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|  | <p style="text-align: center;">CALCULATION SHEET</p> <p style="text-align: center;">PSEG SITE ESP APPLICATION PROJECT PROJECT No. 6468-08-2251</p> | CALC. NO. 2251-ESP-GW-002 |
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the dewatering would appear to pose little threat to the wetland east of the proposed expansion since it is tidally affected and renewed daily.

The areal impact of dewatering may also affect the stability of existing structures. The following existing structures are within the projected zone of dewatering influence.


- HC Cooling Tower
- Salem and HC ISFSI
- Waste Treatment Plant
- HC Switchyard
- Learning and Development Center
- HC Nuclear Island
- Fuel Oil Tank
- Material Center
- Low Level Radioactive Waste Building
- Salem Nuclear Island
- Nuclear Operations Support Facility

The degree to which drawdowns occur in the alluvial or structural fill about these units is largely a function of the competence of the Kirkwood aquitard. Additional data gathered in concert with pumping tests in the proposed expansion areas will be needed to more accurately evaluate the potential drawdowns once the final selection for the expansion technology is determined.

8.3 Anticipated Changes in Shallow Groundwater Flow Patterns

Modeling of post-construction conditions suggests that groundwater flow patterns and water levels would return to the pre-construction conditions over most of the model domain. Only slight increases of about 0.5 feet were noted in some portions of the model. Changes to the site, following construction would also include:

- The presence of the soil retention barriers (likely permanent elements extending from elevation -5 ft NAVD88 to -90 ft NAVD88);
- A localized gap (window) in the Kirkwood aquitard that would be replaced with structural fill;
- Placement of fill to establish a plant grade approximately 27 ft higher than the existing grade;
- The existing shallow perched ponds within the excavation footprint will be removed; and
- Replacement of the existing vegetation with developed hard surface.

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These physical changes will cause some variation in flow patterns; however the projected piezometric heads in the fill and alluvial materials are not expected to be much greater than the current static conditions.

Simulations for post-development suggest a potential average hydrostatic loading of about 3 to 4.5 feet NAVD 88 on the new unit, but based on considerations of potential tidal effects in the Vincenttown, a design loading of 6 feet NAVD 88 is recommended and is consistent with that proposed for the Hope Creek Station. The elevations of the bottom of the new structures may be deeper than the groundwater table; however, under normal conditions, pre- and post-construction water levels in shallow units would appear to be similar across the PSEG facility, with post-construction shallow water levels about a half-foot higher in some areas of existing structures (e.g., the cooling tower or Hope Creek unit) requiring no permanent long-term dewatering at the new unit. The characteristics of the soil retention barriers, which likely will be left in place, may also locally affect the hydrostatic loading.

Generally, because the ground surface for the new plant will be raised to approximately 36.9 feet NAVD88 and the groundwater barrier walls will remain in place, the maximum anticipated groundwater elevations within the groundwater barrier walls is 6 ft NAVD88. Thus the groundwater will be approximately 30 ft below the final plant grade. Thus, the anticipated hydrostatic loading on the future structures is less than the conservative hydrostatic water level on which the Design Control Documents (DCDs) are based.


Therefore, the proposed expansion would appear to alter groundwater flow patterns only slightly from current conditions in the areas of present facilities, and the need for a permanent dewatering system is not envisioned.

8.4 Sensitive Parameters in the Model

During calibration of the model, the most sensitive input parameters were determined to be recharge applied over the model (including seepage losses from ponds in the new plant location), horizontal hydraulic conductivity of the alluvial and Mount Laurel-Wenonah aquifer units, vertical conductivity of the Kirkwood, reference elevation for the Delaware River, and GHB reference heads in the Vincenttown and Mount Laurel-Wenonah Formations. During the dewatering simulations, the most sensitive parameters included the hydraulic conductivity of the Vincenttown and Hornerstown Formations.


8.5 Sensitivity Analyses

Sensitivity analyses were performed on the dewatering model run, varying key parameters of the hydraulic conductivity of the Vincenttown Formation, the vertical hydraulic conductivity of the leaky Navesink aquitard, and the vertical hydraulic conductivity of the Kirkwood aquitard. Averaging short-term initial rates, the model provided best estimates for the construction dewatering from about 5,200 to 5,600 gallons per minute over a year's simulation. The sensitivity analysis indicated that an expected range might vary from about 3,000 to 7,600 gallons per minute (averaging short-term initial rates). Given the larger proposed area for dewatering than that conducted for the Hope Creek unit, the estimated dewatering rates are generally consistent with those documented during the construction of the Hope Creek Generating Station. The estimated dewatering rates do not include storm water which may fall within the excavation limits. Collection sumps and high rate pumps will likely be needed to evacuate storm water from the excavation.

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8.6 Summary

This groundwater model has been completed in support of the ESP application for the PSEG Site. The model provides estimates of the expected groundwater response to dewatering and post-construction scenarios. However, the dewatering scenario and dewatering estimates are intended to be preliminary and are based on the assumed excavation boundaries. Groundwater modeling will be refined after the reactor vendor is selected, and the final excavation geometry is determined. Preparation of the COLA will likely require additional data, which could be obtained from pumping tests or other methods to further refine hydrogeologic parameters and model estimates of dewatering rates and drawdowns beneath existing site structures. Data gathered in support of the ESP combined with the location and size of the proposed plant excavation area has indicated that additional data is needed to refine estimates of dewatering rates and the potential for excessive drawdown at existing structures during the dewatering period. Once the technology and site layout has been determined, pumping tests are recommended at the PSEG site to further refine the groundwater model. The purposes for the pumping tests will be to determine aquifer characteristics of the Vincentown Formation in the proposed area of construction (the increased area of the power block extends beyond the explorations conducted in that area), determine the effectiveness of the Kirkwood aquitard to limit drawdown in the alluvial aquifer and fill (since it is absent in some locations and estimated dewatering drawdown effects in the alluvium are very sensitive to the vertical hydraulic conductivity of the Kirkwood aquitard), to assess potentials for upwelling from the underlying Mount Laurel-Wenonah Formation during dewatering, and assess the potential for encountering recharge boundaries in the Vincentown Formation in the northern portion of the proposed power block area.

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FIGURES



LEGEND

- ▬ Site Boundary
- ▬ New Plant Location
- ▬ East Area Location



PSEG Power, LLC
PSEG Site ESPA
Groundwater Model Calc. Package

Site Location and Proposed
Expansion Locations

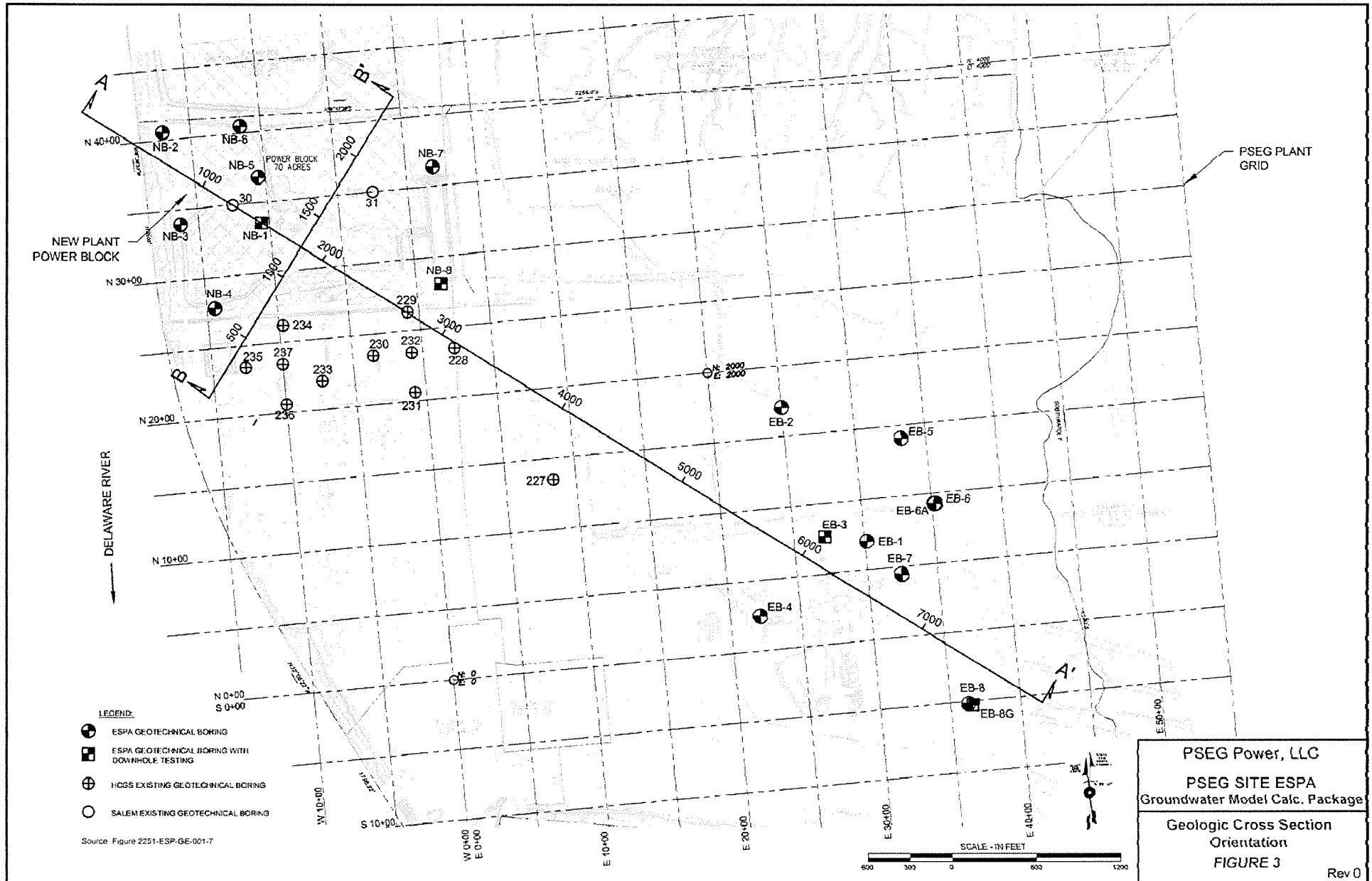
FIGURE 1

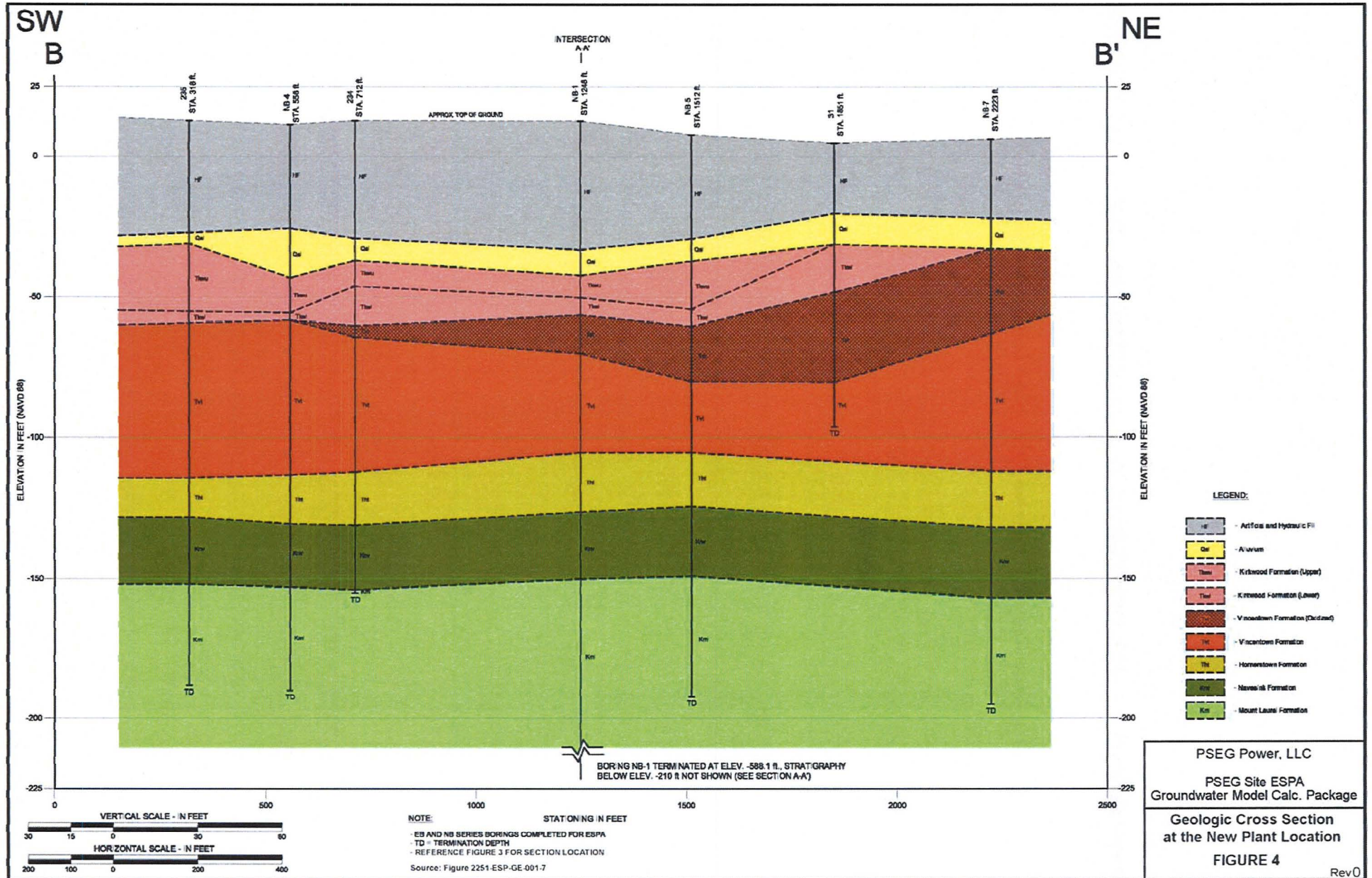
Rev 0

| | | Formation/Unit | | Primary Lithologies | | Geologic Conditions | Unit Thickness | Occurrence in Site Area | |
|----------|-------------------------|-----------------------------|---------------------------|--|--|--|--|---|---|
| CENOZOIC | Quaternary | Holocene | Quaternary Marsh deposits | | muck and peat; silt, sand and clay | | aggradation of Delaware Bay estuary | variable thickness | present over most of the site area in low lying areas |
| | | Pleistocene | unconformity | | | | | | |
| | | | DELAWARE | NEW JERSEY | | | | | |
| | | | | | Delaware Bay Group | Scotts Corners Formation | Cape May Formation | estuarine terrace deposits with coarse to fine sand and pebbles with concentrations of heavy minerals; peat; isolated fluvial deposits? | transgressive and regressive cycles |
| | unconformity | Lynch Heights Formation | | | | | | | |
| | Tertiary | Upper Tertiary (Miocene) | unconformity | | | | regression and erosion | | |
| | | | Kirkwood Formation | | clay silt and sand deposited in two or three marine cycles | | polycyclic transgression and regression phases | 90 feet at southern portions of site area; pinches out northward | subcrop only |
| | | Lower Tertiary | unconformity | | | | regression and erosion | | |
| | | | Shark River Formation | | glauconitic sand and mudstone | | low sediment input | 70 feet (Benson, 2004) | subcrop only |
| | | | unconformity | | | | regression and erosion | | |
| | | | Manasquan Formation | | lower glauconitic member, upper clayey sand to silt member | | low sediment input and bioturbation | 40 feet (Benson, 2004) | subcrop only |
| | | | unconformity | | | | regression and erosion | | |
| | | | Vincentown Formation | | quartz sand to quartz-rich calcareous sand with bryozoans and foraminifera | | | 90 feet (Benson, 2004) | outcrops in NW site area |
| | | | Homerstown Formation | | highly glauconitic sand with distinctive green color | | low sediment input and extreme bioturbation | 30 feet (Benson, 2004) | |
| | | | Upper Cretaceous | Navesink | | fossiliferous, clayey glauconitic sand | | transgression to midshelf conditions | 20 feet (Benson, 2004) |
| | | Mount Laurel Formation | | thinly bedded clays and sands with cross-bedding; thin pebbly sands | | | | | |
| | | Wenonah Formation | | clayey, silty, slightly glauconitic fine sand | | regressive pulse; low sediment input | 100 feet (Benson, 2004) | | |
| | | Marshalltown Formation | | intensely burrowed, very silty fine sand with glauconite | | transgression, low sediment input | 20 feet (Benson, 2004) | | |
| | | Englishtown Formation | | micaceous silt to very fine sand | | regressive pulse | 25 feet (Benson, 2004) | | |
| | Woodbury Formation | | | micaceous, chloritic, silty clay | | | | | |
| | Merchantville Formation | | | glauconitic sand to micaceous silty clay | | transgression and establishment of widespread marine conditions; low sediment rates | 120 feet (Benson, 2004) | | |
| | Magothy Formation | | | beach and estuarine deposits of cross-bedded sand, with clay and silt layers, some lignite | | transition to marine conditions | 50 feet, pinches out north of site location (Benson, 2004) | | |
| | unconformity | | | | | regression and erosion | | | |
| | Lower Cretaceous | Potomac Group (Formation) | | white, gray and red interbedded silts, clays, and quartzose sand | | aggregating a fluvial plain; thermal subsidence | 800 to 1650 feet (Benson, 2004) | subsurface only | |
| | | | | | | | | | |
| | | pre-Cretaceous unconformity | | | | uplift and erosion | | | |
| | Triassic | Upper Triassic | Basement Complex | | | | | | |
| | | | Triassic Basin? | | Fanglomerates and lacustrine sediments; diabase volcanics | | | | |
| | PRECAMBRIAN? PALEOZOIC? | Neoproterozoic to Silurian? | Carolina Superterrane? | Philadelphia Terrane? | meta mafic to felsic plutons and volcanics with sediments, and ultramafic components | aluminous to quartz rich schist with interbedded amphibolites (Wsshickon Formation) with ultramafic components; Wilmington Complex felsic to mafic arc complex | Amalgamation of Pangea followed by rifting to form North America | undetermined | |

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PSEG Site ESPA
Groundwater Model Calc. Package
Hydrogeologic Unit Sequence
FIGURE 2

Rev 0







Source: NJOIT, 2007

LEGEND

- ◆ MACTEC Observation Well
- Production Well
- ARCADIS Well (B Series)
- Observation Wells for SGS and HCGS
- Site Boundary




0 500 1,000
Feet

PSEG Power, LLC
PSEG Site ESPA
Groundwater Model Calc. Package

Location of Water Level
Data Observations

FIGURE 5

Rev 0

| | | |
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TABLES

| Formation | Transmissivity | Hydraulic Conductivity | Porosity | Storage Coefficient | Specific Yield | Specific Capacity | Leakance | Reference |
|---------------------------------------|--|-----------------------------------|----------------|---------------------|----------------|---------------------|-----------------------|-----------|
| Structural Fill | | 0.09 to 4.3 ft/d; 6.5 ft/d | | | | | | 12 |
| Riverbed Sands and Gravel | 13.2 to 440 gpd/ft | 0.9 to 13.1 gpd/ft ² | | | | | | 8 |
| | | 0.12 to 1.75 ft/d; 1.8 to 59 ft/d | | | | | | 7 |
| | | 0.03 to 2.27 ft/d | | | | | | 12 |
| Kirkwood Aquitard | | Kv = 0.00002 to 0.00005 ft/d | | | | | 1e-5/d | 8 |
| Basal Kirkwood-Vincentown Aquifer | 5,000 to 11,000 gpd/ft | 0.95 to 2.5 ft/d | | | | 0.5 to 8.3 gpm/ft | | 10 |
| | 530 ft ² /d | | | | | 0.3 to 1.9 gpm/ft | | 2 |
| | 2,000 to 2,500 ft ² /d | | | | | | | 7 |
| | | 14 ft/d | | | | | | 8 |
| | | 2.95 ft/d | | | | | | 10 |
| | 1,987 to 2,791 ft ² /d | | | | | | | 14 |
| Hornerstown - Navesink Aquitard | | 4 to 8.7 ft/d | 0.522 to 0.543 | | | | | 12 |
| | | Kv = 0.42 gpd/ft ² | | | | | | 2 |
| | | Kv = 0.003 to 9 ft/d | | | | | | 8 |
| | | | | | | | 5e-5/d | 10 |
| | | | | | | | 3.35e-5 to 6.87e-5/d | 11 |
| Mt. Laurel - Wenonah Aquifer | 7,000 gpd/ft | 18.7 ft/d | 0.444 | | | | | 1 |
| | | 10 ft/d | | | | 0.7 to 9 gpm/ft | | 2 |
| | 7,500 to 14,000 gpd/ft | | | | | | | 3 |
| | 4,900 to 8,700 gpd/ft | 0.67 to 4.5 ft/d | | | | 0.2 to 3.8 gpm/ft | | 7 |
| | 360 to 1,430 ft ² /d | 13 to 19 ft/d | | | | | | 8 |
| | 1,000 ft ² /d | | | | | | | 10 |
| | 815 ft ² /d | | | | | | | 13 |
| | 726 to 922 ft ² /d | | | | | | | 11 |
| Marshalltown-Wenonah Aquitard | | 0.001 to 0.01 gpd/ft ² | | | | | | 2 |
| | | Kv = 0.00006 to 0.13 ft/d | | | | | | 8 |
| | | | | | | | 6e-6/d | 10 |
| | | | | | | | 5.91e-6 to 7.13e-6/d | 11 |
| Englishtown Aquifer | | | | | | up to 10 gpm/ft | | 2 |
| | 1,100 to 2,100 ft ² /d | 12 to 67 ft/d | | | | | | 8 |
| | 500 ft ² /d | | | | | | | 10 |
| Merchantville-Woodbury Confining Unit | 415 to 552 ft ² /d | | | | | | | 11 |
| | | Kv = 0.000004 to 0.0004 ft/d | | | | | | 8 |
| | | | | | | | 3e-6/d | 10 |
| Upper PRM Aquifer | | | | | | | 2.15e-6 to 3.85e-6/d | 11 |
| | 10,000 to 25,000 gpd/ft | | | | | | | 4 |
| | 15,000 to 25,000 gpd/ft | | | | | | | 5 |
| | 9,000 to 27,000 gpd/ft | | | | | 10.6 to 26.7 gpm/ft | | 7 |
| | 870 to 24,210 gpd/ft | 240 ft/d | | | | | | 8 |
| | 2,000 ft ² /d | | | | | | | 10 |
| Confining Unit, Upper to Middle PRM | 1,086 to 2,419 ft ² /d | | | | | | | 11 |
| | | Kv = 0.084 ft/d | | | | | | 8 |
| | | | | | | | 2e-6/d | 10 |
| Middle PRM Aquifer | | | | | | | 1.797e-7 to 2.69e-7/d | 11 |
| | 4,700 to 11,500 gpd/ft | | | | | | | 4 |
| | 8,590 gpd/ft | 129.5 ft/d | 0.0025 | | | | | 6 |
| | 670 to 4,000 gpd/ft | | | | | | | 7 |
| | 4,000 ft ² /d | | | | | | | 10 |
| Confining Unit, Middle to Lower PRM | 3,024 to 3,813 ft ² /d | | | | | | | 11 |
| | | | | | | | 5e-6/d | 10 |
| | | | | | | | 7.19e-7 to 1.67e-5/d | 11 |
| Lower PRM Aquifer | 2,300 to 16,600 ft ² /d | | | | | | | 8 |
| | 4,000 to 5,000 ft ² /d | | | | | | | 10 |
| | 4,844 to 5,299 ft ² /d | | | | | | | 11 |
| Notes: | D&M = Dames & Moore USGS = United States Geological Survey NJDEP = New Jersey Department of Environmental Protection NJ = State of New Jersey T = transmissivity K = horizontal hydraulic conductivity Kv = vertical hydraulic conductivity L = leakance n = porosity s = storage coefficient (confined) Sy = specific yield | | | | | | | |
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Table 2
Slug Test Results
for Northern Area
Shallow Aquifers

| Well | Formation | Result feet/day |
|---------|----------------|--------------------|
| Shallow | | |
| NOW-1U | Alluvium | 8.0 |
| NOW-2U | Alluvium | 8.0 |
| NOW-3U | Alluvium | 0.3 |
| NOW-4U | Alluvium | 0.7 |
| NOW-5U | Hydraulic fill | 0.1 |
| NOW-6U | Alluvium | 3.5 |
| NOW-7U | Vincentown | 1.4 |
| NOW-8U | Alluvium | 0.4 |
| | | |
| Deeper | | |
| NOW-1L | Vincentown | 4.5 |
| NOW-2L | Vincentown | 3.6 |
| NOW-3L | Vincentown | 1.4 |
| NOW-4L | Vincentown | 10.7 |
| NOW-5L | Vincentown | 1.7 |
| NOW-6L | Vincentown | 6.2 |
| NOW-7L | Vincentown | 2.4 |
| NOW-8L | Vincentown | 0.3 |

Notes: Individual results are rounded off to one decimal place.
 See Hydraulic Conductivity & Tidal Study Data Report, Rev. A (MACTEC, November 25, 2009) for details on slug testing.

Table 3
Water Level Measurements
January to December 2009
North and East Sites
Alluvium and Vincentown

| | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sept | Oct | Nov | Dec | Ave | Std dev | Range |
|----------------------------|-------|-------|-------|-------|-------|------|-------|------|------|------|-------|------|------|---------|-------|
| North Site Alluvium | | | | | | | | | | | | | | | |
| NOW-1U | | 0.36 | 0.61 | 0.59 | 0.66 | 1.32 | 1.14 | 0.94 | 1.13 | 1.22 | 1.18 | | 0.92 | 0.33 | 0.96 |
| NOW-2U | -0.10 | -0.42 | -0.48 | -0.17 | -0.08 | 2.04 | -0.41 | 1.72 | 2.08 | 2.19 | -0.20 | 0.88 | 0.59 | 1.11 | 2.67 |
| NOW-3U | -0.21 | -0.36 | 0.15 | -0.19 | 0.18 | 1.20 | 0.56 | 0.66 | 1.13 | 1.18 | 0.60 | 1.23 | 0.51 | 0.59 | 1.59 |
| NOW-4UB | | 0.03 | 0.46 | 0.36 | 0.40 | 1.18 | 1.00 | 0.75 | 0.95 | 1.09 | 0.95 | 1.34 | 0.77 | 0.41 | 1.31 |
| NOW-6U | 0.50 | 0.35 | 0.76 | 0.62 | 0.65 | 1.35 | 1.12 | 0.98 | 1.31 | 1.31 | 1.15 | 1.44 | 0.96 | 0.37 | 1.09 |
| NOW-7U | 0.40 | 0.18 | 0.74 | 0.77 | 0.79 | 1.40 | 1.14 | 1.07 | 1.41 | 1.46 | 1.01 | 1.64 | 1.00 | 0.44 | 1.46 |
| NOW-8U | 0.72 | 0.41 | 0.84 | 0.74 | 0.86 | 1.57 | 1.24 | 1.21 | 1.38 | 1.39 | 1.15 | 1.57 | 1.09 | 0.37 | 1.16 |
| Vincentown | | | | | | | | | | | | | | | |
| NOW-1L | | 0.25 | 0.56 | 0.50 | 0.65 | 1.58 | 1.07 | 1.14 | 1.54 | 1.66 | 1.02 | 1.67 | 1.06 | 0.51 | 1.42 |
| NOW-2L | -0.05 | -0.31 | -0.32 | -0.20 | 0.74 | 2.16 | -0.17 | 1.86 | 2.82 | 2.15 | -0.01 | 1.10 | 0.81 | 1.16 | 3.14 |
| NOW-3L | -0.14 | -0.25 | -0.40 | 0.10 | -0.99 | 1.63 | 0.10 | 1.69 | 1.90 | 1.38 | 0.61 | 1.25 | 0.57 | 0.97 | 2.89 |
| NOW-4L | -0.71 | -0.30 | -0.01 | -0.16 | 0.37 | 1.70 | 0.43 | 1.20 | 1.80 | 1.56 | 0.43 | 1.45 | 0.65 | 0.86 | 2.51 |
| NOW-5L | 0.54 | -0.19 | 0.31 | 0.35 | 0.52 | 1.54 | 0.93 | 0.73 | 1.54 | 1.59 | 0.65 | 1.57 | 0.84 | 0.60 | 1.78 |
| NOW-6L | -0.11 | -0.08 | 0.26 | 0.17 | -0.58 | 1.56 | 0.88 | 0.80 | 1.54 | 1.63 | 1.04 | 0.21 | 0.61 | 0.74 | 2.21 |
| NOW-7L | 0.39 | | 0.59 | 0.70 | 0.71 | 1.11 | 0.87 | 0.94 | 1.34 | 1.39 | 0.75 | 1.51 | 0.94 | 0.36 | 1.12 |
| NOW-8L | 0.50 | 0.36 | 0.70 | 0.79 | 0.90 | 1.54 | 1.15 | 1.14 | 1.44 | 1.43 | 1.08 | 1.51 | 1.05 | 0.40 | 1.18 |
| East Site Alluvium | | | | | | | | | | | | | | | |
| EOW-1U | 0.95 | 0.90 | 1.20 | 1.08 | 1.18 | 1.74 | 1.51 | | 2.54 | 1.59 | 1.52 | 1.79 | 1.45 | 0.47 | 1.64 |
| EOW-2U | 2.92 | 2.80 | 2.83 | 2.49 | 2.70 | 3.02 | 2.96 | | 2.74 | 3.09 | 2.87 | 3.40 | 2.89 | 0.24 | 0.91 |
| EOW-5U | 1.03 | 0.83 | 1.16 | 1.10 | 1.19 | 1.70 | 1.45 | 1.43 | 1.61 | 1.59 | 0.51 | 1.78 | 1.28 | 0.38 | 1.27 |
| EOW-6U | 1.00 | 0.79 | 1.20 | 1.12 | 1.16 | 1.71 | 1.45 | 1.43 | 1.59 | 1.60 | 1.49 | 1.78 | 1.36 | 0.30 | 0.99 |
| EOW-8U | 0.72 | 1.02 | 1.47 | 0.95 | 1.27 | | 1.73 | 1.65 | 1.46 | 1.70 | 1.46 | 2.27 | 1.43 | 0.43 | 1.55 |
| EOW-9U | -0.06 | 0.08 | 0.50 | 0.55 | 0.35 | 1.20 | 0.78 | 0.75 | 1.21 | 1.13 | 0.86 | | 0.67 | 0.43 | 1.27 |
| EOW-10U | | 1.43 | 1.37 | 1.32 | 1.39 | 2.07 | 1.58 | 1.52 | 1.71 | 1.85 | 1.86 | 2.30 | 1.67 | 0.32 | 0.98 |
| Vincentown | | | | | | | | | | | | | | | |
| EOW-1L | 0.79 | 0.62 | 0.92 | 0.98 | 0.95 | 1.59 | 1.29 | | 1.59 | 1.59 | 1.27 | 1.59 | 1.20 | 0.36 | 0.97 |
| EOW-2L | 1.06 | 0.74 | 1.25 | 1.18 | 1.12 | 1.74 | 1.42 | 1.39 | 1.76 | 1.67 | 1.43 | 1.72 | 1.37 | 0.32 | 1.02 |
| EOW-4L | 0.62 | 0.51 | 1.09 | 0.90 | 1.00 | 1.75 | 1.33 | 1.19 | 1.85 | 1.91 | | 1.59 | 1.25 | 0.48 | 1.40 |
| EOW-5L | 1.09 | 0.92 | 1.30 | 1.25 | 0.86 | 1.79 | 1.51 | 2.39 | 1.78 | 1.74 | 1.49 | 1.77 | 1.49 | 0.44 | 1.53 |
| EOW-6L | 0.98 | 0.70 | 1.30 | 1.14 | 1.06 | | 1.45 | 0.47 | 1.80 | 0.74 | 1.45 | 1.74 | 1.17 | 0.43 | 1.33 |
| EOW-8L | 0.12 | 0.13 | 0.60 | 0.55 | 0.68 | 1.48 | 0.94 | 0.85 | 1.59 | 1.61 | 1.05 | 1.27 | 0.91 | 0.52 | 1.49 |
| EOW-9L | 0.45 | 0.41 | 0.68 | 0.77 | 0.97 | 1.68 | 1.28 | 1.05 | 1.86 | 1.86 | 1.18 | 1.49 | 1.14 | 0.51 | 1.45 |
| EOW-10L | 0.60 | 0.66 | 1.12 | 0.94 | 0.35 | 1.66 | 1.36 | 1.24 | 1.71 | 1.76 | 1.34 | 1.61 | 1.20 | 0.47 | 1.41 |

Notes:

- Initially considered inconsistent with data set
- Outlier value deleted
- Rejected as inconsistent with data set following outlier analysis

Elevations are in feet relative to NAVD88 datum

See MACTEC, 2010, Groundwater Elevations and Hydraulic Gradients, Calculation Package 2251-ESP-GW-004 for a detailed account of the water level data analysis.

Table 4
Calibrated Model - Input Parameter Values

| Hydraulic conductivities, ft/d | Horizontal | Vertical | |
|--------------------------------|------------|------------------|----------|
| Hydraulic fill | 0.1 | 0.03 | |
| Structural fill | 6.5 | 0.65 | |
| Alluvium | 3.89 | 0.48 | |
| Kirkwood aquitard | 0.02 | 0.003 | |
| Vincentown | 10.7 | 0.2 | |
| Hornerstown | 10.7 | 0.2 | |
| Navesink | 0.4 | 0.0545 | |
| Mount Laurel-Wenonah | 10 | 10 | |
| Recharge | ft/d | in/yr | |
| Zone 1 Wetlands north | 0.00003521 | 0.15 | |
| Zone 2 Buildings, pavement | 0 | 0 | |
| Zone 3 Developed facility | 0.0002907 | 1.27 | |
| Zone 4 Wetlands east | 0.0001385 | 0.61 | |
| Zone 5 Semi-impermeable | 0.0004176 | 1.83 | |
| Zone 6 Near Salem units 1&2 | 0.001826 | 8 | |
| Storage coefficient | Confined | Specific yield | Porosity |
| Aquifers | 0.0001 | 0.2 | 0.35 |
| Aquitards | 0.0001 | 0.1 | 0.4 |
| River package | Ref Elev | Conductance | |
| Delaware River | -0.1 | 56.6 | |
| Ponds | 4 to 5.4 | 0.0282 to 0.0566 | |
| Streams | 0 | 5.66 to 11.3 | |
| General Head Boundaries | Ref Elev | Conductance | |
| Alluvium | -0.5 | 25.1 | |
| Vincentown | 0.5 to 2.0 | 408 to 640 | |
| Hornerstown | 0.5 to 2.0 | 148 | |
| Mount Laurel-Wenonah | -1 to 0.8 | 3590 to 3940 | |

- Notes:
1. Reference elevations are in feet NAVD88
 2. Units of conductance are square feet per day
 3. GHB conductances are a function of thickness and generally vary along their lengths.

Table 5
Residuals Statistical Analysis
Final Calibrated Model

| Name | Easting | Northing | Layer | Observed | Computed | Residual |
|------------------------|---------|----------|-------|----------|----------|----------|
| NOW-1U | 198443 | 234543 | 2 | 0.93 | 1.04 | -0.11 |
| NOW-1L | 198450 | 234564 | 4 | 1.08 | 0.79 | 0.29 |
| NOW-2U | 197755 | 235207 | 2 | 0.53 | 0.50 | 0.03 |
| NOW-2L | 197753 | 235228 | 4 | 0.73 | 0.60 | 0.13 |
| NOW-3U | 197885 | 234553 | 2 | 0.53 | 0.39 | 0.14 |
| NOW-3L | 197898 | 234565 | 4 | 0.60 | 0.64 | -0.04 |
| NOW-4UB | 198147 | 233963 | 2 | 0.79 | 0.90 | -0.11 |
| NOW-4L | 198148 | 233973 | 4 | 0.67 | 0.70 | -0.03 |
| NOW-5L | 198438 | 234927 | 4 | 0.87 | 0.81 | 0.06 |
| NOW-6U | 198314 | 235269 | 2 | 0.98 | 1.13 | -0.15 |
| NOW-6L | 198313 | 235288 | 4 | 0.63 | 0.80 | -0.17 |
| NOW-7U | 199694 | 234976 | 2 | 1.02 | 1.11 | -0.09 |
| NOW-7L | 199676 | 234973 | 4 | 0.93 | 1.01 | -0.08 |
| NOW-8U | 199756 | 234142 | 2 | 1.11 | 1.99 | -0.88 |
| NOW-8L | 199736 | 234139 | 4 | 1.06 | 0.97 | 0.09 |
| EOW-1U | 202758 | 232322 | 2 | 1.40 | 2.04 | -0.64 |
| EOW-1L | 202758 | 232298 | 4 | 1.22 | 0.89 | 0.33 |
| EOW-2U | 202158 | 233275 | 2 | 2.88 | 2.14 | 0.74 |
| EOW-2L | 202178 | 233271 | 4 | 1.40 | 0.98 | 0.42 |
| EOW-4L | 202021 | 231773 | 4 | 1.26 | 0.83 | 0.43 |
| EOW-5U | 203007 | 233057 | 2 | 1.31 | 1.75 | -0.44 |
| EOW-5L | 203021 | 233040 | 4 | 1.46 | 0.96 | 0.50 |
| EOW-6U | 203281 | 232587 | 2 | 1.38 | 1.76 | -0.38 |
| EOW-6L | 203301 | 232588 | 4 | 1.17 | 0.93 | 0.24 |
| EOW-8U | 203520 | 231144 | 2 | 1.41 | 1.51 | -0.10 |
| EOW-8L | 203516 | 231163 | 4 | 0.91 | 0.80 | 0.11 |
| EOW-9U | 202826 | 230917 | 2 | 0.69 | 1.54 | -0.85 |
| EOW-9L | 202845 | 230926 | 4 | 1.14 | 0.76 | 0.38 |
| EOW-10U | 203521 | 231687 | 2 | 1.64 | 1.55 | 0.09 |
| EOW-10L | 203522 | 231707 | 4 | 1.22 | 0.85 | 0.37 |
| Well_BA | 199984 | 230320 | 2 | 1.97 | 1.80 | 0.17 |
| Well_BF | 199322 | 231301 | 2 | 1.88 | 1.96 | -0.08 |
| Well_BG | 199212 | 231829 | 2 | 2.30 | 2.36 | -0.06 |
| Well_BH | 198752 | 231891 | 2 | 1.77 | 1.54 | 0.23 |
| Well_BL | 198390 | 232627 | 2 | 1.69 | 1.38 | 0.31 |
| Well_BM | 198936 | 232658 | 2 | 2.26 | 2.66 | -0.40 |
| Well_BP | 198010 | 233572 | 2 | 1.09 | 0.85 | 0.24 |
| Well_BQ | 198966 | 233401 | 2 | 2.95 | 2.72 | 0.23 |
| Well_BR | 198711 | 234004 | 2 | 1.72 | 1.91 | -0.19 |
| Well_BS | 200475 | 234137 | 2 | 2.71 | 1.77 | 0.94 |
| Well_BT | 199958 | 232909 | 2 | 3.13 | 3.08 | 0.05 |
| Well_BU | 200236 | 231883 | 2 | 2.95 | 3.00 | -0.05 |
| Residual Mean | | | | | | 0.04 |
| Res. Std. Dev. | | | | | | 0.36 |
| Sum of Squares | | | | | | 5.48 |
| Abs. Res. Mean | | | | | | 0.27 |
| Min. Residual | | | | | | -0.88 |
| Max. Residual | | | | | | 0.94 |
| Range in Target Values | | | | | | 2.60 |
| Std. Dev./Range | | | | | | 0.138 |

Note: Target water level data were selected as the average of the acceptable data (see Table 3) with the high and low value also deleted. See Calculation Package 2251-ESP-GW-004 for details on the treatment of water level data collected for this purpose.

Table 6
Calibrated Model - Water Balance

| | Unit | Top | Bottom | GHB | River | Recharge | % error |
|---------|--------------------|-------|--------|-------|-------|----------|-----------|
| Model | All | | | | | | -0.146 |
| In | | * | * | 18480 | 1355 | 6131 | |
| Out | | * | * | 20468 | 5536 | 0 | |
| Layer 1 | Fills | | | | | | 0.0041 |
| In | | * | 4135 | * | 1355 | 6119 | |
| Out | | * | 6073 | * | 5536 | 0 | |
| Layer 2 | Alluvium | | | | | | -0.000086 |
| In | | 6073 | 3874 | 0 | * | 0.46 | |
| Out | | 4135 | 3490 | 2322 | * | 0 | |
| Layer 3 | Kirkwood Aquitard | | | | | | 0.0013 |
| In | | 3490 | 3859 | * | * | 11.7 | |
| Out | | 3874 | 3487 | * | * | 0 | |
| Layer 4 | Vincentown | | | | | | -0.0876 |
| In | | 3487 | 428 | 11256 | * | 0.2 | |
| Out | | 3859 | 10851 | 475 | * | 0 | |
| Layer 5 | Hornerstown | | | | | | -0.0195 |
| In | | 10851 | 428 | 7223 | * | * | |
| Out | | 573 | 17762 | 460 | * | * | |
| Layer 6 | Navesink Aquitard | | | | | | -0.0001 |
| In | | 17763 | 564 | * | * | * | |
| Out | | 573 | 17754 | * | * | * | |
| Layer 7 | Mt. Laurel-Wenonah | | | | | | -0.12 |
| In | | 17754 | * | 0 | * | * | |
| Out | | 564 | * | 17211 | * | * | |

Notes: 1. * - indicates this feature not applicable
2. Rates given are in cubic feet per day

Table 7
Summary Sensitivity Statistical Analysis
Calibrated Base Model

| Input Parameter | Range | | | | |
|------------------------------|-----------|-----------|------------|-----------|-----------|
| | lowest | lower | Calibrated | higher | highest |
| Hydraulic fill, Kh, ft/d | 0.025 | 0.05 | 0.1 | 0.2 | 0.4 |
| ave residual | -0.02 | 0.01 | 0.04 | 0.08 | 0.14 |
| average absolute | 0.29 | 0.28 | 0.27 | 0.29 | 0.31 |
| standard deviation | 0.38 | 0.36 | 0.36 | 0.37 | 0.38 |
| sum of squares | 5.94 | 5.52 | 5.48 | 5.93 | 6.84 |
| Alluvium, Kh, ft/d | 0.97 | 1.94 | 3.89 | 7.88 | 15.76 |
| ave residual | -0.13 | -0.06 | 0.04 | 0.18 | 0.35 |
| average absolute | 0.33 | 0.29 | 0.27 | 0.3 | 0.41 |
| standard deviation | 0.42 | 0.39 | 0.36 | 0.35 | 0.37 |
| sum of squares | 8.19 | 5.49 | 5.48 | 6.46 | 11.1 |
| Kirkwood, Kh, ft/d | 0.005 | 0.01 | 0.02 | 0.04 | 0.08 |
| ave residual | 0.04 | 0.04 | 0.04 | 0.04 | 0.04 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.48 | 5.48 | 5.48 | 5.48 | 5.48 |
| Vincentown, Kh, ft/d | 2.67 | 5.35 | 10.7 | 21.4 | 42.8 |
| ave residual | 0.12 | 0.09 | 0.04 | -0.02 | -0.08 |
| average absolute | 0.3 | 0.28 | 0.27 | 0.26 | 0.26 |
| standard deviation | 0.37 | 0.36 | 0.36 | 0.36 | 0.37 |
| sum of squares | 6.41 | 5.91 | 5.48 | 5.48 | 6.01 |
| Homerstown, Kh, ft/d | 2.67 | 5.35 | 10.7 | 21.4 | 42.8 |
| ave residual | 0.06 | 0.05 | 0.04 | 0.02 | 0 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.26 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.58 | 5.54 | 5.48 | 5.42 | 5.42 |
| Navesink, Kh, ft/d | 0.1 | 0.2 | 0.4 | 0.8 | 1.6 |
| ave residual | 0.04 | 0.04 | 0.04 | 0.04 | 0.04 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.48 | 5.48 | 5.48 | 5.48 | 5.48 |
| Mt. Laurel-Wenonah, Kh, ft/d | 2.5 | 5 | 10 | 20 | 40 |
| ave residual | -0.18 | -0.08 | 0.04 | 0.16 | 0.26 |
| average absolute | 0.3 | 0.27 | 0.27 | 0.31 | 0.36 |
| standard deviation | 0.38 | 0.36 | 0.36 | 0.37 | 0.39 |
| sum of squares | 7.41 | 5.77 | 5.48 | 6.86 | 9.14 |
| Structural fill, Kh, ft/d | 1.62 | 3.25 | 6.5 | 13 | 26 |
| ave residual | 0.01 | 0.03 | 0.04 | 0.05 | 0.06 |
| average absolute | 0.29 | 0.27 | 0.27 | 0.28 | 0.28 |
| standard deviation | 0.38 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.97 | 5.5 | 5.48 | 5.59 | 5.71 |
| Recharge, zone 1, ft/d | 1.761E-05 | 2.641E-05 | 3.521E-05 | 5.282E-05 | 7.042E-05 |
| ave residual | 0.05 | 0.05 | 0.04 | 0.03 | 0.01 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.28 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.37 |
| sum of squares | 5.43 | 5.45 | 5.48 | 5.56 | 5.68 |
| Recharge, zone 3, ft/d | 0.0001453 | 0.0002179 | 0.0002907 | 0.0004361 | 0.0005814 |
| ave residual | 0.27 | 0.15 | 0.04 | -0.19 | -0.41 |
| average absolute | 0.4 | 0.32 | 0.27 | 0.39 | 0.56 |
| standard deviation | 0.45 | 0.38 | 0.36 | 0.48 | 0.72 |
| sum of squares | 11.5 | 7.02 | 5.48 | 11.3 | 28.9 |

Table 7
Summary Sensitivity Statistical Analysis
Calibrated Base Model

| | | | | | |
|------------------------------|-----------|-----------|-----------|-----------|-----------|
| Recharge, zone 4, ft/d | 0.0000693 | 0.0001039 | 0.0001385 | 0.0002078 | 0.000277 |
| ave residual | 0.14 | 0.09 | 0.04 | -0.06 | -0.16 |
| average absolute | 0.27 | 0.26 | 0.27 | 0.31 | 0.37 |
| standard deviation | 0.35 | 0.34 | 0.36 | 0.44 | 0.57 |
| sum of squares | 5.87 | 5.27 | 5.48 | 8.38 | 14.6 |
| Recharge, zone 5, ft/d | 0.0002088 | 0.0003132 | 0.0004176 | 0.0006264 | 0.0008352 |
| ave residual | 0.06 | 0.05 | 0.04 | 0.02 | 0.01 |
| average absolute | 0.28 | 0.27 | 0.27 | 0.28 | 0.29 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.37 |
| sum of squares | 5.6 | 5.51 | 5.48 | 5.58 | 5.9 |
| Recharge, zone 6, ft/d | 0.000913 | 0.001369 | 0.001826 | 0.002283 | 0.002739 |
| ave residual | 0.08 | 0.06 | 0.04 | 0 | -0.03 |
| average absolute | 0.29 | 0.28 | 0.27 | 0.29 | 0.32 |
| standard deviation | 0.38 | 0.37 | 0.36 | 0.38 | 0.43 |
| sum of squares | 6.39 | 5.76 | 5.48 | 6.01 | 7.94 |
| Hydraulic fill, Kv, ft/d | 0.0075 | 0.015 | 0.03 | 0.06 | 0.12 |
| ave residual | 0.01 | 0.02 | 0.04 | 0.06 | 0.07 |
| average absolute | 0.26 | 0.27 | 0.27 | 0.27 | 0.28 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.46 | 5.44 | 5.48 | 5.59 | 5.72 |
| Alluvium, Kv, ft/d | 0.12 | 0.24 | 0.48 | 0.96 | 1.92 |
| ave residual | 0.04 | 0.04 | 0.04 | 0.04 | 0.04 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.5 | 5.49 | 5.48 | 5.48 | 5.48 |
| Kirkwood, Kv, ft/d | 0.00075 | 0.0015 | 0.003 | 0.006 | 0.012 |
| ave residual | -0.73 | -0.3 | 0.04 | 0.28 | 0.43 |
| average absolute | 0.91 | 0.51 | 0.27 | 0.35 | 0.46 |
| standard deviation | 1.11 | 0.63 | 0.36 | 0.39 | 0.49 |
| sum of squares | 73.4 | 20.2 | 5.48 | 9.59 | 17.9 |
| Vincentown, Kv, ft/d | 0.05 | 0.1 | 0.2 | 0.4 | 0.8 |
| ave residual | -0.01 | 0.02 | 0.04 | 0.05 | 0.05 |
| average absolute | 0.28 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.37 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.89 | 5.58 | 5.48 | 5.45 | 5.44 |
| Hornerstown, Kv, ft/d | 0.05 | 0.1 | 0.2 | 0.4 | 0.8 |
| ave residual | 0.03 | 0.04 | 0.04 | 0.04 | 0.04 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.47 | 5.48 | 5.48 | 5.48 | 5.48 |
| Navesink, Kv, ft/d | 0.0136 | 0.0272 | 0.0545 | 0.109 | 0.218 |
| ave residual | -0.01 | 0.02 | 0.04 | 0.05 | 0.06 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.54 | 5.47 | 5.48 | 5.51 | 5.53 |
| Mt. Laurel-Wenonah, Kv, ft/d | 0.3 | 0.6 | 1.2 | 2.4 | 4.8 |
| ave residual | 0.03 | 0.04 | 0.04 | 0.04 | 0.04 |
| average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| sum of squares | 5.47 | 5.48 | 5.48 | 5.48 | 5.48 |
| Structural fill, Kv, ft/d | 0.16 | 0.32 | 0.65 | 1.3 | 2.6 |
| ave residual | 0.04 | 0.04 | 0.04 | 0.04 | 0.04 |

Table 7
Summary Sensitivity Statistical Analysis
Calibrated Base Model

| | | | | | | |
|--|--------------------|-------|-------|------|-------|-------|
| | average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.47 | 5.47 | 5.48 | 5.49 | 5.49 |
| Delaware River, ref head, ft | | -2.1 | -1.1 | -0.1 | 0.9 | 1.9 |
| | ave residual | 0.3 | 0.17 | 0.04 | -0.09 | -0.22 |
| | average absolute | 0.42 | 0.34 | 0.27 | 0.31 | 0.42 |
| | standard deviation | 0.48 | 0.39 | 0.36 | 0.4 | 0.49 |
| | sum of squares | 13.4 | 7.66 | 5.48 | 6.95 | 12.2 |
| Delaware River, conductance, 1/d | | 5.66 | 11.3 | 56.6 | 283 | 566 |
| | ave residual | 0.01 | 0.03 | 0.04 | 0.04 | 0.04 |
| | average absolute | 0.28 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.37 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.61 | 5.49 | 5.48 | 5.51 | 5.51 |
| GHB, Alluvium, ref head, ft | | -2.5 | -1.5 | -0.5 | 0.5 | 1.5 |
| | ave residual | 0.09 | 0.07 | 0.04 | 0.01 | -0.01 |
| | average absolute | 0.29 | 0.28 | 0.27 | 0.27 | 0.29 |
| | standard deviation | 0.37 | 0.36 | 0.36 | 0.36 | 0.37 |
| | sum of squares | 6.12 | 5.68 | 5.48 | 5.53 | 5.83 |
| GHB, Alluvium, conductance, 1/d | | 2.51 | 5.02 | 25.1 | 125.5 | 251 |
| | ave residual | 0.03 | 0.03 | 0.04 | 0.05 | 0.05 |
| | average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.48 | 5.48 | 5.48 | 5.5 | 5.5 |
| GHB, Vincentown, ref head, delta ft | | -2 | -1 | 0 | 1 | 2 |
| | average absolute | 0.76 | 0.47 | 0.27 | 0.39 | 0.69 |
| | standard deviation | 0.38 | 0.36 | 0.36 | 0.38 | 0.43 |
| | sum of squares | 28.9 | 11.7 | 5.48 | 10.1 | 25.6 |
| GHB, Vincentown, conductance, factor | | x 0.1 | x 0.2 | x 1 | x 5 | x 10 |
| | average absolute | 0.28 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.66 | 5.56 | 5.48 | 5.47 | 5.46 |
| GHB, Hornerstown, ref head, delta ft | | -2 | -1 | 0 | 1 | 2 |
| | average absolute | 0.4 | 0.31 | 0.27 | 0.29 | 0.35 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.37 | 0.38 |
| | sum of squares | 8.92 | 6.46 | 5.48 | 5.97 | 7.94 |
| GHB, Hornerstown, conductance, factor | | x 0.1 | x 0.2 | x 1 | x 5 | x 10 |
| | average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.54 | 5.51 | 5.48 | 5.47 | 5.47 |
| GHB, Mount Laurel-Wenonah, ref head, delta ft | | -2 | -1 | 0 | 1 | 2 |
| | average absolute | 0.62 | 0.39 | 0.27 | 0.33 | 0.56 |
| | standard deviation | 0.44 | 0.38 | 0.36 | 0.38 | 0.43 |
| | sum of squares | 23.2 | 10.4 | 5.48 | 8.35 | 19.1 |
| GHB, Mount Laurel-Wenonah, conductance, factor | | x 0.1 | x 0.2 | x 1 | x 5 | x 10 |
| | average absolute | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 |
| | standard deviation | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 |
| | sum of squares | 5.42 | 5.45 | 5.48 | 5.49 | 5.49 |

Table 8
Summary Dewatering Simulation and Sensitivity Results
Dewatering Rates at Times into Simulation

| | Flow rates (gpm) after start of dewatering simulation | | | | | |
|---------------------------------------|---|-----------|---------|-----------|-----------|----------|
| | 0.48days | 1.06 days | 90 days | 90.6 days | 91.3 days | 365 days |
| Base simulation | 6,541 | 4,613 | 3,434 | 5,477 | 4,989 | 3,566 |
| Sensitivity change to input parameter | | | | | | |
| Halve VT/HT Kh to 5.35 ft/d | 4,449 | 2,149 | 1,955 | 3,062 | 2,421 | 2,223 |
| Double VT/HT Kh to 21.4 ft/d | 8,524 | 6,686 | 5,170 | 7,257 | 6,906 | 5,345 |
| Halve Navesink Kv to 0.0272 ft/d | 6,455 | 4,532 | 3,300 | 5,267 | 4,797 | 3,375 |
| Double Navesink Kv to 0.109 ft/d | 6,617 | 4,685 | 3,549 | 5,631 | 5,135 | 3,727 |
| Decrease Kirwood Kv to 0.001 ft/d | 6,507 | 4,569 | 3,329 | 5,315 | 4,845 | 3,495 |
| Increase Kirkwood Kv to 0.01 ft/d | 6,615 | 4,717 | 3,633 | 5,710 | 5,211 | 3,635 |
| Increase MLW Kh to 15 ft/d | 6,592 | 4,473 | 3,587 | 5,692 | 5,072 | 3,796 |
| Increase VT/HT Kv to 1.0 ft/d | 7,022 | 5,076 | 3,744 | 6,129 | 5,633 | 3,883 |

Abbreviations: gpm - gallons per minute
VT/HT - Vincentown and Hornerstown Formations
MLW - Mount Laurel-Wenonah aquifer
Kh - horizontal hydraulic conductivity
ft/d - feet per day
Kv - vertical hydraulic conductivity

Table 9
Estimated Drawdown at Existing
Structures During Dewatering Activities


| Location | Drawdown - feet | |
|---|-----------------|------------|
| | Alluvium | Vincentown |
| Independent Spent Fuel Storage Installation | 11.5 | 40.8 |
| Fuel Oil Tank | 11.0 | 29.2 |
| Waste Water Treatment Plant | 9.5 | 32.8 |
| Hope Creek Switchyard | 9.3 | 33.0 |
| Learning and Development Center | 6.4 | 17.0 |
| Hope Creek Unit 1 | 4.6 | 17.3 |
| Low Level Rad Waste Building | 2.8 | 13.3 |
| Nuclear Operations Support Facility | 2.3 | 10.9 |
| Salem Units 1 and 2 | 0.9 | 6.4 |

Note: Estimated drawdown taken from groundwater model at or near the center of the structure using the base calibrated model simulation (see Table 8 for simulated pumping rates).

Table 10
Summary Dewatering Simulation and Sensitivity Results
Drawdowns and Heads at Selected Locations

| | | | |
|--|------------------|-----------|-----------|
| Base simulation | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 9.3 | 3.3 | 0.9 |
| Vincentown Formation | 33 | 17.7 | 6.4 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -68.3 | | |
| Halve VT/HT Kh to 5.35 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 7.4 | 2.5 | 0.6 |
| Vincentown Formation | 27.3 | 14.9 | 5.4 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -53.9 | | |
| Double VT/HT Kh to 21.4 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 9.6 | 3.4 | 1 |
| Vincentown Formation | 34.2 | 18.4 | 6.7 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -70 | | |
| Decrease Kv of Navesink to 0.0272 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 9.2 | 3.1 | 0.8 |
| Vincentown Formation | 32.8 | 17.1 | 6 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -70.3 | | |
| Increase Kv of Navesink to 0.109 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 9.3 | 3.4 | 0.9 |
| Vincentown Formation | 33.3 | 18 | 6.5 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -66.7 | | |
| Increase Kh of MLW to 15 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 8.7 | 3 | 0.8 |
| Vincentown Formation | 31.4 | 16.8 | 6.2 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -68.3 | | |
| Increase Kv of VT/HT to 1.0 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 8.7 | 3 | 0.8 |
| Vincentown Formation | 31.4 | 16.8 | 6.2 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -66.7 | | |
| Increase Kv of Kirkwood to 0.01 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 20.7 | 10.2 | 2.7 |
| Vincentown Formation | 31.2 | 16.2 | 5.3 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown Formation | -67.6 | | |
| Decrease Kz of Kirkwood to 0.001 ft/d | | | |
| Drawdown, ft at 365 days | HC Cooling Tower | HC Unit 1 | Salem 1&2 |
| Alluvium | 2.6 | 0.4 | 0.1 |
| Vincentown | 34.7 | 19 | 7.2 |
| Head, ft at 365 days at nuclear island | | | |
| Vincentown | -69.2 | | |

Notes: 1) VT/HT = Vincentown and Hornerstown Formations.
2) MLW = Mount Laurel-Wenonah aquifer
3) Kh = horizontal hydraulic conductivity.
4) Kv = vertical hydraulic conductivity
5) The drawdowns are taken at the approximate centers of the listed structures
6) The head is taken in the upper Vincentown (model layer 5) mid-point in the deeper excavation
7) The nuclear island refers to that of the new unit.

| | | |
|---|---|----------------------------------|
|  | CALCULATION SHEET PSEG SITE ESP APPLICATION PROJECT PROJECT No. 6468-08-2251 | CALC. NO. 2251-ESP-GW-002 |
| | | REV. 0 |
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ATTACHMENT A

Comparison of Observed and Computed Water Level Contours



LEGEND

- ◆ MACTEC Observation Well
- HCGS/SGS Production Well
- ARCADIS Well (B Series)
- Site Boundary
- Interpreted Contours Average of Water Level Data
- Model Computed Heads Contoured by Groundwater Vistas (Calibrated Model)



0 100 200
Meters

0 500 1,000
Feet

PSEG Power, LLC


PSEG Site ESPA
Groundwater Model Calc. Package

Comparison of Observed and
Computed Water Level Contours

FIGURE A-1


Rev 0

Note: Groundwater elevations represent data from average water level set
2251-ESP-GW-002 - Groundwater Model Calc Package

| | | |
|---|---|----------------------------------|
|  | CALCULATION SHEET PSEG SITE ESP APPLICATION PROJECT PROJECT No. 6468-08-2251 | CALC. NO. 2251-ESP-GW-002 |
| | | REV. 0 |
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Attachment B

Model-derived Input/Output Figures

| | | |
|---|--|----------------------------------|
|  | <p style="text-align: center;">CALCULATION SHEET</p> <p style="text-align: center;">PSEG SITE ESP APPLICATION PROJECT PROJECT No. 6468-08-2251</p> | CALC. NO. 2251-ESP-GW-002 |
| | | REV. 0 |
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List of Model-Generated Input/Output Figures

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Figure B-8: Bottom of Layer 6 (Navesink Aquitard) Elevation Contours

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Figure B-14: Zones of Hydraulic Conductivity in Model Layer 1 (Fills)

Figure B-15: Zones of Hydraulic Conductivity in Model Layer 2 (Alluvium)

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Figure B-17: Zones of Net Recharge in the Calibrated Model - Layer 1

Figure B-18: Calibrated Model Piezometric Contours (feet NAVD88) in Model Layer 1

Figure B-19: Calibrated Model Piezometric Contours (feet NAVD88) in Model Layer 2

Figure B-20: Calibrated Model Piezometric Contours (feet NAVD88) in Model Layer 4

Figure B-21: Calibrated Model Piezometric Contours (feet NAVD88) in Model Layer 5

Figure B-22: Calibrated Model Piezometric Contours (feet NAVD88) in Model Layer 7


| | | |
|---|---|----------------------------------|
|  | CALCULATION SHEET PSEG SITE ESP APPLICATION PROJECT PROJECT No. 6468-08-2251 | CALC. NO. 2251-ESP-GW-002 |
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Figure B-23: Plot of Model-Computed Heads (feet NAVD88) versus Observed Water Levels

Figure B-24: Location of Soil Retention Barrier and Simulated Dewatering Wells in Alluvium

Figure B-25: Location of Soil Retention Barriers in Upper Vincentown

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Figure B-28: Contours of Drawdown at One Year of Dewatering in the Alluvium

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Figure B-30: Hydrograph -Simulated Drawdown (Feet Below Static Condition) in the Vicinity of the Hope Creek Cooling Tower in the Alluvium

Figure B-31: Hydrograph - Simulated Drawdown (Feet Below Static Condition) in the Vicinity of Hope Creek Unit 1 in the Alluvium

Figure B-32: Hydrograph - Simulated Drawdown (Feet Below Static Condition) in the Vicinity of Salem Units 1 and 2 in the Alluvium

Figure B-33: Hydrograph – Simulated Drawdown (Feet Below Static Condition) in the Vicinity of the Cooling Tower in the Vincentown

Figure B-34: Hydrograph – Simulated Drawdown (Feet Below Static Condition) in the Vicinity of Hope Creek Unit 1 in the Vincentown

Figure B-35: Hydrograph – Simulated Drawdown (Feet Below Static Condition) in the Vicinity of Salem Units 1 and 2 in the Vincentown

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Figure B-37: Extent of Assumed Breach in the Kirkwood for Assessing Hydrostatic Loading

Figure B-38: Simulated Post-Construction Piezometric Heads (feet NAVD88) in the Alluvium or Structural Fill

Figure B-39: Simulated Post-Construction Piezometric Heads (feet NAVD88) in the Upper Vincentown

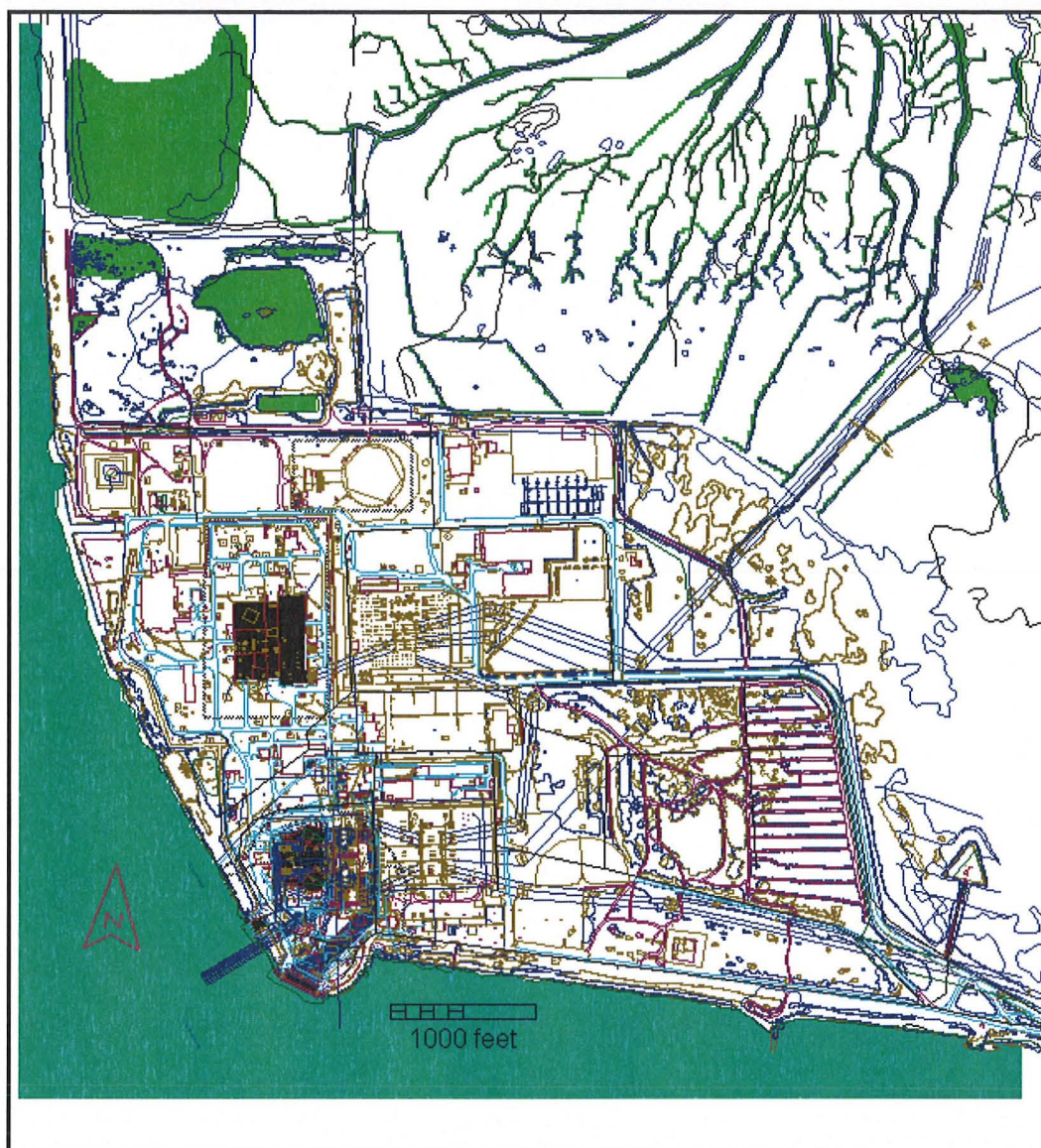


Figure B-1: Model domain and layer 1 boundary conditions. The 20-by-20-foot model grid is not shown as it is relatively fine and obscures the detail in the figure. The model has 376 rows and 350 columns, covering about 1200 acres. The detail in the base map will be deleted in most subsequent figures as it may obscure the focus of certain model outputs and inputs. The green west and south is the Delaware River, represented in the model by MODFLOW river package nodes. The green areas to the north and the northeast represent ponds and wetland areas, again using the MODFLOW river package to represent these boundary conditions. The blackened nodes locate the Hope Creek and Salem units, and use no-flow nodes to indicate impermeable structures into the water table.

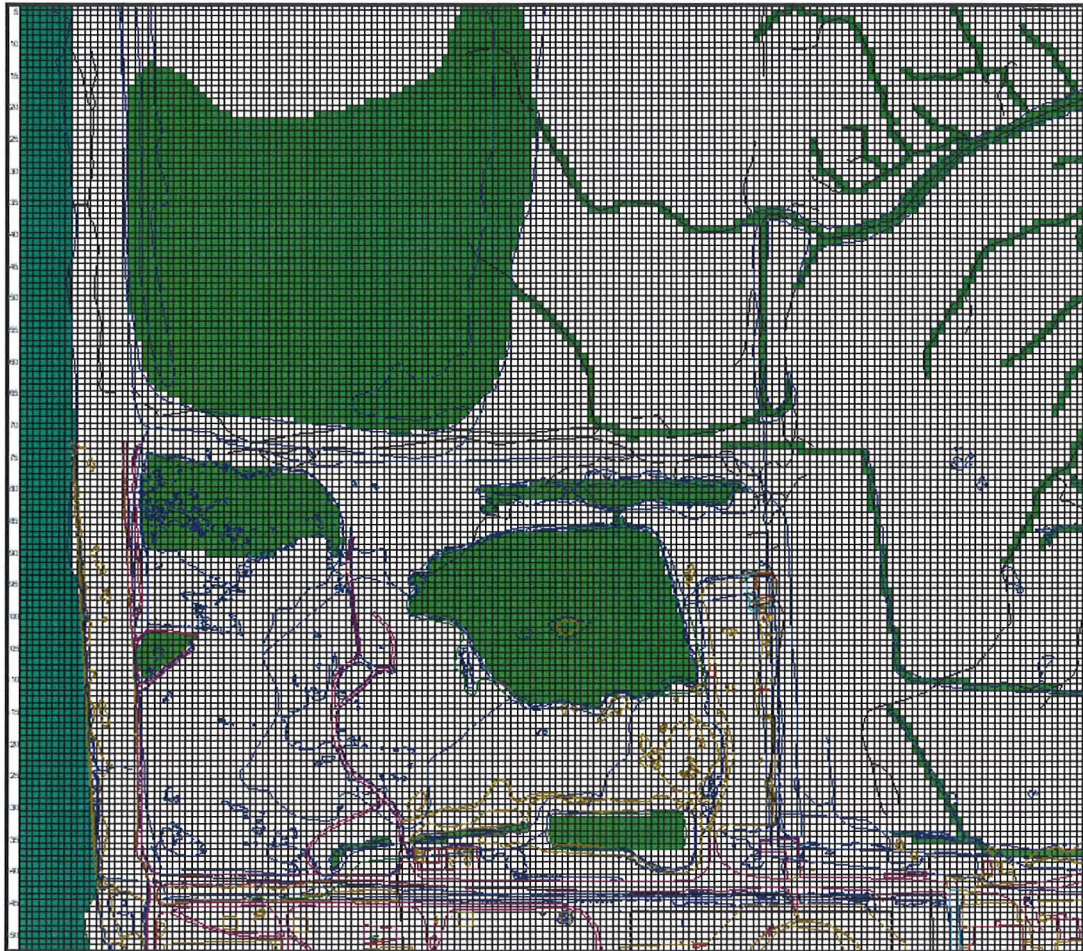


Figure B-2: MODFLOW Model Grid in the New Plant Location. Grid blocks are a uniform 20-by-20 feet, north is upwards. The green to the left represents the Delaware River. Green areas in the area proposed for development are perched ponded areas and associated drainage ways.

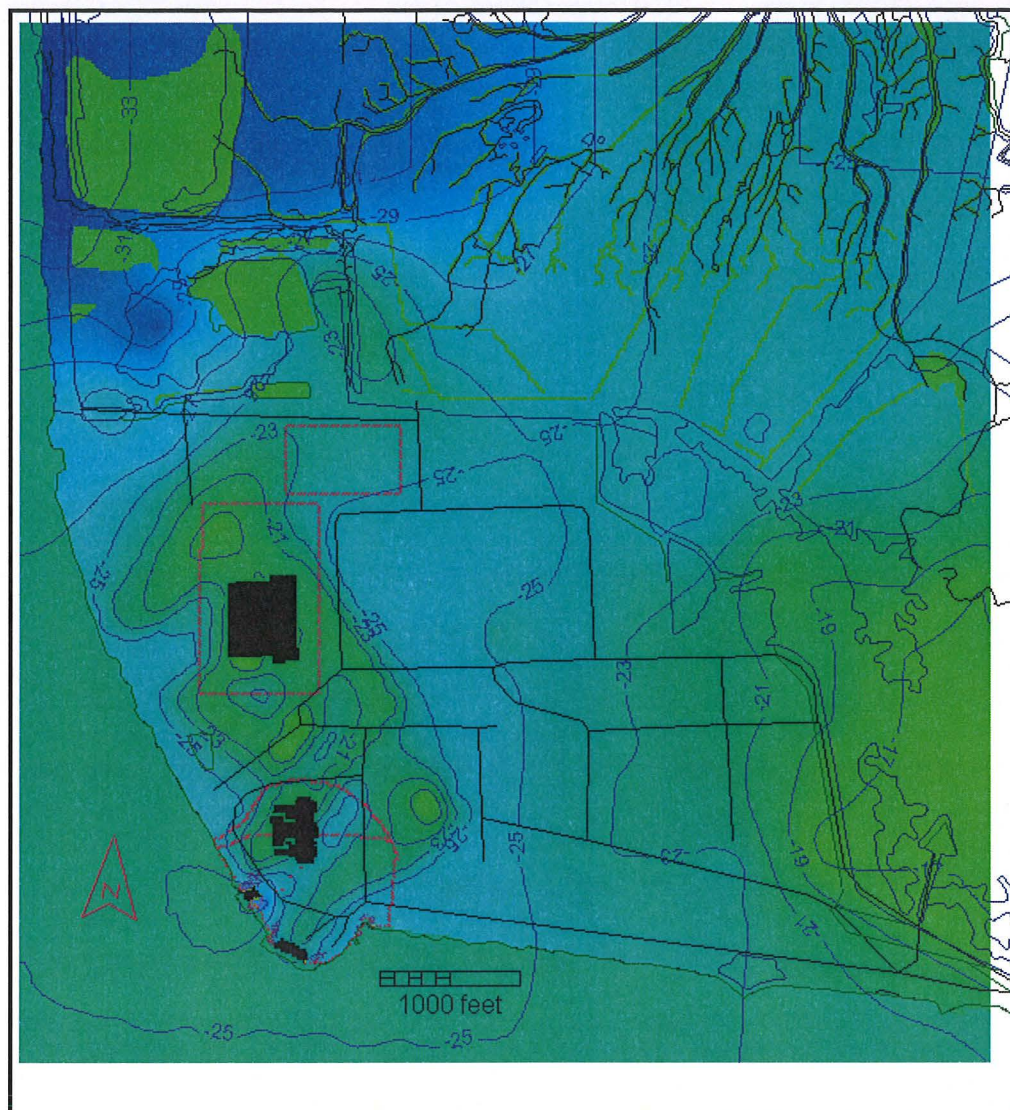


Figure B-3: Bottom of Layer 1 (Fill) Elevation Contours , feet NAVD88 Black areas denote no-flow boundaries in MODFLOW, used to represent impermeable structures extending below the water table through this particular model layer.

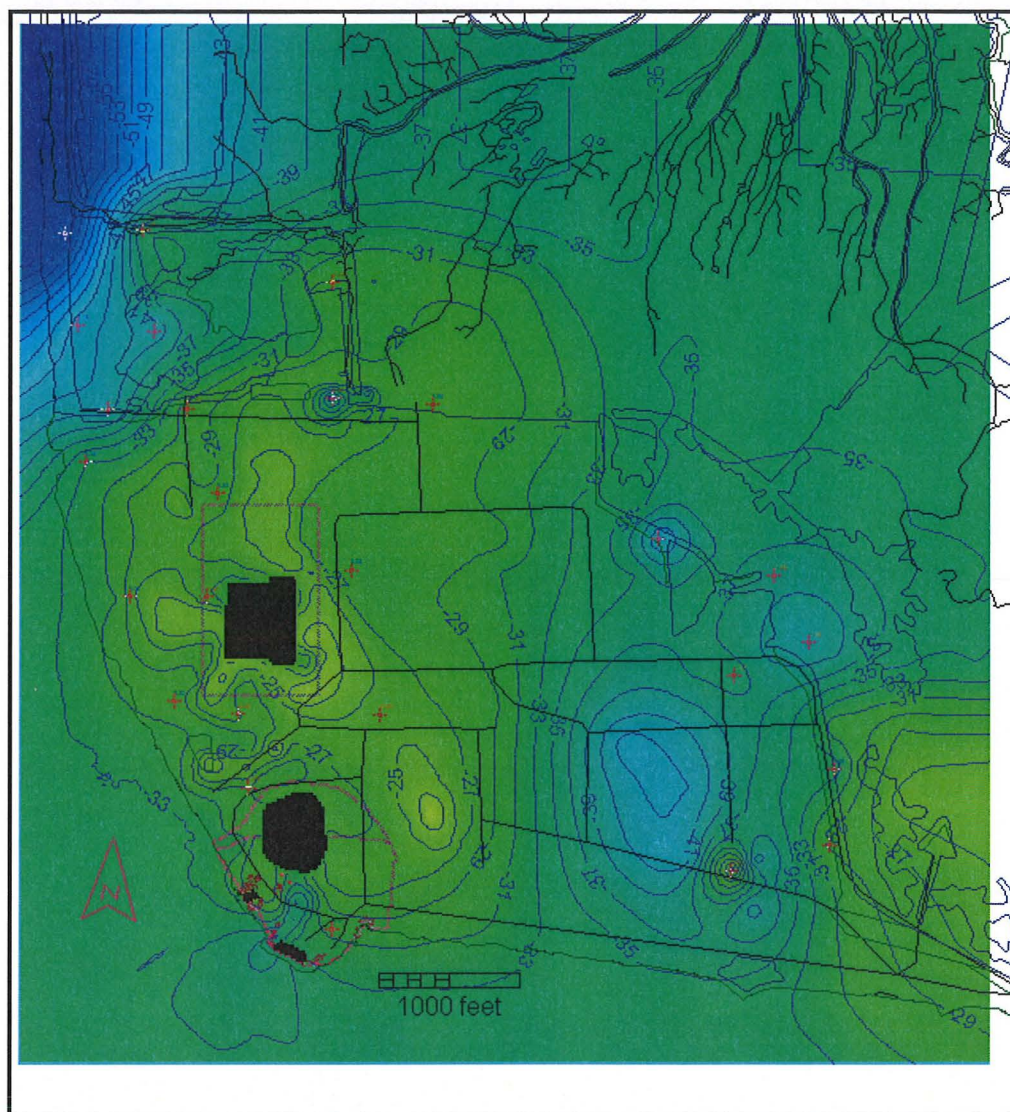


Figure B-4: Bottom of Layer 2 (Alluvium) Elevation Contours, feet NAVD88. Crosses mark locations of water level readings taken in support of the model calibration.

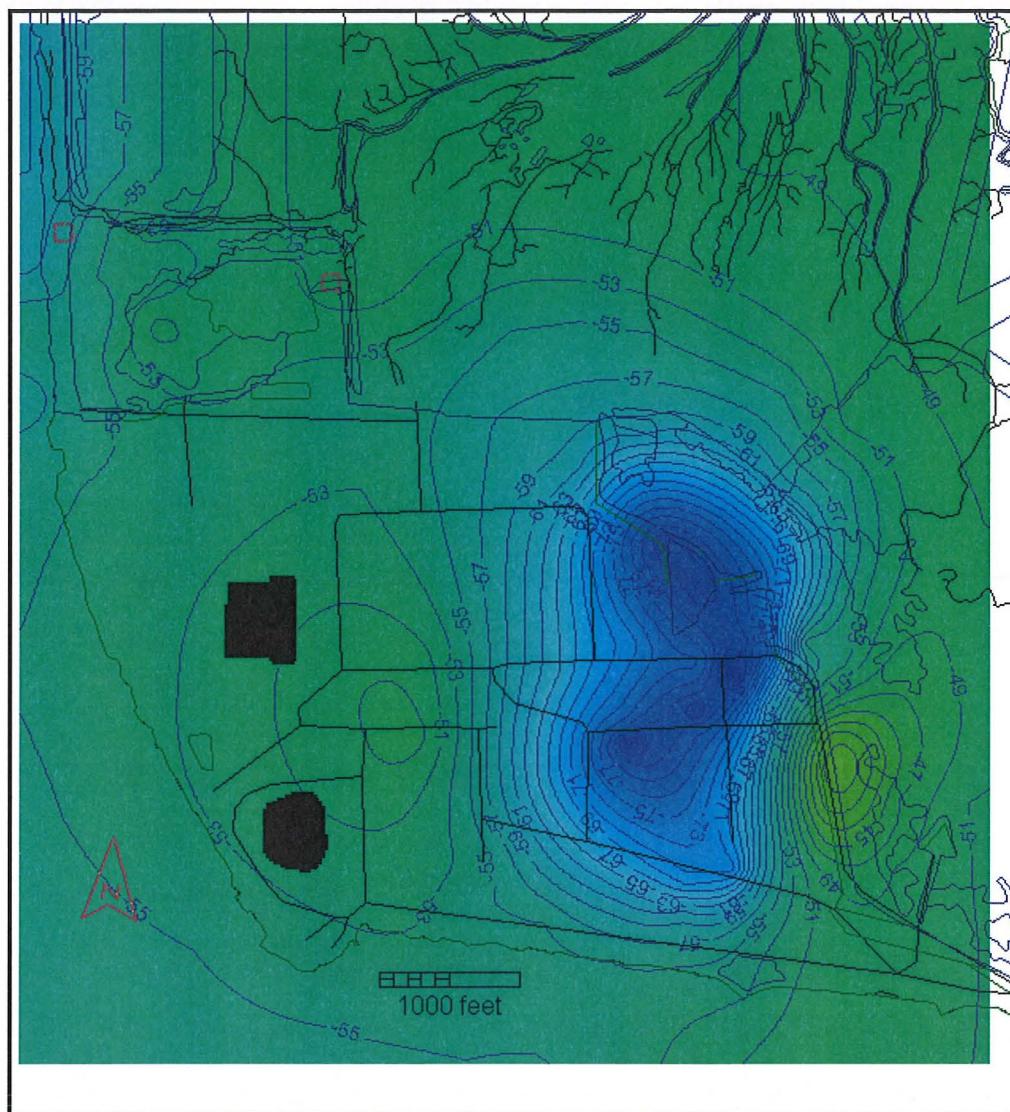


Figure B-5: Bottom of Layer 3 (Kirkwood Aquitard) Elevation Contours, feet NAVD88.

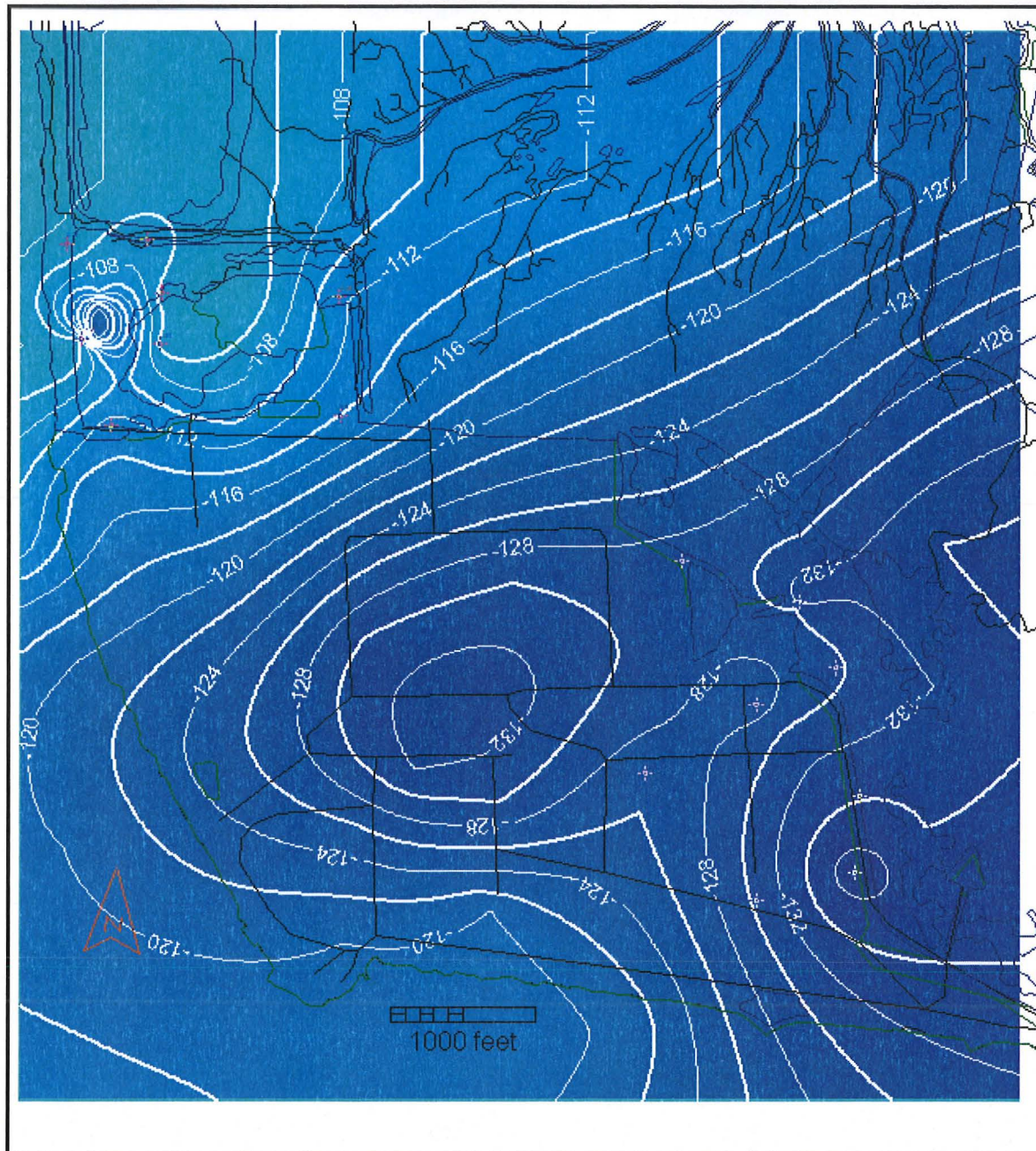


Figure B-6: Bottom of Layer 4 (Vincentown) elevation contours, feet NAVD88. The crosses mark the location of water level data in the formation in support of model calibration.

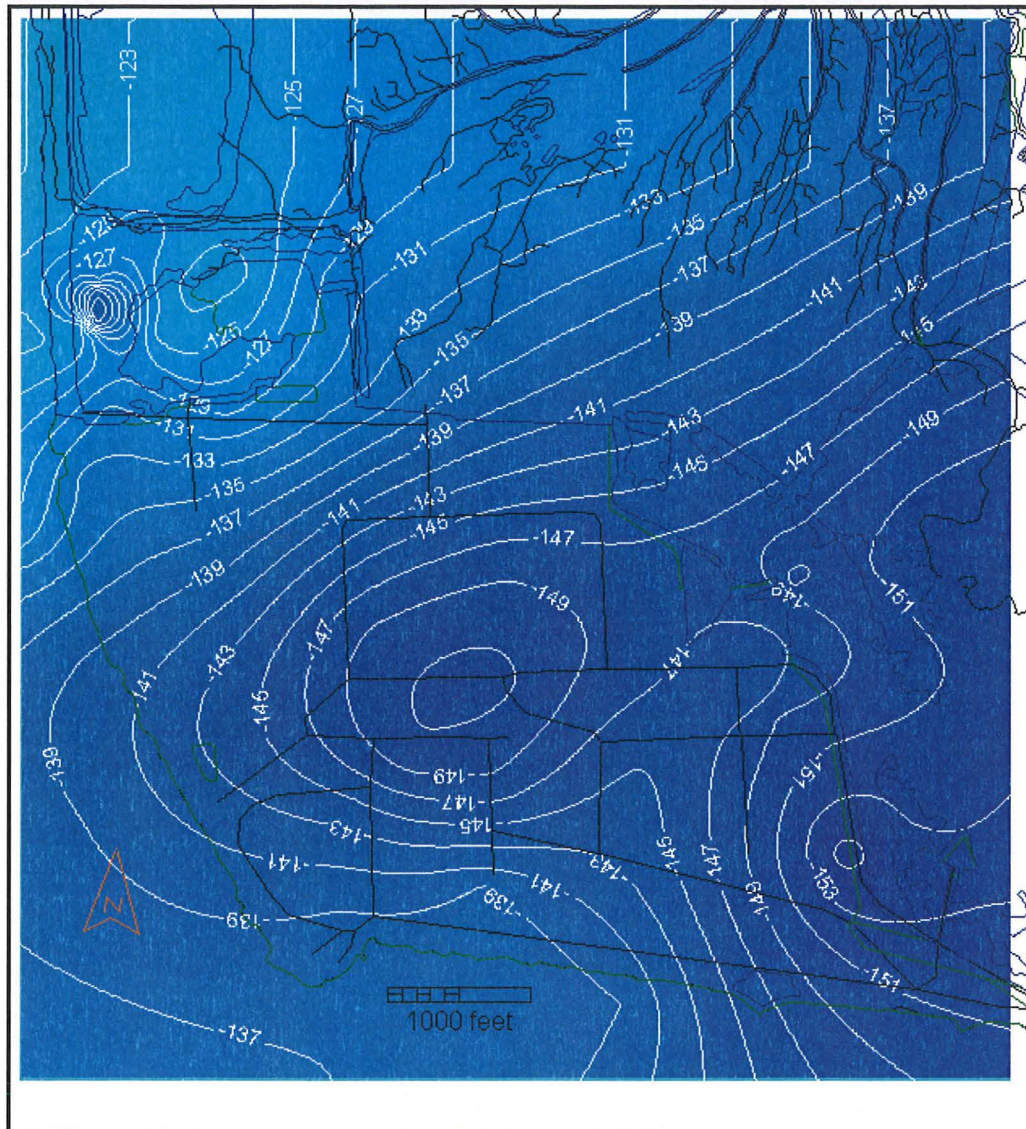


Figure B-7: Bottom of Layer 5 (Hornerstown) elevation contours, feet NAVD88.

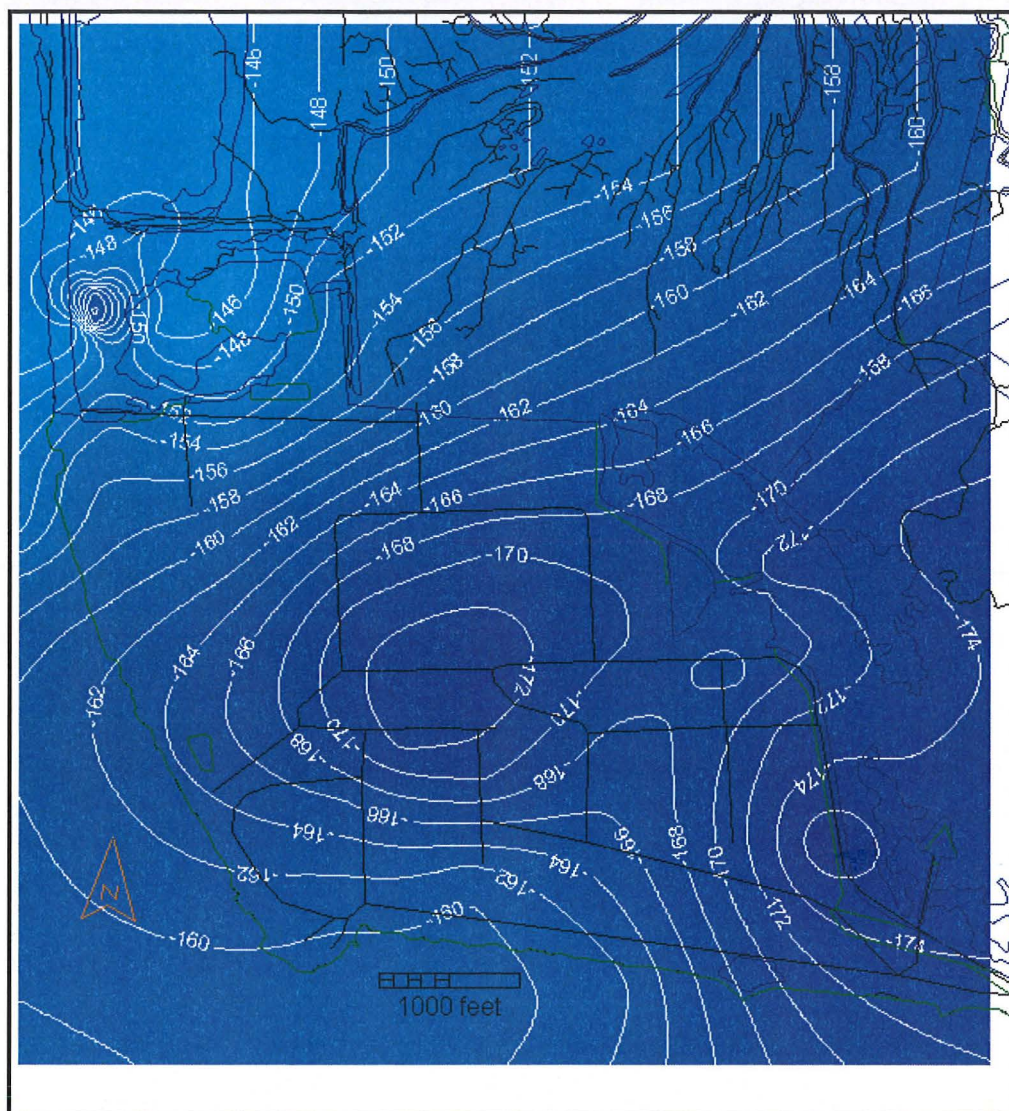


Figure B-8: Bottom of Layer 6 (Navesink Aquitard) elevation contours, feet NAVD88.

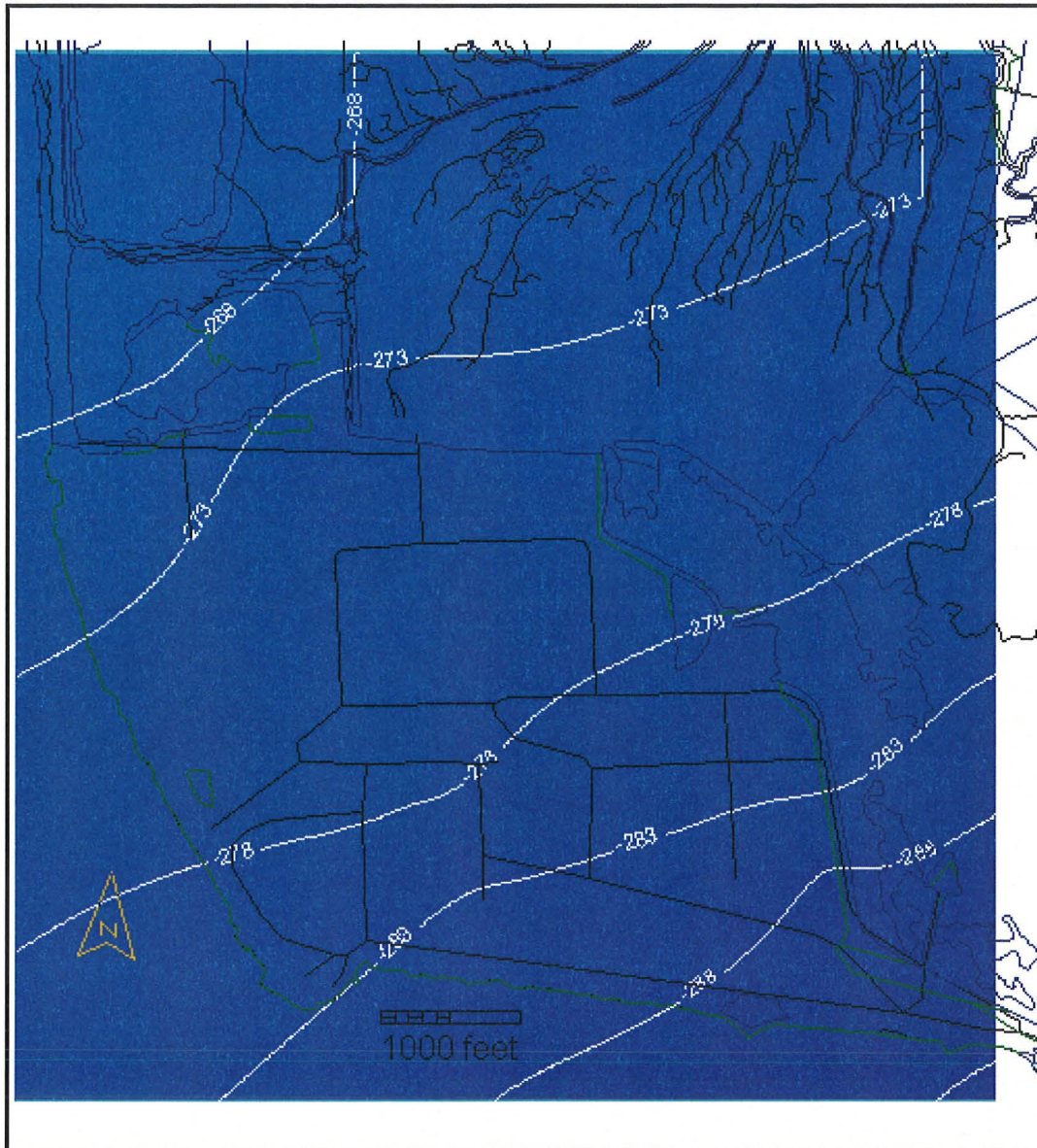


Figure B-9: Bottom of Layer 7 (Mount Laurel-Wenonah) elevation contours, feet NAVD88. This is also the base of the model as the underlying Marshalltown aquitard is thick and competent and is unlikely to allow flow to occur with the next lower aquifer.

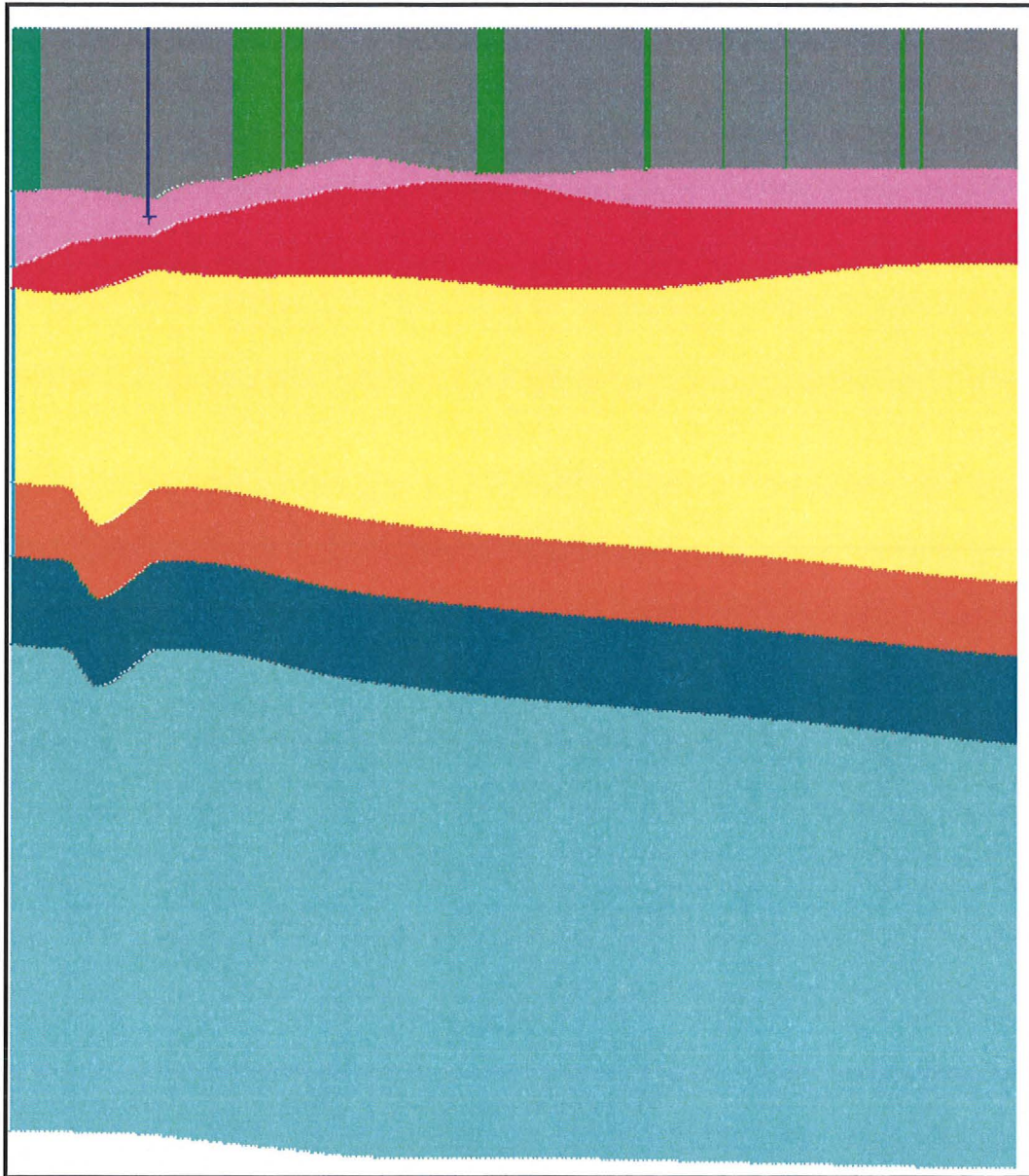


Figure B-10: Section View of model layers along model row 61 and the northern parcel selected for expansion. Starting from the bottom up, the layers represent the Mount Laurel-Wenonah, the Navesink aquitard, the Hornerstown unit, the Vincentown Formation, the Kirkwood aquitard, the alluvium, and the fill materials. Green strips in the fill material layer mark river nodes. The dark blue line in the upper left extending down into the alluvium represents boring NOW-3U.