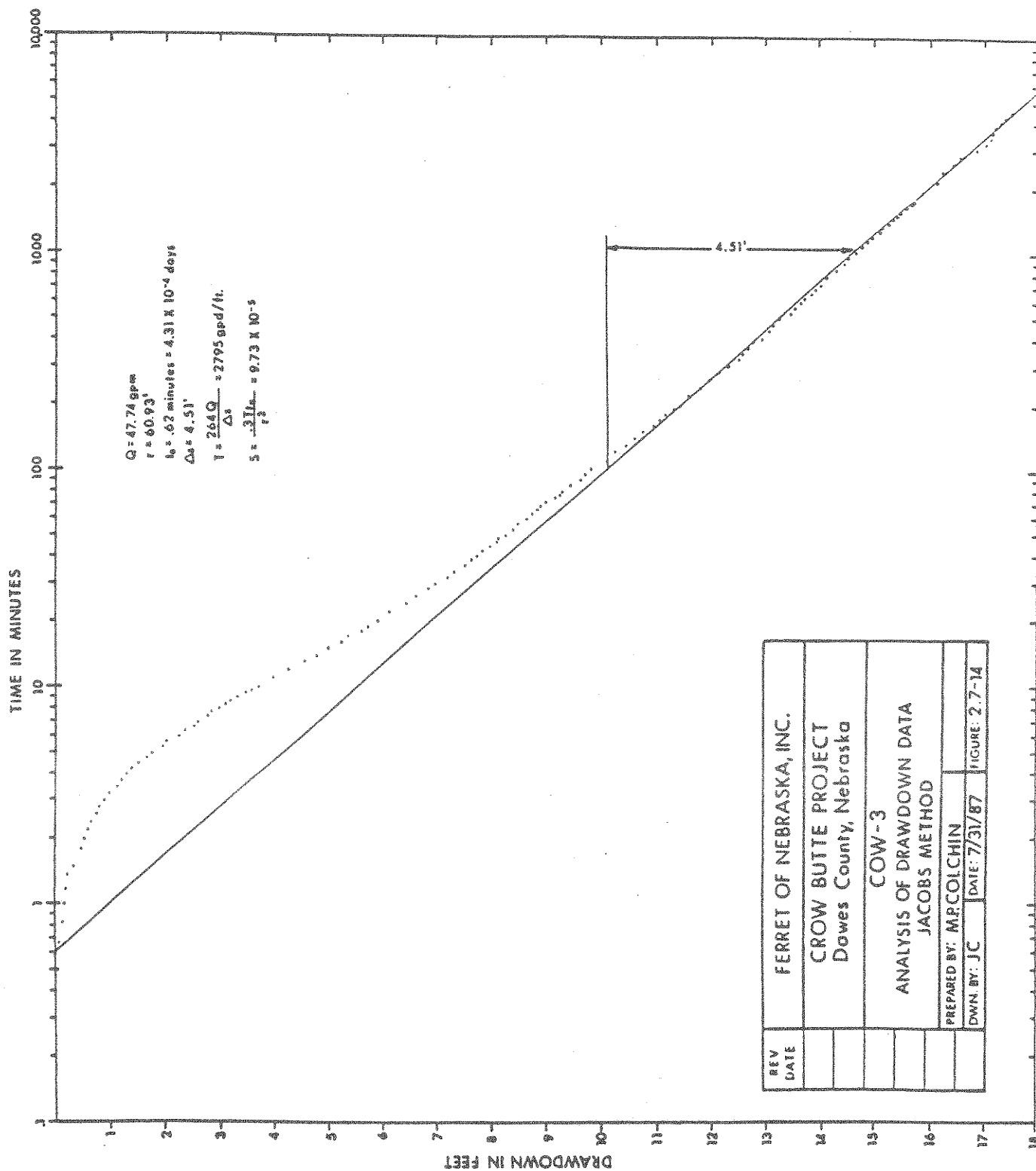
The transmissivities calculated from the drawdown data from the three Basal Chadron Sandstone observation wells (COW-1, COW-2, COW-3), ranged from 2682 gpd/ft (359 ft<sup>2</sup>/day) to 2795 gpd/ft (374 ft<sup>2</sup>/day). The storage coefficients for these wells, calculated from the same analyses, ranges from  $8.44 \times 10^{-3}$  to  $1.31 \times 10^{-4}$ . The transmissivities calculated from the recovery data from the three observation wells are slightly lower, ranging from 2604 gpd/ft (348 ft<sup>2</sup>/day) to 2659 gpd/ft (355 ft<sup>2</sup>/day). The lower transmissivity values calculated from the recovery data are probably the result of the variation in the storage coefficient during pumping and recovery. In theory, the storage coefficient is assumed to be constant during both the pumping and the recovery phases of an aquifer test. This assumption is true if the aquifer is perfectly elastic. In practice, however, a confined aquifer is usually not perfectly elastic. Therefore, it will not rebound vertically during recovery of water levels (recovery of pressure) at the same rate that it consolidates or compresses when pressure is decreased during the preceeding pumping. Therefore, the storage



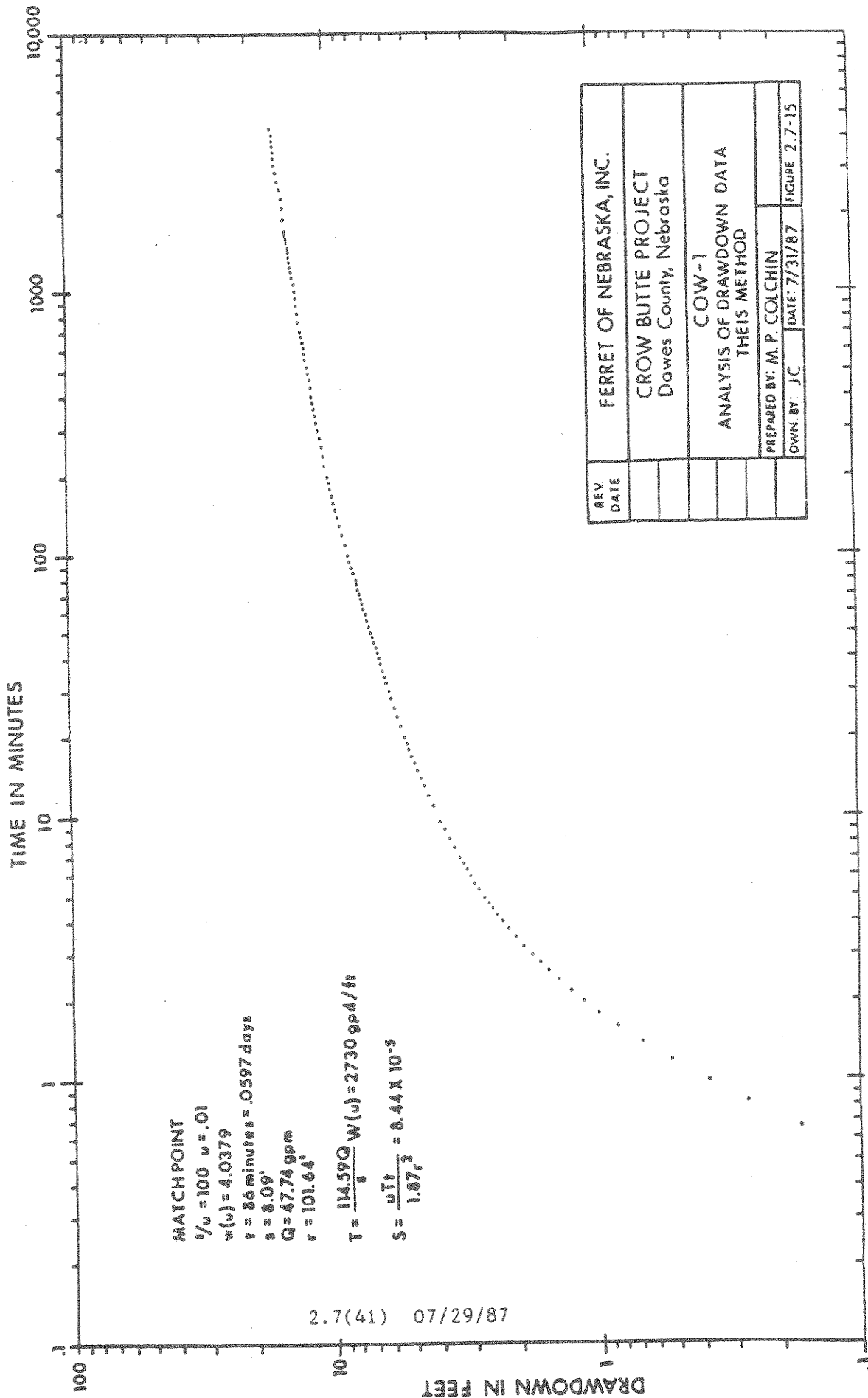


REV	FERRET OF NEBRASKA, INC.		
DATE			
	CROW BUTTE PROJECT		
	Dowes County, Nebraska		
	COW-1		
	ANALYSIS OF DRAWDOWN DATA		
	JACOBS METHOD		
	PREPARED BY: M.P. COLCHIN	DATE: 7/31/87	FIGURE: 2.7-12
	DWN BY: JC		





REV	FERRET OF NEBRASKA, INC.		
DATE			
	CROW BUTTE PROJECT		
	Dawes County, Nebraska		
	COW-3		
	ANALYSIS OF DRAWDOWN DATA		
	JACOBS METHOD		
	PREPARED BY: M.P. COLCHIN		
	DRAWN BY: JC	DATE: 7/31/87	FIGURE: 2.7-14



REV	FERRET OF NEBRASKA, INC.		
DATE			
	CROW BUTTE PROJECT		
	Dawes County, Nebraska		
	COW-1		
	ANALYSIS OF DRAWDOWN DATA		
	THEIS METHOD		
	PREPARED BY: M. P. COLCHIN	DATE: 7/31/87	FIGURE 2.7-15
	DWN BY: JC		

TIME IN MINUTES

10,000

1,000

100

10

1

MATCH POINT

$u/u_0 = 100$   $u = .01$

$W(u) = 4.0379$

$i = 67$  minutes  $= .0465$  days

$s = 8.08'$

$Q = 47.74$  gpm

$r = 78.10$

$T = \frac{114.59Q}{s} W(u) = 2733$  gpd/ft

$S = \frac{uTt}{1.87r^2} = 1.11 \times 10^{-4}$

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DRAWDOWN IN FEET

10

REV	FERRET OF NEBRASKA, INC.		
DATE			
	CROW BUTTE PROJECT		
	Dawes County, Nebraska		
	COW-2		
	ANALYSIS OF DRAWDOWN DATA		
	THEIS METHOD		
	PREPARED BY: M.P. COLCHIN	DATE: 7/31/87	FIGURE 2.7-16
	DWN BY: JC		

TIME IN MINUTE

10,000

1000

100

10

1

MATCH POINT

$$u_o = 100 \quad u = .01$$

$$W(u) = 4.0379$$

$$t = 48 \text{ minutes} = .0333 \text{ days}$$

$$s = 8.11'$$

$$Q = 47.74 \text{ gpm}$$

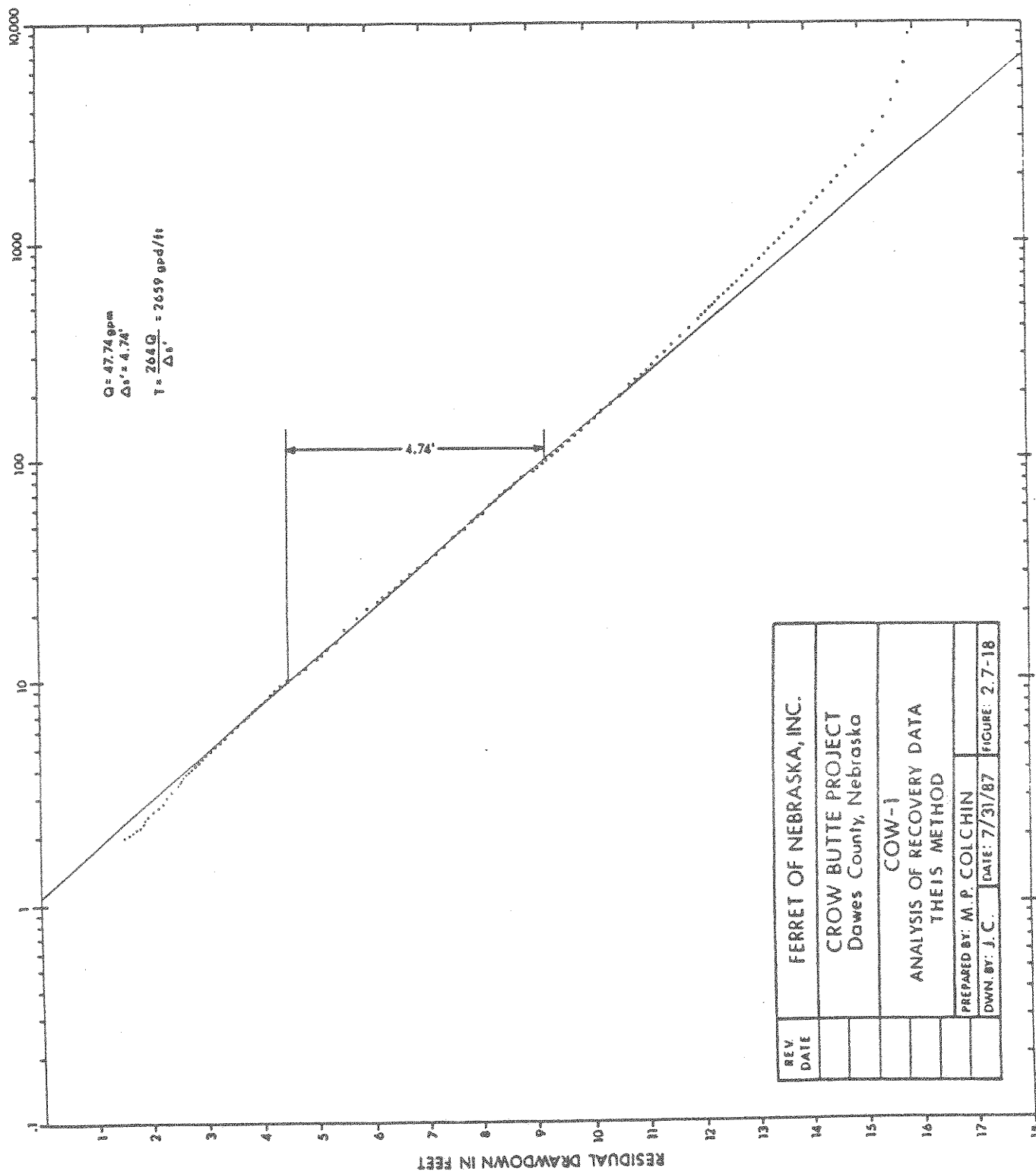
$$r = 60.93'$$

$$T = \frac{114.59Q}{s} \quad W(u) = 2724 \text{ gpd/ft}$$

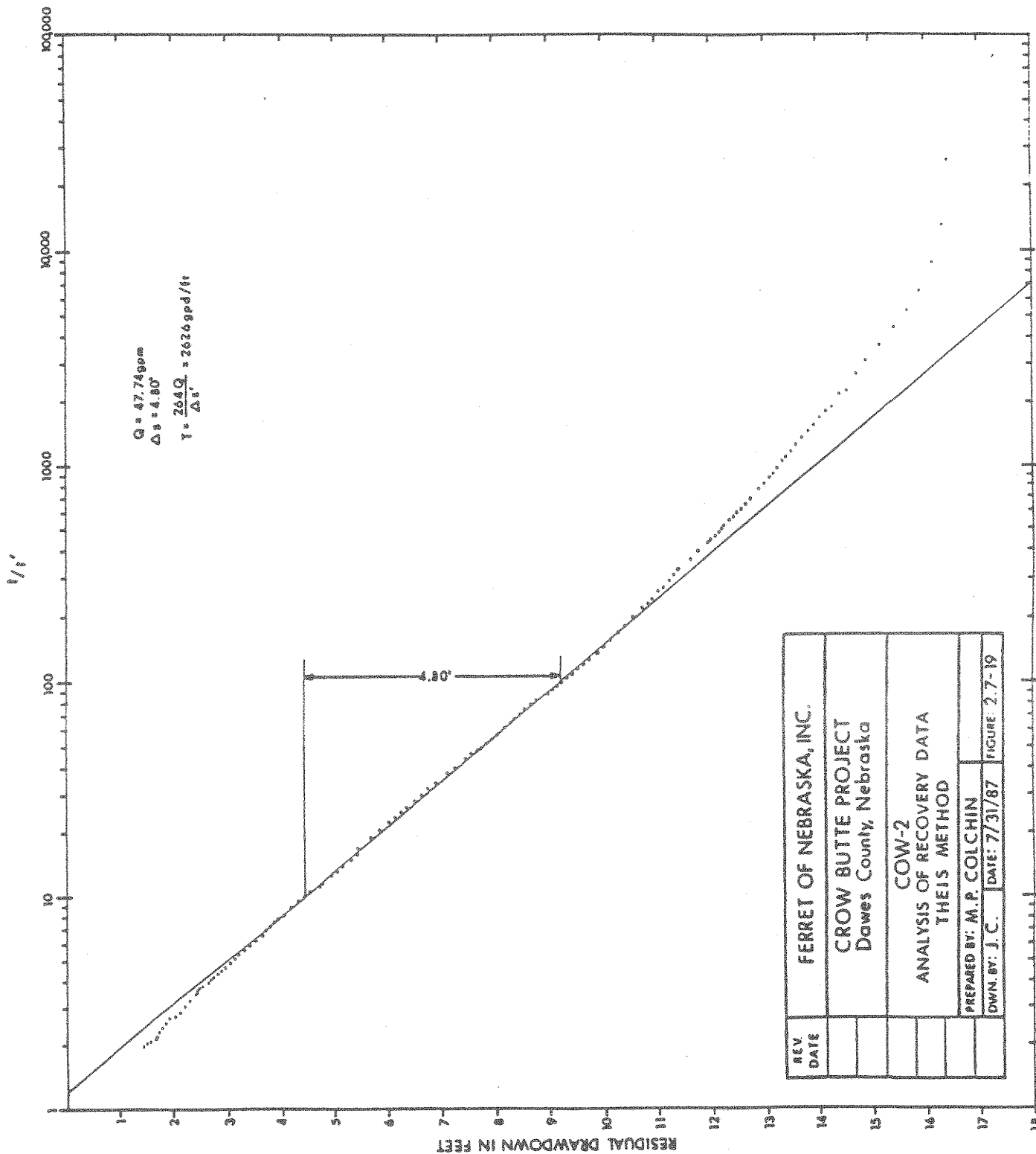
$$S = \frac{uTt}{1.87r^2} = 1.31 \times 10^{-4}$$

2.7(43) 07/29/87

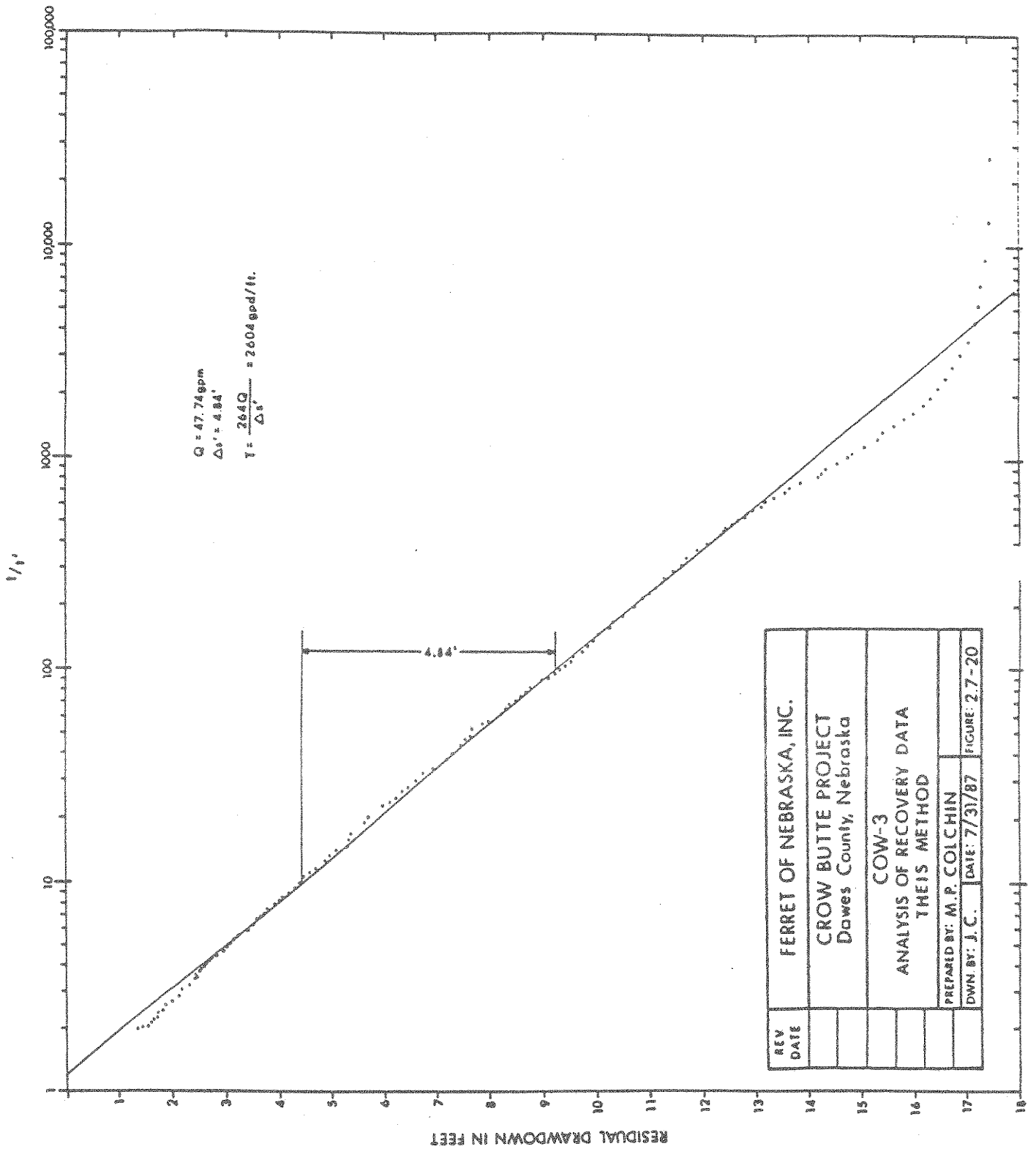
REV	FERRET OF NEBRASKA, INC.		
DATE			
	CROW BUTTE PROJECT		
	Dawes County, Nebraska		
	COW-3		
	ANALYSIS OF DRAWDOWN DATA		
	THEIS METHOD		
	PREPARED BY: M.P. COLCHIN		FIGURE 2.7-17
	DWN. BY: JC		
	DATE: 7/31/87		



REV DATE	FERRET OF NEBRASKA, INC.		
	CROW BUTTE PROJECT Dawes County, Nebraska		
	COW-1		
	ANALYSIS OF RECOVERY DATA THEIS METHOD		
	PREPARED BY: M. P. COLCHIN		FIGURE: 2.7-18
	DWN. BY: J. C.		
	DATE: 7/31/87		







coefficient will vary and is likely to be larger during pumping than during the subsequent recovery (Jacob, 1963). Thus, transmissivity values calculated from pumping data are commonly larger than those calculated from recovery data.

The average thickness of the aquifer at the test site is 40 feet. Therefore, the hydraulic conductivities calculated from the drawdown data ranges from about 67 gpd/ft<sup>2</sup> (8.96 ft/day) to about 70 gpd/ft<sup>2</sup> (9.34 ft/day). The hydraulic conductivities calculated from the recovery data ranged from about 65 gpd/ft<sup>2</sup> (8.7 ft/day) to about 66 gpd/ft<sup>2</sup> (8.89 ft/day). Table 2.7-8 summarizes the results of the analysis of the aquifer test data.

The Hantush Method For Anisotropic aquifers was used to determine the direction and magnitude of the major and minor axes of transmissivity of the Basal Chadron Sandstone. The major axis of transmissivity in the Basal Chadron Sandstone lies along an azimuth of about 51° and has a magnitude of 2760 gpd/ft (369 ft<sup>2</sup>/day) (Figure 2.7-8). The minor axis of transmissivity has an azimuth of about 141° and a magnitude of 2692 gpd/ft 360 ft<sup>2</sup>/day.

#### Overlying and Underlying Confining Layers

The overlying confining layer piezometer (UCP-1) showed no response to the pumping from the Basal Chadron Sandstone during the aquifer test (Figure 2.7-21). However, this piezometer did respond to the rapid changes in barometric pressure that accompanied the passage of a low pressure system and a cold front which confirmed that it was indeed functioning properly. because UCP-1 did not respond to pumping, it was not possible to use the water level data from UCP-1 to calculate the hydraulic properties of the upper confining layer using the Neuman-Witherspoon Method. Therefore, laboratory data from the consolidation tests of core samples from UCP-1 were used to calculate the hydraulic properties of the overlying confining layer.

TABLE 2.7-8

## SUMMARY OF AQUIFER-TEST DATA ANALYSIS

## Jacob Method (Drawdown)

<u>Well</u>	<u>T (gpd/ft)</u>	<u>T (ft<sup>3</sup>/day)</u>	<u>S</u>	<u>K (gpd/ft<sup>2</sup>)</u>	<u>K (ft/day)</u>
COW-1	2682	359	$8.65 \times 10^{-5}$	67	8.98
COW-2	2687	359	$1.14 \times 10^{-4}$	67	8.98
COW-3	2795	374	$9.73 \times 10^{-5}$	70	9.35
Average	2721	364	$9.93 \times 10^{-5}$	68	9.10

## Theis Method (Drawdown)

<u>Well</u>	<u>T (gpd/ft)</u>	<u>T (ft<sup>3</sup>/day)</u>	<u>S</u>	<u>K (gpd/ft<sup>2</sup>)</u>	<u>K (ft/day)</u>
COW-1	2730	365	$8.44 \times 10^{-5}$	68	9.13
COW-2	2733	365	$1.11 \times 10^{-4}$	68	9.13
COW-3	2724	364	$1.31 \times 10^{-4}$	68	9.10
Average	2729	365	$1.09 \times 10^{-4}$	68	9.12

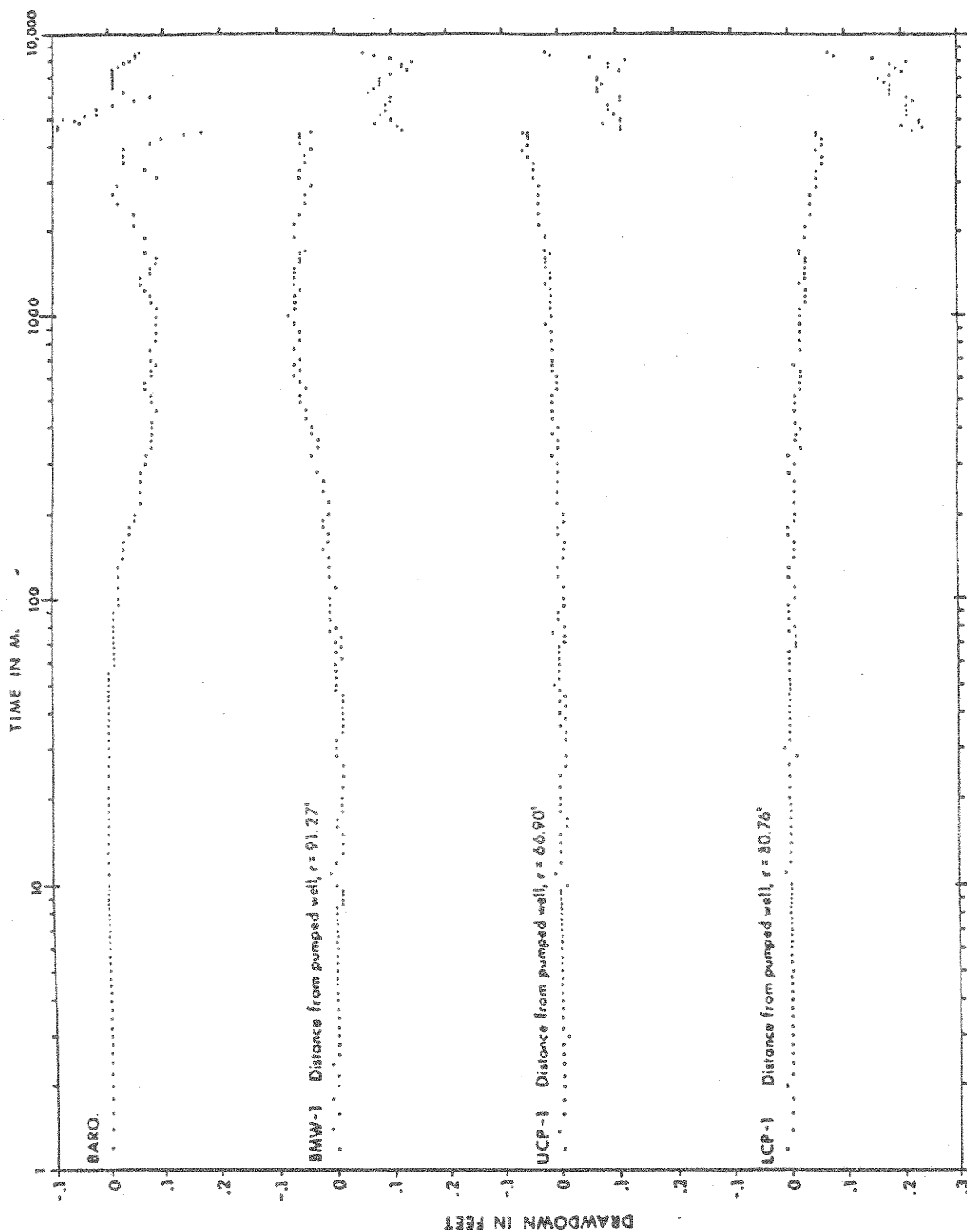
## Theis Recovery Method

<u>Well</u>	<u>T (gpd/ft)</u>	<u>T (ft<sup>3</sup>/day)</u>	<u>K (gpd/ft<sup>2</sup>)</u>	<u>K (ft/day)</u>
COW-1	2659	355	66	8.88
COW-2	2626	351	66	8.78
COW-3	2604	348	65	8.70
Average	2630	351	66	8.79

## Average of Jacob and Theis Methods (Drawdown) \*

<u>Well</u>	<u>T (gpd/ft)</u>	<u>T (ft<sup>3</sup>/day)</u>	<u>S</u>	<u>K (gpd/ft<sup>2</sup>)</u>	<u>K (ft/day)</u>
COW-1	2706	362	$8.55 \times 10^{-5}$	68	9.05
COW-2	2710	362	$1.13 \times 10^{-4}$	68	9.05
COW-3	2760	364	$1.14 \times 10^{-4}$	69	9.23
Average	2725	364	$1.04 \times 10^{-4}$	68	9.11

\* Used in anisotropy calculations.



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REV	FERRET OF NEBRASKA, INC.
DATE	
	CROW BUTTE PROJECT
	Dawes County, Nebraska
	RESPONSE OF BAROMETER, BMW-1,
	UCP-1, AND LCP-1
	DURING AQUIFER TEST
	PREPARED BY: M.P. COLCHIN
DRAWN BY: LC	DATE 8/5/87
	FIGURE 2-7-21

Results of the laboratory consolidation test data from three core samples of UCP-1 are shown earlier in Table 2.7-4. The calculated average coefficient of compressibility,  $a_v$ , of the red clay portion of the overlying confining layer, is  $3.99 \times 10^{-7} \text{ cm}^2/\text{g}$  and the calculated average vertical hydraulic conductivity is  $3.49 \times 10^{-11} \text{ cm/sec}$ . Using these consolidation test data, the calculated specific storage of the red clay portion of the overlying confining layer is  $3.08 \times 10^{-7} \text{ cm}^{-1}$  and the calculated hydraulic diffusivity is  $1.13 \times 10^{-4} \text{ cm}^2/\text{sec}$ . Analysis of drill cuttings and geophysical logs of UCP-1 and exploration holes in the vicinity of the test site show that the lithology of the strata between the Red Clay and the overlying Brule aquifer (Upper Chadron and Lower Brule Formations) is similar to the Red Clay. Therefore, it is reasonable to assume that the hydraulic characteristics of these strata are similar to those of the Red Clay. Given that the red clay is approximately 30 feet thick and the total overlying confining layer is approximately 325 feet thick, the hydraulic resistance,  $c$ , (Kruseman and de Ridder, 1979) is about 830,200 years for the red clay and 8,994,000 years for the entire confining layer. The average porosity of the overlying confining layer calculated from the consolidation test data is 31.8%, therefore, the travel time through the red clay portion of the upper confining layer would be about 264,000 years and that of the entire upper confining layer would be about 2,860,000 years under unit gradient. Table 2.7-9 summarizes the confining layer properties determined by laboratory and field methods as part of this investigation.

The underlying confining layer piezometer (LCP-1) responded to the same rapid changes in barometric pressure which were measured in overlying confining layer piezometer (Figure 2.7-21). However, LCP-1 also showed a trend toward a very small amount of drawdown (.06 feet) during the aquifer test.

Because the vertical hydraulic conductivity of the underlying confining layer (Pierre Shale), as determined from the laboratory consolidation tests, is of the same order of magnitude as the vertical hydraulic conductivity of the upper confining layers ( $10^{-11} \text{ cm/sec}$ ), no drawdown was anticipated in LCP-1 during the test. For this reason, it is suspected that the small amount of drawdown observed in LCP-1 is the result of annular

TABLE 2.7-9

## SUMMARY OF CONFINING LAYER PROPERTIES

<u>Parameters</u>	<u>Red Clay (UCP-1)</u>	<u>Pierre Shale (LCP-1)</u>
Coefficient of compressibility, $a_v$ ( $\text{cm}^2/\text{g}$ )	$3.99 \times 10^{-7}$	$5.13 \times 10^{-7}$
Specific storage, $S_s$ , ( $\text{cm}^{-1}$ )	$3.08 \times 10^{-7}$	$2.78 \times 10^{-7}$
Diffusivity, $D$ , ( $\text{cm}^2/\text{sec}$ )	$1.13 \times 10^{-4}$	$5.22 \times 10^{-3}$
Vertical hydraulic conductivity, $K_v$ , ( $\text{cm}/\text{sec}$ )		
Lab Data	$3.49 \times 10^{-11}$	$3.63 \times 10^{-11}$
Field Data	-----	$1.45 \times 10^{-9}$
Hydraulic resistance, $c$ , (years)		
Lab Data	830,200 (1)	31,929,000
Field Data	-----	799,300
Porosity (percent)	31.8	32.5
Travel time (years)		
Lab Data	264,000 (2)	259,700
Field Data	-----	10,377,000

(1) Red Clay Member only - total overlying confining layer = 8,994,000.

(2) Red Clay Member only - total overlying confining layer = 2,860,000.

leakage between the borehole and the packer which was set to hydraulically isolate the piezometer tip from the overlying Basal Chadron Sandstone. If the packer did not completely seal the borehole above the piezometer tip, the piezometer would be affected by the pressure drop in the pumped aquifer which would be transmitted by the annulus leaks. Thus, the response of the piezometer would be the result of borehole-packer annulus leaks. If this were the case, the Neuman-Witherspoon analysis of the piezometer water levels would only serve to quantify the vertical leakage or hydraulic conductivity of the packer and borehole seal, not the vertical hydraulic conductivity of the underlying confining layer. Recognizing that this problem may exist, a Neuman-Witherspoon analysis was made of the water level data from LCP-1.

Results of the laboratory consolidation test data from two core samples from LCP-1 are shown earlier in Table 2.7-4. The calculated average coefficient of compressibility,  $a_v$ , of the Pierre Shale is  $5.13 \times 10^{-7}$  cm<sup>2</sup>/g and the calculated average vertical permeability is  $3.63 \times 10^{-11}$  cm/sec. Using these consolidation test data, the calculated specific storage of the top 5 feet of the underlying confining layer (Pierre Shale) is  $2.78 \times 10^{-7}$  cm<sup>3</sup> and the calculated hydraulic diffusivity is  $5.22 \times 10^{-3}$  cm<sup>2</sup>/sec. Applying the Neuman-Witherspoon Method to the data from the aquifer test and the consolidation test, produces a field vertical hydraulic conductivity of  $1.45 \times 10^{-9}$  cm/sec. Oil test holes have shown that the Pierre Shale is approximately 1200 feet thick in the vicinity of the aquifer test site. Therefore, the calculated hydraulic resistance,  $c$ , using field measured vertical hydraulic conductivity, is about 799,300 years. The calculated hydraulic resistance using the vertical hydraulic conductivity calculated from the laboratory consolidation tests is about 31,929,000 years. The average porosity of the Pierre Shale calculated from the consolidation test data is 32.5%. Therefore, the travel time through the Pierre Shale would be about 259,770 years using field determined vertical hydraulic conductivity and about 10,377,000 years using laboratory determined vertical hydraulic conductivity under unit gradient.

### Overlying Aquifer

The overlying aquifer monitor well, BMW-1, showed no response to the pumping from the Basal Chadron Sandstone during the aquifer test (Figure 2.7-21). However, this well did respond to barometric changes that occurred during the aquifer test which confirmed that it was functioning properly. Because BMW-1 did not respond to pumping, it is evident that the overlying aquifer is not in hydraulic communication with the Basal Chadron Sandstone. Therefore, no further analysis was made of the test data from BMW-1.

### INTERPRETATION OF DATA

#### Aquifer Response to Pumping

The results of this investigation show that the Basal Chadron Sandstone, which is the ore-bearing aquifer at the Crow Butte site, is a non-leaky, confined, slightly-anisotropic aquifer. The effective transmissivity of the Basal Chadron Sandstone is 2726 gpd/ft. The average thickness of the aquifer at the test site is about 40 feet. Therefore, the average hydraulic conductivity is about 68 gpd/ft<sup>2</sup> (9.10 ft/day). The average storativity is  $1.04 \times 10^{-4}$ . The azimuth and magnitude of the major axis of transmissivity are about 51° and 2760 gpd/ft (369 ft<sup>2</sup>/day). The azimuth and magnitude of the minor axis of transmissivity are about 141° and 2692 gpd/ft (360 ft<sup>2</sup>/day).

The piezometric surface of the Basal Chadron Sandstone is approximately 495 feet above the top of the aquifer. The piezometric surface of the overlying aquifer is about 204 feet above the top of the Brule Sand. The difference between the piezometric surfaces of the two aquifers is about 59 feet. This fact plus the fact that BMW-1 did not respond to pumping from the Basal Chadron Sandstone, are evidence that the Basal Chadron Sandstone is confined and that it is not hydraulically connected to the overlying aquifer.



### Integrity of Confinement

Confined aquifers may receive small amounts of water through vertical recharge from the confining layers. Even confining layers formed of very low permeability may yield small amounts of water if the hydraulic gradient in the aquifer-aquitard system is favorable. The aquitards which overlie and underlie the Basal Chadron Sandstone probably yielded some small amount of water as recharge (leakage) to the aquifer during the pumping of the aquifer test. However, the amount of this recharge or leakage was extremely small as evidenced by the piezometer responses and the drawdown analysis of the Basal Chadron Sandstone. The overlying confining layer piezometer did not show any response attributable to the pumping. The underlying confining layer piezometer did show a maximum drawdown of 0.06 feet about 4300 minutes after pumping began. However, it is suspected that this small amount of drawdown is attributable to leakage at the annulus of the packer and borehole rather than to leakage from the confining layer.

The lack of substantial drawdown in the confining layer piezometers is attributable to the extremely low vertical hydraulic conductivity of the confining layers. The vertical hydraulic conductivity of the overlying confining layer is about  $3.49 \times 10^{-11}$  cm/sec., and that of the underlying confining layer is about  $1.45 \times 10^{-9}$  to  $3.63 \times 10^{-11}$  cm/sec. Confining layers with vertical hydraulic conductivities this low are, by definition, called aquicludes, rather than aquitards.

The integrity of confinement of the ore-zone aquifer (Basal Chadron Sandstone) may be characterized most graphically by the hydraulic resistance, c. The calculated hydraulic resistance of the entire thickness of the overlying aquiclude is about 8,994,000 years and that of the underlying aquiclude is between 799,300 years and 31,900,000 years. The times needed for a given water molecule to travel through the entire thicknesses of the aquicludes under unit gradient (one foot of head loss per foot of movement in the direction of flow) are about 2,860,000 years for the upper aquiclude and about 260,000 years to 10,377,000 years for the lower. Because the gradients would be much smaller during mining, actual travel times would be much longer than those stated above.

### Movement of Groundwater

The piezometric surface of the Basal Chadron Sandstone dips approximately to the north at a gradient of  $7.84 \times 10^{-4}$  which is equal to 1 foot per 1275 feet. Using a directional hydraulic conductivity of 9.11 ft/day, a gradient  $7.84 \times 10^{-4}$  and a porosity of 29 percent, the average pore velocity across this part of the commercial study area is about 9.00 ft/year. The groundwater flux across the test site was computed to be about .29 ft<sup>3</sup>/day per unit width of the aquifer. (Darcy, 1856).

### Extent of Investigated Area

Using the Cooper-Jacob Distance-Drawdown Method (Cooper and Jacob, 1946), the radius of influence of the aquifer test in the Basal Chadron Sandstone was calculated to be about 5000 feet. Therefore, the area investigated and characterized by this test is approximately 1803 acres.