

3E

Design Details and Critical Sections for Safety-Related Category I Structures

This appendix provides details of structural design and analysis for the critical sections relevant to Seismic Category I structures.

Critical Section Selection Criteria

Critical sections are those portions of individual Seismic Category I structures (i.e., shear walls, floor slabs and roofs, structure-to-structure connections), which are particularly important for prevention or mitigation of consequences of postulated design basis accidents, are expected to experience the largest structural demands during design basis conditions, or are needed for safety evaluation of an essentially complete design.

Some selected critical sections may also be typical of other portions of the structure, where the portions they typify are not identified as critical sections due to their strong similarities with the selected critical sections. Critical sections are analytically representative of an essentially complete U.S. EPR design, and their structural design adequacy provides reasonable assurance of overall U.S. EPR structural design adequacy.

U.S. EPR critical section selection is characterized by a three-tier process that includes qualitative, quantitative, and supplementary methodologies to reasonably assure completeness and consistency across each structure. The critical sections for the Seismic Category I structures, are shown in the following tables:

- Table 3E.1-41—Nuclear Island Critical Sections.
- Table 3E.2-9—Emergency Power Generating Building Critical Sections.
- TTable 3E.3-10—Essential Service Water Building Critical Sections.

Qualitative Methodology

The qualitative methodology is applied to portions of the U.S. EPR nuclear island (NI) Common Basemat Structures that are credited in the risk mitigation of the nuclear power plant under design basis loading conditions to provide protection of public safety through the physical plant boundaries. Due to a safety-critical role, some of the U.S. EPR NI Common Basemat Structures are required to achieve major performance requirements for functions whose failures could degrade system or equipment performance of the U.S. EPR design or pose a safety hazard to plant personnel or to the general public. In this regard, they are considered critical structures or critical sections. Unique engineered features in each of the structures can be further broken into portions (e.g., cylindrical walls, liner plates, dome, and dome ring areas of the Reactor Containment Building (RCB)) that are defined as critical sections.

Critical sections identified by the qualitative methodology are:

- RCB—typical liner plate.
- RCB—typical cylinder wall and buttress.
- RCB—typical dome and dome ring areas.
- RCB—typical connection of containment wall to NI basemat.
- RCB—equipment hatch area.
- RBIS—typical primary shield wall/reactor vessel support area.
- NI basemat, including tendon gallery, and RBIS baseslab.
- Fuel Building (FB) internal structures—typical spent fuel pool walls and floor slab.
- RCB—typical airlock and main steam and feedwater (MS/FW) penetrations.
- FB Internal Structures—fuel transfer tube.

Quantitative Methodology

ANSYS, V 11.0 SP1 is used to create a finite element analysis model of the U.S. EPR NI Common Basemat Structures. This model incorporates numerous finite element types to represent the NI geometry.

The quantitative methodology identifies critical sections by analysis of force and moment results extracted from portions of the global static model not already defined as critical sections by the qualitative methodology. Element forces and moments are extracted from the finite element model (FEM) and sorted for each force or moment type using a series of ANSYS macros to identify elements that have maximum and minimum force demand (minimum being the largest negative forces and moments). The quantitative selection methodology identifies critical sections as follows:

- Specific NI Common Basemat Structures to be reviewed are identified.
- Applicable load combinations are identified.
- Element forces and moments per unit length (i.e., T_X , T_Y , T_{XY} , M_X , M_Y , M_{XY} , N_X , N_Y), along with element centroids in the global Cartesian coordinate system, are obtained from the specified load combinations for all elements in the applicable buildings.
- The elements for walls and slabs are sorted in ascending order by force and moment type.

- For each load combination, elements with the maximum and minimum force demands are identified for walls and slabs. This process is repeated for the selected load combinations and results are combined to produce a subset of potential critical elements.
- The subset of potential critical elements is further refined by eliminating duplicate elements for each element type. The final subset contains only unique controlling critical elements.
- Critical element locations are plotted at their respective elevations for each building. XY scatter plots of controlling critical elements are produced for each elevation containing critical elements and are generated at 10 percent intervals from 100 percent of unique critical elements down to 10 percent. As the percentage of the critical elements with highest forces and moments plotted is decreased, limiting critical elements become apparent. Final identification of critical sections is performed based on the maximum value of load type and frequency.

Critical sections identified by the quantitative methodology are:

- Reactor Building Internal Structure (RBIS)—typical operating floor slab area.
- RBIS—typical elevation +4 feet, 11 1/16 inches heavy slab and support walls.
- RBIS—typical steam generator (SG) cubicle area walls and slabs.
- Safeguard Building (SB) 2/3 hardened shell—typical walls from top of NI basemat to grade.
- SB 2/3 internal structures—typical exterior walls from top of NI basemat to elevation +15 feet, 5 inches.
- SB 2/3 internal structures—typical floor slab at elevation -16 feet, 5 inches and 0 feet, 0 inches.
- SBs 1 and 4—typical main steam and feedwater valve room walls and slabs.
- SBs 1 and 4—typical exterior walls from top of NI basemat to elevation +15 feet, 5 inches.
- FB hardened shell—typical walls from top of NI basemat to elevation +12 feet, 0 inches.
- FB internal structures—typical major walls from top of NI basemat to bottom of spent fuel pool slab.
- FB internal structures—typical spent fuel pool walls and floor slab.
- Reactor Shield Building (RSB)—typical wall areas and connection between RSB wall and SB/FB roof slabs.

- RSB—typical dome-to-wall transition areas.
- FB hardened shell—typical roof slab areas for material lock room & support walls.
- FB internal structures—typical floor slab above spent fuel pool area & support walls.
- RSB—lower portion typical wall areas.

Note that spent fuel pool walls and slab were identified as critical sections by both the qualitative and quantitative methodologies.

Supplementary Methodology

In addition to the critical sections identified by the qualitative and quantitative methodologies, there are other portions or sections of the plant that may be safety-related but are not explicitly modeled and considered in the U.S. EPR static finite element model. Seismic Category I structures that perform safety-related functions are reviewed to determine which structural sections are not otherwise selected by either the quantitative or qualitative method. Once these sections are determined, engineering judgment is applied to assess whether they should be identified as critical sections. Critical sections selected using this method are supplementary critical sections.

Supplementary critical sections also include sections that constitute significant portions of the Seismic Category I structures in terms of their physical dimensions (i.e., wall and slab areas). Although these sections are not subject to the limiting structural demands of quantitatively-defined critical sections and can be considered less critical, they are necessary to represent an essentially complete design of each structure and provide reasonable assurance of U.S. EPR design adequacy. This is a significant consideration because quantitatively-determined critical sections represent only those portions of a structure that experience high loads or stress and may not identify intervening structural elements that are not subject to high stress or loading but are needed for evaluating structural functionality.

Because potential supplementary critical sections exist throughout the U.S. EPR design, spatial distribution and significant structural discontinuities are also important factors in supplementary critical sections selection.

Critical sections identified by the supplementary methodology are:

- NI—typical columns and beams.
- NI—Vent Stack.
- Emergency Power Generating Buildings (EPGB)—basemat foundation at elevation 0 feet, 0 inches.

- EPGB—shear wall on column line 11.
- EPGB—reinforced concrete slab and composite beams at elevation 51 feet, 6 inches.
- EPGB—shear wall on column line C.
- EPGB—shear wall on column line E.
- Essential Service Water Buildings (ESWB)—basemat foundation at elevation -16 feet, 0 inches.
- ESWB—shear wall at column line 4.
- ESWB—fan deck slab at elevation 63 feet, 0 inches.
- ESWB—shear wall on column line D.

Information is presented for the NI Common Basemat Structures (3E.1), EPGB (3E.2), and ESWB (3E.3).

The following information is provided:

- Description of the critical section.
- Applicable loadings and design method.
- Results of structural analysis.

A COL applicant that references the U.S. EPR design certification will address critical sections relevant to site-specific Seismic Category I structures.

3E.1 Nuclear Island Structures

Description of Critical Sections in Nuclear Island Structures

The critical sections presented in this section are structures supported on the NI Common Basemat. This includes the Reactor Containment Building containing the Reactor Building Internal Structure, the Fuel Building, Safeguard Building 1, 2, 3, and 4 and the Reactor Shield Building.

The RCB is located inside of the reinforced concrete RSB, and is separated by an annular space to protect against interaction of the two structures when subjected to postulated design basis loading conditions. Figure 3.8-2, Figure 3.8-3, and Figure 3.8-4 show arrangements of the RCB. Figure 3.8.1.1 provides a description of the RCB.

The critical sections relating to the RCB consist of the following major structural elements:

- Reactor Containment Building—Wall to foundation connection (3E.1.1).
- Reactor Containment Building—Equipment hatch area (3E.1.2).
- Reactor Containment Building—Typical cylinder wall and buttress (3E.1.3).

The RB Internal Structures consist of concrete walls and floors, steel framing members, and other concrete and steel structural elements that are located inside of the RCB. Figure 3.8-32, Figure 3.8-33, Figure 3.8-34, Figure 3.8-35, Figure 3.8-36, and Figure 3.8-37 show arrangements of the RB Internal Structures. Section 3.8.3.1 provides a description of the RB Internal Structures.

The critical sections relating to RB Internal Structures consist of the following major structural elements:

- Reactor Building Internal Structures—Steam generator and reactor coolant pump support and typical cavity wall (3E.1.4).
- Reactor Building Internal Structures—Pressurizer support and typical cavity wall (3E.1.5).
- Reactor Building Internal Structures—Operating floor area (3E.1.6).

The RSB completely encloses the RCB, and is connected to the external walls of SB 2 and 3 and the FB. Figure 3E.1-11 shows the arrangement of the RSB and RB Annulus. Section 3.8.4.1.1 provides a description of the RSB.

- The critical section relating to the RSB is the connection of FB and SB 2 and 3 Roofs to RSB Wall (3E.1.7).

The SBs are comprised of four buildings connected around the periphery of the RSB. Figure 3.8-53, Figure 3.8-54, Figure 3.8-55, Figure 3.8-56, Figure 3.8-57, Figure 3.8-58, Figure 3.8-59, Figure 3.8-60, Figure 3.8-61, Figure 3.8-62, and Figure 3.8-63 show the arrangement of SB 1; Figure 3.8-64, Figure 3.8-65, Figure 3.8-66, Figure 3.8-67, Figure 3.8-68, Figure 3.8-69, Figure 3.8-70, Figure 3.8-71, Figure 3.8-72, Figure 3.8-73, and Figure 3.8-74 show the arrangement of SBs 2 and 3; and Figure 3.8-75, Figure 3.8-76, Figure 3.8-77, Figure 3.8-78, Figure 3.8-79, Figure 3.8-80, Figure 3.8-81, Figure 3.8-82, Figure 3.8-83 show the arrangement of SB 4. Section 3.8.4.1.3 provides a description of the SBs.

- The critical section relating to the SBs is external walls below grade level (3E.1.8).

Section 3.8.5.1.1 provides a description of the NI foundation basemat. The NI foundation basemat is a cruciform-shaped, heavily-reinforced concrete slab that supports all NI Common Basemat Structures. Figure 3.8-103 shows the NI foundation with the RB Internal structures base slab.

- The critical section relating to the NI foundation basemat is the foundation of NI Buildings and base slab of the RB Internal structures (3E.1.9).

Materials

Concrete – conforms to the requirements specified in Sections 3.8.1.6.1, 3.8.3.6.1, 3.8.4.6.1, and 3.8.5.6.1 as summarized below:

	Compressive Strength f_c (psi)	Modulus of Elasticity (ksi)	Shear Modulus (ksi)	Poisson's Ratio
Reactor Building	7000	4769	2038	0.17
RB Internal Structures	6000	4415	1887	0.17
Other Seismic Category I Structures ⁽¹⁾	6000	4415	1887	0.17
Foundations	4000	3605	1541	0.17

Notes:

1. EPGB and ESWB concrete material requirements, except foundations, are included in Appendix 3E.2 and 3E.3, respectively.

Reinforcing Steel – deformed steel bars conforming to ASTM A615 Grade 60 with minimum yield strength of $F_y = 60$ ksi, and minimum tensile strength $F_u = 90$ ksi. Minimum bar elongation in 8 inches is based on ASTM A615: 9 percent for bar No. 3 to 6; 8 percent for bar No. 7 and 8; and 7 percent for bar No. 9, 10, 11, 14, and 18.

Structural Steel – conforms to the requirements specified in Section 3.8.1.6.4, Section 3.8.3.6.3, and Table 3.8-8. Liner plate material conforms to SA-516, Grades 55 to 70, with minimum yield strength $F_y = 30$ -38 ksi, and minimum tensile strength $F_u = 55$ -90 ksi.

Floor Live and Dead Load Distribution

Dead and live floor loads are tabulated for each room in the Nuclear Island Common Basemat Structure. Loads are uniformly distributed and/or point loads with associated load application coordinates. Point loads less than 20 kips are accounted for in the uniformly distributed equipment dead and live loads, and loads over 20 kips are applied as point loads. In cases where equipment point loads less than 20 kips are applied to grating, loads are distributed over the area of the grating. Distributed dead and live loads are determined for each room. Concrete self weight is based on concrete density of 150 pcf. Precipitation loads are given in Table 2.1-1 and are not included in the floor live loads. Pool loads are also determined separately.

The following distributed loads are applied to the floors of the Safeguard Buildings:

- Uniformly distributed slab load (live) = 125 psf.
- Uniformly distributed slab load (dead) = 125 psf.
- Uniformly distributed grating load (live) = 175 psf.
- Uniformly distributed grating load (dead) = 25 psf.
- Uniformly distributed wall load (dead) = 25 psf.

The following distributed loads are applied to the floors of the Fuel Building:

- Uniformly distributed slab load (live) = 500 psf.
- Uniformly distributed slab load (dead) = 250 psf.
- Uniformly distributed grating load (live) = 175 psf.
- Uniformly distributed grating load (dead) = 25 psf.
- Uniformly distributed wall load (dead) = 25 psf.

The following distributed loads are applied to the floors of the Reactor Building:

- Uniformly distributed slab load (live) = 500 psf.
- Uniformly distributed slab load (dead) = 250 psf.
- Uniformly distributed grating load (live) = 175 psf.
- Uniformly distributed grating load (dead) = 25 psf.
- Uniformly distributed wall load (dead) = 50 psf.

Table 3E.1-38 provides the distribution of the dead and live floor loads in each building.

Equipment Loads

The weight of major equipment, switchgears, heat exchanges, pumps, conduits, exhaust, pipes, elevators, fans, tanks, filters, and cranes is accounted for in the floor loads and is applied as point load throughout the buildings based on location. Some of the major equipment loads are as follows:

Equipment	Elevation/Location	Weight (kips)
Crane SMF03	UFA29-090	137
Crane SMF02	UFA10-069	407
Polar crane	UJA40-001	1180
Reactor pressure vessel (RPV)	4'-11"	3768
Steam generator (SG)	4'-11"	1776 each

Reactor coolant pump (RCP)	4'-11"	474 each
Pressurizer (PZR)	UJA23-019	639
Equipment hatch	64'-0"	154
Air lock	4'-11"; 64'-0"	61 each

Foundation Stability

The Nuclear Island Common Basemat Structure is evaluated for stability against overturning, sliding, and floatation for the soil profiles used in establishing the certified plant design. The tendon gallery is used as a shear key. The minimum factors of safety for the Nuclear Island Common Basemat Structure are listed in Table 3E.1-39. The calculated factors of safety against overturning, sliding, and floatation satisfy the acceptance criteria.

The sliding and overturning factors are determined using load combination containing dead load (D), lateral earth pressure (H), SSE (E'), hydrostatic load (F) and buoyant force (Fb). It is conservatively assumed that the E' and Fb occur simultaneously. The floatation factor of safety is determined based on dead load (D) and buoyant force (Fb). The dead load used in the analysis includes 25 percent of the live load, which is consistent with the generation of dynamic soil pressure due to SSE.

The static and dynamic bearing pressures for the Nuclear Island Common Basemat Structure are listed in Table 3E.1-40.

Design Criteria

Sections 3.8.1.2, 3.8.2.2, 3.8.3.2, 3.8.4.2, and 3.8.5.2 describe codes, standards, and specifications applicable to the design of the RCB (Concrete), RCB (Steel), RB internal structures, RSB, and NI foundation basemat, respectively.

A global ANSYS Finite Element Model (FEM) (addressed in Sections 3.8.1.4.1, 3.8.3.4.1, 3.8.4.4.2, and 3.8.5.4.2) is developed and loaded with various independent loads and load combinations per the applicable codes and standards and solved to produce forces and moments throughout the structure. Sections 3.8.1.3, 3.8.2.3, 3.8.3.3, 3.8.4.3, and 3.8.5.3 describe loads and loading combinations applicable to the design of NI Common Basemat Structures. The design includes the soil cases shown in Table 3.7.1-6.

The independent loads shown in Table 3E.1-1—Independent Loads Considered in the FEM, are applied to the NI common basemat global ANSYS FEM to analyze and evaluate the overall structural response of the NI Common Basemat Structures as described in Section 3.8.1.4, 3.8.2.4, 3.8.3.4, 3.8.4.4, and 3.8.5.4. Additional loads shown in Table 3E.1-2—Independent Loads Not Considered in the FEM, and addressed in Sections 3.8.1.3, 3.8.2.3, 3.8.3.3, 3.8.4.3, and 3.8.5.3 are not considered by

the ANSYS FEM and are independently added and analyzed for in the design process for completeness.

Results from the global ANSYS analysis provide shell element forces and moments in accordance with Figure 3E.1-1—ANSYS Analysis Results for Nuclear Island Elements. Forces and moments shown in Figure 3E.1-1 are defined as:

T_x = axial or membrane load in x-direction (kips/foot).

T_y = axial or membrane load in y-direction (kips/foot).

T_{xy} = in-plane shear load (kips/foot).

N_x = out-of-plane shear load along y-axis of element (kips/foot).

N_y = out-of-plane shear load along x-axis of element (kips/foot).

M_x = bending moment about y-axis through element (kip-feet/foot).

M_y = bending moment about x-axis through element (kip-feet/foot).

M_{xy} = twisting moment (kip-feet/foot).

3E.1.1 Reactor Containment Building—Wall to Foundation Connection

This critical section presents the structural design of the reinforced concrete containment gusset (cylindrical containment wall to foundation connection) section located between elevations -36 ft – 5 in and -7 ft – 6 ⁹/₁₆ in of the containment shell. The gusset section lies between a radius of approximately 68 ft – 7 ¹³/₁₆ in and 91 ft – 2 ¹/₂ in from the center of the RCB. The RCB is a post-tensioned reinforced concrete structure with a steel liner supported on a non post-tensioned reinforced concrete basemat. The RCB is a safety-related, Seismic Category 1 structure, as described in Section 3.8.1.

Description of the Critical Section and Computer Model

The gusset section is located at the base of the cylindrical RSB and RCB walls and is at the perimeter of the Containment basemat. The gusset is shaped as an annular ring and connects the walls to the basemat. Forces and moments are transferred from the walls of the building structures through the gusset and into the foundation basemat. A cross-section of the gusset is shown in Figure 3E.1-2—Gusset Section of RCB and Figure 3E.1-3—Cross-Section of Gusset.

The design of the reinforced concrete gusset section initiates with the FEM described in Section 3.8.1.4.1. The gusset section portion of the ANSYS FEM is constructed from

solid (Solid45) elements. The typical element dimension is 3 ft – 3 3/8 in, and multiple layers of elements are used throughout the RCB ANSYS FEM.

An approximately 180° segment (Azimuth 270° to Azimuth 90°) of the FEM gusset is shown in Figure 3E.1-4—180° FEM Gusset Segment of Containment Foundation.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configuration for the concrete gusset section of the RCB using forces and moments generated from a FEM of the NI Common Basemat Structures. The design of the containment gusset is performed using hand calculations utilizing the applicable codes, standards, and specifications for the RCB as described in Section 3.8.1.2.

Loads applied to the concrete gusset section of the RCB are described in Section 3.8.1.3.1. Additional loads are generated due to the physical configuration and direct interaction of the RB Internal Structure with the RCB foundation and gusset shown in Figure 3E.1-2, because shear is transferred from the RB Internal Structure into the gusset by bearing. The maximum lateral thrust is 150,000 kips.

A separate analysis was performed to estimate the effects of cracked concrete. Based on the results of this analysis, the thermal moments carried by the portions of the RCB were reduced.

All load combinations applied to the gusset section of the RCB are described in Section 3.8.1.2. This section is also designed for all soil analysis cases shown in Table 3.7.1-6—Soil Profiles for the U.S. EPR Standard Plant - NI Common Basemat Structures SSI Analysis Cases.

Results of Critical Section Design

The gusset section is the transition component between the RCB and RSB walls and the RCB basemat. Table 3E.1-3—Summary of Governing Design Data for the Wall to Foundation Connection shows the governing forces and moments from the applied loads. The results are divided into two main sections of the gusset: primary gusset between elevations -36 ft – 5 in and -14 ft – 1 5/16 in and upper gusset between elevations -14 ft – 1 5/16 in and -7 ft – 6 9/16 in. In general, the load combination that includes: Dead (D) + Live (L) + Post-tension (J) + Relief Valve (G) + Accidental Pressure (P_a) + Accidental Temperature (T_a) + Earthquake (E') + Accident Pipe Reaction (R_a) + Pipe Break (R_r) load controls the design of the gusset section.

The gusset section is designed for the resultant forces and moments determined based on the applicable range of applied loading and soil conditions. It should be noted that

the design maximum or minimum, as appropriate, forces and moments may not occur at the same location and may not be from the same load combination and soil analysis case.

Table 3E.1-4—Summary of Typical Gusset Reinforcement summarizes the reinforcement provided to meet the area of steel required for the associated direction and given forces and moments.

The typical reinforcing pattern described in Table 3E.1-4 is shown in Figure 3E.1-5—Gusset Section - Typical Reinforcement.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.2 Reactor Containment Building—Equipment Hatch Area

This critical section presents the structural design of the reinforced concrete section around the equipment hatch of the RCB, which is located between elevations 48 ft – 6 ¹¹/₁₆ in and 103 ft – 4 ³/₁₆ in and between azimuths 126° and 174°. The RCB is a post-tensioned reinforced concrete structure with a steel liner and is a safety-related, Seismic Category 1 structure, as described in Section 3.8.1.

Description of the Critical Section and Computer Model

The equipment hatch area is located in the RCB wall with the post-tensioned tendons routed around the opening. Figure 3E.1-6—Plan View of Equipment Hatch Area, shows a plan view of the equipment hatch area of the RCB. FEM views of the equipment hatch area are shown in Figure 3E.1-7—FEM of Equipment Hatch Area - Outer View and Figure 3E.1-8—FEM of Equipment Hatch Area - Inner View. Figure 3E.1-9—Cross-Section of Equipment Hatch Area, shows a cross section view of the equipment hatch area.

The concrete section around the equipment hatch area of the RCB is divided into sub-sections and the required reinforcement in each subsection is investigated using ANSYS sub-modeling techniques, which are described in Section 3.8.1.4.1. An elevation view of the equipment hatch area with the sub-sections used to determine the reinforcement configuration is shown in Figure 3E.1-10—Elevation View of Equipment Hatch Area Showing Cuts.

The ANSYS FEM constructed for the equipment hatch area is modeled using solid (Solid92) elements, which are 3D, ten-node tetrahedrons having three degrees of freedom at each node.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configuration for the concrete section around the equipment hatch of the RCB structure, using an ANSYS FEM. All applicable codes, standards, and specifications for the RCB are used for the design of the equipment hatch area, as described in Section 3.8.1.2.

A separate analysis was performed to estimate the effects of concrete cracking on thermal moments. Based on the results of this analysis, the thermal moments carried by the portions of the RCB were reduced.

Loads applied to the equipment hatch area of the RCB are described in Section 3.8.1.3.1. All load combinations applied to the equipment hatch area of the RCB are described in Section 3.8.1.3.2. This section is also designed for all soil analysis cases shown in Table 3.7.1-6.

The equipment hatch area of the RCB is equally divided into 168 sub-sections (42 sub-sections per quadrant about the center line of the equipment hatch), which are dimensioned at approximately 5 ft – 0 in by 5 ft – 0 in as shown in Figure 3E.1-10.

Results of Critical Section Design

A summary of the governing design data for factored loads is shown in Table 3E.1-5—Governing Design Data for the Equipment Hatch Area (Factored Loads); a summary of the governing design data for service loads is shown in Table 3E.1-6—Governing Design Data for the Equipment Hatch Area (Service Loads). The typical reinforcement summary for the equipment hatch area is shown in Table 3E.1-7—Summary of Typical Reinforcement for the Equipment Hatch Area.

The vertical and horizontal cuts are identified in Figure 3E.1-10. The typical reinforcement sketches for the equipment hatch area are shown for section cuts 1-1, 2-2, and 3-3, in Table 3E.1-12—Governing Design Data for Radial Shear Design (Typical RCB Wall Section), Table 3E.1-13—Reinforcing Summary (Typical RCB Wall Section), and Figure 3E.1-14—Reinforcement Pattern for Section 3-3 of the Equipment Hatch Area, respectively. The locations of section cuts 1-1 and 2-2, and 3-3 are shown in Figure 3E.1-11—Sections 1-1 and 2-2 of the Equipment Hatch Area and Figure 3E.1-12, respectively.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.3 Reactor Containment Building—Typical Cylinder Wall and Buttress

This critical section presents the structural design of several portions of the RCB containing a typical wall section, typical section through a wide buttress (width ≈ 19 ft – 8 in), and typical section through a narrow buttress (width ≈ 13 ft – 1 in). The wall sections are designed to be applicable to any typical portion of the RCB wall, between elevations -7 ft – 7 in and +144 ft – 1 in. Non-typical portions of the RCB wall (e.g., at penetrations or discontinuities) may require additional reinforcing.

The RCB is a post-tensioned reinforced concrete structure with a steel liner and is a safety-related, Seismic Category 1 structure, as described in Section 3.8.1.

Description of the Critical Section and Computer Model

The RCB wall contains three buttresses. The buttresses are located at azimuth 0° (Buttress 1), azimuth 112° (Buttress 2), and azimuth 230° (Buttress 3). Two of the buttresses contain major penetrations. Buttress 1 contains a personnel airlock; Buttress 3 contains a construction opening and the emergency airlock. These two buttresses are wider in the vicinity of the penetrations, and narrow above the discontinuities. The third buttress does not contain any major penetrations, and therefore, it maintains the narrow profile over its full height.

Following are several views, taken from the ANSYS FEM, of the portions of the RCB designed in this critical section.

- Figure 3E.1-15—Elevation View of the Entire RCB shows an elevation view of the entire RCB.
- Figure 3E.1-16—Section Cut Through Entire RCB shows a section cut through the entire RCB.
- Figure 3E.1-17—Elevation View of Critical Section shows an elevation view of the portion of the RCB designed by this critical section.
- Figure 3E.1-18—Section View Cut Through Critical Section shows a section cut through the portion the RCB designed by this critical section.
- Figure 3E.1-19—Plan View Cut Through Critical Section shows a plan view cut through the portion of the RCB designed by this critical section.
- Figure 3E.1-20—Elevation View of Buttress 3 shows an elevation view of Buttress 3, which contains both the narrow and wide profiles designed by this critical section.
- Figure 3E.1-18—Section View Cut Through Critical Section shows a plan view cut through the narrow portion of Buttress 3.

- Figure 3E.1-22—Plan View Cut Through Typical Wide Buttress shows a plan view cut through the wide portion of Buttress 3.

A FEM is used to determine the forces and moments necessary to design the typical cylinder wall and buttress sections addressed in this critical section, as addressed in Section 3.8.1.4.1.

Applicable Loadings, Analysis, and Design Methods

The ANSYS global static model considers the independent loadings described in Section 3.8.1.

A separate analysis was performed to determine the magnitude of in-plane shear produced by accidental torsion in the various walls of the NI Common Basemat Structures. The accidental torsion, tangential shear loads, for the RCB are as shown in Table 3E.1-8—Accidental Torsion Loadings for the Typical Cylinder Wall and Buttress Section.

A separate analysis was performed to estimate the effects of concrete cracking on thermal moments. Based on the results of this analysis, thermal moments carried by the portions of the RCB were reduced as shown in Table 3E.1-9—Reduction of Thermal Bending Moments Due to Cracked Concrete for the Typical Cylinder Wall and Buttress Section.

All load combinations applied to the typical wall and buttress sections of the RCB are described in Section 3.8.1.3.2. This section is also designed for all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the three typical sections considered.

The governing design data for the typical RCB wall section is presented in Table 3E.1-10—Governing Design Data for Tangential Shear Design (Typical RCB Wall Section), Table 3E.1-11—Governing Design Data for Membrane and Bending Design (Typical RCB Wall Section), and Table 3E.1-12—Governing Design Data for Radial Shear Design (Typical RCB Wall Section); the reinforcing summary is presented in Table 3E.1-12—Governing Design Data for Radial Shear Design (Typical RCB Wall Section). The reinforcement pattern is shown in Figure 3E.1-23—Containment Wall Reinforcement (Typical Section).

The governing design data for the typical RCB narrow buttress section is presented in Table 3E.1-14—Governing Design Data for Tangential Shear Design (Typical RCB Narrow Buttress Section), Table 3E.1-15—Governing Design Data for Combined

Membrane and Bending Design (Typical RCB Narrow Buttress Section), and Table 3E.1-16—Governing Design Data for Radial Shear Design (Typical RCB Narrow Buttress Section); the reinforcing summary is presented in Table 3E.1-17—Reinforcing Summary (Typical RCB Narrow Buttress Section). The reinforcement pattern is shown in Figure 3E.1-24—Containment Buttress Reinforcement (Typical Narrow Section).

The governing design data for the typical RCB wide buttress section is presented in Table 3E.1-18—Governing Design Data for Tangential Shear Design (Typical RCB Wide Buttress Section), Table 3E.1-19—Governing Design Data for Combined Membrane and Bending Design (Typical RCB Wide Buttress Section), and Table 3E.1-20—Governing Design Data for Radial Shear Design (Typical RCB Wide Buttress Section); the reinforcing summary is presented in Table 3E.1-21—Reinforcing Summary (Typical RCB Wide Buttress Section). The reinforcement pattern is shown in Figure 3E.1-25—Containment Buttress Reinforcement (Typical Wide Section).

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.4 Reactor Building Internal Structures—Steam Generator and Reactor Coolant Pump Support and Typical Cavity Wall

This critical section presents the structural design of the Steam Generator and Reactor Coolant Pump floor slab at elevation 4 ft – 11 $\frac{1}{16}$ in and the typical cavity walls of the RB Internal Structures. The floor slab provides vertical support for the reactor coolant pumps (RCP) and steam generators (SG); the typical cavity walls provide lateral support for the SGs and RCPs.

This critical section focuses on the design of the following cavity walls: walls that separate steam generators, and walls that separate reactor coolant pumps and steam generators. The typical cavity walls span between elevations 4 ft – 11 $\frac{1}{16}$ in and 63 ft – 11 $\frac{11}{16}$ in.

The reinforced concrete walls and slab designed are RB Internal Structure elements that are safety-related, Seismic Category 1 structures, as described in Section 3.8.3.

Description of the Critical Section and Computer Model

The floor slab at elevation 4 ft – 11 $\frac{1}{16}$ in is a circular slab with varying thicknesses. The portion of the slab designed with a thickness of 6 ft – 6 $\frac{3}{4}$ in provides the vertical supports for the four steam generators as well as two of the reactor coolant pumps. A second portion of the slab is designed with a thickness of 3 ft – 3 $\frac{3}{8}$ in and supports two of the reactor coolant pumps. These two areas of the slab with different

thicknesses are divided by the supporting walls underneath, at elevation $-7\text{ ft} - 6\frac{9}{16}\text{ in.}$

Figure 3E.1-26—Floor Slab Plan View at Elevation $4'\text{--}11\frac{1}{16}"$ displays the two floor areas; the center of the RB is the center of Figure 3E.1-26. The larger portion of the slab is designed with a thickness of $6\text{ ft} - 6\frac{3}{4}\text{ in}$ and the smaller portion of the slab is designed with a thickness of $3\text{ ft} - 3\frac{3}{8}\text{ in.}$

There are four SG/RCP wing walls, which divide the reactor coolant pump cavities from the steam generator cavities. The walls span from $16\text{ ft} - 10\frac{3}{4}\text{ in}$ to $63\text{ ft} - 11\frac{11}{16}\text{ in}$ and can be divided into three segments: the first segment is from elevation $16\text{ ft} - 10\frac{3}{4}\text{ in}$ to $30\text{ ft} - 9\frac{5}{16}\text{ in}$ with thickness $3\text{ ft} - 11\frac{1}{4}\text{ in}$, the second segment is from elevation $30\text{ ft} - 9\frac{5}{16}\text{ in}$ to $52\text{ ft} - 5\frac{15}{16}\text{ in}$ with thickness $3\text{ ft} - 3\frac{3}{8}\text{ in}$, and the third segment is from $52\text{ ft} - 5\frac{15}{16}\text{ in}$ to $63\text{ ft} - 11\frac{11}{16}\text{ in}$ with thickness $3\text{ ft} - 3\frac{3}{8}\text{ in}$. This critical section presents the design of the first segment from $16\text{ ft} - 10\frac{3}{4}\text{ in}$ to $30\text{ ft} - 9\frac{5}{16}\text{ in}$ and the third segment from $52\text{ ft} - 5\frac{15}{16}\text{ in}$ to $63\text{ ft} - 11\frac{11}{16}\text{ in}$ since these wall areas contain the lateral supports for the SG and RCP and are considered to be critical.

There are two SG separation walls that separate the SG rooms (SG1 and SG2, SG3 and SG4) and provide lateral support for the SGs. These walls are not continuous along the height of the reactor cavity. Each wall consists of two segments: elevation $16\text{ ft} - 10\frac{3}{4}\text{ in}$ to $30\text{ ft} - 9\frac{5}{16}\text{ in}$ and elevation $52\text{ ft} - 5\frac{15}{16}\text{ in}$ to $63\text{ ft} - 11\frac{11}{16}\text{ in}$. Therefore, there are four SG separation walls. The SG separation walls are designed with a thickness of $3\text{ ft} - 3\frac{3}{8}\text{ in.}$

The typical cavity walls designed in this critical section (i.e., SG/RCP wing walls and SG separation walls) are shown in Figure 3E.1-27—Typical Cavity Walls Plan View, the center of the RB is the center of Figure 3E.1-27.

The design of the reinforced concrete floor slab and typical cavity wall sections initiates with the ANSYS computer model described in Section 3.8.3.4.1. The floor slab and typical cavity walls portion of the ANSYS FEM are constructed from shell (Shell43) elements. The slab and walls are auto-meshed with a typical element dimension of approximately $5\text{ ft} - 0\text{ in}$ by $5\text{ ft} - 0\text{ in}$. An isolated section of the floor slab and typical cavity walls from the ANSYS FEM is presented in Figure 3E.1-28—Isolated View of FEM For Floor Slab and SG/RCP Wing Wall. Figure 3E.1-29—Isolated View of FEM For Floor Slab and SG/RCP Separation Wall, displays the same view; however, the SG/RCP wing wall is removed to show the SG separation walls. This figure depicts the east side of the typical cavity walls and floor slab.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configurations for the concrete sections of the floor slab and typical cavity walls using forces and moments generated from the ANSYS FEM, which is described in Section 3.8.3.4.1. Critical cases are selected for design based on: maximum axial forces, maximum bending moments, maximum out-of-plane shear reinforcement force required, maximum in-plane shear forces, and maximum areas of total required steel. Design of required reinforcement is accomplished by averaging results from elements within a justifiable distance and selecting the maximum reinforcement required from all cases considered for longitudinal and out-of-plane shear reinforcement. The process is done independently for each wall and slab section considered.

Loads applied to the floor slab and typical cavity walls of the RB Internal Structures are described in Section 3.8.3.3.1. In addition to these loads, the upper portion of the SG/RCP wing wall and SG separation wall are subject to a sub-compartment pressurization load of 20 psi. Additional bending moments and out-of-plane shear forces are added to the extracted forces and moments from ANSYS.

Additional shear forces and bending moments are also added to the floor slab to account for the remaining 75 percent of the live load that is not included in the results from the ANSYS FEM, as described in Section 3.8.3.4.4. These loads are conservatively estimated using a plate with an area equal to that of the free span of the floor slab with fixed-fixed boundary conditions and an applied load of 375 psf ($0.75 * 500\text{psf}$).

All load combinations applied to the floor slab and typical cavity walls of the RB Internal Structures are described in Section 3.8.3.3.2. The floor slab and typical cavity walls are designed to accommodate all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

Table 3E.1-22—Governing Design Cases for the SG and RCP Supports and Typical Cavity Wall shows the governing load cases along with the forces and moments from the elements, which are the maximum out of all of the elements that are averaged for the governing load case. The values for M_x and M_y include the addition of M_{xy} .

Table 3E.1-23—Summary of Reinforcement for SG and RCP Floor Slab and
Table 3E.1-24—Typical SG and RCP Cavity Wall Reinforcement summarize the

reinforcement provided to meet the area of steel required for the associated forces and moments.

The typical reinforcing patterns described in Table 3E.1-23 and Table 3E.1-24 are shown in Figure 3E.1-30—Area of Detail for Floor Slab at Elevation 4'-11 1/16" with Thickness 6'-6 3/4", Figure 3E.1-31—Reinforcement of Floor Slab at Elevation 4'-11 1/16", Section 1-1, Figure 3E.1-32—Area of Detail for Floor Slab at Elevation 4'-11 1/16" with Thickness 3'-3 3/8", Figure 3E.1-34—Reinforcement of Floor Slab Section 2-2, Figure 3E.1-33—Reinforcement of Floor Slab Section 3-3, Figure 3E.1-35—Area of Detail for SG/RCP Wing Wall Bottom, Figure 3E.1-36—SG/RCP Wing Wall Bottom Reinforcement, Figure 3E.1-37—Area of Detail for SG/RCP Wing Wall Top, Figure 3E.1-38—SG/RCP Wing Wall Top Reinforcement, Figure 3E.1-39—Area of Detail for SG Separation Wall Bottom, Figure 3E.1-40—SG Separation Wall Bottom Reinforcement, Figure 3E.1-41—Area of Detail for SG Separation Wall Top, and Figure 3E.1-41—Area of Detail for SG Separation Wall Top.

The shaded area shown in Figure 3E.1-30 represents the area of the floor slab at elevation 4 ft – 11 1/16 in that is designed with a thickness of 6 ft – 6 3/4 in. The reinforcement pattern for this area of the slab is shown in Figure 3E.1-31.

The two shaded areas shown in Figure 3E.1-32 represent the area of the floor slab at elevation 4 ft – 11 1/16 in that is designed with a thickness of 3 ft – 3 3/8 in. The reinforcement pattern for the area shaded in solid black (Section 3-3) is shown in Figure 3E.1-33. The reinforcement pattern for the area diagonally hatched (Section 2-2) is shown in Figure 3E.1-34.

Figure 3E.1-35 displays the plan view of the bottom segment of the SG/RCP wing walls (shaded in solid black). The reinforcement pattern for these walls is displayed in Figure 3E.1-36.

Figure 3E.1-37 displays the plan view of the top segment of the SG/RCP wing walls (shaded in solid black). The reinforcement pattern for these walls is displayed in Figure 3E.1-38.

Figure 3E.1-39 displays the plan view of the bottom segment of the SG separation walls (shaded in solid black). The reinforcement pattern for these walls is displayed in Figure 3E.1-40.

Figure 3E.1-41 displays the plan view of the top segment of the SG separation walls (shaded in solid black). The reinforcement pattern for these walls is displayed in Figure 3E.1-42.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.5 Reactor Building Internal Structures—Pressurizer Support and Typical Cavity Wall

This critical section presents the structural design of the reinforced concrete supports and typical cavity wall sections required for the RB Pressurizer (PZR) cubicle. The PZR cubicle is located in the RCB as part of the RB Internal Structures and lies northwest of the Loop 3 RCP (RCP3) as shown in Figure 3E.1-43—Location of PZR Cubical. The PZR cubicle structure is a reinforced concrete, safety-related, Seismic Category I structure, as described in Section 3.8.3.

Description of the Critical Section and Computer Model

The PZR is supported by three brackets anchored to the concrete floor slab at elevation $49\text{ ft} - 1\frac{3}{8}\text{ in}$ and by eight radial pins anchored to the floor slab at elevation $67\text{ ft} - 10\frac{15}{16}\text{ in}$. An isometric view of these supports is shown in Figure 3E.1-44—PZR Isometric View Showing Support Locations. All of these supports apply concentrated loads to the floor slabs. The three brackets (PZR1, PZR2 and PZR3) transfer vertical and horizontal reactions to the floor slab and the eight radial pins (U1 thru U8) transfer only horizontal radial reactions to the floor slab.

Two areas of the PZR cubicle, one floor slab, and one wall panel are selected as critical based on section thickness, span, and loading so that maximum moments and shears are obtained. The slab at elevation $49\text{ ft} - 1\frac{3}{8}\text{ in}$ is considered to be critical due to the fact that it supports the PZR vertical reactions. The wall lying west of the PZR is considered critical because of the placement of the supports and the thickness of the section. This wall spans from elevation $20\text{ ft} - 11\frac{3}{16}\text{ in}$ to $92\text{ ft} - 8\frac{3}{16}\text{ in}$.

The design of the PZR cubicle critical areas initiates with the computer model described in Section 3.8.3.4.1. The PZR cubicle portion of the ANSYS FEM is constructed from shell (Shell 43) elements. The typical element dimensions are approximately $5\text{ ft} - 0\text{ in}$ by $5\text{ ft} - 0\text{ in}$ and multiple layers of elements are used throughout the Internal Structure ANSYS FEM. An element plot of each of the critical areas is shown in Figure 3E.1-45—Plan View of FEM for Floor Slab of PZR Cubical and Figure 3E.1-46—Elevation View of FEM for Wall Section of PZR Cubical.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configuration for the reinforced concrete PZR cubicle using forces and moments generated from the ANSYS FEM. The design of the PZR cubicle is performed using design macros, which utilize the applicable codes, standards, and specifications for RB Internal Structures as described in Section 3.8.3.2.

Loads applied to the PZR cubicle are described in Section 3.8.3.3.1. Additional loads, which are not applied to the PZR cubicle using the ANSYS FEM described in

Section 3.8.3.4.1 and Section 3.8.3.4.4, include a subcompartment pressurization load on the wall section, as well as an additional 75 percent of the applicable live load on the floor slab. These loads are added separately to the results obtained from the ANSYS FEM.

An additional local concentrated moment is also added at the location of the lower supports (PZR1-PZR3).

All load combinations considered in the PZR cubicle critical section are described in Section 3.8.3.3.2. The PZR cubical is designed to accommodate all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

The governing forces and moments from the applied loads are shown in Table 3E.1-25—Summary of Governing Design Data. In general the governing load case is the one that includes the following loads: Dead (D) + Hydrostatic (F) + Live (L) + Accident Temperature (T_a) + Accident Pipe Reaction (R_a) + Internal Flood (F_a) + Accident Pressure (P_a) + Pipe Break (R_r) + Earthquake (E').

Table 3E.1-26—Summary of Typical PZR Cubical Reinforcement, summarizes the reinforcement provided to meet the area of steel requirements for the PZR cubical. The typical reinforcing patterns described in Table 3E.1-26 are shown in Figure 3E.1-47—PZR Floor Slab Section - Reinforcement at Support and Figure 3E.1-48—PZR Wall Section - Typical Reinforcement.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.6 Reactor Building Internal Structures—Operating Floor Area

This critical section presents the structural design of the reinforced concrete operating floor slab at elevation +63 ft – 11 ¹¹/₁₆ in of the RB. The RB operating floor is part of RB Internal Structures, which is reinforced concrete, safety-related, Seismic Category 1 structures, as described in Section 3.8.3.

Description of the Critical Section and Computer Model

The RB operating floor at EL. +63 ft – 11 ¹¹/₁₆ in consists of the slab sections shown in Figure 3E.1-49—RB Operating Floor - Elevation 63'-11 11/16" Showing Section Locations.

The operating floor at elevation +63 ft – 11 ¹¹/₁₆ in is divided into six floor sections shown in Figure 3E.1-50—Plan View of RB Operating Floor Showing Rooms. The areas shown in Figure 3E.1-50 have the following slab thicknesses:

- RM15: 2 ft – 7 ½ in.
- RM22: 2 ft – 7 ½ in.
- RM16-1: 3 ft – 3 ³/₈ in.
- RM16-2: 3 ft – 3 ³/₈ in.
- RM16-3: 4 ft – 3 ³/₁₆ in.
- RM18: 2 ft – 7 ½ in.

The design of the RB operating floor initiates with the computer model described in Section 3.8.3.4.1, and is constructed from shell (Shell43) elements. The typical element size is 5 ft – 0 in by 5 ft – 0 in, which is used throughout the Internal Structures ANSYS FEM.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the design of this critical section is to determine the reinforcement required for the RB operating floor at elevation +63 ft – 11 ¹¹/₁₆ in.

The RB operating floor is part of RB Internal Structures and is designed in accordance with the applicable codes, standards, and specifications for RB Internal Structures as described in Section 3.8.3.2.

The loads used to design the operating floor are described in Section 3.8.3.3.1. An additional 75 percent of the applicable live load is applied to the operating floor separately from the ANSYS FEM, as described in Section 3.8.3.4.4.

All load combinations considered in the operating floor critical section design are described in Section 3.8.3.3.2. The operating floor is designed to accommodate all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

The governing forces and moments from the applied loads are shown in Table 3E.1-27—Summary of Governing Design Data for the Operating Floor Area.

Each RB operating floor sub-area has governing design data for both radial and tangential shear. Table 3E.1-28—Summary of Typical Reinforcement for the Operating Floor Area, summarizes the reinforcement provided to meet the area of steel requirements for the RB operating floor. The typical reinforcing patterns described in Table 3E.1-28 are shown in Figure 3E.1-51—RB Operating Floor Reinforcement - Section 1-1, Figure 3E.1-52—RB Operating Floor Reinforcement - Section 2-2, Figure 3E.1-53—RB Operating Floor Reinforcement - Section 3-3 and 4-4, Figure 3E.1-54—RB Operating Floor Reinforcement - Section 5-5, Figure 3E.1-55—RB Operating Floor Reinforcement - Section 6-6, Figure 3E.1-56—RB Operating Floor Reinforcement - Section 7-7, and Figure 3E.1-57—RB Operating Floor Reinforcement - Section 8-8.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.7 Reactor Shield Building – Connection of FB and SB 2 and 3 Roofs to RSB Wall

This critical section presents the structural design of the reinforced concrete of the connections from the RSB wall to the FB roof slab as well as the roof slab of SB 2 and 3.

The RSB connections to the FB and SB 2 and 3 roofs are considered to be critical sections because these areas are sections of the plant where high levels of stresses are anticipated as a result of seismic loadings and geometry changes. The RSB connections designed are reinforced concrete, safety-related, Seismic Category I structures, as described in Section 3.8.4.

Description of the Critical Section and Computer Model

The RSB wall is a 5 ft – 10 ⁷/₈ in thick wall above the FB roof and the SB 2 and 3 roof, but reduces to a thickness of 4 ft – 3 ³/₁₆ in below the roofs. The vertical boundaries of the shield wall considered at the connection to the FB roof are taken from an elevation of +95 ft – 10 ³/₈ in to an elevation of +121 ft – 9 ⁷/₁₆ in as shown in Figure 3E.1-58—RSB Wall Vertical Design Boundaries. Figure 3E.1-58 also shows that the vertical portion of the shield wall considered at the connection to SB 2 and 3 is taken from an elevation of +77 ft – 5 ¹/₂ in to an elevation of +104 ft – 0 ¹/₁₆ in.

The cylindrical portion of the RSB wall that intersects the FB is taken from azimuth 207.5° to azimuth 332.5° as shown in Figure 3E.1-59—FB Roof and RSB Wall Design Boundaries. The cylindrical portion of the RSB wall that intersects SB 2 and 3 is taken from an azimuth of 27.5° to an azimuth of 152.5° as shown in Figure 3E.1-60—SB 2&3 Roof and RSB Wall Design Boundaries.

Figures 3E.1-59 and 3E.1-60 also show the areas considered for design of the FB and SB 2 and 3 roofs. The FB and SB 2 and 3 roofs are 5 ft – 10 ⁷/₈ in thick. A radial portion of about 11 ft – 9 ³/₄ in away from the outer edge of the RSB wall is considered for the FB and SB 2 and 3. This is shown in Figures 3E.1-59 and 3E.1-60.

The design of the connections of the RSB to the FB and SB 2 and 3 roofs when subjected to dead, live, wind, tornado, and seismic loading initiates with the ANSYS FEM as described in Section 3.8.4.4.2.

The FEM is auto-meshed generating shell (Shell43) elements dimensioned at roughly 5 ft – 0 in throughout the RSB. Figure 3E.1-61—Isometric FEM of FB Roof to RSB Wall Connection and Figure 3E.1-62—Isometric FEM of SB 2 and 3 Roof to RSB Wall Connection, represent an isometric view of the ANSYS FEM displaying the sections analyzed along with the shape of the elements.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configuration for the concrete section of the RSB Wall connection to the FB roof and the SB 2 and 3 roof using the forces and moments generated from the FEM of the NI Common Basemat Structures. The design of the connection is performed using calculations utilizing the applicable codes, standards and specifications described in Section 3.8.4.2.

The ANSYS FEM considers all loads shown in Table 3E.1-1, which are described in Section 3.8.4.3.1 except for additional live loads, thermal loads, and accidental torsion loads, which are applied separately to this critical section.

A precipitation load of 75 psf is applied as a live load on all roofs in addition to what is applied in the ANSYS FEM for all load combinations that include seismic loads. This is due to the ANSYS FEM only considering 25% of the 100 psf live load for all load combinations containing seismic loads. Construction loads are considered to be enveloped by the precipitation load based on the assumption that both are not applied concurrently.

An accidental torsion load of 27.6 kip/ft is applied as an in-plane shear load to the entire RSB area that is considered for design.

All load combinations applied to the RSB wall to roof connection are described in Section 3.8.4.3.2. This section is also designed to accommodate all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

Table 3E.1-29—Controlling Nodal Forces and Moments for FB Roof,
Table 3E.1-30—Controlling Nodal Forces and Moments for SB 2 and 3 Roof,
Table 3E.1-31—Controlling Nodal Forces and Moments for RSB Wall below FB Roof and SB 2 and 3 Roof, and Table 3E.1-32—Controlling Nodal Forces and Moments for RSB Wall above FB Roof and SB 2 and 3 Roof show the governing forces and moments from the applied loads.

Results of the reinforcement design are shown in Table 3E.1-33—Specified Reinforcement Pattern for RSB Wall to Roof Connection, and the typical reinforcement sketch is shown in Figure 3E.1-63—RSB Wall to Roof Connection - Typical Reinforcement.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.8 Safeguard Buildings—Walls Below Grade

This critical section presents the structural design of the reinforced concrete external walls below grade level, from elevation -31 ft – 6 in to 0 ft – 0 in for SBs 1, 2, 3, and 4 and the FB. The walls below grade are chosen as critical sections to assess the impact of the soil on the walls under all applicable load combinations.

The external walls below grade are part of the NI Common Basemat Structures and are therefore considered to be safety-related, Seismic Category I, as described in Section 3.8.4.

Description of the Critical Section and Computer Model

SBs 2 and 3 and the FB are enclosed by a shield structure, only SBs 1 and 4 are analyzed as critical sections because the section thickness is smaller and therefore are considered critical.

Section Location within SB1

The sections under investigation are the SB1 South wall (labeled as A13001) and SB1 West wall (labeled as A13003) below grade (El. -31 ft – 6 in to 0 ft – 0 in), which are shown in Figure 3E.1-64—Location of SB1 South and West Walls Below Grade. SB1 walls below grade span from global coordinate $z = -31 \text{ ft} - 6 \text{ in}$ to $z = 0 \text{ ft} - 0 \text{ in}$ (below grade portion only) in the ANSYS FEM. The thickness of each wall is 4 ft – 11 in.

Wall A13001 is located at coordinate $y = -49 \text{ ft} - 8 \frac{7}{16} \text{ in}$, and spans from $x = -178 \text{ ft} - 11 \frac{5}{8} \text{ in}$ to $x = -115 \text{ ft} - 3 \frac{5}{8} \text{ in}$. This wall is chosen for analysis to incorporate the effects of an East-West earthquake load for in-plane shear. Wall A13003 is located at coordinate $x = -178 \text{ ft} - 11 \frac{5}{8} \text{ in}$, and spans from $y = -49 \text{ ft} - 8 \frac{7}{16} \text{ in}$ to $y = 49 \text{ ft} - 8 \frac{7}{16} \text{ in}$. This wall is chosen for analysis to incorporate the effects of a North-South earthquake load for in-plane shear.

Section Location within SB4

The sections under investigation are the SB4 North wall (labeled as A33008) and SB4 East wall (labeled as A33003) below grade (El. $-31 \text{ ft} - 6 \text{ in}$ to $0 \text{ ft} - 0 \text{ in}$), which are shown in Figure 3E.1-65—Location of SB4 North and East Walls Below Grade. SB4 walls below grade span from global coordinate $z = -31 \text{ ft} - 6 \text{ in}$ to $z = 0 \text{ ft} - 0 \text{ in}$ (below grade portion only) in the ANSYS FEM. The thickness of each wall is $4 \text{ ft} - 11 \text{ in}$.

Wall A33008 is located at coordinate $y = 49 \text{ ft} - 8 \frac{7}{16} \text{ in}$, and spans from $x = 106 \text{ ft} - 4 \text{ in}$ to $x = 178 \text{ ft} - 11 \frac{5}{8} \text{ in}$. This wall is chosen for analysis to incorporate the effects of an East-West earthquake load for in-plane shear. Wall A33003 is located at coordinate $x = 178 \text{ ft} - 11 \frac{5}{8} \text{ in}$, and spans from $y = -49 \text{ ft} - 8 \frac{7}{16} \text{ in}$ to $y = 49 \text{ ft} - 8 \frac{7}{16} \text{ in}$. This wall is chosen for analysis to incorporate the effects of a North-South earthquake load for in-plane shear.

SB4 walls are chosen as critical sections because they are adjacent to buildings that impact the soil loading on these walls.

The design of the SB walls below grade initiates with the computer model described in Section 3.8.4.4.2. The SB walls under investigation in the ANSYS FEM are constructed using shell (Shell43) elements. The typical element is approximately $5 \text{ ft} - 0 \text{ in}$ by $5 \text{ ft} - 0 \text{ in}$.

Isometric views from the FEM for all walls under investigation are shown in Figure 3E.1-66—SB1 Wall A13001 Isometric View, Figure 3E.1-67—SB1 Wall A13003 Isometric View, Figure 3E.1-68—SB4 Wall A33008 Isometric View, and Figure 3E.1-69—SB4 Wall A33003 Isometric View.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of this critical section is to determine the reinforcement configuration for the concrete sections of the walls below grade for SB1 and SB4 from El. $-31 \text{ ft} - 6 \text{ in}$ to $0 \text{ ft} - 0 \text{ in}$. A FEM is generated using ANSYS, as described in Section 3.8.4.4.2, and is used to determine forces and moments for the walls below grade by evaluating various loads, load combinations, and soil analysis cases. These forces and moments are analyzed using a design macro where they are

compared with applicable codes, standards, and specifications for the Safeguard Building walls below grade as described in Section 3.8.4.2.

Loads applied to the Safeguard Building walls below grade are described in Section 3.8.4.3.1. Additional loads due to accidental torsion are analyzed separately from the ANSYS FEM and added to the in-plane shear loads.

All load combinations applied to the SB walls below grade are described in Section 3.8.4.3.2. This section is designed to accommodate all soil analysis cases shown in Table 3.7.1-6.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

Table 3E.1-34—Summary of Governing Design Data for SB External Walls Below Grade shows the governing forces and moments from the applied loads, where bold cells indicate design values, the sign convention is negative for compression and positive for tension, and the averaged numbers are indicated in italics.

In general, the load combination that includes: Dead (D) + Live (L) + Lateral Earth Pressure (H) + Hydrostatic Pressure (F) + Buoyancy Force (F_b) + Earthquake (E') loads controls the design of the SB walls below grade.

The SB walls below grade are designed for the resultant forces and moments determined based on the applicable range of loading combinations and soil conditions. It should be noted that the design maximum (or minimum, as appropriate) forces and moments may not occur at the same location and may not be from the same load combination/soil condition.

Table 3E.1-35—Reinforcement Design for SG Walls Below Grade (A13001, A13003, A33008, and A33003) summarizes the reinforcement provided to meet the area of steel required for the associated direction and forces and moments.

The typical reinforcing patterns for each wall described in Table 3E.1-35 are shown in Figure 3E.1-70—Cross Section of Walls A13001, A13003, A33008, and A33003 Showing Reinforcement.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

3E.1.9 Foundation of Nuclear Island Buildings and Base Slab of the RB Internal Structures

This critical section presents the structural design of the reinforced concrete NI foundation basemat and RB Internal Structures base slab. The structural components designed in this critical section are safety-related, Seismic Category I structures, as described in Sections 3.8.1, 3.8.3, and 3.8.5.

Description of the Critical Section and Computer Model

The NI foundation basemat transfers all of the loads from the buildings located on the NI Common Basemat to the supporting soil. Thus, the foundation basemat is a critical structural member of the NI. With reference to the finished grade level, the foundation basemat lies between elevation $-41\text{ ft} - 4\frac{1}{16}\text{ in}$ and $-31\text{ ft} - 5\frac{15}{16}\text{ in}$ except under the RCB where it is located between elevation $-36\text{ ft} - 5\text{ in}$ and $-25\text{ ft} - 7\frac{1}{16}\text{ in}$.

Figure 3E.1-71—Isometric FEM of NI Foundation Basemat, shows a schematic of the NI foundation basemat. At its largest plan dimension, the foundation basemat is $354\text{ ft} - 8\text{ in}$ by $357\text{ ft} - 11\text{ in}$. The thickness of the foundation basemat ranges from $9\text{ ft} - 10\frac{1}{8}\text{ in}$ to $13\text{ ft} - 1\frac{1}{2}\text{ in}$.

The design of the NI foundation basemat and the base slab for RB Internal Structures initiates with the computer model described in Sections 3.8.1.4.1 for the RCB, 3.8.1.4.1 for RB Internal Structures, and 3.8.5.4.2 for the NI foundation basemat.

The NI foundation basemat and base slab for RB Internal Structures are modeled using solid (Solid45) elements in the ANSYS FEM. In order to accurately model the interface between the RB Internal Structures base slab and the foundation basemat, a sub-model is developed for the analysis and design of the RB Internal Structures base slab. This sub-model is based on the ANSYS FEM used for all structures located on the NI Common Basemat, with appropriate boundary conditions imposed.

The base slab for RB Internal Structures provides support for structures and components that are internal to the RCB. The base slab rests on the foundation basemat of the RCB. The load imposed on the base slab is transferred to the foundation basemat, which in turn bears on the underlying soil.

The RB Internal Structures base slab is located between elevations $-25\text{ ft} - 7\frac{1}{16}\text{ in}$ and $-20\text{ ft} - 2\frac{1}{8}\text{ in}$ and elevations $-25\text{ ft} - 7\frac{1}{16}\text{ in}$ and $-7\text{ ft} - 6\frac{1}{2}\text{ in}$. The base slab is contained in a circular form with a diameter of $154\text{ ft} - 6\text{ in}$.

Figure 3E.1-72—Elevation View of RB Internal Structure Base Slab shows a schematic of the RB Internal Structures base slab.

As shown in Figure 3E.1-72, the portion of the RB Internal Structures base slab between elevation $-25\text{ ft} - 7\frac{1}{16}\text{ in}$ and $-7\text{ ft} - 6\frac{1}{2}\text{ in}$ is $18\text{ ft} - \frac{9}{16}\text{ in}$ in thick and occurs between radii $54\text{ ft} - 1\frac{5}{8}\text{ in}$ and $76\text{ ft} - 9\frac{1}{4}\text{ in}$. Figure 3E.1-72 shows that this portion of the base slab slopes at an angle between radii $73\text{ ft} - 9\frac{3}{16}\text{ in}$ and $76\text{ ft} - 9\frac{1}{4}\text{ in}$. This critical section considers the $18\text{ ft} - 0\frac{9}{16}\text{ in}$ thick portion between radii $54\text{ ft} - 1\frac{5}{8}\text{ in}$ and $68\text{ ft} - 8\text{ in}$.

The $5\text{ ft} - 4\frac{15}{16}\text{ in}$ thick portion of the RB Internal Structures base slab occurs between radii $0\text{ ft} - 0\text{ in}$ and $54\text{ ft} - 1\frac{5}{8}\text{ in}$, as shown in Figure 3E.1-72.

Applicable Loadings, Analysis, and Design Methods

The methodology used for the structural design of the critical section is to determine the reinforcement configuration for the concrete sections of the NI foundation basemat and the Internal Structures base slab.

Loads considered in the analysis of the global FEM of the NI Common Basemat Structures are described in Sections 3.8.1.3.1, 3.8.3.3.1, and Section 3.8.5.3, and are applicable to the RCB, RB Internal Structures, and the NI foundation basemat, respectively.

The applicable load combinations for the sections under investigation are applicable to the RCB, RB Internal Structures, and the NI foundation basemat, and are outlined in Sections 3.8.1.3.2, 3.8.3.3.2, and 3.8.5.3, respectively. All soil analysis cases described in Table 3.7.1-6 are considered for the design of this critical section.

A separate analysis was performed to estimate the effects of concrete cracked on thermal moments. Based on the results of this analysis, the thermal moments carried by the portions of the RCB were reduced.

Results of Critical Section Design

The structural design for the critical sections addressed herein provides reinforcement to resist element forces and moments as described below for each of the sections considered.

The governing load combinations and soil analysis cases for the structural sections with the corresponding forces and moments are reported in Table 3E.1-36—Governing Design Data for the NI Foundation Basemat and RB Internal Structures Base Slab.

The reinforcement detail for this critical section is shown in Figure 3E.1-73—Reinforcement Pattern for NI Foundation Base Mat (Except Below RCB), Figure 3E.1-74—Reinforcement Pattern for RB Internal Structures Base Slab -

Elevation -25'-7" to -20'-2", Figure 3E.1-75—Reinforcement Pattern for RB Internal Structures Base Slab - Elevation -25'-7" to -7'-6 1/2", and Figure 3E.1-76—Reinforcement Pattern for NI Foundation Base Mat below RCB, for the NI Foundation basemat, RB Internal Structures base slab between elevations -25 ft – 7 in and -20 ft – 2 in, and RB Internal Structures between elevations -25 ft – 7 in and -7 ft – 6 1/2 in. The required reinforcement is summarized for each design location in Table 3E.1-37—Reinforcement Summary for the NI Foundation Basemat and RB Internal Structures Base Slab.

Section thicknesses and reinforcing quantities may be optimized based on subsequent analysis results.

Table 3E.1-1—Independent Loads Considered in the FEM

D	Dead Loads
L	Live Loads
J	Post-tensioning Loads
H	Lateral Earth Pressure Loads
F	Hydrostatic Loads
F_b	Buoyancy Loads
E'	Seismic Loads
R_o	Piping Loads (normal operating conditions)
R_a	Piping Loads (accident conditions)
W	Wind Loads (severe environmental)
W_t	Wind Loads (extreme environmental)
P_t	Pressure Loads (test conditions)
P_a (only for containment)	Pressure Loads (accident conditions)
T_a (only for containment)	Temperature Loads (accidental conditions)

Table 3E.1-2—Independent Loads Not Considered in the FEM

G	Relief Valve Loads
R _r	Pipe Rupture Loads
F _a	Compartment Flood Loads
T _o	Temperature Loads (normal operating)
T _t	Temperature Loads (test conditions)
P _v	Containment Wall Pressure Variant Loads
P _a	Sub-compartment pressurization
CL	Construction Loads

Table 3E.1-3—Summary of Governing Design Data for the Wall to Foundation Connection
Sheet 1 of 2

Location	LC	AC	Condition	T _x (k/ft)	T _y (k/ft)	T _{xy} (k/ft)	M _x (k-ft/ft)	M _y (k-ft/ft)	M _{xy} (k-ft/ft)	N _x (k/ft)	N _y (k/ft)
Primary Gusset	D + L + J + G + P _a + T _a + E' + R _a + R _r	Fixed 5u	Membrane & Tangential	0 (315)	-552	-623	0	-1394	-1288	0	-169
		Fixed 4u-M		0 (315)	-575	-647	0	-111	-1011	0	72
		Fixed 4u-M		370	0 (25)	-408 (500)	-987	0	-366	-97	0
		Fixed 4u-M		386	0 (26)	-400 (500)	-973	0	352	97	0
	D + L + J + G + P _a + T _a + E' + R _a + R _r	5u	Membrane & Bending	688	0	113	-1362	0	-56	-23	0
		5u		604	0	100	-1501	0	456	19	0
	D + L + J + G + P _a + T _a + E' + R _a + R _r	-	Membrane & Radial	0	-800	*	0	*	*	0	*
		4u-M		0	-1359	-30	0	-783	0	0	1358

Table 3E.1-3—Summary of Governing Design Data for the Wall to Foundation Connection
Sheet 2 of 2

Location	LC	AC	Condition	T_x (k/ft)	T_y (k/ft)	T_{xy} (k/ft)	M_x (k-ft/ft)	M_y (k-ft/ft)	M_{xy} (k-ft/ft)	N_x (k/ft)	N_y (k/ft)
Upper Gusset	D + L + J + G + P _a + T _a + E' + R _a + R _r	5u	Membrane & Tangential	650	0	37	-1040	0	384	53	0
		Fixed 4u-M		0	219	86	0	655	215	0	18
		Fixed 4u-M		300	0	533	-1049	0	333	90	0
	D + L + J + G + P _a + T _a + E' + R _a + R _r	Fixed 4u-M	Membrane & Radial	0	219	86	0	655	215	0	18
	D + L + J + G + T _o + R _o + P _v + E'	4u-M		0	-376	33	0	1301	-38	0	-168
Upper & Primary Gusset	D + L + J + G + P _a + T _a + E' + R _a + R _r	5u	Membrane & Radial	688	0	113	-1362	0	-56	-23	0
		2sn4u		-227	0	363	-2125	0	-332	-451	0
Primary Gusset	D + F + L + R _a + E'	4u-M	N/A	N/A	0	N/A	N/A	N/A	N/A	N/A	1250

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

* - 800 k/ft corresponds to several load/soil analysis cases, but was chosen because it is a conservative value (least compression) for a series of shears.

In most cases the required reinforcing is based on an envelope of the forces and moments resulting from multiple load combinations/soil analysis cases given in the above table. In some cases, values indicated thusly: (###), the envelope is extended to include a larger range of associated values.

Table 3E.1-4—Summary of Typical Gusset Reinforcement

Location	Type	Direction	Thickness (T)	Required Area of Steel (in ² /ft)			Reinforcement Pattern
				Axial & Bending (per face)	In-Plane Shear	Out-of-Plane Shear	
Primary Gusset	Flexural	Vertical	22 ft – 6 ¹¹ / ₁₆ in	9.0	-	-	2 Layers #14 @ 6 in EF
	Flexural	Horizontal		9.0	-	-	2 Layers #14 @ 6 in EF
	Shear	Vertical		-	11.56	-	#11 @ 12 in
	Shear	Horizontal		-		17.31	#9 @ 12 in
	Stirrup	Horizontal		-	-	-	#5 @ 6 in
Upper (transition) Gusset	Flexural	Vertical	varies	Match wall reinf.	-	-	#14 @ 6 in EF
	Flexural	Horizontal		Match wall reinf.	-	-	#14 @ 6 in EF
	Shear	Vertical		-	12.22	-	5 - #11 @ 6 in (plus one additional bar)
	Shear	Horizontal		-	-	17.70	#9 @ 6 in
	Stirrup	Horizontal		-	-	-	#5 @ 6 in

**Table 3E.1-5—Governing Design Data for the Equipment Hatch Area
(Factored Loads)**

Cut Section	LC	AC	T _x MAX (k/ft)	T _x MIN (k/ft)	T _{xy} (k/ft)	M _x + M _{xy} (k-ft/ft)	N _x (k/ft)
H-Strip#9 V-Cut#8	D+L+J+G+1.5P _a + T _a +R _a	Fixed 5a	1450	-44	117	869	29
V-Strip#10 H-Cut#6	D+L+J+G+P _a +T _a +E'+R _a +R _r	Enveloped	1	-1146	459	3539	168
V-Strip#9 H-Cut#5	D+L+J+G+P _a +T _a +E'+R _a +R _r	Enveloped	179	-718	678	2082	241

*The controlling load components are highlighted in “bold.”

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

**Table 3E.1-6—Governing Design Data for the Equipment Hatch Area
(Service Loads)**

Cut Section	LC	AC	T _x MAX (k/ft)	T _x MIN (k/ft)	T _{xy} (k/ft)	M _x + M _{xy} (k-ft/ft)	N _x (k/ft)
H-Strip#9 V-Cut#11	D+L+J+G+T ₀ +R ₀ + P _v	Enveloped	-1079	-	563	1599	15
V-Strip#10 H-Cut#8	D+L+J+G+T ₀ +R ₀ + P _v	Enveloped	-1258	-	563	1795	112
V-Strip#9 H-Cut#5	D+L+J+T _t +P _t	Enveloped	-372	-	309	518	180

*The controlling load components are highlighted in “bold.”

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-7—Summary of Typical Reinforcement for the Equipment Hatch Area

Location	Type	Thickness (T)	Required Reinforcement, A_s (in ² /ft)		Reinforcement Pattern
			Axial+Flexural+ Tangential-Shear	Out-of-Plane Shear	
Along V-Strips 1 & 14	Hoop	4 ft-3 ³ / ₁₆ in	9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Vertical		9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Stirrup			0.5	1 #8 @ 16 in
Along V-Strips 2& 13	Hoop	6 ft-0 ¹ / ₂ in	9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Vertical		9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Stirrup			0.3	1 #8 @ 16 in
Along V-Strips 3& 12	Hoop	6 ft-4 ¹³ / ₁₆ in	9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Vertical		9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Stirrup			0.3	1 #8 @ 16 in
Along V-Strips 4& 11	Hoop	7 ft-2 in	9.0 E.F. & 4.5 Middle		2 Layers #14 @ 6 in E.F & 1 Layer #14 @ 6 in middle
	Vertical		13.5 E.F. & 4.5 Middle		3 Layers #14 @6 in E.F.& 1 Layer #14 @6 in Middle
	Stirrup			0.3	1 #8 @ 16 in
From V-Strips 5 & 10	Hoop	7 ft-10 ¹ / ₂ in	13.5 E.F. & 4.5 Middle		3 Layers #14 @6 in E.F.& 1 Layer #14 @6 in Middle
	Vertical		20.5 E.F. & 4.5 Middle		3 Layers #18 @ 8 in and 3-#9 Hoop bars E.F., & 1 Layer #14 @ 6 in Middle
	Stirrup			0.73	1 #8 @ 12 in
From V-Strips 6 to 9	Hoop	7 ft-10 ¹ / ₂ in	13.5 E.F. & 4.5 Middle		3 Layers #14 @6 in E.F.& 1 Layer #14 @6 in Middle
	Vertical		13.5 E.F. & 4.5 Middle		3 Layers #14 @6 in E.F.& 1 Layer #14 @6 in Middle
	Stirrup			0.73	1 #8 @ 12 in

Table 3E.1-8—Accidental Torsion Loadings for the Typical Cylinder Wall and Buttress Section

Tangential Shear (k/ft)	Elevation (ft)
25	All

Table 3E.1-9—Reduction of Thermal Bending Moments Due to Cracked Concrete for the Typical Cylinder Wall and Buttress Section

Reduction of Thermal Bending Moment (k-ft / ft)	Applicable Section
400	Typical Wall
400	Typical Narrow Buttress
400	Typical Wide Buttress

**Table 3E.1-10—Governing Design Data for Tangential Shear Design
(Typical RCB Wall Section)**

Location	T_{xy} (k/ft)	T_x (k/ft)	T_y (k/ft)	Comments
RCB Typical Wall	297	-87	-44	Controls for design of hoop reinforcing (A_{sh}).
	297	-87	-44	Controls for design of meridional reinforcing (A_{sm}).
	200	334	129	Controls for design of hoop reinforcing (A_{sh}), coincident with membrane tensions.
	200	334	129	Controls for design of meridional reinforcing (A_{sm}), coincident with membrane tensions.

**Table 3E.1-11—Governing Design Data for Membrane and Bending Design
(Typical RCB Wall Section)**

Location	T_x or T_y (k/ft)	M_x or M_y (k-ft/ft)	Comments
RCB Typical Wall (Factored Loads)	-100	436	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-100	452	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	334	435	Controls for design of hoop reinforcing, coincident with membrane tension in the hoop direction.
	129	538	Controls for design of meridional reinforcing, coincident with membrane tension in the meridional direction.
RCB Typical Wall (Service Loads)	-176	65	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-223	74	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	n/a	n/a	There is no membrane tension in the hoop direction.
	n/a	n/a	There is no membrane tension in the meridional direction.

**Table 3E.1-12—Governing Design Data for Radial Shear Design (Typical
RCB Wall Section)**

Location	N_x or N_y (k/ft)	T_x or T_y (k/ft)	Comments
RCB Typical Wall	6	334	Controls for design of radial shear reinforcing (A_v).

Table 3E.1-13—Reinforcing Summary (Typical RCB Wall Section)

Location	Type	Thickness	Required A _s (in ² / foot)				Reinforcement Pattern
			Membrane and Bending ¹	Tangential Shear ¹	Total Area of Steel	Radial Shear	
RCB Typical Wall	Meridional (Vertical)	4 ft-3 ³ / ₁₆	Compression 8.39	Compression 5.11	13.50	-	Inside Layer #14 @ 6 in o.c.
			Tension 10.80	Tension 2.70			Middle Layer #14 @ 6 in o.c
							Outside Layer #14 @ 6 in o.c
	Hoop (Horizontal)		Compression 8.75	Compression 4.75	13.50	-	Inside Layer #14 @ 6 in o.c
			Tension 11.77	Tension 1.73			Middle Layer #14 @ 6 in o.c
							Outside Layer #14 @ 6 in o.c
	Stirrup		-	-	-	0.05	#3 @18 in o.c. vertical and horizontal

Note:

- Two values of required tangential shear steel were determined. One value covered all situations of membrane compression, and the other covered all situations of membrane tension. This resulted in less required tangential shear reinforcing for situations involving membrane tension, since these situations coincided with smaller values of tangential shear. This was necessary to minimize the amount of reinforcing to be “discounted” when designing for combined membrane tension and bending.

**Table 3E.1-14—Governing Design Data for Tangential Shear Design
(Typical RCB Narrow Buttress Section)**

Location	T_{xy} (k/ft)	T_x (k/ft)	T_y (k/ft)	Comments
RCB Typical Narrow Buttress	225	-481	25	Controls for design of hoop reinforcing (A_{sh}).
	225	-481	25	Controls for design of meridional reinforcing (A_{sm}).
	221	-481 ¹	31	Controls for design of hoop reinforcing (A_{sh}), coincident with membrane tensions.
	221	-481 ¹	31	Controls for design of meridional reinforcing (A_{sm}), coincident with membrane tensions.

Note:

1. There were no membrane tensions in the buttress in the hoop direction. Therefore, the least compressive membrane load was utilized.

Table 3E.1-15—Governing Design Data for Combined Membrane and Bending Design (Typical RCB Narrow Buttress Section)

Location	T_x or T_y (k/ft)	M_x or M_y (k-ft/ft)	Comments
RCB Typical Narrow Buttress (Factored Loads)	-107	426	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-100	741	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	n/a	n/a	There is no membrane tension in the hoop direction.
	207	874	Controls for design of meridional reinforcing, coincident with membrane tension in the meridional direction.
RCB Typical Narrow Buttress (Service Loads)	-700	798	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-400	223	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	n/a	n/a	There is no membrane tension in the hoop direction.
	n/a	n/a	There is no membrane tension in the meridional direction.

Table 3E.1-16—Governing Design Data for Radial Shear Design (Typical RCB Narrow Buttress Section)

Location	N_x or N_y (k/ft)	T_x or T_y (k/ft)	Comments
RCB Typical Narrow Buttress	109	-500	Controls for design of radial shear reinforcing (A_v).

Table 3E.1-17—Reinforcing Summary (Typical RCB Narrow Buttress Section)

Location	Type	Thickness	Required As (in ² /ft)				Reinforcement Pattern
			Membrane and Bending ¹	Tangential Shear ¹	Total Area of Steel	Radial Shear	
RCB Typical Narrow Buttress	Meridional (Vertical)	Varies Effective thickness is 6 ft-17/16 in	Compression 8.70	Compression 3.94	12.64	-	Inside Layer # 14 @ 6 in o.c.
			Tension 8.82	Tension 3.82			Middle Layer # 14 @ 6 in o.c.
							Outside Layer #14 (17 total) ²
	Hoop (Horizontal)		Compression 9.43	Compression 1.65	11.08	-	Inside Layer # 14 @ 6 in o.c.
			Tension 9.49	Tension 1.59			Middle Layer # 14 @ 6 in o.c.
							Outside Layer # 11 @ 9 in o.c.
	Stirrup		-	-	-	0.08	# 4 @ 24 in o.c. Vertical and horizontal

Notes:

- Two values of required tangential shear steel were determined. One value covered all situations of membrane compression, and the other covered all situations of membrane tension. This resulted in less required tangential shear reinforcing for situations involving membrane tension, since these situations coincided with smaller values of tangential shear. This was necessary to minimize the amount of reinforcing to be “discounted” when designing for combined membrane tension and bending.
- Number of vertical bars across outside face of buttress.

**Table 3E.1-18—Governing Design Data for Tangential Shear Design
(Typical RCB Wide Buttress Section)**

Location	T_{xy} (k/ft)	T_x (k/ft)	T_y (k/ft)	Comments
RCB Typical Wide Buttress	225	-554	75	Controls for design of hoop reinforcing (A_{sh}).
	225	-554	75	Controls for design of meridional reinforcing (A_{sm}).
	126	-149 ¹	121	Controls for design of hoop reinforcing (A_{sh}), coincident with membrane tensions.
	126	-149 ¹	121	Controls for design of meridional reinforcing (A_{sm}), coincident with membrane tensions.

Note:

1. There were no membrane tensions in the buttress in the hoop direction. Therefore, the least compressive membrane load was utilized.

Table 3E.1-19—Governing Design Data for Combined Membrane and Bending Design (Typical RCB Wide Buttress Section)

Location	T_x or T_y (k/ft)	M_x or M_y (k-ft/ft)	Comments
RCB Typical Wide Buttress (Factored Loads)	-149	13	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-100	716	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	n/a	n/a	There is no membrane tension in the hoop direction.
	121	728	Controls for design of meridional reinforcing, coincident with membrane tension in the meridional direction.
RCB Typical Wide Buttress (Service Loads)	-744	328	Controls for design of hoop reinforcing, coincident with membrane compression in the hoop direction.
	-511	126	Controls for design of meridional reinforcing, coincident with membrane compression in the meridional direction.
	n/a	n/a	There is no membrane tension in the hoop direction.
	n/a	n/a	There is no membrane tension in the meridional direction.

Table 3E.1-20—Governing Design Data for Radial Shear Design (Typical RCB Wide Buttress Section)

Location	N_x or N_y (k/ft)	T_x or T_y (k/ft)	Comments
RCB Typical Wide Buttress	n/a	n/a	There is no radial shear reinforcing (A_v) required for this section.

Table 3E.1-21—Reinforcing Summary (Typical RCB Wide Buttress Section)

Location	Type	Thickness	Required As (in²/ft)				Reinforcement Pattern
			Membrane and Bending¹	Tangential Shear¹	Total Area of Steel	Radial Shear	
RCB Typical Wide Buttress	Meridional (Vertical)	Varies Effective Thickness is 6 ft-1 7/16 in	Compression 8.50	Compression 3.53	12.03		Inside Layer # 14 @ 6 in o.c.
			Tension 10.56	Tension 1.47			Middle Layer # 14 @ 6 in o.c.
							Outside Layer # 14 (23 total)²
	Hoop (Horizontal)		Compression 9.60	Compression 1.48	11.08	-	Inside Layer # 14 @ 6 in o.c.
			Tension 9.75	Tension 1.33			Middle Layer # 14 @ 6 in o.c.
							Outside Layer # 11 @ 9 in o.c.
	Stirrup	-	-	-	0.00	# 4 @ 24 in o.c. vertical and horizontal	

Notes:

- Two values of required tangential shear steel were determined. One value covered all situations of membrane compression, and the other covered all situations of membrane tension. This resulted in less required tangential shear reinforcing for situations involving membrane tension, since these situations coincided with smaller values of tangential shear. This was necessary to minimize the amount of reinforcing to be “discounted” when designing for combined membrane tension and bending.
- Number of vertical bars across outside face of buttress.

Table 3E.1-22—Governing Design Cases for the SG and RCP Supports and Typical Cavity Wall
Sheet 1 of 2

Location	LC	AC	Controlling Direction	$M_x + M_{xy}$ k-ft/ft	$M_y + M_{xy}$ k-ft/ft	T_x k/ft	T_y k/ft	T_{xy} k/ft	N_x k/ft	N_y k/ft
Floor Slab @ Elevation 4 ft - 11 1/16 in (Thickness 6 ft - 6 3/4 in)	$D + F + L + T_a + R_a + F_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	3r3u-M	y	627	256	-669	1810	97	131	117
		3r3u-M	x	832	115	319	1	53	102	36
		5a-H	out-of-plane	1136	469	-155	179	51	220	336
Floor Slab @ Elevation 4 ft - 11 1/16 in (Thickness 3 ft - 3 3/8 in)	$D + F + L + T_a + R_a + F_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	3r3u-M	y	35	24	-387	425	214	29	11
		5a-H	out-of-plane	76	170	-100	137	20	154	361
	$D + F + L + T_o + R_o + E'$	4u-M	x	60	58	176	-121	102	14	28
SG/RCP Wing Wall Bottom	$D + F + L + T_a + R_a + F_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	5u-H	x	1507	627	-87	-118	64	122	278
		4u-M	y and out-of-plane	1278	764	98	6	60	308	200
SG/RCP Wing Wall Top	$D + F + L + T_a + R_a + F_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	Fixed 4u-M	x	451	359	219	-48	231	90	91
		4u-M	y	455	542	111	74	67	128	136
		4u-M	out-of-plane	676	307	133	10	23	135	309
SG Separation Wall Bottom	$D + F + L + T_a + R_a + F_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	5u-H	x	430	83	189	-17	50	80	4
		Fixed 4u-M	y	49	19	636	136	189	8	6
		4u-M	out-of-plane	163	154	56	-6	31	104	9

Table 3E.1-22—Governing Design Cases for the SG and RCP Supports and Typical Cavity Wall
Sheet 2 of 2

Location	LC	AC	Controlling Direction	$M_x + M_{xy}$ k-ft/ft	$M_y + M_{xy}$ k-ft/ft	T_x k/ft	T_y k/ft	T_{xy} k/ft	N_x k/ft	N_y k/ft
SG Separation Wall Top	$D + F + L + T_a +$ $R_a + F_a + P_a +$ $R_{rr} + R_{rj} + R_{rm} +$ E'	5u-H	x	950	483	-8	-50	53	148	147
		5u-H	y	678	611	-27	70	27	210	140
		5u-H	out-of-plane	951	484	51	53	57	148	147

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-23—Summary of Reinforcement for SG and RCP Floor Slab

Location	Type	Section Thickness, T	Required A_s (in ² /ft)		Reinforcement Pattern
			Axial +Bending & In-Plane Shear Reinforcement (per face)	Out-of-plane shear (s=6 in)	
Floor Slab @ Elevation 4 ft - 11 ¹ / ₁₆ in	Circumferential	6 ft - 6 ³ / ₄ in	12.35		3 Layers # 14 @ 6 in EF
	Radial		5.28		2 layers # 11 @ 6 in EF
	Stirrups (max)			0.25	#4 @ 6 in EF
Floor Slab @ Elevation 4 ft - 11 ¹ / ₁₆ in	Circumferential	3 ft - 3 ³ / ₈ in	4.85		1st Layer: #10 @ 6 in EF 2nd Layer: # 10 @6 in EF
	Radial		2.41		#10 @ 6 in EF
	Stirrups			0.39	#4 @ 6 in EF

Table 3E.1-24—Typical SG and RCP Cavity Wall Reinforcement
Sheet 1 of 2

Location	Type	Section Thickness, T	Required A _s (in ² /ft)		Reinforcement Pattern
			Axial +Bending & In-Plane Shear Reinforcement (per face)	Out-of-plane shear (s=6 in)	
SG/RCP Wing Wall Bottom	Vertical	3 ft – 11 3/4 in	3.52		1st Layer: #10 @ 6 in EF 2nd Layer: # 10 @ 12 in EF
	Horizontal		6.35		1st Layer: #14 @ 6 in EF 2nd Layer: # 14 @12 in EF
	Stirrups (max)			0.53	#5 @ 6 in
SG/RCP Wing Wall Top	Vertical	3 ft - 3 3/8 in	4.50		1st Layer: #10 @ 6 in EF 2nd Layer: # 10 @ 6 in EF
	Horizontal		7.38		1st Layer: #14 @ 6 in EF 2nd Layer: # 11 @6 in EF
	Stirrups (max)			0.57	#5 @ 6 in
			Axial +Bending & In-Plane Shear Reinforcement (per face)	Out-of-plane shear (s=12 in)	
SG Separation Wall Bottom	Vertical	3 ft - 3 3/8 in	2.14		#10 @ 6 in EF
	Horizontal		2.93		#11 @ 6 in EF
	Stirrups (max)			0.15	#4 @6 in *
			*#4 @ 6 in controls as punching shear check		

Table 3E.1-24—Typical SG and RCP Cavity Wall Reinforcement
Sheet 2 of 2

Location	Type	Section Thickness, T	Required A _s (in ² /ft)		Reinforcement Pattern
			Axial +Bending & In-Plane Shear Reinforcement (per face)	Out-of-plane shear (s=6 in)	
SG Separation Wall Top	Vertical	3 ft - 3 ³ / ₈ in	3.95		1st Layer: #11 @ 6 in EF 2nd Layer: # 11 @12 in EF
	Horizontal		6.72		1st Layer: #14 @ 6 in EF 2nd Layer: # 14 @12 in EF
	Stirrups (max)			0.52	#5 @ 6 in

Table 3E.1-25—Summary of Governing Design Data

Location	LC	AC	$M_x + M_{xy}$	$M_y + M_{xy}$	T_x	T_y	T_{xy}	N_x	N_y
			(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)
Floor Slab	D + F + L + $T_a + R_a + F_a$ + $P_a + R_{rr} + R_{rj} + R_{rm} + E'$	4u-M	108	75	76	21	63	53	65
		5a-H	331	135	55	-6	1	97	114
		Fixed 2sn4u-M	58	79	-10	2	6	110	91
			123	73	-5	18	9	103	108
	D + F + L + $T_o + R_o + E'$	4u-M	31	31	-58	-44	77	38	36
		2sn4u-M	64	211	5	52	13	63	45
Wall Section	D + F + L + $T_a + R_a + F_a$ + $P_a + R_{rr} + R_{rj} + R_{rm} + E'$	Fixed 4u-M	18	17	-47	-50	148	3	3
			26	74	10	92	84	9	15
			33	39	-18	-52	-27	9	45
		1u-S	55	79	38	-17	18	7	22

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-26—Summary of Typical PZR Cubical Reinforcement

Location	Section Thickness, t	Type	Minimum Required As (in ² /ft)		Reinforcement Pattern
			Total Area	Out-of-Plane	
Floor Slab	2 ft - 9 ⁷ / ₁₆ in	Longitudinal	3.12	-	#11 @ 6 in
		Transverse	2.10	-	#11 @ 6 in
		Stirrup	-	0.56	#5 @ 6 in
Wall Section	2 ft - 7 ¹¹ / ₁₆ in	Vertical	2.18	-	#11 @ 6 in
		Horizontal	1.33	-	#11 @ 6 in
		Stirrup	-	0.13	#4 @ 12 in
Floor Slab Under Support	2 ft - 7 ¹ / ₂ in	Parallel	6.24	-	Two layers of #11 @ 6 in
		Perpendicular	4.70	-	One layer of #11 @ 6 in 2 #8 @ each side
		Stirrup	-	0.56	#5 @ 6 in

Table 3E.1-27—Summary of Governing Design Data for the Operating Floor Area

Location	LC	AC	T _x	T _y	T _{xy}	M _x	M _y	M _{xy}	N _x	N _y
RM15	D + F + L + Ta + Ra + Fa + Pa + Rrr + Rrj + Rrm + E'	4u-M	62	140	36	-14	-3	3	8	4
RM22		5u-H	417	-1	60	24	-1	8	-2	2
RM22		4u-M	-38	368	-37	18	17	-21	-5	6
RM16-1		4u-M	143	-725	-113	-9	42	1	-1	-2
RM16-1		4u-M	-131	329	8	-19	-57	33	3	-2
RM16-1		4u-M	15	405	-70	46	47	10	10	-2
RM16-2		4u-M	196	161	-272	-55	-31	6	15	-4
RM16-3		4u-M	93	202	167	-66	-153	-48	18	29
RM16-3		4u-M	281	209	215	11	24	20	5	-1
RM18		4u-M	210	-28	3	-7	27	-9	-28	18
RM18		Fixed 4u-M	-72	160	14	1	17	13	11	15

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-28—Summary of Typical Reinforcement for the Operating Floor Area

Location (Sub-Floor #)	Type	Thickness (T)	Required A _s (in ² /ft)		Reinforcement Pattern
			Axial + Flexural + In-Plane- Shear (per face)	Out-of- Plane Shear	
Slab (RM15)	Radial	2 ft-7 ½ in	0.81		1-#9 @ 12 in T&B
	Tangential		1.66		1-#9 @6 in T&B
	Stirrup			(Not Required)	
Slab (RM22)	Radial	2 ft-7 ½ in	4.82		2-#11 @6 in T&B
	Tangential		4.13		1 st Layer 1-#11 @6 in & 2 nd Layer 1-#11 @12 in T&B
	Stirrup			0.60	1 #5 @ 6 in
Slab (RM16-1)	Radial	3 ft-3 ³ / ₈ in	2.53		1-#11 @6 in T&B
	Tangential		3.67		1 st Layer 1-#11 @6 in & 2 nd Layer 1-#11 @12 in T&B
	Stirrup			0.49	1 #5 @ 6 in
Slab (RM16-2)	E-W	3 ft-3 ³ / ₈ in	5.40		2-#11 @6 in T&B
	N-S		4.68		1 st Layer 1-#11 @6 in & 2 nd Layer 1-#11 @12 in T&B
	Stirrup			(Not Required)	
Slab (RM16-3)	E-W	4 ft-3 ³ / ₁₆ in	5.27		2-#11 @6 in T&B
	N-S		4.49		1 st Layer 1-#11 @6 in & 2 nd Layer 1-#11 @12 in T&B
	Stirrup			0.39	1 #5 @ 6 in
Slab (RM18)	E-W	2 ft-7 ½ in	1.89		1-#9 @6 in T&B
	N-S		1.96		1-#9 @6 in T&B
	Stirrup			(Not Required)	

Table 3E.1-29—Controlling Nodal Forces and Moments for FB Roof

Location	LC	AC	Mx +Mxy	My+Mxy	Tx	Ty	Txy	Nx	Ny
			(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)
FB Roof	D + F + L + H + To + Ro + E'	4u-M	414	257	-20	22	294	85	8
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	101	184	-67	59	137	85	40
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	70	123	7	68	336	31	5
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	80	137	21	38	380	16	17
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	354	240	-18	21	298	73	7

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-30—Controlling Nodal Forces and Moments for SB 2 and 3 Roof

Location	LC	AC	Mx +Mxy	My+Mxy	Tx	Ty	Txy	Nx	Ny
			(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)
SB 2 and 3 Roof	D + F + L + H + To + Ro + E'	3r3u-M	244	183	4	-227	59	97	26
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	5u-H	264	91	158	-146	45	65	102
	D + F + L + H + To + Ro + E'	3r3u-M	781	496	111	-177	58	103	48
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	89	146	-421	240	211	19	32
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	177	140	-432	231	178	25	39
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	5u-H	251	79	232	-219	88	40	40

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-31—Controlling Nodal Forces and Moments for RSB Wall below FB Roof and SB 2 and 3 Roof

Location	LC	AC	Mx +Mxy	My+Mxy	Tx	Ty	Txy	Nx	Ny
			(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)
SB 2 and 3 Wall Below Roof	D + F + L + H + To + Ro + E'	4u-M	69	73	-113	-96	99	27	9
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	2sn4u-M	40	198	-274	97	77	3	47
	D + F + L + H + To + Ro + E'	4u-M	171	18	25	-223	205	29	1
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	Fixed 4u-M	79	54	-125	-52	283	5	5
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	Fixed 4u-M	55	47	141	-5	159	7	4
	D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'	Fixed 4u-M	70	56	-126	-52	285	4	5

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-32—Controlling Nodal Forces and Moments for RSB Wall above FB Roof and SB 2 and 3 Roof

Location	LC	AC	Mx +Mxy	My+Mxy	Tx	Ty	Txy	Nx	Ny
			(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)	(k/ft)
SB 2 and 3 Wall Above Roof	$1.4(D + F) + 1.7(L + H + R_o)$	1u-S	782	409	-260	-308	51	104	18
	$D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'$	2sn4u-M	534	425	-421	117	161	51	14
	$D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'$	4u-M	150	102	-222	88	311	10	1
	$D + F + L + H + Ta + Ra + Pa + Rrr + Rrj + Rrm + E'$	Fixed 4u-M	136	81	93	-76	140	14	1
	$D + F + L + H + To + Ro + E'$	4u-M	423	371	-399	35	162	42	10

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-33—Specified Reinforcement Pattern for RSB Wall to Roof Connection

Location	Type	Thickness (T)	Required Area of steel (in ² /ft)		Reinforcement Pattern
			Total Area of Steel (per face) (axial, bending, in-plane)	Out-of-Plane Shear	
Fuel Roof	Radial	5 ft – 10 ⁷ / ₈ in	4.43	-	2 Layers #10 @ 6 in EF
	Circumferential		4.06	-	2 Layers #10 @ 6 in EF
	Stirrup		-	0.00	#5 @ 6 in
SB 2 and 3 Roof	Radial	5 ft – 10 ⁷ / ₈ in	4.79	-	2 Layers #10 @ 6 in EF
	Circumferential		3.70	-	2 Layers #10 @ 6 in EF
	Stirrup		-	0.00	#5 @ 6 in
SB Wall Below Roof	Vertical	4 ft – 3 ³ / ₁₆ in	2.68	-	#11 @ 6 in EF
	Circumferential		2.79	-	#11 @ 6 in EF
	Stirrup		-	0.00	#5 @ 6 in
SB Wall Above Roof	Vertical	5 ft – 10 ⁷ / ₈ in	4.07	-	2 Layers #10 @ 6 in EF
	Circumferential		2.71	-	2 Layers #10 @ 6 in EF
	Stirrup		-	0.00	#5 @ 6 in

Table 3E.1-34—Summary of Governing Design Data for SB External Walls Below Grade
Sheet 1 of 3

Location	Element	LC	AC	T_x (k/ft)	T_y (k/ft)	T_{xy} (k/ft)	M_{xu} (k-ft/ft)	M_{yu} (k-ft/ft)	N_x (k/ft)	N_y (k/ft)
In-Plane Shear										
A13001	393183	$D + F + L + H + T_a + R_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	5u	-125	-240	281	69	11	16	3
A13003	394270		3r3u	-48	-446	228	3	10	5	5
A33008	426416		2sn4u	-63	-251	293	22	42	14	27
A33003	426271	$D + F + L + H + T_o + R_o + E'$	3r3u	83	-521	253	0	10	7	4
Axial/Bending										
A13001	393192	$D + F + L + H + T_a + R_a + P_a + R_{rr} + R_{rj} + R_{rm} + E'$	5u	-440	20	298	155	53	89	48
	393192		5u	-419	-219	291	217	241	87	19
	393190		3r3u	153	-168	49	14	36	25	22
	393189		3r3u	35	-160	63	65	55	13	8
	393228		2sn4u	14	-572	48	43	176	5	39
	393184		5u	-404	-138	501	216	292	78	109
	393228		Fixed4u-M	4	323	80	6	8	4	8
	393228		5u	14	28	23	14	78	4	26

Table 3E.1-34—Summary of Governing Design Data for SB External Walls Below Grade
Sheet 2 of 3

Location	Element	LC	AC	T _x (k/ft)	T _y (k/ft)	T _{xy} (k/ft)	M _{xu} (k-ft/ft)	M _{yu} (k-ft/ft)	N _x (k/ft)	N _y (k/ft)
A13003	393288	D + F + L + H + T _o + R _o + E'	2u-S	-333	-716	298	16	52	8	22
	394321	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	Fixed5a	-53	-118	89	114	27	20	9
	394269		3r3u	262	-68	131	12	25	6	10
	393292		2u-S	81	-232	126	54	64	12	7
	393288	D + F + L + H + T _o + R _o + E'	4u-M	-232	-974	223	17	47	7	18
	393241	1.4(D + F) + 1.7(L + H + R _o)	1u	-61	-403	26	40	196	2	57
	393289	D + F + L + H + T _o + R _o + E'	Fixed4u-M	43	371	131	6	25	5	16
	393289	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	Fixed4u-M	-16	3	84	18	90	5	36
A33008	425155	D + F + L + H + T _o + R _o + E'	5u	-433	40	210	252	114	152	32
	425153	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	3r3u	-347	-347	455	324	464	132	133
	425156		2sn4u	233	-127	308	49	146	49	32
	425153		4u-M	22	87	228	86	125	41	41
	425197		Fixed4u-M	5	-471	94	31	60	1	15
	425153		3r3u	-347	-347	455	324	464	132	133
	425197		Fixed4u-M	-21	223	91	9	25	1	11
	425153		4u-M	36	67	206	70	127	33	41
A33003	426305	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	4u-M	-244	-234	175	10	14	1	0
	426300		Fixed4u-M	-7	-28	73	64	25	26	1
	426270	D + F + L + H + T _o + R _o + E'	3r3u	394	16	167	23	56	5	21

Table 3E.1-34—Summary of Governing Design Data for SB External Walls Below Grade
Sheet 3 of 3

Location	Element	LC	AC	T _x (k/ft)	T _y (k/ft)	T _{xy} (k/ft)	M _{xu} (k-ft/ft)	M _{yu} (k-ft/ft)	N _x (k/ft)	N _y (k/ft)
	426300	D + F + L + H + T _a + R _a + P _a + R _{rr} +	Fixed5u	16	-33	51	66	13	26	0
	426306	R _{rj} + R _{rm} + E'	2sn4u	-33	-1020	126	14	68	0	2
	425273	1.4(D + F) + 1.7(L + H + R _o)	1u	-20	-439	54	34	185	1	45
	426306	D + F + L + H + T _a + R _a + P _a + R _{rr} +	Fixed4u-M	17	255	105	12	38	0	28
	426306	R _{rj} + R _{rm} + E'	5u	4	64	37	22	103	2	54
Out-of-Plane Shear										
A13001	393190	D + F + L + H + T _a + R _a + P _a + R _{rr} +	5u	-266	-399	424	40	249	35	98
	393192	R _{rj} + R _{rm} + E'	5u	-440	20	298	155	53	40	48
A13003	393292		5u	128	290	33	31	25	22	29
	393295		2sn4u	-57	-464	51	24	33	47	6
A33008	425166	D + F + L + H + T _o + R _o + E'	5u	-211	-335	303	71	238	68	103
	425155		5u	-414	-176	210	292	268	89	38
A33003	426296	D + F + L + H + T _a + R _a + P _a + R _{rr} +	Fixed4u-M	43	108	54	18	64	7	59
	426270	R _{rj} + R _{rm} + E'	3r3u	393	16	169	23	55	5	20

Table 3E.1-35—Reinforcement Design for SG Walls Below Grade (A13001, A13003, A33008, and A33003)

Location	Type	Thickness, T	Required A_s (in ² /ft)				Reinforcement Pattern
			Axial and Bending (per face)	In-Plane Shear (per face)	Total Area of Steel (per face)	Out-of-Plane Shear	
Wall A13001	Vertical	4 ft-11 in	4.21	$5.08/2 = 2.54$	6.75	-	3 layers #14 @ 12 in
	Horizontal		4.21	$4.25/2 = 2.13$	6.34	-	3 layers #14 @ 12 in
	Stirrup		-	-	-	0.12	#5 @ 24 in
Wall A13003	Vertical	4 ft-11 in	4.86	$3.77/2 = 1.89$	6.75	-	3 layers #14 @ 12 in
	Horizontal		4.86	$3.27/2 = 1.64$	6.50	-	3 layers #14 @ 12 in
	Stirrup		-	-	-	0.12	#5 @ 24 in
Wall A33008	Vertical	4 ft-11 in	4.05	$5.39/2 = 2.70$	6.75	-	3 layers #14 @ 12 in
	Horizontal		4.05	$4.48/2 = 2.24$	6.29	-	3 layers #14 @ 12 in
	Stirrup		-	-	-	0.12	#5 @ 24 in
Wall A33003	Vertical	4 ft-11 in	4.34	$4.82/2 = 2.41$	6.75	-	3 layers #14 @ 12 in
	Horizontal		4.34	$4.06/2 = 2.03$	6.37	-	3 layers #14 @ 12 in
	Stirrup		-	-	-	0.12	#5 @ 24 in

Table 3E.1-36—Governing Design Data for the NI Foundation Basemat and RB Internal Structures Base Slab
Sheet 1 of 2

Location	LC	AC	T _x	T _y	T _{xy}	M _x	M _y	M _{xy}	N _x	N _y
			(k/ft)	(k/ft)	(k/ft)	(k-ft/ft)	(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)
Y - Radial Direction										
SB 2 and 3	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	4u-M	0	554	18	0	1275	43	0	73
SB 1		2sn4u	0	394	5	0	1068	38	0	22
SB 4		3r3u	0	444	-223	0	-225	419	0	31
FB		2sn4u	0	-195	395	0	-593	768	0	134
RCB (Radial)	D + L + J + G + P _a + T _a + E' + R _a + R _r	2u	0	1380	70	0	-1520	-5	0	240
RBIS (Transverse)	D + F + L + Ta + Ra + Fa + Pa + Rrr + Rrj + Rrm + E'	1u	0	-841	33	0	-2725	-11	0	26
El. -25 ft - 7 in & -20 ft - 2 in										
RBIS		4u	0	1233	0	0	-872	-993	0	77
El. -25 ft - 7 in & 7 ft - 6 1/2 in										
X - Transverse Direction										
SB 2 and 3	D + F + L + H + T _a + R _a + P _a + R _{rr} + R _{rj} + R _{rm} + E'	4u	308	0	171	721	0	283	-36	0
SB 1		2sn4u	563	0	46	1344	0	74	-69	0
SB 4		2u	135	0	-245	444	0	319	-49	0
FB		2sn4u	55	0	354	26	0	79	358	0
RCB (Transverse)	D + L + J + G + P _a + T _a + E' + R _a + R _r	1u	318	0	-566	-439	0	586	-509	0

Table 3E.1-36—Governing Design Data for the NI Foundation Basemat and RB Internal Structures Base Slab
Sheet 2 of 2

Location	LC	AC	T_x	T_y	T_{xy}	M_x	M_y	M_{xy}	N_x	N_y
			(k/ft)	(k/ft)	(k/ft)	(k-ft/ft)	(k-ft/ft)	(k-ft/ft)	(k/ft)	(k/ft)
RBIS (Radial)	D + F + L + Ta + Ra + Fa + Pa + Rrr + Rrj + Rrm + E'	2sn4u	-1002	0	124	-2714	0	-165	-25	0
El. -25 ft - 7 in & -20 ft - 2 in										
RBIS		4u	1233	0	0	-872	0	-993	77	0
El. -25 ft - 7 in & 7 ft - 6 1/2 in										

*LC refers to the governing load combination; AC refers to the governing soil analysis case.

Table 3E.1-37—Reinforcement Summary for the NI Foundation Basemat and RB Internal Structures Base Slab
Sheet 1 of 2

Location	Type	Thickness (in.)	Required A_s (in ² /ft)			Reinforcement Pattern
			Combined Bending Moment + Axial Load	In-Plane Shear	Out-of-Plane Shear	
SB 2 and 3 Basemat	Vertical	118.1	4.5	0.76	0.14	2 Layers #18 @ 12 in EF
	Horizontal		8.0	0.0	0.70	2 Layers #18 @ 12 in EF
	Stirrup		-		0.79	#8 @ 12 in EF
SB 1 Basemat	Vertical	118.1	8.0	0.0	0.42	2 Layers #18 @ 12 in EF
	Horizontal		6.75	0.0	0.26	2 Layers #18 @ 12 in EF
	Stirrup		-		0.79	#8 @ 12 in EF
SB 4 Basemat	Vertical	118.1	4.5	1.79	0.66	2 Layers #18 @ 12 in EF
	Horizontal		6.0	2.16	0.12	2 Layers #18 @ 12 in EF
	Stirrup		-		0.79	#8 @ 12 in EF
FB Basemat	Vertical	118.1	2.0	2.96	0.81	2 Layers #18 @ 12 in EF
	Horizontal		4.5	3.34	0.582	2 Layers #18 @ 12 in EF
	Stirrup		-		0.79	#8 @ 12 in EF
RCB Basemat	Radial	130	16.5	0.06	0.64	4 Layers #18 @ 12 in EF
	Transverse		6.24	7.94	0.84	4 Layers #18 @ 12 in EF
	Stirrup		-		0.79	#8 @ 12 in EF
RB Internal Structures Base Slab (El. -25 ft-7 in to -20 ft-2 in)	Vertical	65	4.0	0.52	0.46	3 Layers #14 @ 12 in EF
	Horizontal		6.75	0.00	1.33	3 Layers #14 @ 12 in EF
	Stirrup		-		1.76	#6 @ 6 in EF (U-Shaped)

Table 3E.1-37—Reinforcement Summary for the NI Foundation Basemat and RB Internal Structures Base Slab
Sheet 2 of 2

Location	Type	Thickness (in.)	Required A_s (in ² /ft)			Reinforcement Pattern
			Combined Bending Moment + Axial Load	In-Plane Shear	Out-of- Plane Shear	
RB Internal Structures Base Slab (El. -25 ft-7 in to -7 ft-6 ½ in)	Vertical	216	14.75	0.0	0.19	3 Layers #18 @ 12 in EF + Additional one layer of #18 @ 18 in EF
	Horizontal		14.75	0.0	0.19	3 Layers #18 @ 12 in EF + Additional one layer of #18 @ 18 in EF
	Stirrup		-		0.20	#4 @ 12 in EF

Table 3E.1-38—Floor Dead and Live Loads
Sheet 1 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	US (ft)	Y-Coordinate Metric (m)	US (ft)		
SAFEGUARD BUILDING DIVISION 1											
-9.60M (-31.5ft)	1UJH01-002	122.33	27.50	point	dead	-31.85	-104.50	11.12	36.50		
	1UJH01-006	110.76	24.90	point	dead	-32.92	-108.00	-2.16	-7.10		
	1UJH01-024	104.53	23.50	point	dead	-45.17	-148.20	3.35	11.00		
	1UJH01-026	141.90	31.90	point	dead	-47.40	-155.50	-9.45	-31.00	150	
	1UJH01-008									200	
	1UJH01-005									125	
	1UJH01-027									150	
-5.00M (-16.4ft)	1UJH05-005	233.98	52.60	point	dead	-36.58	-120.50	7.01	23.00		
	1UJH05-008									250	
	1UJH05-026									175	
0.00	1UJH10-001	160	35.97	point-- free action	live						
		170	38.22	point-- free action	live						
	1UJH10-026	415.02	93.30	point	dead	-47.40	-155.50	-5.79	-19.00	150	
		250	56.21	point-- free action	live						
		250	56.21	point-- free action	live						
	1UJH10-004									175	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 2 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
+4.70M (+15.4ft)	1UJK14-025	160	35.97	point-- free action	live						
		120	26.98	point-- free action	live						
		120	26.98	point-- free action	live						
	1UJK14-026	150	33.72	point	live	-44.50	-146.00	-5.75	-18.86		
		100	22.48	point	live	-50.29	-165.00	-5.75	-18.86		
	1UJK14-027	130	29.23	point	live	-51.82	-170.00	-10.15	-33.30		
	1UJK14-029									175	
	1UJK14-028									225	
+8.10M (+26.6ft)	1UJK18-024									175	
	1UJK18-025									175	
	1UJK18-026	129.89	29.20	point	dead	-29.87	-98.00	5.49	18.00	225	
		97.42	21.90	point	dead	-31.70	-104.00	12.50	41.00		
	1UJK18-027	104.53	23.50	point	dead	-48.16	-158.00	5.49	18.00	200	
+12.00M (39.4ft)	1UJK22-028	86.30	19.40	point	dead	-47.40	-155.50	-6.40	-21.00	150	
	1UJK22-030									150	
	1UJK22-039									150	
	1UJK22-047									175	
	1UJK22-057									175	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 3 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	1UJK22-046									175	
	1UJK22-056									175	
	1UJK22-042									175	
	1UJK22-043									175	
	1UJK22-044									175	
	1UJK22-052									175	
	1UJK22-053									175	
	1UJK22-054									175	
	1UJK22-045									175	
	1UJK22-055									175	
+16.80M (55.1ft)	1UJK26-028	100	22.48	point-- free action	live						
	1UJK26-030	89.45	20.11	point	dead	-35.87	-117.70	11.96	39.25	200	
	2UJE26-001	130.24	29.28	point	dead	-30.97	-101.60	12.34	40.50	150	
+21.00M (68.9ft)	1UJK31-030									150	
	1UJE26-001									175	
	2UJE26-001									150	
	1UJE29-002	207.38	46.62	point	dead	-35.59	-116.75	-3.11	-10.20		
	2UJE29-002	207.38	46.62	point	dead	-35.59	-116.75	3.11	10.20		
+24.70M (+81.0ft)	2UJE34-003									225	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 4 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	1UJK34-025									175	
	1UJK34-031									225	
	1UJE34-002									125	
	2UJE34-002									125	
	1UJE34-023	160	35.97	point	dead	-48.62	-159.51	9.38	30.77	325	
+29.30M (96.1ft)	1UJE39-001	92.08	20.70	point	dead	-30.11	-98.80	-4.51	-14.80	75	100
	2UJE39-001	92.08	20.70	point	dead	-30.11	-98.80	4.57	15.00	75	100
		100	22.48	point-- free action	live	-43.34	-142.19	10.05	32.97		
SAFEGUARD BUILDING DIVISION 2/3											
-9.60M (-31.5ft)	2UJH01-007	122.33	27.50	point	dead	-17.83	-58.50	27.22	89.30		
	2UJH01-009	110.76	24.90	point	dead	-3.35	-11.00	30.21	99.10		
	2UJH01-020	141.90	31.90	point	dead	-15.09	-49.50	46.02	151.00	150	
	2UJH01-024	118.32	26.60	point	dead	-3.20	-10.50	45.05	147.80		
	3UJH01-007	122.33	27.50	point	dead	17.80	58.40	27.22	89.30		
	3UJH01-009	110.76	24.90	point	dead	3.29	10.80	30.21	99.10		
	3UJH01-020	141.90	31.90	point	dead	14.78	48.50	45.11	148.00	150	
	3UJH01-024	118.32	26.60	point	dead	3.05	10.00	45.05	147.80		
	2UJH01-011									225	
	3UJH01-011									225	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 5 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
-5.00M (-16.4ft)	2UJH05-006	233.98	52.60	point	dead	-14.73	-48.50	35.60	116.00	150	
	3UJH05-006	233.98	52.60	point	dead	14.48	47.50	35.60	116.00	150	
	2UJH05-020									175	
	3UJH05-020									175	
0.00	2UJH10-003	250	56.21	point-- free action	live						
	3UJH10-003	250	56.21	point-- free action	live						
	2UJH10-006	250	56.21	point-- free action	live						
	2UJH10-010	260	58.45	point-- free action	live						
										125	
	2UJH10-020	123.66	27.80	point	dead	-15.70	-51.50	45.11		150	
		415.02	93.30	point	dead	-12.04	-39.50	45.72			
		250	56.21	point-- free action	live						
	3UJH10-006	250	56.21	point-- free action	live						
	3UJH10-020	123.66	27.80	point	dead	16.92	55.50	45.11	148.00	150	
		415.02	93.30	point	dead	12.04	39.50	45.72	150.00		

Table 3E.1-38—Floor Dead and Live Loads
Sheet 6 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
		250	56.21	point-- free action	live						
	2UJH10-002									150	
	3UJH10-002									150	
+4.70M (+15.4ft)	2UJK14-002	160	35.97	point-- free action	live						
	3UJK14-002	160	35.97	point-- free action	live						
+8.10M (+26.6ft)	2UJK18-005									175	
	3UJK18-005									175	
	2UJK18-020									200	
	3UJK18-020									200	
	2UJK18-002	104.53	23.50	point	dead	-22.25	-73.00	25.91	85.00	200	
	3UJK18-002	104.53	23.50	point	dead	23.47	77.00	25.91	85.00	200	
	2UJK18-029									150	
	3UJK18-029									150	
+12.00M (+39.4ft)	2UJK22-028									200	
	3UJK22-028									200	
+16.30M (+53.5ft)	2UJK26-002									225	
	3UJK26-002									225	
	2UJK26-020									175	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 7 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	3UJK26-020									175	
	2UJK26-030									225	
	2UJK26-046									225	
	3UJK26-005									225	
+21.00M (+68.9ft)	2UJK31-007									150	
	3UJK31-007									150	
	2UJK31-026									150	
	3UJK31-026									150	
	2UJK31-035									150	
	3UJK31-035									150	
	2UJK31-034									150	
	3UJK31-034									150	
	2UJK31-003									150	
	3UJK31-003									150	
	2UJK31-032									150	
	3UJK31-032									150	
	2UJK31-029									150	
	3UJK31-029									150	
	2UJK31-021	160	35.97	point	dead	-20.01	-65.65	47.16	154.72		
	3UJK31-021	160	35.97	point	dead	20.01	65.65	47.16	154.72		

Table 3E.1-38—Floor Dead and Live Loads
Sheet 8 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
		Metric (m)	US (ft)								
+28.80M (+94.5ft)										75	100
SAFEGUARD BUILDING DIVISION 4											
-9.60M (-31.5ft)	4UJH01-002	122.33	27.50	point	dead	31.49	103.30	11.13	36.50		
	4UJH01-006	110.76	24.90	point	dead	32.61	107.00	-2.16	-7.10		
	4UJH01-024	104.53	23.50	point	dead	45.11	148.00	3.35	11.00		
	4UJH01-026	141.90	31.90	point	dead	46.18	151.50	-9.45	-31.00	150	
	4UJH01-005									125	
	4UJH01-008									200	
	4UJH01-027									150	
-5.00M (-16.4ft)	4UJH05-005	233.98	52.60	point	dead	36.42	119.50	7.01	23.00		
	4UJH05-012	195.71	44.00	point	dead	31.09	102.00	-10.82	-35.50		
	4UJH05-026									175	
0.00	4UJH10-026	415.02	93.30	point	dead	47.09	154.50	-5.79	-19.00	150	
		332.73	74.80	point	dead	48.31	158.50	-10.06	-33.00		
		250	56.21	point--free action	live						
		250	56.21	point--free action	live						
	4UJH10-001	160	35.97	point--free action	live						
		160	35.97	point--free action	live						
	4UJH10-004									175	
+4.70M (+15.4ft)	4UJK14-027	130	29.23	point	live	51.82	170.00	-10.15	-33.30		

Table 3E.1-38—Floor Dead and Live Loads
Sheet 9 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
		100	22.48	point	live	50.27	164.93	-5.75	-18.86		
	4UJK14-026	150	33.72	point	live	44.50	146.00	-5.75	-18.86		
	4UJK14-025	160	35.97	point--free action	live						
	4UJK14-028									225	
	4UJK14-029									175	
+8.10M (+26.6ft)	4UJK18-024									175	
	4UJK18-025									175	
	4UJK18-026	129.89	29.20	point	dead	29.87	98.00	5.49	18.00	225	
		97.42	21.90	point	dead	31.70	104.00	12.19	40.00		
	4UJK18-027	104.53	23.50	point	dead	47.85	157.00	5.79	19.00	200	
+12.00M (+39.4ft)	4UJK22-028	86.30	19.40	point	dead	45.57	149.50	-6.40	-21.00	150	
	4UJK22-030									150	
	4UJK22-039									150	
	4UJK22-042									175	
	4UJK22-043									175	
	4UJK22-044									175	
	4UJK22-045									175	
	4UJK22-046									175	
	4UJK22-052									175	
	4UJK22-053									175	

Table 3E.1-38—Floor Dead and Live Loads
Sheet 10 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	4UJK22-054									175	
	4UJK22-055									175	
	4UJK22-056									175	
+16.80M (+55.1ft)	4UJK26-028	100	22.48	point--free action	live						
	4UJK26-030	89.45	20.11	point	dead	35.87	117.70	11.96	39.25	200	
	3UJE26-001	130.24	29.28	point	dead	30.97	101.60	12.34	40.50	150	
+21.00M (+68.9ft)	4UJK29-030									150	
	4UJE26-001									175	
	3UJE26-001									150	
	4UJE29-002	207.38	46.62	point	dead	-35.59	-116.75	-3.11	-10.20		
	3UJE29-002	207.38	46.62	point	dead	-35.59	-116.75	3.11	10.20		
+24.70M (+81.0ft)	4UJK34-025									175	
	4UJK34-030									225	
	4UJK34-031									225	
	3UJE33-002									125	
	4UJE33-002									125	
	4UJE34-023	160	35.97	point	dead	48.62	159.51	9.38	30.77		
+29.30M (+96.1ft)	4UJE39-001	92.08	20.70	point	dead	30.08	98.70	-4.51	-14.80	75	100
	3UJE39-001	92.08	20.70	point	dead	30.08	98.70	4.57	15.00	75	100
		100	22.48	point--free action	live	43.34	142.19	10.05	32.97		

Table 3E.1-38—Floor Dead and Live Loads
Sheet 11 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
FUEL BUILDING											
-9.60M (-31.5ft)	UFA01-078	160.14	36.00	point	live	9.60	31.50	-44.55	-146.12		
	UFA01-010	160.14	36.00	point	live	-22.28	-73.08	-25.76	-84.50		
	UFA01-004									125	
	UFA01-033									125	
	UFA01-085									125	
	UFA01-083									125	
	UFA01-035									125	
	UFA01-095									125	
	UFA01-061									125	
	UFA01-054									125	
	UFA01-082									125	
	UFA01-075									125	
	UFA01-026									325	
	UFA01-038									300	
	UFA01-072									275	
	UFA01-073									275	
	UFA01-088									300	
	UFA01-042									375	
	UFA01-048									325	

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA01-049									325	
	UFA01-076									300	
	UFA01-077									300	
	UFA01-097									375	
	UFA01-098									350	
	UFA01-099									350	
	UFA01-024										750
	UFA01-001										125
	UFA01-017										100
	UFA01-051										125
	UFA01-079										100
-6.20M (-20.3ft)	UFA05-026	482.19i	108.40	point	dead	-17.53	-57.50	-43.89	-144.00		
	UFA05-076	482.19	108.40	point	dead	19.05	62.08	-44.32	--145.42		
	UFA05-004									125	
	UFA03-001									125	125
	UFA03-002									125	
	UFA03-007									125	
	UFA03-006									125	
	UFA03-056									125	
	UFA03-051									125	125

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA03-058									125	
	UFA03-057									125	
	UFA05-054									125	
	UFA03-017										100
	UFA03-051										125
	UFA03-079										100
	UFA05-077										2600
-3.40M (-11.2ft)	UFA06-035	98.75	22.20	point	dead	-2.59	-8.50	-35.02	-115.00	125	
										300	
	UFA06-085	98.75	22.20	point	dead	-6.55	-21.50	-35.02	-115.00	125	
										300	
	UFA06-038									1220	
	UFA06-088									1220	
	UFA06-001									125	125
	UFA06-023									125	
	UFA06-024									125	
	UFA06-045									125	
	UFA06-095									125	
	UFA06-082									125	525
	UFA06-083									125	525

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA06-075									125	
	UFA06-074									125	
	UFA06-051									125	125
	UFA06-094									125	
	UFA06-084									125	
	UFA05-051										125
	UFA06-017										100
	UFA06-079										100
	UFA06-096										125
0.00	UFA10-084	259.78	58.40	point	dead	7.16	23.50	-33.83	-111.00		
	UFA10-057	230	51.71	point-- free action	live						
	UFA10-015	285	64.07	point--free action	live	-16.17	-53.05	-33.98	-111.48		200
		285	64.07	point--free action	live	-16.17	-53.05	-34.75	-114.01		
		285	64.07	point--free action	live	-16.17	-53.05	-35.52	-116.54		
		285	64.07	point--free action	live	-16.17	-53.05	-36.29	-119.06		
		285	64.07	point--free action	live	-16.17	-53.05	-37.06	-121.59		
		285	64.07	point--free action	live	-16.17	-53.05	-37.83	-124.11		
		285	64.07	point--free action	live	-16.17	-53.05	-38.60	-126.64		
		285	64.07	point--free action	live	-16.17	-53.05	-39.37	-129.17		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
		1401.19	315.00	point-- free action	live						
	UFA10-002									125	
	UFA10-001									125	125
	UFA10-045									125	
	UFA10-021									125	
	UFA10-022									125	
	UFA10-035									125	
	UFA10-085									125	
	UFA10-007									125	
	UFA10-082									125	
	UFA10-070									125	
	UFA10-071									125	
	UFA10-095									125	
	UFA10-051									125	125
	UFA10-017										100
	UFA10-079										100
+3.70M (+12.1ft)	UFA10-015	100	22.48	point-- free action	live						
		100	22.48	point-- free action	live						

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA13-070	1252.17	281.50	point	dead	20.27	66.50	-34.14	-112.00		
	UFA13-071	1252.17	281.50	point	dead	20.27	66.50	-39.62	-130.00		
	UFA15-022									1920	
	UFA13-004									125	
	UFA15-001									125	125
	UFA15-045									125	
	UFA13-057										200
	UFA13-075									125	
	UFA13-056										200
	UFA13-083									125	
	UFA13-082									125	
	UFA13-085									125	
	UFA13-080									125	
	UFA15-096									125	
	UFA13-095									125	
	UFA13-051									125	125
	UFA13-054									125	
	UFA13-072										200
	UFA15-016									125	
	UFA15-002									125	

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA13-017										100
	UFA13-079										100
+7.40M (+24.3ft)	UFA18-015	125	28.10	point-- free action	live					125	
		125	28.10	point-- free action	live						
	UFA18-004									125	
	UFA18-045									125	
	UFA17-025									125	
	UFA17-053									125	
	UFA17-057										200
	UFA17-017										100
	UFA17-051										125
	UFA17-079										100
	UFA18-001										125
	UFA19-021										1960
+11.10M (+36.4ft)	UFA21-081										200
	UFA21-082										200
	UFA21-083										200
	UFA21-084										200
		300	67.45	point	dead	-12.65	-41.50	-34.15	-112.04		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
		300	67.45	point	dead	0.29	0.95	-33.39	-109.55		
	UFA21-004									125	
	UFA21-002									125	
	UFA21-001									125	125
	UFA21-045									125	
	UFA21-015									125	
	UFA21-095									125	
	UFA21-051									125	125
	UFA21-053									125	
	UFA21-057										200
	UFA21-056									125	
	UFA21-097	158.35	35.60	point	dead	13.52	44.50	-44.81	-147.00		
	UFA21-017										100
	UFA21-079										100
+14.80M (+48.5ft)	UFA23-014									2450	
	UFA23-018									2600	
	UFA24-004									125	
	UFA24-001									125	125
	UFA24-045									125	
	UFA24-002									125	

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
	UFA23-015									125	
	UFA24-057										200
	UFA24-071									125	
	UFA24-086									125	
	UFA24-085									125	
	UFA24-070									125	
	UFA24-051									125	125
	UFA24-053									125	
	UFA24-017										100
	UFA24-079										100
	UFA15-022										
+19.5M (+64.0ft)	UFA29-015	200	44.96	point	live	-9.03	-29.63	-47.36	-155.38	125	225
		222.76	50.08	Point	Dead	1.83	6.00	-31.91	-104.70		
		222.76	50.08	Point	Dead	1.83	6.00	-41.51	-136.20		
	UFA29-001									125	125
	UFA29-005									125	
	UFA29-045									125	
	UFA29-090									125	225
	UFA29-054									125	
	UFA29-051									125	125

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
+24.20M (+79.4ft)	UFA10-069	470	105.67	point-- free action	live						
		470	105.67	point-- free action	live						
		120	26.98	point-- free action	dead	15.70	51.51	-51.05	-167.49		
		120	26.98	point-- free action	dead	23.95	78.58	-51.05	-167.49		
	UFA29-015	120	26.98	point-- free action	live-max						
		120	26.98	point-- free action	live-min						
	UFA34-010	180	40.47	point	dead	-22.03	-72.26	-27.13	-88.99		
	UFA35-078	180	40.47	point	dead	11.41	37.43	-44.76	-146.85		
	UFA34-001									125	125
	UFA34-045									125	
+34.00M (+111.5ft)	UKA	120	26.98	point-- free action	live	15.80	51.84	-28.95	-94.98	150	100
		120	26.98	point-- free action	live	15.80	51.84	-41.20	-135.17		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	US (ft)	Y-Coordinate Metric (m)	US (ft)		
REACTOR BUILDING											
-6.15M (-20.2ft)	UJA04-003	152.50	34.29	point	dead	10.77	35.32	-6.64	-21.77		
		152.50	34.29	point	dead	-10.77	-35.32	-6.64	-21.77		
		127.66	28.70	point	dead	13.66	44.80	-3.40	-11.16		
		127.66	28.70	point	dead	-13.66	-44.80	-3.40	-11.16		
		108.89	24.48	point	dead	12.18	39.97	2.20	7.21		
		108.89	24.48	point	dead	-12.18	-39.97	2.20	7.21		
		108.89	24.48	point	dead	5.11	16.75	12.19	40.00		
		108.89	24.48	point	dead	-5.11	-16.75	12.19	40.00		
		114.14	25.66	point	dead	-10.86	-35.63	7.51	24.64		
		114.14	25.66	point	dead	10.86	35.63	7.51	24.64		
	UJA04-002	1979.36	445.00	pint	dead	4.05	13.29	-9.34	-30.63		
		1979.36	445.00	pint	dead	-5.51	-18.08	-9.78	-32.08		
		1979.36	445.00	pint	dead	-0.63	-2.08	-14.67	-48.13		
-2.30M (-7.5ft)	UJA07-013	110	24.73	point--free action	live						
	UJA07-015	110	24.73	point--free action	live						
	UJA07-016	125	28.10	point--free action	live						
	UJA07-018	234.42	52.70	point	dead	-0.15	-0.50	10.36	34.00		
		195.28	43.90	point	dead	-0.46	-1.50	14.02	46.00		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate		Y-Coordinate			
						Metric (m)	US (ft)	Metric (m)	US (ft)		
		195.28	43.90	point	dead	-0.46	-1.50	15.55	51.00		
	UJA07-021	226	50.81	point	dead	-5.82	-19.09	-18.25	-59.86	300	
	UJA07-024									400	
	UJA07-026	328	73.74	point	dead	5.82	19.09	-18.25	-59.86	350	
	UJA07-027	328	73.74	point	dead	-2.89	-9.48	-18.25	-59.86	350	
	UJA07-020									325	
	UJA07-023									350	
+1.50M (+4.9ft)	UJA11-016	170	38.22	point--free action	live					175	125
	UJA11-019	636.10	143.00	point	dead	-0.15	-0.50	14.94	49.00		
	UJA11-024	127.66	28.70	point	dead	4.42	14.50	-14.63	-48.00		
		362	81.38	point	dead	9.81	32.18	-14.65	-48.05		
	UJA11-032	155	34.85	point	dead	17.94	58.84	0.26	0.85		
	UJA11-031	155	34.85	point	dead	-17.94	-58.84	0.26	0.85		
	Airlock	300	67.45	point	dead	0.00	0.00	24.24	79.51		
	UJA11-025									175	125
	UJA11-026									175	125
	UJA11-027									175	125
	UJA11-028									175	125
	UJA11-015									175	125
	UJA11-013									175	125

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
	UJA11-014									175	125
	UJA11-020									175	125
+5.15M (+16.9ft)	UJA15-013	1098.71	247.00	point	dead	-17.83	-58.50	-8.23	-27.00		
		135	30.35	point	dead	-17.29	-56.71	-1.16	-3.80		
	UJA15-014	1098.71	247.00	point	dead	-16.31	-53.50	10.97	36.00		
		135	30.35	point	dead	-17.29	-56.71	1.16	3.80		
	UJA15-015	1098.71	247.00	point	dead	16.31	53.50	10.97	36.00		
		135	30.35	point	dead	17.29	56.71	1.16	3.80		
	UJA15-016	1098.71	247.00	point	dead	17.53	57.50	-8.23	-27.00		
		135	30.35	point	dead	17.29	56.71	-1.16	-3.80		
	UJA15-002	115.5	25.97	point	dead	-5.70	-18.70	-12.75	-41.82		
	UJA15-005	150	33.72	point	dead	-10.24	-33.59	14.18	46.51		
	UJA15-006	150	33.72	point	dead	10.24	33.59	14.18	46.51		
	UJA15-023	110	24.73	point	live	-2.64	-8.66	-14.34	-47.04		
	UJA15-001	1860	418.17	point	dead	-3.81	-12.50	-5.23	-17.15		
		1860	418.17	point	dead	3.81	12.50	-5.23	-17.15		
	UJA15-021									350	
+8.70M (+28.5ft)	UJA18-015	161	36.20	point	dead	11.12	36.47	14.11	46.28	100	
	UJA18-014	161	36.20	point	dead	-10.75	-35.26	14.35	47.07	100	
	UJA18-020	150	33.72	point	dead	-5.56	-18.24	-13.34	-43.76		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
	UJA15-021	130	29.23	point	live	2.31	7.58	-12.99	-42.61		
		130	29.23	point	live	3.50	11.48	-14.94	-49.00		
	UJA15-023	130	29.23	point	live	-1.60	-5.25	-12.57	-41.23		
	UJA15-001	410	92.18	point	live	2.25	7.38	-12.33	-40.44		
		130	29.23	point	live	3.50	11.48	-10.38	-34.05		
	UJA18-013									100	
	UJA18-016									100	
+13.80M (+45.3ft)	UJA23-041									300	
	UJA23-013									50	
	UJA23-014									75	
	UJA23-015									75	
	UJA23-016									50	
+19.50M (+64.0ft)	UJA	200	44.96	point	dead	-3.82	-12.53	1.26	4.13		
		200	44.96	point	dead	-3.82	-12.53	-0.15	-0.49		
		200	44.96	point	dead	-3.82	-12.53	-1.71	-5.61		
		200	44.96	point	dead	-3.82	-12.53	-3.14	-10.30		
		200	44.96	point	dead	-3.82	-12.53	-4.68	-15.35		
		200	44.96	point	dead	-3.82	-12.53	-6.24	-20.47		
		200	44.96	point	dead	-3.82	-12.53	-7.75	-25.43		
		200	44.96	point	dead	-3.82	-12.53	-9.15	-30.02		

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
		200	44.96	point	dead	-3.82	-12.53	-10.51	-34.48		
		200	44.96	point	dead	-3.82	-12.53	-11.62	-38.12		
		200	44.96	point	dead	-3.46	-11.35	3.42	11.22		
		200	44.96	point	dead	-2.59	-8.50	4.63	15.19		
		200	44.96	point	dead	3.82	12.53	1.26	4.13		
		200	44.96	point	dead	3.82	12.53	-0.15	-0.49		
		200	44.96	point	dead	3.82	12.53	-1.71	-5.61		
		200	44.96	point	dead	3.82	12.53	-3.14	-10.30		
		200	44.96	point	dead	3.82	12.53	-4.68	-15.35		
		200	44.96	point	dead	3.82	12.53	-6.24	-20.47		
		200	44.96	point	dead	3.82	12.53	-7.75	-25.43		
		200	44.96	point	dead	3.82	12.53	-9.15	-30.02		
		200	44.96	point	dead	3.82	12.53	-10.51	-34.48		
		200	44.96	point	dead	3.82	12.53	-11.62	-38.12		
		200	44.96	point	dead	3.46	11.35	3.42	11.22		
		200	44.96	point	dead	2.59	8.50	4.63	15.19		
	UJA29-016	685	154.00	point	dead	12.07	39.60	-20.91	-68.60		835
		1125	252.92	point	dead	14.08	46.19	-13.88	-45.54		
		640	143.88	point	dead	8.58	28.15	-9.80	-32.15		
	UJA29-013										835

Table 3E.1-38—Floor Dead and Live Loads
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Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
		640	143.88	point	dead	-8.58	-28.15	-9.80	-32.15		
		171.87	38.64	point	dead	-7.83	-25.70	-15.78	-51.78		
		171.87	38.64	point	dead	-8.42	-27.62	-14.37	-47.14		
		171.87	38.64	point	dead	-9.83	-32.26	-13.78	-45.22		
		171.87	38.64	point	dead	-11.25	-36.90	-14.37	-47.14		
		171.87	38.64	point	dead	-11.83	-38.82	-15.78	-51.78		
		171.87	38.64	point	dead	-11.25	-36.90	-17.20	-56.42		
		171.87	38.64	point	dead	-9.83	-32.26	-17.78	-58.34		
		171.87	38.64	point	dead	-8.42	-27.62	-17.20	-56.42		
		228.14	51.29	point	dead	4.18	13.70	-16.98	-55.70		
		228.14	51.29	point	dead	-4.8	-13.70	-16.98	-55.70		
	UJA29-023	270	60.70	point	dead	-19.35	-63.48	-16.15	-52.99		
+24.10M (+79.0ft)	UJA34-018	130	29.23	point	dead	-3.98	-13.06	10.53	34.55		
		130	29.23	point	dead	-1.84	-6.04	15.10	49.54		
		130	29.23	point	dead	-6.10	-20.01	13.86	45.47		
	UJA34-014	120	26.98	point	dead	-16.68	-54.72	-12.92	-42.39	275	
		445	100.04	point	dead	-11.97	-39.27	13.81	45.31		
	UJA34-015									275	
+28.50M (+93.5ft)	UJA	300	67.45	point	dead	12.70	41.67	13.86	45.47		835
		150	33.72	point	dead	6.05	19.85	19.10	62.66		

Table 3E.1-38—Floor Dead and Live Loads
Sheet 27 of 27

Elevation	Room Number	Magnitude of Load		Load Description	Load Type	Application				Area Dead Load (psf)	Area Live Load (psf)
		Metric (kN)	US (kips)			X-Coordinate Metric (m)	X-Coordinate US (ft)	Y-Coordinate Metric (m)	Y-Coordinate US (ft)		
		200	44.96	point	dead	-4.79	-15.72	19.53	64.07		
		230	51.71	point	dead	-9.20	-30.18	9.79	32.12		
		230	51.71	point	dead	9.20	30.18	9.79	32.12		
		650	146.13	point	dead	4.29	14.07	13.56	44.49		
	UJA41-003	128	28.78	point	dead	-10.33	-33.89	-7.43	-24.38		
	UJA41-004	128	28.78	point	dead	-10.33	-33.89	7.43	24.38		
	UJA41-008	128	28.78	point	dead	10.33	33.89	-7.43	-24.38		
	UJA41-007	128	28.78	point	dead	10.33	33.89	7.43	24.38		
+34.45M (+113ft)	SG Platform										1253

Table 3E.1-39—Minimum Factors of Safety for the Nuclear Island Common Basemat Structure

Analysis Case	Sliding			Overturning			Flotation	
	Required	Calculated X-DIR	Calculated Y-DIR	Required	Calculated X-DIR	Calculated Y-DIR	Required	Calculated
5ae-h	1.1	1.1	1.1	1.1	1.7	2.0		
4ue-m	1.1	1.1	1.1	1.1	1.7	1.9		
1n2ue-s	1.1	1.9	1.9	1.1	2.6	2.9		
1n5ae-h	1.1	1.2	1.1	1.1	1.7	2.0		
hfub	1.1	3.8	3.7	1.1	5.6	5.3		
hflb	1.1	4.1	4.0	1.1	6.7	7.7		
hfbe	1.1	3.6	3.8	1.1	6.0	6.7		

**Table 3E.1-40—Maximum Static and Dynamic Bearing Pressures for the NI
Common Basemat Structure**

Analysis Case	Static [ksf] (Dead Load)	Dynamic [ksf] (Seismic plus Dead Load)
	Edge	Edge
1n2ue	23.1	30.3
1n2ue-cr	23.1	30.2
1n5ae	18.9	31.4
1n5ae-cr	18.9	27.1
2sn4ue	21.9	30.1
2sn4ue-cr	21.9	30.5
4ue	22.9	31.9
4ue-cr	22.9	30.0
5ae	21.1	26.9
5ae-cr	21.1	26.8
hfbe	18.8	24.1
hfbe-cr	18.8	22.2
hflb	18.9	23.9
hflb-cr	18.9	23.6
hfub	18.4	23.4
hfub-cr	18.4	22.5
Maximum	23.1	31.9

Notes:

1. Analysis cases indicated with “-cr” represent the cracked case.

Table 3E.1-41—Nuclear Island Critical Sections
Sheet 1 of 2

Section	Description of Critical Section
NI 1	Reactor Containment Building—Typical Liner Plate
NI 2	Reactor Containment Building—Typical Cylinder Wall and Buttress
NI 3	Reactor Containment Building—Typical Dome and Dome Ring Areas
NI 4	Reactor Containment Building—Typical Connection of Containment Wall to Nuclear Island Basemat
NI 5	Reactor Containment Building—Equipment Hatch Area
NI 6	Reactor Building Internal Structures—Typical Primary Shield Wall / Reactor Vessel Support Area
NI 7	Nuclear Island Basemat, including tendon gallery, & Reactor Building Internal Structures Baseslab
NI 8	Reactor Building Internal Structures— Typical Operating Floor Slab Area
NI 9	Reactor Building Internal Structures—Typical Elevation 4'-11" Heavy Slab & Support Walls
NI 10	Reactor Building Internal Structures—Typical SG Cubicle Area Walls & Slabs
NI 11	Safeguard Building 2/3 Hardened Shell—Typical Walls from Top of Nuclear Island Basemat to Grade
NI 12	Safeguard Building 2/3 Internal Structures—Typical Exterior Walls from Top of Nuclear Island Basemat to Elevation 15'-5"
NI 13	Safeguard Building 2/3 Internal Structures—Typical Floor Slab at Elevation -16'-5" & 0'-0"
NI 14	Safeguard Buildings 1 and 4—Typical Main Steam and Feedwater Valve Room Walls and Slabs
NI 15	Safeguard Buildings 1 and 4—Typical Exterior Walls from Top of Nuclear Island Basemat to Elevation 15'-5"
NI 16	Fuel Building Hardened Shell—Typical Walls from Top of Nuclear Island Basemat to Elevation 12'-0"
NI 17	Fuel Building Internal Structures—Typical Major Walls from Top of Nuclear Island Basemat to Bottom of Spent Fuel Pool Slab
NI 18	Fuel Building Internal Structures—Typical Spent Fuel Pool Walls and Floor Slab
NI 19	Reactor Shield Building —Typical Wall Areas & Connection Between Reactor Shield Building Wall and Safeguard / Fuel Building Roof Slabs
NI 20	Reactor Shield Building—Typical Dome to Wall Transition Areas
NI 21	Fuel Building Hardened Shell—Typical Roof Slab Areas for Material Lock Room & Support Walls
NI 22	Fuel Building Internal Structures—Typical Floor Slab Above Spent Fuel Pool Area & Support Walls
NI 23	Reactor Shield Building—Lower Portion Typical Wall Areas

Table 3E.1-41—Nuclear Island Critical Sections
Sheet 2 of 2

Section	Description of Critical Section
NI 24	Nuclear Island—Typical Columns and Beams
NI 25	Reactor Containment Building—Typical Airlock and MS/FW Penetrations
NI 26	Fuel Building Internal Structures—Fuel Transfer Tube
NI 27	Vent Stack

Figure 3E.1-1—ANSYS Analysis Results for Nuclear Island Elements

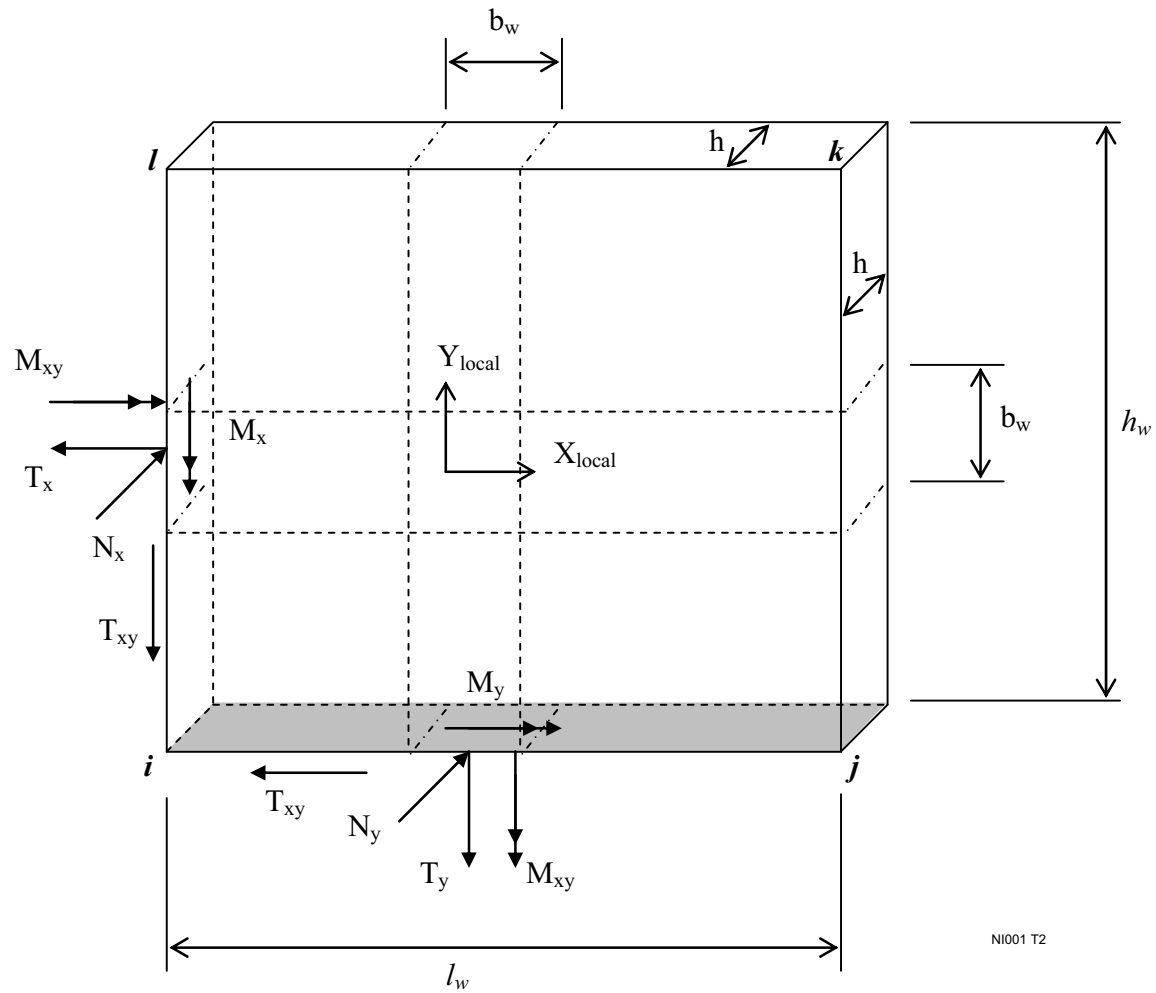


Figure 3E.1-2—Gusset Section of RCB

Figure 3E.1-3—Cross-Section of Gusset

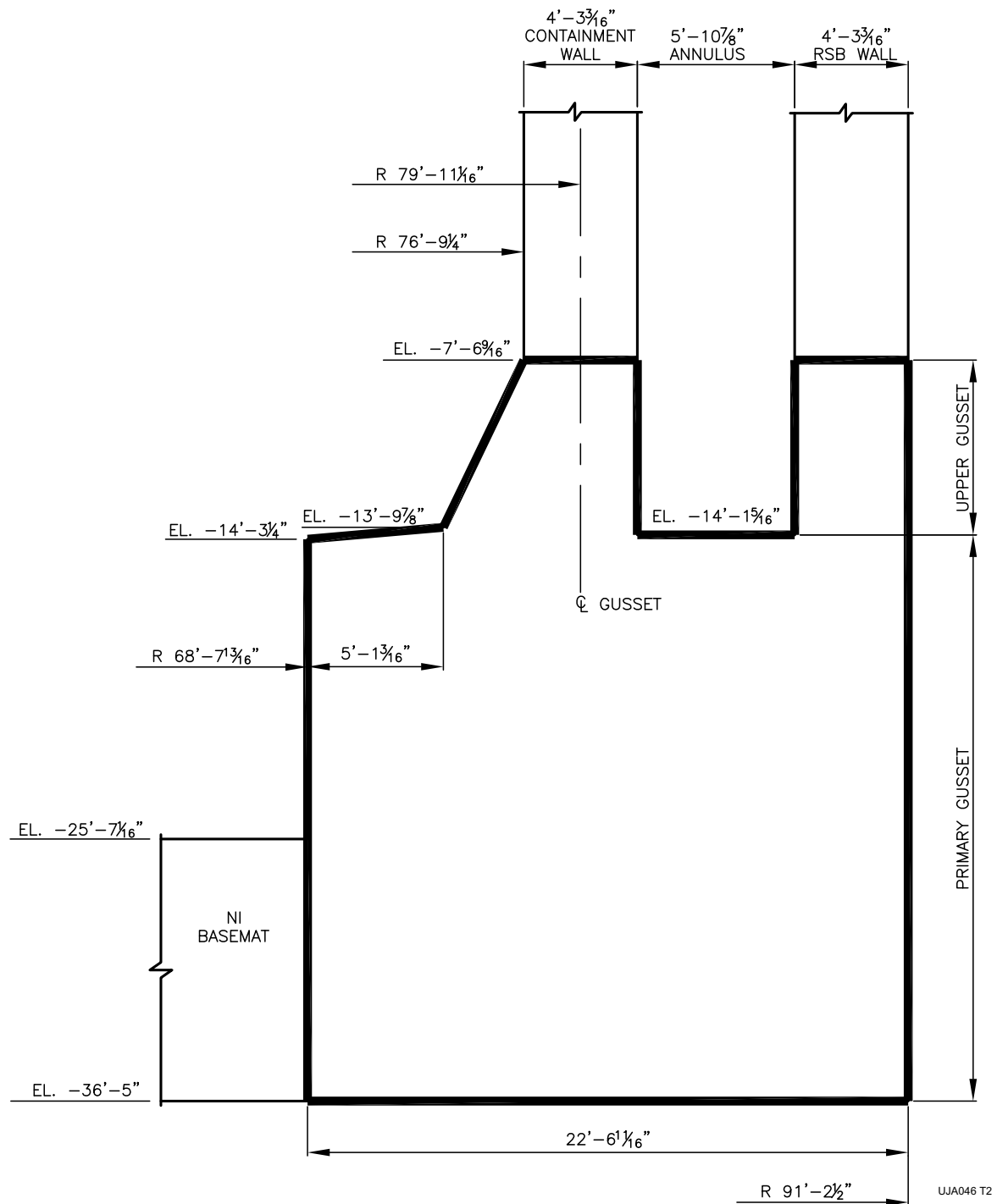
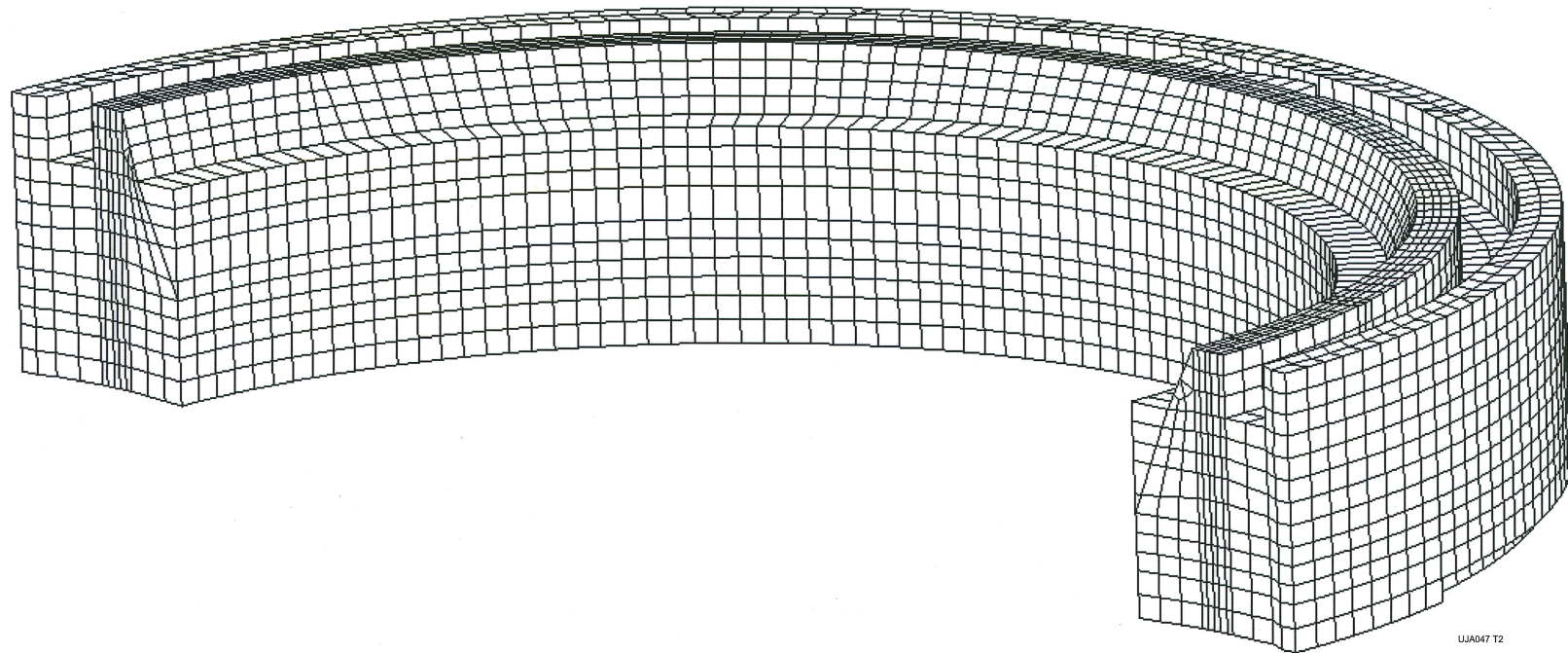


Figure 3E.1-4—180° FEM Gusset Segment of Containment Foundation



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Figure 3E.1-5—Gusset Section - Typical Reinforcement

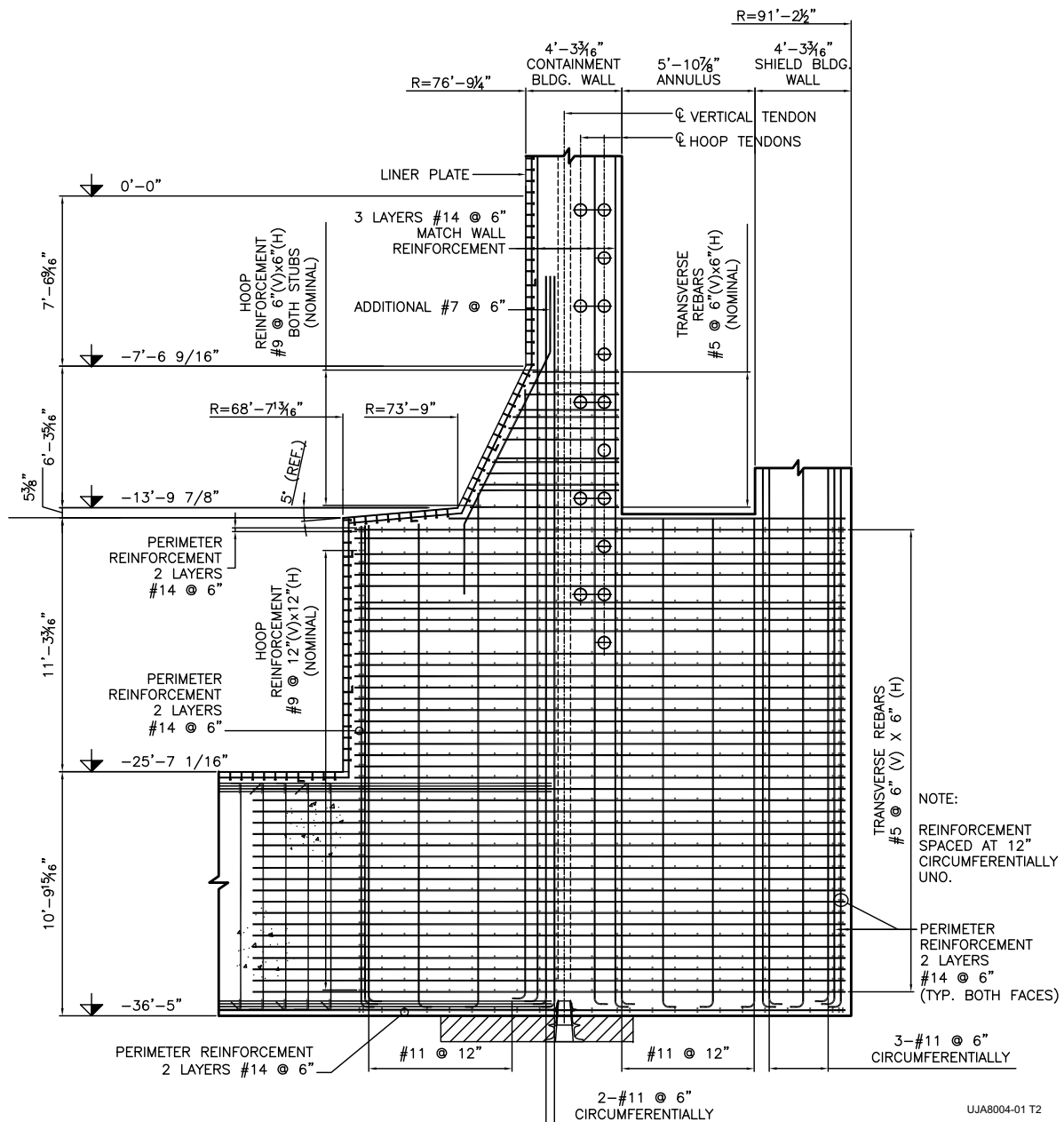


Figure 3E.1-6—Plan View of Equipment Hatch Area

Figure 3E.1-7—FEM of Equipment Hatch Area - Outer View

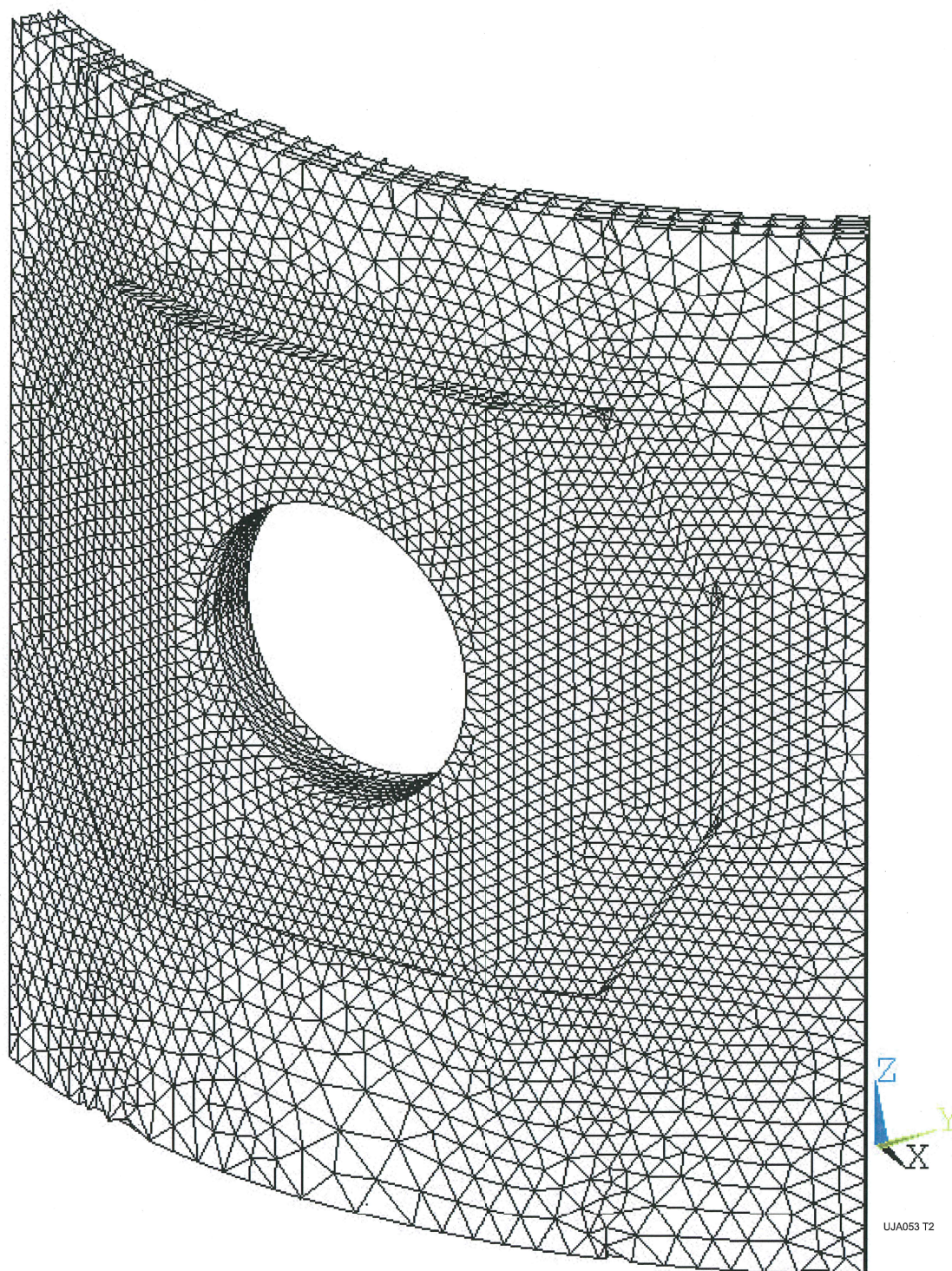
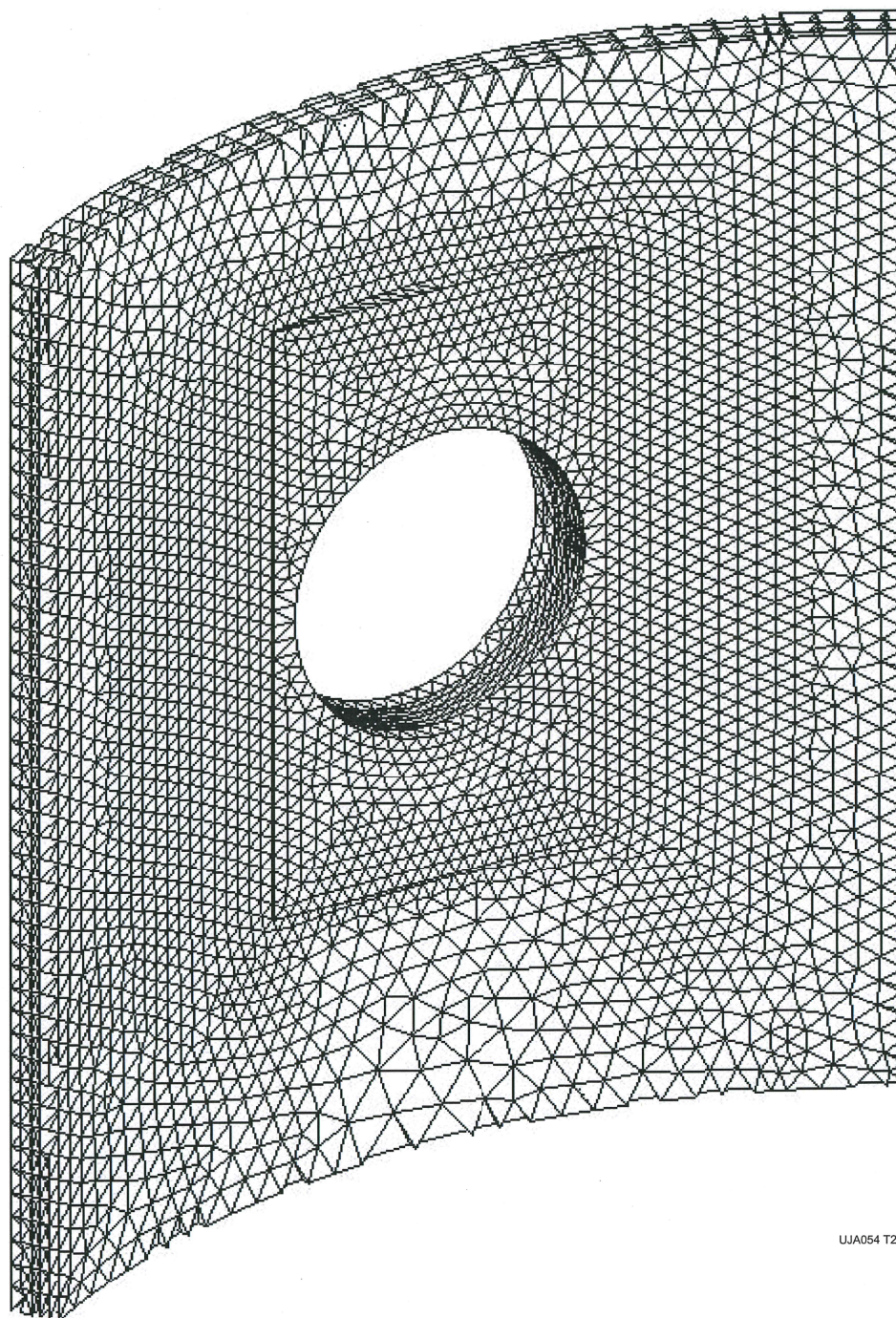


Figure 3E.1-8—FEM of Equipment Hatch Area - Inner View



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Figure 3E.1-9—Cross-Section of Equipment Hatch Area

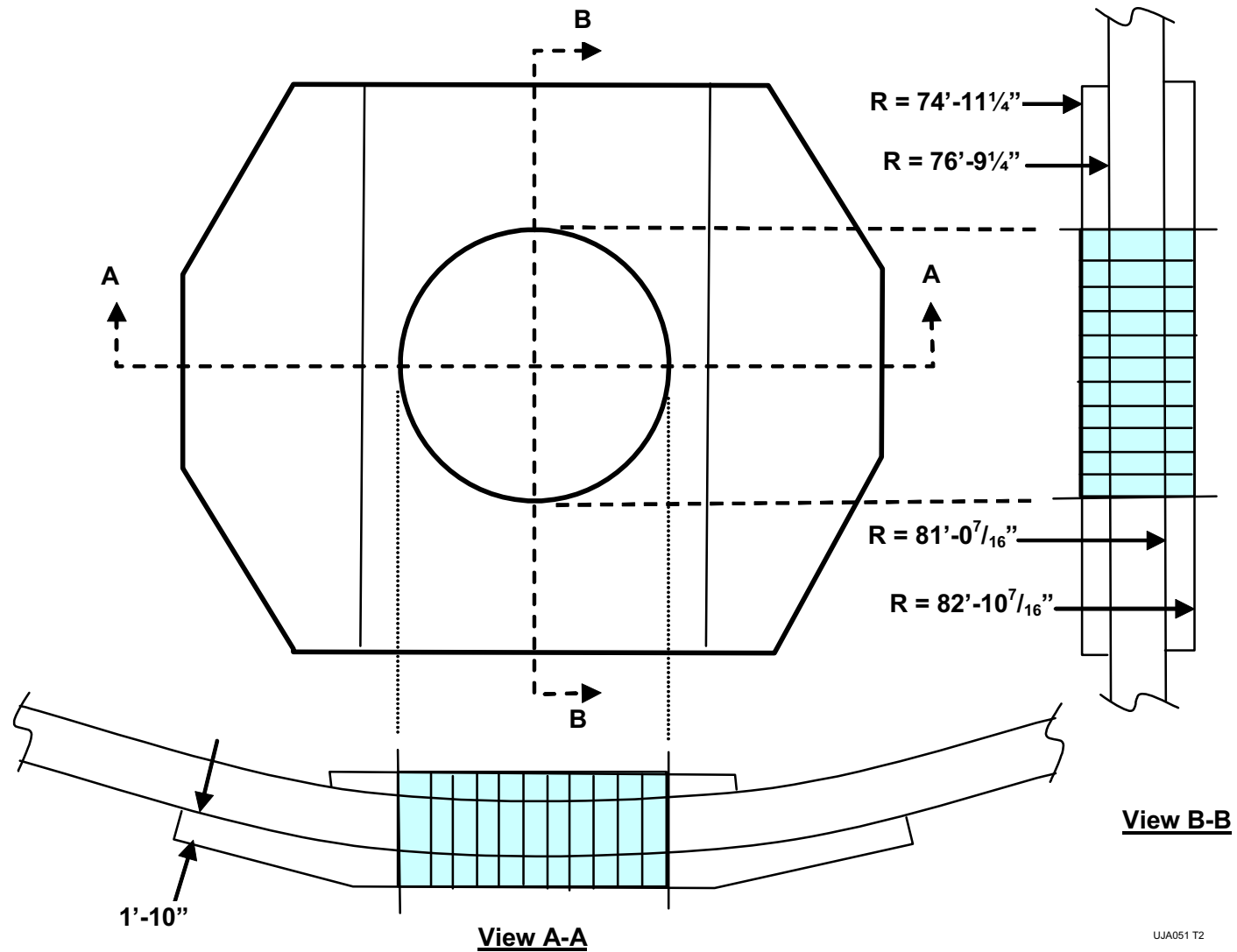


Figure 3E.1-10—Elevation View of Equipment Hatch Area Showing Cuts

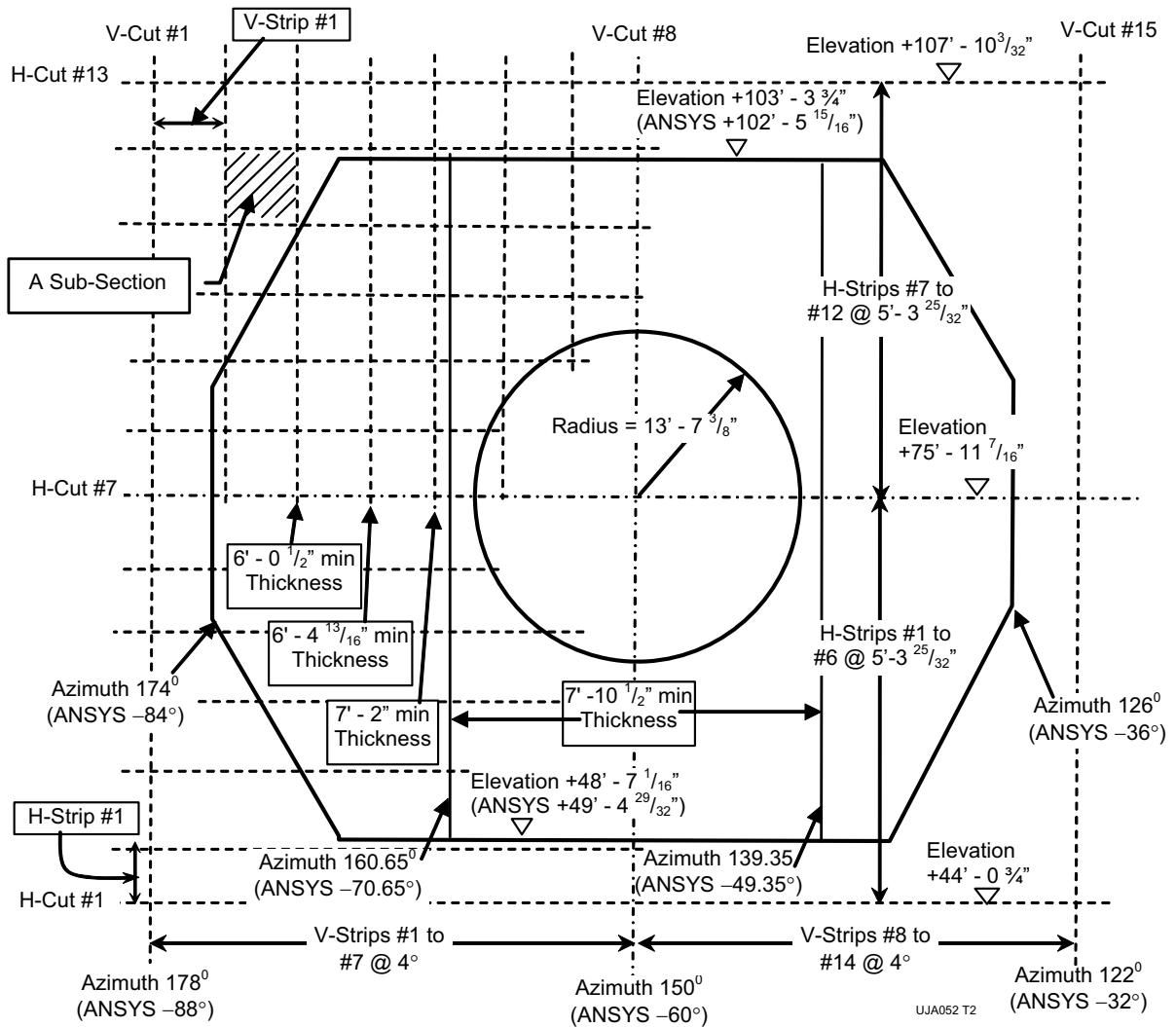


Figure 3E.1-11—Sections 1-1 and 2-2 of the Equipment Hatch Area

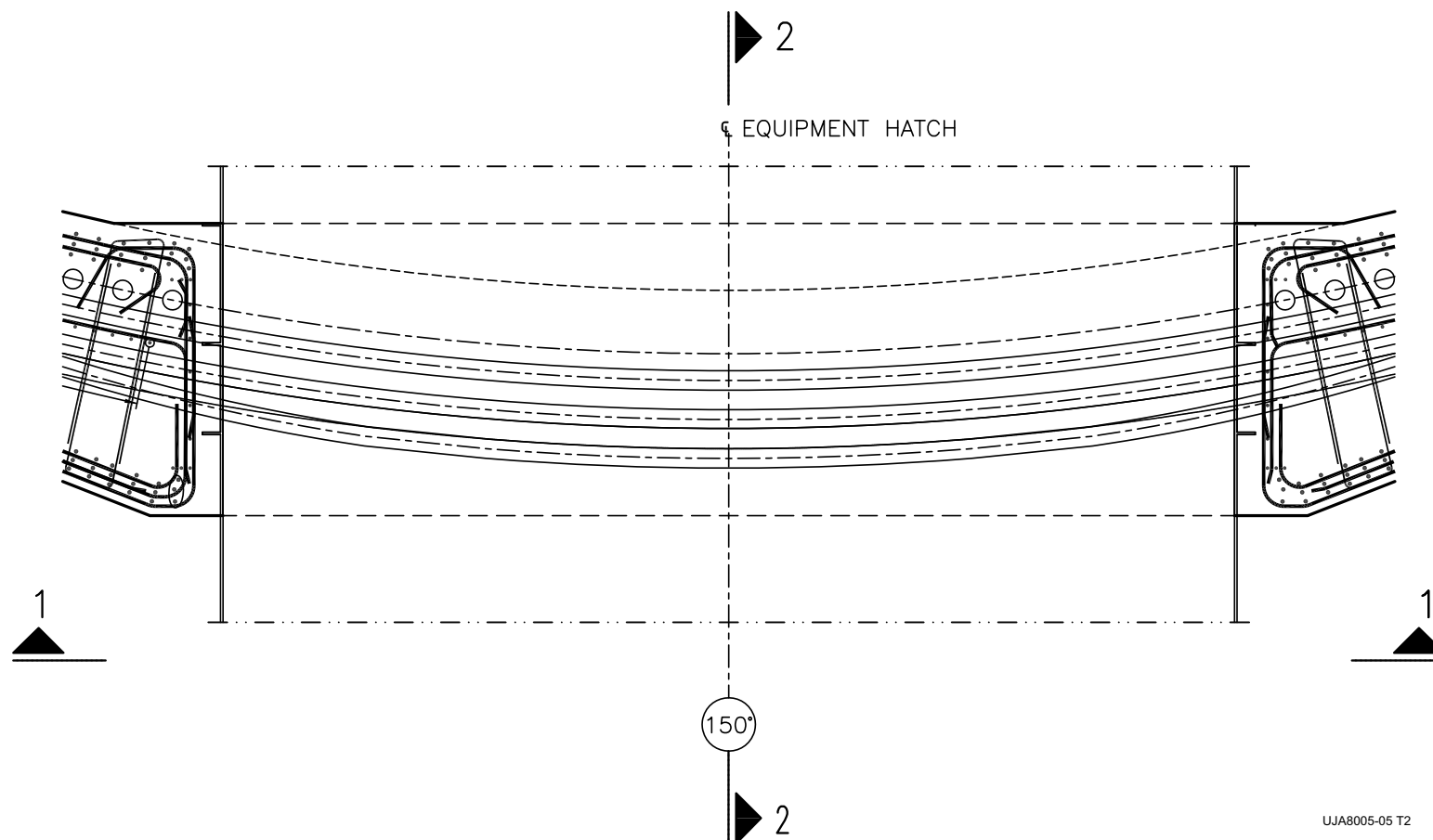


Figure 3E.1-12—Reinforcement Pattern for Section 1-1 of the Equipment Hatch Area

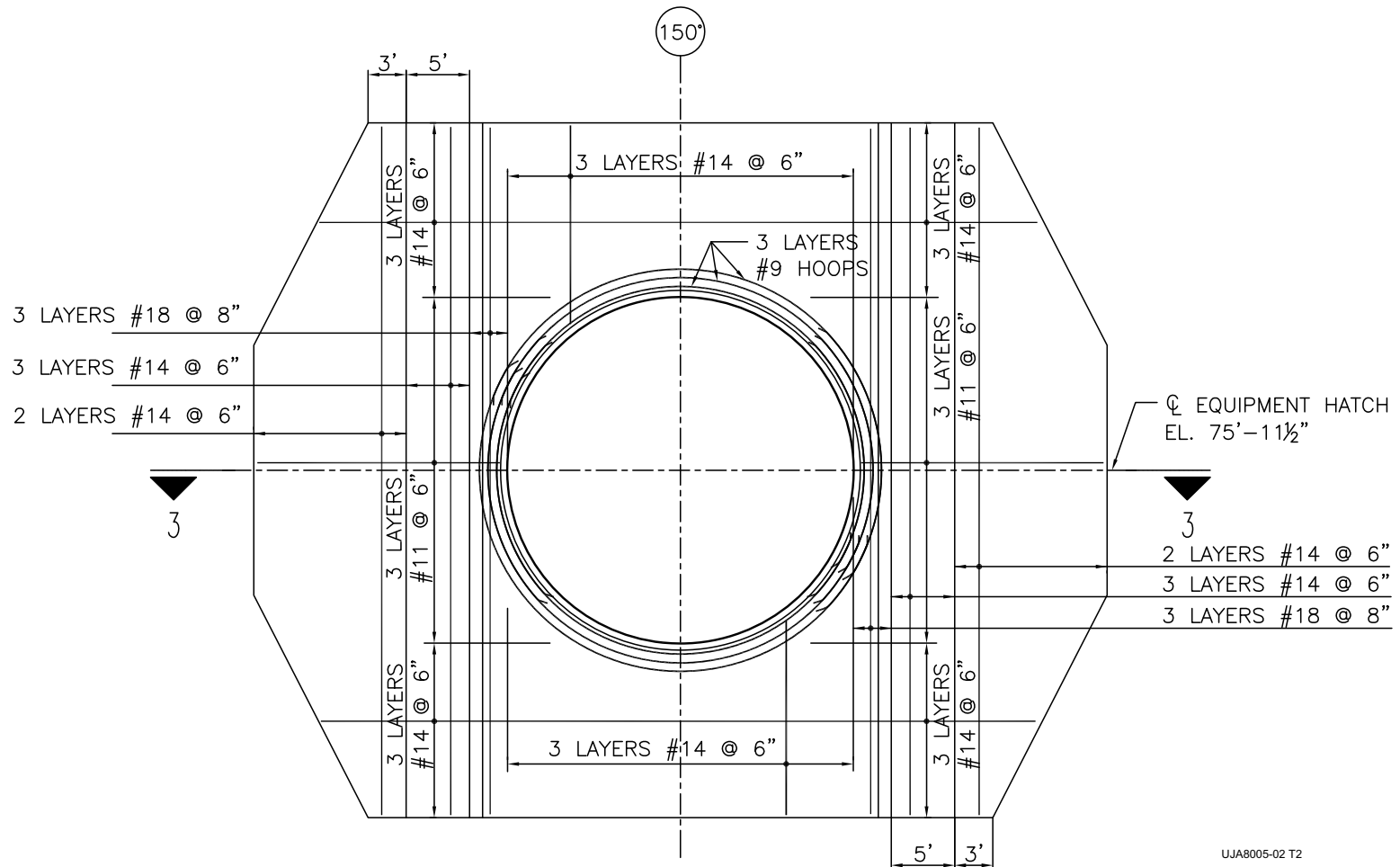


Figure 3E.1-13—Reinforcement Pattern for Section 2-2 of the Equipment Hatch Area

