

LABORATORY NOTEBOOK

CNWRA / SWRI

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Scientific Notebook # 263
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INSTRUCTIONS

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 - Avoid making notes on loose paper to be recycled.
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6. This notebook and its contents are the exclusive property of the Company. It is confidential and the contents are not to be disclosed to anyone unless authorized by the Company. You must return it when completed, upon request, or upon termination of employment. It should be kept in a protected place.
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Two Dimensional Repository Scale Thermal-Mechanical Model

- Objective: To examine spatial and temporal variations of tunnel stability at the repository horizon considering spatial variations of rock-mass quality (derived from ESF mapping) and possible temporal variations caused by degradation of *intact* rock under sustained long-term loading and wall-rock weathering of rock joints.
1. Analysis will be based on a north-south vertical 2D section along the exhaust main (drift).
 2. Section is about 3,500 m long north to south. Each model will be executed in two stages because of the large vertical extent required to simulate the appropriate thermal and mechanical boundary conditions: First, a model extending over a vertical height of about 1,000 m (elevation 500 to 1,500 m above sea level), referred to as the *full model*, will be analyzed using a coarse mesh to develop the histories of temperature and vertical displacement at the top and base of the second, smaller model. The vertical height of the second model, referred to as the *submodel*, will be limited; its exact top and base elevations will be determined from preliminary analyses of the full model.
 3. Only the emplacement drifts will be represented in the model. Each emplacement drift will be modeled as a square opening separated from adjacent drifts by pillars.
 4. Each analysis will consist of a heat-conduction stage followed by a thermal-mechanical stage based on linear elasticity. Mechanical analyses of the submodel will include processing of the calculated stresses using a failure criterion to identify zones of overstress.
 5. Rock-mass strength will be modeled using the Hoek-Brown failure criterion, with parameters calculated using NGI Q values developed by the DOE based on ESF data. Rock-mass degradation due to joint wall-rock weathering will be represented through a degradation of Q values.
 6. The hypothesized rock-mass degradation process is likely to depend on time and the availability of moisture, warm temperature, and oxygen. However, the dependence of rock-mass quality on these factors is currently not known quantitatively. We will be able to estimate a long-term rock-mass quality from the current Q values and our hypothesis of rock-mass degradation from wall-rock weathering.
 7. Our strategy for evaluation of the analysis results is to examine tunnel stability (through over-stress analyses) at selected times using both the current and estimated long-term rock-mass quality (Q) values. Any differences between the tunnel stability calculated using the current and long-term Q values may help determine possible impacts of rock-mass degradation on repository design and performance.

Investigator

Initial: GLO

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Problem Geometry

Pages 2-11 entries

by GUD 3/2/98

Problem geometry was determined using information from two DOE documents:

(1) Repository Thermal Loading Management Analysis.

CRWMS/M&O Document Number

B00000000-01717-0200-00135 Rev 00

(GUD 3/2/98)
Document referred to hereafter as
RTLM-97.

(2) Repository Subsurface Layout Configuration Analysis.

CRWMS/M&O Document Number

BCA000000-01717-0200-00008 Rev 00

Document referred to hereafter as
RSLC-97.

Information Used to Set Problem Geometry

- (1) Emplacement drift diameter is 5.5 m
Concrete lining thickness = 0.2 m
Therefore internal diameter of emplacement drift is 5.1 m.

[p. 21 of RSLC-97]

- (2) For the selected thermal loading of 85 MTU/acre [RTLM-97, p. 25] the drift (center-to-center) spacing is 28 m [RTLM-97, p. 38] (GUD 3/2/98)

- (3) The minimum waste-package spacing is 13.26 m to accommodate ~~can~~ (GUD 3/2/98) HLW packages between the commercial SNF packages.

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- (4) Length of emplacement drifts required on a year-by-year basis is given on Table 7.7-1, p. 54, of RTLM-97. The total length of emplacement drift required is 107,150 m.
- (5) Average usable length for individual emplacement drifts is 1080 m [RTLM-97, p. 53].
- (6) The average usable length of 1080 m per drift implies that 100 drifts will be required to provide the total drift length of 107,150 m.
- (7) The proposed repository layout gives number of emplacement drifts as 105 with ~~an~~ (GUD 3/2/98) additional 15 drifts for contingency [RSLC-97, p. 33] (GUD 3/2/98) Fig. 7-1. 100 drifts will be modeled.

Summary of Problem Geometry

- (1) Problem domain is 2800 m long (south to north). Domain will be extended 200 m to the south and 200 m to the north to reduce truncation of the temperature and displacement fields. Zero-perturbation conditions (no normal displacement and no temperature change) will be applied at the north and south extended boundaries.

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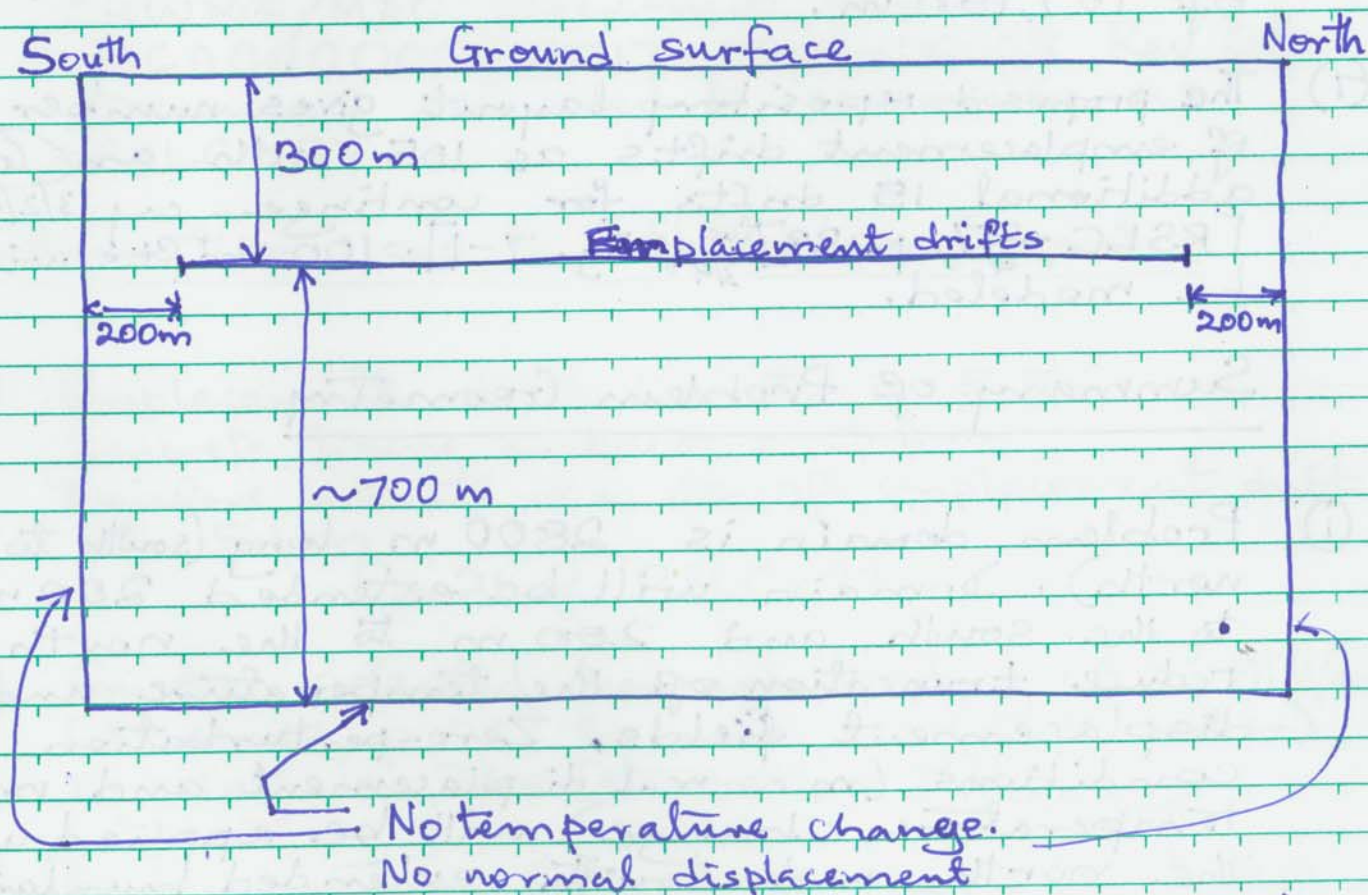
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- (2) The top of the model domain, i.e., the ground surface, will be assumed to be horizontal.
- (3) Emplacement drifts will be placed at a depth of about 300 m below the ground surface.
- (4) The base of the model will be placed at about 700 m below the repository (1000 m below the ground surface) to reduce truncation of temperature and displacement fields. Zero perturbation conditions will be applied at the model base.



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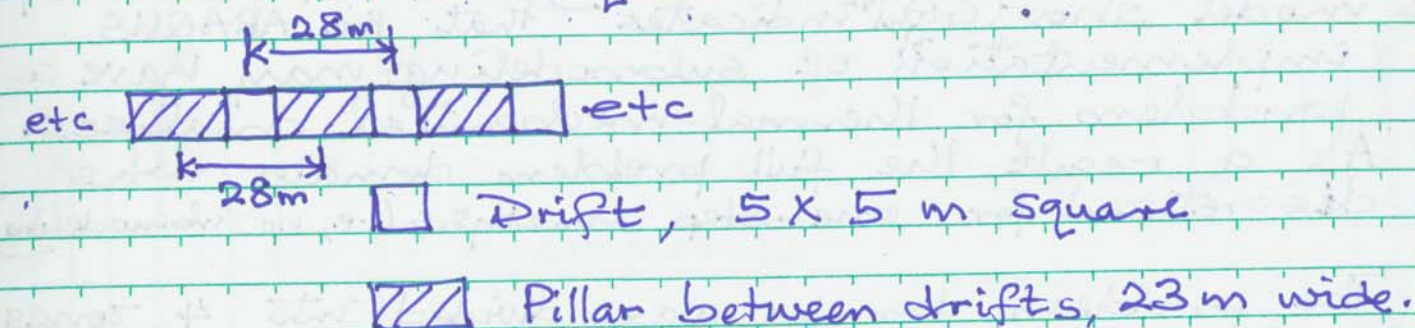
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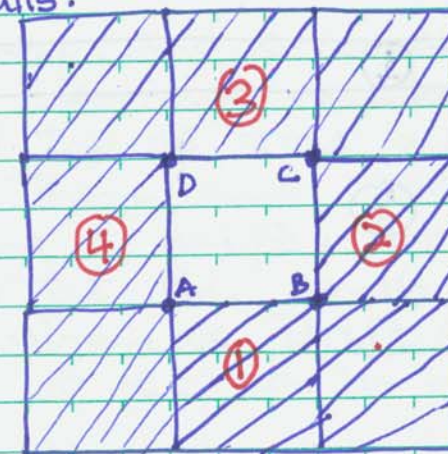
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- (5) Each emplacement drift will be represented as a 5 x 5 m square.



Tunnel Lining

To simplify model, tunnel lining will be simulated using beam elements placed on the tunnel walls.



AB, BC, AD, and DC are beam elements as well as the near-tunnel edges of elements ①, ②, ④, and ③, respectively.

Each of the fictitious beams will be assigned relatively high Young's modulus and low Poisson's ratio to simulate stiff tunnel lining.

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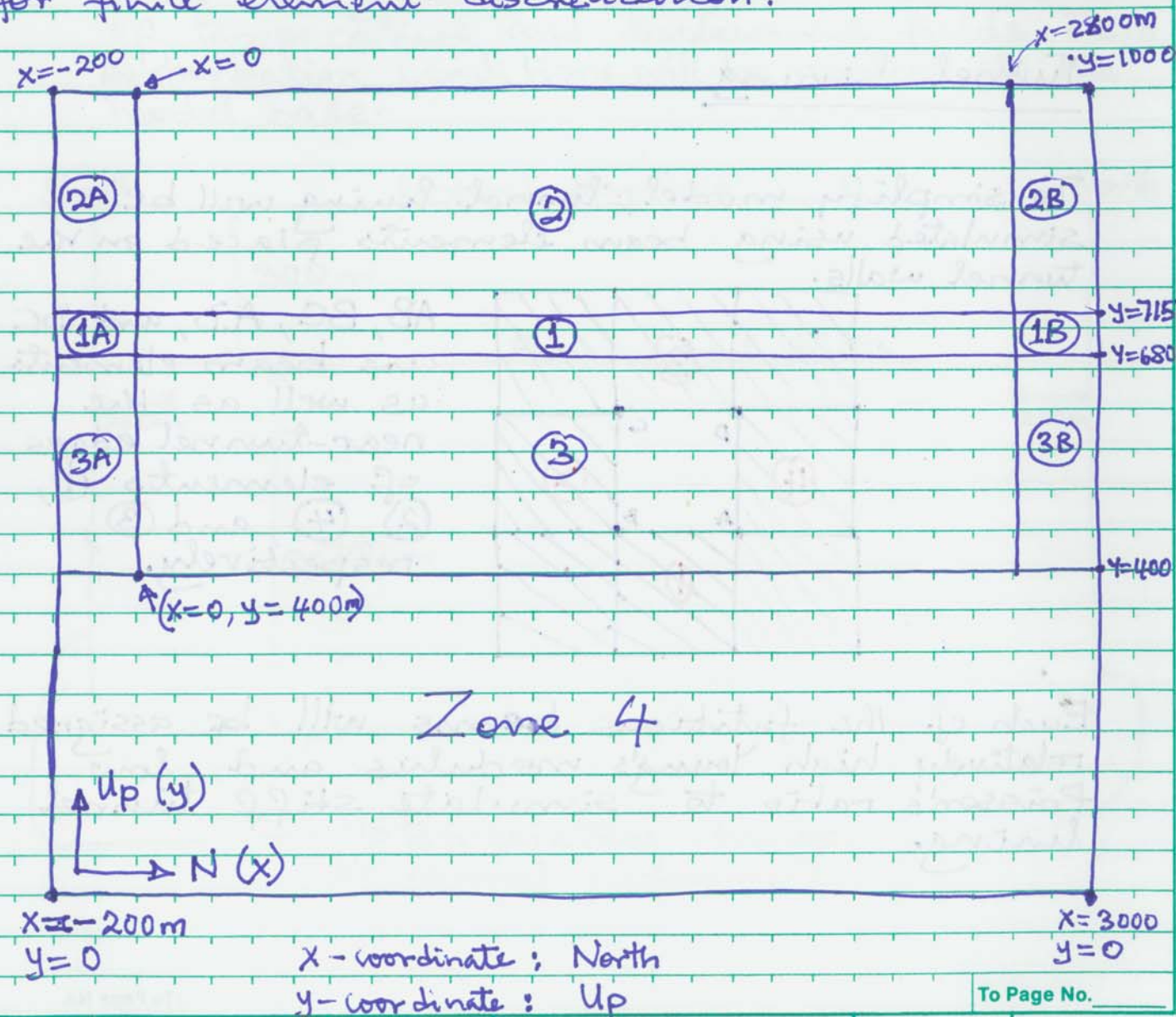
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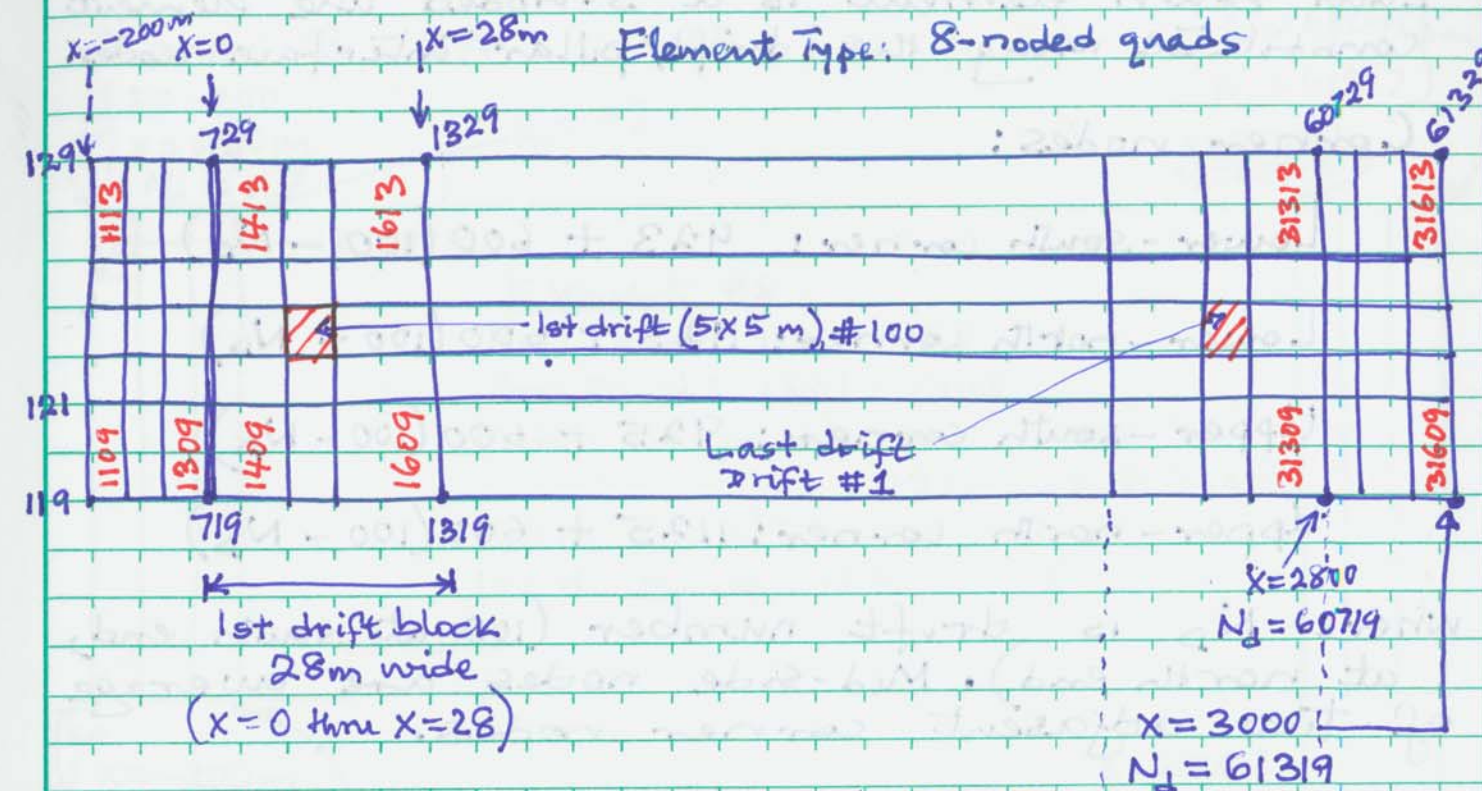
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Submodeling approach described on p. 1 was abandoned — because testing on a preliminary model show (Gip¹⁹²) indicates that the ABAQUS implementation of submodeling may have a problem for thermal-mechanical analyses. As a result, the full problem domain was discretized for one-step analysis (i.e., no submodeling).

The problem domain was divided into 4 zones for finite element discretization:



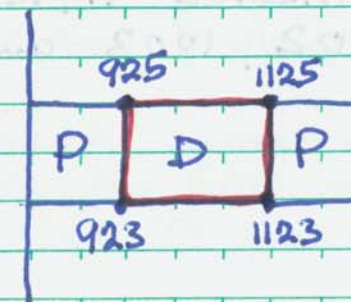
Element Type: 8-noded quads



Node numbers and x-coordinates (m) in blue. Element numbers in red. Last drift block $y = 715$ m at top of zone and 680 m (ending at $x = 2800$) at base. There are 100 drift blocks each containing 1 drift. Drift #100, first drift in the model is at the south end of the drift array whereas Drift #1, last drift in the model, is at the north end.

Beam Elements

Example: Drift #100
at South end
of drift array.



Corner nodes: 923 1123
925 1125

D = drift (opening)
P = pillar (rock) To Page

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Each beam element is a 3-noded line element constituted using the drift/pillar interface nodes.

Corner nodes:

Lower-south corner: $923 + 600(100 - N_D)$

Lower-north corner: $1123 + 600(100 - N_D)$

Upper-south corner: $925 + 600(100 - N_D)$

Upper-north corner: $1125 + 600(100 - N_D)$

where N_D is drift number (100 at south end, 1 at north end). Mid-side nodes are average of two adjacent corner nodes.

Beam element numbers are:

Floor beam: $11 + 10(100 - N_D)$

North side-wall beam: $12 + 10(100 - N_D)$

Roof beam: $13 + 10(100 - N_D)$

South side-wall beam: $14 + 10(100 - N_D)$

Hence beam elements are numbered 11, 12, 13 and 14 for drift #100; and 1001, 1002, 1003 and 1004 for drift #1 (north end).

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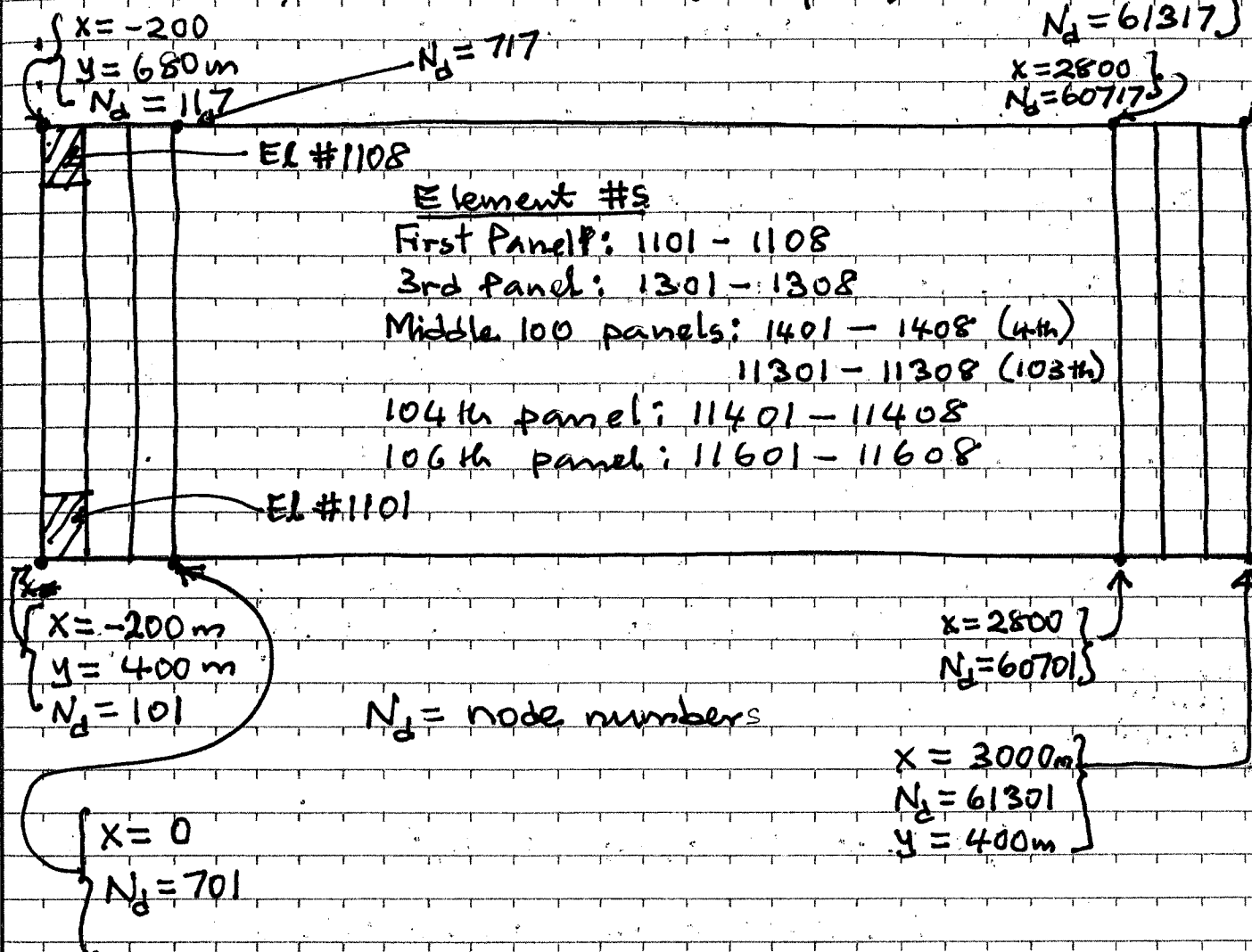
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Node and Element Numbering for Zone 3 (Base Zone)

Zones 3, 3A and 3B (see p. 6)



106 vertical panels (3 panels from $x = -200$ to $x = 0$
100 panels from $x = 0$ to $x = 2800$
3 panels from $x = 2800$ to $x = 3000$)

Each panel has 8 elements (stacked vertically)

Node numbers increment horizontally by 200 within first 3 panels, by 600 within panels 4 - 103, and by 200 within last 3 panels.

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$$N_d = 101 + 200(P_n - 1) \quad P_n = 1 - 3$$

$$701 + 600(P_n - 1) \quad P_n = 4 - 103$$

$$60701 + 200(P_n - 1) \quad P_n = 104 - 106$$

P_n = panel number.

$$El = 1101 + 100(P_n - 1) \quad \text{at base}$$

$$1108 + 100(P_n - 1) \quad \text{at top.}$$

El = element number.

Element and Node Numbers in Zone 2 (2, 2A, and 2B): Top Zone

$$\begin{aligned} x &= -200 \\ y &= 1000 \\ N_d &= 147 \end{aligned}$$

$$N_d = 131 + 200(P_n - 1) \quad P_n = 1 - 3$$

$$731 + 600(P_n - 1) \quad P_n = 4 - 103$$

$$60731 + 200(P_n - 1) \quad P_n = 104 - 106$$

$$El = 1114 + 100(P_n - 1) \quad \text{Base}$$

$$1121 + 100(P_n - 1) \quad \text{Top}$$

$$\begin{aligned} x &= -200 \\ y &= 715 \\ N_d &= 131 \end{aligned}$$

731

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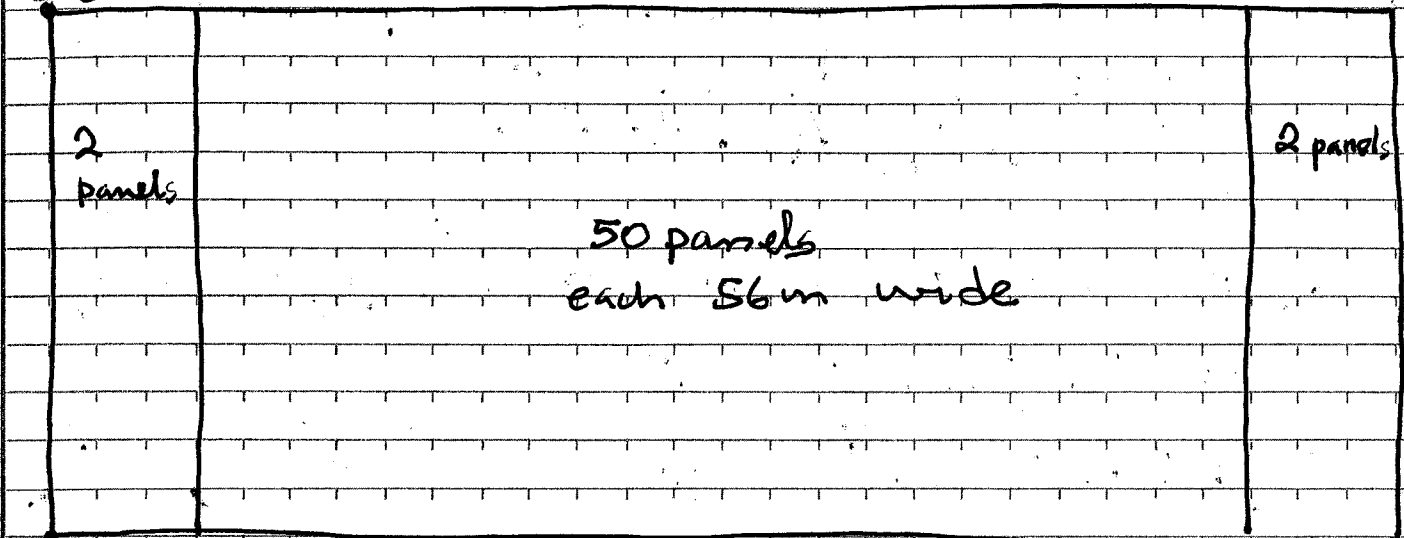
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Element and Node Numbering for Zone 4 (Far Base)

$$\begin{aligned} y &= 400 \\ x &= -200 \\ N_d &= 63109 \end{aligned}$$



$$\begin{aligned} y &= 0 \\ N_d &= 63101 \end{aligned}$$

$$N_d = 63101 + 200(P_n - 1) \quad \text{base}$$

$$63109 + 200(P_n - 1) \quad \text{top}$$

$$P_n = 1 - 54.$$

$$El = 33101 + 100(P_n - 1) \quad \text{base}$$

$$33104 + 100(P_n - 1) \quad \text{top.}$$

Drift number D_n and panel number P_n are related as follows:

$$D_n = 104 - P_n \quad \left. \begin{array}{l} \text{for } 1 \leq D_n \leq 100 \\ \text{for } 4 \leq P_n \leq 103 \end{array} \right\}$$

$$P_n = 104 - D_n$$

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Thermal load will be modeled as a volumetric heat source distributed uniformly within and among the emplacement drifts. The volumetric source q is given by

$$q = \left(\frac{Q}{100} \right) \left(\frac{1}{V_d} \right) (S_y) (10^6)$$

V_d = emplacement drift volume (of 1 drift)

$$= 5 \times 5 \times 1080 \text{ m}^3$$

$$S_y = 3.156 \times 10^7 \text{ s/yr}$$

Q = total heat source (10^6 J/s) per unit time due to all 10,938 waste packages.

Values of Q were obtained from Table V-1 (Attachment V) of Reference 1 (see p. 2). The table of Q versus yr (following emplacement) is reproduced on p. 13.

Heat load will be applied at the same time over all 100 drifts.

Thermal Properties

All elements are assigned TSW2 thermal properties (from Reference 1 cited on p. 2) as follows:

$$\text{Density } \rho = 2274 \text{ kg/m}^3$$

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Years HeatSource (MW)

0 71.499486

5 63.518801

10 57.65222

20 48.598077

30 41.428829

40 36.000998

50 31.544301

60 27.912077

70 24.90998

80 22.386533

90 20.530418

100 18.942351

150 14.040354

200 11.499602

300 9.02496

400 7.585398

500 6.574892

600 5.77481

700 5.149266

800 4.610581

900 4.179814

1000 3.817243

2000 2.04884

3000 1.59641

4000 1.425174

5000 1.328105

6000 1.229386

7000 1.151405

8000 1.078041

9000 1.017151

10000 0.965009

\uparrow
(10^6 J/s)

GW
3/27/98

Thermal conductivity

$$K = 2.1 \text{ J/s.m.K}$$

$$= 6.6271 \times 10^7 \text{ J/yr.m.K}$$

Specific heat capacity: Function of temperature as follows:

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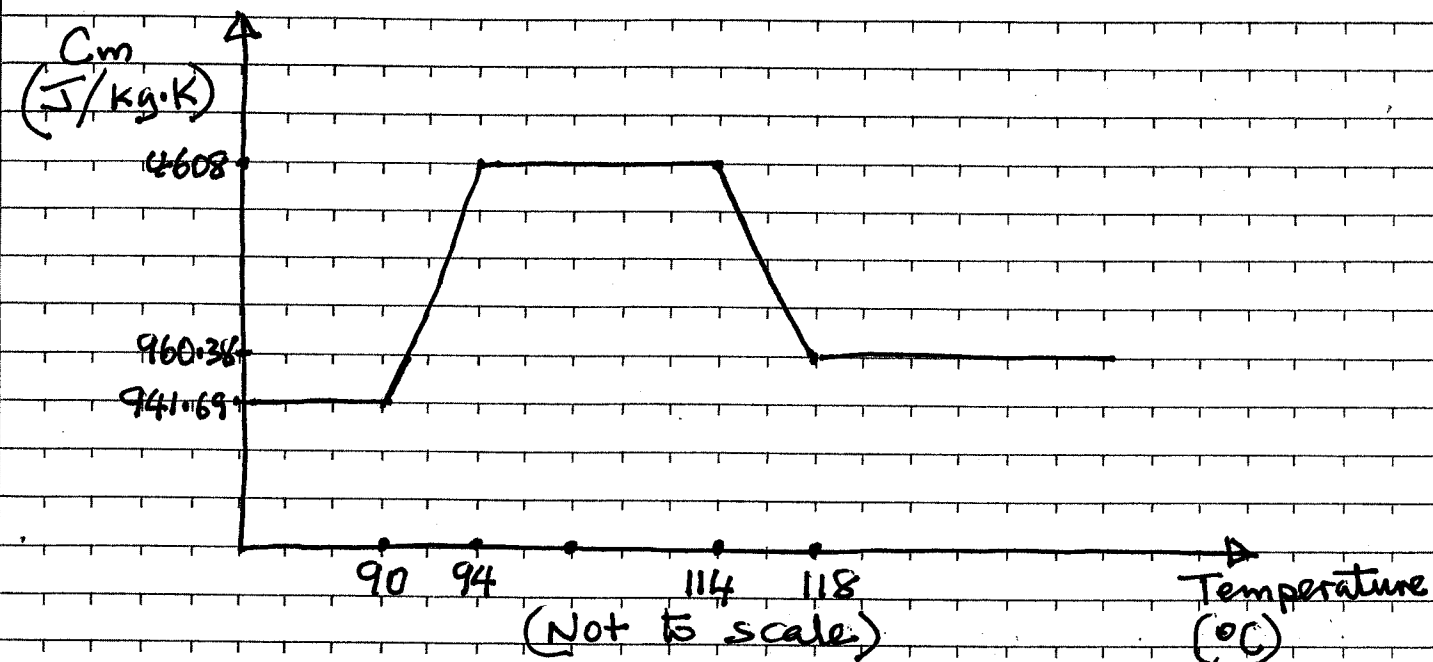
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Temperature (°C)	C_v (J/m ³ ·K)	C_m (J/kg·K)
25 - 94	2.1414×10^6	941.69
94 - 114	10.4786×10^6	4608.0
> 114	2.1839×10^6	960.38

The heat capacity vs temperature relationship will be implemented as follows to avoid infinite gradient



Mechanical Properties

Thermal expansivity

Young's modulus

Poisson's ratio

Friction angle

Cohesion

Unconfined compressive strength

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Thermal expansivity is implemented as a function of temperature as follows:

Temperature (°C)	Thermal expansivity %/°C
0.0	5.07×10^{-6}
29.0	5.07×10^{-6}
51.0	7.30×10^{-6}
98.0	7.30×10^{-6}
102.0	8.19×10^{-6}
148.0	8.19×10^{-6}
152.0	8.97×10^{-6}
200.0	8.97×10^{-6}

Poisson's ratio was assigned constant value of 0.21 (from Reference 3 on this page).

Unconfined compressive strength of intact rock (σ_c) is assigned constant value of 180 MPa (from Reference 4 on this page).

Values of Young's modulus (E), friction angle (ϕ) and cohesion (c) were determined from rock-mass quality Q , which varies spatially in the north-south direction as was determined from DOE data (attached as Attachment 1 on p. 16-17).

References

(3) Repository Ground Support Analysis for Viability Assessment. CRWMS/M&O Document Number BCAD00000-01717-0200-00004 Rev 00.

(4) Brechtel, C.E., M. Lin, E. Marting and D.S. Kessel. Geotechnical Characterization of the North Ramp of the Exploratory Studies Facility, Volumes I & II. Sandia Report SAN95-0488/1, (1995).

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ESF Design Confirmation

Attachment 1

ESF ROCK MASS QUALITY

Thermo-mechanical Unit*

Rock Mass Quality, Q

1000

100

10

1

0.1

0.01

2 5 2 1

3

4

2 1 1
3 1 3 4 3 2

Nine-Term Moving Geometric Mean of
Q Values Collected at 5-m Intervals Along
the ESF Main Loop

*Thermo-mechanical Units
in the ESF Main Loop

1 — PTn 4 — TSw2
2 — TCw 5 — UO
3 — TSw1

— Scanline Q Data

— Full-Peripheral Q Data

00+00

10+00

20+00

30+00

40+00

50+00

60+00

70+00

80+00

ESF Tunnel Station (m)

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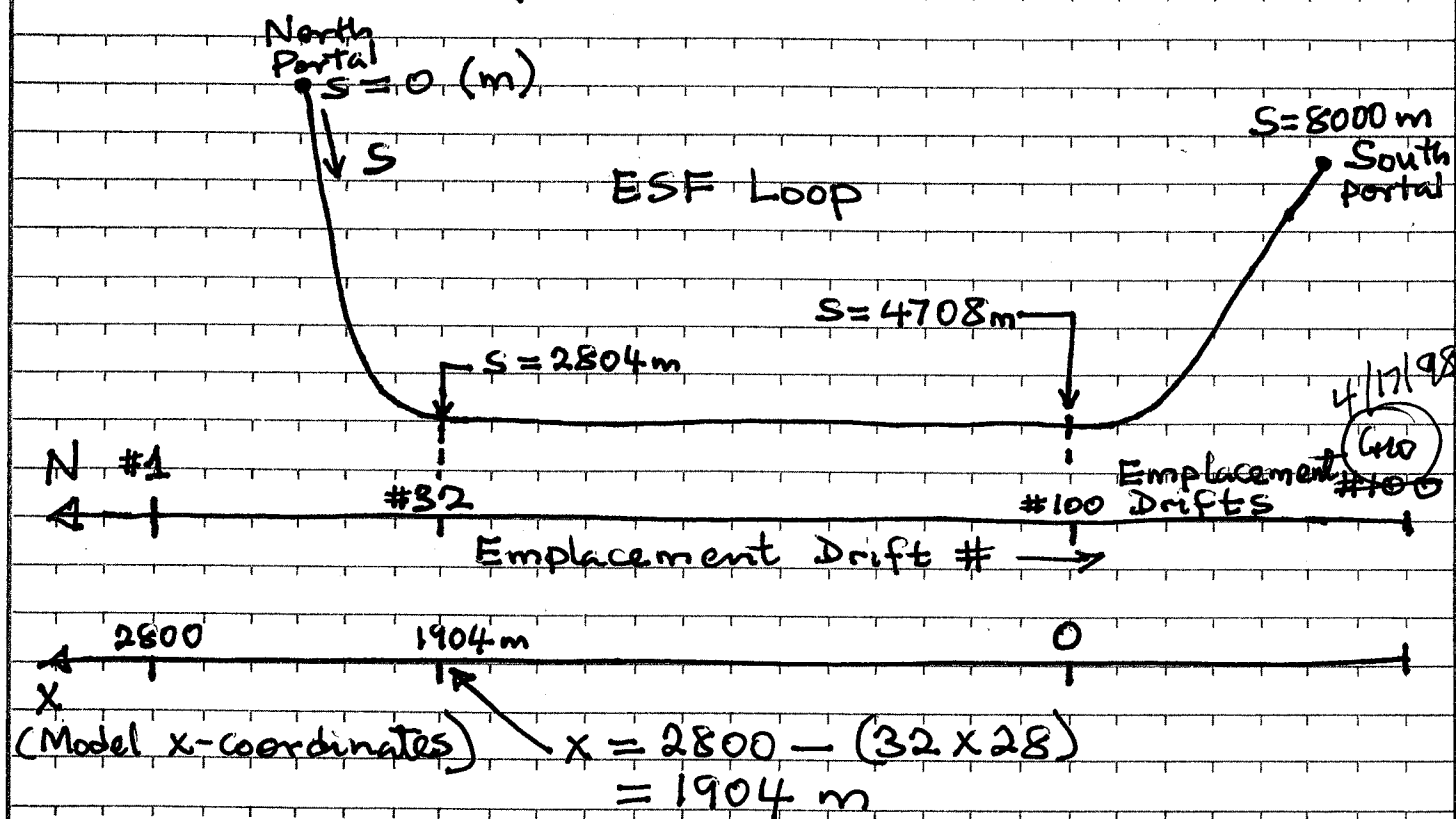
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Rock Mass Quality (Q) Pages 18-24 GW
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$Q = Q(x, t)$; i.e., Q is a function of north-south coordinate x and time t .

Function $Q(x, t=0)$ is evaluated from ESF data (p. 16-17). ESF data gives Q as a function of North-to-South distance, S , along the ESF main drift.



$$X = 1904 - (S - 2804) \quad 2804 \leq S \leq 5000$$

$$S = 2804 + (1904 - X) \quad -200 \leq X \leq 1904$$

Q values from the full-peripheral line (p. 16-17) between $S=2804m$ and $S=5000m$ were used to obtain Q values for $-200 \leq X \leq 1904m$.

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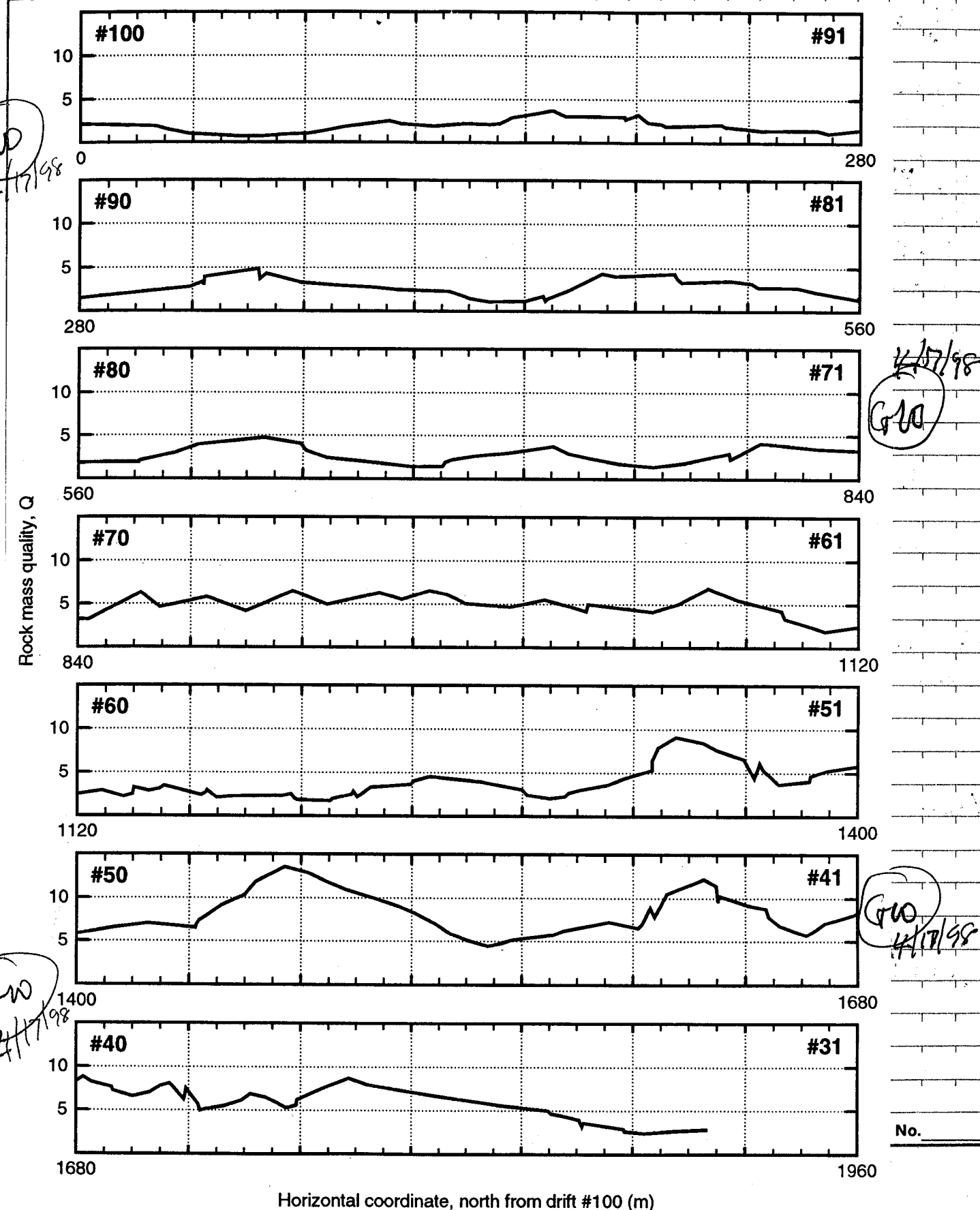
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Values of Q at $X > 1904m$ were set to the value at $X=1904m$.



From Page No. _____ Mohr-Coulomb Parameters for Rock Mass

$$\tau_f = c + \sigma_n \tan \phi$$

where τ_f = shear strength of rock mass on an internal plane experiencing normal stress σ_n . The strength parameters, cohesion (c) and friction angle (ϕ) can be estimated from Q using relationships from Hoek and Brown (1997) [Hoek, E. and E.T. Brown. 1997. Practical estimates of rock mass strength. Int. J. Rock Mech. Min. Sci., 34(8): 1165-1186].

These relationships require:

- (1) Hoek-Brown intact rock strength parameter, m_i ;
- (2) Hoek-Brown geological strength index, GSI; and
- (3) unconfined compressive strength of intact rock, σ_{ci} (see p. 15).

From Hoek (1994) [Hoek, E. 1994. Strength of rock and rock masses. ISRM News Journal, 2(2): 4-16]

$$GSI = 9 \ln Q + 44$$

Also, from Ref (4) on p. 15,

$$m_i = 10 \text{ for TSw2 rock unit.}$$

Charts on Figures 7 and 8 of Hoek and Brown (1997) [cited on this page] are used to evaluate the functions

$$\phi = \phi(GSI, m_i = 10)$$

$$\frac{c}{\sigma_{ci}} = \frac{c}{\sigma_{ci}}(GSI, m_i = 10)$$

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GSI	ϕ ($m_i = 10$)	c/σ_{ci} ($m_i = 10$)
1.0	10 degrees	0.006
10	18.3 ✓	0.013
20	22.5 ✓	0.018
30	26.2 ✓	0.03
40	28.0 ✓	0.035
50	31.2 ✓	0.041
60	33.2 ✓	0.054
70	36.2 ✓	0.07
80	37.5 ✓	0.10
90	38.5 ✓	0.15

Information potentially subject to copyright protection was redacted from this location. The redacted material is from the following reference:

Hoek, E. and E.T. Brown. "Practical Estimates of Rock Mass Strength." International Journal of Rock Mechanics-Mineral Science. Vol. 34, No. 8. pp. 1,165-1,186. 1997.

Information potentially subject to copyright protection was redacted from this location. The redacted material (Figure 7) is from the following reference:

Hoek, E. and E.T. Brown. "Practical Estimates of Rock Mass Strength." International Journal of Rock Mechanics, Mineral and Science. Vol. 34, No. 8. pp. 1,165-1,186. 1997.

Rock Mass Young's Modulus

Young modulus, E , for rock mass is evaluated using relationships from Hoek (1994) [cited on p. 20]

$$E = 10^{[(RMR-10)/40]} \text{ GPa}$$

$$RMR = 9 \ln(Q) + 49$$

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Drucker-Prager Strength Parameters from Mohr-Coulomb Parameters

Rock mass strength is modeled using the Drucker-Prager strength criterion, which defines rock strength in terms of a friction angle β and a shear-strength intercept d .

For plane strain conditions, β and d are calculated from C and ϕ as follows:

$$\left. \begin{aligned} \tan \beta &= \frac{\sqrt{3} \sin \phi}{\sqrt{1 + \frac{1}{3} \sin^2 \phi}} \\ d/c &= \frac{\sqrt{3} \cos \phi}{\sqrt{1 + \frac{1}{3} \sin^2 \phi}} \end{aligned} \right\} \begin{aligned} &\text{For associative flow,} \\ &\text{i.e., } \psi/\phi = 1 \\ &\text{where } \psi \text{ is dilation} \\ &\text{angle.} \end{aligned}$$

$$\left. \begin{aligned} \tan \beta &= \sqrt{3} \sin \phi \\ d/c &= \sqrt{3} \cos \phi \end{aligned} \right\} \begin{aligned} &\text{For non-dilatant flow,} \\ &\text{i.e., } \psi/\phi = 0 \end{aligned}$$

Analyses are conducted with dilation angle equal to half of friction angle. Therefore, the average values of β and d/c between the above two sets of equation (i.e., $\psi/\phi = 1$ and $\psi/\phi = 0$ cases) are used.

North-South Variation of Properties

Spatial variation of E , C , and ϕ is implemented by specifying these parameters as functions of a field variable that is assigned the value of X -coordinate at every node.

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From Page No. _____ Initial Temperature

Initial nodal temperatures are specified using the following information from Reference (1) [cited on p. 2] and the value of y-coordinate at every node.

Y-coordinate at ground surface (top of model): 1000m

Temperature at ground surface: 18.7°C

Depth (m)	Temperature gradient (°/m)
0 - 150	0.02
150 - 400	0.018
400 - 700	0.03
> 700	0.008

Thermal Boundary Conditions

Temperature fixed at initial value at all exterior-boundary nodes, with the initial value calculated as described above.

Mechanical Boundary Conditions

Free-surface condition (zero traction) at the ground surface (top of model) and zero boundary-normal displacement at all other exterior boundaries.

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Analysis Procedure

Analyses consist of sequentially coupled thermal (heat conduction) and ~~thermal~~ mechanical and mechanical procedures. Temperature calculated in the thermal analysis are used as input for the mechanical analysis.

Thermal Analysis Steps

Step Number	Time at End of Step (yr)	Remarks
1	2×10^{-6}	Dummy Step to synchronize thermal and mechanical analyses.
2	150.0	Time-varying volumetric heat source (p. 12) applied to simulate thermal load of emplaced waste.

Mechanical Analysis Steps

Step Number	Time at End of Step (yr)	Remarks
1	1×10^{-6}	Initial static equilibrium under gravitational loading and boundary restraint. No drifts.
2	1×10^{-6}	Drifts excavated
3	150	Temperatures from Step 2 of thermal analysis applied in this step
4	151	Drift-support removed.

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There is no change in nodal temperature between Steps 3 and 4 of the mechanical analyses. So any differences between calculated responses at the end of these steps arises from the removal of rock support.

~~Task~~ G10
4/23/96

Degradation of Rock-Mass Strength

Two sets of mechanical analyses are conducted as follows (using temperature distributions from one thermal analysis):

Case 1:

Young's modulus and all strength parameters assigned values corresponding to the values of Q on p. 19 with $\sigma_{ci} = 180$ MPa as stated on p. 15.

Case 2:

- (i) Young's modulus assigned same value as in Case 1.
- (ii) All strength parameters assigned values corresponding to $0.1Q$ (where Q is given on p. 19) and $\sigma_{ci} = 90$ MPa.

Both Poisson's ratio and thermal expansivity are not modified.

Keeping Young's modulus, Poisson's ratio, and thermal expansivity at their current values (while strength parameters are reduced) is intended to simulate the occurrence of stress buildup (during the first 75 yr or so) prior to onset of rock-mass degradation.

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Stress buildup is controlled by these elastic parameters.

Degradation of intact rock strength from 180 MPa to 90 MPa (a factor of 0.5) is intended to simulate the difference between the strength of intact rock under rapid loading (laboratory test condition) and sustained loading (in situ condition), as documented in Lajtai and Schmidtke (1986) [Lajtai, E. Z. and R. H. Schmidtke. 1986. Delayed failure in rock loaded in uniaxial compression. Rock Mechanics and Rock Engineering, 19:11-25].

Degradation of Q to $0.1Q$ arises from an expectation that the ratio J_r/J_a (J_r is joint roughness number and J_a is joint alteration number) in the formula for Q can decrease by one order of magnitude due to fracture wallrock alteration under exposure to heat and moisture. The formula for Q can be found in rock engineering texts or Barton et al. (1974) [Engineering classification of rock masses for the design of tunnel support. Rock Mechanics, 6:189-236].

Input Files: Thermal Analysis

ht0150.inp

Main ABAQUS input file.

Commands in this file cause ABAQUS to read input information from the following auxiliary input files:

allNodes.def
htElems.def
drftHeatSrc.def
iniTemp.def
bndTemp.def

Node definitions
Element definitions
Heat Source history definition
Initial nodal temperature
Fixed boundary temperature

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Input Files: Mechanical Analysis: Case 1 Property Set

mCBase.inp Main ABAQUS input file for Analysis Step 1 and Step 2 (p. 25).

mC150.inp Main ABAQUS input file for Analysis Step 3 and Step 4 (p. 25).

Auxiliary Input Files (Called from mCBase.inp and mC150.inp)

allNodes.def	Node definitions
mElemCur.def	Element definitions
iniTemp.def	Initial nodal temperature
mdfld1.def	Defines x-coordinate as a field variable used to assign material property values
ElPropCurrent.def	Elastic properties (current values)
FricAngCurrent.def	Friction properties (current values)
CohesionCurrent.def	Cohesion (current values)
ht0150.fil	Results file from thermal analysis

Input Files: Mechanical Analysis: Case 2 Property Set

mLBase.inp Main ABAQUS input file for Analysis Step 1 and Step 2 (p. 25)

mL150.inp Main ABAQUS input file for Analysis Steps 3 and 4 (p. 25).

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Auxiliary Input Files:

Called from mLBase.inp and mL150.inp

allNodes.def
mElemLt.def
iniTemp.def
mdfld1.def

Node definitions
Element definitions
Initial nodal temperature
Field variable (x-coordinate)
for mech property definitions.

ElPropCurrent.def
FricAngLangTerm.def
CohesionLangTerm.def
ht0150.fil

Current elastic properties
Degraded frictional properties
Degraded cohesion properties
Thermal analysis results file

All of the input files listed on p. 27 - current page are ASCII files, except ht0150.fil, which is a binary file (generated from the thermal analysis run).

Results Processing

Analysis results are presented in terms of contour plots of the magnitude of inelastic strain. The contours are produced using ABAQUS/POST by invoking the following file

cPlotDef.s.jnl

Invoked from within ABAQUS/POST
This file causes ABAQUS/POST to read the following files:

plotElems.def

Lists elements within ± 17.5 (GEO) of repository elevation (dist axis)

cSettings.def

Defines settings for contour plotting

peContours.def

Plots inelastic strain contours.

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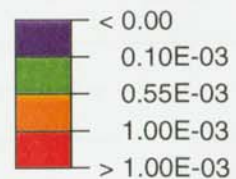
Invented by

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Permanent Strain



Emplacement Drifts at 150 Years Degraded Rock Mass Stiff Tunnel Liners

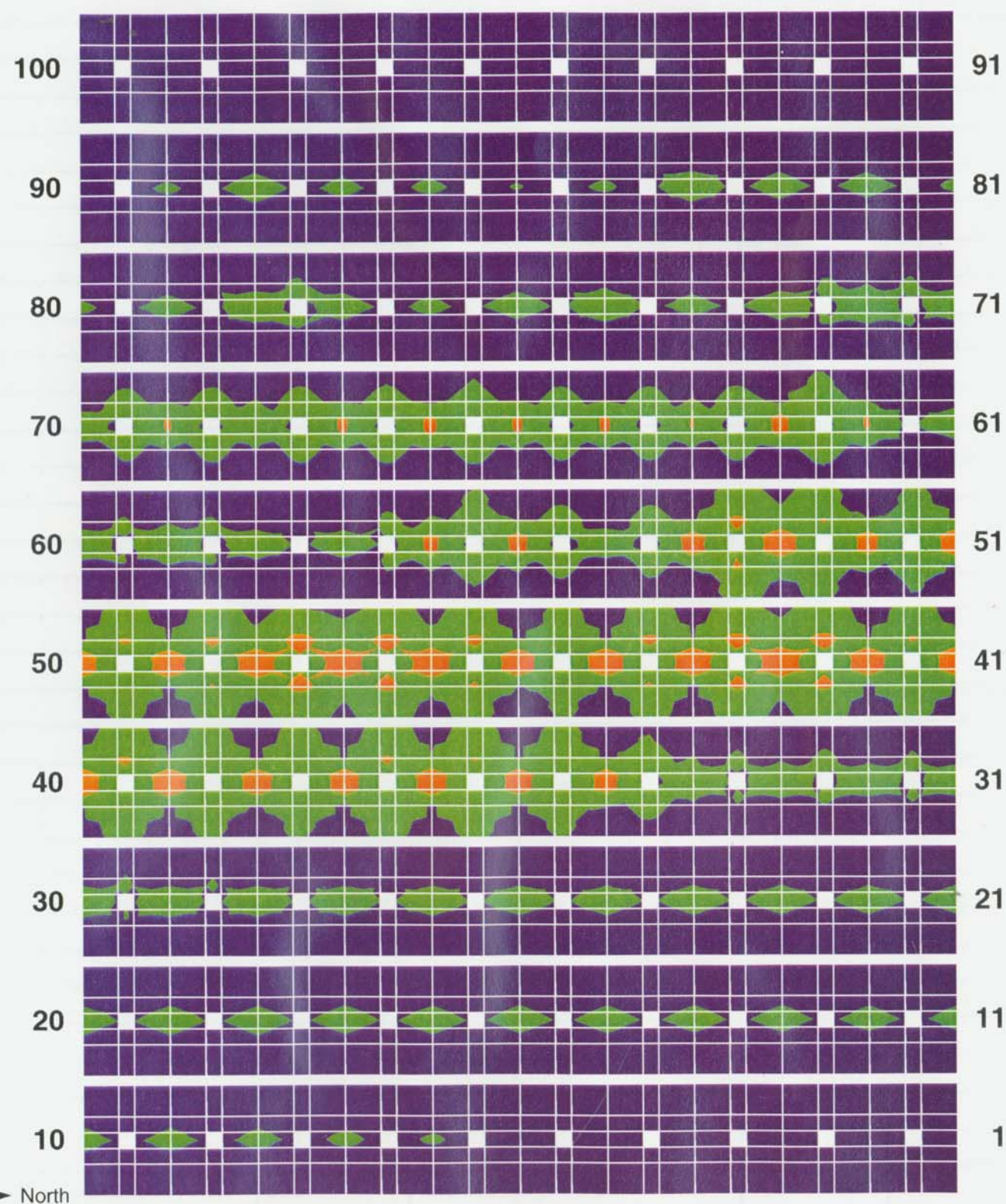
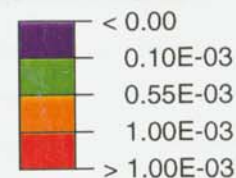
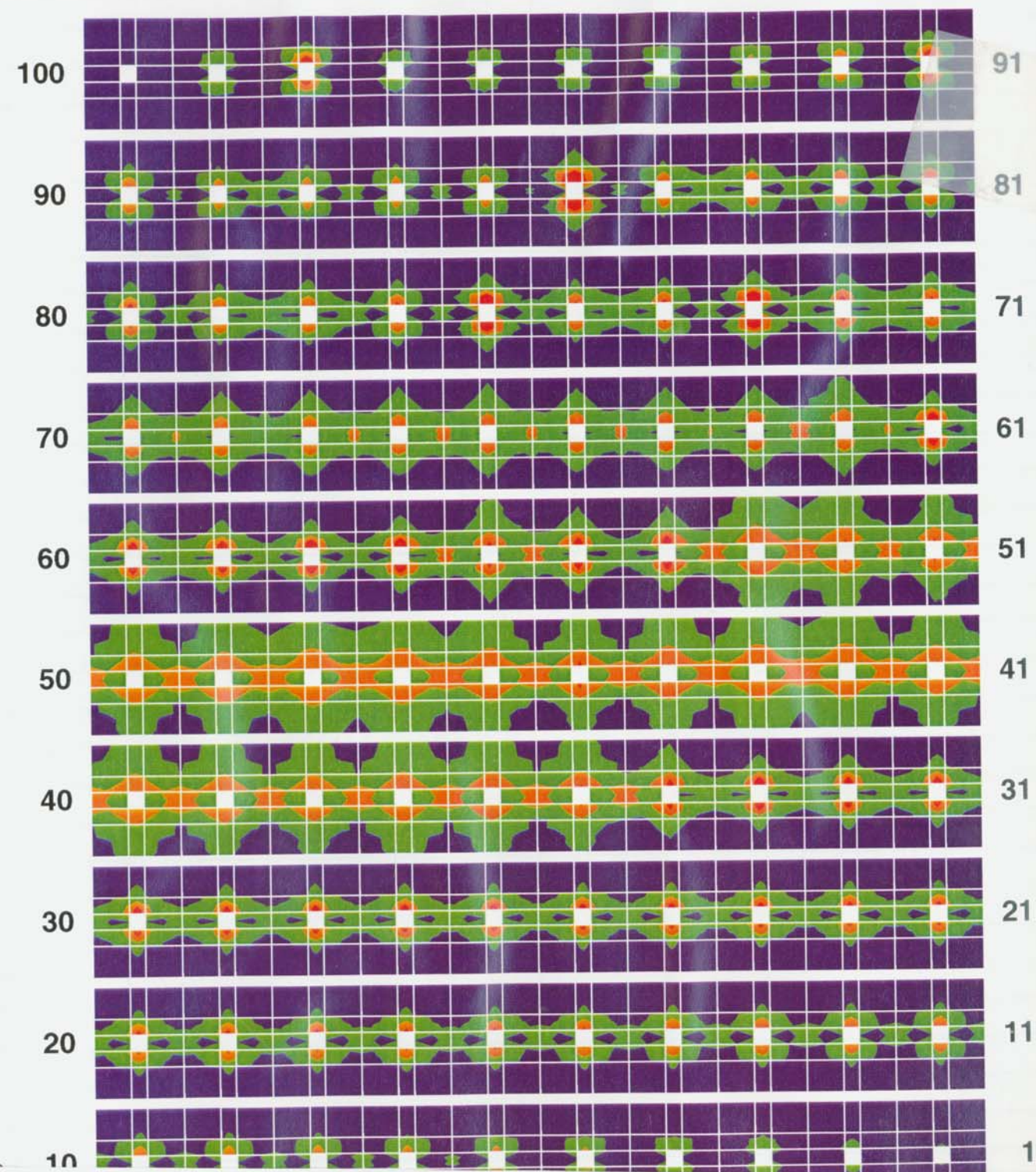


Figure Distribution of permanent (i.e., inelastic) strain intensity for the analysis case with current values of Young's modulus, degraded values of friction angle, cohesion, and intact-rock strength, and nondegraded concrete lining. Result is presented in ten sections, each 280 m long (in north-south direction) by 35 m high (vertical dimension). Ten white squares within each section represent emplacement drifts, and the numbers for the end drifts of each section are shown. Drift #100 is at the south end of the figure

Permanent Strain



Emplacement Drifts at 150 Years Degraded Rock Mass Degraded Tunnel Liners



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entire
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```

RmqAsField.cpp
Page 3

if (!nn) {
    sprintf(messageBuffer, "No valid input in file %s", matPFile);
    return(ERROREND);
}

*xCoord = new float[nn];
*rmQ = new float[nn];
if (!(*xCoord) || !(*rmQ)) {
    sprintf(messageBuffer, "Memory allocation failure on matP vectors");
    return(ERROREND);
}

Fin.seekg(0, ios::beg); // Rewind input file
int i = 0;
while (Fin) {
    Fin.getline(buf, 100);
    istrstream inPline(buf, nBuf);
    inPline >> x1 >> q >> yml >> phil >> cpl;
    if (inPline.good()) {
        (*xCoord)[i] = x1;
        (*rmQ)[i] = q;
        if (++i >= nn)
            break;
    }
}

*numData = nn;
return(NORMALEND);
}

// Function LinearInterp evaluates the relationship f(t) at t=atTemp
// using linear-polynomial assumption for f(t). Calling program provides
// pointers to arrays of t and f values that define the f(t) relationship.
// Function LinearInterp assumes that the t array gives values of t in
// increasing order and that no two t values are equal.
//

float LinearInterp(const float* t, const float* f, float atTemp, int n)
{
    if (atTemp <= t[0])
        return(f[0]);
    if (atTemp >= t[n-1])
        return(f[n-1]);
    for (int i=1; i<n; i++)
        if (atTemp <= t[i])
            return(
                f[i-1] + (f[i] - f[i-1])*(atTemp - t[i-1])/(t[i] - t[i-1])
            );
    return(f[n-1]); // This line is never reached: it was added to avoid
    // Borland-Compiler warning
}

void DeleteVectors(float** x, float** q)
{
    delete[] *x;
    delete[] *q;
}

void DumpAndQuit()
{
    cerr << "\n" << messageBuffer << "\n";
    delete[] messageBuffer;
    cerr << "Press any key to end: ";
    getch();
}

RmqAsField.cpp
Page 4
exit(1);
}

```

Page 3

Results of the dummy analysis (to display Q as a user-defined field) are processed following the procedure described on p. 29. The input file invoked from within ABAQUS/POST is

gPlotDefs.jnl

which causes POST to read the following files:

gPlotElem.def
gPlotSettings.def
gContours.def

The ABAQUS/POST result is a file that is processed using ABAQUS/POST to obtain 7 postscript files.

These postscript files

were later assembled using Adobe Illustrator to obtain the file (figure)

gFields.ai

that is shown on p. 35.

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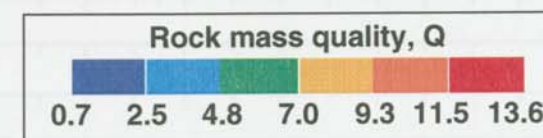
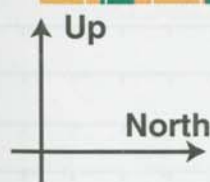
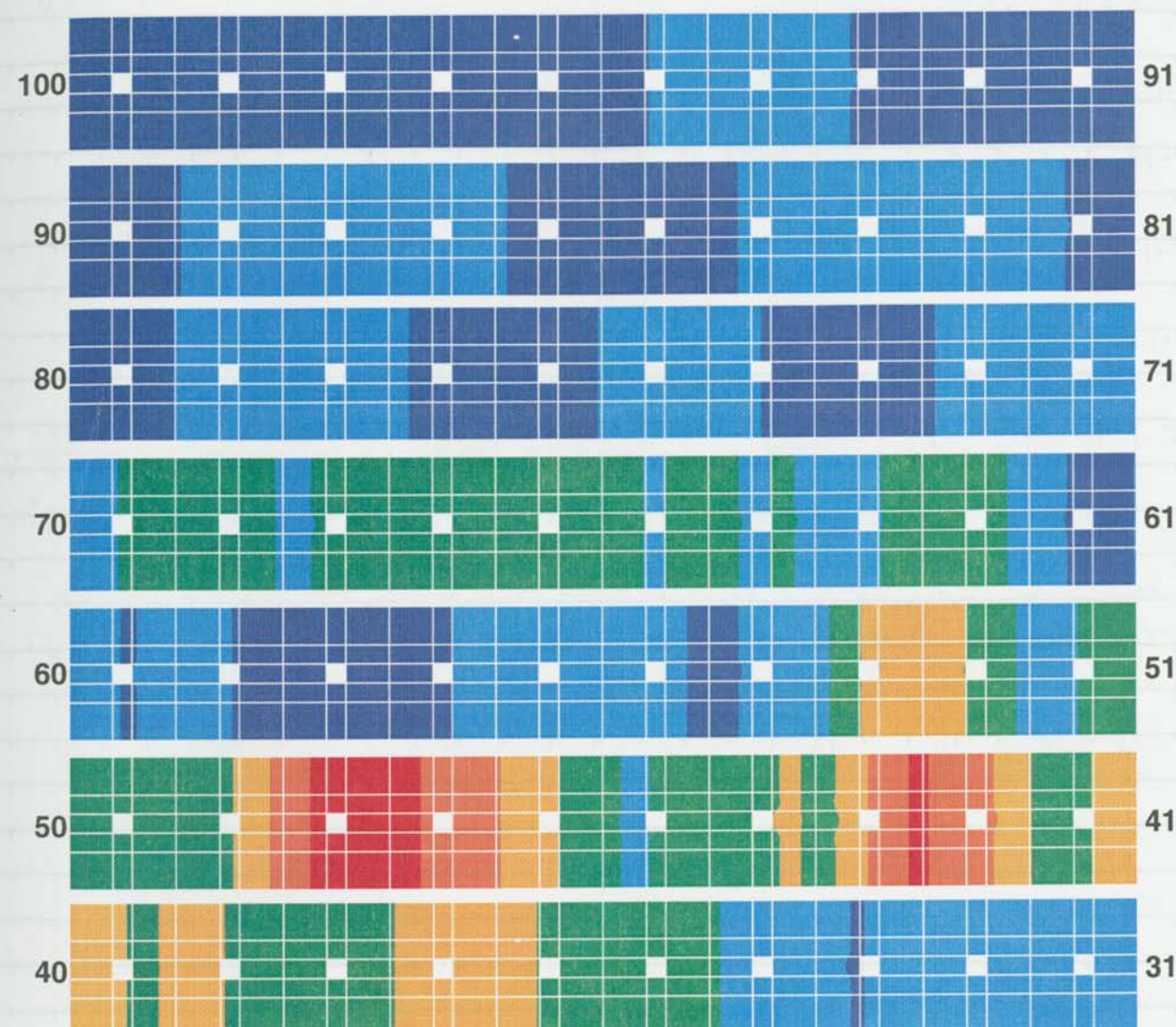
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Distribution of rock-mass quality, Q, within 17.5m above and below the drift axis, presented following the format described on pp. 30 and 31. Values of Q from Drifts 1-30 (not shown) are the same as for Drifts 31-32.

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The following plots are desired:

 Γ^N vs Q

Γ^N vs E ($E =$ Young's modulus)

μ^N vs ϕ (ϕ = friction angle)

~~Grain~~ τ^N vs c (c = cohesion parameter)

A table of X and Y coordinates and T^N values at element integration points was obtained from ABAQUS for all 680 solid elements in the following zones: 10/12/98

Zone (1), (2) and (3) [p. 6]

This table was prepared in three steps as follows:

1. Run ABAQUS POST-OUTPUT job to obtain element integration-point coordinates.

- 2 Run ABAQUS POST-OUTPUT job to obtain integration-point FN values.

3. Combine tables from both analyses to obtaining one table, using C++ code documented on pp. 37-38.

~~The results were plotted using Kaleidograph, and the postscript files produced by Kaleidograph were assembled.~~

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Page 1

est rnMag.cpp

```

//strnBuf
This code combines two ABAQUS output tables in two separate files into one.
The first file (xyFile) contains element integration-point coordinates
and the second file (strnFile) gives values of inelastic strain intensity
numbers. Because strnFile does not include integration points the
coordinates are also included in xyFile. The combined
table will include every point, assigning zero values to points not found
in strnFile.

G. I. Ogasawara
October 8, 1998
C++ Compiler
Code developed using Borland C++Builder 3

//*****
#include <stdio.h>
#include <iostream.h>
#include <fstream.h>
#include <string.h>
#include <conio.h>
#include <stdlib.h>
#pragma hdrstop
#include <unistd.h>

struct DPoint{
    int element;
    float xValue;
    float yValue;
    float pmax;
};

DPoint int eNum, int ipoint, float xValue, float yValue){
    struct DPoint next;
    if (fscanf(stdin,"%d %f %f",&eNum,&xValue,&yValue) != 0){
        printf("Error reading input\n");
        return 0;
    }
    next = {eNum,xValue,yValue,pmax=0.0};
}

char* outfile = "SOL50XyStrn.dat";
char* strnfile = "SOL50Penag.dat";
char* xyfile = "intpCoord.dat";

char* messageBuffer;
int MAXLENB = 10;
int MSGLENB = 0;

void StorePoint(DPoint* current, DPoint** first, DPoint** last);
bool IsGreater(DPoint* current, DPoint* old);
int InsertStrnValue(DPoint* first, int element, int ipt, float strn);
void PrintPoints(DPoint* first);
void DeletePoint(DPoint* first);
void DumpAndQuit();

//-----
//prgname argstyp
//ent main(int argc, char **argv)
// Allocate memory for possible error messages

```

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```

PstrnMag.cpp
return (ERROREND);
}
ofstream fout(outFile);
if (!fout) {
    cerr << "Unable to open file " << outFile << endl;
    return (ERROREND);
}
fout << setw(10) << "Element"
    << endl;
    << setw(5) << "ID"
    << setw(15) << "Q"
    << setw(15) << "E"
    << setw(15) << "phi"
    << endl;
    while (first) {
        fout << setw(10) << first->element
            << endl;
            << setw(15) << first->Q
            << setw(15) << first->E
            << setw(15) << first->phi
            << endl;
            << first->next;
        }
    }
    return (NORMALEND);
}

void DeletePoints(DPoint** first)
{
    DPoint *old;
    DPoint *p = *first;
    while (p) {
        old = p;
        p = p->next;
        delete old;
    }
    void DumpAndQuit()
    {
        cerr << "n" << messageBuffer << "n";
        delete messageBuffer;
        getch();
        exit(1);
    }
}

```

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```

PstrnMag.cpp
P = *first;
/* current is the very first item in the list */
if (!(*last)) {
    *current->next = 0;
    *last = current;
    return;
}
else {
    if (old) {
        /* Insert current at this point in the list */
        old->next = current;
        current->next = p;
        return;
    }
    /* current becomes new first element */
    *first = current;
    return;
}
/* last->next = current; /* current becomes new last element */
*last->next = NULL;
*last = current;

bool IsGreater(DPoint* current, DPoint* old)
{
    if ((current->element) < (old->element))
        return (false);
    if ((current->element) > (old->element))
        return (true);
    // current element same as old element: check integration points
    if ((current->ipt) > (old->ipt))
        return (true);
    return (false);
}

int InsertStrnValue(DPoint* first, int element, int ipt, float strn)
{
    if (!(*first)) {
        *first = new DPoint(element, ipt, strn);
        return (messageBuffer, "Empty list passed to InsertStrnValue");
    }
    while (first) {
        if (((first->element) == element) && ((first->ipt) == ipt)) {
            first->strn = strn;
            return (NORMALEND);
        }
        first = first->next;
    }
    sprintf(messageBuffer, "Elem %d ipt %d not found in array", element, ipt);
    return (ERROREND);
}

int PrintPoints(DPoint* first)
{
    if (!(*first)) {
        sprintf(messageBuffer, "Empty list passed to PrintPoints");
        return (ERROREND);
    }
    int element, ipt, float strn;
    while (first) {
        element = first->element;
        ipt = first->ipt;
        strn = first->strn;
        printf("Element %d, Integration Point %d, Strn %f\n", element, ipt, strn);
        first = first->next;
    }
}

```

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Having obtained the X-Y-TN table the X values were interpolated into tables of Q-vs-X, E-vs-X, phi-vs-X, and C-vs-X (the material property tables) to obtain values of Q, E, phi, and C at the element integration points, using code StrnVsProps (pp. 40-41).

The material property tables were developed from the ESF rock-mass quality data using procedure described on pp. 18-22 and implemented through code mechProps (pp. 42-44).

The digitized ESF Q data (file esfRmQ.txt) used to develop the material property tables (with code mechProps) are reproduced on p. 45.

The resulting scatter plots of TN-vs-Q, TN-vs-E, TN-vs-phi, and TN-vs-C (based on results at element integration points) are presented on pp. 46-49. Each mechanical parameter is plotted as a scaled value, defined as

$$\text{ScaledValue} = \frac{(\text{ActualValue} - \text{MinimumValue})}{(\text{MaximumValue} - \text{MinimumValue})}$$

Values of minimum and maximum values are as follows (obtained as by-product of code StrnVsProps on pp. 40-41):

		mechPMaxMin.txt	
Current	PropertyName	MinimumValue	MaximumValue
Based on degraded Q and phi values.	NGI Q	0.729	13.573
	Ymod GPa	2.432	11.060
	FricAngle	22.66	30.16
	Cohesion MPa	1.667	3.514

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```
// StrNvsProps.cpp
// Setup vectors for mechanical property vs x-coordinate tables and read the
// tab data from file

float xCoord = 0;
float yCoord = 0;
float yMod = 0;
float fric = 0;
float cpar = 0;
int numData;
if (ReadMatrProps(xCoord,xMod,xrmQ,yCoord,yfric,cpar,numData) == ERRORDEN){
    DeleteVectors(xCoord,xrmQ,yCoord,yfric,cpar);
    DumpAndQuit();
}
cout << "Found " << numData << " lines in material property file\n";

char buf[100];
size_t nbuf = sizeof(buf);
while (fgets(buf,nbuf,stdin)!=NULL){
    if (!feof(stdin))
        sprintf(messagebuffer,"Unable to open file %s",strnFile);
        DeleteVectors(xCoord,xrmQ,yCoord,yfric,cpar);
        DumpAndQuit();
}

fstream fout(outfile);
if (!fout.is_open())
    sprintf(messagebuffer,"Unable to open file %s",outfile);
DeleteVectors(xCoord,xrmQ,yCoord,yfric,cpar);
DumpAndQuit();

out << setw(10) << "x-cr'd m"
<< setw(10) << "y-cr'd m"
<< setw(10) << "Wt Q"
<< setw(10) << "Ymod"
<< setw(10) << "fricAng"
<< setw(15) << "Cohesion"
<< setw(15) << "Microstrain"
<< endl;

int element,iPoint;
float x,y;
float strn;
float q,yM,phi,cp;

while (fscanf(buf,"%d",&iPoint);
        fscanf(buf,"%d",&x);
        fscanf(buf,"%d",&y);
        fscanf(buf,"%d",&q);
        fscanf(buf,"%d",&yM);
        fscanf(buf,"%d",&phi);
        fscanf(buf,"%d",&cp);
        fscanf(buf,"%d",&p));
        if (impline.good() && (strn > 0.0)) {
            if (impline.good()){
                q = LinearInterp(xCoord,rMQ.x.numData);
                ym = LinearInterp(xCoord,yMod.x.numData);
                phi = LinearInterp(xCoord,fric.x.numData);
                L = LinearInterp(xCoord,cpar.x.numData);
                Fout << setiosflags(ios::fixed | ios::showpoint)
                    << setprecision(3)
                    << setw(10) << x
                    << setw(10) << y
                    << setw(10) << q
                    << setw(10) << ym // << setw(10) << (ym/1000.)
                    << setw(10) << phi
                    << setw(15) << cp
                    << resetiosflags(ios::fixed)
                    << setiosflags(ios::scientific)
                    << setprecision(3)
                    << setw(15) << L
                    << setw(15) << p
                    << endl;
            }
        }
}
```

```

StrNvSProps.cpp *****
//*****
StrNvSProps

This code prepares a table of x and y coordinates, mechanical parameters
and material properties. It also calculates the element number and
interpolating results from a finite element model. The mechanical-property
values for the output table are scaled as follows:

ScaledValue = (ActualValue - MinimumValue)/(MaximumValue - MinimumValue)

As a result, every mechanical-property value in the output table lies between
0.0 and 1.0. Information is read from two files.

1. strnfile, which gives x and y coordinates and inelastic strain intensity
at element integration points.
2. matFFile, which gives values of NGI Q, Young's modulus, friction angle,
and cohesion as functions of x coordinate.

For each input line read from strnfile, the x coordinate is
interpolated in the material property tables to obtain values of NGI Q,
Young's modulus, friction angle, and cohesion for the integration point.

Author: G. I. Okegbu
Date: October 8 1998
C++ Compiler
(System developed using Borland C++Builder 3)

#include <stdio.h>
#include <iostream.h>
#include <fstream.h>
#include <iomanip.h>
#include <stream.h>
#include <string.h>
#include <conio.h>
#include <stdlib.h>
#pragma hdrstop
#include <condefs.h>

char* outFile = "s0150scldProp.dat";
char* strFile = "d:\\kystr\\strnfile.txt";
char* matFile = "d:\\kystr\\matFFile.txt";
char* maxminFile = "d:\\ThermMech\\ReposScale\\mechMaxMin.txt";

char* messageBuffer;
void DumpAndQuit();
void DetectVectors(float** x, float** g, float** ym, float** phi, float** cp);

int ERROREND = 1;
int NORMALEND = 0;
int ReadAtProps(float** x, float** rmQ, float** ym, float** phi,
float** cp, int num);
float LinearInter(const float* t, const float* f, float atEmp, int n);

//*****
Programme argued
int main(int argc, char **argv)
{
    // Allocate memory for possible error messages
    messageBuffer = new char [151];
    if (!messageBuffer)
        cerr << "Memory allocation error for messageBuffer\n";
    cerr << "Press any key to end : ";
    getch();
    return (1);
}

```

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```

StrNumProps.cpp

Fin,getline(buf,100);
istream impline(buf,dbuf);
impline >> xl >> q >> yml >> phil >> cpl;
if (!getline,good){
    (*XCOORD)[] = xl;
    (*WOOD)[] = (q - qmin)/qMax - qmin;
    (*WOOD)[] = (yml - ymlMin)/ymlMax - ymlMin;
    (*Frac)[] = (phil - phiMin)/phiMax - phiMin;
    (*cpar)[] = (cpl - cplMin)/cplMax - cplMin;
    if (++i) >= nn)
        break;
}

```

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```

StrN Drops.cpp

DeleteVectors(xCoord, rMq, fMod, fRic, fCarpar);
delete[] messageBuffer;
cout << "nDone ... Press any key to end: ";
getch();
return 0;
}

uint ReadMatProp(float** xCoord, float** rMq, float**
float** cpar, int* numData)
{
ifstream Fin(matPFile);
if (!Fin)
    printf("messageBuffer, \"Unable to open file %s\n\",
    matPFile);
return(ENRONEED);
}

ofstream Tout(maxinFile);
if (!Tout)
    printf("messageBuffer, \"Unable to open file %s\n\",
    matPFile);
return(ENRONEED);
}

```

```
char buf[100];
size_t nBuf = sizeof(buf);
float x1, y1, phi1, cpl;
float xMax = 0.01;
float yMax = 0.01;
float phiMax = 0.01;
float cpMax = 0.01;
float qMin = 1.0E10;
float yMin = 1.0E10;
float phiMin = 1.0E10;
float cpMin = 1.0E10;
int nn = 0;
```

```
while (Fin){
    Fin.getline(buf, nBuf);
    istreamline ignore(buf, nBuf);
    ignoreline ">";
    if (ignoreline == "good()"){
        if (q < qMin) qMax = q;
        if (q < qMin) qMin = q;
        if (y1 < yMinMax) yMax = y1;
        if (y1 > yMinMax) yMin = y1;
        if (p1 < p1Max) p1Max = p1;
        if (p1 > p1Min) p1Min = p1;
        if (cp1 < cpMax) cpMax = cp1;
        if (cp1 > cpMin) cpMin = cp1;
    }
    ++nPop;
}
```

```
if (!nn){
    printf(messageBuffer,"No valid input in file %s",matFile);
    return(ERROREND);
}
```

```
*xCoord = new float[m];
*mq = new float[n];
*yMod = new float[m];
*mc = new float[n];
*rc = new float[m];
if (!*xCoord || !*mq || !*yMod || !*mc || !*rc) {
    sprintf(msgBuf, "Memory allocation failure on matp vectors");
    return (ERROREND);
}
```

```

Fin.seekg(0,ios::beg); // Rewind input file
int i = 0;
while (Fin)

```

strnvsprops.cpp

```

return(f[n-1]); // This line is never reached; it was added to avoid
                // Borland-Compiler warning

void DeleteVectors(float** x, float** q, float** ym, float** phi, float** cp)
{
    delete[] *x;
    delete[] *q;
    delete[] *ym;
    delete[] *phi;
    delete[] *cp;
}

void DumpandQuit()
{
    cerr << "\n" << messageBuffer << "\n";
    delete[] messageBuffer;
    cerr << "Press any key to end: ";
    getch();
    exit(1);
}

```

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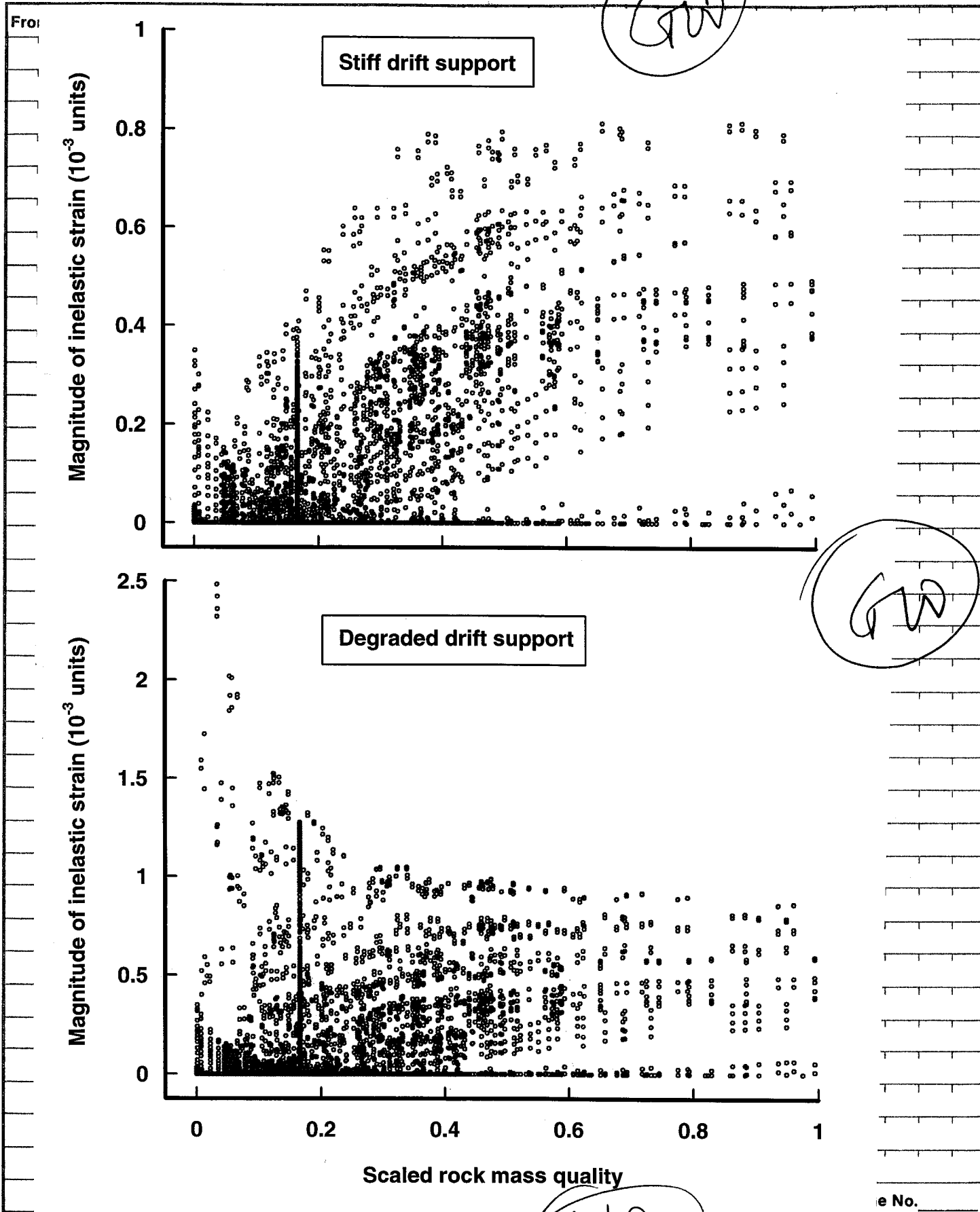
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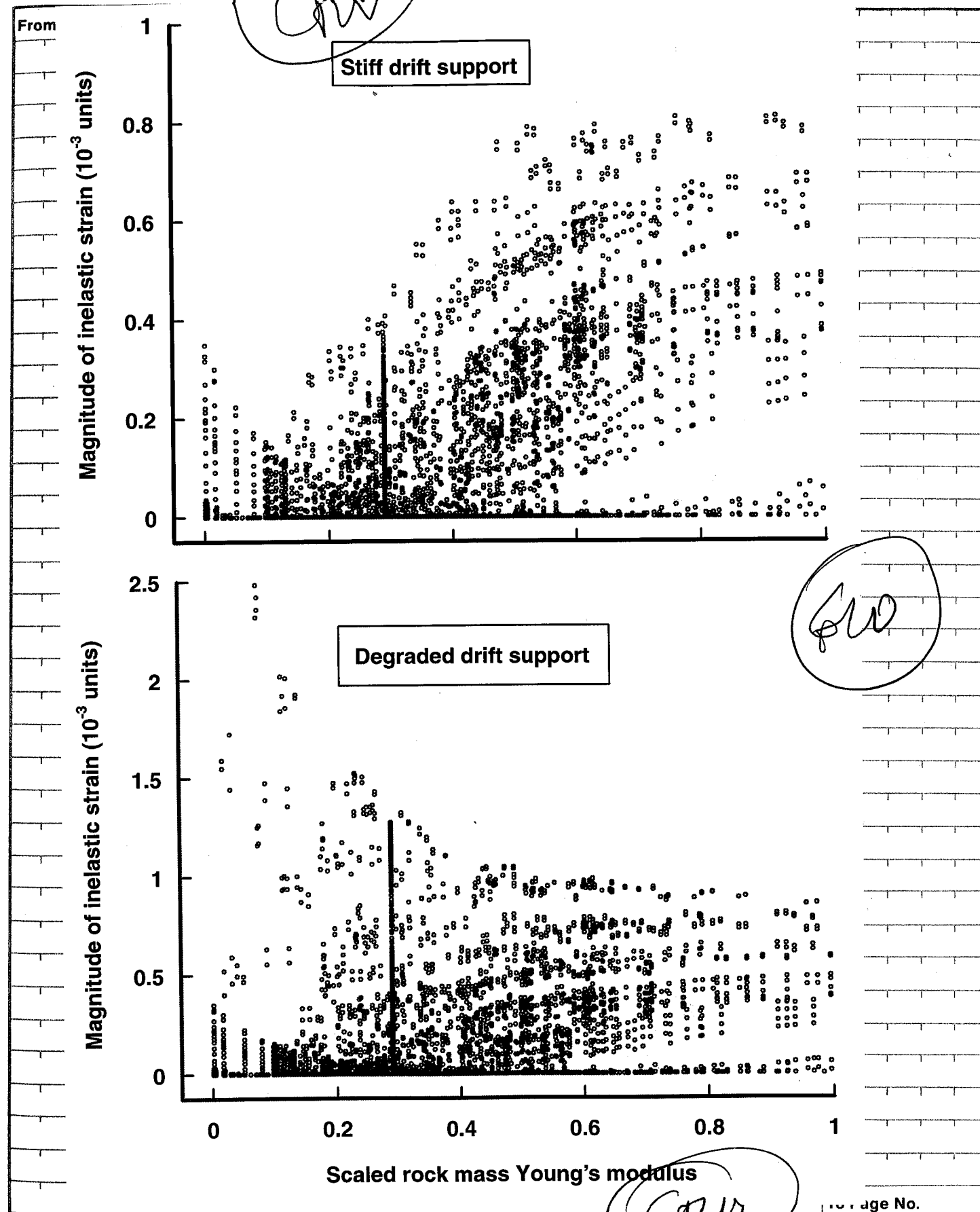
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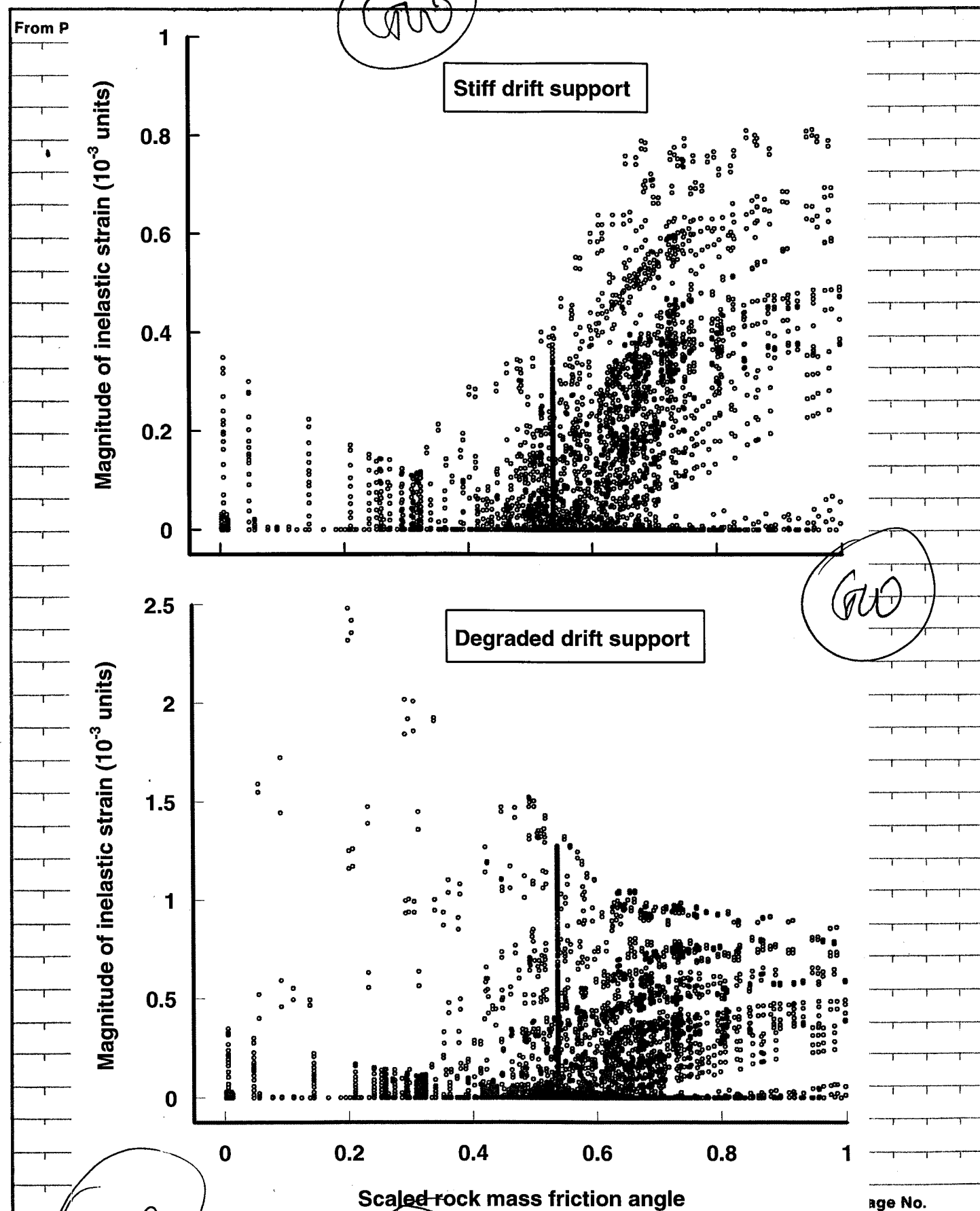
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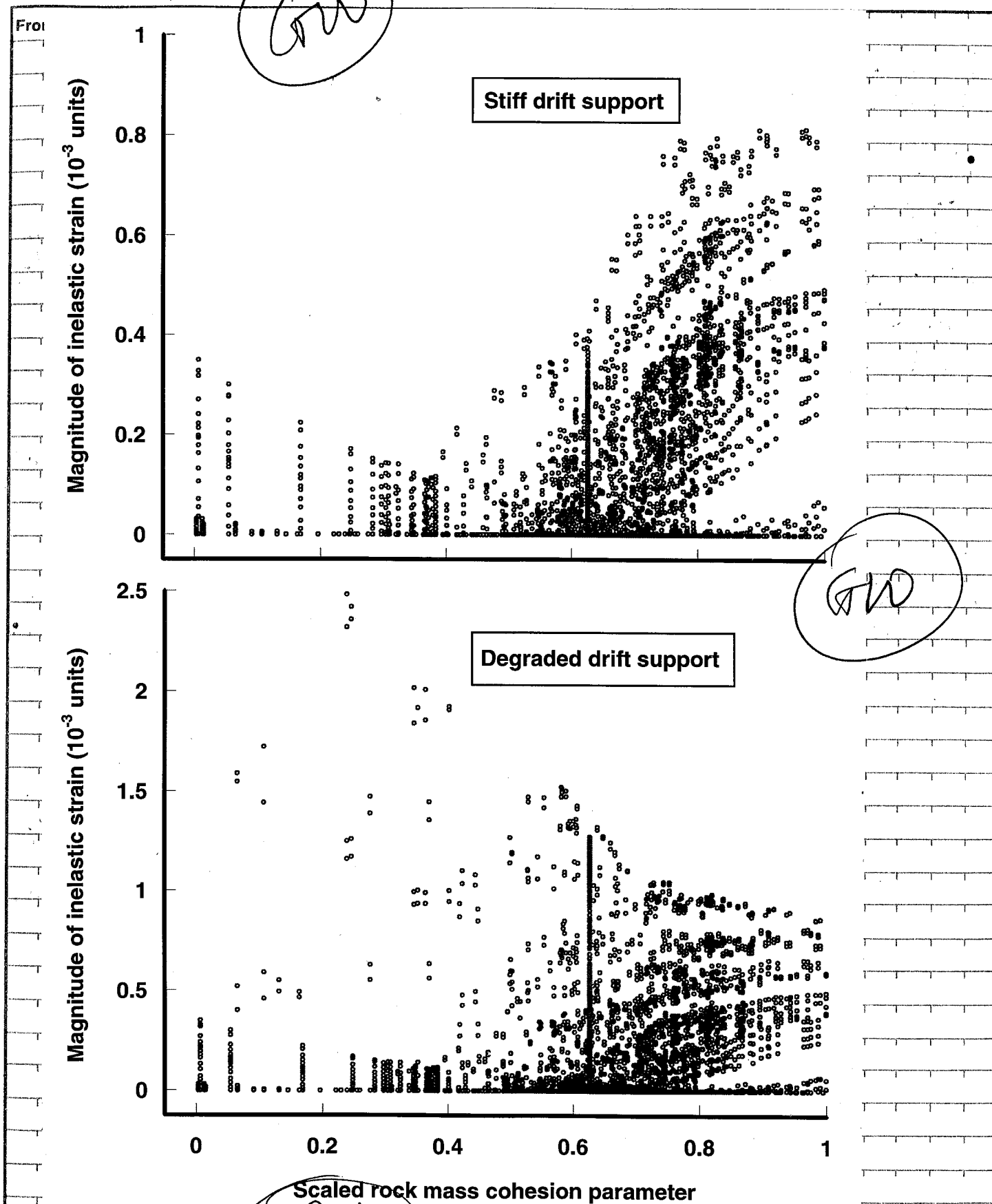
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From Page No. _____ Analysis to Develop Cumulative Percentile Curves of (Distributions) of Rock Mass Quality. GW 10/12/98

This analysis uses the output file from code StrnlvSProps (referred to as outFile on p. 40). The file gives values of x and y coordinates, rock mass quality (Q), Young's modulus, friction angle, and cohesion at a large number of finite-element integration points. Two such output files are analysed:

S0150ScldProp.dat: Case of stiff drift support
U0150ScldProp.dat: Case of degraded drift support.

The files are processed using SPLUS (statistical analysis code) through the following functions:

function(fig = F, tbl = F)

```
{
  subDir <- "RepoScale/PlasticModel/"
  infile <- paste(subDir, "u0150ScldProp.dat", sep = "")
  figfil <- paste(subDir, "rmQ.ps", sep = "")
  tblfil <- paste(subDir, "rmQ.dat")
  mat <- list(x = 0, y = 0, q = 0, eMod = 0, fric = 0, cpar = 0, strn = 0)

  mat <- scan(infile, what = mat, skip = 1)
  rmq <- mat$q
  pLevels <- seq(0, 1, 0.05)
  rmqLevels <- quantile(rmq, probs = pLevels)
  plot(rmqLevels, 100 * pLevels, type = "l", xlab = "Scaled rock mass quality", ylab = "Occurrence percentile")

  if(fig) {
    ps.portrait()
    dev.print(device = postscript, file = figfil)
  }

  if(tbl) {
    qMin <- 0.729
    qMax <- 13.573
    xq <- qMin + (qMax - qMin) * rmqLevels
    cat(paste("ScaledRMQ", "RMQ", "Percentile", "\n", sep = " "),
        file = tblfil)
    cat(paste(format(rmqLevels), format(xq), format(100 * pLevels),
        "\n", sep = " "), file = tblfil, append = T)
  }
}
```

Analysis for total integration-point population, irrespective of yield status.

Reads table into memory using format defined on previous line, which corresponds with input file format.

Calculates percentile values of rock mass quality at the percentage levels in pLevels.

Prints output to table.

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```
function(fig = F, tbl = F)
{
  subDir <- "RepoScale/PlasticModel/"
  infile <- paste(subDir, "s0150ScldProp.dat", sep = "")
  figfil <- paste(subDir, "s0150g00rmq.ps", sep = "")
  tblfil <- paste(subDir, "s0150g00rmq.dat")
  mat <- list(x = 0, y = 0, q = 0, eMod = 0, fric = 0, cpar = 0, strn = 0)

  mat <- scan(infile, what = mat, skip = 1)
  strn <- mat$strn
  rmq <- mat$q[strn > 0]
  pLevels <- seq(0, 1, 0.05)
  rmqLevels <- quantile(rmq, probs = pLevels)
  plot(rmqLevels, 100 * pLevels, type = "l", xlab = "Scaled rock mass quality", ylab = "Yield percentile")

  if(fig) {
    ps.portrait()
    dev.print(device = postscript, file = figfil)
  }

  if(tbl) {
    qMin <- 0.729
    qMax <- 13.573
    xq <- qMin + (qMax - qMin) * rmqLevels
    cat(paste("ScaledRMQ", "RMQ", "CumYield%", "\n", sep = " "),
        file = tblfil)
    cat(paste(format(rmqLevels), format(xq), format(100 * pLevels),
        "\n", sep = " "), file = tblfil, append = T)
  }
}
```

Analysis for integration points under plastic state for case of stiff drift support.

Extracts Q values for all points with $FN > 0$.

Calculates Q percentiles for this population.

Prints result.

Results are written to output files as follows:

ScaledRMQ	RMQ	Percentile
0.000	0.729000	0
0.058	1.473952	5
0.089	1.872116	10
0.108	2.116152	15
0.131	2.411564	20
0.161	2.796884	25
0.166	2.861104	30
0.166	2.861104	35
0.166	2.861104	40
0.166	2.861104	45
0.166	2.861104	50
0.166	2.861104	55
0.177	3.002388	60
0.208	3.400552	65
0.262	4.094128	70
0.302	4.607888	75
0.356	5.301464	80
0.406	5.943664	85
0.472	6.791368	90
0.581	8.191364	95
0.994	13.495936	100

rmQ.dat

File name

See code on p. 50, where this file is identified as variable tblfil.

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Rock-mass quality distribution in terms of cumulative percentiles for total population of integration points.

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ScaledRMQ	RMQ	CumYield%
0.0000	0.729000	0
0.0540	1.422576	5
0.1076	2.111014	10
0.1400	2.527160	15
0.1660	2.861104	20
0.1660	2.861104	25
0.1660	2.861104	30
0.1660	2.861104	35
0.1660	2.861104	40
0.1940	3.220736	45
0.2330	3.721652	50
0.2740	4.248256	55
0.3080	4.684952	60
0.3480	5.198712	65
0.3710	5.494124	70
0.4100	5.995040	75
0.4590	6.624396	80
0.4900	7.022560	85
0.5680	8.024392	90
0.6930	9.629892	95
0.9940	13.495936	100

s0150g00rmq.dat

File name.
See variable
tblfil in code
on p. 51.
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Rock-mass quality distribution in terms of
cumulative percentiles for integration
points under yield state for case of stiff
drift support.

ScaledRMQ	RMQ	CumYield%
0.000	0.729000	0
0.053	1.409732	5
0.091	1.897804	10
0.124	2.321656	15
0.155	2.719820	20
0.166	2.861104	25
0.166	2.861104	30
0.166	2.861104	35
0.166	2.861104	40
0.166	2.861104	45
0.196	3.246424	50
0.237	3.773028	55
0.279	4.312476	60
0.318	4.813392	65
0.356	5.301464	70
0.392	5.763848	75
0.439	6.367516	80
0.475	6.829900	85
0.551	7.806044	90
0.687	9.552828	95
0.994	13.495936	100

u0150g00rmq.dat

File Name

Rock-mass quality
distribution in
terms of cumulative
percentiles for
integration points
under yield state
for case of
degraded drift
support.

The three distributions (files on pp. 51 and 52)
were plotted together using Kaleidograph.
The resulting postscript file was read into

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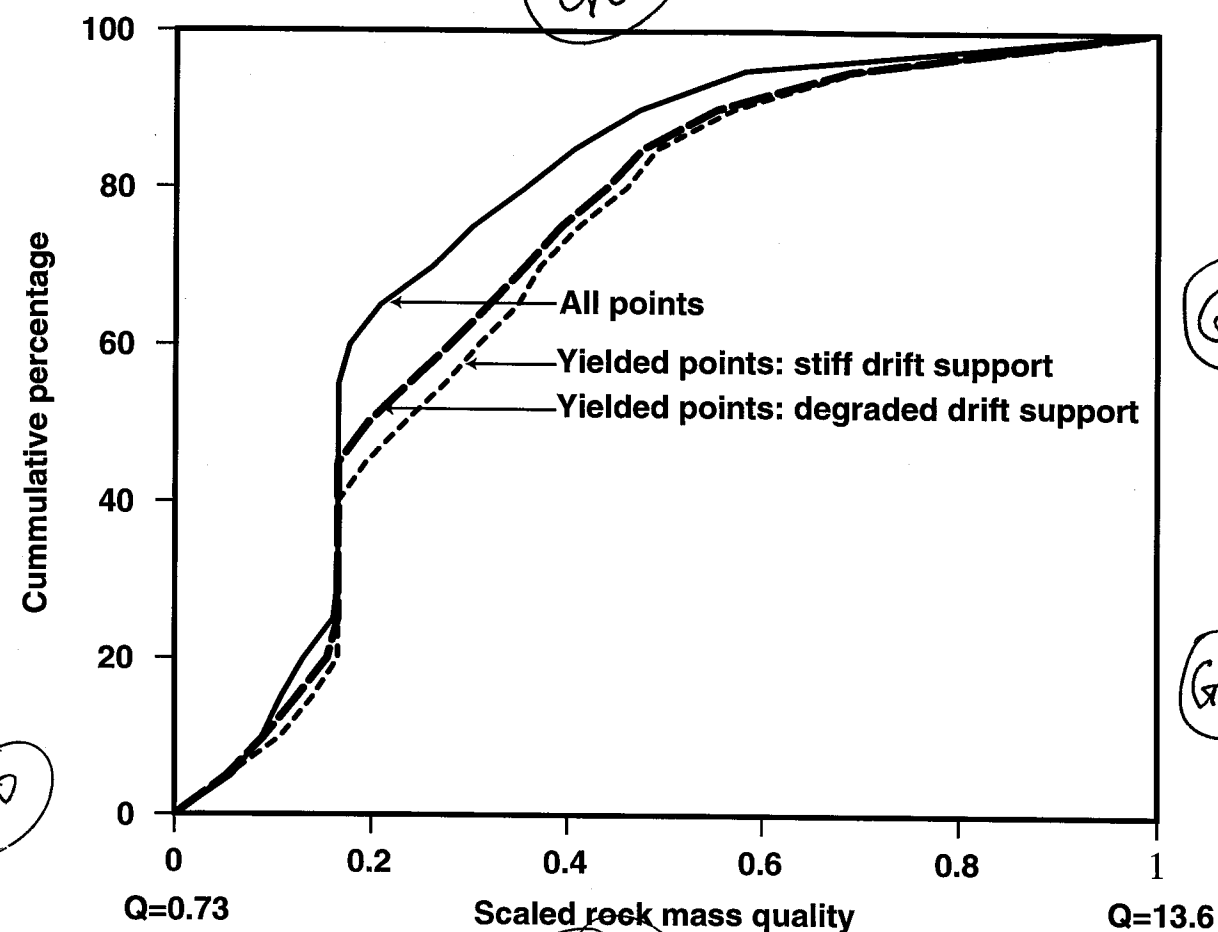
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Adobe Illustrator where the annotations
were enhanced.



$$\text{ScaledRockMassQuality} = \frac{Q - Q_{\min}}{Q_{\max} - Q_{\min}}$$

$$\left. \begin{array}{l} Q_{\min} = 0.73 \\ Q_{\max} = 13.6 \end{array} \right\} \text{see p. 39.}$$

Pages 32-53 entries
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Changes in Fracture Porosity and Permeability from Mechanical Deformation

Changes in fracture porosity and permeability can result from both elastic and inelastic deformations. The following discussion concerns changes due to inelastic deformations. Elastic changes will be considered later.

Relationship Between Fracture Porosity Change and Inelastic Volumetric Strain

Definitions

v_f = fracture volume

v_t = total volume (sum of fracture volume and solid-rock volume)

ϕ_f = fracture porosity = v_f/v_t

Consider a small rock body subjected to a complete cycle of loading and unloading such that all recoverable deformations caused by the loading is recovered during unloading. Let the rock body undergo inelastic (i.e., non-recoverable) deformation as a result of the loading and unloading cycle, such that its volume changes from v_{to} at the beginning of the cycle to v_{t1} at the end. The change in volume is accounted for entirely by a change in fracture volume from v_{fo} at the beginning of the cycle to v_{f1} at the end.

The inelastic volumetric strain ϵ^N is given by

$$\epsilon^N = \frac{v_{t1} - v_{to}}{v_{to}} = \frac{v_{f1} - v_{fo}}{v_{to}} = \frac{\Delta v_f}{v_{to}} \quad (1)$$

where $\Delta v_f = v_{f1} - v_{fo}$. The initial fracture porosity ϕ_{fo} is

$$\phi_{fo} = \frac{v_{fo}}{v_{to}} \quad (2)$$

and the final fracture porosity ϕ_{f1} is given by

$$\phi_{f1} = \frac{v_{f1}}{v_{t1}} = \frac{v_{fo} + \Delta v_f}{v_{to} + \Delta v_f} \quad (3)$$

which, dividing both numerator and denominator by v_{to} and using the result obtained earlier for ϵ^N and ϕ_{fo} , gives

$$\phi_{f1} = \frac{\phi_{fo} + \epsilon^N}{1 + \epsilon^N} \quad (4)$$

Because $1 + \epsilon^N \approx 1$, the foregoing equation implies

$$\phi_{f1} = \phi_{fo} + \epsilon^N \quad (5)$$

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which, using $\Delta\phi_f = \phi_{f1} - \phi_{fo}$, gives

$$\Delta\phi_f = \epsilon^N \quad (6)$$

That is, the change in fracture porosity resulting from inelastic deformation is equal to the inelastic volumetric strain.

Fracture Porosity and Fracture Aperture

It is assumed that all the fractures in the hypothetical rock body can be consolidated into a single fracture of aperture b . The fracture volume $v_f = bA_f$, where A_f is the surface area of the consolidated fracture (i.e., area of an imaginary surface parallel to and midway between the fracture walls). The total rock volume $v_t = A_f\lambda_c$, where λ_c is a characteristic length of the rock body normal to the fracture surface. For example, if the rock body is rectangular with the consolidated fracture parallel to one of the faces, then λ_c is the fracture-normal dimension of the rectangle.

It can be shown using the definition of fracture porosity that the consolidated fracture apertures at the beginning and end of the load-unload cycle, b_o and b_1 , respectively, are related to fracture porosity as follows:

$$b_o = \lambda_c\phi_{fo} \quad \text{and} \quad b_1 = \lambda_c(\phi_{fo} + \Delta\phi_f) \quad (7)$$

Fracture Permeability, Fracture Porosity, and Inelastic Volumetric Strain

Fracture permeability κ_f is related to fracture aperture through the equation

$$\kappa_f = \frac{b^2}{12} \quad (8)$$

Therefore, using the consolidated-fracture concept, the fracture permeabilities κ_{fo} and κ_{f1} , at the beginning and end, respectively, of the load-unload cycle are:

$$\kappa_{fo} = \frac{(\lambda_c\phi_{fo})^2}{12} \quad \text{and} \quad \kappa_{f1} = \frac{[\lambda_c(\phi_{fo} + \Delta\phi_f)]^2}{12} \quad (9)$$

Using these relationships, the fracture permeability at the end of a given deformation sequence (such as the hypothetical load-unload cycle introduced earlier) can be related to the initial fracture permeability and porosity as follows:

$$\kappa_{f1} = R_\kappa \kappa_{fo} \quad \text{where} \quad R_\kappa = \left(1 + \frac{\Delta\phi_f}{\phi_{fo}}\right)^2 = \left(1 + \frac{\epsilon^N}{\phi_{fo}}\right)^2 \quad (10)$$

The only input data required to calculate R_κ using this procedure (equation 10) is the

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From Page No. _____ initial fracture porosity ϕ_{fo} (where the word "initial" refers to the strain-free state). Fracture porosity data for the stratigraphic units at YM is given in the following document (cited hereafter as TSPA-VA2)

Chapter 2, Total System Performance Assessment - Viability Assessment Analyses Technical Basis Document: Unsaturated Zone Hydrology Model.

CRWMS/M&O Document Number
B00000000-01717-4301-00002 REV 00 (Aug 14 1998)

Table 2-19 of the document (reproduced on p. 57) gives ϕ_{fo} for the TSw unit & that ranges from 8.92×10^{-5} to 4.92×10^{-4} . A value of ϕ_{fo} of 1×10^{-4} will be applied in the model to obtain order-of-magnitude estimates of R_k .

The procedure described on p. 54-55 for calculation of permeability change associated with inelastic deformation was implemented in ABAQUS as a user-defined code module (see file KChange.f reproduced on p. 58).

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Table 2-19. Matrix (a) and Fracture (b) Parameter Sets for the DKM Base Case (Minimum V_f Present-day Infiltration Divided by Three). (DTN: LB971212001254.002)

Model Layer / Block Name	matrix permeability k_m (m ²)	matrix porosity ϕ_m (-)	matrix van Genuchten alpha α_m (1/Pa)	matrix van Genuchten m m_m (-)	matrix residual saturation S_{mr} (-)	matrix saturated saturation S_{ms} (-)
tcw11	5.37E-18	0.066	1.18E-06	0.232	0.13	1.00
tcw12	5.37E-18	0.066	1.32E-06	0.236	0.13	1.00
tcw13	4.90E-17	0.140	6.46E-07	0.427	0.33	1.00
ptn21	3.09E-14	0.369	3.80E-05	0.231	0.10	1.00
ptn22	3.02E-15	0.234	8.71E-06	0.488	0.14	1.00
ptn23	8.32E-14	0.353	4.57E-05	0.287	0.17	1.00
ptn24	1.15E-13	0.469	4.27E-05	0.349	0.10	1.00
ptn25	2.46E-13	0.464	1.95E-04	0.279	0.10	1.00
tsw31	4.90E-17	0.042	1.00E-05	0.237	0.11	1.00
tsw32	2.75E-16	0.146	2.29E-05	0.273	0.04	1.00
tsw33	1.15E-17	0.135	6.76E-06	0.248	0.06	1.00
tsw34	4.07E-18	0.089	1.02E-06	0.322	0.18	1.00
tsw35	1.55E-17	0.115	3.31E-06	0.229	0.08	1.00
tsw36	8.91E-17	0.092	7.41E-07	0.414	0.18	1.00
tsw37	1.29E-17	0.020	1.55E-06	0.387	0.50	1.00
ch1zc	1.38E-17	0.193	8.32E-07	0.366	0.36	1.00
ch2zc	9.12E-18	0.240	1.95E-06	0.220	0.20	1.00
ch3zc	9.12E-18	0.240	1.95E-06	0.220	0.20	1.00
ch4zc	1.55E-17	0.169	7.76E-07	0.477	0.33	1.00
ch1vc	1.32E-12	0.265	6.81E-05	0.190	0.04	1.00
ch2vc	2.57E-13	0.321	7.41E-05	0.224	0.06	1.00
ch3vc	2.57E-13	0.321	7.41E-05	0.224	0.06	1.00
ch4vc	2.57E-13	0.321	7.41E-05	0.224	0.06	1.00
pp3vp	2.82E-15	0.274	1.74E-05	0.311	0.07	1.00
bf3vb	2.82E-15	0.274	1.74E-05	0.311	0.07	1.00
tm3vt	2.82E-15	0.274	1.74E-05	0.311	0.07	1.00
pp2zp	5.75E-17	0.197	1.66E-06	0.316	0.18	1.00
bf2zb	5.75E-17	0.197	1.66E-06	0.316	0.18	1.00
tsw37/pcM37	6.08E-19	0.036	3.37E-07	0.372	0.20	1.00
ch1zc/pcM1z	5.40E-19	0.288	1.90E-07	0.359	0.36	1.00
ch2zc/pcM2z	4.50E-20	0.332	4.21E-06	0.228	0.20	1.00
ch2zc/pcM6z	4.50E-19	0.332	4.21E-06	0.228	0.20	1.00
ch3zc/pcM3z	4.50E-19	0.332	4.21E-06	0.228	0.20	1.00
ch4zc/pcM4z	8.40E-19	0.266	1.50E-07	0.476	0.33	1.00

(a)

Model Layer / Block Name	vertical fracture permeability k_v (m ²)	horizontal fracture permeability k_h (m ²)	fracture porosity ϕ_f (-)	fracture van Genuchten alpha α_f (1/Pa)	fracture van Genuchten m m_f (-)	fracture residual saturation S_{fr} (-)	fracture saturated saturation S_{fs} (-)	fracture frequency f (1/m)	fracture-matrix connection area modification factor X_m (-)
tcw11	2.29E-11	6.21E-12	2.33E-04	5.73E-04	0.492	0.01	1.00	1.020	1.90E-01
tcw12	1.38E-11	6.03E-12	2.99E-04	4.88E-04	0.492	0.01	1.00	1.830	1.90E-01
tcw13	2.82E-12	2.40E-13	7.05E-05	5.67E-04	0.492	0.01	1.00	1.270	1.90E-01
ptn21	5.25E-13	5.25E-13	4.84E-05	5.85E-04	0.492	0.01	1.00	0.870	1.00E+00
ptn22	1.95E-13	1.95E-13	4.83E-05	2.74E-04	0.492	0.01	1.00	0.290	1.00E+00
ptn23	2.57E-13	2.57E-13	1.30E-04	6.18E-04	0.492	0.01	1.00	0.290	1.00E+00
ptn24	6.17E-14	6.17E-14	6.94E-05	2.35E-04	0.492	0.01	1.00	0.630	1.00E+00
ptn25	7.76E-14	7.76E-14	3.86E-05	4.14E-04	0.279	0.01	1.00	0.650	1.00E+00
tsw31	1.07E-11	1.00E-12	8.92E-05	3.98E-05	0.481	0.01	1.00	1.100	5.00E-01
tsw32	1.51E-11	7.08E-13	1.29E-04	2.53E-04	0.488	0.01	1.00	1.010	1.90E-01
tsw33	2.63E-11	8.91E-13	1.05E-04	2.76E-04	0.492	0.01	1.00	0.690	1.90E-01
tsw34	6.76E-12	4.27E-13	1.24E-04	1.98E-04	0.492	0.01	1.00	1.880	1.90E-01
tsw35	3.80E-12	9.12E-13	3.29E-04	3.40E-04	0.492	0.01	1.00	1.810	1.90E-01
tsw36	1.20E-12	1.20E-12	3.99E-04	3.92E-04	0.492	0.01	1.00	2.100	1.90E-01
tsw37	1.20E-12	1.20E-12	4.92E-04	5.77E-04	0.492	0.01	1.00	2.880	1.90E-01
ch1zc	2.40E-14	2.40E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
ch2zc	1.18E-14	1.18E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
ch3zc	1.18E-14	1.18E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
ch4zc	1.55E-14	1.55E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
ch1vc	1.74E-13	1.74E-13	7.14E-05	5.42E-04	0.492	0.01	1.00	0.420	1.00E+00
ch2vc	2.88E-13	2.88E-13	7.14E-05	5.42E-04	0.492	0.01	1.00	0.420	1.00E+00
ch3vc	2.88E-13	2.88E-13	7.14E-05	5.42E-04	0.492	0.01	1.00	0.420	1.00E+00
ch4vc	2.88E-13	2.88E-13	7.14E-05	5.42E-04	0.492	0.01	1.00	0.420	1.00E+00
pp3vp	6.92E-13	6.92E-13	7.14E-05	2.53E-04	0.492	0.01	1.00	0.420	1.90E-01
bf3vb	6.92E-13	6.92E-13	7.14E-05	2.53E-04	0.492	0.01	1.00	0.420	1.90E-01
tm3vt	6.92E-13	6.92E-13	7.14E-05	2.53E-04	0.492	0.01	1.00	0.420	1.90E-01
pp2zp	6.46E-14	6.46E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
bf2zb	6.46E-14	6.46E-14	1.10E-05	2.85E-04	0.492	0.01	1.00	0.067	1.00E+00
tsw37/pcM37	3.04E-19	3.04E-19	4.92E-04	3.37E-07	0.372	0.20	1.00	2.880	1.00E+00
ch1zc/pcM1z	1.20E-18	1.20E-18	1.10E-05	1.90E-07	0.359	0.36	1.00	0.067	1.00E+00
ch2zc/pcM2z	3.50E-19	3.50E-19	1.10E-05	4.21E-06	0.228	0.20	1.00	0.067	1.00E+00
ch2zc/pcM6z	4.50E-19	4.50E-19	1.10E-05	4.21E-06	0.228	0.20	1.00	0.067	1.00E+00
ch3zc/pcM3z	4.50E-19	4.50E-19	1.10E-05	4.21E-06	0.228	0.20	1.00	0.067	1.00E+00
ch4zc/pcM4z	8.40E-19	8.40E-19	1.10E-05	1.50E-07	0.476	0.33	1.00	0.067	1.00E+00
tsw37/pwF37	3.04E-19	1.50E-13	4.92E-04	3.37E-07	0.372	0.20	1.00	2.880	1.00E+00

(b)

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August 1998

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KChange.f

```

SUBROUTINE UVARM(UVAR,DIRECT,T,TIME,DTIME,CMNAME,ORNAME,
1  NUVARM,NOEL,NPT,LAYER,KSPT,KSTEP,KINC,NDI,NSHR)
C
C  INCLUDE 'ABA_PARAM.INC'
C  CHARACTER*8 CMNAME,ORNAME,FLGRAY(15)
C  DIMENSION UVAR(NUVARM),DIRECT(3,3),T(3,3),TIME(2)
C  DIMENSION ARRAY(15),JARRAY(15)
C
C  This code computes fracture-permeability change ratio (Kr)
C  following procedure documented in CNWRA Scientific Notebook
C  #263, pp. ---
C
C  Values of inelastic strain required for the calculation are
C  obtained through ABAQUS user-interface subroutine GETVRM
C  The externally supplied input parameter is:
C
C  PHI0F      Average consolidated fracture porosity
C              (i.e., total fracture volume per unit bulk volume)
C              at strain-free state.
C
C  The calculated value of Kr is stored in UVAR(1)
C
C  PHI0F = 1.0D-4
C
C  Obtain current values of inelastic (plastic) strain components
C
C  JRCD = 0
C  CALL GETVRM('PE',ARRAY,JARRAY,FLGRAY,JRCD)
C  IF (JRCD.NE. 0) THEN
C      WRITE(6,1000) NOEL,NPT,TIME(2)
C      RETURN
C  END IF
C
C  DPHIF = ARRAY(1) + ARRAY(2) + ARRAY(3)
C  RATIO = 1.0 + DPHIF/PHI0F
C  UVAR(1) = RATIO*RATIO
C  RETURN
C
C-----67--1-----2-----3-----4-----5-----6-----7--*
C
1000 FORMAT(/,'ERROR IN UVARM-CALL FOR VARIABLE IE',/,
1  10X,'FOR ELEMENT NUMBER = ',I5,/,
2  10X,'INTEGRATION POINT = ',I5,/,
3  10X,'AT TIME = ',E12.3)
END

```

Gw

Gw 10/23/98

Pages 54-58 entries
Gw 10/23/98

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Elastic-plastic behavior was hitherto applied over the entire model, but previous results indicate that plastic behavior is restricted to a thin zone above and below the repository horizon. As a result, the model is now simplified (by restricting elastic-plastic material behavior to a selected zone) to reduce both analysis time (computer time) and size of results files.

Elastic Zone

All elements more than 80 m above or more than 80 m below repository horizon. The element-group names are

ELTZ	All Zone 2 (p. 6) elements above $y \approx 795$ m
ELBZ	All Zone 3 (p. 6) elements below $y \approx 600$ m
Z4ELEM	All Zone 4 Elements (p. 6)

Poisson's ratio and expansivity are as defined previously (p. 15). Young's modulus varies with x coordinate as defined previously (p. 22).

Elastic-Plastic Zone

All elements within 80 m above and 80 m below the emplacement drifts, i.e., the elastic-plastic zone is about 165 m thick.

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The element-group names for the elastic-plastic zone are:

PLTZ All zone 2 elements (p. 6) below $y \approx 795$ m

PLBZ All zone 3 elements (p. 6) above $y \approx 600$ m

MZELEM All zone 1 elements (p. 6).

Nov 10 1998

Five analysis cases will be performed as follows

Case 1: Q varies with x as defined on p. 18

Case 2: Q is uniform and equal to minimum Q (i.e., Q at $x = 55$ m) from Case 1.

Case 3: Q is uniform and equal to maximum value (i.e., Q at $x = 1474$ m) from Case 1

Case 4: Same as Case 1, but analysis performed using current (instead of degraded) values of strength parameters

Case 5: Same as Case 1, but maximum

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friction-angle curve from the Hoek-Brown chart (p. 22) is used.

Case 1 Details

Input files:

m01Base.inp

m01Base.inp

Mechanical analysis to end of excavation (no heat load)

m01.inp

Mechanical analysis from end of excavation (and instant waste emplacement) to 150 yr.

Auxiliary input files

allNodes.def
mElem01.def
iniTemp.def
mdfld1.def

Node definitions
Element definitions
Initial nodal temperature
X-as-field definition.

mElem01.def incorporates the following files

ElPropCurrent.def
FricAngLongTerm.def

Elastic properties. Current values
Drucker-Prager frictional behavior. Long-term values

CohesionLongTerm.def

Drucker-Prager Cohesion Parameters. Long-term values.

GW

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Case 2 Details $Q = 0.7295$ (from $x = 55$ m in Case 1) $E = 8.02 \times 10^3$ MPa (Young's modulus) $\beta = 33.05^\circ$ Drucker-Prager friction angle
(equivalent to Mohr-Coulomb friction angle of 23°)Dilation angle = 16.52° *GW* 10/29/98

Drucker-Prager Cohesion = 2.63 MPa (equivalent to Mohr-Coulomb cohesion of 1.67 MPa).

Input files (See Case 1 for brief description of each file's content)m02Base.inp
m02.inpallNodes.def
mElem02.def
iniTemp.def.November 16 1998Case 3 Details $Q = 13.57$ (from $x = 1474$ m in Case 1) $E = 3.65 \times 10^4$ MPa $\beta = 40.46^\circ$ (equivalent $\phi = 30.2^\circ$)Dilation angle = 20.23° Drucker-Prager Cohesion = 5.16 MPa (equivalent $c = 3.5$ MPa)

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Input filesm03Base.inp
m03.inpallNodes.def
mElem03.def
iniTemp.def.Case 4 DetailsInput Files (Compare with Case 1)m04Base.inp
m04.inpAuxiliary Input FilesallNodes.def
mElem04.def
iniTemp.def
m04fld1.def

mElem04.def incorporates the following files

- ElPropCurrent.def
- FricAngCurrent.def
- CohesionCurrent.def

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Case 5 DetailsInput Filesm05Base.inp
m05.inpallNodes.def
mElem05.def
iniTemp.def
mdf1d1.defElPropCurrent.def
CohesionLongTerm.def
L+2FricDP.defDrucker-Prager friction
properties for case of friction
angle from maximum curve
on Hoek-Brown chart.Pages 59-64
entries by GW 10/29/98Results ProcessingPages 64-69 entries
by GW 11/23/98
November 23 1998Each analysis result is processed using
ABAQUS/POST and the appropriate .res
file from ABAQUS. Contour plots of
plastic strain, permeability ratio R_k (p. 55)
or rock-mass quality Q (p. 32-35). The contours
are plotted by executing the following
command files while inside ABAQUS/POST

peContPlots.jnl for plastic strain

input, file=peContPars.def
input, file=grpsOf10.def
input, file=g10PeContours.def

} File peContPlots.jnl

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input, file=kContPars.def
input, file=grpsOf10.def
input, file=g10KContours.defFile kContPlots.jnl for R_k input, file=qContPars.def
input, file=grpsOf10.def
input, file=g10QContours.defFile qContPlots.jnl for Q The auxiliary command files pulled in by
these files are reproduced here:set, fill
set, outline=per
set, outlin=0
set, clev=5
set, clegendsize=.2
set, clegend=(.012, .99)
set, cmin=0.2e-3
set, cmax=2.0e-3

File peContPars.def

set, fill
set, outline=per
set, outlin=0
set, clev=5
set, clegendsize=.2
set, clegend=(.012, .99)
set, cmin=10
set, cmax=50

File kContPars.def

set, fill
set, outline=per
set, outlin=0
set, clev=5
set, clegendsize=.2
set, clegend=(.012, .99)
set, cmin=2.5
set, cmax=11.5

File qContPars.def

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File grps of 10. def

elset, elset=pgrp01, gen
1409, 4309, 100
1410, 4310, 100
1411, 4311, 100
1412, 4312, 100
1413, 4313, 100

elset, elset=pgrp02, gen
4409, 7309, 100
4410, 7310, 100
4411, 7311, 100
4412, 7312, 100
4413, 7313, 100

elset, elset=pgrp03, gen
7409, 10309, 100
7410, 10310, 100
7411, 10311, 100
7412, 10312, 100
7413, 10313, 100

elset, elset=pgrp04, gen
10409, 13309, 100
10410, 13310, 100
10411, 13311, 100
10412, 13312, 100
10413, 13313, 100

elset, elset=pgrp05, gen
13409, 16309, 100
13410, 16310, 100
13411, 16311, 100
13412, 16312, 100
13413, 16313, 100

elset, elset=pgrp06, gen
16409, 19309, 100
16410, 19310, 100
16411, 19311, 100
16412, 19312, 100
16413, 19313, 100

elset, elset=pgrp07, gen
19409, 22309, 100
19410, 22310, 100
19411, 22311, 100
19412, 22312, 100
19413, 22313, 100

elset, elset=pgrp08, gen
22409, 25309, 100
22410, 25310, 100
22411, 25311, 100
22412, 25312, 100
22413, 25313, 100

elset, elset=pgrp09, gen
25409, 28309, 100
25410, 28310, 100
25411, 28311, 100
25412, 28312, 100
25413, 28313, 100

elset, elset=pgrp10, gen
28409, 31309, 100
28410, 31310, 100
28411, 31311, 100
28412, 31312, 100
28413, 31313, 100

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*** Plot Drifts #100 thru 91 (counting down, i.e., South to North)

detail, elset=pgrp01
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #90 thru 81 (counting down, i.e., South to North)

detail, elset=pgrp02
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #80 thru 71 (counting down, i.e., South to North)

detail, elset=pgrp03
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #70 thru 61 (counting down, i.e., South to North)

detail, elset=pgrp04
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #60 thru 51 (counting down, i.e., South to North)

detail, elset=pgrp05
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #50 thru 41 (counting down, i.e., South to North)

detail, elset=pgrp06
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #40 thru 31 (counting down, i.e., South to North)

detail, elset=pgrp07
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #30 thru 21 (counting down, i.e., South to North)

detail, elset=pgrp08
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #20 thru 11 (counting down, i.e., South to North)

detail, elset=pgrp09
set, clegend=on
cont, v=peeq
pause

*** Plot Drifts #10 thru 1 (counting down, i.e., South to North)

detail, elset=pgrp10
set, clegend=on
cont, v=peeq

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g10KContours.def

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*** Plot Drifts #100 thru 91 (counting down, i.e., South to North)

detail,elset=pgrp01
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #90 thru 81 (counting down, i.e., South to North)

detail,elset=pgrp02
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #80 thru 71 (counting down, i.e., South to North)

detail,elset=pgrp03
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #70 thru 61 (counting down, i.e., South to North)

detail,elset=pgrp04
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #60 thru 51 (counting down, i.e., South to North)

detail,elset=pgrp05
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #50 thru 41 (counting down, i.e., South to North)

detail,elset=pgrp06
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #40 thru 31 (counting down, i.e., South to North)

detail,elset=pgrp07
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #30 thru 21 (counting down, i.e., South to North)

detail,elset=pgrp08
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #20 thru 11 (counting down, i.e., South to North)

detail,elset=pgrp09
set,clegend=on
cont,v=uvaml
pause

*** Plot Drifts #10 thru 1 (counting down, i.e., South to North)

detail,elset=pgrp10
set,clegend=on
cont,v=uvaml

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*** Plot Drifts #100 thru 91 (counting down, i.e., South to North)

detail,elset=pgrp01
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #90 thru 81 (counting down, i.e., South to North)

detail,elset=pgrp02
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #80 thru 71 (counting down, i.e., South to North)

detail,elset=pgrp03
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #70 thru 61 (counting down, i.e., South to North)

detail,elset=pgrp04
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #60 thru 51 (counting down, i.e., South to North)

detail,elset=pgrp05
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #50 thru 41 (counting down, i.e., South to North)

detail,elset=pgrp06
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #40 thru 31 (counting down, i.e., South to North)

detail,elset=pgrp07
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #30 thru 21 (counting down, i.e., South to North)

detail,elset=pgrp08
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #20 thru 11 (counting down, i.e., South to North)

detail,elset=pgrp09
set,clegend=on
cont,v=fv1
pause

*** Plot Drifts #10 thru 1 (counting down, i.e., South to North)

detail,elset=pgrp10
set,clegend=on
cont,v=fv1

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g10QContours.def

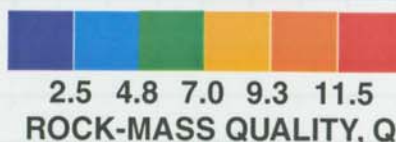
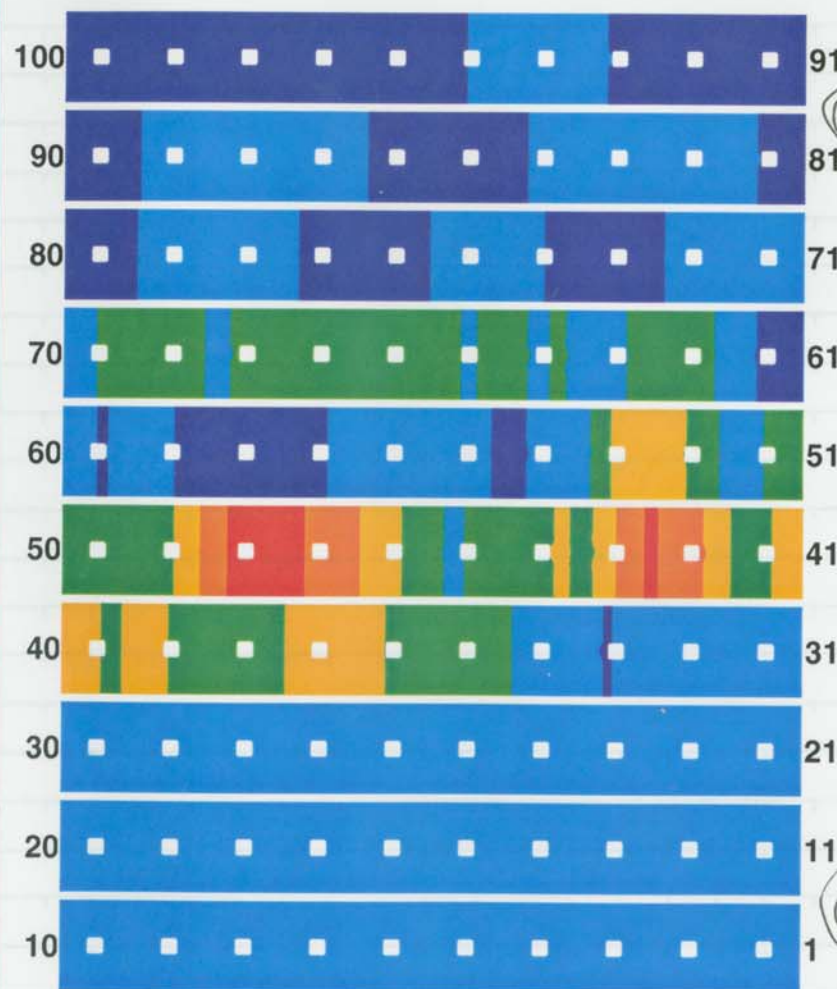
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From Page No. The plotting routines described on pp. 64-69 give contours of inelastic strain magnitude (Γ^N), permeability ratio R_p , and rock mass quality Q , over 35-m high strips of the model. Each strip is centered on the repository axis (15 m above roof, and 15 m below floor, of emplacement drifts) and covers 10 emplacement drifts. The strips were combined and assembled using ADOBE ILLUSTRATOR to obtain the composite plots presented here. Numbers at the ends of each drift strip indicate the range of emplacement drifts covered.

DISTRIBUTION OF ROCK-MASS QUALITY (Q)
ALONG EAST BOUNDARY OF PROPOSED DRIFTS



(1) Recall that Q value at drift #32 was applied over drifts #31 thru #1 because of lack of Q data in that area.

(2) This figure shows Q decreasing southward generally.

(3) Maximum Q is 13.57 (\approx GSI of ~ 67.5).

(4) Minimum Q is about 0.7295 (\approx GSI of ~ 41.2).

(5) Minimum Q occurs ^{blw} drift #99 and #98.

(6) Maximum Q occurs ^{blw} drift #48 and #47.

(7) This figure is same as ~~shown~~ shown on p. 35.

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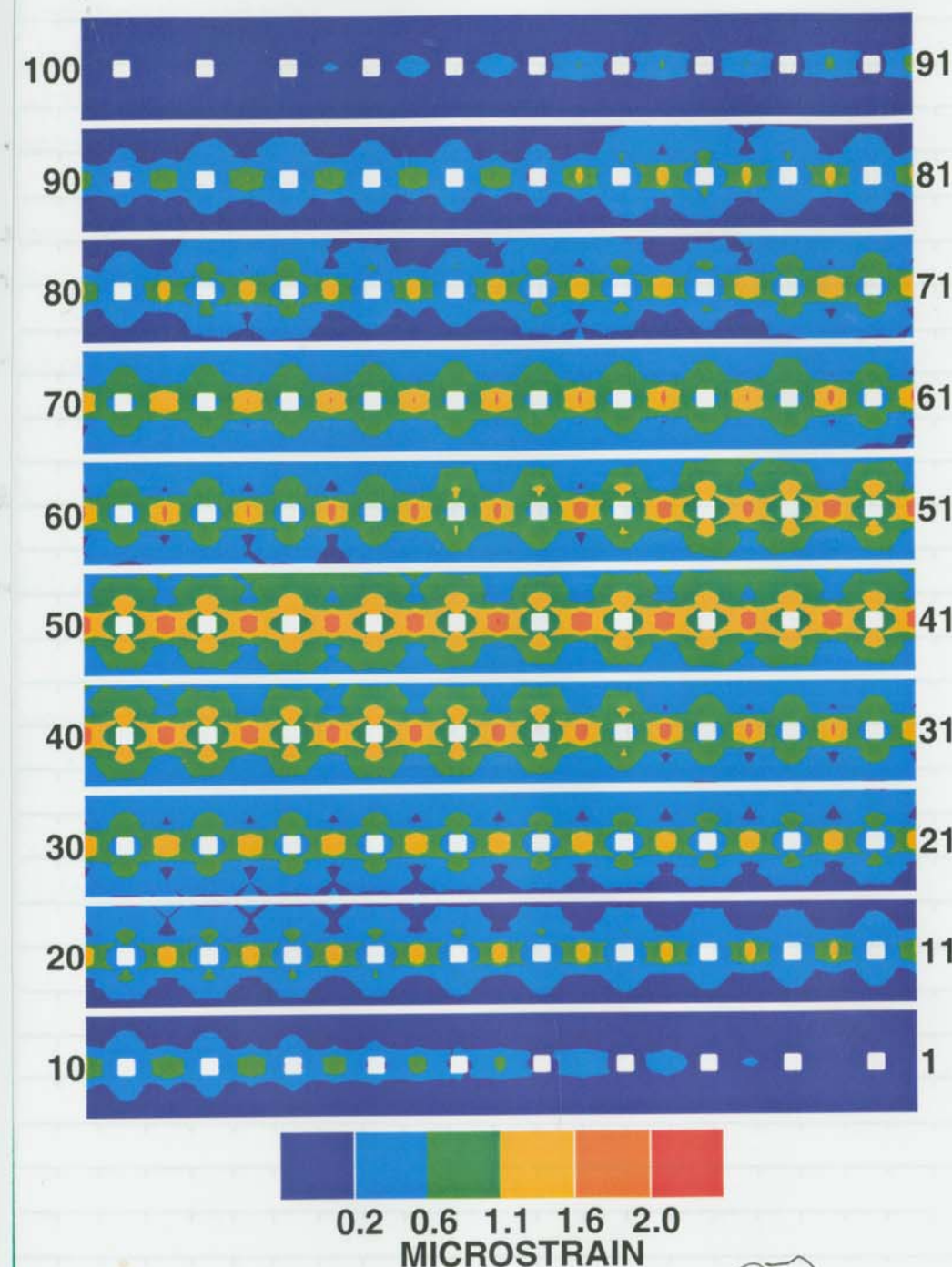
Invented by

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Recorded by

TITLE

INELASTIC STRAIN DISTRIBUTION AT 150 YR
STIFF DRIFT SUPPORT



$$\Gamma^N = \sum \sqrt{\frac{2}{3} \left[(\Delta \epsilon_{11})^2 + (\Delta \epsilon_{22})^2 + 2(\Delta \epsilon_{12})^2 \right]}$$

and $\Delta \epsilon_{11}$, $\Delta \epsilon_{22}$, and $\Delta \epsilon_{12}$ are the components of inelastic strain increment and the summation indicates accumulation of the square-root term through the simulation time. Therefore, the distribution of Γ^N at the end of the 150-yr simulation time, for example, represents all inelastic processes that occurred in the model from zero time to end of 150 yr.

This figure shows distribution of Γ^N at 150 yr in the presence of stiff drift support.
(1) Γ^N attains max values at pillar centers and secondary maxima at roof and floor areas.
(2) Highest Γ^N values occur in areas of higher Q.
(3) Γ^N distribution strongly influenced by Q distribution.

See bottom of figure for definition of Γ^N .

This result from Case 1 model.

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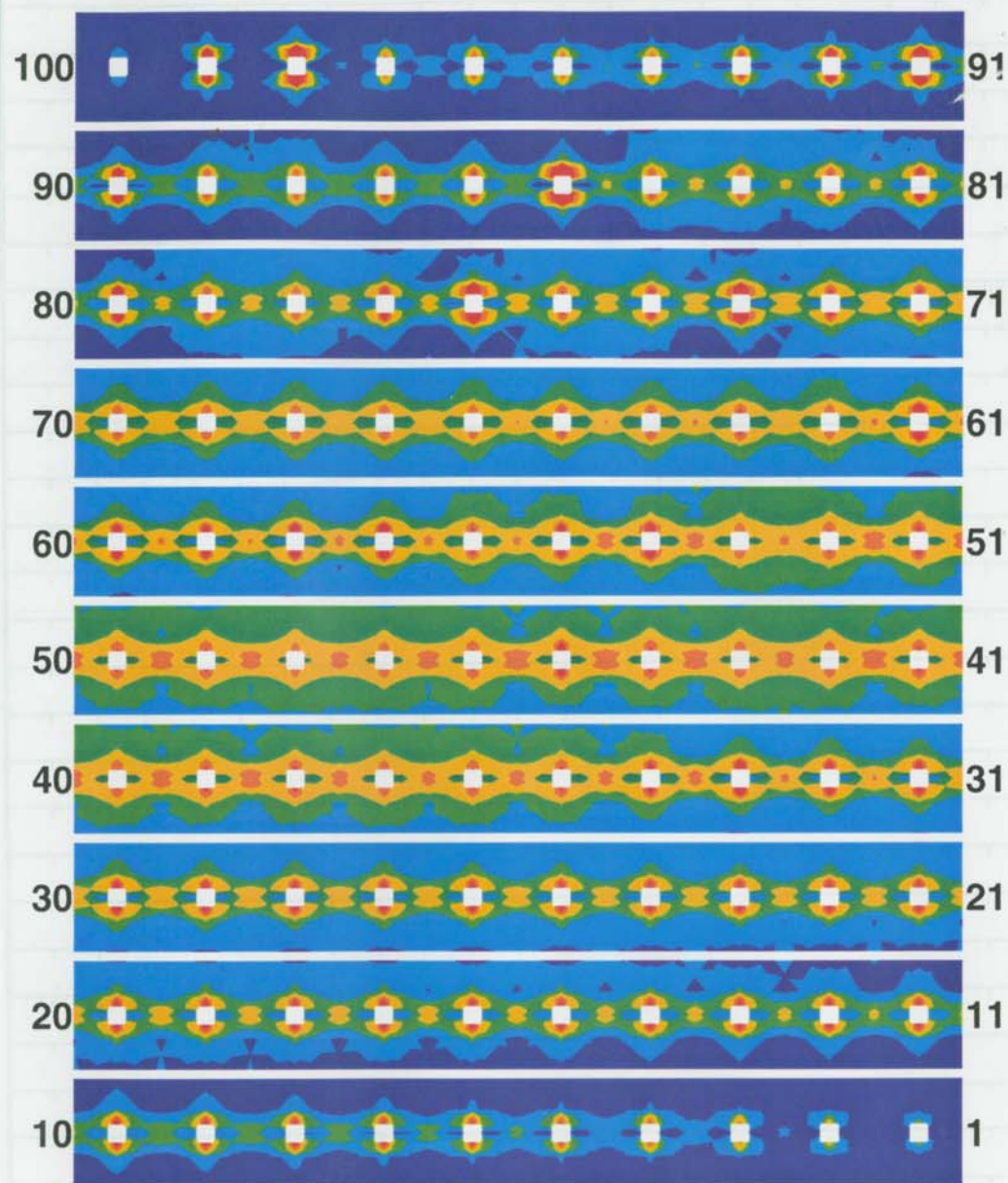
Date

Invented by

Date

Recorded by

INELASTIC STRAIN DISTRIBUTION AT 150 YR DEGRADED DRIFT SUPPORT



(1) Support degradation causes increased Γ^N everywhere, especially in roof and floor areas.

(2) Γ^N increases more in area of lower Q (compare distributions near drift #98 for example)

(3) Loss of confinement near openings owing to degradation of support has more severe effects in areas of lower Q .

This result from Case 1 model.

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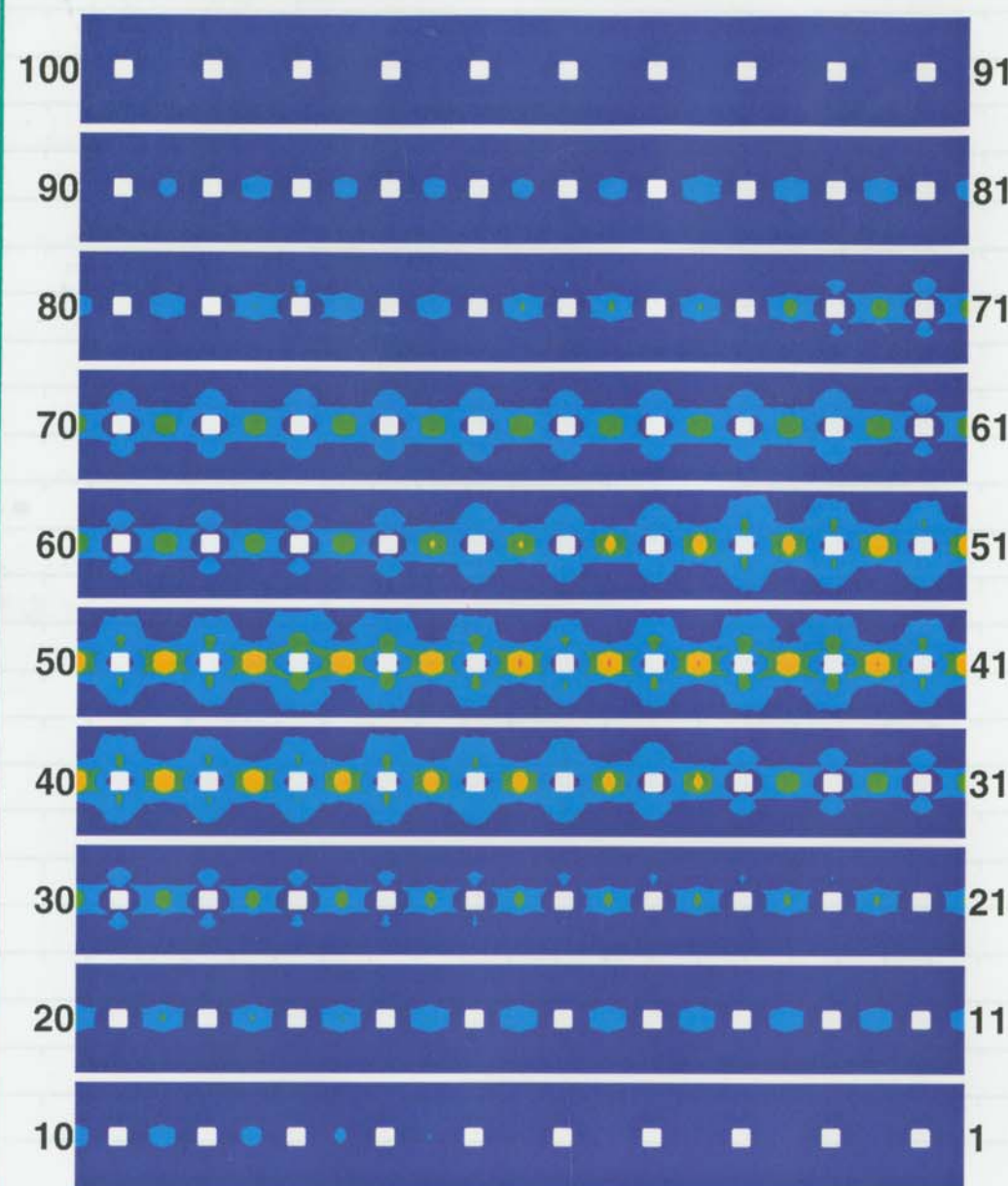
Date

Invented by

Date

Recorded by

SHEAR-INDUCED PERMEABILITY CHANGE STIFF DRIFT SUPPORT



Distribution of R_k from Case 1 in the presence of stiff drift support

(1) R_k attains maximum at pillar centers with secondary maxima at roof and floor areas.

(2) R_k is higher in areas of higher Q .

See p. 55 for definition of R_k and p. 70 for explanation of plot organization.

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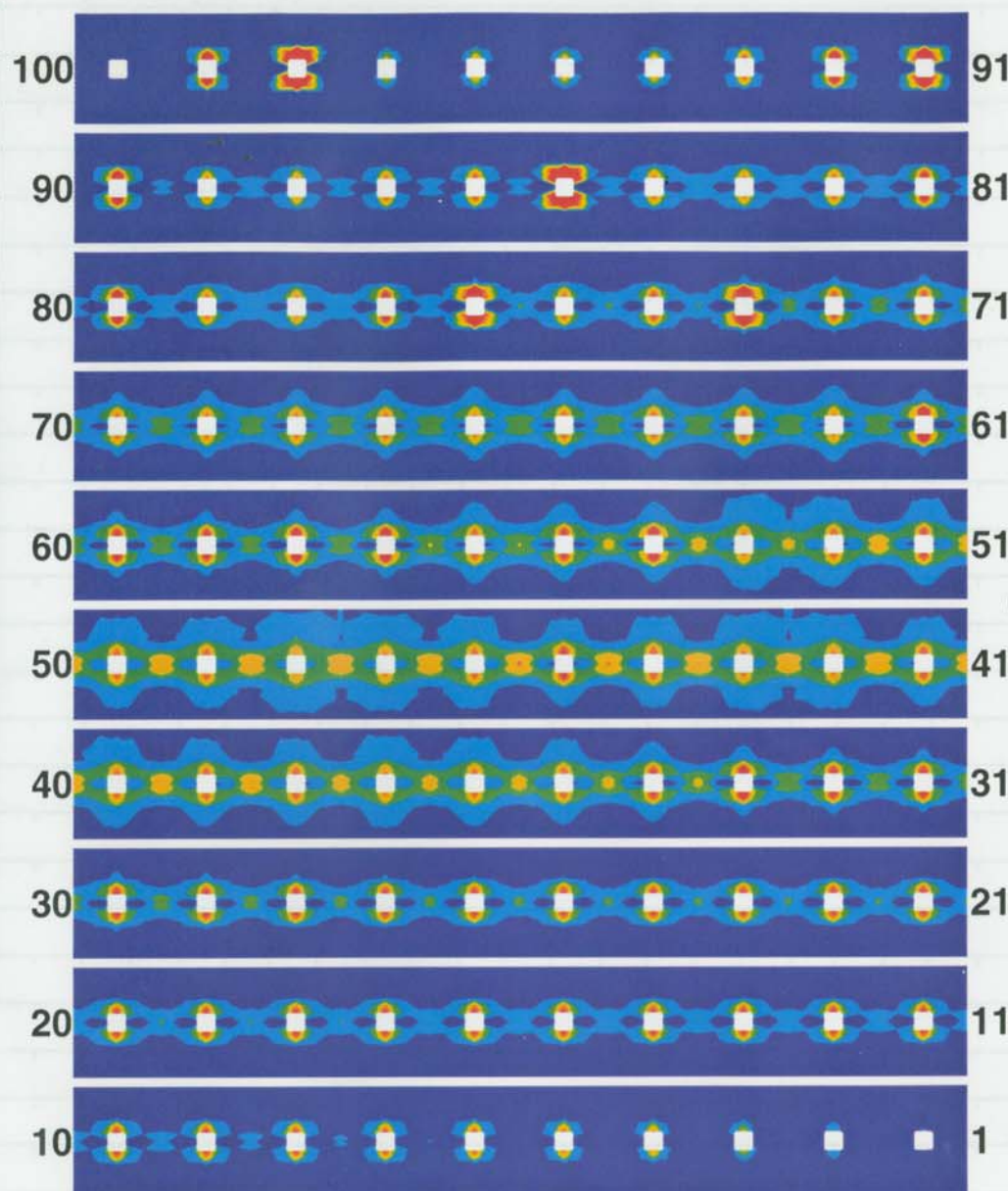
Date

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Date

Recorded by

SHEAR-INDUCED PERMEABILITY CHANGE DEGRADED DRIFT SUPPORT



10 20 30 40 50
PERMEABILITY RATIO

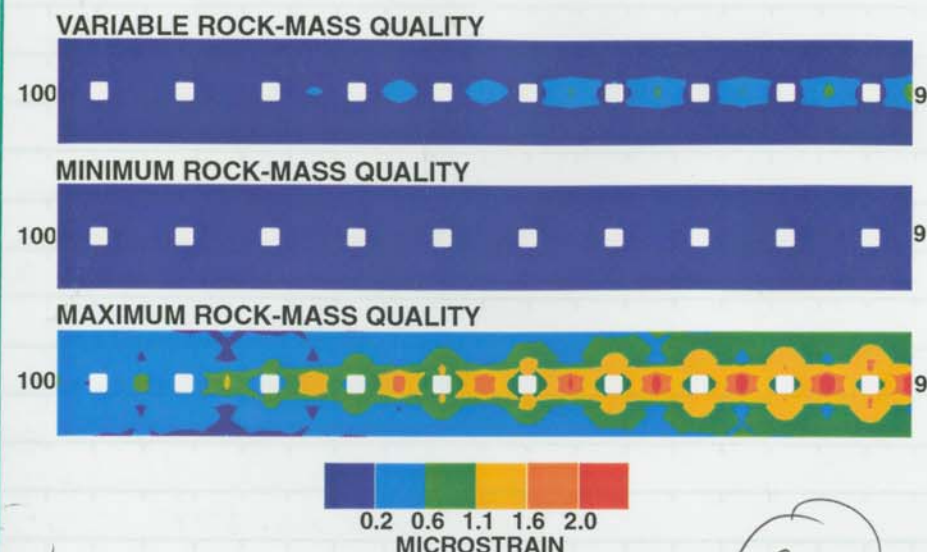
Distribution of R_p from Case 1 for degraded drift support.

(1) Support degradation causes increased R_p everywhere, especially in roof and floor areas.

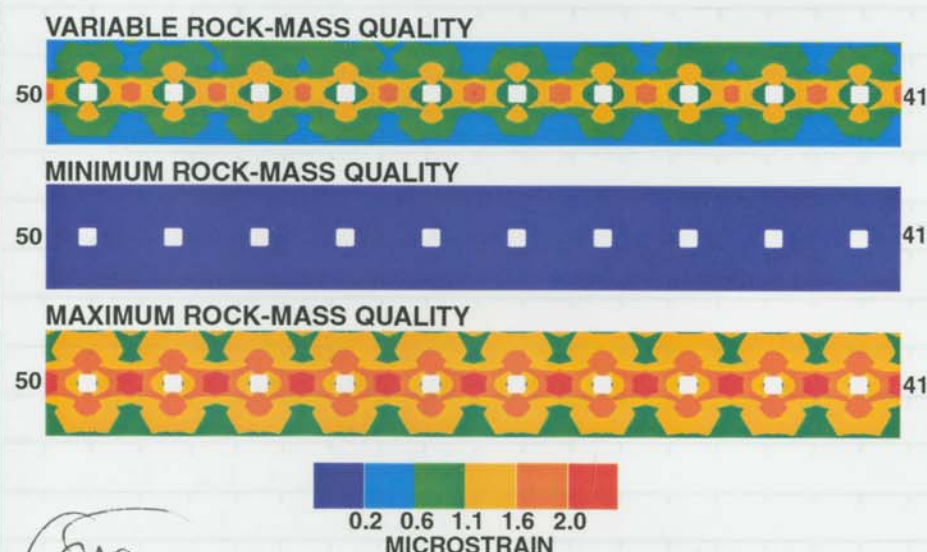
(2) Support degradation has more severe effects in areas of higher Q lower σ

12/14/98

INELASTIC-STRAIN DISTRIBUTIONS WITHIN 15 m ABOVE AND BELOW DRIFTS #91-100 (STIFF DRIFT SUPPORT)



INELASTIC-STRAIN DISTRIBUTIONS WITHIN 15 m ABOVE AND BELOW DRIFTS #41-50 (STIFF DRIFT SUPPORT)



Comparison of results from Cases 1, 2, and 3 with stiff ground support. Top plot is for low Q area of Case 1 and bottom plot for high Q area.

In the presence of stiff supports:
(1) Use of minimum Q in analysis would result in σ_N values that are too small everywhere.

(2) Use of maximum Q in analysis would result in σ_N values that are too large everywhere.

(3) If support load increases with σ_N (to be investigated) then use of minimum Q would result in under estimate of support load whereas use of maximum Q would result in over estimates.

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Date _____

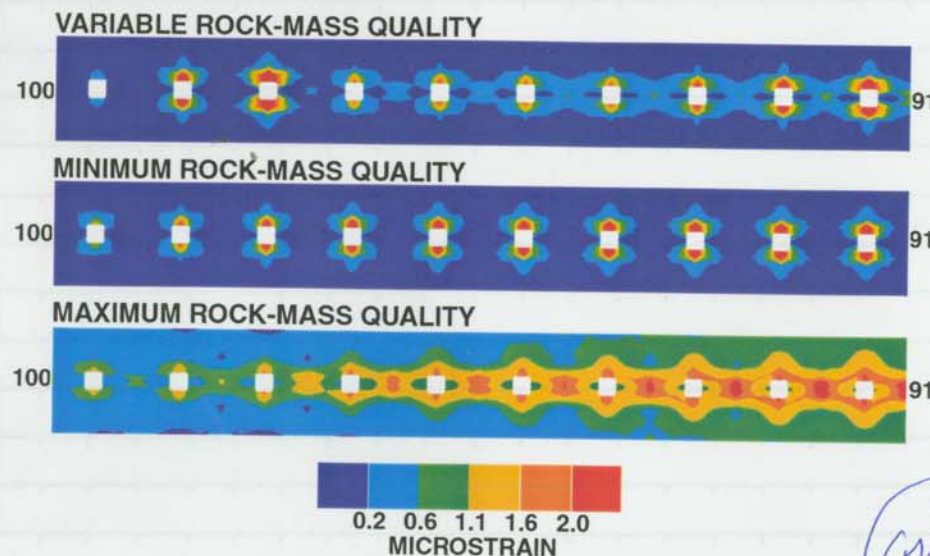
Invented by _____

Date _____

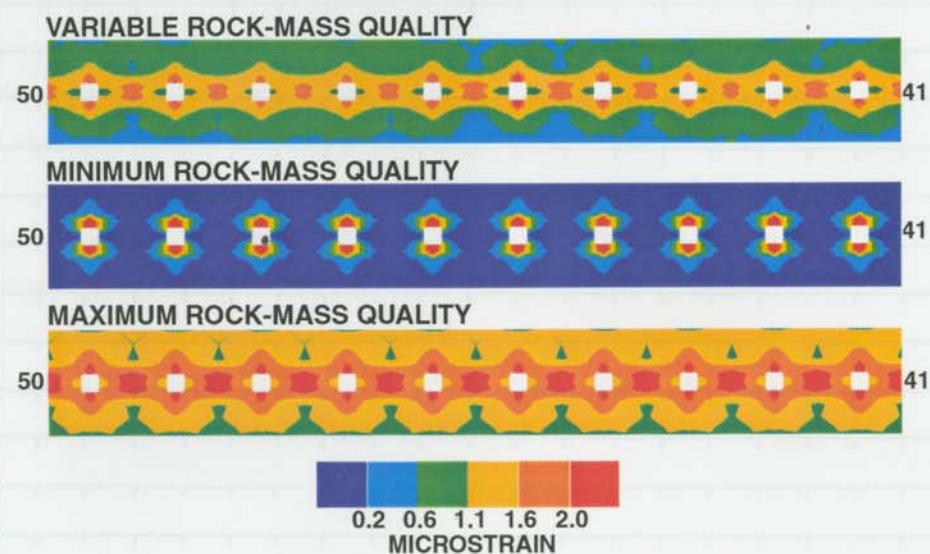
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INELASTIC-STRAIN DISTRIBUTIONS
WITHIN 15 m ABOVE AND BELOW DRIFTS #91-100
(DEGRADED DRIFT SUPPORT)



INELASTIC-STRAIN DISTRIBUTIONS
WITHIN 15 m ABOVE AND BELOW DRIFTS #41-50
(DEGRADED DRIFT SUPPORT)



- estimation of the intensity of fracturing in the inter-drift pillars.

Pages 70-76
enriched by GW 12/14/98

Comparison of results from Cases 1, 2, and 3 with degraded drift support.

(1) Use of minimum Q everywhere would cause underestimation of the extent of potential damage zones around in the roof and floor areas. The intensity of potential fracturing in inter-drift pillars is also likely to be underestimated in analyses based on a uniform minimum Q .

On the other hand, use of uniform maximum Q will likely lead to underestimation of the extent of damage in the roof and floor areas, and over-

Two drift-scale models are set up to examine the following:

- (1) Shape and size of damaged zone (zone of high plastic strain in the roof area of opening)
- (2) Distribution of contact pressure on drift lining
- (3) Distribution of the permeability-change ratio R_k

The drift-scale models will be synchronized with the repository-scale model in terms of (i) coordinates, (ii) spatial variation of mechanical properties, and (iii) Drift number. Results from the repository-scale model are used to set the boundary conditions for the drift-scale models.

Two drift-scale models are analyzed:

- ① Drift #48 of repository-scale model. Boundary x coordinates (middle of the bounding pillars) are.

$$X_{\min} = 28(100 - D_n) = 28 \times 52 \quad (D_n = 48) \\ = 1456 \text{ m}$$

$$X_{\max} = 1456 + 28 = 1484 \text{ m}$$

$$X_0 = X \text{ at drift axis} = \frac{1}{2}(1456 + 1484) = 1470 \text{ m}$$

- ② Drift #85 of repository-scale model

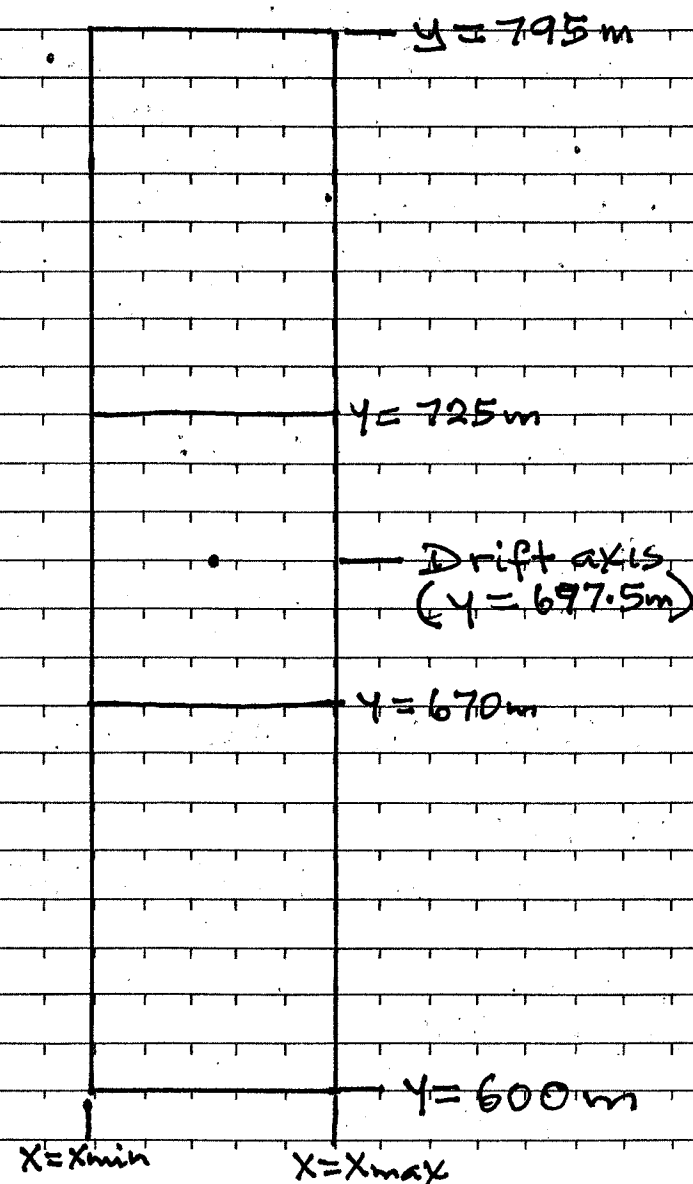
$$X_{\min} = 28(100 - 85) = 420 \text{ m}$$

$$X_{\max} = 420 + 28 = 448 \text{ m}$$

$$X_0 = \frac{1}{2}(420 + 448) = 434 \text{ m}$$

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Drift #48 is in the maximum Q area and #85 in the minimum Q area of the repository-scale model. Minimum Q actually occurs between drifts 98 and 99 (p. 70) but these drifts lie in the zone of significant lateral temperature gradient and relatively low temperature. Drift #85 was (cf. p. 70) was chosen to represent low- Q areas to avoid the effects of lateral temperature gradient and relatively low temperature on the calculated response.



This figure illustrates schematically the domain of each drift-scale model. The domain extends about 200m vertically, i.e. 100m above and below the drift axis. Boundary conditions at the exterior boundaries are set using temperature and x and y displacement histories calculated in the repository-scale model. Such boundary conditions are applied using the ABAQUS SUBMODEL facilities.

Each drift is modeled using as a circular section with external diameter of 5.4m, including a 0.2m thick concrete lining.

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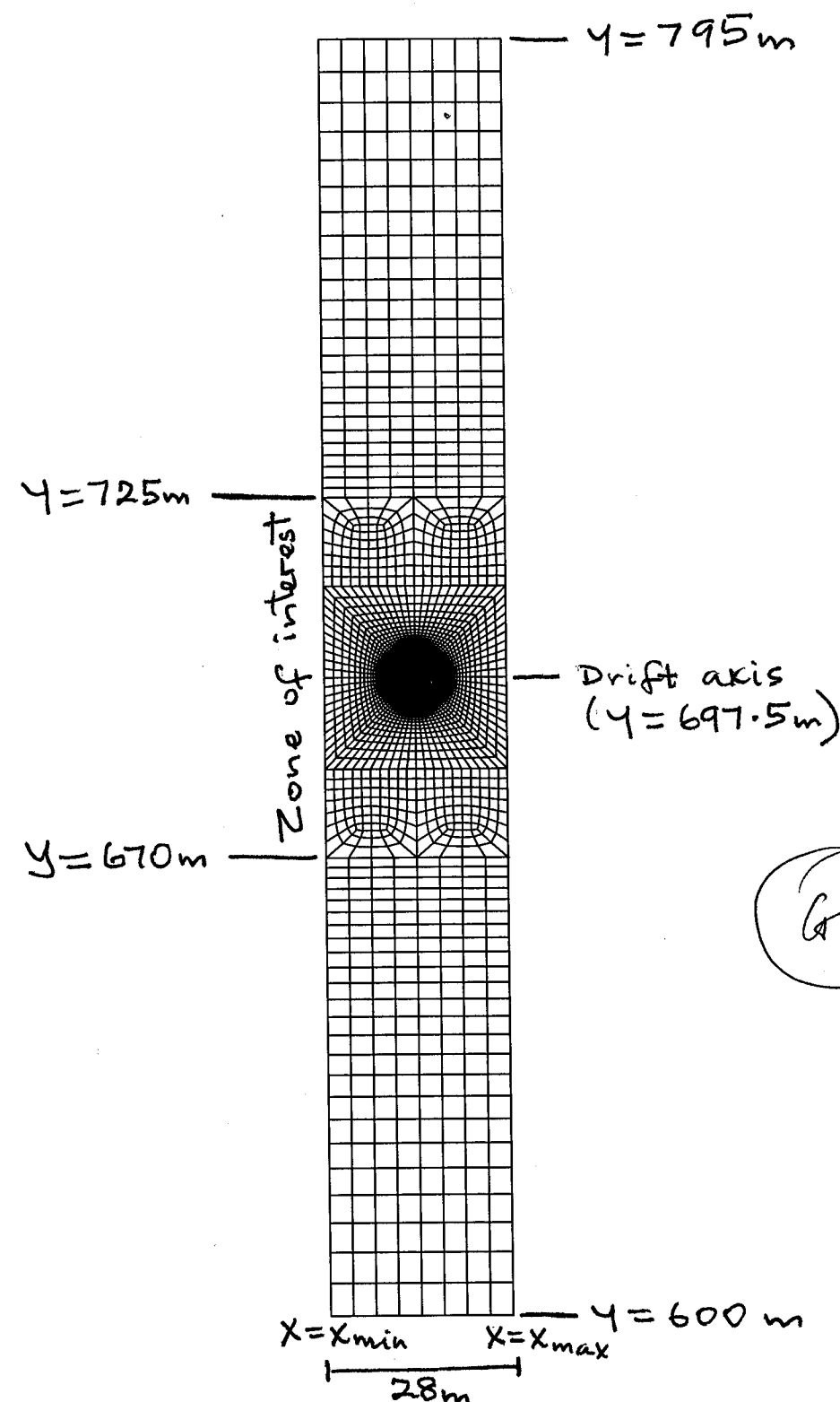
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Finite element mesh used for drift-scale models. See p. 80 for more details.

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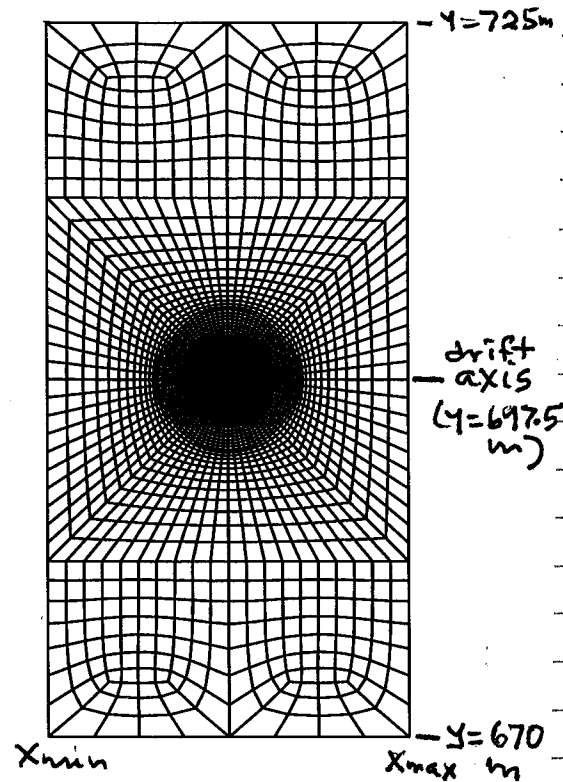
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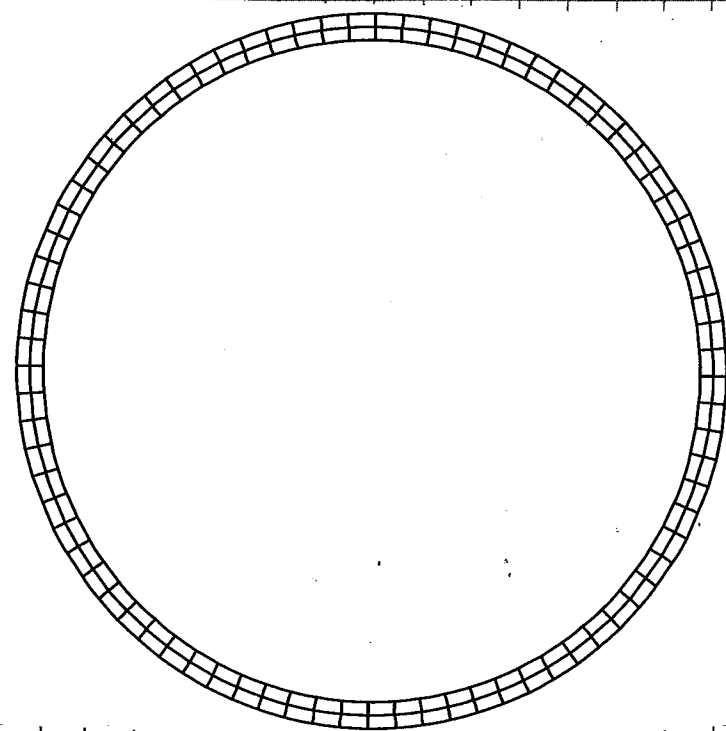
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Finite element mesh within the zone of interest, which extends vertically 27.5m above and below the drift axis. Materials within this zone, except the liner and tunnel section, are modeled as linear elastic-plastic using the Drucker-Prager strength criterion.



Finite element mesh used to model liner. The liner has internal and external diameters of 5.0 and 5.4m, respectively. Pressure distribution at the exterior surface of the liner is monitored as part of the analysis.

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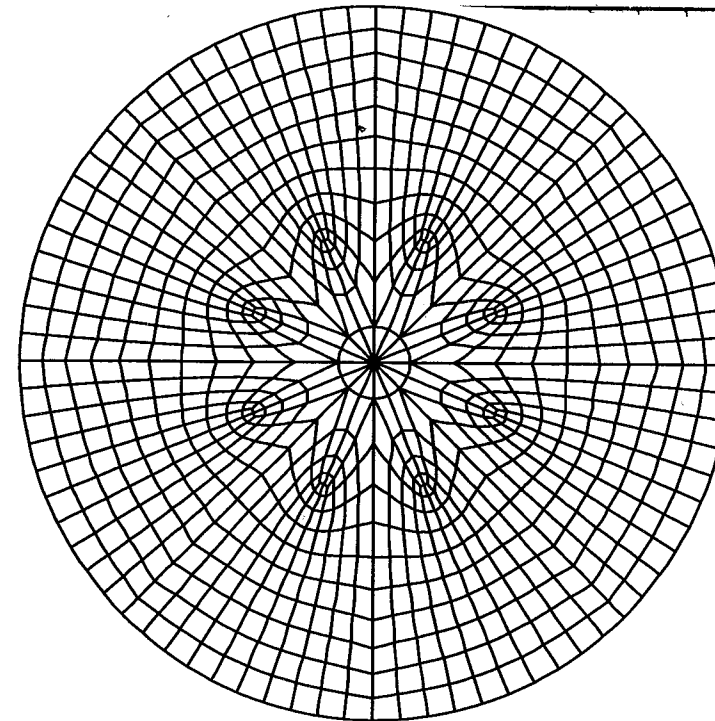
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Finite element mesh of the tunnel section. Material within this zone is treated as a uniform volumetric heat source in the thermal analysis. In the mechanical analysis, the tunnel-section elements are removed to

simulate excavation. The interior surface of the liner coincides with the exterior surface of the tunnel section.

Material Properties

Zones: (1) Elastic-plastic zone (see p. 80).

(2) Elastic zone: includes the tunnel section, areas above ^{GW 2/15/99} the upper elastic-plastic zone, and areas below the lower elastic-plastic zone (i.e., areas above $y=725m$ and areas below $y=670m$ on p. 78).

(3) Liner: modeled as linear elastic with concrete properties.

Rock mass thermal and mechanical properties are assigned the same values (some as functions of X) described on p. 18-23. The properties of concrete liner are assigned from literature as follows:

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Properties of Concrete

Density: 2100 kg/m^3 [p. 15 of reference (1) cited on p. 2].

Thermal conductivity: 1.37 J/(s.m.K)
 $= 4.32 \times 10^7 \text{ J/(yr.m.K)}$

[p. 15 of same reference]

Specific heat capacity = 880 J/(kg.K)

[p. 15 of same reference]

Young's modulus: $2.76 \times 10^4 \text{ MPa}$

Poisson's ratio: 0.25 — changed to 0.21

→ [p. 8 of reference (3) cited on p. 15]

Thermal expansivity: $9.9 \times 10^{-6}/\text{K}$

[p. 8 of same reference].

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Input Files

① Repository-scale model to m06 (same as m01 described on p. 61) is used to generate histories of x and y displacements required to set drift-scale model boundary conditions.

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Input files for model m06: (Global model)

m06.inp

allNodes.def

mElem01.def and auxiliary files listed on p. 61

d1GmNodes.def defines driver node set for model d1

d2GmNodes.def defines driver node set for d2

② Drift-Scale Models (Submodels).Model d1 — Drift #48

2.1 d1t.inp Thermal analysis input files

2.2 d1Nodes.def Node definitions.

2.3 nodeSets.def Node set definitions.

2.4 drivenNodes.def Node sets that get temperature and displacement histories from model m06.

2.5 tElms.def Element connectivity definitions for thermal model

2.6 elemSets.def Element set definitions

2.7 contacts.def Defines liner-to-rock contact sets.

2.8 tProps.def Thermal property definitions

2.9 iniTemp.def Initial temperature definitions

2.10 CDriftSrc.def Heat source definition

2.11 mElms.def Element connectivity for mechanical model

2.12 mProps.def Mechanical property definitions

2.13 ndxFld.def Defines a field variable assigned value of x-coordinate at each node.

2.14 d1m.inp Mechanical analysis input files

2.15 d1EMat.def Young's modulus and Poisson's ratio

2.16 d1Friction.def Drucker-Prager friction properties

2.17 d1Cohesion.def Drucker-Prager cohesion properties

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Model 2 d2 Drift # 85

2.18	d2t.inp	Thermal analysis input file
2.19	d2Nodes.def	Node definitions:
2.20	d2ndxFld.def	Defines a field variable assigned value of x-word at every node.
2.21	d2m.inp	Mechanical analysis input
2.22	d2Emat.def	Young's modulus & Poisson's ratio
2.23	d2friction.def	Drucker-Prager friction properties
2.24	d2Cohesion.def	Drucker-Prager cohesion properties
2.25	d2mprops.def	Mechanical properties definitions

The following input files were listed for model d2 one also used by model d2.

2.3 thru 2.11

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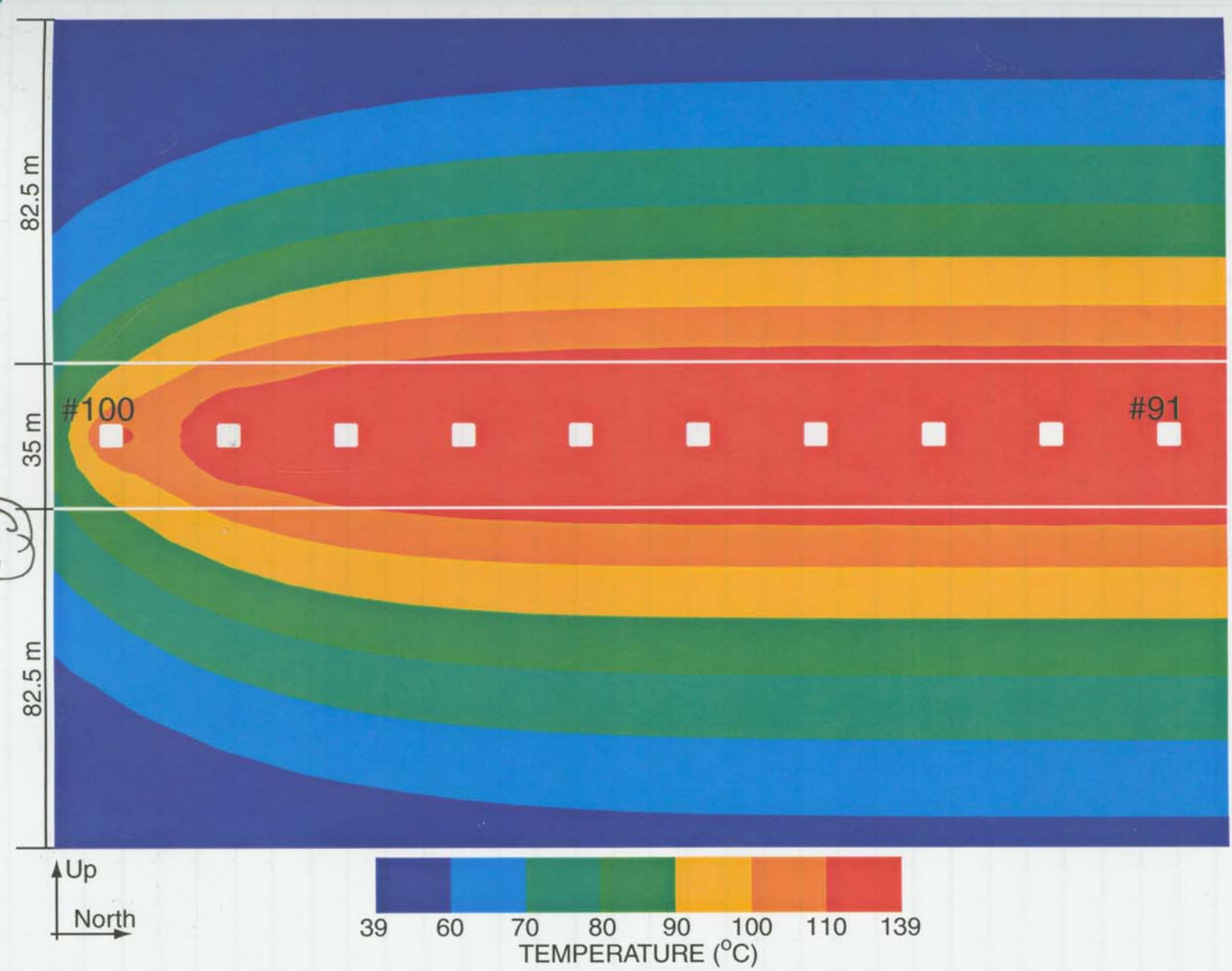
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Project No. 85-95
Book No. 85-95

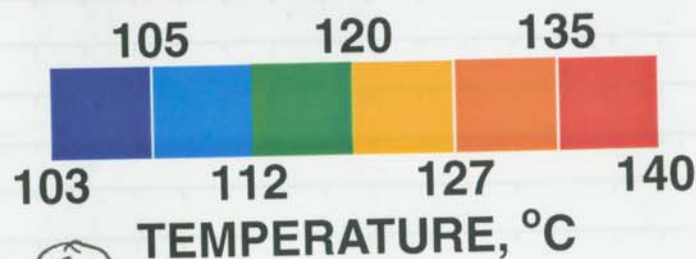
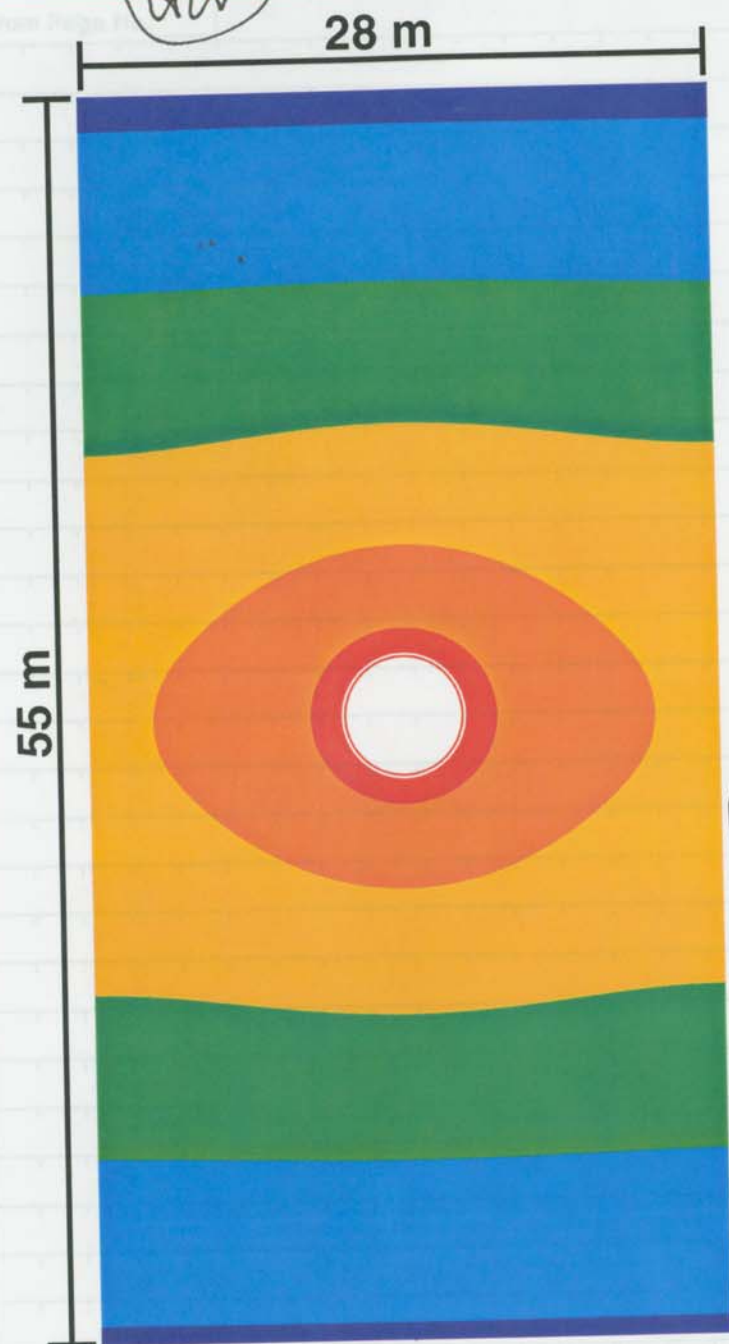
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Temperature distribution at 150 yr, over a 200-m high section of the repository-scale model from Drift #100 through Drift #91.

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TEMPERATURE, °C

Temperature distribution at 150 yr within the 55-m high zone of interest as in the drift-scale models. Temperatures range from about 103°C to about 140°C. The 55-m high zone centered on the drift lies within the two inner contour zones of the figure on p. 86, in which temperatures also range from about 100°C to about 140°C.

GW The two sets of results (p. 86 and this page) indicate consistency between the repository-scale and drift-scale models.

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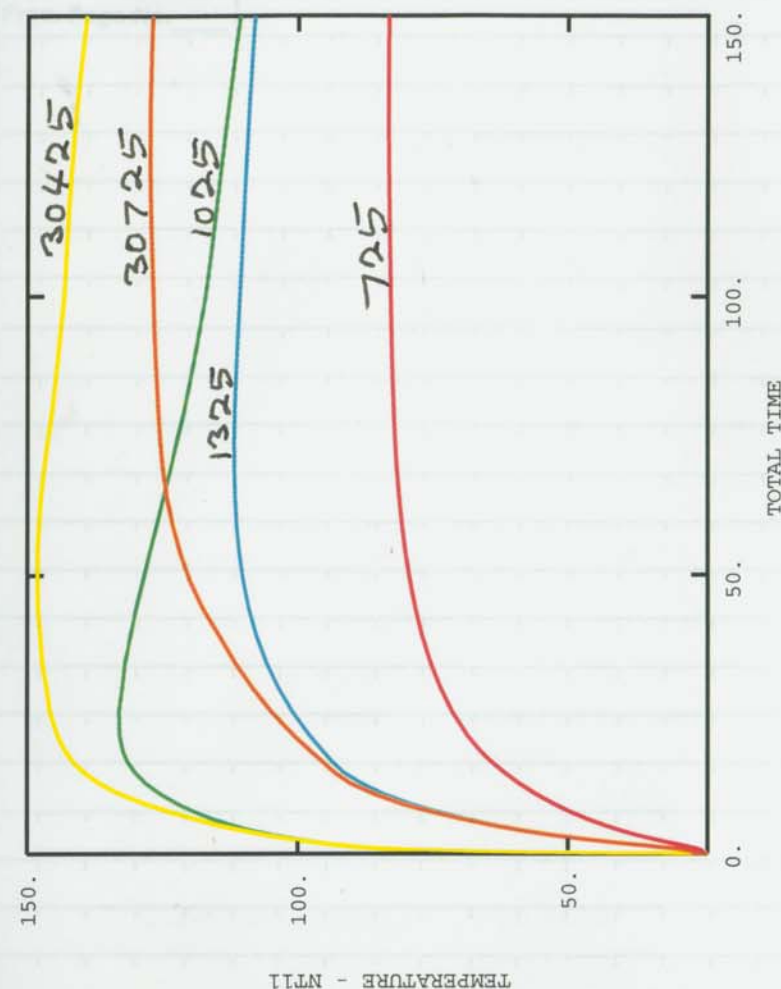
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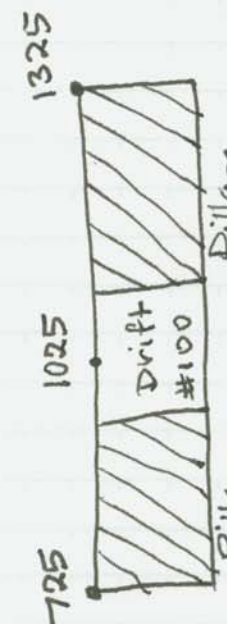
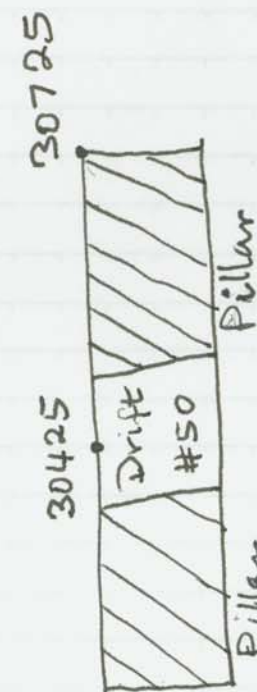
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XMIN 0.000E+00
XMAX 1.500E+02
YMIN 2.440E+01
YMAX 1.482E+02



Temperature histories at 5 nodes from the repository-scale model. The locations of the nodes relative to drifts #100 and #50 are illustrated. Drift #100 is an end drift, whereas #50 is an interior drift.

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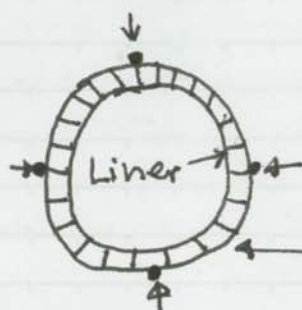
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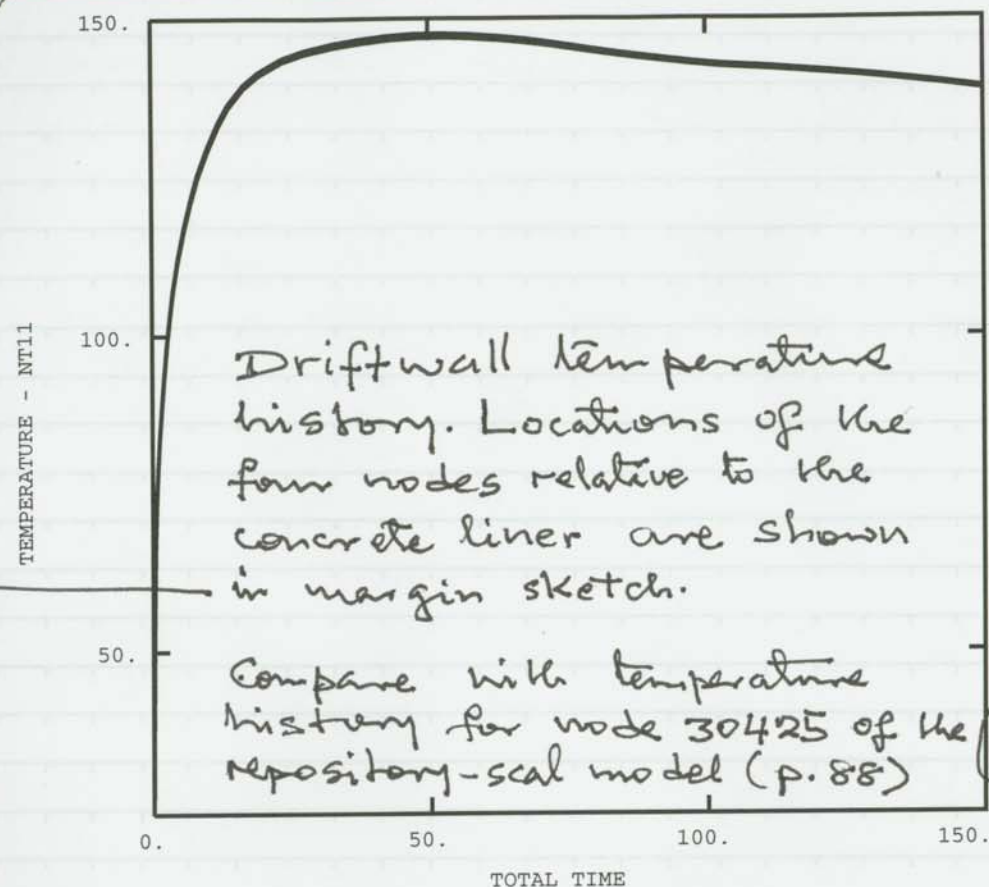
Date _____

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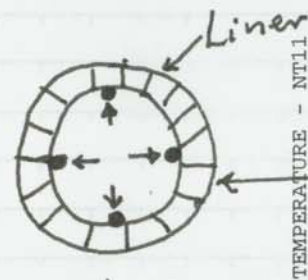
— TD_13926
— TD_14031
— TD_14201
— TD_14371



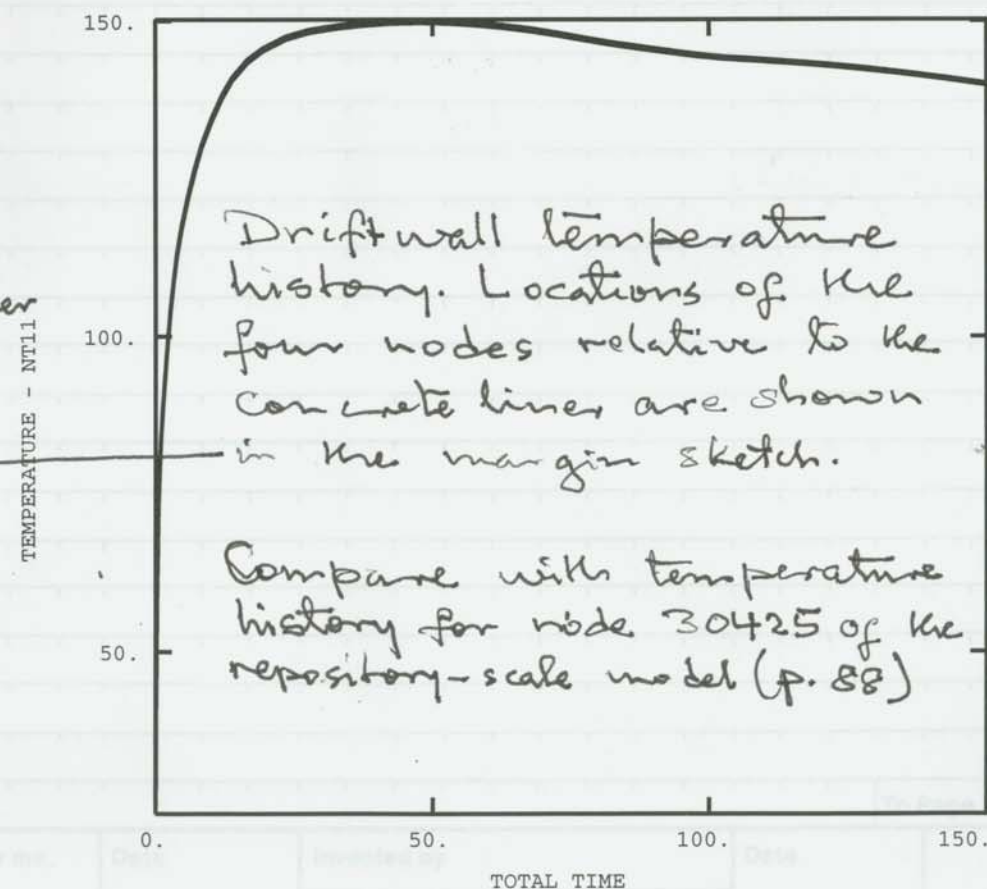
XMIN 2.000E-06
XMAX 1.500E+02
YMIN 2.440E+01
YMAX 1.476E+02



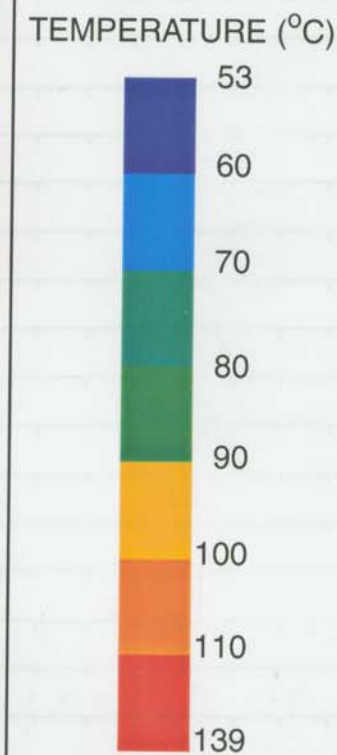
— TL_10727
— TL_11104
— TL_11818
— TL_12532



XMIN 2.000E-06
XMAX 1.500E+02
YMIN 2.440E+01
YMAX 1.502E+02



Recorded by



Temperature distribution at 150 yr over the full drift-scale model. The distribution is plotted following the same format used for the repository-scale model distribution shown on p. 85. Both plots cover a vertical interval of about 200 m centered on the drift axis. The temperature distribution from the drift-scale model should be closely similar to the repository-scale distribution ~~for~~ over a vertical (28-m wide) ship through one of the interior drifts, such as Drift #91 (p. 85). The two distributions appear to be identical. The only difference is that the lowest temperature is 53°C in the drift-scale model and 39°C in the repository-scale model. The lower minimum temperature in repository-scale model occurs near the top-south end of the model and is due to end cooling that is not accounted for in the drift-scale model.

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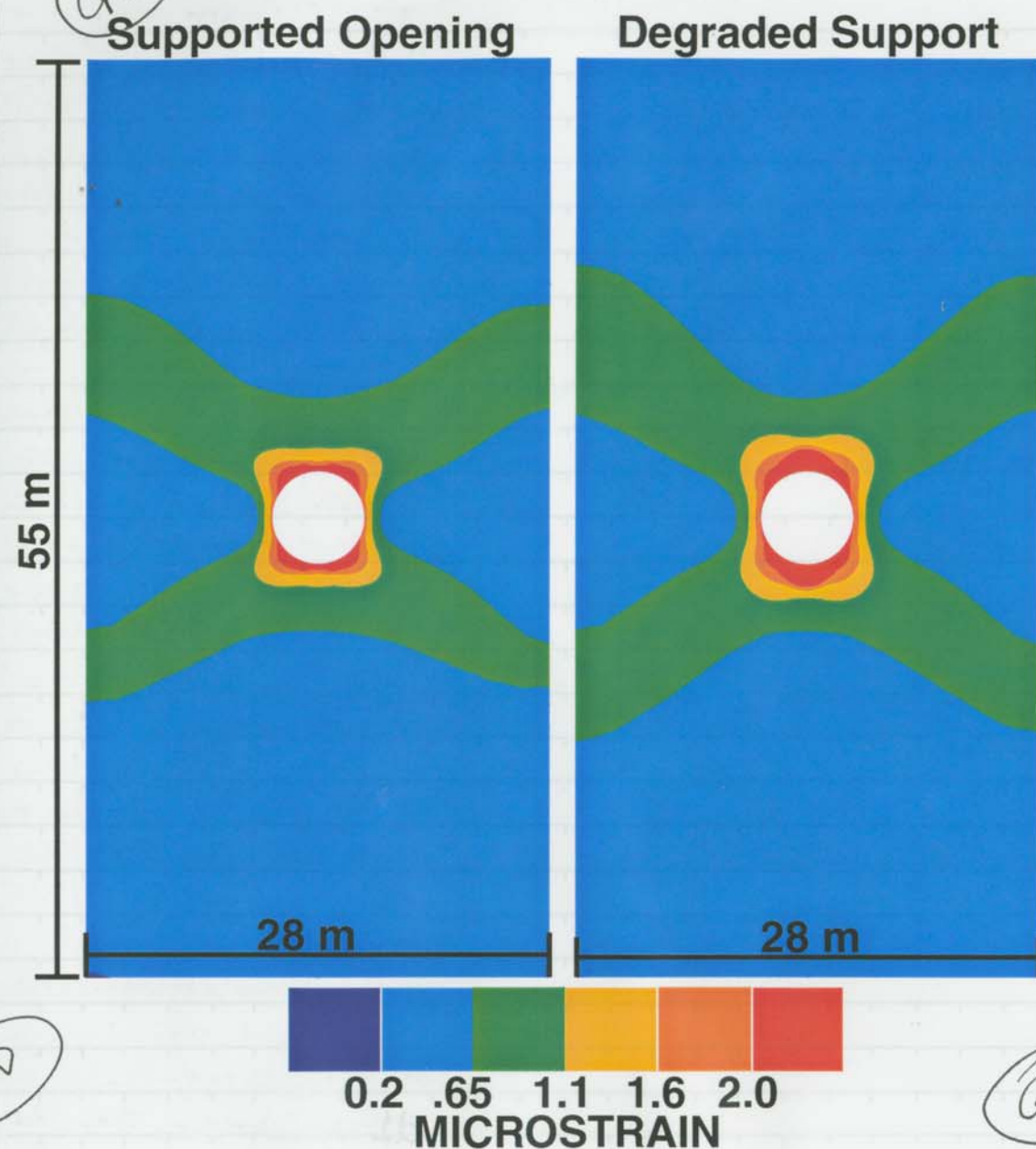
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Inelastic strain (Γ^N) distributions from
d1 (this page) and d2 (next page) models.

- (1) Magnitudes of Γ^N in the pillars are higher in model d1 (highest α area) than in model d2 (lowest α area).
- (2) Magnitudes of Γ^N close to the opening are higher in model d2 (lowest α area) than in model d1 (highest α area).

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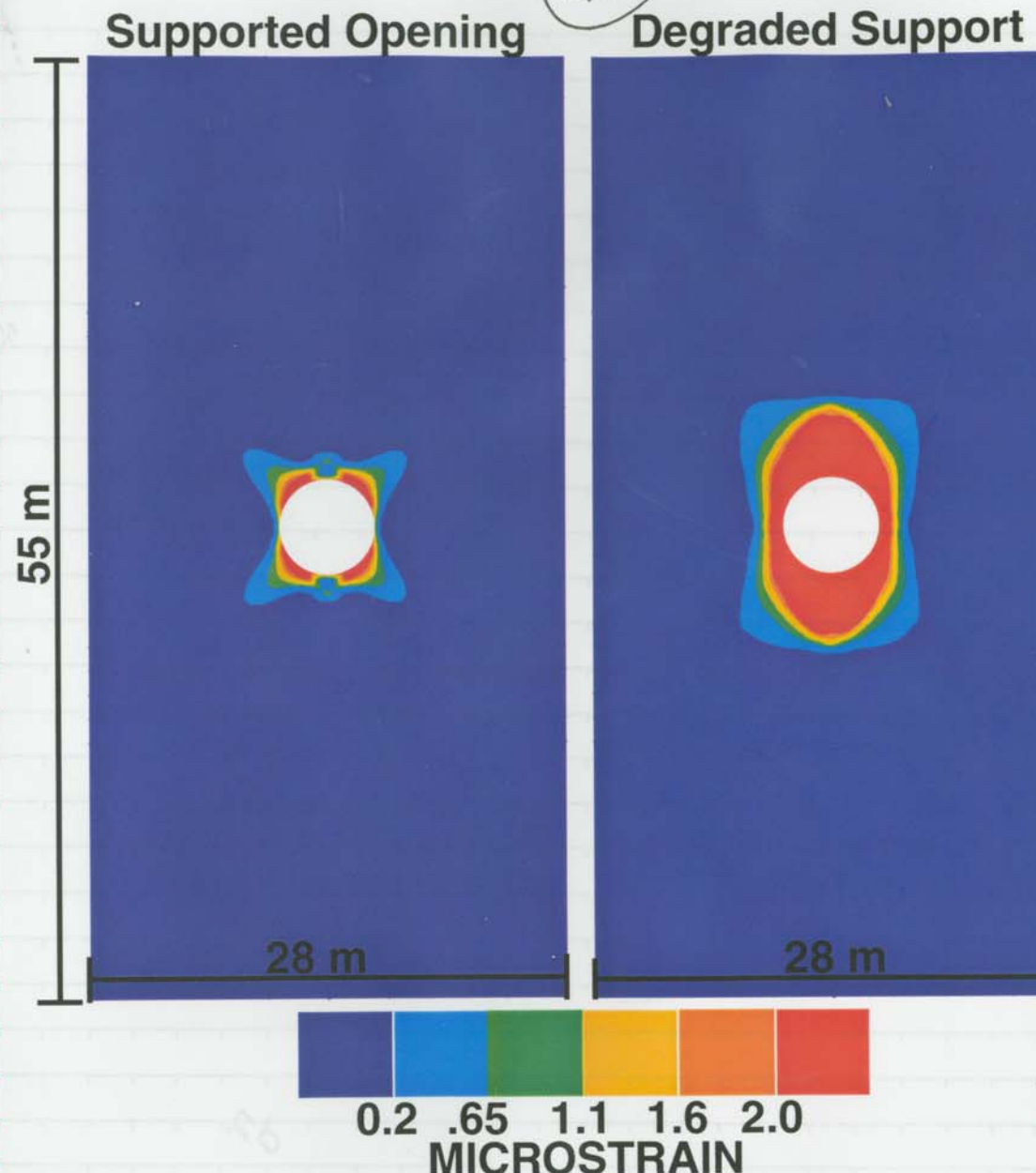
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- (3) Support degradation causes increased magnitudes of Γ^N everywhere, but much more in lowest- α areas than in highest- α areas.
- (4) The high Γ^N values close to the opening in lowest- α areas may indicate collapse of the opening in such areas.
- (5) The stiffness of the concrete lining, which is greater than rock stiffness in lowest- α areas (model d2) but smaller in highest- α areas (model d1) may have an effect on the Γ^N distribution.

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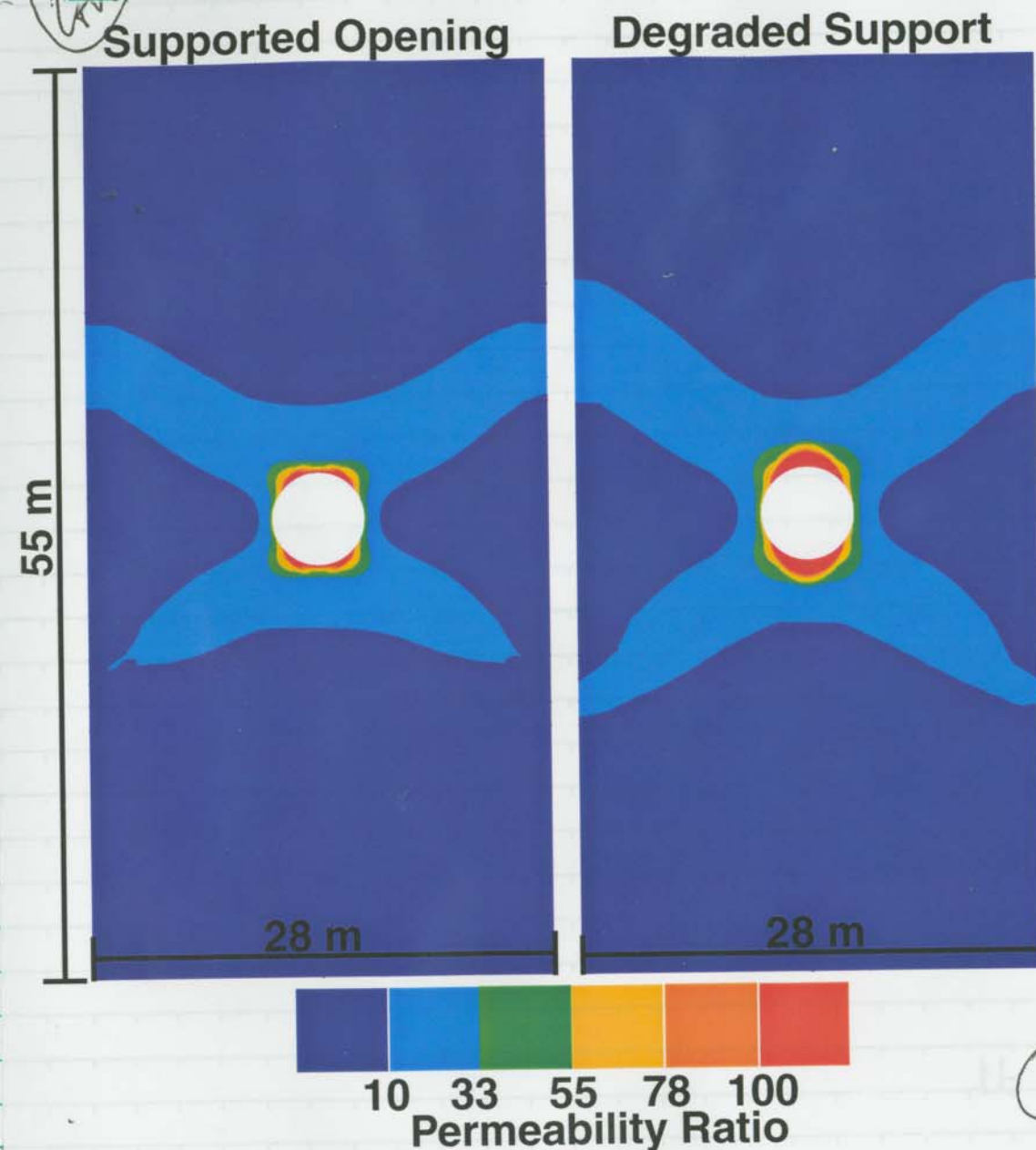
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from D1 Model



Distributions of permeability change ratio R_k (see p. 53) from model d1 (this page) and model d2 (next page).

- (1) R_k distributions are similar to the corresponding distributions of Γ^N .
- (2) R_k values in lowest- Q areas (model d2) are concentrated in the roof and floor areas, whereas changes permeability changes extend more

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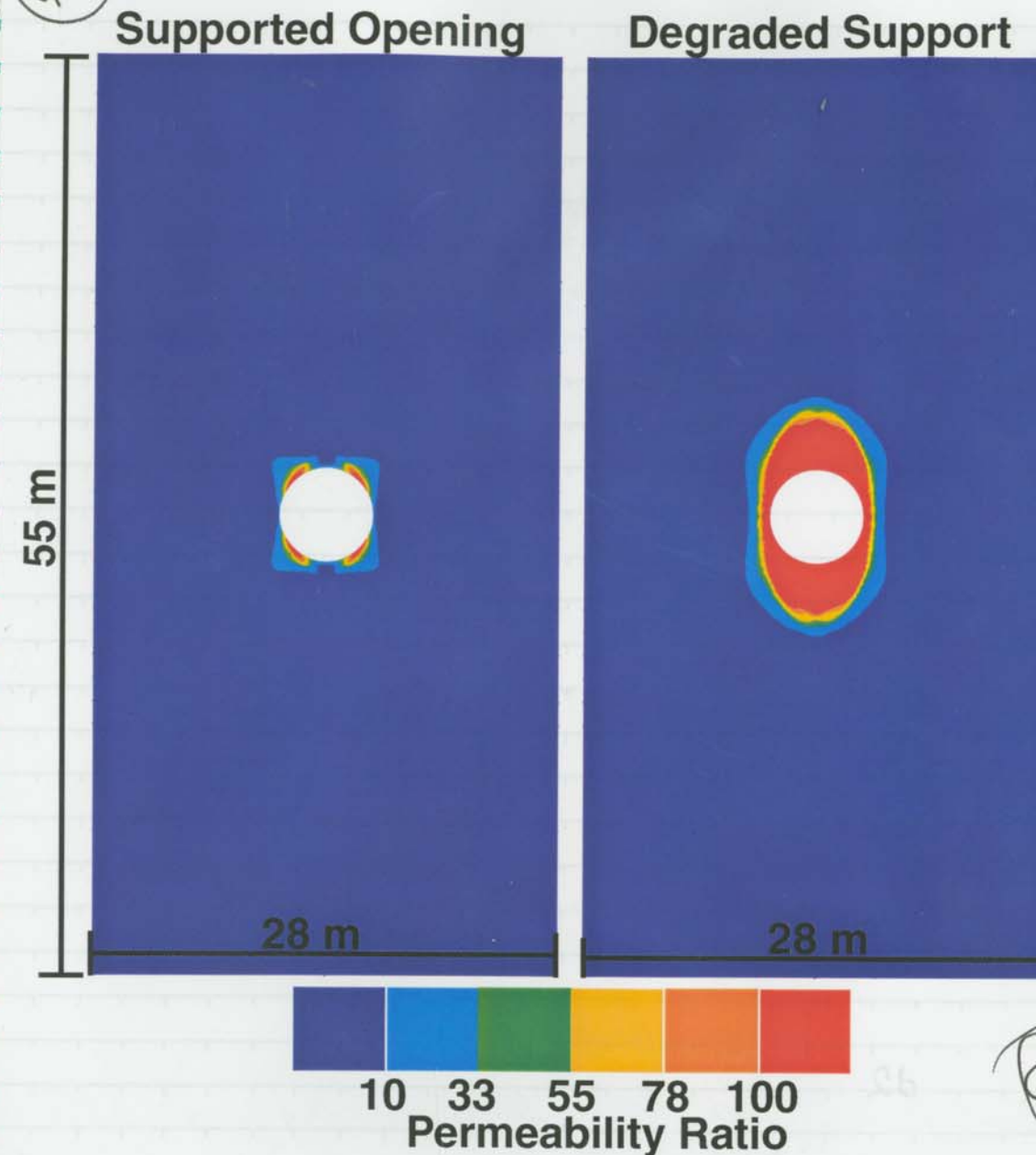
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from D2 Model



into the pillars in highest- Q areas.

- (3) The zones of relatively high R_k values near the roof and floor may not be significant because the rock within such zones may have collapsed into the openings.
- (4) Support degradation has more severe effects on R_k distributions in the lowest- Q areas than in the highest- Q areas.

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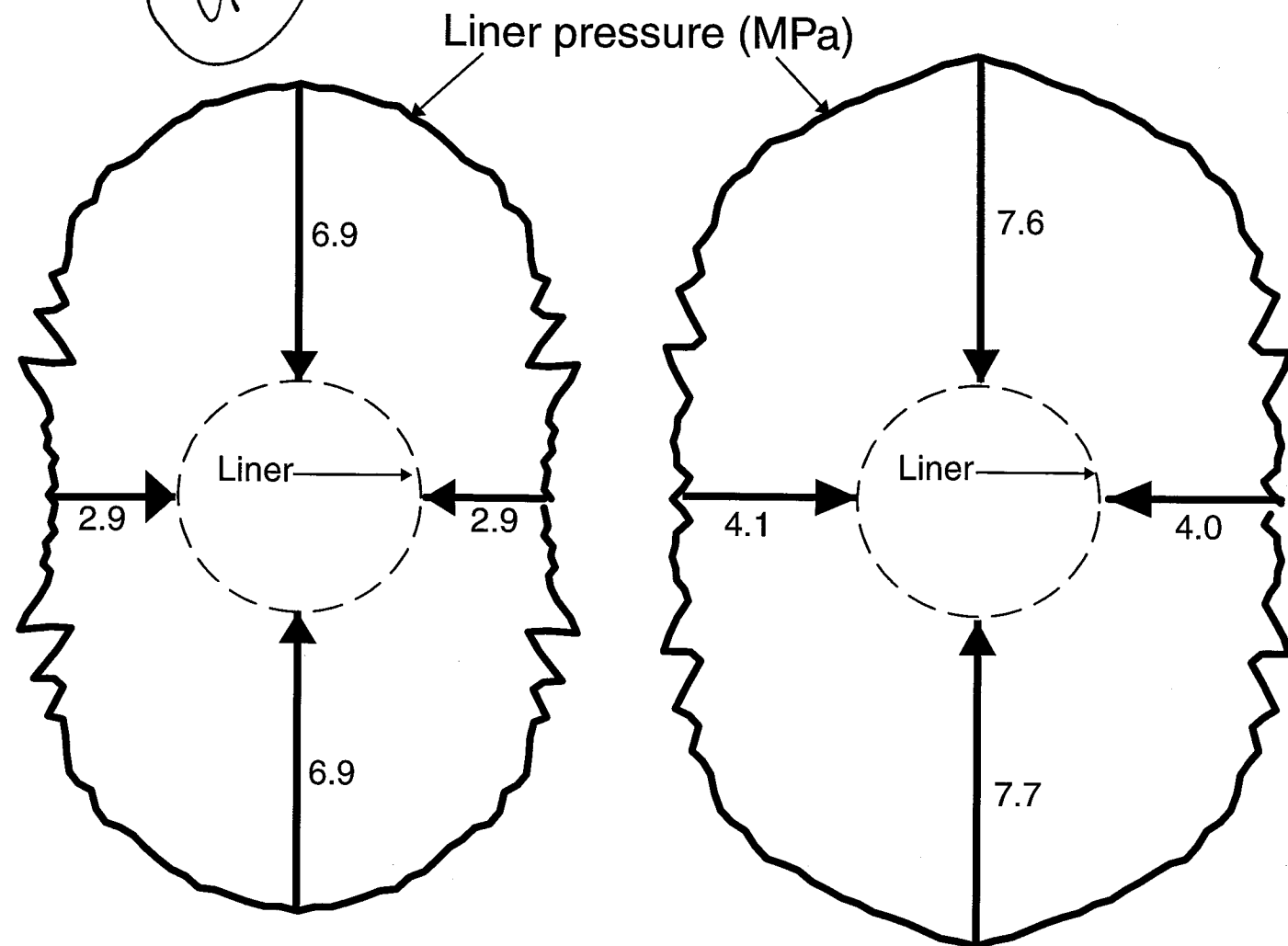
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Highest Rock-Mass Quality

Lowest Rock-Mass Quality

Distributions of liner pressure at 150 yr from models d1 (highest-Q area) and model d2 (lowest-Q area). Values of liner pressure are plotted as proportional radial distance from the liner on this page and as a function of angular distance on the next page.

- (1) Liner pressure is generally higher in the lowest-Q area than in the highest-Q area.
- (2) Liner pressure is greater in the roof and

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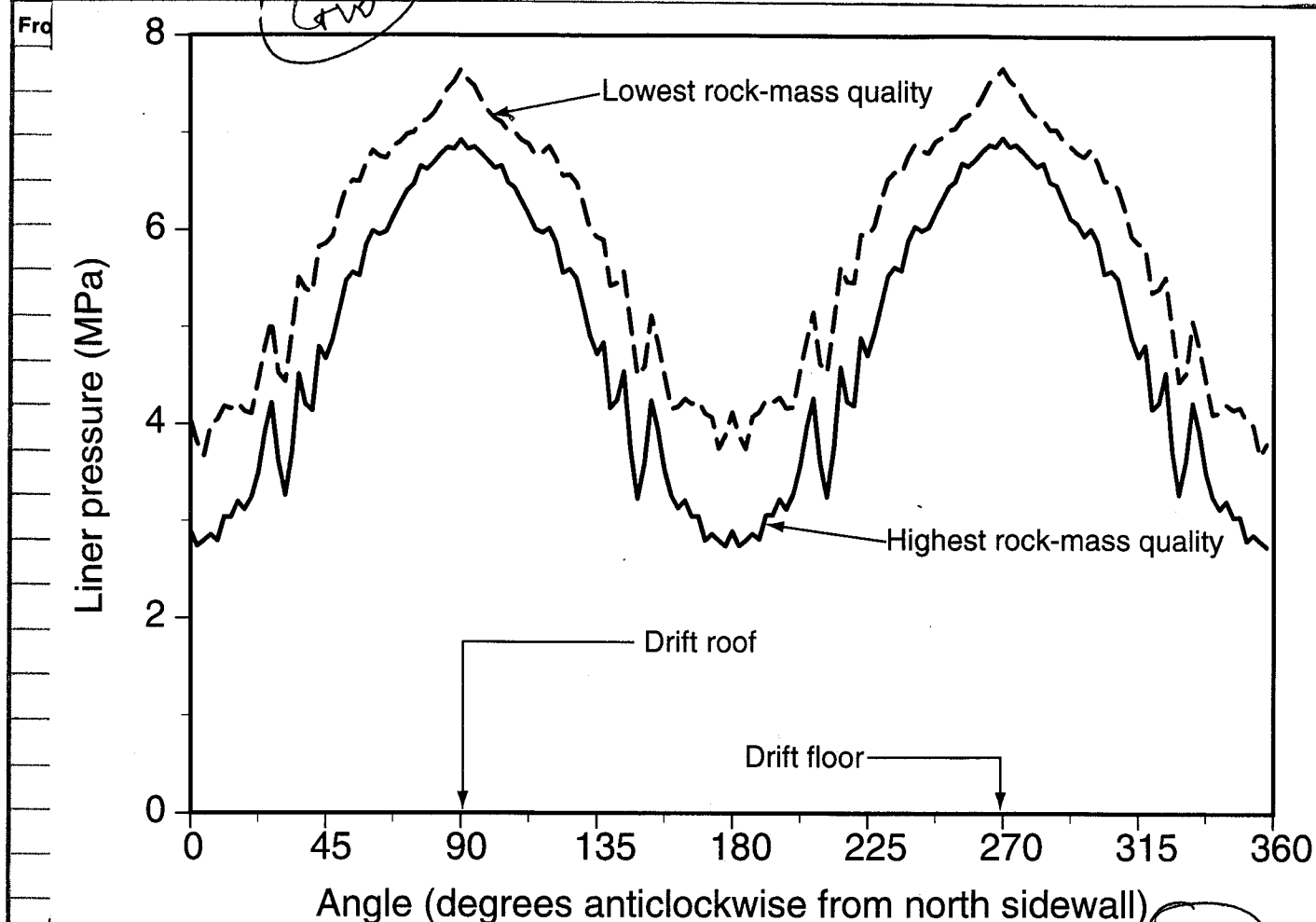
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floor of the liner than in the sidewalls. So potential liners may be subjected to nonhydrostatic loading.

- (3) The liner pressures plotted here may have been influenced by the frictional properties applied to the rock-liner interface in the model. The liner was modeled as fully bonded to the rock in both model d1 and model d2.
- (4) Possible effects of the applied rock-liner interface model will be investigated further.

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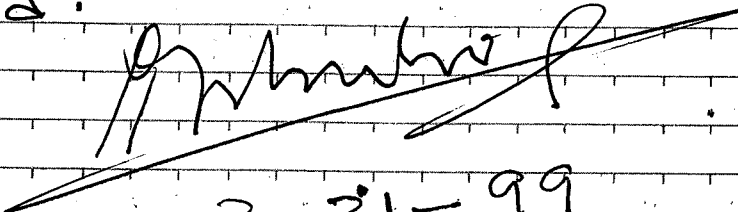
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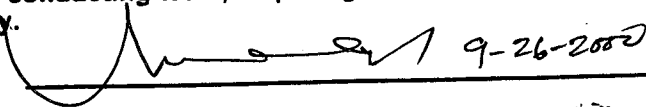
Notes on this project are
continued in CNWRA Scientific
Notebook Number 321

This notebook (No. 263) is now
closed.


3-31-99

Element Managers are requested to put the following statement at the conclusion of "manual" Scientific Notebooks:

"I have reviewed this scientific notebook and find it in compliance with QAP-001. There is sufficient information regarding procedures used for conducting tests, acquiring and analyzing data so that another qualified individual could repeat the activity."

 9-26-2000
(Element Manager signature and date above line, MGFE
Name of Element beneath line)"

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