


Return to Engineering Standard [Home Page](#), [Index](#)

SRS Engineering Standards Manual	Manual: WSRC-TM-95-1
	ENGINEERING STANDARD
Structural Design Criteria	STANDARD NO: 01060 DATE: 9/99 REVISION: 4 ESB TECH COMMITTEE: Civil/Structural

REVISION HISTORY

REV	DATE	DESCRIPTION OF REVISION
0	8/1/95	INITIAL ISSUE
1	10/1/96	Included DOE Order 420.1, the SBC, requirements for concrete anchors and chipping, deleted unused acronyms and references, and editorials.
2	07/28/97	Included site specific design spectra for PC3 and PC4, UBC 1997 seismic loads and loading combinations and editorials. Revised basic wind speed from "fastest mile" to "three seconds gust".
3	07/98	Added UBC seismic ductility provisions and earthquake load factor of 1.2 for new PC-3 and PC-4 structures, increase PC-3 design basis tornado speed to 178 mph, added automobile in the PC-3 tornado missile criteria, took out dates of reference National Codes and Standards, and added notes to some loading combinations.
4	09/99	Added reference criteria for stacks, removed the SBC, added dynamic settlement load factor of 1.2 for new PC-3 and PC-4 structures, added correlation between Functional Classification and Performance Categories, revised PC1 and PC2 design wind speeds, and revised PC-3 design basis spectra.

UNCLASSIFIED

1.0 PURPOSE AND SCOPE

1.1 This document provides minimum structural design criteria for all new facilities and modifications to existing facilities, both permanent and temporary, at the Savannah River Site (SRS).

1.2 The code year is established by the issue date of the project criteria documents. For modifications and additions to existing facilities, the Code of Record shall be established by the Design Authority. DOE Order 420.1 (Ref. 6.1.3) requires validation of existing designs, including designs with natural phenomena hazards assessment older than 10 years, against the current DOE natural phenomena hazard design criteria.

2.0 DOE ORDER AND STANDARDS APPLICABILITY

2.1 DOE Orders as included in WSRC S/RID and DOE and Standards applicable for the evaluation, modification or addition to SRS facilities are listed in Section 6.1.

2.2 Conflict between the DOE Orders as included in WSRC S/RID and DOE Standards, National Codes and Standards, and this Engineering Standard shall be brought to the attention of Engineering Standards Board's Civil/Structural Committee Chairman for resolution.

3.0 NATIONAL CODES AND STANDARDS APPLICABILITY

3.1 National Codes and Standards incorporated by reference in this document shall be the revision number and date at the time this document is invoked in the Design Output Documents, or as otherwise noted.

3.2 The Design Output Document shall specify the applicable Codes and Standards utilized and their associated revision and date.

3.3 The applicability of the codes and standards given in this section is limited to the extent of the references in the text.

4.0 ACRONYMS & DEFINITIONS

4.0 DEFINITIONS

4.1 Acronyms

The following is a list of acronyms and shortened titles used for the reference documents in this Engineering Standard. Load related symbols and factors are defined in Sections 5.2 and 5.3.

ACI	American Concrete Institute
AASHTO	American Association of State Highway and Transportation Officials
ANSI	American National Standards Institute
APC	Atmospheric Pressure Change
API	American Petroleum Institute
AREMA	American Railway Engineering and Maintenance-of-Way Association
ASCE	American Society of Civil Engineers
ASD	Allowable Stress Design
AWWA	American Water Works Association
CFR	Code of Federal Regulation
CMAA	Crane Manufacturers Association of America
DBFL	Design Basis Flood
DOE	Department of Energy
DOE-STD	Department of Energy Standard
MBMA	Metal Building Manufacturers Association
NPH	Natural Phenomena Hazard
OSHA	Occupational Safety and Health Administration
PC	Performance Category
PGA	Peak Ground Acceleration
SCDHEC	South Carolina Department of Health and Environmental Control
SD	Strength Design
SDI	Steel Deck Institute
SGS	Site Geotechnical Services
S/RID	Standards/Requirements Identification Document
SRS	Savannah River Site
SSC	Structures, Systems, and Components
SQP	Structural Qualification Program for Savannah River Site Facilities
UBC	Uniform Building Code
WSRC	Westinghouse Savannah River Company

4.2 Definitions

Key words and terms used in this document are defined as follows:

4.2.1 Natural Phenomena Hazard (NPH) - An act of nature (e.g., earthquake, wind, hurricane, tornado, flood, rain or snow precipitation, volcanic eruption, lightning strike, or extreme cold or heat) which threatens workers, the public, or the environment by potential damage to structures, systems, and components.

4.2.2 Performance Category (PC) - A classification based on a graded approach which is used to establish the NPH design and evaluation requirements for SSCs in accordance with DOE Standard 1021 (Ref. 6.1.2). Table 7.1.4 provides the methodology for assigning Performance categories for all SSCs.

4.2.3 SQP - Structural Qualification Program for Savannah River Site Facilities, Revision 3, 1995 - The SQP provides

requirements to assess the structural design basis of SRS pre-operational and operating facilities to current DOE Orders and Standards, industry consensus codes and SRS standards and guidelines. DOE 420.1(Ref. 6.1.3) requires the contractor/operator to establish an implementation plan for evaluating and upgrading existing SSC for NPH mitigation. This program provides a plan for backfit evaluation for each facility, which will satisfy the requirements in DOE 420.1, Section 4.4.3. The methodologies used in this program are for the evaluation and upgrade of existing facilities.

5.0 REQUIREMENTS

5.1 Performance Categories

There are five Performance Categories, PC-0 through PC-4, associated with the SSC at the SRS (DOE Guide 420.1-Y, Ref. 6.1.4, and DOE-STD-1021, Ref. 6.12). The Performance Categories are based on the performance goals associated with these SSC and are expressed in terms of the annual probability of exceedance of the design basis natural phenomena hazards (See Table 7.1.1).

Interaction effects shall be considered in accordance with DOE-STD-1020 and DOE-STD-1021. See Ref. 6.2.14.7 for guidance.

5.2 Design Loads

SSC shall be designed for the loads prescribed in this standard and as supplemented by project specific criteria.

For additions or modifications to existing facilities, the effects of new loads transmitted to the existing SSC shall be considered. Modifications and additions shall not degrade the performance of existing SSC to the extent that they will not withstand NPH loads, provide confinement, or provide safe operation of essential facilities, protection of government property and the protection of life safety for occupants.

5.2.1 Dead Load (D)

Dead loads are loads that remain permanently in place.

5.2.2 Live Load (L & L_r)

5.2.2.1 Live loads (L) are those loads produced by the use and occupancy of a building or other structure. Live loads on a roof (L_r) are those produced (1) during maintenance by workers, equipment, and materials and (2) during the life of the structure by movable objects such as temporary equipment. Also considered as live loads are the dynamic effects of operating equipment (such as cranes and pumps).

5.2.2.2 Live loads on roofs shall be as stipulated in ASCE 7, (Ref. 6.2.6.2), AASHTO HB, (Ref. 6.2.1.1) or AREMA Manual (Ref. 6.2.5.1) as applicable.

5.2.2.3 For modifications or analysis of existing structures, loading based on the actual use of the structure may be used. Loads identified as live loads in the original design, when known, may be counted as dead loads for the evaluation of existing structures.

5.2.2.4 For yard structures exposed to wheel traffic, HS 20-44 (Ref. 6.2.1.1) truck loading shall be used, as a minimum, for wheel loading design. In areas subject to yard cranes, the crane wheel, track loading

or outrigger loads during lifts shall be considered.

5.2.3 Soil Load (H)

Structures and elements of structures retaining soil shall be designed for the lateral earth pressure and any surcharge plus any hydrostatic pressure corresponding to the maximum probable groundwater level.

5.2.4 Fluid Load (F)

The fluid load is the load resulting from the pressure of the fluid.

5.2.5 Rain Load (R)

Rain loads shall be obtained in accordance with ASCE 7. The minimum design shall be for a 25-year, 6 hour rainfall event at all performance categories. The curves provided in Engineering Standard 01110 (Ref. 6.2.14.2) shall be used to quantify the rainfall event.

5.2.6 Snow Load (S)

Snow loads shall be obtained in accordance with ASCE 7. Development of SRS specific snow hazard curves is not planned. The importance factor shall be 1.0 for PC-1 and PC-2 structures, and 1.2 for PC-3 and PC-4 structures.

5.2.7 Wind Load (W)

Wind load design for buildings and other structures shall be determined in accordance with the procedures in ASCE 7, using exposure "C", with the basic three-second gust wind speeds given in Table 7.1.1. The effect of the importance factor "I" is incorporated in the tabulated values for the basic wind speeds; "I" shall be taken as 1.0. The design wind speeds are derived from a DOE Memorandum (Ref. 6.1.5). For Probabilistic analysis wind hazard curve for SRS in three-second-gust speeds is available in Ref. 6.2.14.8.

For PC-3 and PC-4 structures, the design shall include consideration of wind driven missiles in accordance with DOE-STD-1020 (Ref. 6.1.1). The wind-driven missiles and the barrier thicknesses to preclude damage are provided in DOE-STD-1020. Barriers other than those given in DOE-STD-1020 may be analyzed or designed per the missile barrier criteria given in ASCE-58 (Ref. 6.2.6.3).

5.2.8 Tornado Load (W_t)

Tornado wind load design for buildings and other structures shall be also determined in accordance with the procedures in ASCE 7, using exposure "C", with the basic wind speed obtained from Table 7.1.1. The importance factor "I" shall be taken as 1.0. Tornado hazard curve for SRS in three-second -gust speeds is available in Ref. 6.2.14.10.

Tornado -driven missile criteria are given in Table 7.1.1. They are derived from requirements in DOE-STD-1020 and from recommendations in Ref. 6.2.14.10. The barrier thicknesses to preclude damage from tornado missiles are provided in DOE-STD-1020. Barriers other than those given in DOE-STD-1020 may be analyzed or designed per the missile barrier criteria given in ASCE-58.

APC shall apply for enclosed structures per provisions in DOE-STD-1020. Partially enclosed or open structures shall follow provisions in ASCE 7.

5.2.9 Earthquake Load (E)

5.2.9.1 General

Earthquake load design for buildings and other structures shall be determined in accordance with DOE-STD-1020 and UBC (Ref. 6.2.9.1) for PC-1 and PC-2, and in accordance with DOE-STD-1020 and ASCE 4 (Ref. 6.2.6.1) dynamic analysis methods for PC-3 and PC-4.

5.2.9.2 PC-1 and PC-2 Structures

For the purposes of earthquake load design for PC-1 and PC-2 structures using the UBC, the following modifications shall be made:

I = Importance factor,
= 1.0 for PC-1
= 1.25 for PC-2

The Soil Profile type at SRS may be taken as S_D . A review by a qualified Geotechnical Engineer of the subsurface conditions is recommended for the determination of the facility specific soil profile type.

For PC-1 and PC-2 structures regulated by SCDHEC (Ref. 6.2.15.1) UBC requirements for PC-2 shall be used for earthquake load.

5.2.9.3 PC-3 and PC-4 Structures

Earthquake loads for PC-3 and PC-4 structures shall be determined in accordance with DOE-STD-1020. The site specific response spectra for PC-3 (Ref. 6.2.14.11) and PC-4 (Ref. 6.2.14.3) are given in Figure 7.2.1 and Table 7.1.3 for horizontal ground motion. The vertical site specific response spectra shall be taken as two-thirds of the horizontal spectra given in Figure 7.2.1 and Table 7.1.3.

The spectra given in Table 7.1.3 and Figure 7.2.1 are considered "preliminary" in accordance with WSRC E7 Manual. To become "confirmed" SGS must review the facility specific soil conditions.

For structures with deep foundations, design response spectra appropriate for the depth shall be used.

For PC-3 and PC-4 structures regulated by Ref. 6.2.15.1, the earthquake loads given in this standard shall be considered to meet the seismic loading requirements of SCDHEC.

5.2.9.4 Systems and Components

For systems and components, additional NPH evaluation requirements apply as described in Section 2.4 of DOE-STD-1020. PC-1 and PC-2 systems and components shall be designed for loads defined by the UBC. Seismic loads for PC-3 and PC-4 sub-structures or sub-systems shall be based on in-structure response spectra, or in-structure acceleration and displacement time histories. Where in-structure response spectra are needed, the methods given in Section 3.4.2 or ASCE 4 (Ref. 6.2.6.1) shall be used. Earthquake loads on new PC-3 and PC-4 systems and components shall be increased by twenty percent either through amplification of in-structure response spectra or in-structure time histories by a factor of

1.2, or through the use of the increased load factor for E in loading combinations given in Section 5.3.2.

5.2.10 Self Limiting Loads (T & T_a)

Three types of self limiting loads (T , unless otherwise noted) shall be considered: thermal loads, creep and shrinkage, and settlement.

5.2.10.1 Thermal Loads

The design of structures shall consider the effects of stresses and movements resulting from variations in temperature.

5.2.10.2 Creep and Shrinkage

Concrete and masonry structures shall be investigated for stresses and deformations induced by creep and shrinkage.

5.2.10.3 Settlement

Buildings and structures shall be designed for the total and differential foundation settlements, resulting from the combined static and dynamic loads. The dynamic settlement is due to dissipation of pore pressure and/or redistribution of soil stresses from the effects of a design basis earthquake. The dynamic portion of the settlement is to be factored by 1.2 for new facilities. The combined static and dynamic differential settlement is denoted as T_a in the load combinations. The dynamic component of settlement is not taken concurrent with E.

For existing facilities, the dynamic settlement factor may be taken as 1.0.

5.2.11 Accident Load (P_a)

The consequences of a design basis accident resulting in internal pressurization shall be considered. Consequences of accidental explosions (internal or external to the facility) and generated missiles shall be considered in the design or evaluation of facilities. Consequences of potential heavy load drops shall be considered in the design or evaluation of structures.

Loads resulting from internal pressure, explosions, missile impact and heavy load drop shall be considered a P_a loading.

5.2.12 Pipe Break Load (Y)

Effects of pipe breaks on structures, including reaction, jet, and movement, shall be considered.

5.2.13 Flood Load (F_a)

The structure shall be designed for the flooding and wave action consequences associated with flooding events with return periods of 500, 2000, 10,000, or 100,000 years for PC-1, PC-2, PC-3, or PC-4 respectively per DOE-STD-1020. Loads resulting from flooding and wave action shall be considered per Table 7.1.1.

5.3 Load Combinations

Load combinations used for the design of an SSC are based on the SSC Performance Category and its structural material. For load combinations where a load reduces the effect of other loads, and at any time it may not be present, the load combinations shall be considered assuming the worst conditions for the load present or not.

Where loads may reverse, such as wind and seismic, both directions of loading shall be considered.

Floor live load shall be considered to vary from zero to its full value to determine the worst condition of loading for each element. Crane hook loads need not be combined with roof live load or with more than three fourths of the snow load or one half of the wind load.

5.3.1 Performance Categories 0, 1, and 2

NPH loads need not be considered for PC-0 SSC.

For Allowable Stress Design (ASD), the PC-1 and PC-2 SSC shall be designed for the most critical condition resulting from the following load combinations as modified from the UBC:

ASD

1. $A = D$
2. $A = D + L + F + H + (L_r \text{ or } S \text{ or } R)$
3. $f_1 A = D + L + F + H + (W \text{ or } E/1.4)$
4. $f_1 A = D + L + F + H + W + 0.5S$
5. $f_1 A = D + L + F + H + S + 0.5W$
6. $f_1 A = D + L + F + H + S + E/1.4$
7. $f_2 A = D + L + F + H + (L_r + S + R) + (W \text{ or } E/1.4) + T$
8. $f_2 A = D + L + F + H + 0.5W + F_a$

Where:

- A is the required allowable stress capacity.
- $f_1 = 1.33$ for all stresses.
- $f_2 = 1.4$ for shear and 1.5 for all other stresses.
- For axial tension, the factored allowable stress ($1.33A$, or $1.5A$) shall not exceed 0.7 times the ultimate tensile strength of the material.

When the design of the SSC is based on ultimate strength design (concrete) or load resistance factor design (steel), each element shall be designed to resist the most critical effects of the load factors and load combinations from the following load combinations as modified from the UBC:

Concrete and Masonry (SD)

1. $U = 1.4D + 1.7 L_T + 1.4F$

2. $U = 1.05D + 1.3L_T + 1.05F + 1.3H + 1.3W$
3. $U = 0.9D + 1.05F + 1.3H + 1.3W$
4. $U = 1.1 (1.2D + 1.0 L_T + 1.0F + 1.0H + 1.0E + 1.0T)$
5. $U = 1.1 (0.9D + 1.0F + 1.0H + 1.0E + 1.0T)$
6. $U = 1.4D + 1.7L_T + 1.4F + 1.7H$
7. $U = 0.9D + 1.4F + 1.7H$
8. $U = 1.05 D + 1.3L_T + 1.05F + 1.05T$
9. $U = 1.4D + 1.4T$
10. $U = 1.05D + 1.3 L_T + 1.05F + 0.8W + 1.3H + 1.3F_a$
11. $U = 0.9D + 1.05F + 0.8W + 1.3H + 1.3F_a$

Where

- U is the required ultimate strength capacity.
- L_T is the most critical value obtained from the following:

$$\begin{aligned}
 &= L + 0.5 L_r, & &= 0.5L + L_r, \\
 &= L + 0.5S, & &= 0.5L + S, \\
 &= L + 0.5R, & &= 0.5L + R
 \end{aligned}$$

Steel (SD)

1. $U = 1.4D$
2. $U = 1.2D + 1.6L + 1.3F + 1.6H + 0.5 (L_r \text{ or } S \text{ or } R)$
3. $U = 1.2D + (0.5L \text{ or } 0.8W) + 1.3F + 1.3H + 1.6 (L_r \text{ or } S \text{ or } R)$
4. $U = 1.2D + 0.5L + 1.3F + 1.3H + 0.5 (L_r \text{ or } S \text{ or } R) + 1.2T$
5. $U = 1.2D + 0.5L + 0.2S + 1.3F + 1.3H + 1.0E + 1.2T$
6. $U = 0.9D + 1.3F + 1.3H + (1.3W \text{ or } 1.0E) + 1.2T$
7. $U = 1.2D + 0.5L + 1.3F + 1.3H + 0.8W + 1.3F_a$
8. $U = 0.9D + 1.3F + 1.3H + 0.8W + 1.3F_a$

Where

- U is the required ultimate strength capacity.

5.3.2 Performance Categories 3 and 4

For Allowable Stress Design (ASD), the PC-3 and PC-4 SSC shall be designed for the most critical condition resulting from the following load combinations as modified from the UBC:

ASD

1. $A = D$
2. $A = D + L + F + H + (L_r \text{ or } S \text{ or } R)$
3. $f_1 A = D + L + F + H + (W \text{ or } 1.2E^*)$
4. $f_1 A = D + L + F + H + W + 0.5S$

5. $f_2 A = D + L + F + H + S + 0.5W$
6. $f_4 A = D + L + F + H + S + 1.2E^* + T + P_a + Y$
7. $f_3 A = D + L + F + H + (L_r \text{ or } S \text{ or } R) + (W \text{ or } 1.2E^*) + T$
8. $f_3 A = D + L + F + H + 0.5W + F_a$
9. $f_3 A = D + F + H + W_t$
10. $f_3 A = D + L + L_r + F + H + W_t$
11. $f_3 A = D + L + L_r + F + H + W_t + T$
12. $f_4 A = D + L + L_r + F + H + T_a^{**}$
13. $f_4 A = D + L + F + H + (L_r \text{ or } S \text{ or } R) + 1.2E^* + T + P_a + Y$

* For existing SSC the load factor for E may be taken as 1.0. For new systems and components the load factor for E may be taken as 1.0 if the in-structure response spectra has been amplified by a factor of 1.2 as stated in Section 5.2.9.4.

**For settlement, T_a , see Section 5.2.10.3

Where

- A is the required allowable stress capacity.
- $f_1 = 1.33$ for all stresses.
- $f_2 = 1.4$ for shear and 1.5 for all other stresses.
- $f_3 = 1.4$ for shear and 1.6 for all other stresses.
- $f_4 = 1.4$ for shear and 1.7 for all other stresses

For axial tension the factored allowable stress (1.33A, 1.5 A, 1.6A, or 1.7A) shall not exceed 0.7 times the ultimate tensile strength of the material.

When the design of the SSC is based on ultimate strength design (concrete) or load resistance factor design (steel), each element shall be designed to resist the most critical effects of the load factors and load combinations from the following load combinations as modified from the UBC:

Concrete and Masonry (SD)

1. $U = 1.4D + 1.7L_T + 1.4F$
2. $U = 1.0D + 1.15L_T + 1.0F + 1.15H + 1.15W$
3. $U = 0.9D + 1.0F + 1.15H + 1.15W$
4. $U = 1.1 (1.2D + 1.0L_T + 1.0F + 1.0H + 1.2E^* + 1.0T)$
5. $U = 1.1 (0.9D + 1.0F + 1.0H + 1.2E^* + 1.0T)$
6. $U = 1.4D + 1.7L_T + 1.4F + 1.7H$
7. $U = 0.9D + 1.4F + 1.7H$
8. $U = 1.05D + 1.3L_T + 1.05F + 1.05T$
9. $U = 1.4D + 1.4T$
10. $U = 1.05D + 1.3L_T + 1.05F + 1.3H + 0.8W + 1.0F_a$

$$11. U = 0.9D + 1.05F + 1.3H + 0.8W + 1.0F_a$$

$$12. U = 1.0D_{@} + 1.0F + 1.0H + 1.0W_t + 1.0T$$

$$13. U = 1.0D_{@} + 1.0L_T + 1.0F + 1.0H + 1.0W_t + 1.0T$$

$$14. U = 1.1 (1.2D + 1.0L_T + 1.0F + 1.0H + 1.2E^* + 1.0T_a^{**} + 1.0P_a + 1.0Y)$$

$$15. U = 1.0D_{@} + 1.0T_a^{**} + 1.25P_a$$

* For existing SSC the load factor for E may be taken as 1.0. For new systems and components the load factor for E may be taken as 1.0 if the in-structure response spectra has been amplified by a factor of 1.2 as stated in Section 5.2.9.4.

@ Combination with 0.9D, in place of 1.0D, shall also be considered.

**For settlement, T_a , see Section 5.2.10.3.

Where

- U is the required ultimate strength capacity.

- L_T is the most critical value obtained from the following:

$$= L + 0.5 L_r, \quad = 0.5L + L_r,$$

$$= L + 0.5S, \quad = 0.5L + S,$$

$$= L + 0.5R, \quad = 0.5L + R$$

Steel (SD)

$$1. U = 1.4D$$

$$2. U = 1.2D + 1.6L + 1.3F + 1.6H + 0.5 (L_r \text{ or } S \text{ or } R)$$

$$3. U = 1.2D + (0.5L \text{ or } 0.8W) + 1.3F + 1.3H + 1.6 (L_r \text{ or } S \text{ or } R)$$

$$4. U = 1.2D + 0.5L + 1.3F + 1.3H + 0.5 (L_r \text{ or } S \text{ or } R) + 1.2T$$

$$5. U = 1.2D + 0.5L + 0.2S + 1.3F + 1.3H + 1.2E^* + 1.2T$$

$$6. U = 0.9D + 1.3F + 1.3H + (1.3W \text{ or } 1.2E^*) + 1.2T$$

$$7. U = 1.2D + 0.5L + 1.3F + 1.3H + 0.8W + 1.0F_a$$

$$8. U = 0.9D + 1.3F + 1.3H + 0.8W + 1.0F_a$$

$$9. U = 1.0D_{@} + 1.3F + 1.3H + 1.0W_t + 1.0T$$

$$10. U = 1.0D_{@} + 1.0L + 1.0L_r + 1.3F + 1.3H + 1.0W_t + 1.0T$$

$$11. U = 1.0D_{@} + 1.0L + 1.0L_r + 1.0F + 1.0H + 1.2E^* + 1.0T_a^{**} + 1.0P_a + 1.0Y$$

$$12. U = 1.0D_{@} + 1.0F + 1.0H + 1.0T_a^{**} + 1.25P_a$$

* For existing SSC the load factor for E may be taken as 1.0. For new systems and components see also the load factor for E may be taken as 1.0 if the in-structure response spectra has been amplified by a factor of 1.2 as stated in Section 5.2.9.4.

@ Combination with 0.9D, in place of 1.0D, shall also be considered.

**For settlement, T_a , see Section 5.2.10.3.

Where

- U is the required ultimate strength capacity.

5.3.3 Load Combinations for Highway and Railway Structures

Load combinations for highway structures shall be in accordance with AASHTO HB (Ref. 6.2.1.1). Load combinations for railway structures shall be in accordance with AREMA Manual (Ref. 6.2.5.1).

5.4 Materials

The UBC shall be followed consistent with the code used in conjunction with Sections 5.2 and 5.3 for allowable material strength capacities and detailing requirements for all structures except as noted below.

Highway Structures:	AASHTO HB (Ref. 6.2.1.1)
Railway Structures;	AREMA Manual (Ref. 6.2.5.1)
Steel Decks:	SDI-28 and SDI DDM02 (Ref. 6.2.13.1 and 6.2.13.2)
Pre-engineered Buildings:	MBMA Metal Building System Manual (Ref. 6.2.10.1)
Austenitic Stainless Steel:	UBC ASD steel provisions with N690 (Ref. 6.2.3.1) allowables for axial compression
Cold Formed Stainless Steel	ANSI/ASCE 8 (Ref. 6.2.6.5)
PC-1 and PC-2 Atmospheric Steel Water Tanks:	AWWA D100 (Ref. 6.2.7.1)
PC-1 and PC-2 Atmospheric Steel Petroleum Storage Tanks	API 650 (Ref. 6.2.4.1)
Sanitary Concrete Structures:	ACI 350 (Ref. 6.2.2.5)
PC-3 and PC-4 Concrete Anchorage:	ACI 349, Appendix B (Ref. 6.2.2.4)
PC-1 and PC-2 Fiber Reinforced Plastic	ASCE Manual 63 (Ref. 6.2.6.4)
Concrete Chimneys	ACI 307 (Ref. 6.2.2.6)
Steel Stacks	ASME STS-1 (Ref. 6.2.12.1)

5.5 Miscellaneous Requirements

5.5.1 Sliding, Overturning, and Buoyancy

Buildings and structures shall be designed to resist overturning, sliding and buoyancy due to earthquake, wind, tornado, lateral earth pressures, or fluid loads in accordance with Section 5.2 of this Engineering Standard with minimum safety factors provided in Table 7.1.2.

5.5.2 Foundation Design

Foundations shall be designed in accordance with the requirements of the UBC, except as noted below:

- Drilled piers shall comply with ACI 336.3R (Ref. 6.2.2.3).
- Design of piers, posts or poles per UBC is acceptable for depths less than 20 feet and provided that the ratio of length to relative stiffness factor is greater than 5 (see ACI 336.3R, Ref. 6.2.2.3).
- Ribbed-mat slabs shall comply with ACI 336.2R (Ref. 6.2.2.2).

5.5.3 Shoring Design

Shoring design shall comply with requirements of UBC, and OSHA 29 CFR Part 1926 (Ref. 6.2.11.1).

5.5.4 Craneway Design

Crane runway and supporting structure shall be designed per CMAA Specification #70 (Ref. 6.2.8.1) or #74 (Ref. 6.2.8.2) requirements as applicable.

5.5.5 Fire Enclosure Evaluations of Concrete Elements

In the structural evaluation of concrete elements for fire events, where necessary, the provisions found in ACI 216 (Ref. 6.2.2.1) shall be followed.

5.5.6 Concrete Anchors

PC-1 and PC-2 concrete anchors shall be designed per requirements of UBC (Ref. 6.2.9.1).

PC-3 and PC-4 concrete anchors shall be designed per requirements of ACI 349 Appendix B (Ref. 6.2.2.4).

Information useful for the design, selection and qualification of concrete expansion anchors, cast-in-place anchors and grouted anchor bolts is provided in 03251-G (Ref. 6.2.14.4). Information useful for installation and testing of concrete expansion anchors, cast-in-place anchors and grouted anchor bolts is provided in 03252-G (Ref. 6.2.14.5).

5.5.7 Coring, Chipping, and Drilling in Concrete

Coring, chipping and drilling in concrete elements or structures shall be per 03010-G (Ref. 6.2.14.6).

5.5.8 Concrete for Confinement

Concrete used for confinement shall consider liquid containment strategies given in Ref. 6.2.6.6.

5.5.9 Fragility Analysis

The existing SSC that do not meet the deterministic limits of this standard may be further evaluated using median failure limits and variabilities calculated for the given SSC.

5.5.10 Ductility Provisions

Seismic ductility provisions for UBC Seismic Zone 3 shall be used for new PC-3 and PC-4 structures.

6.0 REFERENCES

6.1 DOE Orders as included in WSRC S/RID, and DOE Orders and Standards and Memoranda

6.1.1 DOE-STD-1020-94 - NPH Design and Evaluation Criteria for DOE Facilities, Change Notice 1, January 1996.

6.1.2 DOE-STD-1021-93 - NPH Performance Categorization Criteria for Structures, Systems, and Components, Change Notice 1, January 1996.

6.1.3 DOE Order 420.1 - Facility Safety, Change 2 dated October 24, 1996.

6.1.4 DOE Guide 420.1-Y, Interim Guidelines for the Mitigation of Natural Phenomena Hazards for DOE Nuclear Facilities and Non-Nuclear Facilities.

6.1.5 A Memorandum of January 22, 1998 from DOE to the Office of Nuclear Safety Policy and Standards: H. Chander, Newsletter (Interim Advisory on Straight Winds and Tornadoes).

6.2 Codes and Standards

6.2.1 American Association of State Highway and Transportation Officials (AASHTO)

6.2.1.1 Standard Specification for Highway Bridges (AASHTO HB)

6.2.2 American Concrete Institute (ACI)

6.2.2.1 ACI 216 Guide for Determining the Fire Endurance of Concrete Elements

6.2.2.2 ACI 336.2R Suggested Analysis and Design Procedures for Combined Footings and Mats

6.2.2.3 ACI 336.3R Design and Construction of Drilled Piers

6.2.2.4 ACI 349 Code Requirements for Nuclear Safety Related Concrete Structures

6.2.2.5 ACI 350 Environmental Engineering Concrete Structures

6.2.2.6 ACI 307 Standard Practice for the Design and Construction of Reinforced Concrete Chimneys

6.2.3 American Institute of Steel Construction (AISC)

6.2.3.1 ANSI/AISC N690 Nuclear Facilities: Steel Safety-Related Structures for Design, Fabrication and Erection

6.2.4 American Petroleum Institute (API)

6.2.4.1 API 650 Welded Steel Tanks for Oil Storage

6.2.5 American Railway Engineering and Maintenance-of-Way Association (AREMA)

6.2.5.1 AREMA Manual for Railway Engineering, Volume I and II

6.2.6 American Society of Civil Engineers (ASCE)

6.2.6.1 ASCE 4 Seismic Analysis of Safety Related Nuclear Structures

6.2.6.2 ANSI/ASCE 7 Minimum Design Loads for Buildings and Other Structures

6.2.6.3 ASCE 58 Structural Analysis and Design of Nuclear Plant Facilities

6.2.6.4 ASCE Manual 63 Structural Plastic Design Manual

6.2.6.5 ANSI/ASCE 8 Specification for the Design of Cold-Formed Stainless Steel Structural Members

6.2.6.6 Concrete Watertight Structures and Hazardous Liquid Containment, Robert Hengst, ASCE

6.2.7 American Water Works Association (AWWA)

6.2.7.1 AWWA D100 Welded Steel Tanks for Water Storage

6.2.8 Crane Manufacturers Association of America (CMMA)

6.2.8.1 CMMA Specification #70, Electrical Overhead Traveling Cranes

6.2.8.2 CMMA Specification #74, Specifications for Top Running and Under Running Types of Single Girder Electric Overhead Traveling Cranes

6.2.9 International Conference of Building Officials (ICBO)

6.2.9.1 Uniform Building Code (UBC)

6.2.10 Metal Building Manufacturers Association (MBMA)

6.2.10.1 *Low Rise* Building Systems Manual

6.2.11 Occupational Safety and Health Administration (OSHA)

6.2.11.1 29 CFR Part 1926 OSHA Safety and Health Standards

6.2.12 American Society of Mechanical Engineers

6.2.12.1 ASME STS-1 Steel Stacks

6.2.13 Steel Deck Institute (SDI)

6.2.13.1 SDI - Publ 28 Design Manual for Composite Decks from Decks and Cellular Metal Deck with Electrical Distribution

6.2.13.2 DDM02 Diaphragm Design Manual

6.2.14 SRS Standards, Reports and Calculations

6.2.14.1 SQP Seismic Qualification Program, Rev. 3, 1995

6.2.14.2 Engineering Standard 01110, Civil Site Design Criteria

6.2.14.3 SRS Seismic Response Analysis and Design Basis Guidelines, WSRC-TR-97-0085, Rev. 0, R. C. Lee, M. E. Maryak and M. D. McHood, March 1997

6.2.14.4 Concrete Expansion Anchors, Cast-in Place Anchors and Grouted Anchor Bolts Engineering Guide 03251-G

6.2.14.5 Installation and Testing of Concrete Anchors, Engineering Guide 03252-G

6.2.14.6 Coring, Chipping and Drilling in Concrete, Engineering Guide 03010-G

6.2.14.7 Evaluation of Seismic Spatial Interactions Between Facility Structures, Systems, and Components, Engineering Guide, 11520-G

6.2.14.8 Tornado, Maximum Wind Gust, and Extreme Rainfall Event Occurrence Frequencies at the Savannah River Site (U), WSRC-TR-98-00329, September 1998.

6.2.14.9 Engineering Standard 01060, Rev. 2, Data Letter, TCDL-037, Rev. 0, August 1997.

6.2.14.10 Tornado Hazard Assessment, Memo from McDonald-Mehta Engineers to Brent Gutierrez, November 9, 1997.

6.2.14.11 "Revised Envelope of the Site Specific PC3 Surface Ground Motion", Memo from Brent Gutierrez to Lawrence Salomone and Fred Loceff, September 9, 1999.

6.2.15 State of South Carolina, Department of Health and Environmental Control (SCDHEC)

6.2.15.1 Final Regulation, Department of Health and Environmental Control, Chapter 61, 61-104, Hazardous Waste Management Location Standards, Statutory Authority: 1976 Code Section 44-56-30, -35, et. seq.

6.2.16 U.S. Nuclear Regulatory Commission (USNRC)

6.2.16.1 NUREC-0800, Standard Review Plan for the Review of Safety Analysis Reports for Nuclear Power Plants, U.S. NRC

7.0 TABLES and FIGURES

7.1 Tables

7.1.1 Natural Phenomena Hazards Criteria

7.1.2 Factors of Safety

7.1.3 PC-3 and PC-4 Site Specific Spectra, Horizontal 5% Damping

7.1.4 Performance Categories

7.2 Figures

7.2.1 PC-3 and PC-4 Site Specific Spectra, Horizontal 5% Damping

TABLE 7.1.1 - Natural Phenomena Hazards Criteria						
	Performance Category	PC-0	PC-1	PC-2	PC-3	PC-4
	Performance Goal Annual Probability of Exceedance	NA	1×10^{-3}	5×10^{-4}	1×10^{-4}	1×10^{-5}
S* E I S M I C	Required Minimum Annual Hazard Exceedance Probability	NA	2×10^{-3}	1×10^{-3}	5×10^{-4}	1×10^{-4}
	Peak Ground Acceleration(PGA)**	NA	**	**	**	**
W	Annual Hazard Exceedance Probability	NA	2×10^{-2}	1×10^{-2}	1×10^{-3}	1×10^{-4}
	Three Second Wind Speed, mph	NA	100	107	133	160

I N D	Missile Criteria	NA	NA	NA	2x4 timber plank 15 lb. @ 50 mph (horiz); max height 30 ft.	2x4 timber plank 15 lb. @ 50 mph (horiz); max height 50 ft.
	Annual Hazard Exceedance Probability	NA	NA	NA	2×10^{-5}	2×10^{-6}
	Three Second Wind Speed, mph	NA	NA	NA	178	TBD
	Atmospheric Pressure Change (APC)	NA	NA	NA	TBD	TBD
T O R N A D O	Missile Criteria	NA	NA	NA	2x4 timber plank 15 lb. @ 100 mph (horiz); max height 150 ft; 70 mph (vert) 3 in dia std steel pipe, 75 lb @ 50 mph (horiz); max height 75 ft; 35 mph (vert) 3000 lb automobile @ 19 mph rolls and tumbles	2x4 timber plank 15 lb. @ 150 mph (horiz); max height 200 ft; 100 mph (vert) 3 in dia std steel pipe, 75 lb @ 75 mph (horiz); max height 100 ft; 50 mph (vert) 3000 lb automobile @ 25 mph rolls and tumbles
	Annual Hazard Exceedance Probability	NA	2×10^{-3}	5×10^{-4}	1×10^{-4}	1×10^{-5}
F L O O D	Roof Design	NA	TBD	TBD	TBD	TBD
	DBFL	NA	TBD	TBD	TBD by Facility	TBD by Facility

* The SCDHEC (Ref. 6.2.15.1) seismic provisions for Hazardous Waste Management locations of 10% probability of occurrence in 250 years is equivalent to a return period of about 2400 years. This corresponds to an annual exceedance probability of approximately $4.2\text{E-}4$. The PGA determined for a PC-3 (annual hazard exceedance probability of $5\text{E-}4$) is 0.16g. The typical PGA for PC-2 exceeds that required by the SCDHEC provisions.

** For PC-1 and PC-2 seismic provisions, see Section 5.2.9.2. For PC-3 and PC-4 seismic provisions, see Section 5.2.9.3.

Table 7.1.2 - Factors of Safety					
Performance Category	Analysis	DBE	DBT	DBFL	DBW
PC-4 ¹	Overturing	1.1 ³	1.1 ⁵		1.5 ⁴
	Sliding	1.1 ³	1.1 ⁵		1.5 ⁴
	Buoyancy			1.1 ⁶	
PC-3 ¹	Overturing	1.1 ³	1.1 ⁵		1.5 ⁴
	Sliding	1.1 ³	1.1 ⁵		1.5 ⁴
	Buoyancy			1.1 ⁶	
PC-2 ²	Overturing	1.0 ³			1.5 ⁴
	Sliding	1.0 ³			1.5 ⁴
	Buoyancy			1.0 ⁶	
PC-1 ²	Overturing	1.0 ³			1.5 ⁴
	Sliding	1.0 ³			1.5 ⁴
	Buoyancy			1.0 ⁶	

Notes:

1. For PC-3 and PC-4 facility foundation design, these values are taken from the Standard Review Plan, NUREG 0800, Section 3.8.5 (Ref. 6.2.16.1). These factors of safety are *for* combinations provided in Notes 3, 4, 5, or 6, as applicable.
2. For PC-2 and PC-1, the factors of safety are consistent with the SBC and UBC for

earthquake loads and ASCE 7 for normal wind loads and are *for* combinations provided in Notes 3, 4, and 6, as applicable.

3. $D + H + (1.2E*/1.4)$

4. $D + H + W$

5. $D + H + (W_t/1.4)$

6. $D + F_a$

Where:

D = Dead Load

H = Lateral Earth Pressure

W = Wind Load

E = Earthquake Loads

F_a = Buoyant Forces

W_t = Tornado Loads

* For existing structures the 1.2 load factor for E may be taken as 1.0
For clarification on the unfactored earthquake load see the Data Letter (Ref. 6.2.14.9).

Table 7.1.3 — PC-3 and PC-4 Site Specific Spectra, Horizontal 5% Damping	
PC3 Reference 6.2.14.11	
Frequency (Hz)	Horizontal Spectral Acceleration (g)
0.10	0.0075
0.20	0.030
0.25	0.049
0.30	0.065
0.35	0.085
0.40	0.104
0.45	0.121
0.50	0.135
0.60	0.1677
2.00	0.375
9.00	0.375
33.0	0.160
100.0	0.160
PC4 Reference 6.2.14.3	
Frequency (Hz)	Horizontal Spectral Acceleration (g)
0.1	0.0067
0.2	0.0311
0.3	0.0632
0.4	0.1519
0.5	0.2411

U.S	U.S411
0.7	0.4813
1.5	0.760
7	0.655
18	0.350
33	0.227
100	0.227

Table 7.1.4 Performance Categories

Performance Categories are used as a means to ensure a graded level of protection for structures, systems, and components during Natural Phenomena Hazard (NPH) events. Functional Classifications are used to determine the minimum Performance Categories (PC) of SSCs in accordance with DOE Order 420.1 and DOE-STD-1021-93. Performance Categories PC-0 through PC-4 are assigned based on the function of the SSC as determined during the Functional Classification process. The table below provides the correlation between Functional Classification and Performance Categories. For replacement SSCs, Natural Phenomena Hazard (NPH) qualification, as a minimum, shall be the same as the original SSC through the provisions of DOE Order 420.1 Section 4.4.2. For all other conditions, SSCs shall be NPH qualified commensurate with the assigned Performance Category. This table provides the minimum requirements for compliance with DOE Order 420.1. Factors other than Functional Classification that are to be considered in determining the PC include programmatic mission, cost and replaceability of the SSC. It may be prudent based on other considerations such as mission importance or economic reasons to increase the performance category to provide a higher level of assurance.

Functional Classification (Note 1)	Performance Category
Safety Class (Reactors)	PC-4
Safety Class (Non-reactor)	PC-3
Safety Significant	PC-2
Production Support (Note 2) and General Services (Notes 2 and 5) except for the following:	PC-1
Criterion 3 for Production Support in Attachment 8.5 to the Procedure 2.25, Note 1, (Emergency Plan)	PC-2
Assembly of more than 300 people (Note 3)	PC-2
Emergency Equipment (Note 4)	PC-2

Notes:

(1) Criteria for Functional Classifications are identified in Manual E7, Procedure 2.25.

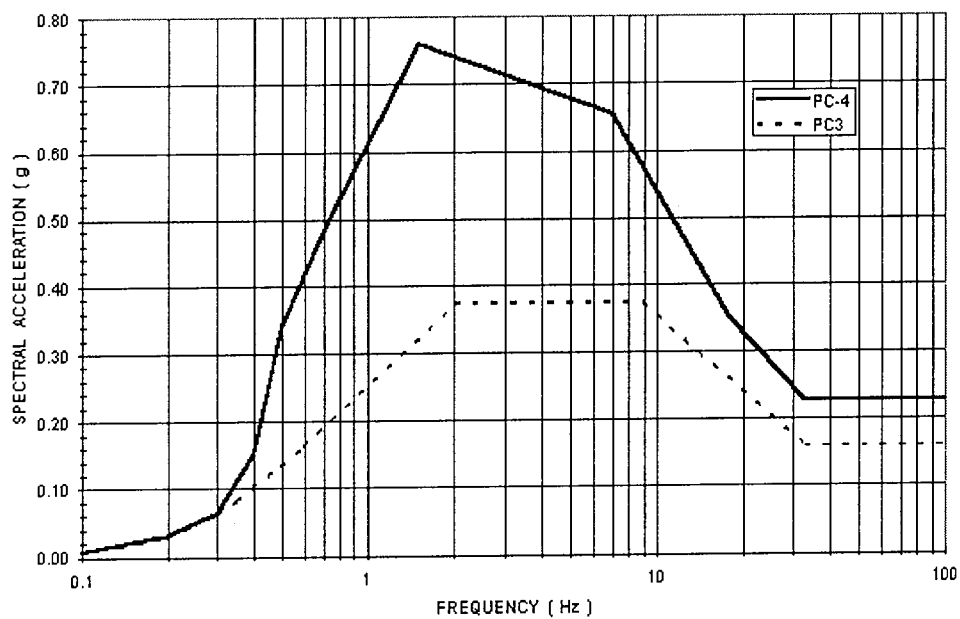
(2) In buildings that have routine human occupancy, the structure and any system or component whose failure could cause fatality or serious injury to in-facility workers shall be designed, as a minimum, to PC1.

(3) This Performance Category is defined in DOE Order 420.1 and DOE Standard 1021-93.

(4) Per DOE STD-1021-93, this includes SSCs such as emergency handling (fire station, medical facilities), hazard recovery, those related to emergency preparedness (emergency operations facilities) and communications that may be needed to preserve the health and safety of workers and visitors.

(5) Some General Services SSCs may be designed without consideration of NPH loads (PC0). This may be done when there is non human occupancy, no safety, and no mission considerations, and it is more cost effective to replace or repair the SSC than to design it to withstand NPH effects. Any SSC whose failure can affect the performance of a Performance Category 1,2,3 or 4 SSC, as determined by an interaction (two over one) evaluation, shall not be placed in PC-0.

Figure 7.2.1
Site Specific Spectra, Horizontal 5% Damping



K-TRT-F-00001

Rev. 0

May 1999

F-Area Northeast Expansion Report (U)

Site Geotechnical Services Department

UNCLASSIFIED

**Westinghouse Savannah River Company
Savannah River Site
Aiken, SC 29808**

Prepared for the U.S. Department of Energy Under Contract No. DE-AC09-89SR18035

UNCLASSIFIED

WESTINGHOUSE SAVANNAH RIVER COMPANY
INTEROFFICE MEMORANDUM



Date: February 26, 2001

PEC-SGS-2001-00011

To: Distribution

From: Frank Syms, 730-2B/1084 *FS*

Subject: Revisions to K-TRT-F-00001, F-Area Northeast Expansion Report (U)

Summary of revisions made to K-TRT-F-00001, Rev 1, dated January 2001 include:

1. Pages 12 and 13- Text revised on engineering stratigraphy.
2. Table 3.3-1- Stratigraphic picks revised on CPT soundings 176, 180 and 186.
3. Table 3.3-3- Soft zone thickness revised for SPT boring FB-20.

If you have questions, please contact me at (803) 952-6927.

c: Russ Beckmeyer, 730-2B/130
Mike Lewis, 730-2B/116
Larry Salomone, 730-B/304
Chester Reeves, 730-2B/1083
SGS Files, 730-2B/1102

Project:

F-Area Northeast Expansion

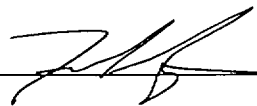
Document:

K-TRT-F-00001, Rev.0

Title:

F-Area Northeast Expansion Report (U)

Prepared by:



F. H. Syms, Geotechnical Engineering, SGS Department

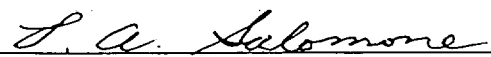
5-28-99
Date

Approvals:



M.R. Lewis, Manager, Geotechnical Engineering, SGS Department

5/28/99
Date



L. A. Salomone, Site Chief Geotechnical Engineer, SGS Department

6/1/99
Date

DISCLAIMER

This product was prepared as an account of work sponsored by an agency of the United States Government. Neither the United States Government nor any agency thereof, nor any of their employees, makes any warranty, express or implied, or assumes any legal liability or responsibility for the accuracy, completeness, or usefulness of any information, apparatus, product, or process disclosed, or represents that its use would not infringe privately owned rights. Reference herein to any specific commercial product, process, or service by trade name, trademark, manufacturer, or otherwise does not necessarily constitute or imply its endorsement, recommendation, or favoring by the United States Government or any agency thereof. The views and opinions of authors expressed herein do not necessarily state or reflect those of the United States Government or any agency thereof.

TABLE OF CONTENTS

TABLE OF CONTENTS	iv
LIST OF TABLES	v
LIST OF FIGURES	vi
1 INTRODUCTION.....	1
1.1 Purpose and Objectives	1
1.2 Report Organization	2
1.3 Quality Assurance	2
2. SUBSURFACE EXPLORATION	4
2.1 Field Test Location and Clearance	5
2.2 Equipment and Field Test Methods	5
2.2.1 <i>Exploration Contractor(s) and Equipment</i>	6
2.2.2 <i>Standard Penetration Test (SPT)</i>	7
2.2.3 <i>Undisturbed Sampling</i>	8
2.2.4 <i>Piezcone Penetration Test Soundings</i>	9
2.2.5 <i>Borehole and Penetration Abandonment</i>	9
2.3 Sample Prep, Handling, Storage, Transportation and Control.....	10
3. SUBSURFACE CONDITIONS	11
3.1 Shallow Engineering Stratigraphy.....	11
3.1.1 <i>TR1 and TR1A Layers</i>	12
3.1.2 <i>TR2A and TR2B Layers</i>	12
3.1.3 <i>TR3/4 and DB1/3 Layers</i>	13
3.1.4 <i>DB4/5, ST1 and ST2 Layers</i>	13
3.1.5 <i>GC Layer</i>	14
3.2 Groundwater Conditions	15
3.3 Engineering Soil Characteristics.....	15
3.3.1 <i>Soft Zone Characteristics</i>	16
3.3.1.1 <i>Northeast Expansion Area Soft Zones</i>	17
3.3.1.2 <i>APSF Area Soft Zones</i>	18
3.3.1.3 <i>Soft Zone samples and Analysis</i>	18
4. GEOTECHNICAL AND FOUNDATION ASSESSMENT	20
5. CONCLUSIONS AND RECOMMENDATIONS	21
6. REFERENCES.....	23

APPENDICES

- Appendix A Borings Logs
- Appendix B Laboratory Test Data
- Appendix C Cone Penetrometer Test Data

LIST OF TABLES

Table 3.3-1 Summary of CPT Layer Picks

Table 3.3-2 Comparison of Engineering Soil Properties

Table 3.3-3 Summary of SCPTU Soft Zone Intervals

Table 3.3-4 Summary of Soft Zone Soil Properties

LIST OF FIGURES

Figure 1.0-1	Location Map of F-Area and the Northeast Expansion Area
Figure 1.0-2	Location Map of the Northeast Expansion Area and APSF
Figure 2.0-1	Exploration Location Map
Figure 3.0-1	Subsurface Cross-section 1
Figure 3.0-2	Subsurface Cross-section 2
Figure 3.0-3	Subsurface Cross-section 3
Figure 3.0-4	Subsurface Cross-section 4
Figure 3.0-5	Subsurface Cross-section 5
Figure 3.0-6	Subsurface Cross-section 6
Figure 3.0-7	Subsurface Cross-section 7
Figure 3.0-8	Subsurface Cross-section 8
Figure 3.1-1	Typical CPTU Sounding Plot Showing Stratigraphic Layers
Figure 3.2-1	Hydrograph of Water Table Measurements for Well NBG-5
Figure 3.3-1	Range of SPT N-value Measurements
Figure 3.3-2	CPT Corrected Tip Resistance
Figure 3.3-3	CPT CPT Friction Ratio
Figure 3.3-4	Average Shear Wave Velocity
Figure 3.3-5	Map of APSF Area and Mapped Extent of Lower Soft Zone
Figure 3.3-6	Map of APSF Area and Mapped Extent of Upper Soft Zone

1. INTRODUCTION

A geotechnical program has been completed in F-Area at the Savannah River Site (SRS) in South Carolina. This program investigated the subsurface conditions for the area known as the "northeast expansion" located in the F-Area (Figure 1.0-1). The primary focus was to gather subsurface information within the expansion area and tie this information with the detailed studies completed for the Actinide Packaging and Storage Facility (APSF) located southwest and adjacent to the northeast expansion area (Figure 1.0-2) and the balance of F-Area.

Data acquired from the APSF investigation includes both field exploration and laboratory data, which are included within this report for summary and comparison purposes. Results of this investigation are intended to be used as baseline subsurface conditions for the siting of potential new mission facilities. Further, this data will augment specific foundation design investigations for the proposed new mission facilities to be constructed in this area. Specifically, the program consisted of a field exploration program consisting of Standard Penetration Test (SPT) borings and Seismic Piezocone Penetration Test (SCPTU) soundings; a laboratory testing program and an evaluation of subsurface conditions.

This geotechnical program was performed by SRS Projects Engineering and Construction Division (PE&CD), Site Geotechnical Services (SGS) Department, in conformance with DOE Order 420.1, Procedure Manual E7, (WSRC, 1996a), and SGS Procedure Manual E9 (WSRC, 1996b).

1.1 Purpose and Objectives

The purpose of the investigation was to obtain geotechnical information to characterize the subsurface conditions within the northeast expansion area and compare these conditions with the adjacent APSF area. Specific objectives included:

- define the engineering stratigraphy and compare the continuity, thickness and relative elevation of stratigraphic units across the study site respective to the APSF area;

- determine the index properties of each stratigraphic layer and make a direct comparison to properties determined for the APSF stratigraphy ;
- evaluate the presence, thickness and stratigraphic position of soft zones; and
- evaluate the subsurface conditions in terms of relative geotechnical and foundation capability.

1.2 Report Organization

The text of this report includes five sections. These sections are: Section 1, Introduction; Section 2, Subsurface Exploration; Section 3, Subsurface Conditions; Section 4, Geotechnical and Foundation Assessment, Section 5, Conclusions and Recommendations; and Section 6, References. These sections are followed in succession by tables and figures.

Appendices to this report include: Appendix A, Boring Logs; Appendix B, Laboratory Test Data; and Appendix C, Seismic Piezocone Penetrometer Test Soundings.

This report is divided into two volumes: Volume 1 contains the text, tables, and figures, Volume 2 contains appendices A , B and, C.

1.3 Quality Assurance

Quality related activities performed by WSRC/BSRI organizations during the Geotechnical Investigation were controlled in accordance with the WSRC QA Program as delineated in WSRC Procedure Manual 1Q. Activities were also controlled via compliance to the applicable administrative and technical procedures contained in WSRC Procedure Manual E9, "Site Geotechnical Services."

Cone Penetration Testing was conducted in accordance with the Quality Assurance Plan for WSRC Subcontract AA82276N, with Applied Research Associates, Inc. (ARA) and the ARA Quality Assurance Program for Cone Penetration Testing, Revision 3 (7/30/96). Subcontractor compliance with their implementing procedures and instructions (ARA-Q-101 through 107) also ensured the integrity of the CPT results and interpretations.

Soil testing performed by Law Engineering of Atlanta, Georgia (WSRC Subcontract No. AB80111N) was accomplished through compliance with the Law Engineering QA Program as delineated in the Law Engineering Quality Assurance Manual, Revision 1 (7/25/97), and applicable national/industry test standards (as specified in procurement specification K-SPC-G-00016, Revision 0).

SGS QA provided quality oversight over all quality related activities of the geotechnical investigation. SGS QA oversight activities included: the review and approval of all technical and quality procedures and instructions developed specifically for the investigation; monitoring field activities, sample handling, and soil testing laboratory activities; and providing direct QA oversight over seismic piezocone penetration testing activities.

QA/QC activities were also performed by Law Engineering and Applied Research & Associates personnel as prescribed in their respective QA plans, QA programs, and QA technical procedures.

2. SUBSURFACE EXPLORATION

Between August, 1995 and June, 1998, field exploration programs to support various design stages of the Actinide Packaging and Storage Facility (APSF) were completed. The information from both programs forms the basis for this report. The exploration program for the F-Area northeast expansion area was completed between February 1998 and April 1999. Within this time frame, the majority of the field investigation was completed between February 1998 and May 1998 after which work was suspended until the following fiscal year. Field work resumed in March of 1999 and was completed in April 1999.

The exploration programs consisted of a series of Standard Penetration Test (SPT) borings, Piezocone Penetration Test (CPTU) and Seismic Piezocone Penetration Test (SCPTU) soundings, both of which are referred to as CPT hereafter. Some SPT borings and CPT sounding locations were paired so that a site-specific comparison of results could be obtained. Figure 2.0-1 shows the locations of all borings and soundings.

The borings and soundings were advanced in a grid pattern, roughly 150 feet by 150 feet, covering the expansion area to the north and east of the APSF area. However, in heavily wooded areas, areas where site topography was too steep, or where access could not be obtained due to existing construction trailers, the exploration locations were not performed. This was limited to eight points in the northern and eastern most extent of the area (Figure 2.0-1). Exploration in these areas would be required as construction of new facilities is planned.

The SCPTU was used as the primary exploration technique. SPT borings were located adjacent to selected soundings to measure N-values and retrieve soil samples for laboratory classification testing. Test methods, equipment, and general field procedures, are summarized in the following sections. The following list summary field exploration completed for the northeast expansion investigation:

- 39 Seismic Piezocone Penetrometer Test (SCPTU) Soundings
- 4 Cone Penetrometer Samples
- 9 Standard Penetration Test (SPT) Borings (4 of which were continuous)

2.1 Field Test Location and Clearance

The selection of the boring locations, CPT soundings, and other field work was based primarily on the following criteria and factors:

- Existing Data;
- Data coverage;
- Site conditions (topography, wooded areas, etc.);
- Type of data required;
- Under-and-above ground interferences; and
- Presence of known soft zones.

Approval of the selected location for the field work was preceded by a series of work coordination steps as summarized below (the organization responsible for each step is noted in parentheses):

- Selection of general area based upon the factors listed above (SGS);
- Preliminary interference research (Construction Layout);
- Ground penetrating radar survey (Operations Department);
- Preparation of work package (SGS);
- Work Process Control (Operations Department); and
- Field survey (Construction Layout).

This detailed site clearance routine was essential for safe field operations. Any obstacles or restrictions encountered in any step during this process required the relocation of the proposed boring or sounding location, and therefore the re-initiation of the process.

2.2 Equipment and Field Test Methods

All equipment used in the field investigations met applicable ASTM standards and site standards and procedures as listed below:

- WSRC E9 SGS-GT-202 - Drilling Practices;
- WSRC E9 SGS-GT-203 - Sample Preparation, Handling and Storage;
- WSRC E9 SGS-GT-206 - Engineering Soil Descriptions;
- WSRC E9 SGS-GT-207 - Field Log Preparation;
- WSRC E9 SGS-GT-210 - Standard Penetration Test;

- WSRC E9 SGS-GT-211 - Cone Penetration Test Soundings;and
- WSRC 3Q5 Manual - Hydrogeologic Data Collection.

2.2.1 Exploration Contractor(s) and Equipment

One drilling contractor was utilized for the borings, SPT testing and undisturbed soil sampling (Shelby tubes) and one contractor was used for all SCPT soundings. A description of the scope of each contractor and the equipment used is provided below.

Graves Environmental, Inc.

Graves Environmental, Inc., of Jackson, South Carolina performed the drilling and sampling for SPT and undisturbed borings. All Graves Environmental drillers involved with the drilling and sampling activities were experienced with geotechnical investigations and performed the drilling and sampling for the APSF investigation. The drilling equipment utilized is described below.

Failing 1500

The Failing 1500 drill rig is gas-driven with a 40-foot mast. The rig has a 23-foot Kelly assembly which allows for a 20-foot stroke and is capable of mud rotary, augering, and rotary coring techniques. The drill string is controlled by the Kelly arrangement, as well as, by a mechanical winch. This rig was used for all deeper borings requiring mud rotary.

Applied Research Associates (ARA)

Applied Research Associates (ARA) of Royalton, Vermont performed all CPT field and data processing activities including the CPT soundings completed for the APSF investigation. The CPT rig used for this investigation is described below.

Mac I

The Mac I CPT rig is a 22 ton rig capable of 30 ton mass push when fully ballasted. The push rod and piezocone utilized conformed with ASTM D5778 (ASTM 1995) consistent with WSRC E9 SGS-GT-211 - Cone Penetration Test Soundings. This rig was equipped with a hydraulic skid coupled to the surface beneath the rig for generating a shear wave source. Compressional waves were generated with a hydraulic vertical hammer located on the outside of the rig. All components were controlled by the operator.

2.2.2 Standard Penetration Test (SPT)

Tests were performed in accordance with WSRC E9 SGS-GT-210 using a standard 24-inch long by 2-inch outside diameter (OD), split-spoon sampler with a 2-foot bleeder and check valve located above the sampler, NX drill stem, and a 140-lb safety hammer falling 30 inches. SPT N-values were determined by adding the number of blows required to drive the split-spoon sampler the last 12 inches of the standard 18-inch drive.

The general test procedure, as noted in sequence, is outlined below:

1. Split spoon is lowered into nominal 4-inch diameter borehole;
2. Depth is checked and any rod settlement noted;
3. Six-inch intervals, totaling 18 inches, are marked on the drill rod above the turntable;
4. Sampler is driven by blows applied using a 30-inch stroke with the rope wrapped twice over the cathead;
5. Sampler retrieved and recovery noted;
6. Sampled interval reamed and drilled out to next sample interval; and
7. Process repeated.

Prior to each SPT test, the Geotechnical Oversight professional verified that the spoon was properly assembled, making sure the bleeder and check valve were clean and the drive shoe was in good condition.

2.2.3 Undisturbed Sampling

All undisturbed soil samples referenced within this report pertain to samples collected for the APSF investigation. No undisturbed samples were obtained specifically from the northeast expansion area.

Undisturbed (UD) soil samples were obtained for laboratory testing with direct push shelby tubes. The shelby tubes used were either brass or galvanized steel with a 3 inch OD, 0.065 inch wall thickness, and a length of 30 inches. Sampling was performed in accordance with ASTM D1587 (ASTM 1996).

The selection of the sampling interval was based on the results of previously pushed CPT soundings and/or SPT borings located within 10 to 15 feet from the UD boring. Prior to sampling, a sampling plan was developed for each UD boring. Generally, sampling was performed on the more cohesive soil layers encountered.

Drilling requirements for undisturbed sampling boreholes required that fluid pressures be kept as low as practical, while maintaining fluid return up the borehole. Drill bits with side discharge, or, in the case of tricone bits, with bottom deflectors, were required for reaming and advancing the borehole. Drilling was accomplished by mud rotary methods to the predetermined sampling depth. The drill stem was then tripped out and the bit removed. The Shelby tube head with a ball check valve was then attached and lowered to the bottom of the borehole. Borehole depth was checked against the drilled depth and noted. The maximum push length was marked on the drill stem and the rod hydraulically advanced a full 24 inches or until 600 psi hydraulic pressure was reached. Once the advance was made, the tube was allowed to sit for 5 minutes. When ready to retrieve the sample, the drill string was rotated about 90 degrees to shear the sample off the surrounding soil. When each sample was brought out of the borehole, the bottom and top were capped with plastic slip-on caps. If a gap was noted between the bottom tube edge and sample, a filler material was placed in the gap prior to placing the cap. Details of final sample preparation are provided in Section 2.3.

2.2.4 Piezocone Penetration Soundings

CPTU, including seismic (SCPTU) soundings, were performed in accordance with ASTM D5778. The CPT was used because of the relatively quick and clean operation and its ability to provide a continuous soil profile for determining stratigraphy and defining the extent of soft and/or loose soil zones. In general, all CPT soundings included shear wave velocity surveys at 3-foot intervals. Target depths were based upon the estimated elevation of the top of the Congaree formation (approximately El. 130 feet MSL), a dense sandy layer (see section 3) that is considered incompressible. However, actual depths varied, depending upon ground surface elevations and subsurface conditions.

2.2.5 Borehole and Penetration Abandonment

Abandonment of borings and soundings was performed per WSRC Manual 3Q5, Hydrogeologic Data Collection (WSRC, 1992). The standard grout mix consisted of the following:

- One sack Type 1 Portland Cement (94 lb sack);
- Two pounds of dry sodium bentonite; and
- 6.5 to 7.5 gallons of potable water.

All borings were abandoned immediately upon completion of testing. Grouting was accomplished via the tremie method. The grout pipe was lowered to the bottom of the boring and grout was injected until the boring fluid was displaced and grout returned to the surface. All borings were grouted to the surface and topped off until the column remained static.

Cone penetrometer soundings were abandoned by pressure grouting through a push rod which was re-pushed down to the bottom of the sounding. A grout tube extending to the bottom of the push rod was used to pump grout into the hole as the push rod was retracted. Holes were topped off until the column remained static.

2.3 Sample Preparation, Handling, Storage, Transportation, and Control

Samples were prepared and handled in accordance with WSRC E9 SGS-GT-203 - Sample Preparation, Handling and Storage. Shelby tubes were checked for conformance with ASTM D1587-83 (ASTM 1996).

The undisturbed samples were maintained in vertical tube boxes capable of holding four tubes as prescribed by ASTM D 4220 (ASTM 1996). Once the samples were obtained, the samples were trimmed, measured, and sealed. Plastic caps were placed over both ends of each tube, then taped and each tube labeled. For SPT borings, a sample was collected from the top and bottom of the sample spoon. If a material change occurred within the sample, additional samples were collected, as appropriate. Samples were placed in 8-ounce glass jars. The tops were closed tightly, wrapped, sealed with electrical tape, and samples were labeled on both the jar and the lid. Prior to sample turnover to Law Engineering, all samples were stored in accordance with WSRC E9 Procedure SGS-GT-203.

All soil samples selected for testing were turned over to Law Engineering for transporting to their laboratory in Atlanta. All tube samples tested by Law Engineering in Atlanta were transported in tube boxes and were maintained in a vertical position. Once in Atlanta, the samples were maintained in a controlled area according to the Law Engineering Quality Assurance Program.

3. SUBSURFACE CONDITIONS

Information obtained from the field exploration program has been used to characterize the subsurface (surface to about 180 feet in depth) conditions in the F-Area northeast expansion area. This included establishing the engineering stratigraphy and soil index properties and making a direct comparison with the adjacent APSF subsurface soils. Further, the presence of soft sediments defined as zones with measured tip resistances less than 15 tons per square foot (tsf) or SPT N-values of 5 or less, were evaluated. Groundwater conditions were determined from nearby monitoring well information. Eight subsurface cross-sections (Figures 3.0-1 through 3.0-8) were developed to show the engineering stratigraphy across the northeast expansion area, as well as, the APSF area. These subsurface sections are based on presently available information. Some variation from these conditions can be expected.

3.1 Engineering Stratigraphy

The subsurface engineering stratigraphy was determined from CPT measurements including tip resistance, sleeve resistance, friction ratio, and pore pressure signatures, shear wave velocity as well as correlations with adjacent soil boring data. The layering system is based on observed changes in the CPT measurements that are correlatable between soundings and nearby borings. The layer nomenclature was developed for mapping subsurface units across various parts of the SRS. For specific application to the APSF and F-Area northeast expansion area, it is only used to differentiate units based on similar engineering characteristics that can be mapped in the investigation area. Typical stratigraphic layering is shown on Figure 3.1-1.

The layer nomenclature follows an alphanumeric system with layer numbers increasing from top to bottom. Subdivided layers are identified with a letter designation (e.g., TR1A). Some layer boundaries correspond to geologic formations. Some upper portion of the layers TR1 and TR1A layers are most probably the Altamaha formation overlying the Tobacco Road formation, however, due to the similar material properties and an irregular erosional surface which separates these units, differentiating them is difficult. In some parts of the F-Area, the TR1 and TR2 layers have been subdivided to recognize sublayers with distinct soil properties (TR1A, TR2A, and TR2B). As described in the F-

Area Geotechnical Characterization Report (WSRC, 1996c), the TR3/4 layer was first correlated to the lower portion of the Tobacco Road formation but based on more recent geologic investigations in the area has been reassigned to the upper portion of the Dry Branch formation. Layers DB1 through DB3 were combined into a DB1/3 layer because of similar properties. Likewise, layers DB4 and DB5 were combined into a DB4/5 layer. The DB1/3 layer corresponds to the Dry Branch formation while the DB4/5 layer corresponds to the upper Santee/Tinker formation. The Santee/Tinker formation, is the most variable layer in the shallow subsurface. It has been further subdivided into the ST1 and ST2 layer where practical. The green clay, which is an informal stratigraphic interval at the SRS, is considered the basal unit for the shallow engineering stratigraphy and is labeled as GC. This geologic unit is locally continuous and provides a reliable marker bed. The Green Clay overlays the Congaree Formation which is predominantly dense silty sands and where applicable is labeled CG.

The following sections describe the physical attributes used to delineate each layer, as well as, depositional environment and lithologic variability with the exception of the CG layer.

3.1.1 TR1 and TR1A Layers

The TR1 and TR1A layers are most probably the Altamaha formation consisting of red, purple and brown poorly sorted sands ranging from fine to gravel size with the dominant soil classification being clayey to silty sands (SC to SM). The depositional environment of these sediments is characterized as high energy fluvial such as river and stream channels. The base of the Altamaha is distinguished by an irregular erosional surface and can reach thicknesses of up to 70 feet at the SRS. This layer ranges in thickness from roughly 7 feet to nearly 36 feet thick. The TR1 layer is characterized by moderate SCPTU tip resistances and relatively high friction ratios while the underlying TR1A layer is generally less dense (lower tip resistances).

3.1.2 TR2A and TR2B Layers

The TR2A and TR2B layers have been used to differentiate the Tobacco Road formation. Sediments of the Tobacco Road formation were deposited in low energy shallow marine transitional environments such as tidal flats. Much of the sediments are laminated or otherwise bioturbated (mixed by burrowing

organisms after deposition) red, purple and brown poorly sorted sands and clayey sands.

The TR2A, and TR2B layers are predominantly sands and clayey sands (SP-SM to SP-SC) as determined by laboratory classification tests. The TR2B layer is distinguished from the overlying TR1A layer by increased tip resistance and notably lower sleeve friction values resulting in a lower friction ratio. The TR2A ranges from about 20 feet to 40 feet thick while the TR2B layer ranges from about 10 to 27 feet thick.

3.1.3 TR3/4 and DB1/3 Layers

The Dry Branch Formation consists of sands and clays deposited in a transitional sequence between near shore and bay or lagoon environments. The upper contact of the TR3/4 layer is defined by a marked decrease in CPT tip resistance and an increase in both the friction ratio and pore pressure measurements. As determined by laboratory classification tests, the TR3/4 layer is predominantly clays and sandy clays (SC). Thickness of this layer ranges from around 5 feet to around 10 feet thick over the balance of the area.

The DB1/3 layers correspond to the Irwinton Sands. On the CPT logs, the DB1/3 layer is a zone of variable, but generally high, CPT tip resistances and low friction ratios. In general, pore pressures are low or slightly above hydrostatic. The dominant unified soil classification for the DB1/3 is SP-SM with minor layers of CL material occurring as laminations. Thickness of this layer ranges from 25 to 35 feet thick.

3.1.4 DB4/5, ST1 and ST2 layers

The Santee/Tinker Formations represent the most complex geologic unit in the shallow subsurface of F-Area. It is depositionally complex and highly variable in both its lithology and material properties. Soils in the Santee/Tinker range from sands to silty sands (SP-SM). The contact between the Santee/Tinker Formation and the overlying Dry Branch Formation is generally seen on the CPT logs as a sharp decrease in the pore pressure measurement. This layer is characterized by thin, alternating layers of low and high CPT tip resistances and friction ratios. Characteristically, CPT soundings in this layer show a pronounced sawtooth trace with large variations over relatively small vertical intervals. This

highly variable pattern suggests interfingering of alternating lenses of clayey and silty sands with more resistant, silica-cemented sediments and less resistant, calcareous sediments, and appears to be a result of rapid lateral and vertical changes in the nature of the materials originally deposited in this interval. The unit consists of complex sequences of limestones, carbonate muds, carbonate sands, and muddy sands.

The soils of the DB4/5 interval are much more plastic than the overlying Irwinton Sand (DB1/DB3) and the underlying ST1 layer. Soils of the DB4/5 typically classify as SM to CL materials. The DB4/5 layer has moderate to low tip resistances and moderate friction ratios. The DB4/5 layer has been subject to extensive characterization within the APSF area because of observed soft zones (tip resistances less than 15 tsf and N-values of 5 or less). The thickness of this layer ranges from about 6 to 10 feet thick. The ST1 layer is characterized by higher tip resistances than the overlying DB4/5 layer underlying ST2 layer. Although not all soundings penetrated this layer, the ST1 layer ranges in thickness from about 13 to 22 feet. Soils of the ST2 layer are generally characterized by lower tip resistances and sleeve resistances than the overlying ST1 layer. Soils of the ST1 and ST2 layers generally classify and SM to SP-SM materials. Based on the number of soundings that fully penetrated the ST2 layer, the thickness ranges from about 6 to 14 feet thick.

In F-Area and elsewhere at SRS, the Santee/Tinker formation has been a primary focus of foundation investigations. In fact, nearly all foundation remediation programs have targeted this unit because of drilling problems such as lost drill fluid circulation or rod drops.

3.1.5 GC Layer

The "green clay" (GC) is an informal stratigraphic name at SRS for stiff, green to gray clays, silts, and clayey sands that are commonly found at the base of the Santee/Tinker Formation. In general, these soils classify as SM to ML with varying amounts of clay. This layer is locally continuous at F-Area and has been used to define the lower boundary of the shallow stratigraphy. Layer elevations and thicknesses have been determined from those borings and soundings that penetrate this layer. Most borings and CPT soundings do not reach or penetrate the GC layer. The top of the layer ranges from around El. 126 feet MSL in the

south and northwestern portions of the area to a high of around 140 feet MSL in the east-central part of the area. This is consistent with the correlating Gordon Confining Unit as mapped by Aadland (1995) which is correlatable to the "green clay" unit.

3.2 Groundwater Conditions

Groundwater data was derived from water table monitoring wells located in various locations around and within F-Area as well as from WSRC-TR-98-00045, The Regional Water Table of the Savannah River Site and Related Coverages. Monitoring well NBG-5 is the nearest to the F-Area northeast expansion area (see Figure 1.0-2). As shown on Figure 3.2-1, the water table elevation from May, 1986 to May, 1997, ranges from about El. 214 feet MSL to about El. 221 MSL. The dominant water table gradient in the F-Area expansion area is oriented to the north and is largely controlled by Upper Three Runs Creek located immediately north of the F-Area. Groundwater Table contours are shown on Figure 2.0-1.

3.3 Engineering Soil Characteristics

The engineering stratigraphy of the F-Area northeast expansion area was correlated with the APSF area and the engineering characteristics compared. This comparison was based on layer continuity and thickness, as well as, measured SPT N-values, CPT measurements and laboratory soil classification data. Layer continuity across the area is shown on eight subsurface cross-sections (Figures 3.0-1 through 3.0.8).

A summary of all CPT engineering layer picks for the F-Area northeast expansion area and APSF is given on Table 3.3-1, along with a generalized (average) stratigraphy for the area. Figures 3.3-1 through 3.3-3 show mean and standard deviations of shear wave velocities (V_s), CPT tip resistance (q_t) and CPT friction ratio (R_f) with the generalized average engineering stratigraphy. SPT N-values are plotted on Figure 3.3-4 to show the range of values versus elevation, as well as, the generalized engineering stratigraphy. Laboratory test results are included in Appendix C.

A comparison of engineering soil properties between the F-Area northeast expansion area, APSF and the balance of F-Area is summarized in Table 3.3-2. As can be seen, the tabulated properties are very similar for all three areas.

3.3.1 Soft Zone Characteristics

Weight of rod and occasional rod drops have been described in numerous drilling reports for monitoring wells and geotechnical borings located in the central part of the SRS. Early subsurface investigations performed by the United States Army Corps of Engineers (COE) frequently described these zones as soft zones, or even voids, and numerous subsequent subsurface investigations have described these same conditions at the SRS. These soft zones typically occur in the carbonate-bearing sediments of the Santee Limestone, Utley Limestone, and the Griffins Landing Member of the lower Dry Branch Formation. The prevailing assumption about the origin of these soft zones is dissolution of carbonate-rich, clastic sediments, resulting in vugular porosity (open pore space). When drilling these zones, the drill rod meets little shear resistance and drops (COE, 1951). However, much of the time, recovery of soil in the sampler precludes the zone from being characterized as a void.

Soft zones are defined by SPT-N values ≤ 5 or CPT tip resistance ≤ 15 tsf. Also, thickness should be considered as well. These zones are generally restricted to the lower Dry Branch Formation and the Santee/Tinker Formation. However, soft zones can be found in other horizons at the SRS. The following sections discuss soft zones found in the northeast expansion and APSF areas.

Soft zones in two different horizons were identified on CPT sounding profiles. Two CPT soundings in the APSF area also had soft zone hits at about El. 240 feet MSL however these were considered to be isolated. Nineteen CPT soundings and five SPT borings had tip resistances and N-values meeting soft zone criteria however thirteen of these CPT soundings were pushed to delineate a soft zone within the APSF area (Figure 3.3-1). Four of the remaining six represented isolated hits in the APSF area and the other two were within the northeast expansion area. Of the five SPT borings, one was located in the APSF area while the other four were located in the northeast expansion area. Soft zone intervals noted on all CPT soundings and SPT borings are summarized on Table 3.3-3.

3.3.1.1 *Northeast Expansion Area Soft Zones*

Soft zone intervals were noted in only two of the soundings pushed for the F-Area northeast expansion investigation (soundings 103 and 157). Sounding 103 had a soft zone between El. 177.6 and El. 171.2 feet MSL with a cumulative soft zone thickness in this interval of about 2.9 feet thick. Revisions made to the boundary of the northeast expansion area placed sounding 103 outside of the investigation area, therefore this area was not investigated further (see Figure 2.0-1). SPT boring FB-19 had measured soft zones from El. 189.6 to about El. 186.6 feet MSL. CPT Sounding 114 was pushed adjacent to FB-19 (prior to FB-19 being drilled) with no measured soft zones.

Sounding 157 had two soft zone intervals. The upper most interval was between El. 215.1 and 211.2 feet MSL with a cumulative soft zone thickness of about 3.4 feet thick. The lower soft zone interval was between El. 179.8 and El. 173.5 feet MSL with a cumulative soft zone thickness of about 6.2 feet thick. SPT boring FB-17 was drilled about ten feet away from sounding 157. The upper soft zone interval between El. 215.1 and 211.2 feet MSL was not encountered in the SPT boring. In fact, N-values of about 10 were measured through this interval. The lower soft zone interval was encountered in FB-17. An interval of low blow counts, including weight of rod, corresponding to the lower interval measured in sounding 157 was encountered between El. 183.1 to El. 176.1 feet MSL. In SPT boring FB-17, a lower interval with low blow counts and weight of rods was also encountered from El. 157.6 to 151.6 feet MSL. This interval corresponds to a low tip resistance interval in CPT sounding 157 however tip resistances are higher than 15 tsf (about 20 tsf).

Additional indications of soft zones were noted in SPT boring FB-20 and FB-20A. FB-20 was drilled adjacent to sounding 179 for the purpose of obtaining a paired sounding and SPT boring. At about El. 179, a weight of rod over 37 inches was measured in boring FB-20. Drill fluid circulation was lost and the hole was abandoned. FB-20A was drilled adjacent to FB-20 and the interval from El. 194.9 to El. 181.4 was sampled with measured N-values greater than 20. At El. 181.4 however, circulation was lost in FB-20A and the hole was abandoned. These intervals in FB-20 and FB-20A correspond to a low tip resistance interval in sounding 179 which has thin layers of tip resistances measuring less than 15 tsf but have a cumulative thickness less than 2 feet.

3.3.1.2 APSF Area Soft Zones

The APSF area was extensively investigated for the extent and thickness of soft zones. Boring FB-1 located in the APSF area had low SPT N-values and weight of rod drop from El. 155.5 feet MSL to about El. 146.6 feet MSL. Soils from this interval were described as tan very fine silty and clayey sand with traces of shell fragments. Further investigation around this area with CPT soundings provided a reasonable mapped geometry of this soft zone. As shown on Figure 3.1-1, the soft zone actually occurred in two distinct horizons. The lowermost horizon within the Santee formation was delineated as shown on Figure 3.3-5. The upper Santee formation soft zone as shown on Figure 3.3-6 was not as continuous as the lower zone. Based on this investigative work it was reasonable to estimate the size, thickness and stratigraphic position of these soft zone intervals. SPT boring FB-4 in the APSF area also had a noted soft zone with low blow counts and weight of rod measurements from about El. 177.8 to 172.1 feet MSL.

3.3.1.3 Soft Zone Samples and Analysis

Four CPT tubes were taken from the soft zones identified in the APSF area. From these tubes the following laboratory tests were performed to identify soil parameters:

- 5 unit weight and moisture content tests
- 2 consolidation tests
- 5 Atterberg Limit tests
- 7 hydrometer suites including grain size analysis
- 1 unconfined compression test

Additional CPT samples were acquired from the lower soft zone interval in the vicinity of sounding 157 for the purpose of measuring the unit weight and moisture content for comparison with results from the APSF investigation. Results of the unit weights and moisture content were consistent with those made for the APSF area samples. The results of these tests are provided in Appendix B and are summarized on Table 3.3-5. It is important to note that the sample size for this type of sampler is smaller than that obtained from a Shelby tube or other standard sampler. Arguments can be made that due to the smaller

diameter (1.75 inches compared to 3 inches for a shelby tube), disturbances will be greater. Additionally there is no precedent or standardized procedure for testing these smaller size samples. Therefore, the consolidation and strength results are solely for interpretation and engineering judgement and may not be representative of the in-situ properties of the soils tested.

4. GEOTECHNICAL AND FOUNDATION ASSESSMENT

The conditions encountered during this program are not unlike conditions found elsewhere at the SRS. In fact, they are very similar in terms of:

- Geology and soil classification;
- SPT N-values;
- CPT resistances;
- Shear wave velocity; and
- Presence of soft zones.

Figures 3.3-1 through 3.3-4 show the range of SPT N-values, the mean and range of CPT corrected tip resistance (q_t), CPT friction ratio (R_f) and shear wave velocity (V_s) from this investigation. Based on these results, the soils encountered can support structure and foundation loads currently constructed at the SRS with no adverse consequences. Typical foundation loading for existing critical facilities at the SRS is in the range of 4 to 7 kips per square foot (ksf). Higher loads could be supported depending on the layout, geometry and foundation depths of the proposed facilities and the results of a structure specific geotechnical investigation program, which is required for the proposed new mission facilities.

The regional water table is approximately 60 to 70 feet below the ground surface, however, perched water should be expected. Construction cut slopes for the nearby APSF excavation have been stable since excavation in August and September of 1998, with slopes of one horizontal to one vertical. The slopes were benched and protected with a thin layer of emulsified asphalt. Heave markers placed within the excavation footprint measured heave between one-half to one inch for the 30 to 35 feet deep excavation. Thus, from a construction standpoint, there appears to be no apparent unusual issues.

5. CONCLUSIONS AND RECOMMENDATIONS

The shallow stratigraphy and average engineering properties determined for the F-Area northeast expansion are directly comparable to those determined for the Actinide Packaging and Storage Facility (APSF) area as well as the balance of the F-Area. Geologic conditions are also directly comparable between these two areas.

Soft zone intervals detected in the F-Area northeast expansion area are consistent with soft zone sediments encountered at the APSF area. Siting and design of new facilities in this area should account for the presence of these soils either by avoiding the placement of critical facilities where these zones are known to exist, or determining the potential settlement and designing the facility to accommodate the estimated movement. A thorough review of the data included in this report is recommended for planning further investigations.

Design and construction of new PC-3 and higher facilities, heavily loaded structures or capital investment projects in the F-Area northeast expansion area should not require extensive geotechnical characterization. However, structure specific investigations for foundation design and construction, as well as, proper characterization of soft zone intervals are required. Foundation specific investigations should consider structure size, geometry, foundation type and depth, performance classification and functional classification, etc. A limited program of field testing to confirm dynamic soil properties may be required to obtain baseline subsurface information such that a site-specific comparison with results of this investigation can be made.

Heave monitoring is required for excavations greater than ten feet deep. Settlement monitoring is required for all major and/or critical new facilities throughout the construction phase until final turn-over or when operations commence. After operations commence, settlement monitoring is required on an established interval. Settlement results should be compiled and reviewed by competent geotechnical and structural engineers.

New critical facilities should consider seismic instrumentation in the structure design and facility operation. An SRS Engineering Standard for seismic

instrumentation is currently under development. This standard will provide specifications for seismic instrumentation installation and performance.

6. REFERENCES

- Aadland, R. K., Gellici, J. A., and Thayer, P. A. (1995), "Hydrogeologic Framework of West-Central South Carolina," S. C. Department of Natural Resources, Water Resources Division, Report 5.
- Annual Book of ASTM Standards (1996), American Society for Testing and Materials.
- WSRC (1992), "Procedure Manual 3Q5, Hydrogeologic Data Collection"
- WSRC (1995) "In-Tank Precipitation (ITP) and H-Tank Farm (HTF) Geotechnical Report," WSRC-TR-95-0057, Rev. 0, September.
- WSRC (1996a), "Procedure Manual E7, Conduct of Engineering and Technical Support."
- WSRC (1996b), "Procedure Manual E9, Site Geotechnical Services."
- WSRC (1996c), "F-Area Geotechnical Characterization Report, WSRC-TR-96-0069, Rev. 0, September 1996.
- WSRC (1998a), " APSF Confirmatory Drilling Program Results, PECD-SGS-98-0115, June, 1998.
- WSRC (1998b)," The Regional Water Table of the Savannah River Site and Related Coverages" WSRC-TR-98-00045, September 1998.

Layer Elevation (ft above MSL)											
	CPT-1	CPT-2	CPT-4	CPT-5	CPT-6	CPT-7	CPT-8	CPT-9	CPT-10	CPT-11	CPT-12
SURFACE	290.50	290.50	292.18	291.29	289.66	282.74	285.08	285.30	287.16	288.59	288.89
TR1	290.50	290.50	292.18	291.29	289.66	282.74	285.08	285.30	287.16	288.59	288.89
TR1A	273.01	274.96	277.99	272.89	276.94	275.88	273.16	278.92	273.80	274.94	271.70
TR2A	258.98	255.83	260.97	259.95	262.96	262.94	260.97	261.90	261.21	253.89	254.91
TR2B	231.98	228.94	230.92	229.95	234.01	237.87	233.56	234.96	234.15	234.93	236.93
TR3/TR4	209.01	203.45	210.00	209.93	210.99	213.87	209.04	211.89	209.94	211.90	215.29
DB1/DB3	203.02	200.03	206.00	201.94	204.00	203.90	203.27	200.90	205.27	207.95	208.94
DB4/DB5	172.02	176.62	178.00	175.98	175.96	176.93	173.37	172.90	172.98	176.90	177.22
ST1	164.03	169.86	176.00	169.99	169.92	171.88	168.90	167.96	168.50	168.96	171.14
ST2	151.96	152.01	151.99	152.93	151.99	152.88	149.90	149.90	151.88	151.89	149.00
GC	141.93	139.24	-----	142.94	142.01	142.91	138.87	140.01	136.64	145.89	142.04
CG	132.99	133.11	-----	136.00	133.93	133.88	132.72	-----	132.22	141.90	-----
BASE	130.50	132.50	148.17	134.29	132.66	132.74	129.08	139.30	130.16	138.59	137.89
Layer Thickness (ft)											
	CPT-1	CPT-2	CPT-4	CPT-5	CPT-6	CPT-7	CPT-8	CPT-9	CPT-10	CPT-11	CPT-12
TR1	17.49	15.54	14.19	18.40	12.72	6.86	11.92	6.38	13.36	13.65	17.19
TR1A	14.03	19.13	17.02	12.94	13.98	12.94	12.19	17.02	12.59	21.05	16.79
TR2A	27.00	26.89	30.05	30.00	28.95	25.07	27.41	26.94	27.06	18.96	17.98
TR2B	22.97	25.49	20.92	20.02	23.02	24.00	24.52	23.07	24.21	23.03	21.64
TR3/TR4	5.99	3.42	4.00	7.99	6.99	9.97	5.77	10.99	4.67	3.95	6.35
DB1/DB3	31.00	23.41	28.00	25.96	28.04	26.97	29.90	28.00	32.29	31.05	31.72
DB4/DB5	7.99	6.76	2.00	5.99	6.04	5.05	4.47	4.94	4.48	7.94	6.08
ST1	12.07	17.85	24.01	17.06	17.93	19.00	19.00	18.06	16.62	17.07	22.14
ST2	10.03	12.77	-----	9.99	9.98	9.97	11.03	9.89	15.24	6.00	6.96
GC	8.94	6.13	-----	6.94	8.08	9.03	6.15	-----	4.42	3.99	-----
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1A Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-13	CPT-14	CPT-15	CPT-16	CPT-17	CPT-18	CPT-19	CPT-20	CPT-21	CPT-22	CPT-22A
SURFACE	289.79	289.70	295.13	283.80	287.10	293.00	293.10	291.20	291.70	290.60	291.40
TR1	289.79	289.70	295.13	283.80	287.10	293.00	293.10	291.20	291.70	290.60	291.40
TR1A	280.02	271.70	277.94	270.65	271.95	278.03	278.00	279.99	279.08	276.70	274.98
TR2A	264.96	254.91	257.97	259.93	259.00	264.07	260.11	262.01	261.03	258.50	258.03
TR2B	238.91	236.93	222.93	232.86	230.99	236.99	233.04	234.00	235.03	230.02	227.96
TR3/TR4	216.91	215.29	197.97	209.97	211.95	213.98	211.94	210.98	211.95	204.85	199.01
DB1/DB3	208.98	208.94	195.02	203.15	202.90	208.00	204.04	204.96	205.00	200.90	196.02
DB4/DB5	176.93	177.22	172.01	174.93	176.00	179.00	176.03	178.01	179.04	176.80	170.02
ST1	171.93	171.14	163.02	169.00	169.99	172.98	170.95	173.96	171.04	168.73	167.03
ST2	152.91	149.00	149.00	149.16	153.96	151.03	150.92	151.97	155.00	152.27	149.95
GC	139.93	142.04	137.97	137.43	143.01	137.03	138.96	138.00	139.00	140.17	137.01
CG	132.99	133.07	131.98	132.90	135.98	130.96	-----	-----	133.02	-----	-----
BASE	129.79	102.70	128.13	129.80	133.10	129.48	130.18	129.54	130.96	138.96	130.88
Layer Thickness (ft)											
	CPT-13	CPT-14	CPT-15	CPT-16	CPT-17	CPT-18	CPT-19	CPT-20	CPT-21	CPT-22	CPT-22A
TR1	9.77	18.00	17.19	13.15	15.15	14.97	15.10	11.21	12.62	13.90	16.42
TR1A	15.06	16.79	19.97	10.72	12.95	13.96	17.89	17.98	18.05	18.20	16.95
TR2A	26.05	17.98	35.04	27.07	28.01	27.08	27.07	28.01	26.00	28.48	30.07
TR2B	22.00	21.64	24.96	22.89	19.04	23.01	21.10	23.02	23.08	25.17	28.95
TR3/TR4	7.93	6.35	2.95	6.82	9.05	5.98	7.90	6.02	6.95	3.95	2.99
DB1/DB3	32.05	31.72	23.01	28.22	26.90	29.00	28.01	26.95	25.96	24.10	26.00
DB4/DB5	5.00	6.08	8.99	5.93	6.01	6.02	5.08	4.05	8.00	8.07	2.99
ST1	19.02	22.14	14.02	19.84	16.03	21.95	20.03	21.99	16.04	16.46	17.08
ST2	12.98	6.96	11.03	11.73	10.95	14.00	11.96	13.97	16.00	12.10	12.94
GC	6.94	8.97	5.99	4.53	7.03	6.07	8.78	8.46	5.98	-----	6.13
CG	-----	30.37	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1B Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-23	CPT-24	CPT-25	CPT-26	CPT-27	CPT-28	CPT-29	CPT-30	CPT-31	CPT-32	CPT-32A
SURFACE	290.90	290.00	290.10	289.50	290.30	289.80	288.10	288.80	290.70	289.50	288.00
TR1	290.90	290.00	290.10	289.50	290.30	289.80	-----	288.80	290.70	289.50	288.00
TR1A	274.94	277.04	272.99	278.99	277.01	280.97	288.10	275.36	274.95	276.94	280.02
TR2A	258.03	261.99	255.97	265.00	259.04	263.96	265.97	267.01	257.94	267.75	265.00
TR2B	229.97	233.96	230.00	239.04	232.01	236.94	239.09	240.08	230.00	239.98	238.02
TR3/TR4	209.02	207.03	207.02	217.01	205.04	213.03	217.95	215.92	206.98	217.10	213.88
DB1/DB3	204.02	201.96	201.02	210.00	199.99	207.96	212.03	207.98	201.02	212.97	209.96
DB4/DB5	177.00	174.94	176.09	179.05	174.01	176.94	181.04	180.01	176.05	182.10	181.03
ST1	171.95	167.97	170.04	172.88	167.96	171.00	175.00	174.02	172.05	178.10	176.99
ST2	150.01	147.04	153.98	152.12	150.13	154.03	150.94	155.98	148.98	153.94	154.99
GC	137.94	134.99	139.97	143.03	137.03	138.03	144.98	146.00	135.97	140.89	139.94
CG	133.19	-----	133.05	-----	131.98	-----	140.03	-----	-----	-----	-----
BASE	131.92	128.31	132.04	139.34	131.73	131.05	138.91	143.33	131.96	133.51	133.86
Layer Thickness (ft)											
	CPT-23	CPT-24	CPT-25	CPT-26	CPT-27	CPT-28	CPT-29	CPT-30	CPT-31	CPT-32	CPT-32A
TR1	15.96	12.96	17.11	10.51	13.29	8.83	0.00	13.44	15.75	12.56	7.98
TR1A	16.91	15.05	17.02	13.99	17.97	17.01	22.13	8.35	17.01	9.19	15.02
TR2A	28.06	28.03	25.97	25.96	27.03	27.02	26.88	26.93	27.94	27.77	26.98
TR2B	20.95	26.93	22.98	22.03	26.97	23.91	21.14	24.16	23.02	22.88	24.14
TR3/TR4	5.00	5.07	6.00	7.01	5.05	5.07	5.92	7.94	5.96	4.13	3.92
DB1/DB3	27.02	27.02	24.93	30.95	25.98	31.02	30.99	27.97	24.97	30.87	28.93
DB4/DB5	5.05	6.97	6.05	6.17	6.05	5.94	6.04	5.99	4.00	4.00	4.04
ST1	21.94	20.93	16.06	20.76	17.83	16.97	24.06	18.04	23.07	24.16	22.00
ST2	12.07	12.05	14.01	9.09	13.10	16.00	5.96	9.98	13.01	13.05	15.05
GC	4.75	6.68	6.92	-----	5.05	6.98	4.95	-----	-----	7.38	6.08
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1C Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-33	CPT-34	CPT-35	CPT-36	CPT-37	CPT-38	CPT-39	CPT-40	CPT-41	CPT-42	CPT-43
SURFACE	290.10	290.30	290.00	291.70	292.00	290.00	290.00	286.60	289.50	287.50	289.00
TR1	290.10	290.30	290.00	291.70	292.00	290.00	290.00	-----	289.50	-----	289.00
TR1A	275.99	274.97	275.96	276.94	278.35	274.97	272.01	286.60	270.97	287.50	270.00
TR2A	260.02	256.01	262.98	263.01	259.99	256.01	254.92	266.04	252.97	264.02	250.91
TR2B	232.04	229.97	236.06	236.16	232.21	238.00	233.92	240.04	232.97	237.05	234.94
TR3/TR4	206.00	204.03	212.01	213.83	212.05	217.05	211.98	218.06	208.99	217.01	215.02
DB1/DB3	201.97	199.97	205.94	205.85	204.81	210.02	204.95	211.08	205.04	210.08	210.03
DB4/DB5	175.90	174.93	176.94	180.04	174.37	178.96	174.98	178.02	169.99	178.07	-----
ST1	169.99	169.03	171.00	172.61	171.70	175.01	167.96	173.96	166.96	173.00	-----
ST2	151.07	147.99	151.01	155.49	154.93	154.93	153.04	153.97	147.01	153.98	-----
GC	138.10	137.00	136.95	139.08	137.36	-----	139.07	141.03	133.99	148.95	-----
CG	133.00	-----	130.02	134.93	132.96	-----	-----	-----	-----	-----	-----
BASE	132.83	131.73	129.91	134.81	132.10	147.02	133.32	136.78	130.85	145.66	206.13
Layer Thickness (ft)											
	CPT-33	CPT-34	CPT-35	CPT-36	CPT-37	CPT-38	CPT-39	CPT-40	CPT-41	CPT-42	CPT-43
TR1	14.11	15.33	14.04	14.76	13.65	15.03	17.99	0.00	18.53	0.00	19.00
TR1A	15.97	18.96	12.98	13.93	18.36	18.96	17.09	20.56	18.00	23.48	19.09
TR2A	27.98	26.04	26.92	26.85	27.78	18.01	21.00	26.00	20.00	26.97	15.97
TR2B	25.06	25.94	24.05	22.33	20.16	20.95	21.94	21.98	23.98	20.04	19.92
TR3/TR4	5.01	4.06	6.07	7.98	7.24	7.03	7.03	6.98	3.95	6.93	4.99
DB1/DB3	26.07	25.04	29.00	25.81	30.44	31.06	29.97	33.06	35.05	32.01	-----
DB4/DB5	5.91	5.90	5.94	7.43	2.67	3.95	7.02	4.06	3.03	5.07	-----
ST1	18.92	21.04	19.99	17.12	16.77	20.08	14.92	19.99	19.95	19.02	-----
ST2	12.97	10.99	14.06	16.41	17.57	7.91	13.97	12.94	13.02	5.03	-----
GC	5.10	-----	6.93	4.15	4.40	-----	5.75	4.25	-----	3.29	-----
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1D Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-43A	CPT-44	CPT-45	CPT-46	CPT-48	CPT-51	CPT-52	CPT-53	CPT-103	CPT-104	CPT-113
SURFACE	289.00	285.50	287.40	286.00	285.80	286.70	288.80	289.90	271.70	274.20	276.70
TR1	289.00	285.50	-----	-----	-----	-----	288.80	289.90	-----	-----	-----
TR1A	267.99	271.89	287.40	286.00	285.80	286.70	269.89	276.77	271.70	274.20	276.70
TR2A	248.98	262.89	263.96	259.97	263.02	261.01	260.76	264.55	256.91	257.89	259.82
TR2B	234.93	236.97	238.05	235.00	235.95	235.94	236.80	235.74	238.88	240.95	236.01
TR3/TR4	213.95	217.93	217.04	213.01	212.96	215.04	216.81	215.48	209.90	211.89	210.03
DB1/DB3	208.95	207.94	210.02	204.02	205.03	204.97	209.98	209.12	199.94	199.96	203.85
DB4/DB5	-----	177.95	177.97	176.93	173.92	179.90	176.99	177.47	177.86	178.88	181.92
ST1	-----	171.87	172.99	171.03	166.94	172.92	172.98	173.78	167.91	171.87	172.08
ST2	-----	155.00	155.02	152.97	149.94	154.01	149.92	153.35	151.93	155.92	152.68
GC	-----	148.96	142.00	-----	138.01	138.01	139.04	143.64	142.93	147.92	138.73
CG	-----	145.01	-----	-----	-----	132.98	132.95	-----	132.89	-----	133.52
BASE	204.71	144.30	136.46	139.70	130.71	132.10	132.44	139.90	115.85	143.08	130.76
Layer Thickness (ft)											
	CPT-43A	CPT-44	CPT-45	CPT-46	CPT-48	CPT-51	CPT-52	CPT-53	CPT-103	CPT-104	CPT-113
TR1	21.01	13.61	0.00	0.00	0.00	0.00	18.91	13.13	0.00	0.00	0.00
TR1A	19.01	9.00	23.44	26.03	22.78	25.69	9.13	12.22	14.79	16.31	16.88
TR2A	14.05	25.92	25.91	24.97	27.07	25.07	23.96	28.81	18.03	16.94	23.81
TR2B	20.98	19.04	21.01	21.99	22.99	20.90	19.99	20.26	28.98	29.06	25.98
TR3/TR4	5.00	9.99	7.02	8.99	7.93	10.07	6.83	6.36	9.96	11.93	6.18
DB1/DB3	-----	29.99	32.05	27.09	31.11	25.07	32.99	31.65	22.08	21.08	21.93
DB4/DB5	-----	6.08	4.98	5.90	6.98	6.98	4.01	3.69	9.95	7.01	9.84
ST1	-----	16.87	17.97	18.06	17.00	18.91	23.06	20.43	15.98	15.95	19.40
ST2	-----	6.04	13.02	-----	11.93	16.00	10.88	9.71	9.00	8.00	13.95
GC	-----	3.95	5.54	-----	7.30	5.03	6.09	-----	10.04	4.84	5.21
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1E Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-114	CPT-115	CPT-116	CPT-126	CPT-127	CPT-128	CPT-129	CPT-143	CPT-147	CPT-148	CPT-149
SURFACE	273.60	272.00	270.10	278.90	279.30	279.30	277.20	286.10	284.80	284.10	274.80
TR1	273.60	272.00	-----	278.90	279.30	279.30	277.20	286.10	284.80	284.10	-----
TR1A	267.64	266.26	270.10	265.32	267.78	262.88	265.95	265.85	267.88	274.16	274.80
TR2A	259.83	260.15	258.03	258.49	259.91	250.83	253.90	246.90	252.67	252.56	256.92
TR2B	236.66	236.94	238.06	230.15	231.85	227.91	236.94	234.98	235.70	235.35	245.89
TR3/TR4	212.40	209.10	211.44	203.78	203.01	205.92	211.87	210.87	210.02	209.84	220.96
DB1/DB3	203.28	202.01	202.94	199.16	196.88	196.88	203.90	202.92	204.58	204.93	214.93
DB4/DB5	178.04	174.96	169.91	175.98	176.02	176.94	174.88	175.89	173.25	172.18	188.94
ST1	170.06	167.77	-----	164.29	165.36	167.91	167.96	165.93	164.98	161.78	179.96
ST2	149.81	148.01	149.95	147.13	148.53	148.94	149.95	152.94	148.88	149.57	158.92
GC	142.45	140.91	140.72	140.83	135.07	141.88	140.88	141.91	135.60	137.01	149.94
CG	136.15	133.47	-----	131.75	131.65	-----	131.89	131.91	129.73	128.58	136.90
BASE	128.02	127.98	137.51	125.93	114.60	132.04	129.12	130.02	127.34	126.62	131.92
Layer Thickness (ft)											
	CPT-114	CPT-115	CPT-116	CPT-126	CPT-127	CPT-128	CPT-129	CPT-143	CPT-147	CPT-148	CPT-149
TR1	5.96	5.74	0.00	13.58	11.52	16.42	11.25	20.25	16.92	9.94	0.00
TR1A	7.81	6.11	12.07	6.83	7.87	12.05	12.05	18.95	15.21	21.60	17.88
TR2A	23.17	23.21	19.97	28.34	28.06	22.92	16.96	11.92	16.97	17.21	11.03
TR2B	24.26	27.84	26.62	26.37	28.84	21.99	25.07	24.11	25.68	25.51	24.93
TR3/TR4	9.12	7.09	8.50	4.62	6.13	9.04	7.97	7.95	5.44	4.91	6.03
DB1/DB3	25.24	27.05	33.03	23.18	20.86	19.94	29.02	27.03	31.33	32.75	25.99
DB4/DB5	7.98	7.19	19.96	11.69	10.66	9.03	6.92	9.96	8.27	10.40	8.98
ST1	20.25	19.76	0.00	17.16	16.83	18.97	18.01	12.99	16.10	12.21	21.04
ST2	7.36	7.10	9.23	6.30	13.46	7.06	9.07	11.03	13.28	12.56	8.98
GC	6.30	7.44	-----	9.08	3.42	9.84	8.99	10.00	5.87	8.43	13.04
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1F Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-151	CPT-155	CPT-156	CPT-157	CPT-159	CPT-163	CPT-167	CPT-171	CPT-172	CPT-173	CPT-174
SURFACE	290.10	289.10	289.20	283.10	293.70	293.90	296.70	298.50	295.40	297.60	301.50
TR1	290.10	289.10	289.20	283.10	293.70	293.90	296.70	298.50	-----	297.60	301.50
TR1A	269.57	276.66	278.85	273.38	277.77	278.97	280.31	284.98	295.40	287.49	292.68
TR2A	254.70	260.61	261.87	260.20	259.97	265.96	265.38	269.96	265.99	262.56	269.97
TR2B	232.48	232.44	233.47	239.80	232.20	237.99	239.72	232.80	239.98	241.93	241.63
TR3/TR4	210.09	204.92	209.68	216.16	210.68	217.00	218.16	212.85	220.96	221.08	219.01
DB1/DB3	203.10	199.95	204.57	207.35	204.09	213.62	211.86	204.59	215.03	214.07	212.00
DB4/DB5	171.03	167.94	164.94	180.04	171.58	178.00	-----	180.05	189.03	186.01	184.19
ST1	165.12	163.61	163.07	173.37	168.37	174.53	-----	173.57	185.00	177.57	171.52
ST2	143.80	146.95	149.97	158.34	149.71	154.00	-----	-----	163.02	161.54	159.53
GC	138.42	135.35	135.67	145.43	138.55	140.99	-----	-----	149.00	153.66	143.58
CG	134.66	128.40	130.04	135.92	132.25	132.10	-----	-----	138.07	-----	135.63
BASE	134.14	126.05	128.15	130.92	129.13	128.87	181.49	166.43	133.28	147.41	130.12
Layer Thickness (ft)											
	CPT-151	CPT-155	CPT-156	CPT-157	CPT-159	CPT-163	CPT-167	CPT-171	CPT-172	CPT-173	CPT-174
TR1	20.53	12.44	10.35	9.72	15.93	14.93	16.39	13.52	0.00	10.11	8.82
TR1A	14.87	16.05	16.98	13.18	17.80	13.01	14.93	15.02	29.41	24.93	22.71
TR2A	22.22	28.17	28.40	20.40	27.77	27.97	25.66	37.16	26.01	20.63	28.34
TR2B	22.39	27.52	23.79	23.64	21.52	20.99	21.56	19.95	19.02	20.85	22.62
TR3/TR4	6.99	4.97	5.11	8.81	6.59	3.38	6.30	8.26	5.93	7.01	7.01
DB1/DB3	32.07	32.01	39.63	27.31	32.51	35.62	30.37	24.54	26.00	28.06	27.81
DB4/DB5	5.91	4.33	1.87	6.67	3.21	3.47	-----	6.48	4.03	8.44	12.67
ST1	21.32	16.66	13.10	15.03	18.66	20.53	-----	-----	21.98	16.03	11.99
ST2	5.38	11.60	14.30	12.91	11.16	13.01	-----	-----	14.02	7.88	15.95
GC	3.76	6.95	5.63	9.51	6.30	8.89	-----	-----	10.93	-----	7.95
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1G Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)											
	CPT-175	CPT-176	CPT-177	CPT-178	CPT-179	CPT-180	CPT-181	CPT-182	CPT-183	CPT-185	CPT-186
SURFACE	303.10	297.60	301.70	305.60	301.40	302.30	304.90	304.80	301.70	308.20	304.50
TR1	303.10	297.60	301.70	305.60	301.40	302.30	304.90	-----	301.70	308.20	304.50
TR1A	287.49	288.74	278.78	295.07	288.11	287.00	294.89	304.80	281.93	294.90	290.82
TR2A	280.12	267.97	267.07	272.77	276.49	245.96	271.29	278.00	272.72	271.45	267.44
TR2B	244.00	246.19	244.43	246.03	248.28	224.92	244.29	259.01	246.32	242.18	246.99
TR3/TR4	220.02	221.70	225.62	219.93	225.11	213.99	212.65	219.00	228.60	217.08	222.31
DB1/DB3	214.01	216.89	217.00	213.16	219.97	-----	202.88	208.97	220.00	202.02	211.96
DB4/DB5	184.17	187.83	186.07	186.21	192.24	184.02	178.85	183.96	190.87	181.06	184.01
ST1	173.01	179.91	179.84	173.30	188.46	-----	172.25	180.03	187.31	172.97	178.90
ST2	157.58	160.86	-----	158.69	-----	-----	-----	-----	-----	-----	161.96
GC	143.80	158.27	-----	144.96	-----	-----	-----	-----	-----	-----	153.98
CG	137.70	-----	-----	140.93	-----	-----	-----	-----	-----	-----	-----
BASE	136.17	150.95	169.10	137.25	177.46	181.45	166.17	164.70	186.74	171.34	149.52
Layer Thickness (ft)											
	CPT-175	CPT-176	CPT-177	CPT-178	CPT-179	CPT-180	CPT-181	CPT-182	CPT-183	CPT-185	CPT-186
TR1	15.61	8.86	22.92	10.53	13.29	15.30	10.01	0.00	19.77	13.30	13.68
TR1A	7.37	20.77	11.71	22.30	11.62	41.04	23.60	26.80	9.21	23.45	23.38
TR2A	36.12	21.78	22.64	26.74	28.21	21.04	27.00	40.01	26.40	29.27	20.45
TR2B	23.98	24.49	18.81	26.10	23.17	10.93	31.64	10.03	17.72	25.10	24.68
TR3/TR4	6.01	4.81	8.62	6.77	5.14	29.97	9.77	28.94	8.60	15.06	10.35
DB1/DB3	29.84	29.06	30.93	26.95	27.73	0.00	24.03	3.93	29.13	20.96	27.95
DB4/DB5	11.16	7.92	6.23	12.91	3.78	-----	6.60	3.56	3.56	8.09	5.11
ST1	15.43	19.05	-----	14.61	-----	-----	-----	-----	-----	-----	16.94
ST2	13.78	2.59	-----	13.73	-----	-----	-----	-----	-----	-----	7.98
GC	6.10	-----	-----	4.03	-----	-----	-----	-----	-----	-----	-----
CG	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1H Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

Layer Elevation (ft above MSL)					
	CPT-188	CPT-189	CPT-196	AVG	STD DEV
SURFACE	310.40	309.20	290.10	290.08	8.24
TR1	310.40	-----	290.10	290.95	7.27
TR1A	296.70	309.20	273.07	278.42	8.71
TR2A	273.60	276.04	260.22	261.51	6.51
TR2B	246.28	242.99	235.74	236.46	5.59
TR3/TR4	221.38	219.99	212.36	213.08	5.63
DB1/DB3	210.87	212.96	208.42	206.30	5.26
DB4/DB5	186.99	187.98	173.50	177.99	5.13
ST1	181.00	179.95	168.87	171.72	5.13
ST2	-----	-----	-----	152.42	3.75
GC	-----	-----	-----	141.10	4.66
CG	-----	-----	-----	133.87	3.29
BASE	168.56	175.16	168.31		
Layer Thickness (ft)					
	CPT-188	CPT-189	CPT-196	AVG	STD DEV
TR1	13.70	0.00	17.03	11.66	6.17
TR1A	23.10	33.16	12.85	16.90	5.79
TR2A	27.32	33.05	24.48	25.28	5.02
TR2B	24.90	23.00	23.38	23.05	3.22
TR3/TR4	10.51	7.03	3.94	7.23	3.97
DB1/DB3	23.88	24.98	34.92	27.72	5.37
DB4/DB5	5.99	8.03	4.63	6.44	2.74
ST1	-----	-----	-----	18.15	3.53
ST2	-----	-----	-----	11.24	3.15
GC	-----	-----	-----	6.65	2.06
CG	-----	-----	-----	30.37	-----

NOTE: ELEVATIONS ARE TO TOP OF LAYER

Table 3.3-1I Summary of CPT Layer Picks (Layer Tops and Layer Thicknesses for APSF and Northeast Expansion)

	Source	Fill	TR1	TR1A	TR2A	TR2B	TR3/4	DB1/3	DB4/5	ST*	GC
SPT N-Value (blows/foot)	F-Area Report	23	25	25	28	36	18	33	15	47	21
	APSF Data	-	33	27	34	38	19	50	21	46	49
	NEC Data	N/A	31	31	37	39	27	37	29	43	39
qt/N	F-Area Report	4.9	3.7	4.8	5.2	5.5	3.1	5.1	4.1	2.8	2.7
	APSF Data	-	4.3	2.5	4.0	4.1	1.9	3.3	2.5	3.0	1.6
	NEC Data	N/A	3.8	4.1	5.2	4.6	4.1	5.9	4.5	2.9	4.6
Shear Wave Velocity (ft/sec)	F-Area Report	978	1455	1348	1256	1254	1074	1157	1140	1353	1675
	APSF Data	-	1637	1464	1284	1215	1020	1197	1231	1223	1160
	NEC Data	N/A	1544	1454	1257	1165	1056	1176	1180	1273	1319
Corrected Tip Resistance (tons/foot ²)	F-Area Report	112	91	120	147	201	55	172	61	131	58
	APSF Data	-	142	68	136	154	37	166	52	137	79
	NEC Data	N/A	95	103	146	164	73	194	67	138	97
Friction Ratio (%)	F-Area Report	2	4	2	2	1	2	1	2	2	2
	APSF Data	-	2	4	1	1	2	1	2	1	2
	NEC Data	N/A	3	3	1	1	2	1	2	1	2
Percent Fines (%)	F-Area Report	25	33	30	17	19	64	14	22	29	39
	APSF Data	-	25	37	16	11	34	9	21	18	52
	NEC Data	N/A	34	30	14	10	36	14	20	19	33
Plasticity Index (%)	F-Area Report	15	17	14	10	18	58	19	28	18	47
	APSF Data	-	11	22	10	NP	19	NP	11	25	30
	NEC Data	N/A	23	20	9	12	19	16	11	14	27
Liquid Limit (%)	F-Area Report	32	38	36	33	41	96	44	48	40	83
	APSF Data	-	30	46	33	NP	54	NP	45	49	57
	NEC Data	N/A	43	35	28	24	54	41	45	23	42
Water Content (%)	F-Area Report	13	15	19	17	22	51	27	39	29	32
	APSF Data	-	16	20	21	24	42	27	38	30	28
	NEC Data	N/A	18	19	17	18	34	25	36	30	32

Notes

1. Data for the APSF Investigation includes CPTs 1-17, 36, 37, and 44 and Borings FB-3 through FB-12
2. The ST layer was not subdivided for analysis
3. Data for the Northeast Expansion included CPT's 103-196 and borings FB-17 through FB-30
4. Laboratory data for a portion of the DB1/3 layer from FB-22 (samples SS50-SS53) were omitted due to a significant material change considered to be localized in the vicinity of FB-22

Table 3.3-2 Summary of Engineering Soil Properties

CPT No.	Top Elv	Bot Elv	Appx. Thickness
2	150.68	144.47	5.97
	173.28	170.65	2.63
3	170.35	166.05	3.93
6	145.10	138.46	6.28
	175.86	170.43	5.04
18	138.70	136.51	2.19
20A	150.90	148.79	2.11
	146.05	139.70	5.44
23	208.90	204.25	4.65
24	174.27	169.56	4.71
25	142.87	138.35	4.52
27	147.08	144.87	2.21
29	151.08	146.66	3.45
	181.30	175.34	5.21
32	144.31	139.78	3.50
32A	144.00	140.15	3.36
33	140.18	138.77	1.41
	174.68	170.67	2.62
38	178.74	175.41	3.03
39	207.44	205.50	1.94
46	176.43	172.44	3.99
51	177.90	172.98	4.07
103	177.60	171.21	2.88
157	215.13	211.21	3.39
	179.80	173.46	6.19
SPT No.	Top Elv	Bot Elv	Appx. Thickness
FB-1	155.1	146.6	8.5
FB-4	177.8	172.1	5.70
FB-17	183.10	176.10	7.00
	157.60	151.60	6.00
FB-19	189.60	186.60	3.00
FB-20	179.00	172.90	6.10

SOFT ZONE THICKNESS CRITERIA:

1. For a CPT to be labelled as a soft zone "hit" it must have a zone with a corrected tip stress of less than 15 tsf over a 2 ft thick (or greater) interval. Professional judgment may include a few zones that approach 2 ft in thickness. For a boring to be labelled as a soft zone "hit" it must have a WR, WH or N-value ≤ 5 over a 2 ft or greater interval.
2. About two-thirds of the CPTs have soft zone thicknesses which consist of two or more intervals having <15 tsf tip stresses. The criteria for combining these separate soft intervals into a single zone is an intervening harder layer less than 1 foot in thickness. In these instances the Top Elv, Bot Elv, and appx. zone thickness were calculated excluding the harder layer as shown in the following example:

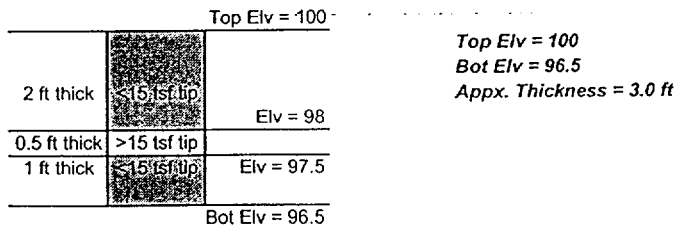


Table 3.3-3 Summary of Soft Zone Intervals

SAMPLE NUMBER	ADJACENT CPT	TOP ELEV.	TOP DEPTH	BOTTOM DEPTH	SOFT INTERVAL	LAB TESTING ASSIGNMENTS
F235 CSS38	38	177.5	112	114	UPPER ST	UW, Cons, Hyd, AL, MC
F235 CSS38	38	175.5	114	116	UPPER ST	UW, Cons, Hyd, AL, MC
F235 CSS53B	53	154.9	135	137	LOWER ST	UW, Hyd, AL, MC
F235 CSS 53	53	154.9	135	139	LOWER ST	UW, Hyd, AL, MC
F235 CSS32	32	141.5	148	148.8	LOWER ST	Hyd, Cons, UCC, AL
F235 CSS32	32	140.7	148.8	150	LOWER ST	Hyd
F235 CSS32	32	139.5	150	150.5	LOWER ST	Hyd

SAMPLE NUMBER	Dry UW lb/ft^3	MC %	Wet UW lb/ft^3	%Sand	%Fines	%Clay	D50 mm	LL %	PI %	Class
F235 CSS38	65.2	61.56	105.3	61.8	38.2	29.5	0.14	44	20	SC
F235 CSS38	77.8	42.37	110.7	85.5	14.5	11.5	0.17	NL	NP	SM
F235 CSS53B	83	34.6	111.7	90.6	9.4	1.9	0.14	NL	NP	SP-SM
F235 CSS 53	75.1	42	106.7	90.9	9.1	1.7	0.13	NL	NP	SP-SM
F235 CSS32	97.9	25	122.4	48.3	51.7	24.2	0.07	44	24	CL
F235 CSS32				64.9	35.1	5.9	0.09			SM
F235 CSS32				62.6	37.4	6	0.08			SM

SAMPLE NUMBER	Cc	eo	SG	Pc ksf	Po' ksf	OCR	su ksf
F235 CSS38							
F235 CSS38	0.23	1.1954	2.65	0.71	11.3	0.1	
F235 CSS53B							
F235 CSS 53							
F235 CSS32	0.26	0.6902	2.65	10.7	13.3	0.8	3.4
F235 CSS32							
F235 CSS32							

Note: This data is presented solely for interpretation. Refer to Section 3.3.1.3 in the report text for more information.

Table 3.3-4 Summary of Soft Zone Soil Properties

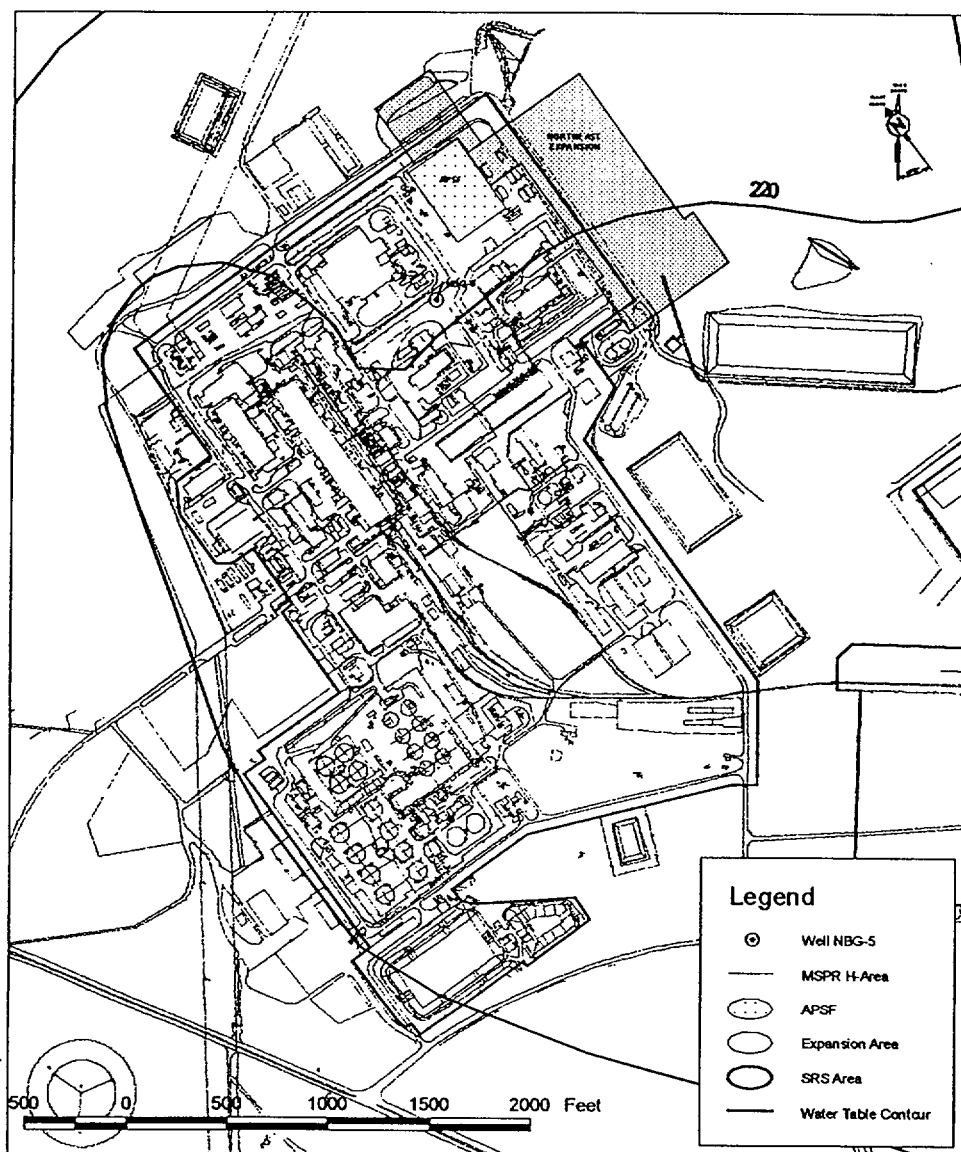


Figure 1.0-1 Location Map of F-Area and the Northeast Expansion Area

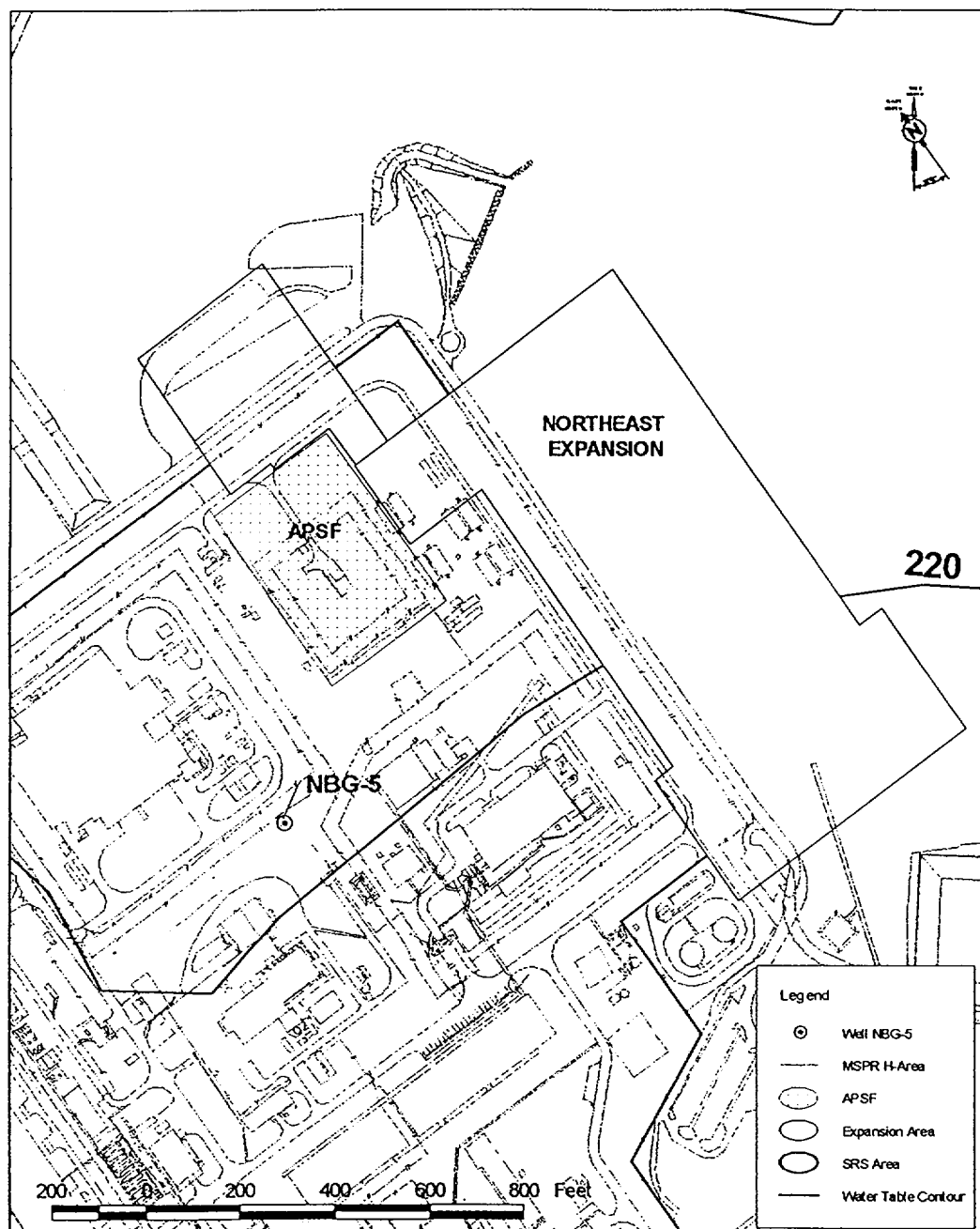
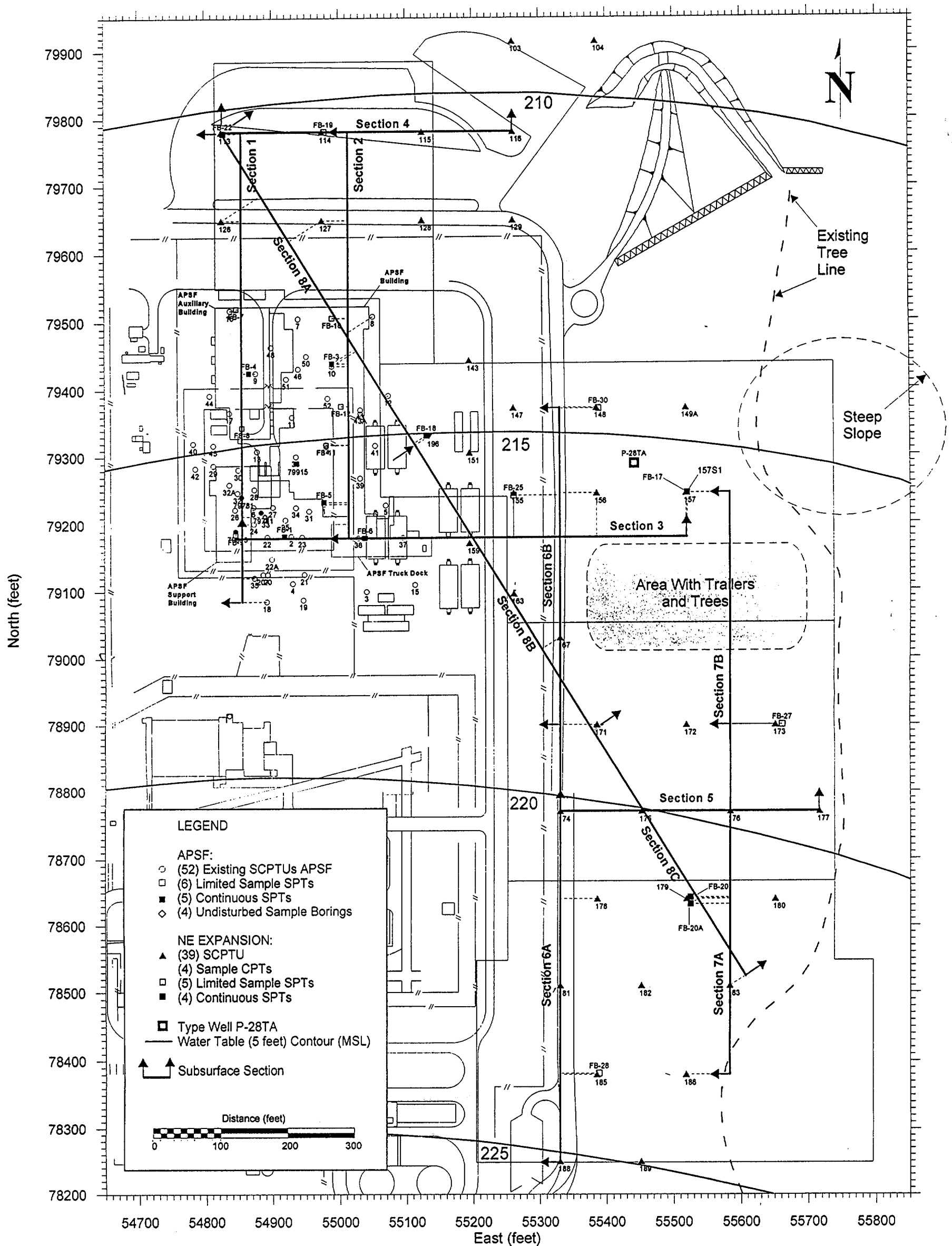


Figure 1.0-2 Location Map of the Northeast Expansion Area and APSF



d:/gelines/cpt/apsf/miscfigs/expanded/expand12.grf 06-01-99
uses expand6.xls expansion

APSF structure/facility outline provided by project team (SRS Drawing C-CG-F-0042) and is considered approximate.
F-Area facility boundaries obtained from SRS maps MSRP0884.DGN and MSRP0854.DGN.

Figure 2.0-1 Exploration Location Map

Prepared By: Robert Kelenis

Checked By: FS.

Approved By: [Signature]

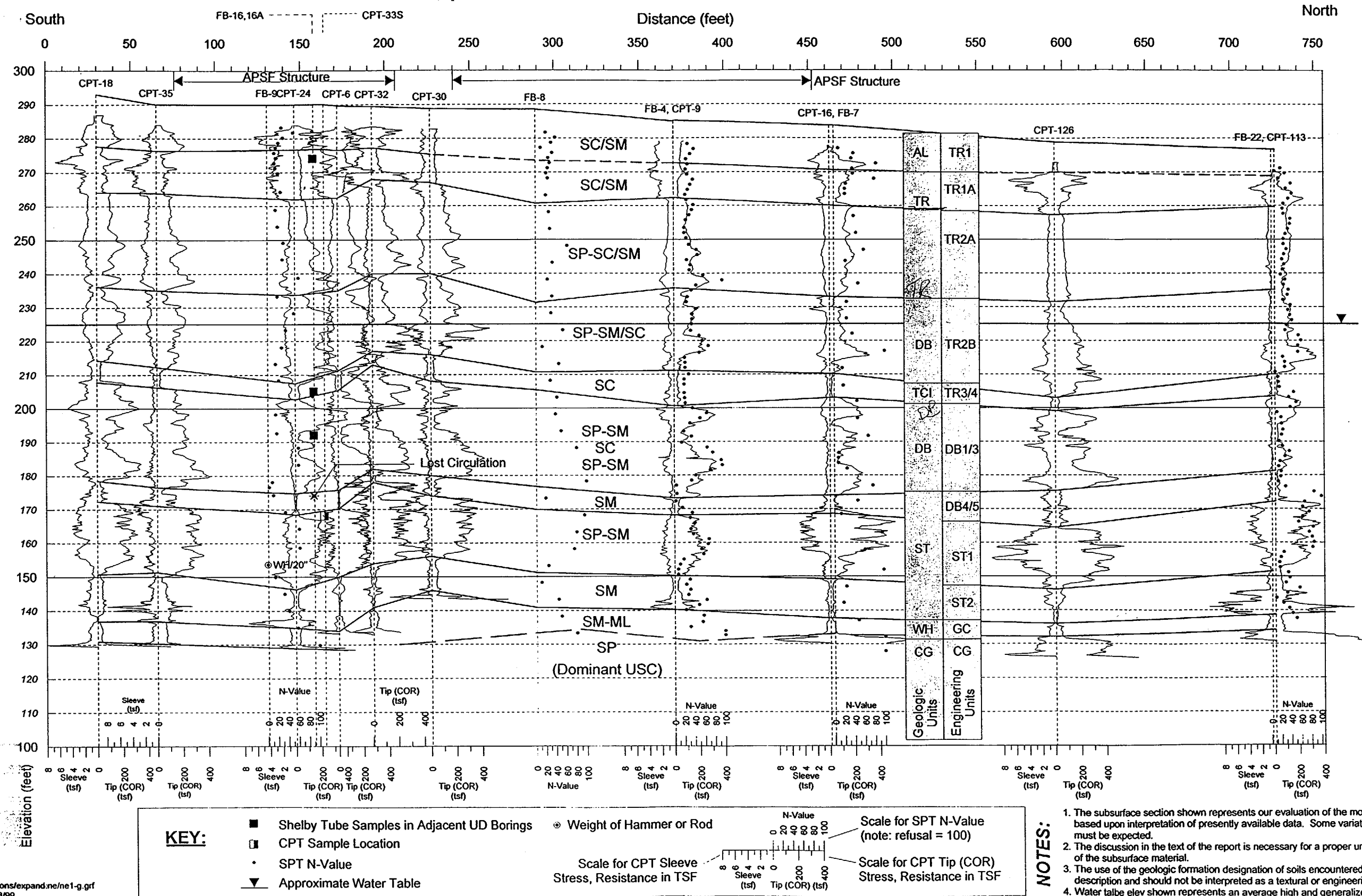
Revision: 0

Date: 5-27-99

APSF NE Expansion Section Line 1

PRELIMINARY

Vertical Exaggeration = 2X



Subsurface Cross-section 1

Figure 3.0-1

Prepared By: Robert J. Williams Checked By: F.S.

Approved By: [Signature] Revision: 0 Date: 5-22-99

APSF NE Expansion Section Line 2

Vertical Exaggeration = 2X

PRELIMINARY

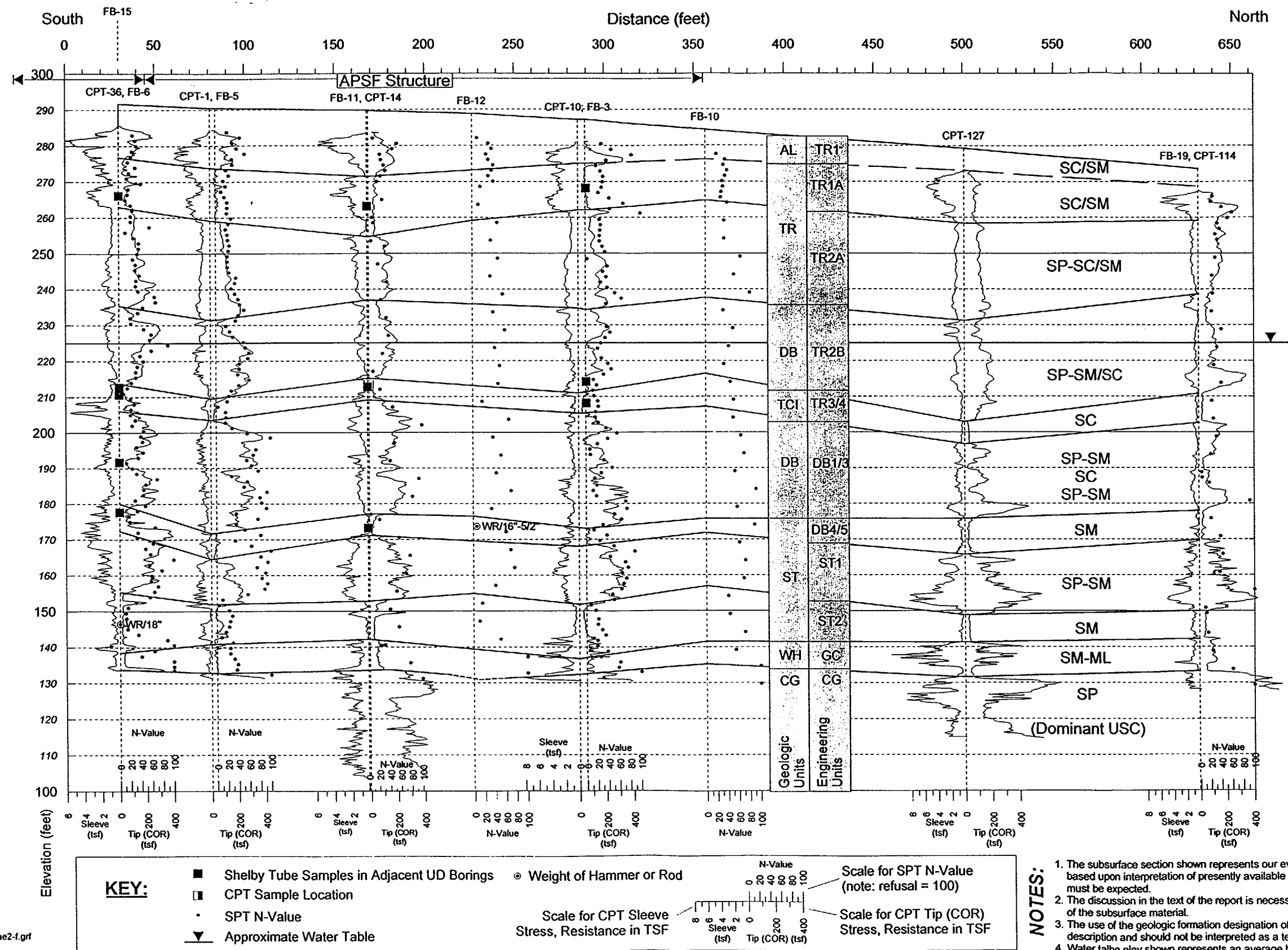
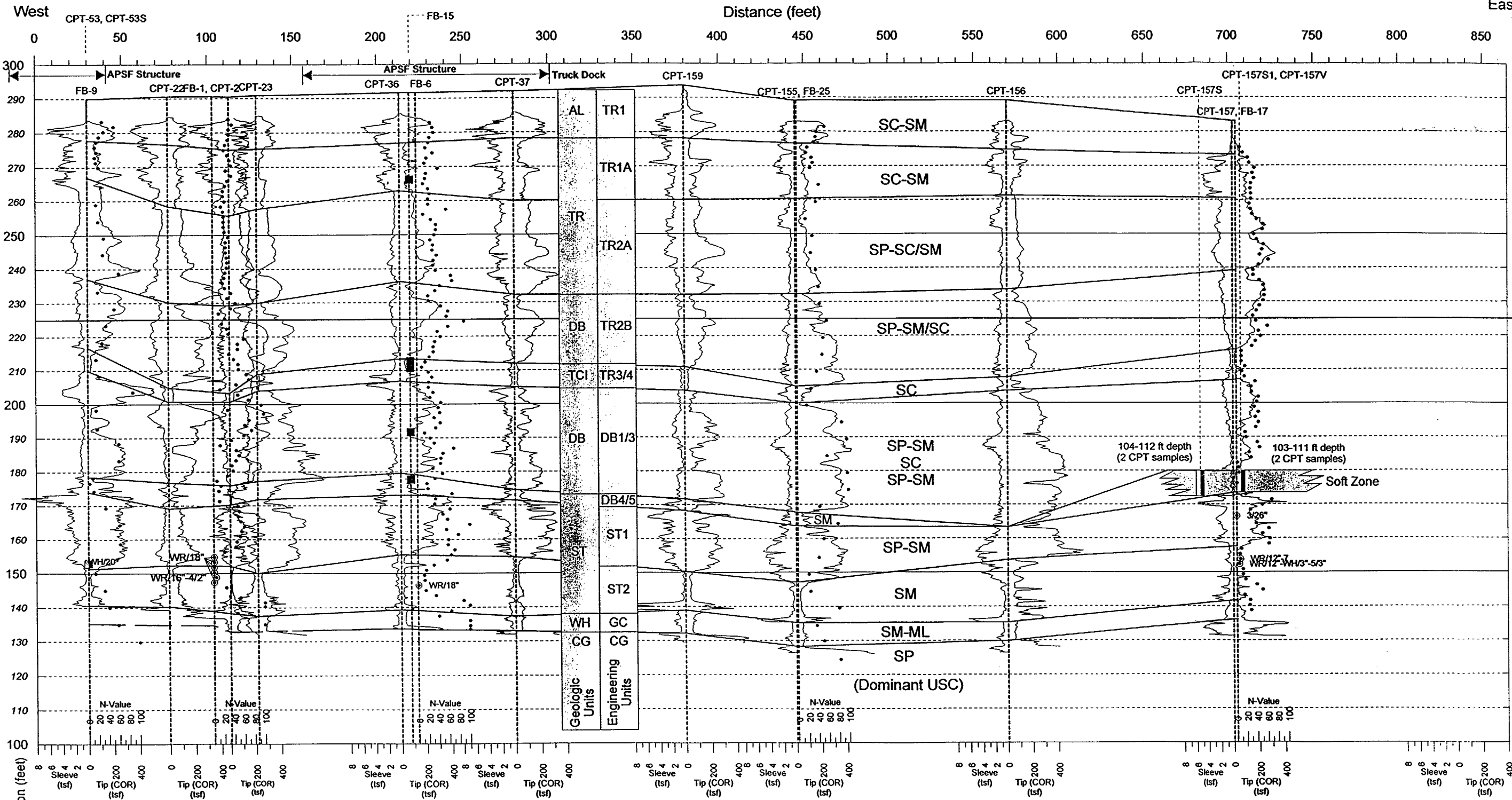


Figure 3.0-2 Subsurface Cross-section 2

Prepared By: Robert Helgeson Checked By: R.S.
Approved By: [Signature] Revision: 0 Date: 5-27-99



KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- ▼ Approximate Water Table
- Weight of Hammer or Rod
- Scale for CPT Sleeve Stress, Resistance in TSF
- Scale for SPT N-Value (note: refusal = 100)
- Scale for CPT Tip (COR) Stress, Resistance in TSF

NOTES:

1. The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
2. The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
3. The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
4. Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
5. USC = Unified Soil Classification.

Figure 3.0-3 Subsurface Cross-section 3

Prepared By: Robert Kelynis

Checked By: F.S.

Approved By: [Signature]

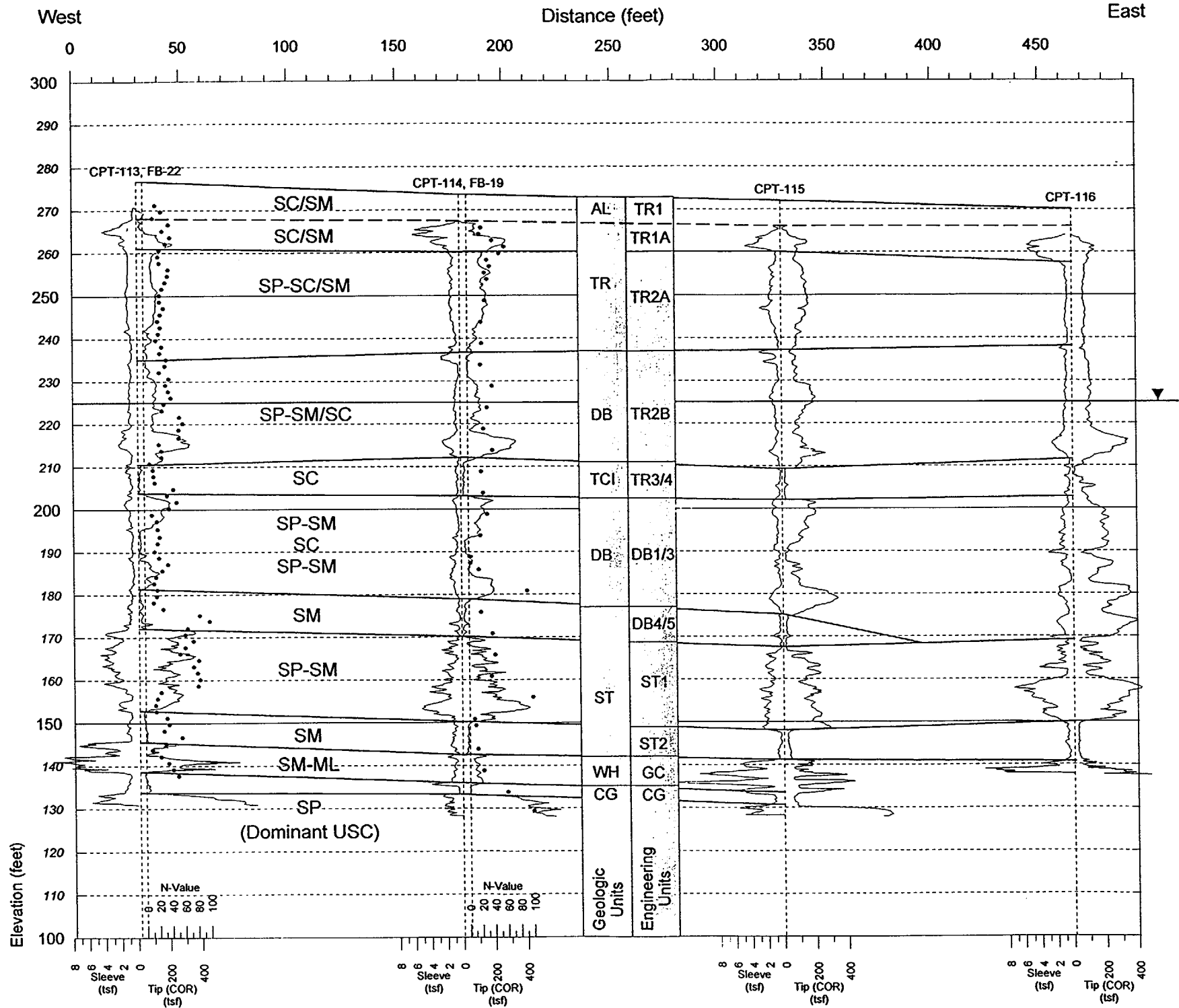
Revision: 0

Date: 5-27-99

APSF NE Expansion Section Line 4

PRELIMINARY

Vertical Exaggeration = 2X

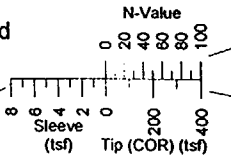


KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- ▼ Approximate Water Table

Weight of Hammer or Rod

Scale for CPT Sleeve Stress, Resistance in TSF



Scale for SPT N-Value (note: refusal = 100)

Scale for CPT Tip (COR) Stress, Resistance in TSF

NOTES:

1. The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
2. The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
3. The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
4. Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
5. USC = Unified Soil Classification.

Prepared By: Robert J. Gelinas

Checked By: F.S.

Approved By: [Signature]

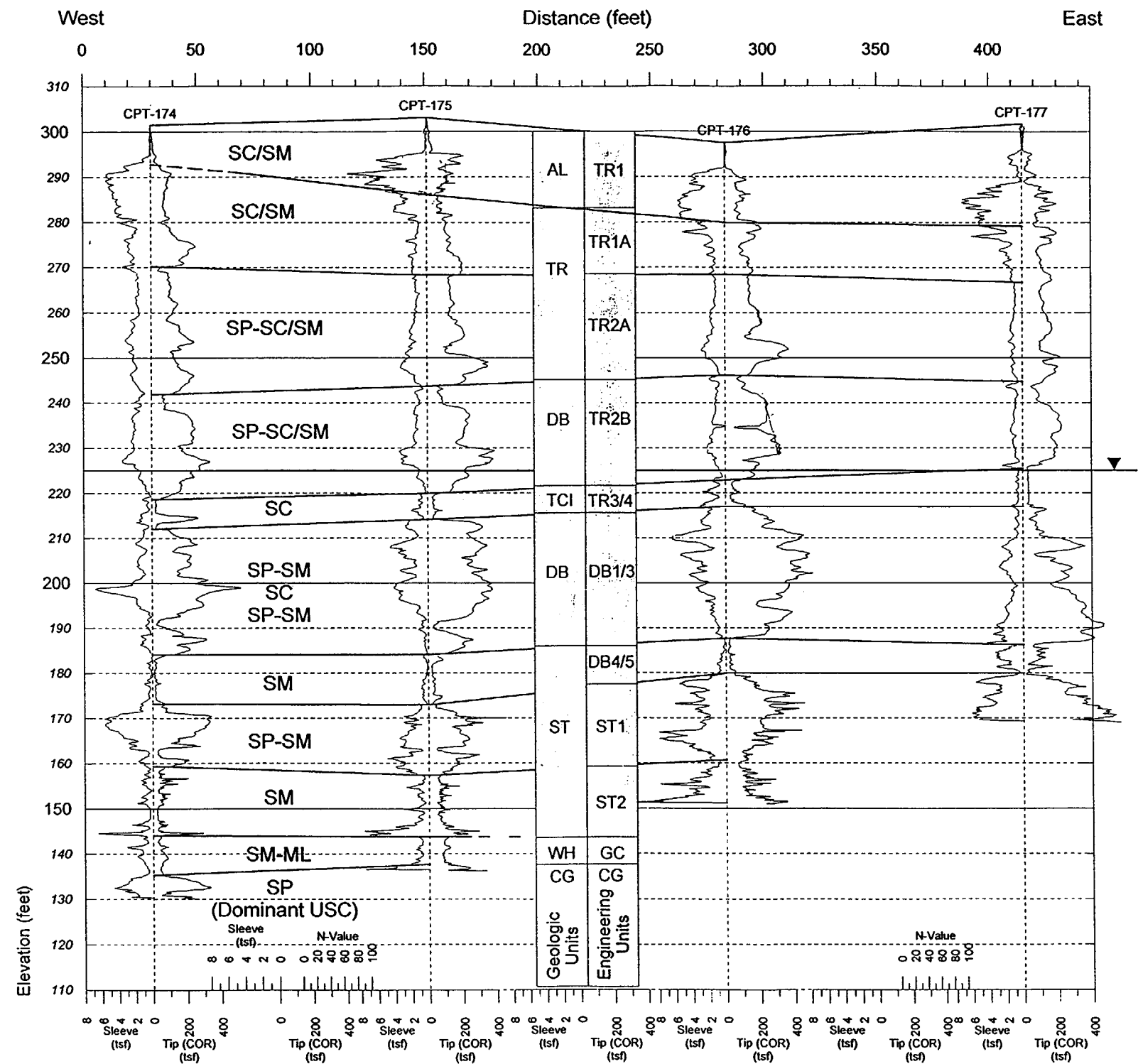
Revision: 0

Date: 5-27-99

APSF NE Expansion Section Line 5

Vertical Exaggeration = 2X

PRELIMINARY



KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- Approximate Water Table
- Weight of Hammer or Rod
- Scale for CPT Sleeve Stress, Resistance in TSF
- Scale for SPT N-Value (note: refusal = 100)
- Scale for CPT Tip (COR) Stress, Resistance in TSF

NOTES:

- The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
- The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
- The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
- Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
- USC = Unified Soil Classification.

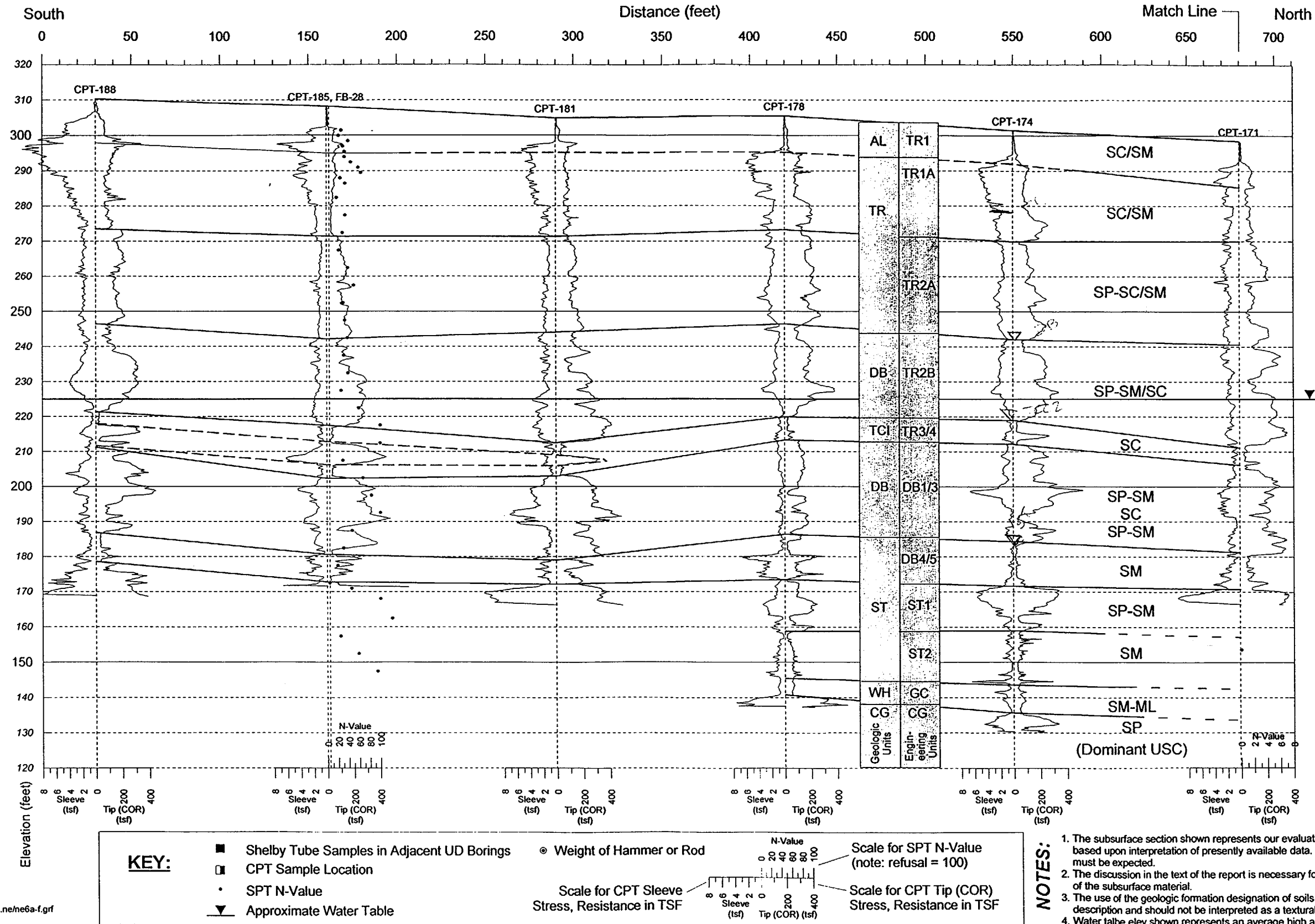
Figure 3.0-5 Subsurface Cross-section 5

PRELIMINARY

APSF NE Expansion Section Line 6A

Vertical Exaggeration = 2X

Prepared By: Robert J. Gelinas Checked By: F.S.
Approved By: [Signature] Revision: 0 Date: 5-27-99



Subsurface Cross-section 6A

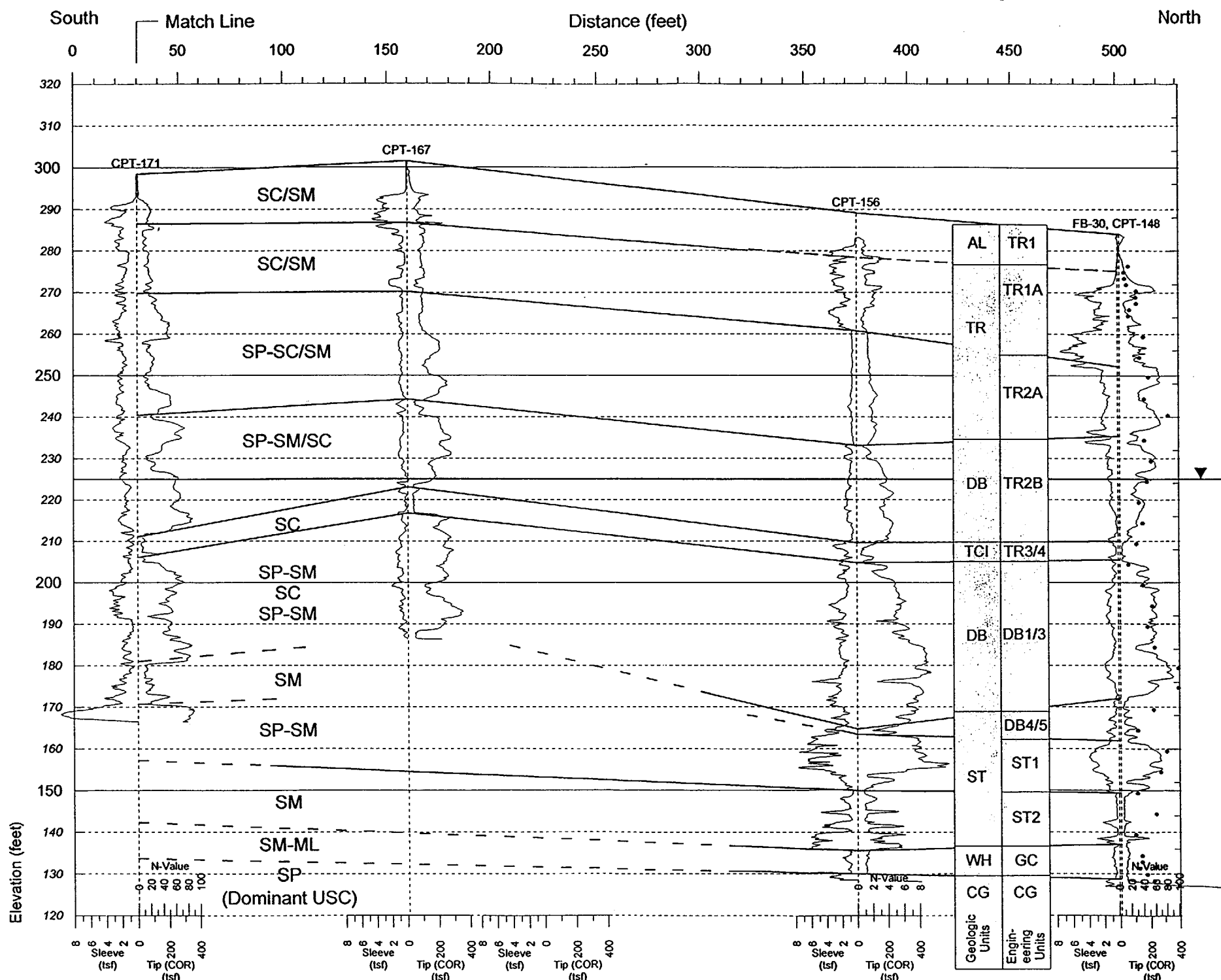
Figure 3.0-6

PRELIMINARY

APSF NE Expansion Section Line 6B

Vertical Exaggeration = 2X

Prepared By: Robert Helms Checked By: F.S.
Approved By: [Signature] Revision: 0 Date: 5-27-99

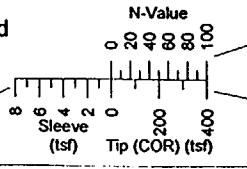


KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- ▼ Approximate Water Table

⊙ Weight of Hammer or Rod

Scale for CPT Sleeve Stress, Resistance in TSF



Scale for SPT N-Value (note: refusal = 100)

Scale for CPT Tip (COR) Stress, Resistance in TSF

NOTES:

1. The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
2. The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
3. The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
4. Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
5. USC = Unified Soil Classification.

Subsurface Cross-section 6B

Figure 3.0-6

Figure 3.0-7
Subsurface Cross-section 7A

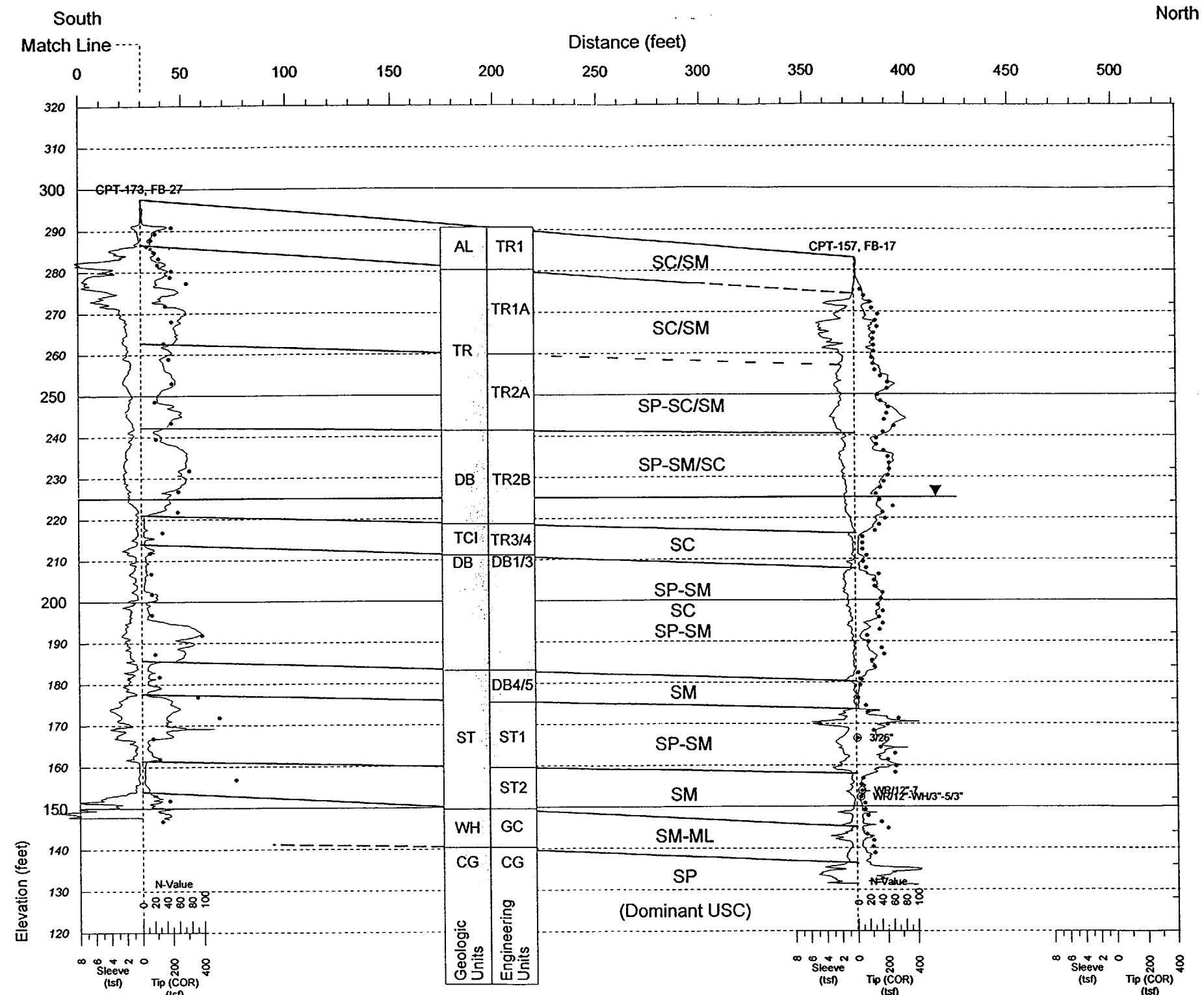
Prepared By: Robert Gelinas Checked By: F.S.

Approved By: [Signature] Revision: 0 Date: 5-27-99

APSF NE Expansion Section Line 7B

Vertical Exaggeration = 2X

PRELIMINARY



KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- Approximate Water Table

NOTES:

- The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
- The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
- The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
- Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
- USC = Unified Soil Classification.

Scale for CPT Sleeve Stress, Resistance in TSF
Scale for CPT Tip (COR) Stress, Resistance in TSF
Scale for SPT N-Value (note: refusal = 100)

Figure 3.0-7 Subsurface Cross-section 7B

Prepared By: Robert Pelina

Checked By: R.S.

Approved By: [Signature]

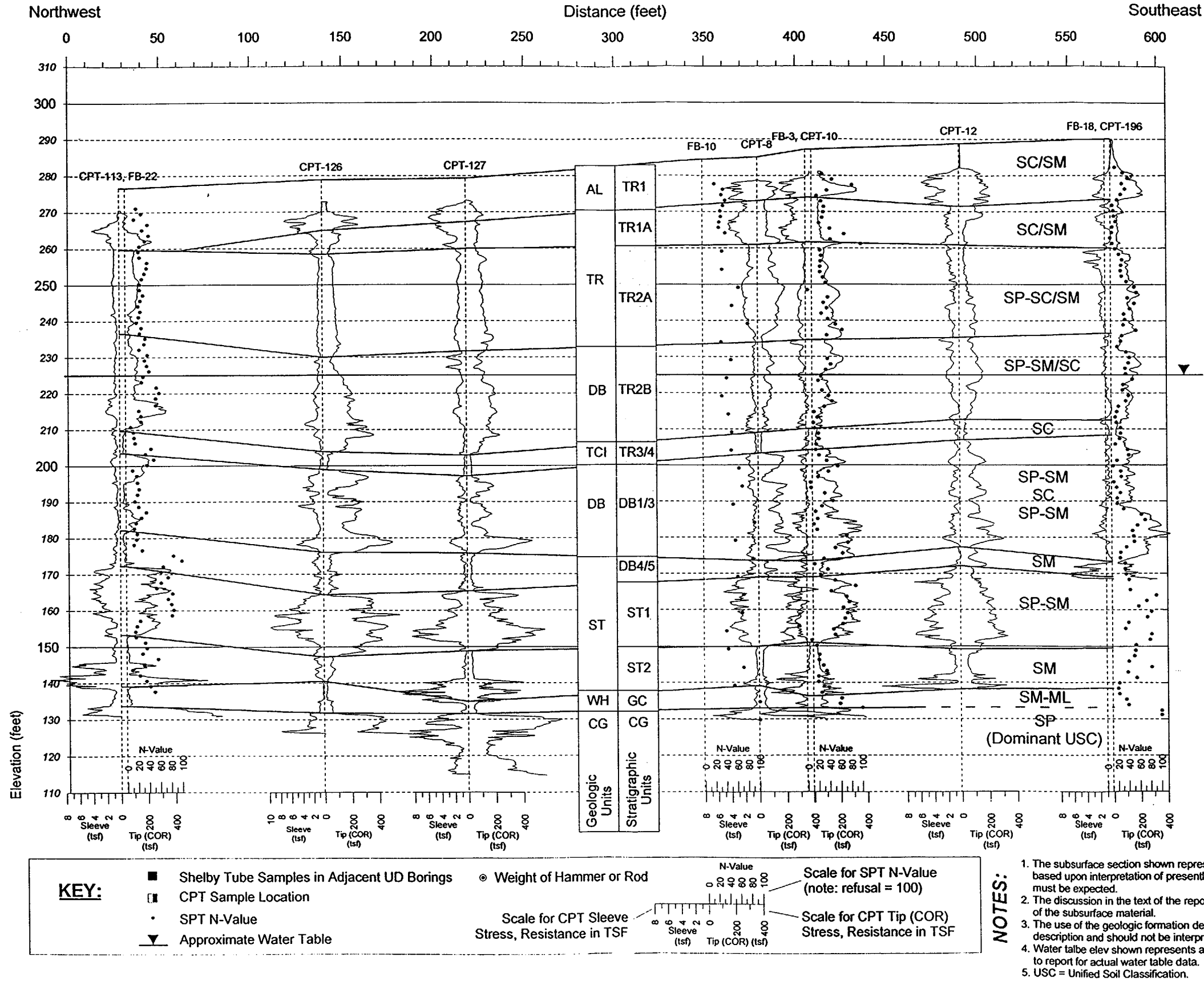
Revision: 0

Date: 5-27-99

APSF NE Expansion Section Line 8A

Vertical Exaggeration = 2X

PRELIMINARY

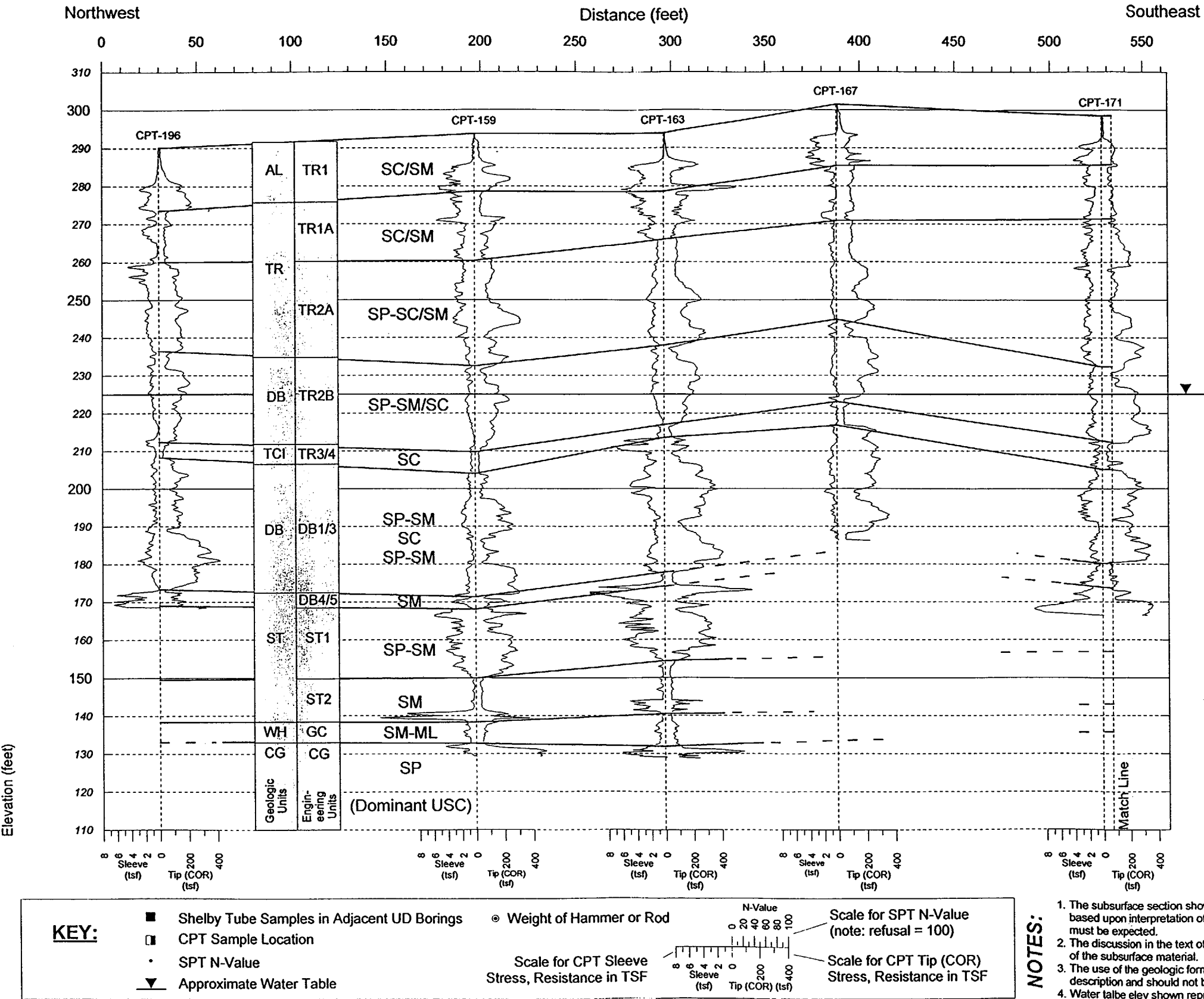


PRELIMINARY

APSF NE Expansion Section Line 8B

Vertical Exaggeration = 2X

Prepared By: Robert Gelinas Checked By: F.S.
Approved By: [Signature] Revision: 0 Date: 5-27-95

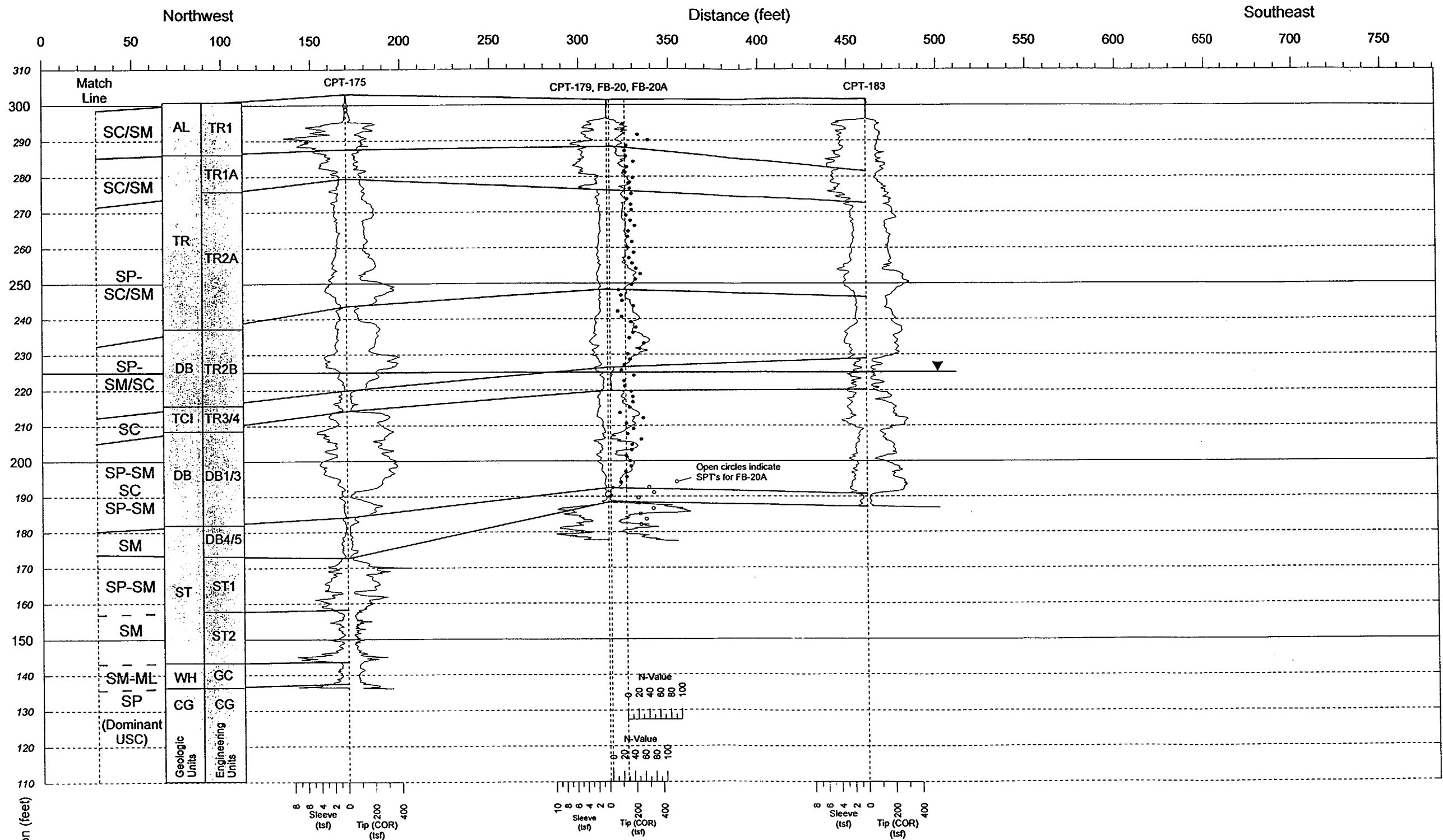


PRELIMINARY

APSF NE Expansion Section Line 8C

Vertical Exaggeration = 2X

Prepared By: Robert J. Gelinas Checked By: F.S.
Approved By: [Signature] Revision: 0 Date: 5-27-99



KEY:

- Shelby Tube Samples in Adjacent UD Borings
- CPT Sample Location
- SPT N-Value
- ▼ Approximate Water Table
- ⊙ Weight of Hammer or Rod
- Scale for CPT Sleeve Stress, Resistance in TSF
- Scale for SPT N-Value (note: refusal = 100)
- Scale for CPT Tip (COR) Stress, Resistance in TSF

NOTES:

1. The subsurface section shown represents our evaluation of the most probable conditions based upon interpretation of presently available data. Some variation from these conditions must be expected.
2. The discussion in the text of the report is necessary for a proper understanding of the nature of the subsurface material.
3. The use of the geologic formation designation of soils encountered is used for ease of description and should not be interpreted as a textural or engineering description.
4. Water table elev shown represents an average high and generalized across the area. Refer to report for actual water table data.
5. USC = Unified Soil Classification.

Subsurface Cross-section 8C
Figure 3.0-8

Hydrograph of Well NBG-5

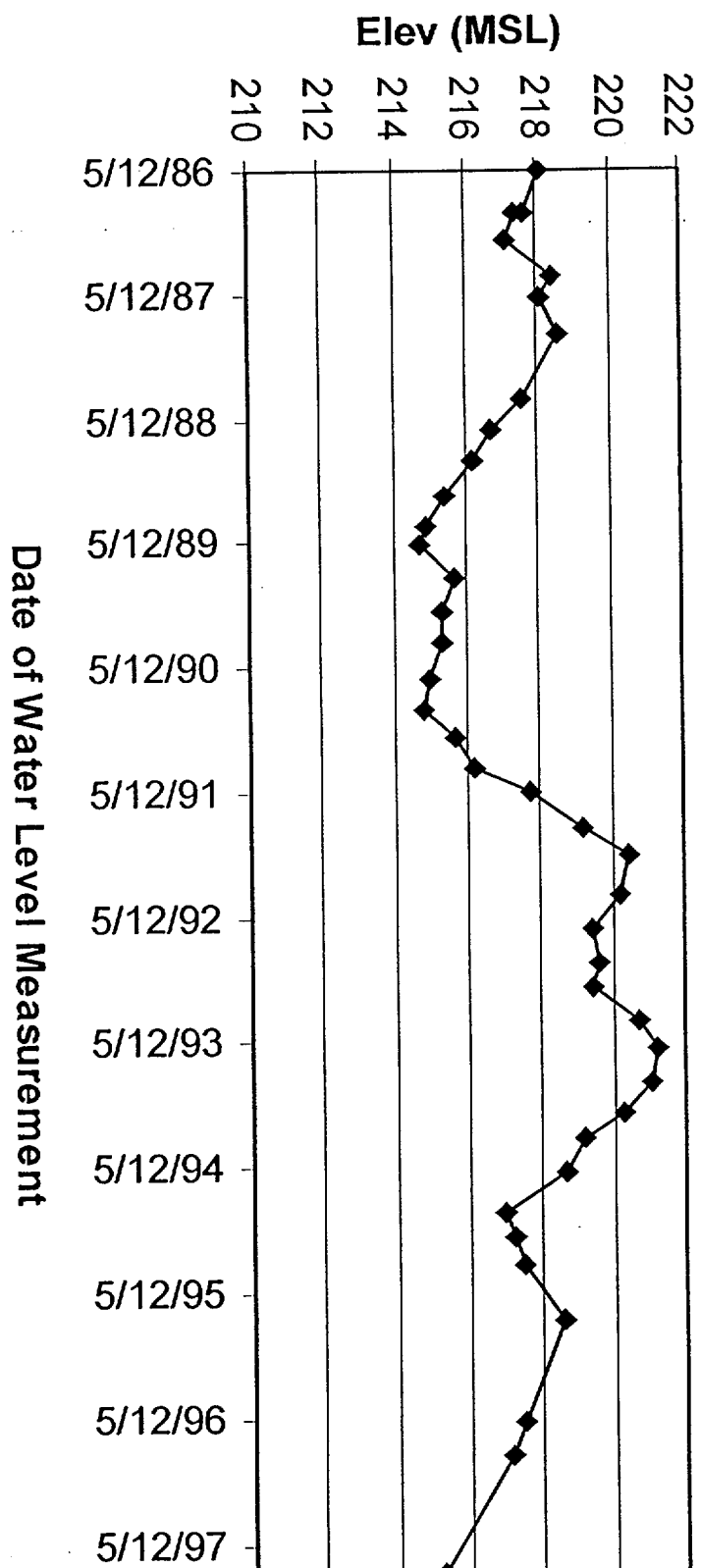


Figure 3.2-1 Hydrograph of Water Table Measurements for Well NBG-5

Northeast Characterization

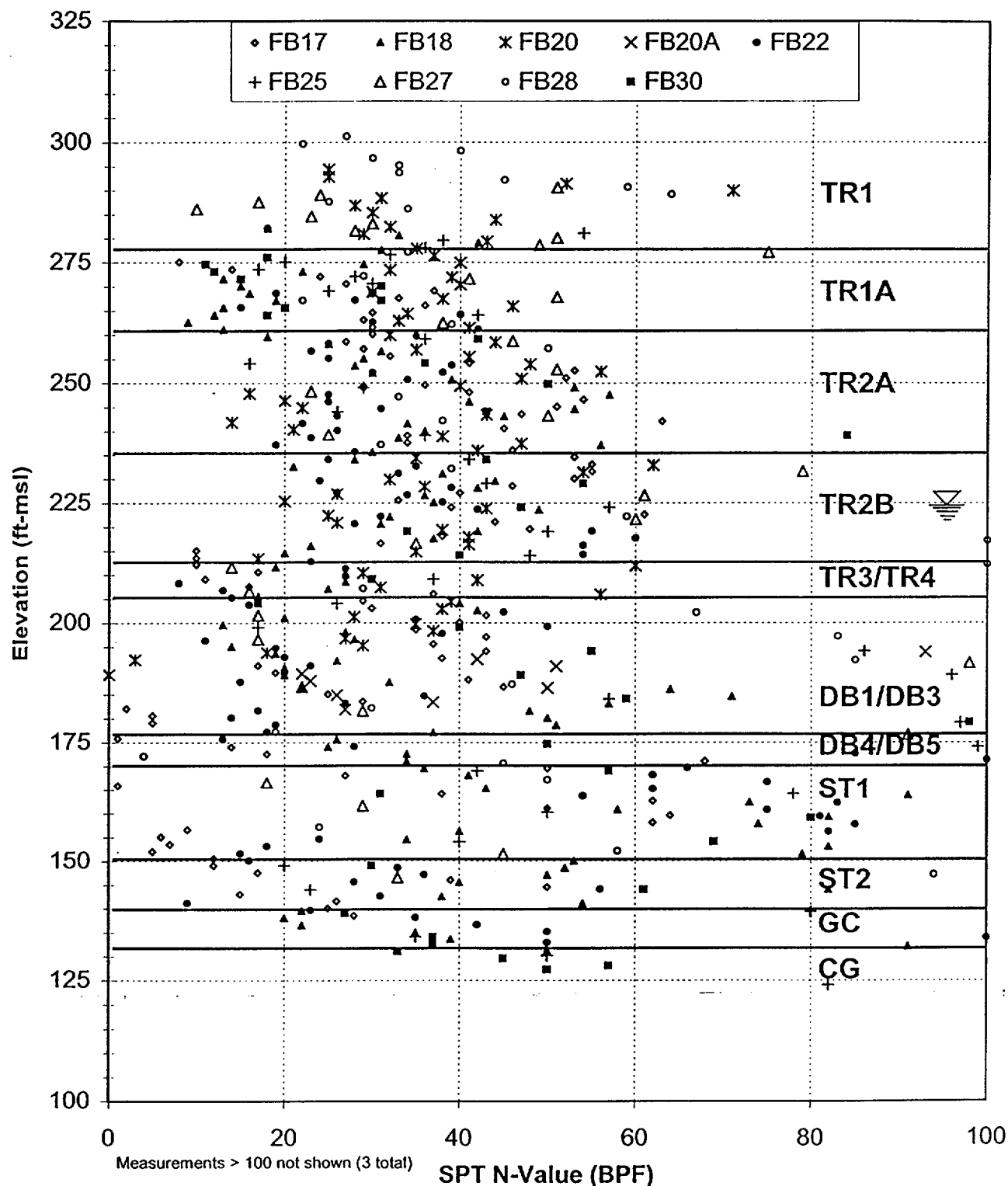


Figure 3.3-1 Range of SPT N-value Measurements

Northeast Characterization

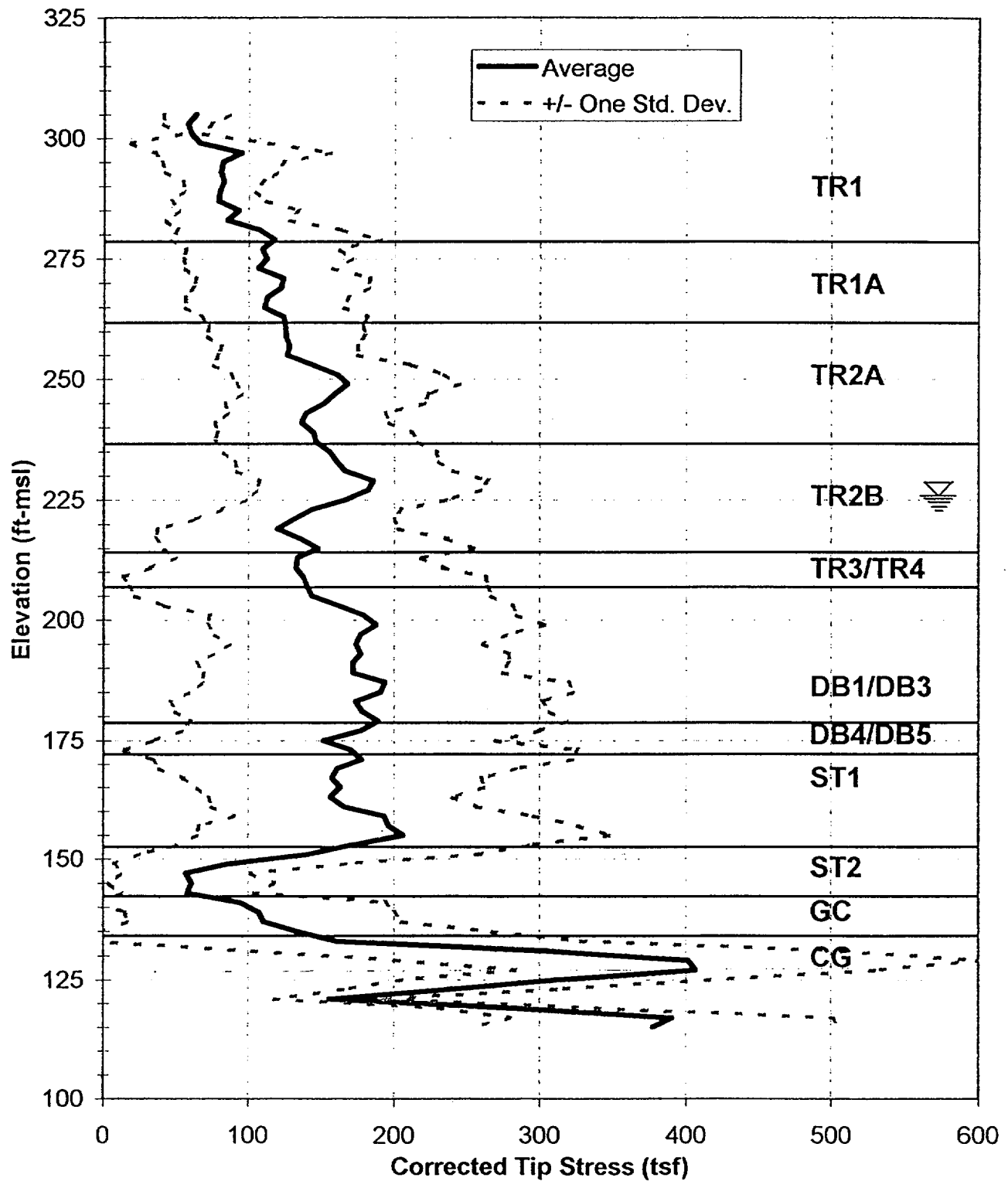


Figure 3.3-2 CPT Corrected Tip Resistance (q_t)

Northeast Characterization

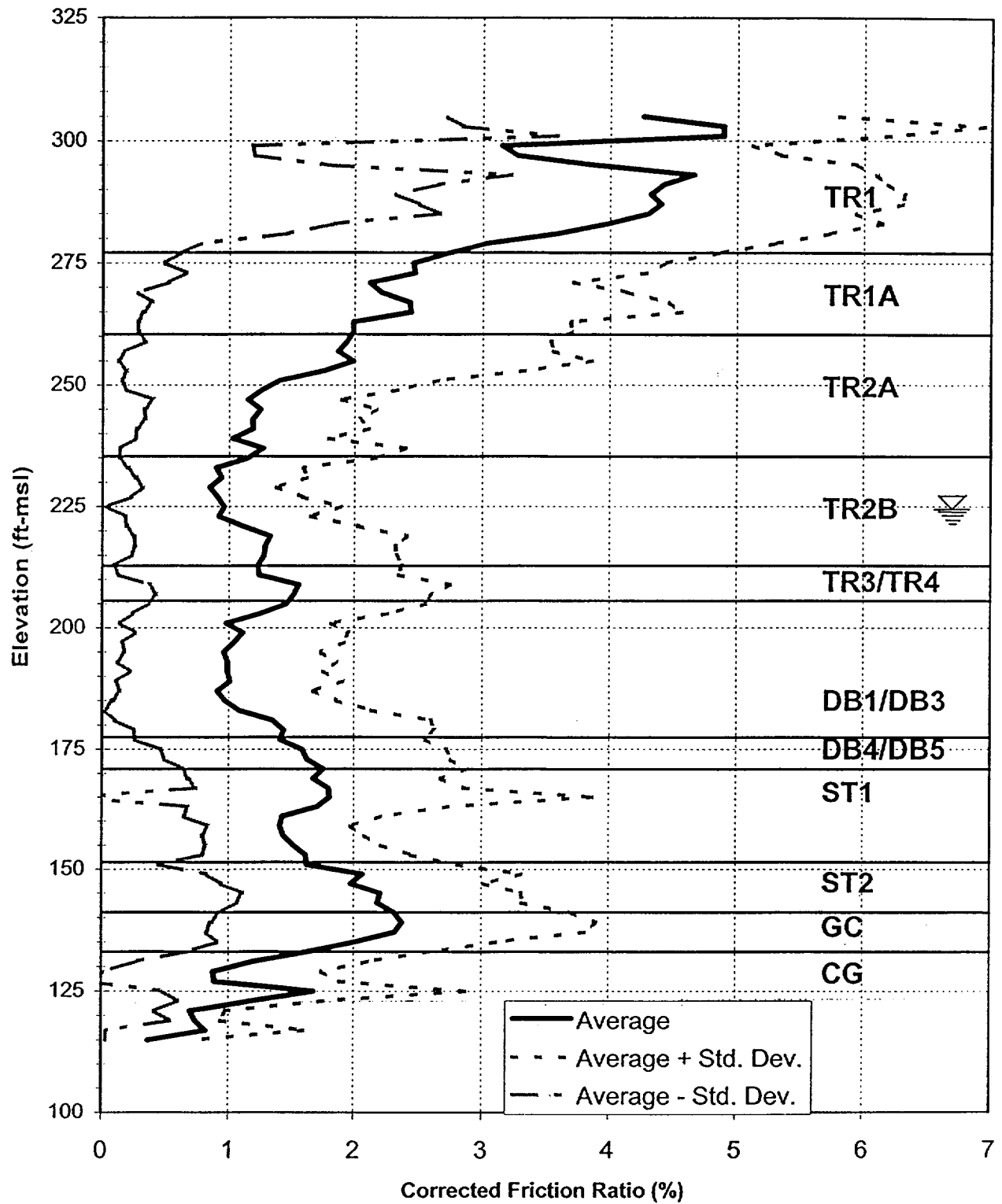


Figure 3.3-3 CPT Friction Ratio (Rf)

Northeast Characterization

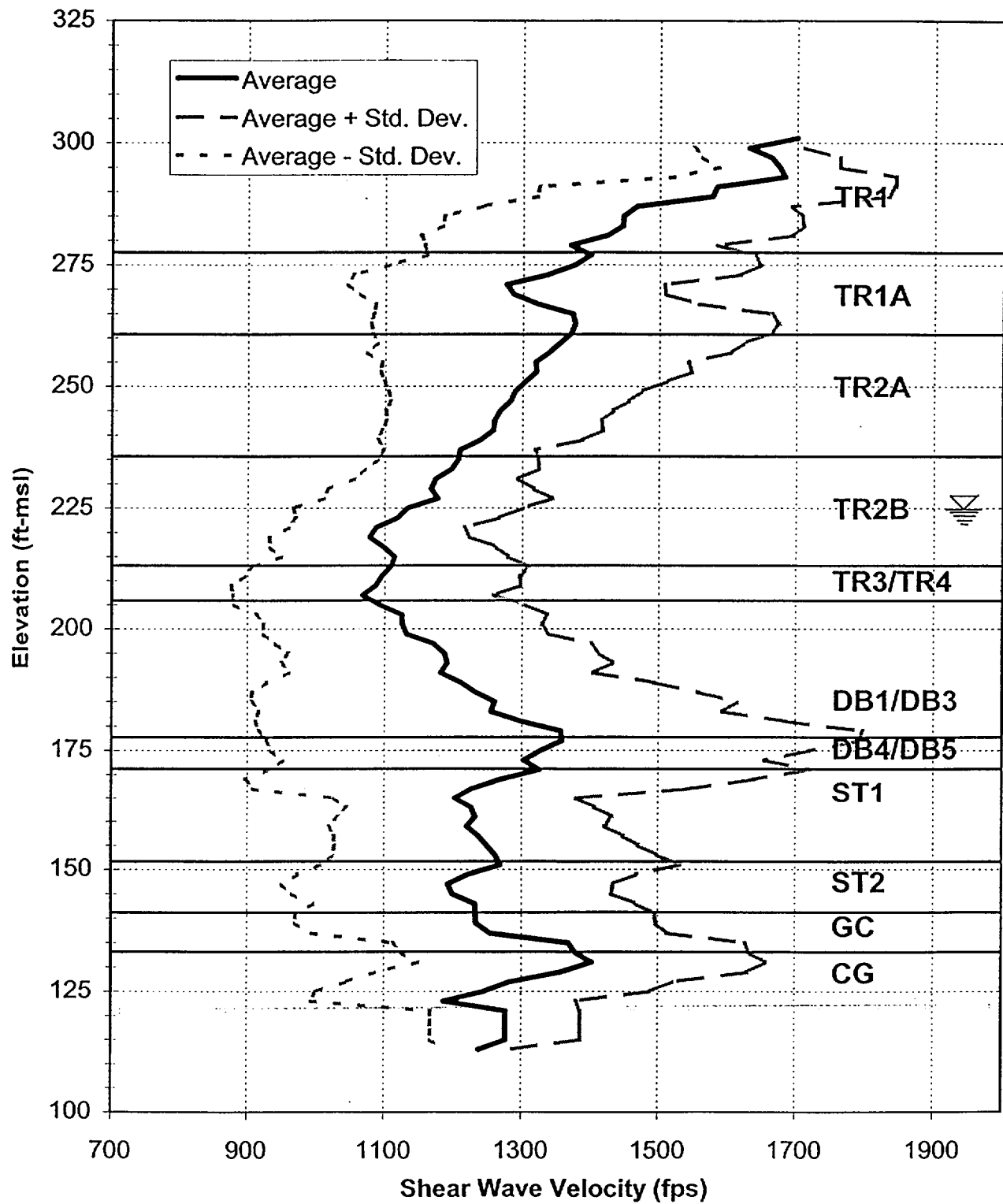


Figure 3.3-4 Average Shear Wave Velocity



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 1 OF 6	HOLE NO. FB-1
SITE Pu Repackaging Facility F-Area			COORDINATES N 79,182 E 54,918			ANGLE FROM HORIZONTAL Vertical		
BEGUN 8-22-95	COMPLETED 9-8-95	DRILLER EEI	DRILL MAKE AND MODEL Mobile B-57	HOLE SIZE 8"/5"	SAMPLE HAMMER WEIGHT/FALL 140 Lbs/ 30"	TOTAL DEPTH 156.5		
GROUND EL. 290.6		DEPTH/EL. GROUND WATER 69.0/221.6 69'	LOGGED BY: F.H. Syms					
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80		290.6					
			289.1			Red brown Clayey Sand (SC) with cobbles and quartz pebbles, dry.	Hole advanced using 8" od hollow stem augers from 0.0' to 70.5'.	
						Light brown Poorly Graded Sand (SP) mostly medium to fine sand, dry	Mud rotary drilling methods using a 5" drag bit was used from 70.5' to 156.5'. Aw rods were used for split spoon from 0.0' to 91.5'. "N" rods were used for split spoon sampling from 91.5' to 156.5'.	
				5			Recovery 18"/18"	
			284.1			similar to above material	Recovery 18"/18"	
SS 1	▲	8 12 20				Red brown Clayey Sand (SC) 70% fine sand, 30% fines, medium plasticity, dense, cohesive, mottled color.	Recovery 18"/18"	
SS 2	▲	11 16 22				similar to above material	Recovery 18"/18"	
SS 3	▲	13 13 19		10		similar to above material except, becoming less dense, less plastic	Recovery 18"/18"	
SS 4	▲	12 15 17				similar to above material	Recovery 12"/18"	
SS 5	▲	12 12 18				similar to above material except, grain size is increasing slightly to include very few medium sand.	Recovery 14"/18"	
SS 6	▲	11 13 12	276.1			similar to above material	Recovery 16"/18"	
SS 7	▲	8 10 12	275.6	15		Red brown Silty Sand (SM) 70% very fine sand, 30% fines, medium plasticity, medium dense	Recovery 12"/18"	
SS 8	▲	9 15 14				Light brown Clayey Sand (SC) 5% rounded coarse sand, 70% fine sand, 25% fines, medium plasticity, dense, slightly moist.	Recovery 13"/18"	
SS 9	▲	9 15 18				similar to above material	Recovery 12"/18"	
SS 10	▲	12 19 17		20		similar to above material except grain size increases to ~10% medium sand, slightly micaceous and high dry strength.	Recovery 12"/18"	
SS 11	▲	10 13 13				similar to above material	Recovery 15"/18"	
SS 12	▲	12 16 20				similar to above material except mottled purple brown color.	Recovery 15"/18"	
SS 13	▲	13 14	266.1			similar to above material	Recovery 14"/18"	
						Purple Silty Sand (SM) mostly fine to medium sand,		

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-1





GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				F AREA INVESTIGATION			3 OF 6	FB-1
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
SS 33	▲	8 7 12	233.6	55		similar to above material except moisture content is increasing	Recovery 13"/18"	
SS 34	▲	12 10 13				similar to above material	Recovery 13"/18"	
SS 35	▲	12 15 20				Brown red Poorly Graded Sand (SP) ~90% fine to medium sand, <10% fines, nonplastic, medium dense, low to no dry strength.	Recovery 15"/18"	
SS 36	▲	13 13 15				similar to above material	Recovery 14"/18"	
SS 37	▲	16 21 23	227.8	60		similar to above material	Recovery 18"/18"	
SS 38	▲	11 16 19				similar to above material	Recovery 17"/18"	
SS 39	▲	10 5 6				Light brown Clayey Sand (SC) medium plasticity, soft.	Recovery 13"/18"	
SS 40	▲	9 11 12				Light brown Clay with sand (CH) kaolinitic, some medium sand, mostly fines highly plastic, moist	Recovery 16"/18"	
SS 41	▲	10 7 10	223.1	65		Light brown Clayey Sand (SC) mostly fine to medium sand, medium plasticity, kaolinitic laminations.	Recovery 17"/18"	
SS 42	▲	7 11 16				similar to above material		
SS 43	▲	10 13 17				Light brown Silty Sand (SM) 70% medium sand, 30% fines, medium plasticity to nonplastic, wet.	Recovery 15"/18"	
SS 44	▲	17 23 37				similar to above material except becoming red brown.	Recovery 13"/18"	
SS 45	▲	21 27 21	218.6	70		Red brown Well Graded Sand with Clay (SP-SC) mostly medium to coarse sand, medium plasticity, wet.	Recovery 8"/18"	
SS 46	▲	11 19 29				Red brown Clayey Sand (SC) ~80% fine sand, 20% fines low plasticity.	Recovery 12"/18"	
SS 47	▲	15 14 17				similar to above material	Recovery 8"/18"	
SS 48	▲	13 17 23				similar to above material	Recovery 8"/18"	
SS 49	▲	15 20 29	214.1	75		Light brown Poorly Graded Sand with Clay (SP-SC) mostly fine sand, medium plasticity to nonplastic, wet.	Recovery 8"/18"	
SS 50	▲	19 22 28				similar to above material	Recovery 7"/18"	
SS 51	▲	18 28 37				similar to above material	Recovery 6"/18"	
SS 51	▲					similar to above material	Recovery 8"/18"	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						HOLE NO.		
FINAL LOG						FB-1		



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				F AREA INVESTIGATION		4 OF 6	FB-1
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80						
SS 52		19 34 36	208.1			Brownish red Poorly Graded Sand (SP) 95% medium sand, <5% fines, nonplastic, dense.	Recovery 6"/18"
SS 53		9 24 21		85		similar to above material except becoming coarser (medium to coarse sand)	Recovery 5"/18"
SS 54	▲	6 6 7	205.1			Yellow brown Clay (CH) highly plastic, laminated with silt and sand.	Recovery 14"/18" -HCl
SS 55		13 22 26	203.9 203.4			Silty Sand (SM)	Recovery 16"/18" -HCl
SS 56		18 30 39	202.6 202.2			Yellow brown Clay (CH) highly plastic, laminated with silt and sand.	Recovery 12"/18" -HCl
SS 57		26 44 50		90		Yellow brown Clayey Sand (SC) mostly fine to coarse sand	Recovery 12"/18" -HCl
SS 58	▲	8 9 19				Light brown Poorly Graded Sand (SP) mostly fine to medium sand, dense.	Recovery 12"/18" -HCl
SS 59		33 50/4"				similar to above material	Recovery 12"/18" -HCl
SS 60		36 35 35		95		similar to above material except with a 2" layer of clay highly plastic, laminated, kaolin.	Recovery 6"/18" -HCl
SS 61		25 28 34	194.6 193.6			similar to above material	Recovery 8"/10" -HCl
SS 62		27 37 38				similar to above material	Recovery 7"/18" -HCl
SS 63		34 28 32	191.6	100		Light brownish yellow Well Graded Sand with Clay (SW-SC) mostly well graded fine to coarse quartz sand, with fat clay layers up to 1/4" thick.	Recovery 10"/18" -HCl
SS 64		17 23 12				Light brown Poorly Graded Sand (SP) mostly fine quartz sand, <5% fines, dense wet.	Recovery 8"/18" -HCl
SS 65	▲	7 6 7	188.4			similar to above material	Recovery 11"/18" -HCL
SS 66		14 21 24	186.3	105		Light brown Poorly Graded Sand with Silt (SP-SM) mostly fine quartz sand, trace coarse quartz sand, laminated.	Recovery 10"/18" -HCl
SS 67		13 23 27				similar to above material	Recovery 10"/18" -HCl
SS 68		20 21 23				similar to above material	Recovery 18"/18" -HCl
SS 69		8 15 21				Brownish yellow Clayey Sand (SC) ~20% coarse sand, 30% fine to medium sand, 30% fines, 20% lignite, soft to medium stiff	Recovery 10"/18" -HCl
SS 70		17 20 18		110		similar to above material	Recovery 10"/18" -HCl
						Light brown Poorly Graded Sand (SP) mostly medium sand trace lignite.	Recovery 10"/18" -HCl
						similar to above material	Recovery 16"/18" -HCl
						similar to above material	Recovery 5"/18" -HCl
						occasional rounded pebbles	Recovery 8"/18" -HCl
						similar to above material except becoming well graded medium to coarse sand, dense, <5% fines.	Recovery 8"/18" -HCl
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				FINAL LOG			HOLE NO. FB-1



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				F AREA INVESTIGATION		5 OF 6	FB-1
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80						
SS 71	▲	13 16 11	177.9			similar to above material	Recovery 6"/18" -HCl
SS 72	▲	8 3 3				Light brownish red Clayey Sand (SC) mostly fine to coarse sand, medium plasticity, soft.	Recovery 6"/18" -HCl
SS 73	▲	7 6 8				similar to above material except becoming highly plastic.	Recovery 24"/18" -HCl, sample expanded
SS 74	▲	6 5 5	171.1	115		Similar to above material except becoming laminated with lignite and clay stringers, soft to medium stiff, highly plastic.	Recovery 20"/18" -HCl, sample expanded
							overdrilled from 117.0 to 118.5'
SS 75	▲	1/8" 4 7				similar to above material	Recovery 20"/18" -HCl, losing fluid stabilizing @ 20-25'
SS 76	▲	10 23 24	164.6	120		Yellowish brown Poorly graded Sand with Clay (SP-SC) 70% fine sand, 20% fines, 10% lignite, low plasticity, moist, clay wisps. similar to above material	Recovery 7"/18" -HCl
SS 77	▲	18 23 27				similar to above material	Recovery 12"/18" -HCl
SS 78	▲	19 21 27				similar to above material except lignite content decreasing and clay content increasing	Recovery 10"/18" -HCl
SS 79	▲	19 25 28	155.1	125		similar to above material	Recovery 13"/18" -HCl
SS 80	▲	28 34 26				Light brown Poorly Graded Sand (SP) 90% fine sand, trace medium to coarse sand, 10% fines dense, wet.	Recovery 8"/18" -HCl
SS 81	▲	17 27 28				similar to above material except fines content decreasing to <1%	Recovery 7"/18" -HCl
SS 82	▲	24 24 28	151.9	130		similar to above material	Recovery 8"/18" -HCl PP 1.25 TSF
SS 83	▲	20 21 24				similar to above material except becoming very fine sand	Recovery 10"/18" -HCl PP 1.00 TSF
SS 84	▲	22 22 21				similar to above material	Recovery 10"/18" -HCl PP 1.25 TSF
SS 85	▲	11 14 7	155.1	135		trace shell fragments in 1" layer, fragments <1/16". similar to above material	Recovery 6"/18" -HCl PP 0.75 TSF
SS 86	▲	WR/18"				similar to above material	Recovery 20"/18" sample expanded -HCl PP 0.50 TSF
SS 87	▲	WR/18"				Light brown Clayey Sand (SC) 70% very fine sand, 30% fines medium plasticity. similar to above material except becoming highly plastic	Recovery 19"/18" sample expanded -HCl PP 0.50 TSF
SS 88	▲	WR/18"				similar to above material	Recovery 0"/18"
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						FINAL LOG	
SITE						HOLE NO. FB-1	



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 6 OF 6	HOLE NO. FB-1
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
SS 89		WR/18"		140		sand, mostly fines, highly plastic, soft, laminated with wispy layers of very thin clay, silicified shell fragments, low dry strength, slightly moist. similar to above material	Recovery 18"/18" -HCl PP 0.0 TSF	
SS 90	▲	WR/16"				similar to above material except denser and possibly slight silicification	Recovery 18"/18" -HCl PP 0.0 TSF PP 3.90 TSF (bottom)	
SS 91		WR/18"				similar to above material	Recovery 18"/18" -HCl PP 2.5 TSF	
SS 92	▲	7 9 15				similar to above material	Recovery 81"/18" -HCl PP 3.5 TSF	
			145.1	145		similar to above material except becoming less plastic, less moist, and medium stiff.	Recovery 20"/18" sample expanded -HCL PP 3.5 TSF	
SS 93	▲	17 23 29				similar to above material except light olive green color.	Recovery 16"/16" -HCl PP +4.5 TSF	
SS 94		24 34 50/4"	142.3			becoming mottled light olive and brown, appears burrowed, medium plasticity, stiff.	Recovery 10"/8" sample expanded -HCl PP +4.5 TSF	
SS 95		34 50/2"	141.6 141.3	150		Brown Poorly Graded sand (SP) mostly fine sand, cemented.	Recovery 18"/18" -HCl PP 4.0 TSF	
SS 96	▲	23 21 25				Light olive green Silt with sand (ML) mottled, stiff some fine sand to pebble size, mostly fines, dry, highly plastic with water added. similar to above material. becoming Dark Green		
				155				
SS 97		24 43 50/4"	134.6 134.1			Congaree Brown Well Graded Sand (SW) mostly quartz sand, < 20% fines, dense, moist.	Recovery 16"/16" -HCl PP +4.5 TSF	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-1



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 1 of 6	HOLE NO. FB-2
SITE Pu Repackaging Facility			COORDINATES N 79,102 E 54,920			ANGLE FROM HORIZONTAL Vertical		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH		
9-11-95	9-18-95	EEI	Mobile B-57	6"	N/A	151.0		
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY: F.H. Syms					
292.2		65.0/227.2 65'						
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20 40 60 80		292.2			Hand excavated from surface to 3.0' SP-SC type material with cobbles and road aggregate.	Hole was advanced using mud rotary drilling methods with a 5" drag bit from 0.0' to 156.5'. "N" rods were used for shelly tube sampling from 0.0' to 156.5'.
ST 1				289.2			Brown Sand (SP) 95% fine sand, 5% fines, medium dense to dense.	Recovery 24"/24"
ST 2					5		similar to above material	Recovery 24"/24"
ST 3				285.4			similar to above material except Light brown color	Recovery 24"/24"
ST 4				283.2			similar to above material except brown color	Recovery 24"/24"
ST 5				281.3	10		Tan Silty Sand (SM)	Recovery 22"/12" slough in tube
ST 6				279.3			Reddish brown and brown Clayey Sand (SC) 70% fine to medium sand, 30% fines, medium plastic.	Recovery 23"/24"
ST 7					15		similar to above material except red brown	Recovery 24"/24"
ST 8							grades to gray Sandy Clay	Recovery 0"/21"
ST 9								Recovery 0"/12"
ST 10					20			Recovery 16"/24"
ST 11							Red Brown Clayey Sand (SC)	Recovery 24"/24"
				267.8			grades into Light purple Silty Sand (SM)	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-2



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 2 OF 6	HOLE NO. FB-2
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ WATER CONTENT % + ATT. LIMITS %						
		20 40 60 80						
ST 12				265.8			similar to above material	Recovery 15"/23"
ST 13							grades in to reddish brown Clayey Sand (SC)	Recovery 17.5"/24"
ST 14					30		similar to above material	Recovery 21"/24"
				261.2			similar to above material drilled to 35.0' without sampling	
ST 15					35		Clayey Sand (SC)	Recovery 14"/24"
				255.6			Drilled to 40.0' without sampling	
ST 16					40		Brownish red Clayey Sand (SC)	Recovery 17"/17"
				250.8			Drilled to 50.0' without sampling	
					45			Sampling intervals determined from adjacent CPT-2
ST 17					50		light brown and white Clayey Sand (SC)	Recovery 11"/18"
				240.7			Drilled to 60.0' without sampling	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-2



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 3 OF 6	HOLE NO. FB-2
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS % 20 40 60 80	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
ST 18			230.2	55		Light brown Sand (SP)	Recovery 10"/24"	
				60		Drilled to 75.0' without sampling		
				65				
				70				
ST 19			215.8	75		Red brown Clayey Sand (SC) with coarse quartz sand	Recovery 17"/24"	
				80		Drilled to 83.0' without sampling		

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-2



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				F AREA INVESTIGATION			4 OF 6	FB-2
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
ST 20			207.2	85		Red brown Clayey Sand (SC)	Recovery 24"/24"	
						Drilled to 100.0' without sampling		
				90				
				95				
				100		Red Brown Clayey Sand (SC) fine to medium quartz sand.	Recovery 12"/12"	
ST 21			191.2			Drilled to 120.0' without sampling		
				105				
				110				

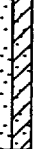
SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-2



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 5 OF 6	HOLE NO. FB-2
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ WATER CONTENT % + ATT. LIMITS % 20 40 60 80						
ST 22				170.9	120		Light brown Poorly Graded Sand with Clay (SP-SC) mostly fine sand.	Recovery 16"/16"
					125			
					130			
					135			
							Drilled to 145.0' without sampling	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-2



GEOTECHNICAL LOG				PROJECT F AREA INVESTIGATION		JOB NO.	SHEET NO. 6 OF 6	HOLE NO. FB-2
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ WATER CONTENT % + ATT. LIMITS % 20 40 60 80	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
				140				
ST 23				145		Light brown Silt with Sand (ML)		Recovery 20"/24"
ST 24						similar to above material		Recovery 20"/24"
ST 25				150		similar to above material except yellow brown in color.		Recovery 19"/24"
			141.2			Total depth of boring 151.0 feet		

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-2



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	1 OF 5	FB-3			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF			N 79439 E 54990			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/16/98	2/19/98	Graves/S. Rodgers	Failing 1500		3 7/8 in	140 lb/ 30 in		154.5			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
287.2		2 /	R. Gelinis/SAIC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY %									
		+ ATT. LIMITS %									
		20	40	60	80		287.2				
								5			Hand auger to 6 feet to check for underground interferences, then used 5 3/4 tricone for 1st 20 feet or so (19.5).
SS 1						5-14-15				CLAYEY SAND (SC); medium grayish brown and medium red; medium dense; damp; subangular; poorly graded; fine to medium grained	Shoe came off in hole.
SS 2						9-26-21	279.7			no recovery	
SS 3						23-40-44	278.2			same as above; medium reddish brown; very dense; moist	
SS 4						14-25-13	276.7	10		same as above; with some gray zones; dense	
SS 5						6-8-11	275.2			same as above; medium brownish red with some yellowish brown and gray zones; medium dense	
SS 6						11-12-15	273.7			SANDY LEAN CLAY (CL); with clayey sand interbeds; medium red; very stiff; damp; medium plasticity; sand fraction is fine to medium grained	
SS 7						10-15-17	272.2	15		same as above; with some yellowish gray oxidation zones; hard	
SS 8						10-14-17	270.7			same as above; dark red; sand fraction is fine grained	
SS 9						8-16-13	269.2			CLAYEY SAND (SC); with sandy lean clay portions; medium brownish red; medium dense; moist; subangular; poorly graded; fine to medium grained	Tobacco Road
SS 10						6-10-13	267.7	20		SANDY LEAN CLAY (CL); with clayey sand portions; medium red; very stiff; moist; medium plasticity; sand is fine to medium grained	
SS 11						10-17-26	266.2			same as above; medium brownish red; hard	
SS 12						10-33-36	264.7			no recovery	
SS 13						15-23-21	263.2			CLAYEY SAND (SC); medium reddish brown; dense; moist; subangular; poorly graded; fine to medium grained	
SS 14						36-50/5in	261.7	25		same as above; very dense	
SS 15						11-13-12	260.8			same as above; medium yellowish brown; medium dense	
SS 16						9-13-14	258.7			SILTY SAND (SM); with clay; medium grayish brown and reddish brown; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 17						11-12-14	257.2	30		CLAYEY SAND (SC); medium yellowish brown and brownish red; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 18						14-13-12	255.7			same as above	
SS 19						18-13-12	254.2			same as above	
							252.7				

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-3



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	2 OF 5	FB-3
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
SS 20		21-16-14	251.2			same as above; medium brownish yellow and yellowish brown	Dry Branch	
SS 21		16-18-17	249.7			SILTY SAND (SM); medium yellowish brown; dense; moist; subangular; poorly graded; fine to medium grained		
SS 22		2-2/24in				no recovery		
SS 23		14-20-19	247.2	40		same as above; with clay; medium brownish red reddish brown and some yellowish brown; wet		
SS 24		18-14-17	245.7			CLAYEY SAND (SC); medium reddish brown; dense; wet; subangular; poorly graded; fine to medium grained		
SS 25		24-20-17	244.2			same as above; medium yellowish brown and brownish red with few light gray zones		
SS 26		19-14-13	242.7	45		POORLY GRADED SAND WITH SILT (SP-SM); some portions are silty sand; light brown to medium reddish brown; medium dense; wet; subangular; poorly graded; fine to medium grained		
SS 27		12-15-24	241.2			CLAYEY SAND (SC); with silty sand interbeds; medium brown; dense; wet; subangular; poorly graded; fine to medium grained with trace lower coarse		
SS 28		22-29-24	239.7			POORLY GRADED SAND WITH SILT (SP-SM); medium yellowish brown; very dense; wet; angular; poorly graded; medium to lower coarse grained		
SS 29		40-31-34	238.2			no recovery		
SS 30		18-16-21	236.7	50		CLAYEY SAND (SC); dark red; dense; wet; subangular; poorly graded; fine to lower coarse grained		
			235.2					
				55				
SS 31		12-16-22				SILTY SAND (SM); medium reddish brown; dense; wet; subangular; poorly graded; fine to medium grained	Driller overdrilled interval? (52').	
SS 32		19-20-24	228.7			same as above; medium brown; fine to lower coarse grained		
SS 33		16-17-18	227.2	60		same as above; medium brown and yellowish brown		
SS 34		8-15-12	225.7			CLAYEY SAND (SC); medium yellowish brown with light gray zones; medium dense; moist; subangular; poorly graded; very fine to fine grained		
SS 35		10-10-11	224.2			same as above; wet; fine to medium grained		
			222.7	65			Driller didn't clean out and took spoon over previous interval (13-16-12) bad blow count.	
SS 36		15-12-16				same as above; medium brown; very fine to fine grained		
SS 37		17-17-23	220.0			SILTY SAND (SM); medium brown; dense; wet; subangular; poorly graded; fine to medium grained		
SS 38		20-22-24	218.5			CLAYEY SAND (SC); medium reddish brown; dense; wet; angular; well graded; fine to coarse grained		
SS 39		26-19-13	217.0	70		same as above		
SS 40		5-6-7	216.0			SILTY SAND (SM); light brown to medium brown; dense; wet; subangular; poorly graded; fine to medium grained		
			215.5			CLAYEY SAND (SC); medium yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained; trace lower coarse		
SS 41		9-10-9	214.0			same as above; medium brown; moist; angular; well graded; fine to upper coarse grained		
			213.2					
			212.5					
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE		FINAL LOG			HOLE NO. FB-3



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-3
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 42	20	8-7-5	211.0			same as above; wet; subangular; poorly graded; fine to med grained	
SS 43	40	8-6-8	209.5			same as above; med yellowish brown; fine to lower med grained	
SS 44		9-11-10	208.0			same as above; fine to medium grained with some fine to coarse grained layers	Core catcher is good.
SS 45		6-9-13	207.6	80		same as above; fine to medium grained	Top Tan Clay
SS 46		3-9-8	206.9			SANDY LEAN CLAY (CL); lt yellowish brown with some dk gray patches; very stiff; moist; med plasticity; sand fraction is fine to med grained	
SS 47		5-5-11	206.5			same as 79.2-79.6; two jars	Bottom Tan Clay
SS 48		4-6-16	205.0			SANDY LEAN CLAY (CL); med yellowish brown; very stiff; moist; med plasticity; sand fraction is fine to upper med grained	
SS 49		10-13-23	203.5			SILTY SAND (SM); med yellowish brown; med dense; wet; subangular; poorly graded; fine to med grained	
SS 50		19-24-32	202.0	85		POORLY GRADED SAND WITH SILT (SP-SM); light brown; medium dense; wet; subangular; poorly graded; fine to upper medium grained	
SS 51		19-22-17	200.5			SILTY SAND (SM); light brown; dense; wet; subangular; poorly graded; fine to medium grained	Catcher is good.
SS 52		8-12-8	199.0			POORLY GRADED SAND WITH SILT (SP-SM); light brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 53		2-3-3	197.5	90		SILTY SAND (SM); portions with clay; medium yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	Driller overdrilled interval? (52')
SS 54		4-2-5	196.0			CLAYEY SAND (SC); medium yellowish brown; loose; wet; subangular; poorly graded; fine to medium grained	
SS 55		9-12-20	194.5			same as above	
SS 56		9-21-26	193.0	95		SILTY SAND (SM); portions are poorly graded sand with silt; medium brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 57		15-13-14	191.5			no recovery	Catcher is good.
SS 58		2-6-9	189.4			SILTY SAND (SM); light brown to medium brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 59		6-7-12	187.9	100		CLAYEY SAND (SC); some portions are silty sand; medium brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 60		6-3-9	186.4			POORLY GRADED SAND WITH CLAY (SP-SC); some silty sand layers; med brown; med dense; wet; subangular; poorly graded; fine to lower cse grading down to fine to med	
SS 61		4-7-11	184.4	105		CLAYEY SAND (SC); med yellowish brown; med dense; wet; subangular; poorly graded; fine to med grained	
SS 62		13-31-41	182.9			same as above; some thin poorly graded sand with clay layers; medium brown	
SS 63		14-30-44	181.4			no recovery	Catcher is good.
SS 64		17-22-29	179.9			POORLY GRADED SAND WITH SILT (SP-SM); medium brown; very dense; wet; subangular; poorly graded; fine to medium grained	
SS 65		18-32-32	177.9	110		same as above; with some silty sand and poorly graded sand with clay interbeds; light brown	Driller didn't clean out and took spoon over previous interval 65.7-67.2 ft. (13-16-12) bad blow count.
SS 66		15-16-14	176.4			same as above; light brown to medium brown; angular; medium grained	
SS 67		4-5-8	174.9			POORLY GRADED SAND (SP) trace silt; with some poorly graded sand with silt layers; light brown to medium brown; medium dense; wet; angular; poorly graded; fine to lower coarse grained	Santee? Changed to metal
SS 67			173.4			no recovery	HOLE NO. FB-3

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	4 OF 5	FB-3
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
SS 68	20 40 60 80	8-14-23	171.9			CLAYEY SAND (SC); light grayish brown and yellowish brown; dense; moist; subangular; poorly graded; fine grained; distinctive thin laminations	catcher.	
SS 69		6-10-14	170.4			same as above; medium yellowish brown and yellowish gray; medium dense; fine to medium grained		
SS 70		14-21-29	168.9			same as above; with shell fragments; dense; wet; angular; well graded; fine to coarse grained		
SS 71		15-40-48	167.4	120		same as above; medium yellowish brown and reddish brown; very dense; subangular; poorly graded; very fine to fine grained	Metal catcher is good.	
SS 72		18-24-19	165.9			SILTY SAND (SM); trace fine shell fragments; medium brownish yellow and light yellowish gray; dense; wet; subangular; poorly graded; very fine to fine grained		
SS 73		22-32-39	164.4			same as above; medium yellowish brown; very dense; fine grained; trace upper medium		
SS 74		25-35-41	162.9	125		same as above; fine grained		
SS 75		26-21-44	161.4			no recovery	Plastic orange catcher has two teeth missing.	
SS 76		17-31-42	159.9			POORLY GRADED SAND WITH SILT (SP-SM); medium yellowish brown; very dense; wet; subangular; poorly graded; fine to medium grained		
SS 77		23-30-38	158.4			SILTY SAND (SM); portions are poorly graded sand with silt; medium yellowish brown; very dense; wet; subangular; poorly graded; fine to lower medium grained		
SS 78		17-33-31	156.9	130		same as above; with thin orangish brown clayey sand interbed; light gray; very fine to fine grained		
SS 79		19-21-26	155.4			same as above; few very thin light gray clay laminations; medium yellowish brown; dense; very fine grained		
SS 80		14-29-21	153.9			same as above; with light gray limestone fragments and some clayey sand interbeds; fine to lower coarse grained; limestone fragments to 0.5 inch diameter		
SS 81		6-3-4	152.4	135		same as above; trace turritella shells; loose; very fine to fine grained		
SS 82		6-6-10	150.9			CLAYEY SAND (SC); medium yellowish brown; medium dense; wet; subangular; poorly graded; very fine to fine grained		
SS 83		5-9-12	149.4			same as above; with turritella shells; light yellowish brown and yellowish gray; moist		
SS 84		7-9-11	147.0	140		same as above; trace shell and limestone fragments; medium yellowish brown and light gray		
SS 85		8-12-16	145.5			same as above		
SS 86		12-10-24	144.0			same as above; with sandy lean clay interbeds; dense; wet; angular; fine to lower coarse grained		
SS 87		10-8-11	142.5	145		SANDY SILT (MH); medium yellowish brown; very stiff; wet; low plasticity; sand fraction is very fine to fine grained	Warley Hill	
SS 88		2-7-12	141.0			no recovery	Plastic orange catcher is broken.	
SS 89		3-18-20	139.5			no recovery	New catcher has several teeth broken off.	
SS 90		13-8-17	138.0			SANDY LEAN CLAY (CL); silicified nodule at 149.7 ft; dark yellowish brown and yellowish gray; very stiff; moist; medium plasticity; sand fraction is very fine to fine grained		
SS 91		11-21-40	136.5	150		no recovery	New catcher has several teeth missing.	
SS 92		17-20-38	135.0			CLAYEY SAND (SC); medium reddish brown and grayish brown; very dense; moist; angular; poorly graded; fine to lower coarse grained		
SS 93		40-50/3in	133.6			same as above; dark grayish green; subangular; fine to medium grained	Congaree	
			132.7					
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE		FINAL LOG			HOLE NO. FB-3



GEOTECHNICAL LOG					PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
					APSF		APSF	5 OF 5	FB-3			
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
		○ RECOVERY %	+ ATT. LIMITS %									
		20	40	60	80							
										same as above; with lean clay and silty sand interbeds (highly variable); medium reddish brown, dark grayish red, and dark grayish green; wet; angular; fine to lower coarse grained Total depth of boring 154.5 feet.	Hole abandoned with with grout mix per 3Q5.	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE					HOLE NO. FB-3	

FINAL LOG



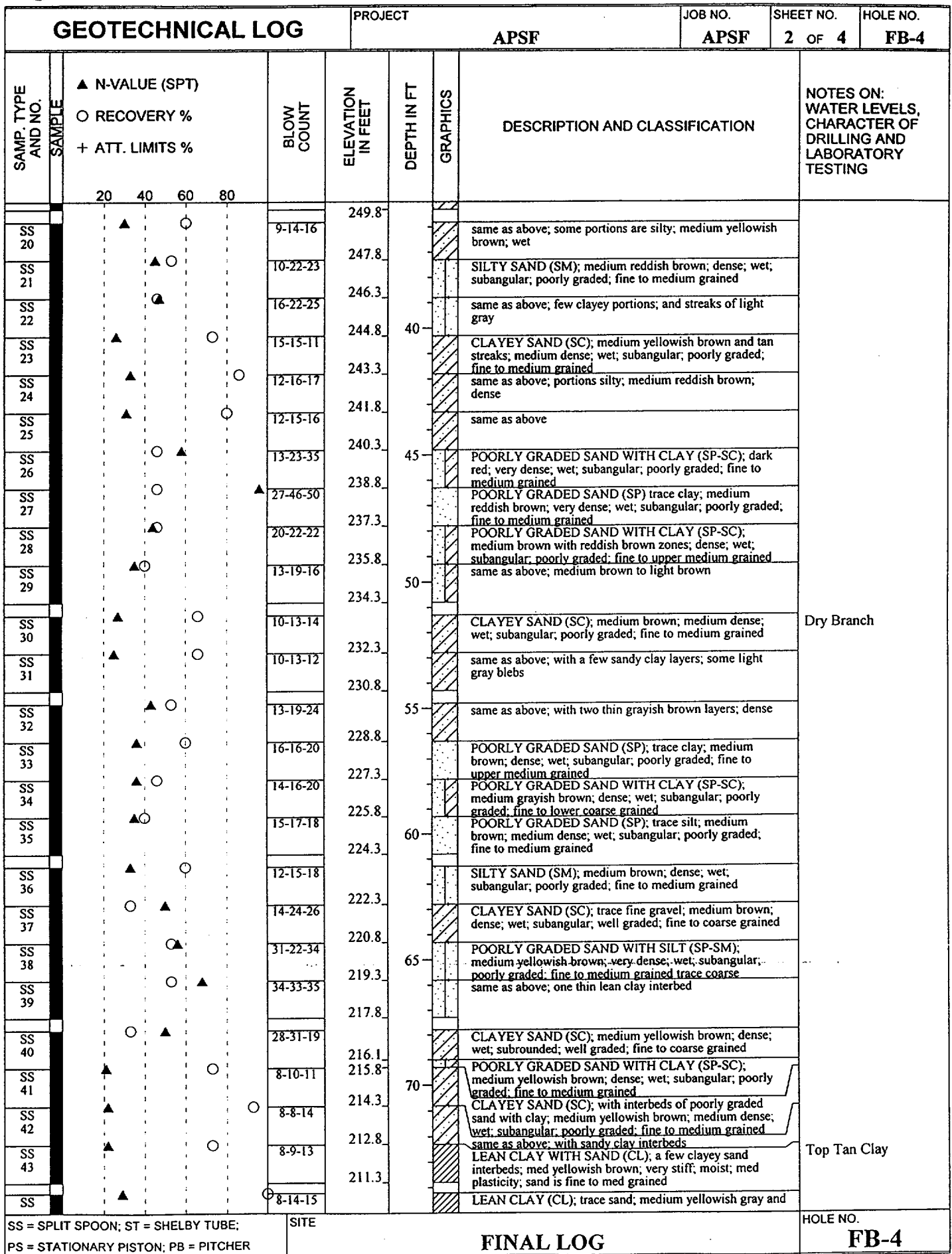
GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	1 OF 4	FB-4			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF			N 79425 E 54865			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/27/98	3/4/98	Graves/S. Rodgers	Failing 1500		3 7/8 in	140 lb/ 30 in		152.4			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
285.1		▽ / ▽ /	R. Gelinis/SAIC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						285.1					Hand auger to 6 feet to check for underground interferences.
								5			
SS 1		○	▲			10-14-14	277.6			CLAYEY SAND (SC); medium reddish brown; medium dense; wet; subangular; poorly graded; fine to medium grained; lost shoe in hole	Lost shoe down hole.
SS 2			▲		○	10-20-20	276.1			SANDY LEAN CLAY (CL); medium red; hard; moist; medium plasticity; sand fraction is fine to medium grained	
SS 3			▲		○	9-13-20	274.6	10		same as above	
SS 4			▲		○	6-12-13	273.1			same as above; medium brownish red; very stiff	
SS 5			▲		○	9-13-15	271.6			CLAYEY SAND (SC); medium brownish red and yellowish brown; medium dense; moist; subangular; poorly graded; fine to medium grained	Tobacco Road
SS 6			▲		○	7-11-13	270.1	15		same as above; medium red	
SS 7		○	▲		○	8-13-14	268.6			same as above; medium brownish red; fine grained	
SS 8			▲		○	18-19-19	267.1			same as above; dense; wet; fine to lower coarse grained	
SS 9			▲		○	2-14-19	265.6	20		CLAYEY SAND WITH GRAVEL (SC); medium reddish brown; dense; wet; subangular; well graded; fine to coarse grained; gravel to 2 inch diameter	
SS 10			▲		○	3-12-16	264.1			SANDY LEAN CLAY (CL); medium reddish brown, maroon, yellowish brown, and tan; very stiff; moist; medium plasticity; sand fraction very fine to fine grained	
SS 11			▲		○	6-9-14	262.6			SANDY SILT (ML); with some clayey sand interbeds; medium red, maroon, medium gray, and grayish yellow; very stiff; moist; low plasticity; sand fraction is very fine grained	
SS 12			▲		○	11-13-12	261.1	25		CLAYEY SAND (SC); medium yellowish brown with light yellowish gray blebs; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 13			▲		○	13-18-21	259.6			same as above; portions silty; dense; moist	
SS 14			▲		○	11-16-20	258.1			same as above	
SS 15			▲		○	10-16-15	256.6			same as above	
SS 16			▲		○	8-10-14	255.1	30		same as above; medium yellowish brown and reddish brown; medium dense	
			▲		○						
SS 17			▲		○	8-11-10	252.8			same as above; medium reddish brown	
SS 18			▲		○	8-10-11	251.3			same as above; medium yellowish brown with some reddish brown and light gray zones	
SS 19			▲	○		11-11-14				same as above; medium reddish brown	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-4





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 4	FB-4
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
44	20		209.4			yellowish brown; very stiff, moist; medium plasticity; sand fraction is very fine grained	Bottom Tan Clay Catcher has one tooth broken.
SS 45		7-8-12	207.9			same as above; abundant black Mn? staining; sand is fine grained	
SS 46		6-8-12	206.4			no recovery	
SS 47		5-8-12	203.8	80		CLAYEY SAND (SC); medium yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained; trace coarse grains	
SS 48		6-11-10	202.3			same as above; moist; very fine to fine grained	
SS 49		5-7-15	200.8			same as above; light gray, tan, and yellowish brown; wet; fine to medium grained; black Mn? staining	
SS 50		12-19-17	199.3	85		same as above; very light yellowish green with medium orange mottles; dense	
SS 51		15-28-36	197.8			POORLY GRADED SAND WITH CLAY (SP-SC); light brown and reddish brown; very dense; wet; subangular; poorly graded; medium grained; trace heavies	
SS 52		22-27-24	196.3			same as above with very light brown band of cleaner, finer material	
SS 53		11-14-17	194.8	90		same as above; some zones are borderline clayey sand; dense	Santee
SS 54		7-7-12	192.4			CLAYEY SAND (SC); medium yellowish brown and grayish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 55		17-18-16	190.9			POORLY GRADED SAND (SP); trace silt; light brown; dense; wet; angular; poorly graded; medium to coarse grained	
SS 56		7-7-8	189.1	95		CLAYEY SAND (SC); light yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 57		15-26-39	187.6			POORLY GRADED SAND (SP); trace silt; light brown; very dense; wet; subangular; poorly graded; medium to coarse grained; tip of spoon as above but clayey	
SS 58		17-35-41	186.8			same as above	
			186.1			CLAYEY SAND (SC); light yellowish brown; very dense; wet; subangular; poorly graded; fine to medium grained	
SS 59		21-48-45	184.5	100		POORLY GRADED SAND (SP); trace silt; light brown; very dense; wet; subangular; poorly graded; medium to coarse grained	
SS 60		18-44-51	183.8			CLAYEY SAND (SC); medium brown; very dense; wet; subangular; poorly graded; fine to medium grained	
SS 61		20-35-36	182.3			POORLY GRADED SAND (SP); trace silt; light brown; very dense; wet; subangular; poorly graded; medium to coarse grading down to lower medium	
SS 62		16-18-24	180.8	105		same as above; trace silt and gravel; medium brown; fine to lower coarse grained	
SS 63		17-20-14	179.3			same as above; trace clay; light brown to medium brown; dense; medium to lower coarse grained	
SS 64		5-2-2	177.8			POORLY GRADED SAND WITH SILT (SP-SM); light brown to medium brown; dense; wet; subangular; poorly graded; fine to lower coarse grained	
			176.3			CLAYEY SAND (SC); medium yellowish brown, brown, and dark gray; very loose; wet; subangular; poorly graded; fine to lower coarse grained; Mn? staining	
SS 65		1-2-3	174.5	110		same as above; loose	
SS 66		1-2-1/9in	173.8			SANDY FAT CLAY (CH); light brownish gray, tan, and black; firm; moist; high plasticity; sand fraction is fine to medium grained; Mn? staining	
			172.1			same as above; jar #1; soft	
SS		7-7-9	171.5			CLAYEY SAND (SC); light grayish brown, tan, and yellowish brown; very loose; wet; subangular; poorly graded; fine to medium grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE	FINAL LOG		HOLE NO. FB-4



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	4 OF 4	FB-4
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
67		5-14-21	169.7			same as above; medium yellowish brown and yellowish gray; medium dense	
SS 68						same as above; dense	
69		6-14-17	168.2			same as above; fine to lower coarse grained	
SS 70		11-19-21	167.1			POORLY GRADED SAND WITH SILT (SP-SM); medium yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 71		16-16-16	166.7			CLAYEY SAND (SC); trace shell material; medium yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained; black Mn? staining	
SS 72		13-18-19	165.2	120		SILTY SAND (SM); med yellowish brown; dense; wet; subangular; poorly graded; fine grained	
SS 73		27-34-34	163.8			CLAYEY SAND (SC); med yellowish brown; dense; wet; subangular; poorly graded; fine to med grained trace lower coarse	
SS 74		21-32-34	163.4			SILTY SAND (SM); med yellowish brown; dense; wet; subangular; poorly graded; v. fine to fine grained; trace Mn? staining	
SS 75		19-30-23	161.9	125		same as above; It yellowish brown; very dense	
SS 76		17-23-34	160.4			same as above; thin lt gray lean clay laminations; med yellowish brown	
SS 77		8-8-10	158.9			same as above; fine to lower medium grained	
SS 78		8-7-4	157.4			POORLY GRADED SAND WITH SILT (SP-SM); med brownish yellow with orange mottles; very dense; wet; subangular; poorly graded; fine to med grained; trace heavies	
SS 79		1-3-4	155.9	130		same as above; lt brownish white with med orange laminae; med dense; fine grained	At 130 feet depth check OK.
SS 80		6-2-5	154.4			POORLY GRADED SAND WITH CLAY (SP-SC); lt brownish white with lt orangish brown zones; med dense; wet; subangular; poorly graded; fine grained; trace heavies and mica	
SS 81		WR 6in-11-9	152.9			same as above; med yellowish brown with very lt brown wisps; loose	
SS 82		7-10-13	151.4	135		CLAYEY SAND (SC); med yellowish brown; loose; wet; subangular; poorly graded; fine grained; trace heavies and mica	
SS 83		11-15-16	149.9			same as above; slightly higher clay content and lt green wisps; med dense; black wisps (Mn?); turritella fragments	
SS 84		4-9-18	148.4			same as above; med reddish brown with lt green wisps and dk reddish orange mottles	
SS 85		9-35-28	146.9	140		same as above; with zones of poorly graded sand with clay; med yellowish brown; dense; fine to med grained	At 138 feet depth check OK.
SS 86		13-24-24	145.4			same as above; med dense; fine grained; black mottles (Mn?); turritella casts	
SS 87		11-11-11	143.9	145		same as above; very dense	
SS 88		7-50/4in	142.4			FAT CLAY WITH SAND (CH); medium brownish yellow; hard; wet; high plasticity; sand is fine grained; trace mica; trace heavies; shoe lost downhole	Warley Hill
SS 89		11-21-34	140.9			same as above; very stiff; (material may be sloughed)	
SS 90		9-13-18	139.4	150		same as above; very hard; refusal caused by a silicified nodule lodged in the shoe	At 146 depth check off adjusted to 146.2.
SS 91		31-50/4in	138.1			LEAN CLAY (CL); trace sand; with interlaminated sand; very light gray with orange mottles; hard; wet; medium plasticity; sand is fine grained; mottles are clayey sand no recovery; shoe lost downhole	
SS 92		50/2in	135.9			CLAYEY SAND (SC); with gravel; very dark grayish green; very dense; wet; subangular; well graded; fine to coarse grained; 'green clay'	
			134.4			no recovery	
			133.6			Total depth of boring 152.4 feet.	Hole abandoned with grout mix per 3Q5.
			132.7				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-4



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	1 OF 5	FB-5			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF			N 79233 E 54977			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/12/98	2/20/98	Graves/A. Jackson	Failing 1500		3 7/8 in	140 lb/ 30 in		158.8			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
290.5		7 / 7	N. Kidd/SAIC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						290.5					Hand auger to 6 feet to check for underground interferences.
SS 1		▲	○			4-8-13	283.0			POORLY GRADED SAND (SP) trace clay; light brown; medium dense; dry; subangular; poorly graded; fine to medium grained	
SS 2			▲	○		12-18-27	281.5			CLAYEY SAND (SC); medium brownish red and light brown; dense; damp; subangular; poorly graded; fine to medium grained; black mottles (organic material?)	
SS 3		▲			○	8-14-17	280.0	10		POORLY GRADED SAND WITH CLAY (SP-SC); medium red with light brown mottles; dense; damp; subangular; poorly graded; fine to medium grained	
SS 4		▲			○	12-17-22	278.5			CLAYEY SAND (SC); light yellowish brown, light yellow, and light brown; dense; damp; subangular; poorly graded; fine to medium grained	
SS 5			▲		○	16-28-26	277.0			same as above; medium red, light yellow, and white; very dense	
SS 6		▲			○	10-13-17	275.5	15		same as above; medium red; medium dense; moist	
SS 7		▲			○	12-16-15	274.0			same as above; damp	
SS 8		▲			○	8-9-10	272.5			LEAN CLAY WITH SAND (CL); medium red with light yellow zones; very stiff; damp; medium plasticity; sand is fine to medium grained	
SS 9		▲			○	6-10-14	271.0			same as above; coarse fraction is fine grained sand to fine gravel	
SS 10		▲			○	6-8-6	269.5	20		same as above; stiff; with slightly more fine gravel	
SS 11		▲			○	7-10-8	268.0			SANDY LEAN CLAY (CL); medium red; very stiff; damp; medium plasticity; sand fraction is fine to coarse grained	
SS 12		▲			○	4-5-5	266.5			CLAYEY SAND (SC); medium red; loose; damp; subangular; poorly graded; fine to medium grained	Tobacco Road
SS 13		▲			○	4-7-9	265.0	25		same as above; medium dense; with occasional coarse grains	
SS 14		▲			○	7-10-11	263.5			SILTY SAND (SM); medium red with white and yellow bands and sparse purple mottles; medium dense; damp; subangular; poorly graded; fine grained	
SS 15		▲			○	8-9-12	262.0			same as above; trace mica; no mottles	
SS 16		▲			○	8-9-11	260.5	30		SILT (ML) trace sand; medium red, purple, white, orange, and yellow; very stiff; damp; medium plasticity; sand fraction is fine grained	
SS 17		▲			○	7-12-16	259.0			same as above; low plasticity	
SS 18		▲			○	7-11-12	257.5			CLAYEY SAND (SC); silty in places; light yellow and med red; med dense; moist; subangular; poorly graded; fine to med grained; top of the interval is silt	
SS 19		▲			○	8-9-9	256.0			POORLY GRADED SAND WITH CLAY (SP-SC); med yellowish brown and reddish brown; med dense; damp; subangular; poorly graded; fine to med grained	

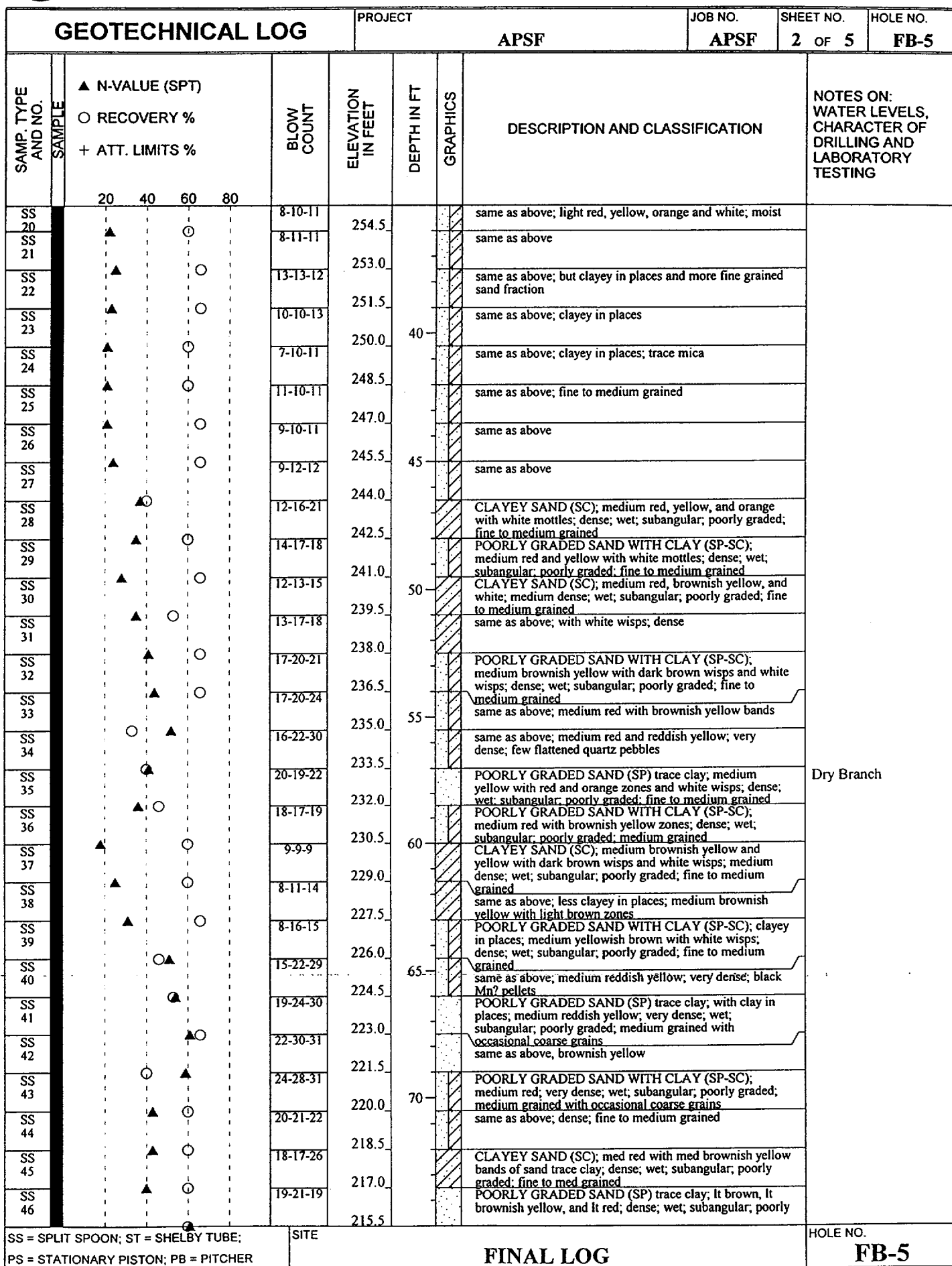
SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-5








GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-5
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 47		17-29-32				graded: fine to med grained	Sample fell out of the tube onto the ground.
SS 48		18-20-22	214.0			POORLY GRADED SAND WITH CLAY (SP-SC); med reddish brown, lt brown, and brownish yellow; very dense; wet; subangular; poorly graded; fine to med grained	
SS 49		12-13-15	212.5			WELL GRADED SAND (SW) trace clay; lt brown and lt reddish brown; dense; wet; subrounded; well graded; fine to coarse grained	Top Tan Clay
SS 50		10-11-12	211.0			same as above; med dense	
SS 51		10-10-10	209.5	80		POORLY GRADED SAND (SP) trace clay; lt brown few lt brownish yellow zones; med dense; wet; subrounded; poorly graded; fine to med grained and stly more clay at top of interval; fine to cse grained at bottom of interval; black Mn? pellets	Bottom Tan Clay
SS 52		2-1-2	208.0			POORLY GRADED SAND WITH CLAY AND GRAVEL (SP-SC); lt brown with lt yellowish brown zones; med dense; wet; subrounded; well graded; fine grained sand to gravel size; some zones less clayey; black Mn? pellets	
SS 53		5-7-10	206.5			SILTY SAND (SM); lt yellowish brown; very loose; wet; subangular; poorly graded; fine grained	Top Tan Clay
SS 54		7-8-10	205.0	85		LEAN CLAY (CL) trace sand; lt brownish yellow with orange sand stringers; very stiff; wet; medium plasticity; interlaminated orange fine sand	
SS 55		6-8-9	203.5			same as above; sand stringers are dk brown and black; Mn? pellets	Bottom Tan Clay
SS 56		7-12-17	202.0			same as above; with sand and interbedded clayey sand; sand is fine to coarse grained; black Mn? pellets	
SS 57		21-27-30	200.5	90		POORLY GRADED SAND (SP) trace clay; lt brown; med dense; wet; subangular; poorly graded; fine to med grained	Top Tan Clay
SS 58		38-50/5in	199.0			same as above; very dense	
SS 59		50-50/4in	198.1			same as above; with a band of clayey sand	Bottom Tan Clay
SS 60		27-38-35	196.7			same as above; very lt brown; trace heavy minerals	
SS 61		22-33-35	194.5	95		same as above; with a zone of poorly graded sand with clay	Top Tan Clay
SS 62		20-22-34	193.0			POORLY GRADED SAND WITH CLAY (SP-SC); lt red with very lt brown zone at top; very dense; wet; subangular; poorly graded; fine to med grained	
SS 63		25-28-37	191.5			same as above; clayey in places; med red with lt red zones	Bottom Tan Clay
SS 64		39-36-41	190.0	100		same as above; med red grades into lt brown zone at bottom of interval	
SS 65		7-11-17	188.5			POORLY GRADED SAND (SP) trace clay; med brown with a lt red band; very dense; wet; subangular; poorly graded; med grained	Top Tan Clay
SS 66		21-24-33	187.0			CLAYEY SAND (SC), some zones less clayey; lt yellowish brown with white wisps; med dense; wet; subangular; fine to coarse grained	
SS 67		8-18-33	185.5	105		POORLY GRADED SAND WITH CLAY (SP-SC); lt brownish yellow; very dense; wet; subangular; poorly graded; fine to med grained; black Mn? wisps	Bottom Tan Clay
SS 68		27-43-50	184.0			POORLY GRADED SAND (SP) trace clay; lt brown; very dense; wet; subangular; poorly graded; fine to med grained; black Mn? pellets	
SS 69		34-40-40	182.5			same as above; without black pellets	Top Tan Clay
SS 70		28-41-42	181.0	110		same as above; with occasional coarse grains; trace heavies	
SS 71		33-42-50	179.5			same as above	Bottom Tan Clay
SS 72		18-18-18	178.0			same as above; with sporadic clay pods	
SS		12-34-42	176.5			POORLY GRADED SAND WITH CLAY (SP-SC), and lt brownish yellow clay stringers; lt brown and lt yellow; dense; wet; subangular; poorly graded	Top Tan Clay
SS						CLAYEY SAND (SC), with less clayey zones and a clean	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE	FINAL LOG		HOLE NO. FB-5



GEOTECHNICAL LOG				PROJECT		APSF		JOB NO.	SHEET NO.	HOLE NO.
						APSF			4 OF 5	FB-5
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING		
		○ RECOVERY %								
		20 40 60 80								
73										
SS 74			19-19-17	175.0			sand zone at top of interval; lt brown, lt brownish yellow, and red; very dense; wet; subangular; poorly graded; fine to med grained; few lt yellow clay balls			
SS 75			4-21-31	173.5			POORLY GRADED SAND WITH CLAY (SP-SC); clayey and trace clay zones; med yellowish brown and lt brown with lt yellow clay wisps; dense; wet; subangular; poorly graded; fine to med grained; black Mn? mottles			Santee
SS 76			23-44-37	173.0			same as above; very dense			
SS 77			7-12-22	172.8			SANDY FAT CLAY (CH); dk brown; hard; wet; high plasticity			
SS 78			14-24-40	172.0			POORLY GRADED SAND (SP) trace clay; lt brown; very dense; wet; subangular; poorly graded; fine to med grained			
SS 79			49-49-50/4in	170.5	120		same as above; occasional coarse sand			
SS 80			21-33-48	169.0			POORLY GRADED SAND WITH CLAY (SP-SC); med yellowish brown with lt yellow clay laminae; dense; wet; subangular; poorly graded; fine to med grained; becomes lighter colored and med grained at bottom of interval			
SS 81			24-44-50	167.5			SILTY SAND (SM); lt brown and lt yellowish brown with white wisps; very dense; wet; subangular; poorly graded; fine grained; black Mn? wisps			
SS 82			40-35-39	166.2			POORLY GRADED SAND (SP) trace clay; very lt brown becoming orange brown near the bottom of interval; very dense; wet; subangular; poorly graded; fine to med grained; clay content increases sltly with depth			
SS 83			12-40-50	164.5			same as above; trace silt; lt brown and lt brownish yellow with lt yellow clay laminae; some zones contain sltly more silt			
SS 84			28-33-50	163.0			same as above; trace clay; fine to med grained			
SS 85			47-48-45	161.5			same as above; occasional clay stringers; fine grained; trace heavies			
SS 86			31-38-50	160.0	130		same as above; trace silt; occasional black Mn? mottles			
SS 87			26-29-28	158.5			same as above; with lt brown and white clayey bands and lt yellow clay laminae; trace mica; top 0.2 ft is clean lt brown sand; borderline with silt			
SS 88			8-3-7	157.0			same as above; no white zone and no brown sand			
SS 89			1/3in-1/6in-2/9in	155.5			same as above; lt brown clean sand at very top of interval			
SS 90			WR/9in-6/3in-16	154.0	135		POORLY GRADED SAND WITH SILT (SP-SM); lt yellowish brown with lt brown zones; very dense; wet; subangular; poorly graded; fine to med grained; black Mn? pellets; trace mica			
SS 91			7-13-15	152.5			CLAYEY SAND (SC); becomes less clayey toward bottom of interval; med yellowish brown with lt greenish brown mottles; loose; wet; subangular; poorly graded; fine grained; trace mica			
SS 92			4-11-13	151.0			same as above; no mottles; very loose; black Mn? pellets			
SS 93			7-10-12	150.0	140		same as above; med brownish yellow; med dense			
SS 94			5-8-8	149.5			same as above; white wisps			
SS 95			5-4-4	148.0			same as above; with lt greenish gray laminae; clay content increases with depth			
SS 96			WR/8in-11-8	146.5			same as above			
SS 97			7-10-14	145.0	145		POORLY GRADED SAND WITH CLAY (SP-SC); lt brown with med yellow clay balls; med dense; wet; subangular; poorly graded; fine to med grained			
SS 98			WR/6in-10-4	143.5			CLAYEY SAND (SC); med brownish yellow and lt brown; loose; wet; subangular; poorly graded; fine grained; trace heavy minerals			2/18/98 water level is @ 75-78' bls.
SS 99			11-15-17	142.0			same as above; interbedded clay with sand; med yellowish brown; med dense; fine to med grained			
				140.5	150		same as above; zones of interbedded lt brown sand with clay and sandy fat clay			
				139.0			SANDY FAT CLAY (CH); med brownish yellow with greenish yellow mottles; very stiff; wet; high plasticity; sand fraction is fine grained; trace heavy minerals			Warley Hill
				137.5			same as above; with interlaminated orange sand; med reddish brown with lt greenish gray laminations; hard; sand fraction is fine to coarse grained			
				136.0						
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE		FINAL LOG				HOLE NO. FB-5



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	5 OF 5	FB-5			
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY % + ATT. LIMITS %									
		20	40	60	80						
SS 100			▲			12-15-23	134.5			 CLAYEY SAND (SC); interlaminated lean clay; lt brownish yellow with lt greenish gray laminations; dense; wet; subangular; poorly graded; fine to med grained; lower part of interval has thicker clay laminae  POORLY GRADED SAND WITH CLAY (SP-SC); lt brown; dense; wet; subangular; poorly graded; fine to med grained with occasional coarse  SANDY FAT CLAY (CH); med reddish yellow with lt greenish gray clay laminae; hard; wet; high plasticity (CH); fine to coarse grained same as above; very hard CLAYEY SAND (SC); med greenish brown and brownish orange; very dense; wet; subrounded; poorly graded; med to coarse grained (probable congarree); black Mn? mottles Total depth of boring 158.8 feet.	Congaree Hole abandoned with grout mix per 3Q5.
SS 101						16-17-19	133.8				
SS 102						15-48-50/4in	133.0				
							131.9 131.7				

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-5



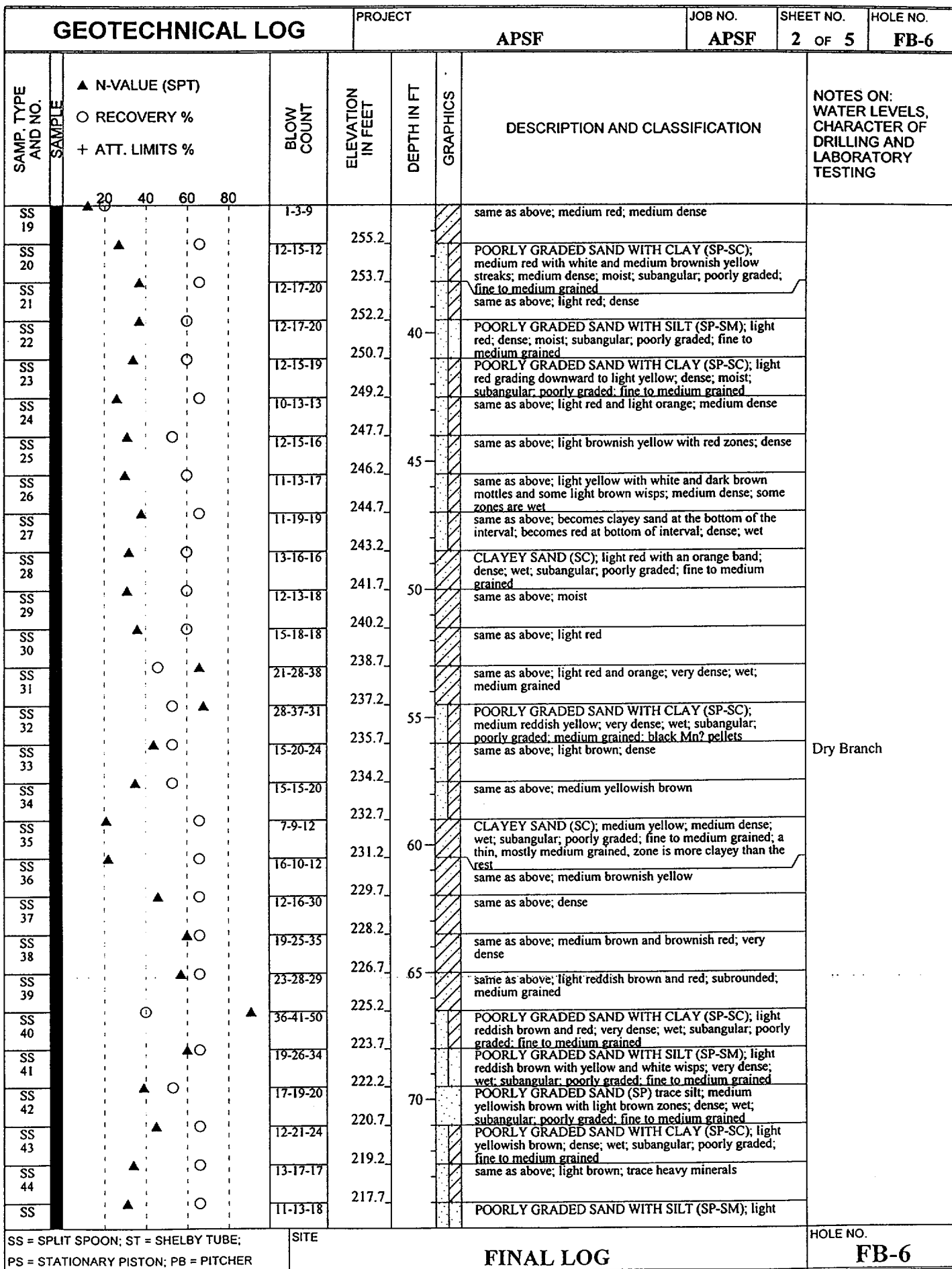
GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	1 OF 5	FB-6			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF			N 79179 E 55038			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/2/98	2/11/98	Graves/A. Jackson & E. Plush	Failing 1500		3 7/8 in	140 lb/ 30 in		158.3			
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:							
291.7		2 1 / 2		N. Kidd/SAIC							
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						291.7					Hand auger to 6 feet to check for underground interferences.
								5			
SS 1		▲			○	12-14-12	282.2	10		CLAYEY SAND (SC); light brown with red bands; medium dense; dry; subangular; poorly graded; fine to medium grained	
SS 2		▲			○	9-15-16	280.7			same as above; dark red with light brown mottles; dense; damp	
SS 3		▲			○	12-15-18	279.2			same as above; medium red with light yellow mottles	
SS 4		▲			○	9-14-11	277.7			same as above; no mottles; medium dense	
SS 5		▲			○	15-11-12	276.2	15		same as above	
SS 6		▲			○	7-7-10	274.7			SANDY LEAN CLAY (CL); light yellowish red; very stiff; damp; low plasticity; sand fraction is fine to medium grained	
SS 7						6-13-18	273.2			no recovery	
SS 8		▲			○	6-9-10	271.7	20		LEAN CLAY (CL); trace sand and trace gravel; medium red; very stiff; moist; medium plasticity; thin layer of gravelly clay (<0.1 foot thick); sand fraction is fine to medium grained	
SS 9		▲			○	6-7-8	270.2			CLAYEY SAND (SC); medium red; medium dense; moist; subangular; poorly graded; fine to medium grained	Tobacco Road
SS 10		▲			○	6-20-21	268.7			same as above; dense; well graded; fine sand to fine gravel grain size	
SS 11		▲			○	5-8-10	267.2			SILT WITH SAND (ML); medium reddish gray with medium grayish red zones; very stiff; dry; low plasticity; sand fraction is fine grained; trace mica	
SS 12		▲			○	5-7-9	265.7	25		same as above	
SS 13		▲			○	6-6-6	264.2			same as above; stiff	
SS 14		▲			○	5-10-12	262.7			same as above; with a plug of medium purple lean clay near the top of the interval; very stiff	
SS 15		▲			○	8-14-11	261.2	30		CLAYEY SAND (SC); medium brownish yellow with white mottles; medium dense; moist; subangular; poorly graded; fine to medium grained; plug of yellow clay (<0.1' thick) at top of interval	
SS 16		▲			○	10-10-12	259.7			same as above	
SS 17		▲			○	8-11-11	258.2			same as above	
SS 18		▲			○	10-32-25	256.7			same as above; with light red zones; very dense	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-6





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-6
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
45 SS	20	13-15-16	216.2			yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
46						same as above	
47	40	10-11-12	214.7			POORLY GRADED SAND (SP) trace clay; light yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
48	60	9-7-8	213.2			CLAYEY SAND (SC); med brownish yellow with lt brown band at bottom and black band in middle of interval; med dense; wet; subangular; poorly graded; fine to med grained; black band and black pellets are Mn?; 0.1 ft thick med yellow sandy clay near middle of interval	
49		2-2-6	211.7	80		same as above; with interbedded clay; loose	
50		5-7-14	210.2			SILTY SAND (SM); with interbedded lean clay; lt brown with dk reddish brown bands; clay is med yellow and v. lt brown; med dense; wet; subangular; poorly graded; fine to med grained	Top Tan Clay
51		8-12-16	208.7			LEAN CLAY (CL); few sand stringers; light brownish yellow with black and dark reddish brown wisps; very stiff; damp; medium plasticity; sand fraction is fine grained	
52		7-10-12	207.2	85		same as above; sandy, with interbedded silt and sand; moist; sand fraction is fine to very coarse grained	
53		4-8-13	205.7			same as above	
54		6-11-19	204.2			CLAYEY SAND (SC); light brown with light yellowish brown and black mottles; medium dense; wet; subangular; poorly graded; fine to medium grained; top 0.1 foot is same as above	Bottom Tan Clay
55		7-8-16	202.7			same as above; with interbedded sand, clay, and silty sand	
56		12-20-25	201.2	90		POORLY GRADED SAND WITH CLAY (SP-SC); light brown; dense; wet; subangular; poorly graded; fine to medium grained	
57		11-16-24	199.7			same as above; silty in places; light and medium brownish yellow	
58		18-25-18	198.2			no recovery	
59		10-15-17	196.7	95		CLAYEY SAND (SC); with silty zones and thin interlaminated clays; medium brown with light brownish yellow zones and thin black wisps; dense; wet; subangular; poorly graded; fine to medium grained; black Mn? wisps	
60		16-25-19	195.2			POORLY GRADED SAND WITH CLAY (SP-SC); clayey sand in places; light brownish yellow; dense; wet; subangular; poorly graded; fine to medium grained	
61		13-18-17	193.7			same as above; light brown with white wisps	
62		9-6-7	192.2	100		CLAYEY SAND (SC); lt yellowish brown and lt brown with lt yellow zones and white wisps; med dense; wet; subangular; poorly graded; fine to med grained	
63		8-11-13	190.7			same as above; with bands of sandy clay; fine to lower coarse grained	
64		13-13-17	189.2			POORLY GRADED SAND WITH CLAY (SP-SC); with zones of clayey sand; light brown with light yellow and white wisps; dense; wet; subangular; poorly graded; fine to medium grained	
65		22-30-40	187.7	105		same as above; very dense; trace coarse sand	
66		25-25-24	186.2			WELL GRADED SAND WITH CLAY (SW-SC); light brown; dense; wet; subangular; well graded; fine to medium grained with some coarse sand and gravel; black Mn? pellets	
67		16-19-26	184.7			same as above; with light yellow and white wisps; top 0.1 ft clayey	
68		29-25-19	183.2			same as above; light brown; subrounded; fine to coarse grained with some fine gravel	
69		14-12-17	181.7	110		POORLY GRADED SAND WITH CLAY (SP-SC); lt yellowish and lt brown with black and lt yellow clay stringers; med dense; wet; subangular; poorly graded; fine to med grained with trace cse and fine gravel; black wisps are Mn?	
70		23-24-24	180.2			same as above; with dark brown mottles; dense; trace heavy minerals	
71		10-18-15	178.7			CLAYEY SAND (SC); with sandy clay zones; very light brown with light orange and purple wisps; dense; wet;	
			177.2				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			FINAL LOG				HOLE NO. FB-6



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	4 OF 5	FB-6
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 72	20	3-5-12	175.7			subangular; well graded; fine to upper coarse grained with trace gravel	Santee
SS 73	40	6-10-9	175.4			SANDY LEAN CLAY (CL); with interbedded silt; lt gray, lt green, and lt purple; very stiff; wet; low plasticity; sand is fine to medium grained	
SS 74	60	5-21-45	174.5			same as above	
SS 75	80	14-19-15	172.7			POORLY GRADED SAND WITH CLAY (SP-SC); med brown with lt brown mottles; med dense; wet; subangular; poorly graded; fine to med grained	
SS 76		5-10-22	171.2	120		same as above; med to coarse grained trace gravel	
SS 77		15-29-33	169.7			same as above; grading to orange and yellow; very dense; fine to med grained; clay content increases downward	
SS 78		20-24-24	168.2			same as above; lt brown few dark brown wisps; dense; with sparse coarse sand and fine gravel	
SS 79		12-21-36	166.7	125		CLAYEY SAND (SC); med brownish yellow; dense; wet; subangular; poorly graded; fine to med grained, trace coarse sand and fine gravel	
SS 80		15-21-50/5.5in	165.2			POORLY GRADED SAND WITH CLAY (SP-SC); med yellowish red with lt brown mottles; very dense; wet; subangular; poorly graded; fine grained	
SS 81		30-27-28	163.7			same as above; med reddish yellow; dense	Catcher teeth broken off.
SS 82		22-36-42	162.2			CLAYEY SAND (SC); some places are silty; very lt greenish yellow with med reddish yellow speckles; very dense; wet; subangular; poorly graded; fine grained	
SS 83		23-23-36	160.7	130		POORLY GRADED SAND (SP); with clay in places; very lt greenish brown, lt brown, and lt yellow; very dense; wet; subangular; poorly graded; fine grained; upper 0.2 feet are fine to med grained	
SS 84		25-27-31	159.2			no recovery	
SS 85		22-33-38	157.7			POORLY GRADED SAND WITH CLAY (SP-SC); med brownish yellow; very dense; wet; subangular; poorly graded; fine grained; trace heavies	
SS 86		20-27-37	156.2	135		no recovery	
SS 87		26-24-32	155.0			same as above; with white bands and orange wisps	
SS 88		15-13-17	153.5			same as above; lt brownish yellow with lt brown zones	
SS 89		7-9-6	153.2			same as above; trace mica	
SS 90		4-5-6	151.7	140		same as above; lt brown; fine to med grained	Worley Hill
SS 91		4-6-6	150.2			same as above; lt yellowish brown with lt brown zones and orange wisps; fine grained; trace mica and heavy minerals	
SS 92		WR/18in	148.7			same as above; lt brown; fine to med grained	
SS 93		4-5-9	147.2	145		same as above; med yellowish brown; med dense; fine grained; black Mn? mottles	
SS 94		WR/4in-2-10-24	145.7			same as above	
SS 95		42-42-46	144.2			CLAYEY SAND (SC); med yellowish brown with lt green wisps; med dense; wet; subangular; poorly graded; fine grained	
SS 96		20-27-50/4in	143.1	150		POORLY GRADED SAND (SP) trace silt; lt brown; med dense; wet; subangular; poorly graded; fine to med grained	
SS 97		18-30-34	142.7			CLAYEY SAND (SC); med yellowish brown with interlaminated lt green wisps; med dense; wet; subangular; poorly graded; fine grained; black Mn? pellets	
SS 98		6-16-24	141.2			same as above; some zones of clay with sand; very loose; several silicified turritella casts	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE	FINAL LOG		HOLE NO. FB-6



GEOTECHNICAL LOG			PROJECT		JOB NO.	SHEET NO.	HOLE NO.
			APSF		APSF	5 OF 5	FB-6
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 99	20 40 60 80 ▲	2-20-50/4in	135.4			same as above; hard	
SS 100	○	20-50-50/2in	134.0			same as above; sandy; very hard; sand fraction fine to coarse grained	Congaree
SS 101	▲	50/3in	133.4			WELL GRADED SAND WITH CLAY (SW-SC), clayey sand in places; lt brown; very dense; wet; subangular; well graded; fine to coarse grained; congaree? no recovery; congaree? Total depth of boring 158.3 feet.	Hole abandoned with grout mix per 3Q5.

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-6



GEOTECHNICAL LOG				PROJECT APSF		JOB NO. APSF	SHEET NO. 1 OF 5	HOLE NO. FB-7
SITE APSF			COORDINATES N 79520 E 54847			ANGLE FROM HORIZONTAL 90		
BEGUN 2/20/98	COMPLETED 2/23/98	DRILLER Graves/S. Rodgers	DRILL MAKE AND MODEL Failing 1500	HOLE SIZE 3 7/8 in	SAMPLE HAMMER WEIGHT/FALL 140 lb/ 30 in	TOTAL DEPTH 156.0		
GROUND EL. 283.8		DEPTH/EL. GROUND WATER 7 / 7 /	LOGGED BY: R. Gelinis/SAIC					
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
SS 39	20 40 60 80	50/4in	283.8				Hand auger to 6 feet to check for underground interferences.	
SS 1	▲	2-2-7	276.3	5		CLAYEY SAND (SC); medium reddish brown; loose; moist; subangular; poorly graded; fine to medium grained	Shoe lost down hole. Tobacco Road	
SS 2	○	3-16-23	274.8			no recovery		
SS 3	▲	11-19-16	273.3	10		SANDY LEAN CLAY (CL); medium reddish brown and yellowish brown; hard; damp; medium plasticity; sand fraction is fine to medium grained		
SS 4	○	29-43-41	271.8			CLAYEY SAND (SC); medium brownish red; very dense; wet; subangular; poorly graded; fine to upper medium grained		
SS 5	▲	13-18-19	270.3			same as above; medium reddish brown and brownish gray; dense; moist; fine to medium grained		
SS 6	○	14-20-18	268.8	15		same as above; with sandy clay portions; medium reddish brown and yellowish brown		
SS 7	▲	37-40-40	267.3			same as above; very dense; wet; fine to upper medium grained		
SS 8	○	11-13-11	265.8			SILTY SAND (SM); medium brown; medium dense; moist; subangular; poorly graded; very fine to lower medium grained		
SS 9	▲	13-12-10	264.3	20		same as above; portions are sandy silt; micaceous		
SS 10	○	10-11-11	262.8			SANDY LEAN CLAY (CL); medium reddish brown and grayish brown; very stiff; moist; low plasticity; sand fraction is very fine grained		
				25				
SS 11	▲	17-19-20	256.3			SILTY SAND (SM); with clay; light yellowish brown and light gray; dense; moist; subangular; poorly graded; fine to medium grained		
				30				
SS 12	○	18-24-20	251.3			same as above; with clayey sand interbeds	Had driller pull spoon and clean out wash/cave material back down to 31 feet.	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-7



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	2 OF 5	FB-7
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 13		○▲	50-34-25	246.3			same as above; portions with clay; medium brown, orangish brown, and light brownish gray; very dense	First two blow count intervals in washed/caved material. Dry Branch?
SS 14		▲○	2-3-20	243.0	40		same as above; with clay; medium dense; wet.	
					45			
SS 15		▲	22-21-27	236.3			POORLY GRADED SAND WITH CLAY (SP-SC); medium brown; dense; wet; subangular; poorly graded; fine to medium grained	
					50			
SS 16		▲○	9-11-14	230.8			SILTY SAND (SM); with clay; medium reddish brown and light gray; medium dense; moist; subangular; poorly graded; fine to upper medium grained	
					55			
SS 17		▲○	9-12-13	225.9			POORLY GRADED SAND WITH CLAY (SP-SC); medium brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
					60			
SS 18		▲○	13-18-18	221.4			CLAYEY SAND (SC); medium brown; dense; moist; subangular; poorly graded; fine to medium grained	
					65			
SS 19		○	103 59-50-53/3in	216.3			POORLY GRADED SAND WITH CLAY (SP-SC); light brown to medium brown; very dense; wet; subangular; poorly graded; fine to medium grained	
					70			
SS 20		▲○	2-8-8	211.2			same as above; medium yellowish brown; medium dense	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

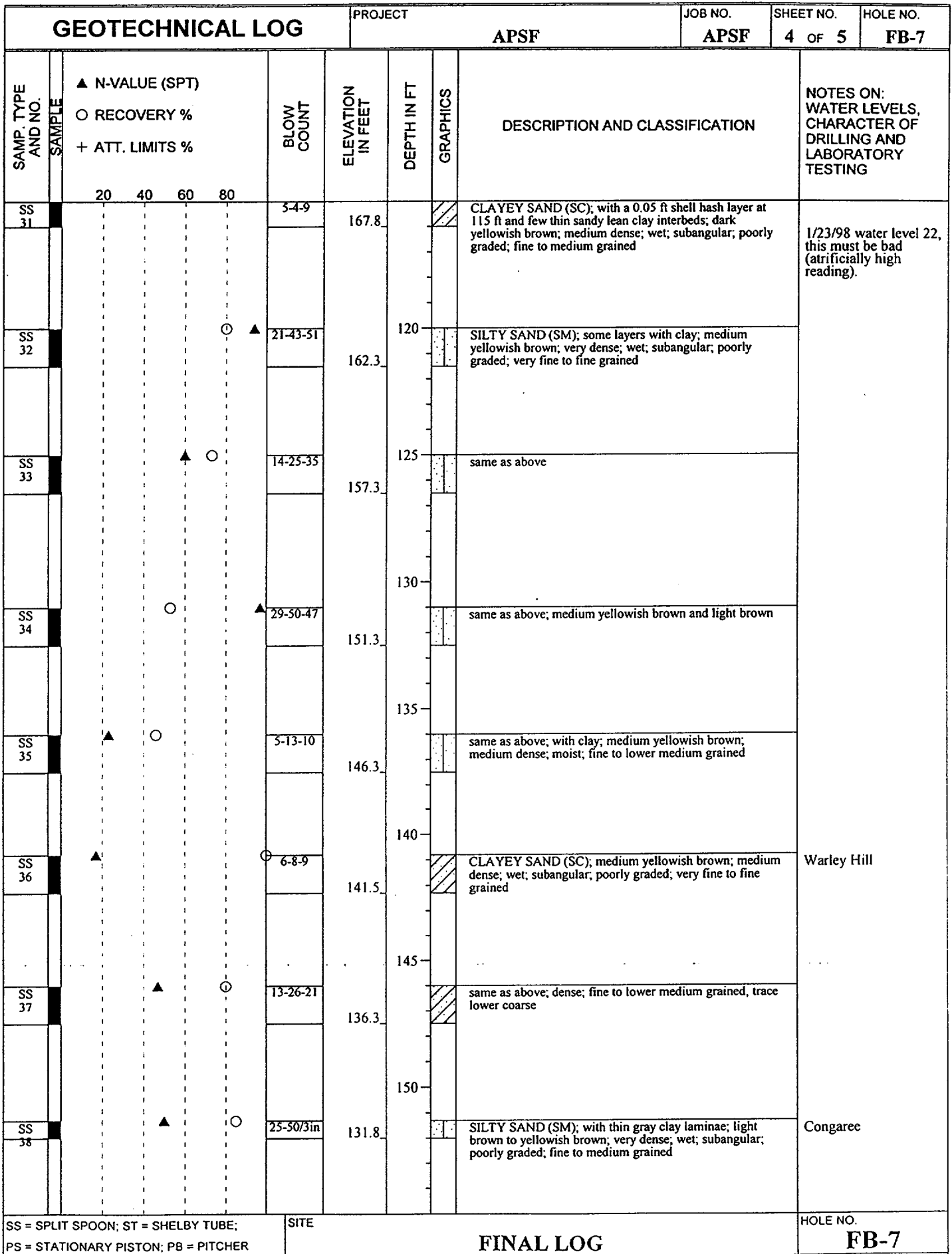
SITE

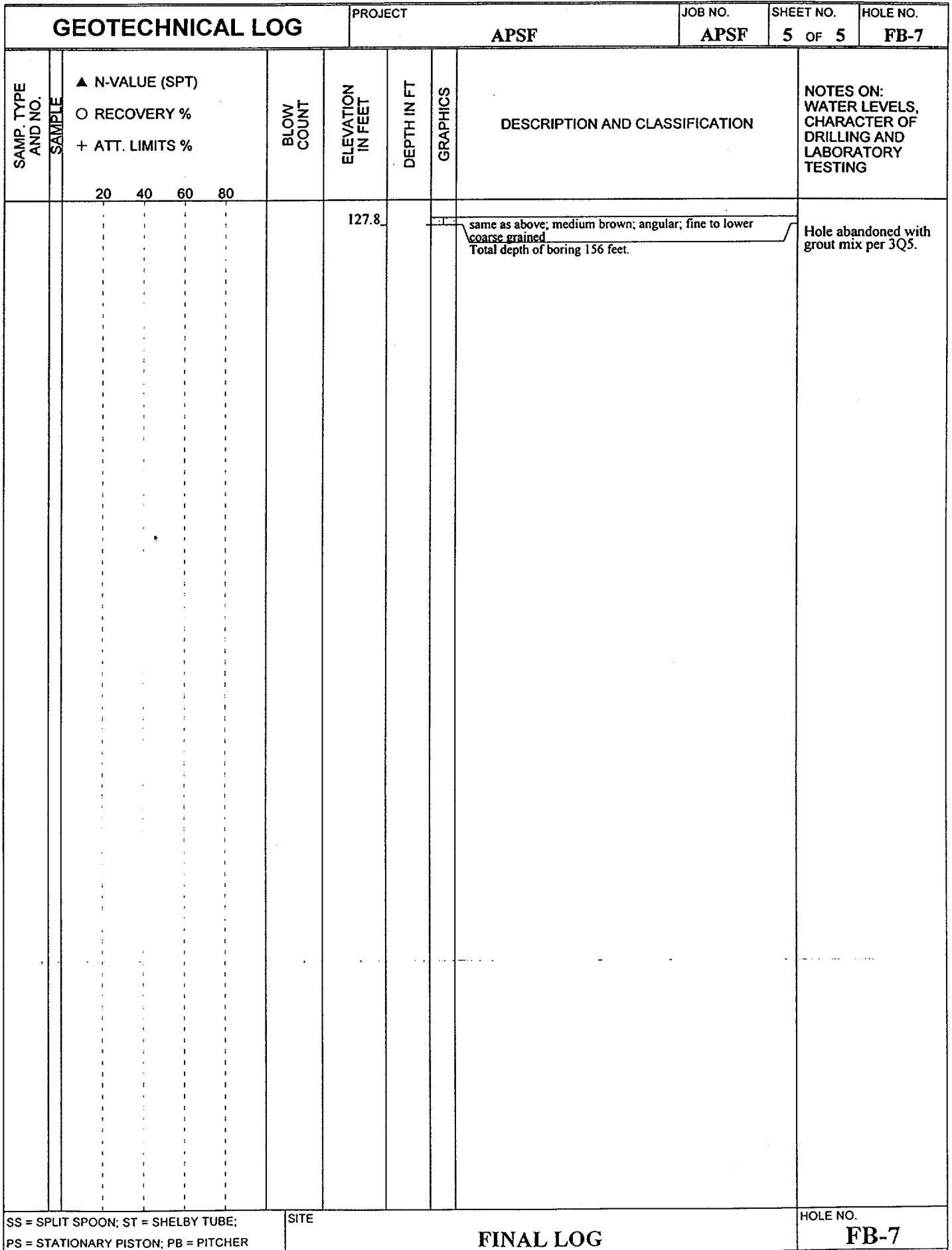
FINAL LOG

HOLE NO.
FB-7



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-7
SAMP TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 21	▲ 20 40 60 80	5-10-8	206.0	80	III	ELASTIC SILT WITH SAND (MH); medium yellowish brown; very stiff; moist; low plasticity; sand fraction is very fine to fine grained	Tan Clay Interval
SS 22	▲ 20 40 60 80	8-19-26	201.4	85	II	CLAYEY SAND (SC); grading down to poorly graded sand with clay; medium brown with a few black Mn zones; dense; wet; subangular; poorly graded; fine to medium grained; trace coarse sand	
SS 23	○ 20 40 60 80	40-28-18	196.3	90	I	POORLY GRADED SAND WITH SILT (SP-SM); light brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 24	▲ 20 40 60 80	21-34-33	191.0	95	I	same as above; very dense	
SS 25	▲ 20 40 60 80	3-5-4	186.0	100	II	CLAYEY SAND (SC); medium brown and grayish brown; loose; wet; angular; poorly graded; fine to lower coarse grained	
SS 26	▲ 20 40 60 80	2-3-5	184.5		II	same as above; medium yellowish brown, dark brown, and light grayish brown; subangular; fine to medium grained	Santee
SS 27	▲ 20 40 60 80	2-4-4	183.0		II	same as above; trace black charcoal fragments; medium yellowish brown; angular; fine to lower coarse grained	
SS 28	▲ 20 40 60 80	5-7-18	181.3	105	II	same as above; grading to poorly graded sand with silt at bottom of interval; medium dense; subangular; fine to medium grained	
SS 29	○ 20 40 60 80	20-35-41	176.3	110	I	POORLY GRADED SAND (SP) trace silt; grading down to poorly graded sand with clay; medium brown; very dense; wet; subangular; poorly graded; fine to upper medium grained	
SS 30	▲ 20 40 60 80	12-13-33	171.8		I	POORLY GRADED SAND WITH SILT (SP-SM); medium brown; dense; wet; subangular; poorly graded; fine to lower coarse grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-7







GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
SITE				COORDINATES		ANGLE FROM HORIZONTAL					
APSF				N 79344 E 54855		90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/23/98	2/24/98	Graves/A. Jackson	Failing 1500		3 7/8 in	140 lb/ 30 in		155.8			
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:							
288.5		▽ / ▽ /		N. Kidd/SAIC							
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
							288.5				Hand auger to 6 feet to check for underground interferences.
SS 1		○	▲			2-6-16				CLAYEY SAND (SC); medium red with orange band; medium dense; dry; subangular; poorly graded; fine to medium grained	
SS 2			▲			13-18-23	281.0			SANDY LEAN CLAY (CL); medium red with brownish yellow mottles; hard; dry; medium plasticity; sand fraction is fine to medium grained	
SS 3			▲			12-15-18	279.5			same as above	
SS 4		○	▲			3-3-9	278.0	10		same as above; stiff; bentonite present in tube	
SS 5			▲			10-15-20	276.5			LEAN CLAY (CL) with sand; medium red; hard; damp; medium plasticity; sand fraction is fine to medium grained	
SS 6			▲			7-12-15	275.0			same as above; very stiff	
SS 7			▲			7-11-19	273.5	15		same as above	
SS 8			▲			4-11-15	272.0			same as above; with brownish yellow mottles and slightly more sand	
SS 9			▲			8-10-13	270.5			SANDY LEAN CLAY (CL); medium red; very stiff; damp; medium plasticity; sand is fine to medium grained	
SS 10			▲			7-11-15	269.0	20		CLAYEY SAND (SC); medium red with brownish yellow mottles; medium dense; damp; subangular; poorly graded; fine to medium grained	
							267.5				
SS 11			▲			8-10-12	262.5	25		same as above; light brownish yellow with white wisps; trace mica	
SS 12			▲			7-12-16	257.5	30		same as above; with red zones	
			▲								

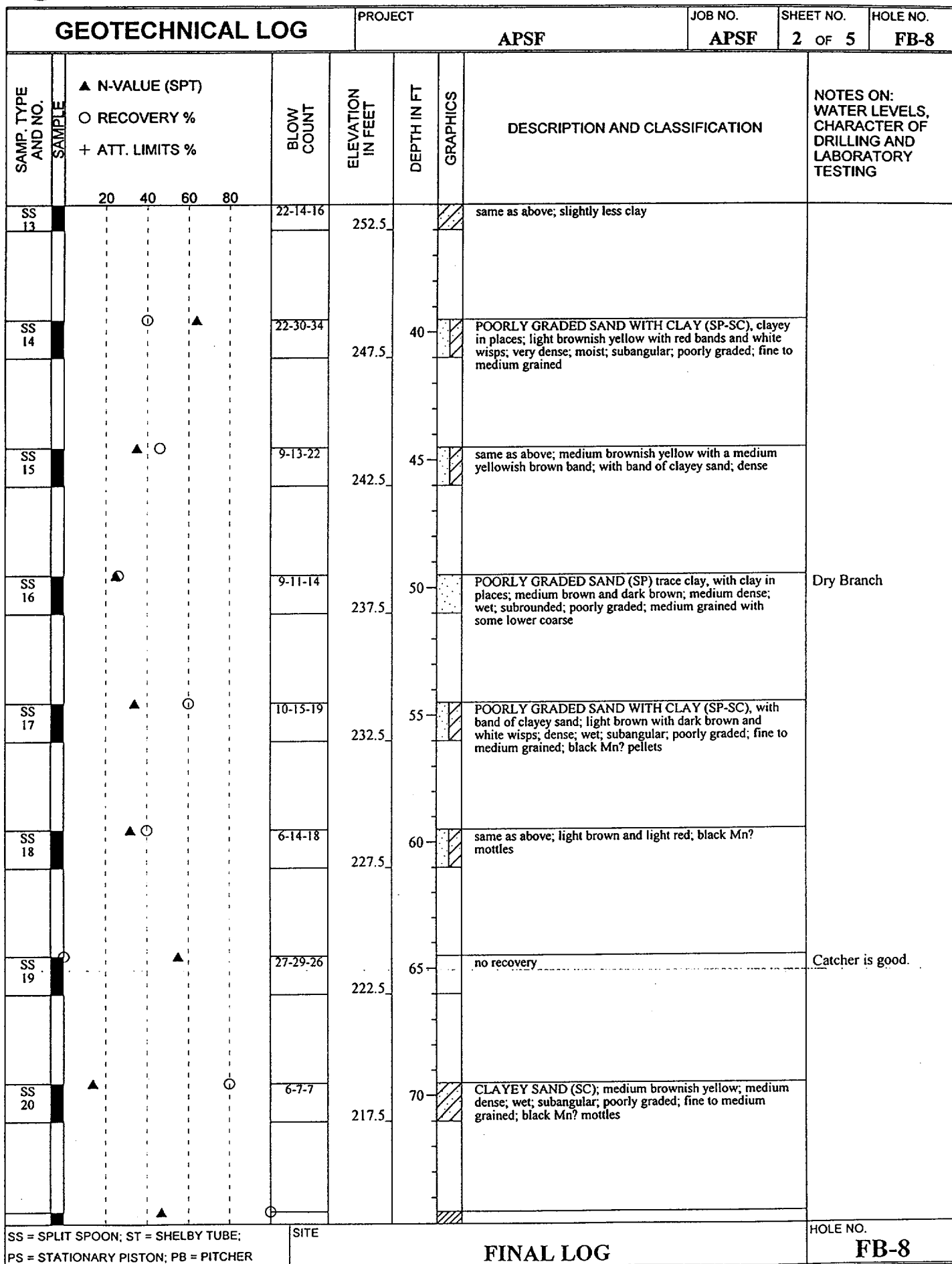
SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

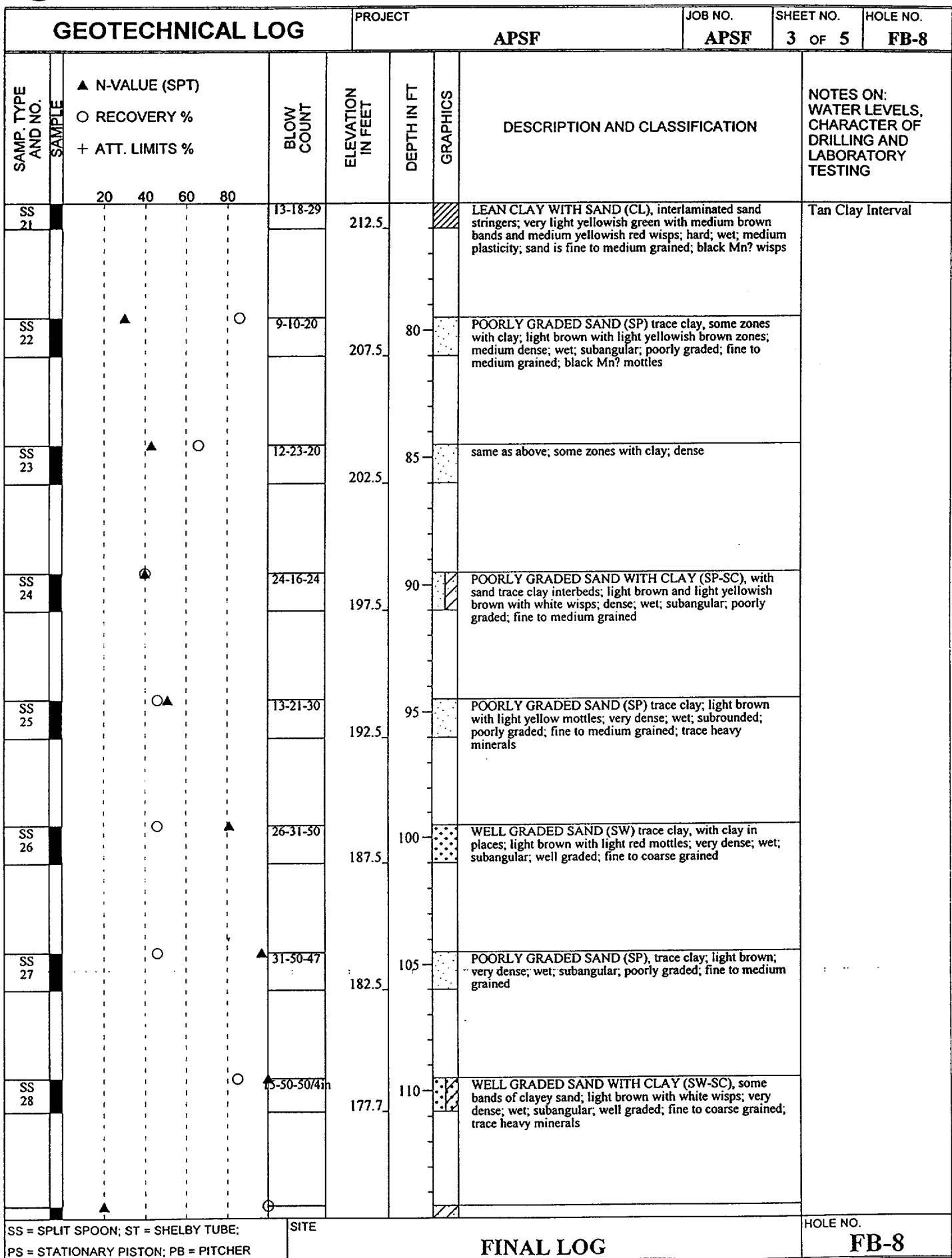
SITE

FINAL LOG

HOLE NO.

FB-8







GEOTECHNICAL LOG					PROJECT		APSF		JOB NO.	APSF	SHEET NO.	4 OF 5	HOLE NO.	FB-8
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING			
		20	40	60	80									
SS 29						13-8-12	172.5			CLAYEY SAND (SC); medium brownish yellow with very light green mottles; medium dense; wet; subangular; poorly graded; fine to medium grained	Santee			
SS 30						43-49-47	167.5	120		POORLY GRADED SAND (SP), trace clay; medium brownish yellow; very dense; wet; subangular; poorly graded; fine to lower medium grained; trace mica				
SS 31						24-31-50	162.5	125		same as above; with a <0.1 ft layer of medium yellow lean clay				
SS 32						22-39-37	157.5	130		same as above; with yellow bands and yellow clay pods; black Mn? pellets				
SS 33						13-12-13	152.5	135		POORLY GRADED SAND WITH CLAY (SP-SC), clayey sand in places; medium brownish yellow; medium dense; wet; subangular; poorly graded; fine to medium grained; trace heavy minerals				
SS 34						3-4-7	147.5	140		CLAYEY SAND (SC), some zones contain less clay; medium brownish yellow with very light brown zones; medium dense; wet; subangular; poorly graded; fine grained; trace heavy minerals	Warley Hill			
SS 35						11-17-28	142.5	145		FAT CLAY (CH), trace sand; very light brown with medium orange flecks; hard; wet; high plasticity; sand fraction is fine grained; trace heavy minerals				
SS 36						28-25-26	137.5	150		CLAYEY SAND (SC), with light brown clean sand layer at top of interval; medium brownish yellow; very dense; wet; subangular; poorly graded; fine to medium grained; most of interval is composed of silicified chips				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE		FINAL LOG				HOLE NO. FB-8		



GEOTECHNICAL LOG				PROJECT		APSF		JOB NO.	APSF	SHEET NO.	HOLE NO.
										5 OF 5	FB-8
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, DRILLING AND LABORATORY TESTING			
SS 37		20 40 60 80	28-31-30/3in	132.9 132.7			same as above; very dark greenish gray, green clay POORLY GRADED SAND (SP) trace clay; medium yellowish red; very dense; wet; subangular; poorly graded; fine to medium grained; overlying 'green clay' grades into this interval; the transition is darker orange than the sand and includes some light gray lean clay Total depth of boring 133.8 feet.	Congaree Hole abandoned with grout mix per 3Q5.			
FINAL LOG											
SITE				HOLE NO.				FB-8			

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
SITE				COORDINATES		ANGLE FROM HORIZONTAL					
APSF				N 79185 E 54845		90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH					
2/24/98	2/26/98	Graves/S. Rodgers	Failing 1500	3 7/8 in	140 lb/ 30 in	160.3					
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:							
290.0		70.1/219.9 2/26/98		R. Gelinis/SAIC							
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY %	+ ATT. LIMITS %								
		20	40	60	80		290.0				
											Hand auger to 6 feet to check for interferences.
							287.5			Some asphaltic fill.	
								5			
SS 1			▲		○	6-14-15				SILTY SAND (SM), trace clay; medium brown, reddish brown, and grayish brown; medium dense; damp; subangular; poorly graded; fine to medium grained	
SS 2				▲	○	9-26-26	282.5			same as above; with clay; medium brownish red; very dense; moist	
SS 3			▲		○	14-16-16	281.0			same as above; with two thin light brown layers; dense	
SS 4			▲		○	9-11-12	279.5	10		SANDY LEAN CLAY (CL), medium brownish red and some light brown; very stiff; moist; low plasticity; sand fraction is very fine to fine grained	
SS 5		▲			○	5-8-10	278.0			same as above; wet; sand fraction is fine to medium grained	
SS 6		▲			○	7-7-8	276.5			same as above; stiff; moist	
SS 7		▲			○	6-9-9	275.0	15		CLAYEY SAND (SC); medium reddish brown; medium dense; moist; subangular; poorly graded; fine to medium grained	Tobacco Road
SS 8		▲			○	5-7-8	273.5			same as above; medium brown	
SS 9		▲			○	7-7-10	272.0			same as above; medium brown and reddish brown	
SS 10		▲			○	1-10-12	270.5	20		same as above; medium brownish red, yellowish brown, and light gray	
							269.0				
SS 11			▲		○	9-12-15	263.5	25		SANDY LEAN CLAY (CL), with clayey sand interbeds; medium yellowish brown with light gray and maroon zones; very stiff; moist; medium plasticity	
SS 12		▲			○	7-9-8	258.2	30		CLAYEY SAND (SC), with some sandy clay interbeds; medium brownish red; medium dense; moist; subangular; poorly graded; fine to medium grained	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

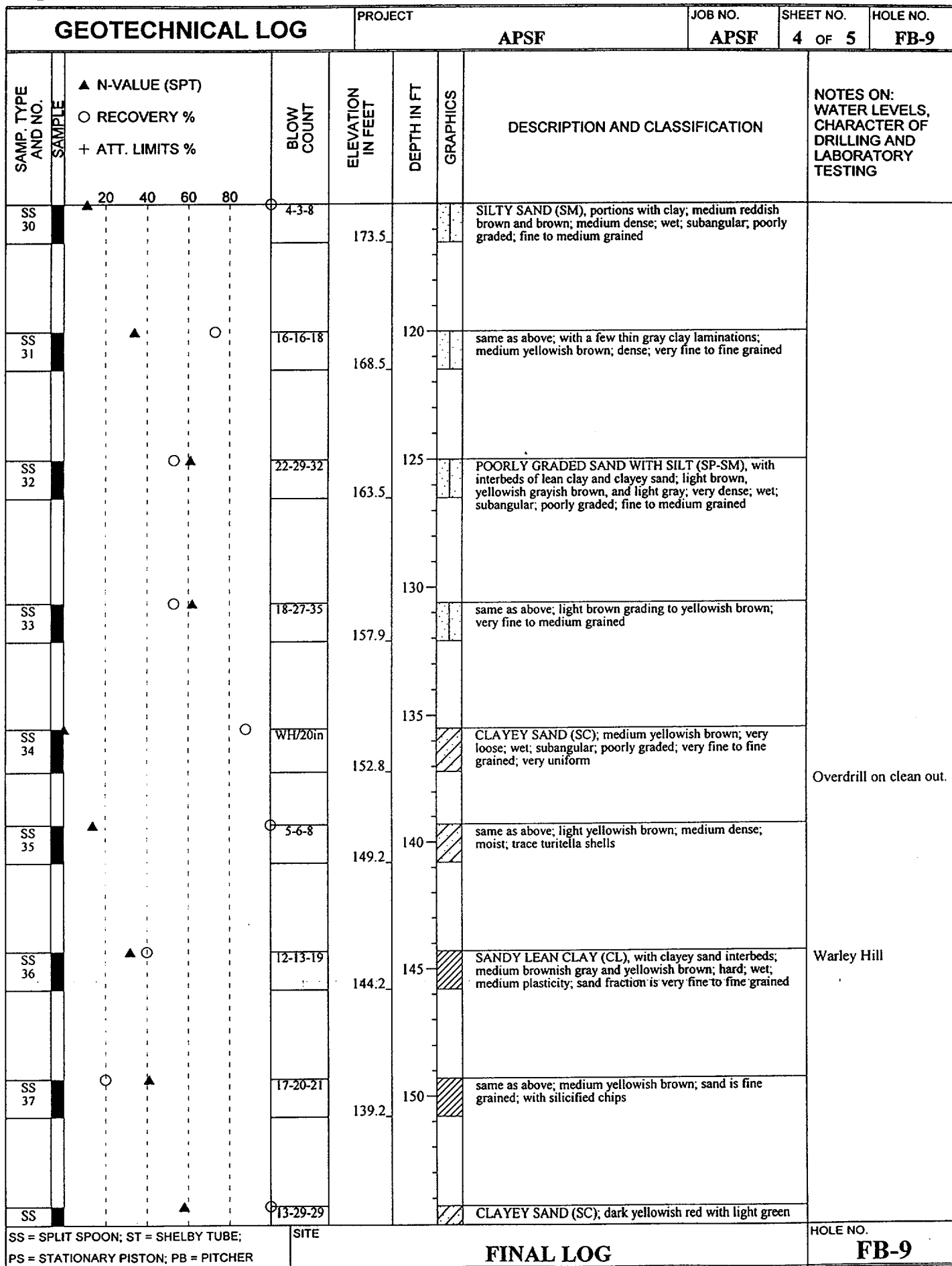
HOLE NO.
FB-9



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	2 OF 5	FB-9
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 13	▲ 20 40 60 80	10-9-12	253.2			same as above	Dry Branch
SS 14	▲ 20 40 60 80	12-16-16	248.4	40		same as above; medium yellowish brown; dense	
SS 15	▲ 20 40 60 80	13-15-15	243.5	45		same as above; medium dense	
SS 16	▲ 20 40 60 80	13-24-37	238.0	50		SILTY SAND (SM); medium brown, light brown, and some red oxidation spots; very dense; wet; angular; poorly graded; fine to lower coarse grained	
SS 17	▲ 20 40 60 80	8-9-11	232.5	55		CLAYEY SAND (SC); medium yellowish brown; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 18	▲ 20 40 60 80	15-17-35	227.5	60		SILTY SAND (SM); medium brown; very dense; wet; subangular; well graded; fine to coarse grained trace fine water rounded gravel	
SS 19	▲ 20 40 60 80	14-17-19	222.5	65		CLAYEY SAND (SC), with some thin clayey sand portions; light brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 20	▲ 20 40 60 80	11-13-15	217.4	70		SILTY SAND (SM), with thin clayey sand layers; light brown to medium brown; medium dense; wet; subangular; poorly graded; fine to lower coarse grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-9



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	3 OF 5	FB-9
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
SS 21	▲	4-6-10	212.5	80		LEAN CLAY (CL), with sand and highly variable sandy clay and clayey sand layers; medium yellowish gray and yellowish brown; very stiff; wet; medium plasticity; sand fraction is fine to medium grained trace coarse	Tan Clay Interval	
SS 22	▲	3-8-14	207.6	85		CLAYEY SAND (SC), with sandy clay layers; medium yellowish gray and yellowish brown; subangular; medium dense; wet; poorly graded; fine to medium grained		
SS 23	○▲	38-50/5in	203.1	90		POORLY GRADED SAND WITH SILT (SP-SM); light brown; very dense; wet; subangular; poorly graded; fine to medium grained		
SS 24	▲	5-6-10	197.5	95		CLAYEY SAND (SC), with sandy clay interbeds; medium yellowish brown; medium dense; wet; subangular; poorly graded; very fine to fine grained; with black Mn? staining		
SS 25	▲	10-9-9	191.9	100		same as above; medium yellowish brown and grayish brown; fine to medium grained		
SS 26	○▲	18-30-30	187.6	105		POORLY GRADED SAND WITH SILT (SP-SM); light brown to medium brown; very dense; wet; subangular; poorly graded; fine to upper medium grained		
SS 27	○▲	18-30-30	182.5	110		same as above	Santee	
SS 28	▲	4-4-3	177.4			CLAYEY SAND (SC); medium yellowish brown; loose; wet; subangular; poorly graded; fine to medium grained; with black Mn? staining		
SS 29	▲	1-1-2	175.9			same as above; dark yellowish brown; very loose		
							Overdrill	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE		FINAL LOG	HOLE NO. FB-9	





GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	5 OF 5	FB-9			
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
38							134.2			wispy laminae; very dense; wet; subangular; well graded; fine to coarse grained	1/26/98 water level @ 710.1 feet.
SS 39				▲	○	50/3in	129.7	160		SILTY SAND (SM); light brown to medium brown; very dense; wet; subrounded; well graded; fine to coarse grained	Congaree Hole abandoned with grout mix per 3Q5.

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-9



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	1 OF 5	FB-10			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF			N 79506 E 54990			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
2/26/98	2/27/98	Graves/A. Jackson	Failing 1500		3 7/8 in	140 lb/ 30 in		154.8			
GROUND EL.		DEPTH/VEL. GROUND WATER		LOGGED BY:							
284.4		▽ / ▽ /		N. Kidd/SAIC							
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						284.4					
SS 1		▲		○	5-7-13	276.9				SANDY LEAN CLAY (CL); medium red with light yellow and white mottles; very stiff; dry; medium plasticity; sand is fine to medium grained	
SS 2		▲			15-14-22	275.4				same as above; hard	
SS 3		▲			8-13-20	273.9	10			same as above; moist; no mottles	
SS 4		▲		○	11-17-23	272.4				same as above	
SS 5		▲		○	9-16-20	270.9				CLAYEY SAND (SC); medium red; dense; damp; subangular; poorly graded; fine to coarse grained	
SS 6		▲		○	7-14-17	269.4	15			same as above; fine to medium grained; fragment of typical Altamaha cobble in spoon	
SS 7		▲		○	8-13-19	267.9				LEAN CLAY WITH SAND (CL); medium red; hard; damp; medium plasticity; sand is fine to medium grained	
SS 8		▲		○	7-12-17	266.4				SANDY LEAN CLAY (CL); medium red; very stiff; moist; medium plasticity; sand is fine to medium grained	
SS 9		▲		○	6-12-16	264.9				LEAN CLAY WITH SAND (CL); medium red with light yellow and light purple mottles; very stiff; moist; medium plasticity (CL); sand is fine grained; trace mica	
SS 10		▲		○	9-14-26	263.4	20			SILT (ML), sandy in places; red, white, yellow, multicolored; hard; moist; sand fraction is fine grained; trace mica	
SS 11		▲		○	10-14-20	258.4	25			SILTY SAND (SM); light red with white mottles and occasional medium yellow bands; dense; moist; subangular; poorly graded; fine to medium grained	
SS 12		▲		○	9-14-20	253.4	30			same as above; sand is mostly fine grained	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-10



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	2 OF 5	FB-10
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 13		15-27-37	248.4			POORLY GRADED SAND WITH SILT (SP-SM); light red and medium orange; very dense; moist; subangular; poorly graded; fine to medium grained	Dry Branch
SS 14		14-24-28	243.4	40		POORLY GRADED SAND WITH CLAY (SP-SC); portions silty; light brownish yellow with white laminae; very dense; moist; subangular; poorly graded; fine to medium grained	
SS 15		20-32-49	238.4	45		same as above; light yellowish brown with light reddish purple zones; wet; subrounded	
SS 16		9-15-17	233.4	50		CLAYEY SAND (SC); medium yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 17		16-20-30	228.4	55		WELL GRADED SAND WITH CLAY (SW-SC); medium yellowish brown with medium reddish brown and medium brownish yellow zones; very dense; wet; subangular; well graded; fine to coarse grained	
SS 18		12-19-23	223.4	60		POORLY GRADED SAND WITH CLAY (SP-SC); clayey in places; medium brownish yellow with white wisps and light reddish brown zones; dense; wet; subangular; poorly graded; fine to medium grained	
SS 19		10-14-19	218.4	65		same as above; occasional coarse sand grains; black Mn? pellets	
SS 20		6-16-29	213.4	70		same as above; with interlayered medium yellow clay and dark orange sand; black Mn? wisps; trace heavy minerals; flattened quartz pebbles at top of interval	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				HOLE NO.
			FINAL LOG				FB-10



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-10
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80						
SS 21		16-23-28	208.4			LEAN CLAY (CL), trace sand; light brownish yellow with dark orange laminae; hard; moist; medium plasticity; sand is fine to coarse grained; black Mn? pellets	Tan Clay Interval
SS 22		8-19-31	203.4	80		POORLY GRADED SAND WITH CLAY (SP-SC); light brown; dense; wet; subangular; poorly graded; fine to medium grained; trace heavy minerals; top half of the interval is light yellow sandy clay	
SS 23		15-25-39	198.4	85		same as above; clayey in places; medium yellowish brown; very dense; black Mn? wisps; light yellow interlaminated clay	
SS 24		25-39-31	193.4	90		POORLY GRADED SAND (SP), trace clay; light brown with very light brown zones; very dense; wet; subrounded; poorly graded; fine to medium grained; black Mn? pellets; almost no clay in the top of the interval; trace heavy minerals	
SS 25		26-23-30	188.4	95		same as above; with clay at the bottom of the interval; light brownish yellow; light brown at the top of the interval	
SS 26		26-42-50	183.4	100		same as above; trace clay; light brown; black Mn? wisps	
SS 27		17-24-33	178.4	105		POORLY GRADED SAND WITH CLAY (SP-SC); light brown and light yellow; very dense; wet; subangular; poorly graded; fine to medium grained; black Mn? wisps; also heavy mineral stringers	
SS 28		25-41-48	173.4	110		no recovery	1/24/98 water level @ 26.68 ft. Water level not consistent with other measurements taken in nearby borings. Plastic catcher is good.
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-10



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	4 OF 5	FB-10
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 29		20-31-30	168.4			POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish yellow; very dense; wet; subangular; poorly graded; fine grained; black Mn? mottles; trace heavy minerals	Santee
SS 30	○	26-36-36	163.4	120		same as above; with interlaminated very light green clay wisps	
SS 31	○	34-33-36	158.4	125		same as above; color grades downward to light yellow; dark reddish brown; iron cemented sand nodule at top of interval	
SS 32	▲	9-16-24	153.4	130		same as above; no light yellow zones; medium dense	
SS 33	▲	17-21-22	148.4	135		CLAYEY SAND (SC), with clay stringers; very light brown, light yellowish brown, and orange with light green stringers; dense; wet; subangular; poorly graded; fine grained; trace heavy minerals; trace mica; silicified turritella shells	
SS 34	▲	19-31-40	143.4	140		same as above; with no light green clay stringers; very dense	
SS 35	▲	7-18-36	138.4	145		CLAYEY SAND (SC), with interbedded lean clay, trace fine to coarse sand layers; medium yellowish brown with orange zones and light brownish yellow mottles; very dense; wet; subangular; poorly graded; fine grained; trace mica; trace heavy minerals; silicified turritella shells	
SS 36	▲	50/5in	134.5	150		SANDY FAT CLAY (CH); very dark grayish green with very light green band; hard; wet; high plasticity; sand is fine to coarse grained; 'green clay'	Warley Hill
	▲		129.6				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-10

1/27/98 water level @ 36.68 ft. Water level not consistent with other water level readings in the area.



GEOTECHNICAL LOG					PROJECT		JOB NO.	SHEET NO.	HOLE NO.
					APSF		APSF	5 OF 5	FB-10
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
SS 37		20 40 60 80	50/4in				no recovery Total depth of boring 154.8 feet.	Hole abandoned with grout mix per 3Q5.	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				FINAL LOG		HOLE NO. FB-10



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	1 OF 5	FB-11
SITE			COORDINATES			ANGLE FROM HORIZONTAL		
APSF			N 79317 E 54981			90		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH		
2/19/98	2/20/98	Graves/A. Jackson	Failing 1500	3 7/8 in	140 lb/ 30 in	158.7		
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:					
289.8		2 / 1 /	N. Kidd/SAIC					
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80		289.8					
				5				
SS 1	▲	1-2-9	281.3			CLAYEY SAND (SC); dark brown; medium dense; dry; subangular; poorly graded; fine to medium grained; some cement in interval (from nearby CPT push?);	Hand auger to 6 feet to check for underground interferences.	
SS 2	○	19-26-29	279.8			no soil recovery; cement in shoe (from nearby CPT push?); very dense		
SS 3	▲	15-22-25	278.3	10		CLAYEY SAND (SC); medium red with light brown mottles; dense; damp; subangular; poorly graded; fine to medium grained		
SS 4	▲	2-4-20	276.8			same as above; medium dense; bentonite is present within the interval; actual soil recovery is less (about 0.3 feet)		
SS 5	▲	9-13-13	275.3			same as above with no bentonite		
SS 6	▲	11-15-16	273.8	15		same as above; dense		
SS 7	▲	11-16-18	272.3			same as above		
SS 8	▲	10-13-12	270.8			same as above; slightly higher clay content; medium dense		
SS 9	▲	6-8-10	269.3	20		same as above; sand is fine grained	Tobacco Road?	
SS 10	▲	7-12-16	264.3	25		SANDY LEAN CLAY (CL); medium red; very stiff; damp; medium plasticity; sand fraction is fine to coarse grained		
SS 11	▲	4-4-5	259.3	30		CLAYEY SAND (SC); medium red; loose; damp; subangular; poorly graded; fine to medium grained		
SS 12	▲	4-5-6	257.8			same as above; medium dense; moist		

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-11



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	2 OF 5	FB-11
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 13	▲	3-2-5	252.8			same as above; loose	Dry Branch
SS 14	▲	6-15-18	251.3			same as above; medium red, medium purple, and medium yellow; dense	
SS 15	▲	6-9-11	246.3			same as above; some zones contain less clay; medium brownish yellow with white wisps; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 16	○	11-16-20	241.3			POORLY GRADED SAND WITH CLAY (SP-SC); medium yellowish brown with dark brown wisps; dense; wet; subrounded; poorly graded; medium grained	
SS 17	▲	5-11-14	236.3			CLAYEY SAND (SC); medium brownish yellow; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 18	○	12-18-17	231.3			same as above; with medium gray zones; dense	
SS 19	○	16-22-17	226.3			POORLY GRADED SAND WITH CLAY (SP-SC); clayey in places; very light brown and very light brownish yellow with white wisps; dense; wet; subangular; poorly graded; fine to medium grained	
SS 20	○	9-15-13	221.3			same as above; medium dense; trace mica	
SS 21	○	3-5-5	216.3			CLAYEY SAND (SC); some zones contain less clay; interbedded clay layer at bottom of interval; medium red grading downward to grayish red; loose; wet; subangular; poorly graded; fine to medium grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-11



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	3 OF 5	FB-11
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80						
SS 22	▲	8-11-12	211.3	80		same as above; interbedded sandy lean clay; medium brownish yellow with white wisps and dark brown and dark gray mottles; medium dense	Tan Clay Interval?
SS 23	▲ ○	10-24-22	206.3	85		POORLY GRADED SAND WITH CLAY (SP-SC); light brown with light brownish yellow zones; dense; wet; subangular; poorly graded; fine to medium grained	
SS 24	○	27-50-50/51	201.4	90		POORLY GRADED SAND (SP) trace clay; light brown; very dense; wet; subrounded; poorly graded; fine to medium grained	
SS 25	○ ▲	15-23-26	196.3	95		same as above; dense; trace heavies	
SS 26	▲ ○	6-14-27	191.3	100		CLAYEY SAND (SC); some zones are clay with sand; medium brownish yellow with white wisps; dense; wet; subangular; poorly graded; fine to medium grained; black pellets (Mn?)	
SS 27	○ ▲	26-50-44	186.3	105		POORLY GRADED SAND (SP) trace clay; some zones contain slightly more clay; light brown and brownish yellow; very dense; wet; subrounded; poorly graded; fine to medium grained	
SS 28	○ ▲	19-37-45	181.3	110		same as above except no brownish yellow color	
SS 29	▲	2-2-3	176.3			FAT CLAY (CH) with interlaminated sand; clay becomes sandy in places; white and very light brown; firm; wet; high plasticity; sand is fine to coarse grained; possible weathered shell fragments; black pellets (Mn?)	Santee
SS 30	▲	7-8-13	174.8			SANDY FAT CLAY (CH); with interlaminated sand and	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				HOLE NO.
			FINAL LOG				FB-11



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	4 OF 5	FB-11
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80					clay layers; very light green with medium yellowish brown mottles; very stiff; wet; high plasticity; sand is fine to medium grained with occasional coarse grains; black pellets (Mn?)	
SS 31	○ ▲	12-24-33	169.8	120		CLAYEY SAND (SC); very light brown and medium brownish yellow; very dense; wet; subangular; poorly graded; fine grained; black pellets (Mn?)	
SS 32	○ ▲	18-34-43	164.8	125		POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish yellow; very dense; wet; subangular; poorly graded; fine grained; trace heavies	
SS 33	○ ▲	22-26-44	159.8	130		same as above with wisps of light green clay	
SS 34	○ ▲	24-28-24	154.8	135		same as above; medium yellowish brown with light yellow mottles	
SS 35	○ ▲	20-23-17	149.8	140		same as above; light yellowish brown with dark brown clay laminae; dense; the very top of the interval is medium orange clayey sand, which grades downward into the interval (color and clay content); turritella shell at very top of interval	
SS 36	○ ▲	11-26-30	144.8	145		FAT CLAY (CH); sandy in places; light brownish yellow; hard; wet; high plasticity; sand fraction is fine grained; turritella shell fragments	Warley Hill
SS 37	○ ▲	15-11-20	139.8	150		SANDY FAT CLAY (CH); medium yellowish red and brownish yellow with light green wisps; hard; damp; high plasticity; sand is fine to coarse grained	
SS 38	○ ▲	28-37-40	134.8			POORLY GRADED SAND WITH CLAY (SP-SC); medium yellowish brown; very dense; wet; subrounded;	Congaree
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-11



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	5 OF 5	FB-11			
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
SS 39	○			▲		50-50/2m	131.1			poorly graded; medium to coarse grained (congaree ?)	
										A small amount of clayey and sandy/gravelly material was pulled from the shoe. this material is probably sloughed	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

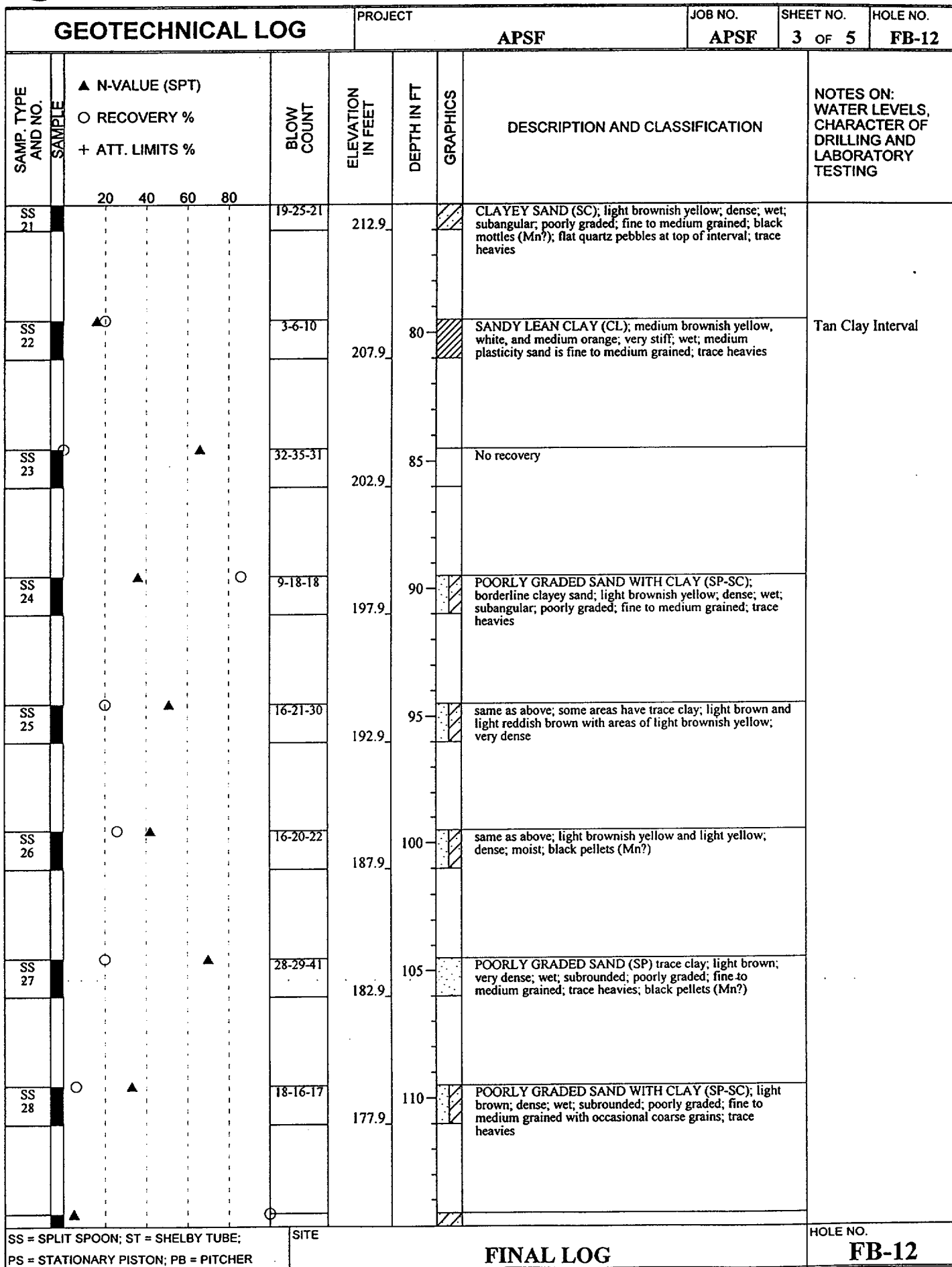
HOLE NO.
FB-11



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
SITE				COORDINATES		APSF	1 OF 5	FB-12
APSF				N 79376 E 55003		ANGLE FROM HORIZONTAL 90		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH		
2/27/98	3/3/98	Graves/A. Jackson	Failing 1500	3 7/8 in	140 lb/ 30 in	156.3		
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:				
288.9		V / V /		N. Kidd/SAIC				
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80		288.9					
SS 1	▲	1-2-6	281.4	5		CLAYEY SAND (SC); medium red; loose; moist; subangular; poorly graded; fine to medium grained	Hand auger to 6 feet to check for interferences.	
SS 2	▲	8-12-17	279.9			same as above; medium dense		
SS 3	▲	8-13-22	278.4	10		same as above; dense		
SS 4	▲	8-12-13	276.9			same as above; medium dense; clay content increases with depth		
SS 5	▲	7-12-17	275.4			same as above; clay content is consistent through the interval		
SS 6	▲	8-18-20	273.9	15		POORLY GRADED SAND WITH CLAY (SP-SC); light red; dense; moist; subangular; poorly graded; fine to medium grained	Tobacco Road	
SS 7	▲	14-15-20	272.4			same as above; fine to lower medium grained		
SS 8	▲	12-12-17	270.9			same as above; medium dense; fine to medium grained		
SS 9	▲	11-19-19	269.4	20		same as above; dense; borderline clayey sand		
SS 10	▲	12-7-7	267.9			same as above; medium dense		
SS 11	▲	4-5-5	262.9	25		CLAYEY SAND (SC); medium red; loose; moist; subangular; poorly graded; fine grained; occasional medium to coarse grains		
SS 12	▲	12-18-27	257.9	30		same as above; dense; damp; fine to medium grained with occasional coarse sand and fine gravel		
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE		FINAL LOG		
						HOLE NO. FB-12		

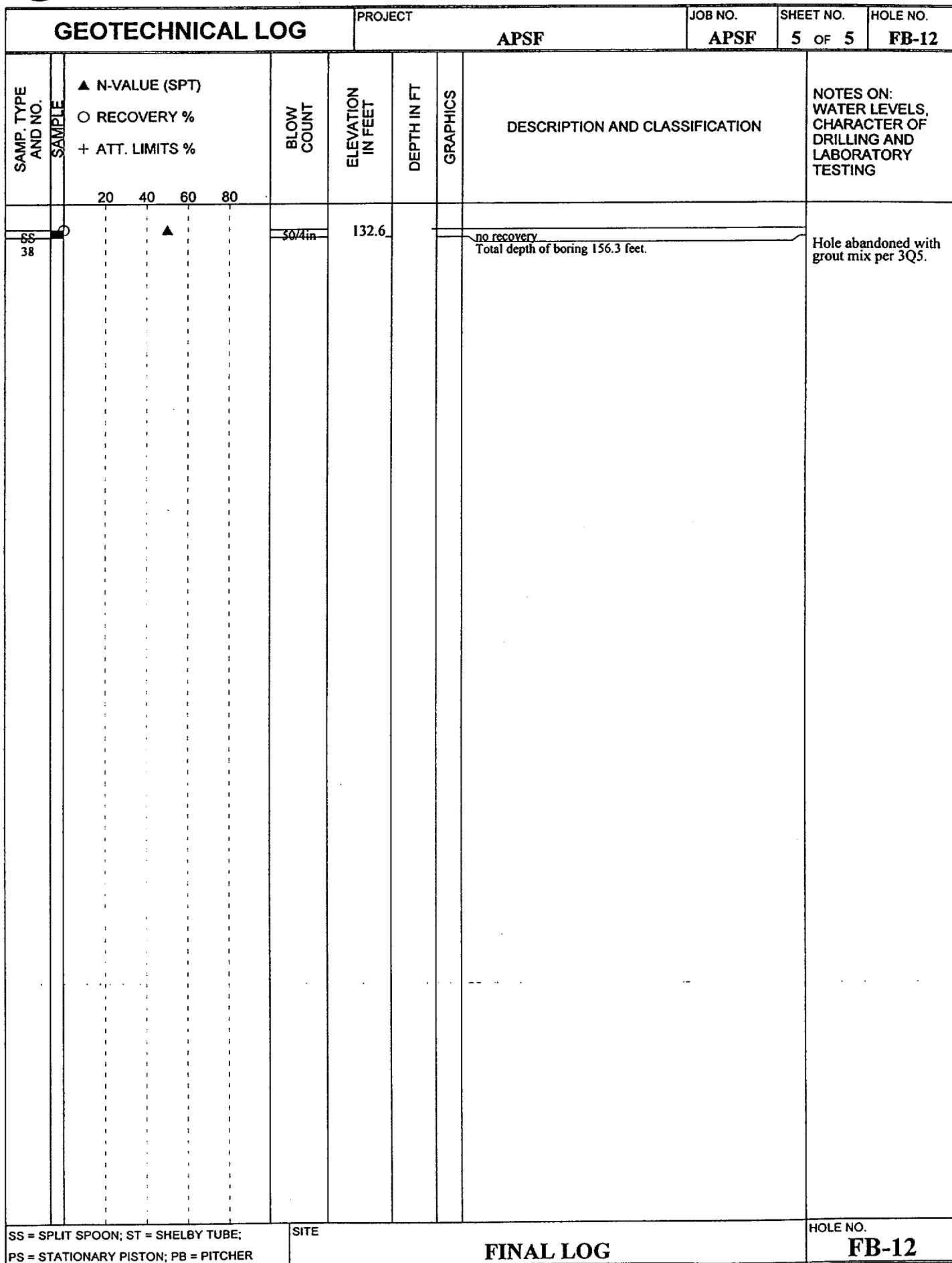


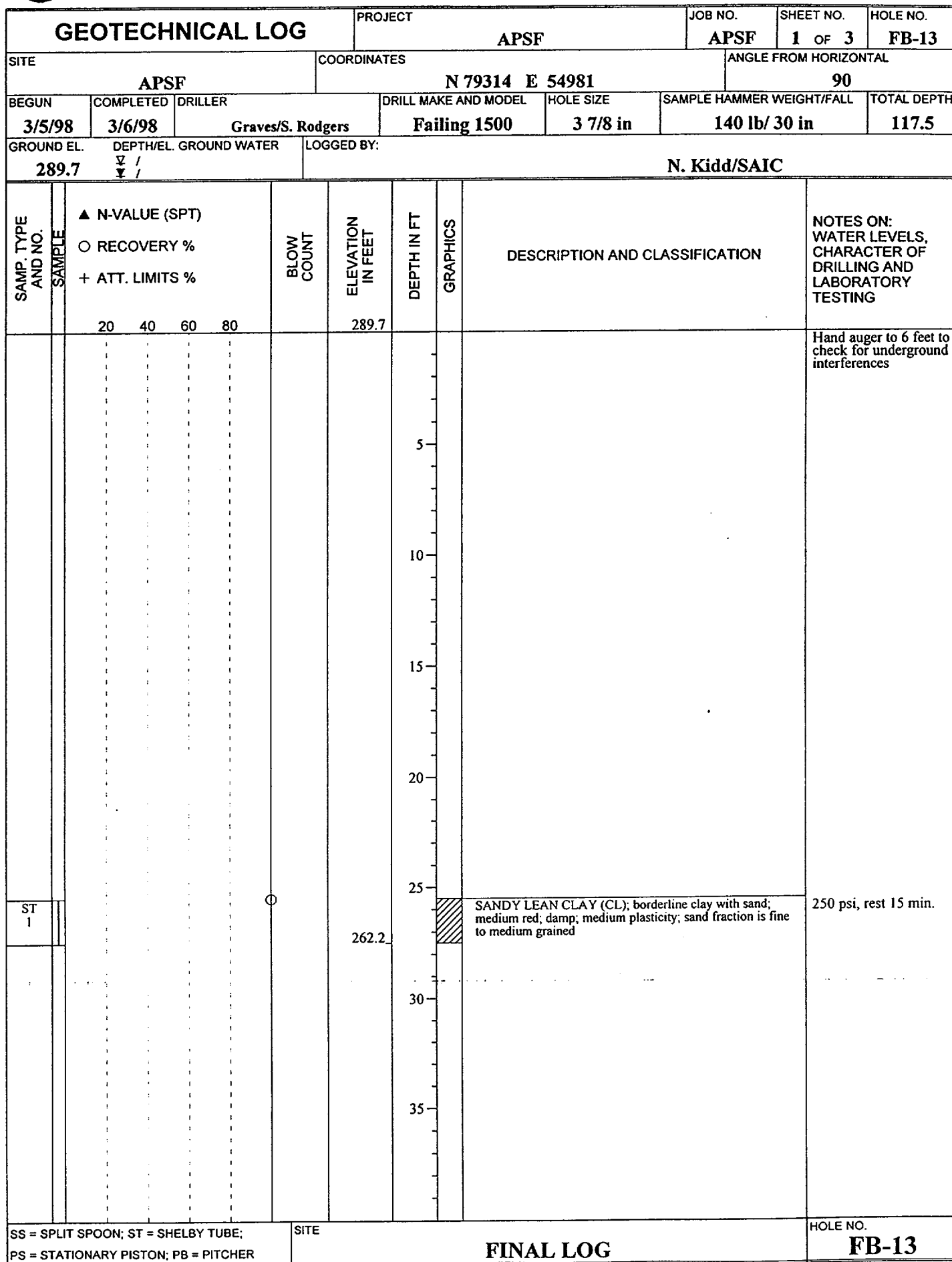
GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	2 OF 5	FB-12
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 13	20 40 60 80	10-15-18	252.9			same as above; light reddish yellow; moist; fine to lower medium grained	Dry Branch Water level 16.92 ft. This water level reading is inconsistent with the water level measurements from other borings in the area.
SS 14		14-21-25	247.9	40		POORLY GRADED SAND WITH CLAY (SP-SC); borderline trace clay; light reddish yellow with medium red bands; dense; damp; subangular; poorly graded; fine to medium grained	
SS 15		8-14-18	242.9	45		CLAYEY SAND (SC); light reddish yellow with light yellowish red zones and white wisps; dense; moist; subangular; poorly graded; fine to medium grained	
SS 16		18-30-25	237.9	50		POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish yellow; very dense; moist; subrounded; poorly graded; fine to medium grained	
SS 17		9-14-23	232.9	55		CLAYEY SAND (SC); medium yellow with dark yellowish brown zones; dense; moist; subangular; poorly graded; medium grained	
SS 18		15-25-34	227.9	60		POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish red; very dense; wet; subangular; poorly graded; fine to medium grained	
SS 19		25-19-21	222.9	65		same as above; clayey in places; medium brownish yellow; dense	
SS 20		18-21-28	217.9	70		same as above; trace heavies	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-12





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				APSF	APSF	4 OF 5	FB-12
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 29		WR/16in-5/2in	172.9			CLAYEY SAND (SC); medium yellowish brown; loose; wet; subangular; poorly graded; fine grained; trace heavies	Santee
SS 30		38-24-36	171.4			same as above; zone of very sticky sandy fat clay; very dense; fine to medium grained	
				120			
SS 31		17-27-42	166.4			POORLY GRADED SAND WITH CLAY (SP-SC); light yellowish brown; very dense; wet; subangular; poorly graded; fine to medium grained; trace heavies; trace mica	
				125			Warley Hill
SS 32		26-37-39	161.4			same as above	
				130			
SS 33		17-12-28	156.4			same as above with white bands; dense; fine to lower medium grained	
				135			
SS 34		5-7-8	151.4			CLAYEY SAND (SC); medium reddish brown with very light brown zones; medium dense; wet; subangular; poorly graded; fine grained; black mottles (Mn?); trace heavies	
				140			
SS 35		4-3-7	146.4			same as above; loose; clay content is slightly higher	
				145			
SS 36		12-19-30	141.4			SANDY FAT CLAY (CH); very light green and light brownish yellow with medium orange wisps; color grades downward to dark orange; hard; wet; high plasticity; sand is fine to medium grained; trace heavies	
				150			
SS 37		11-34-50/3in	136.6			same as above; medium brownish red with light green wisps; very hard	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				HOLE NO.
			FINAL LOG				FB-12



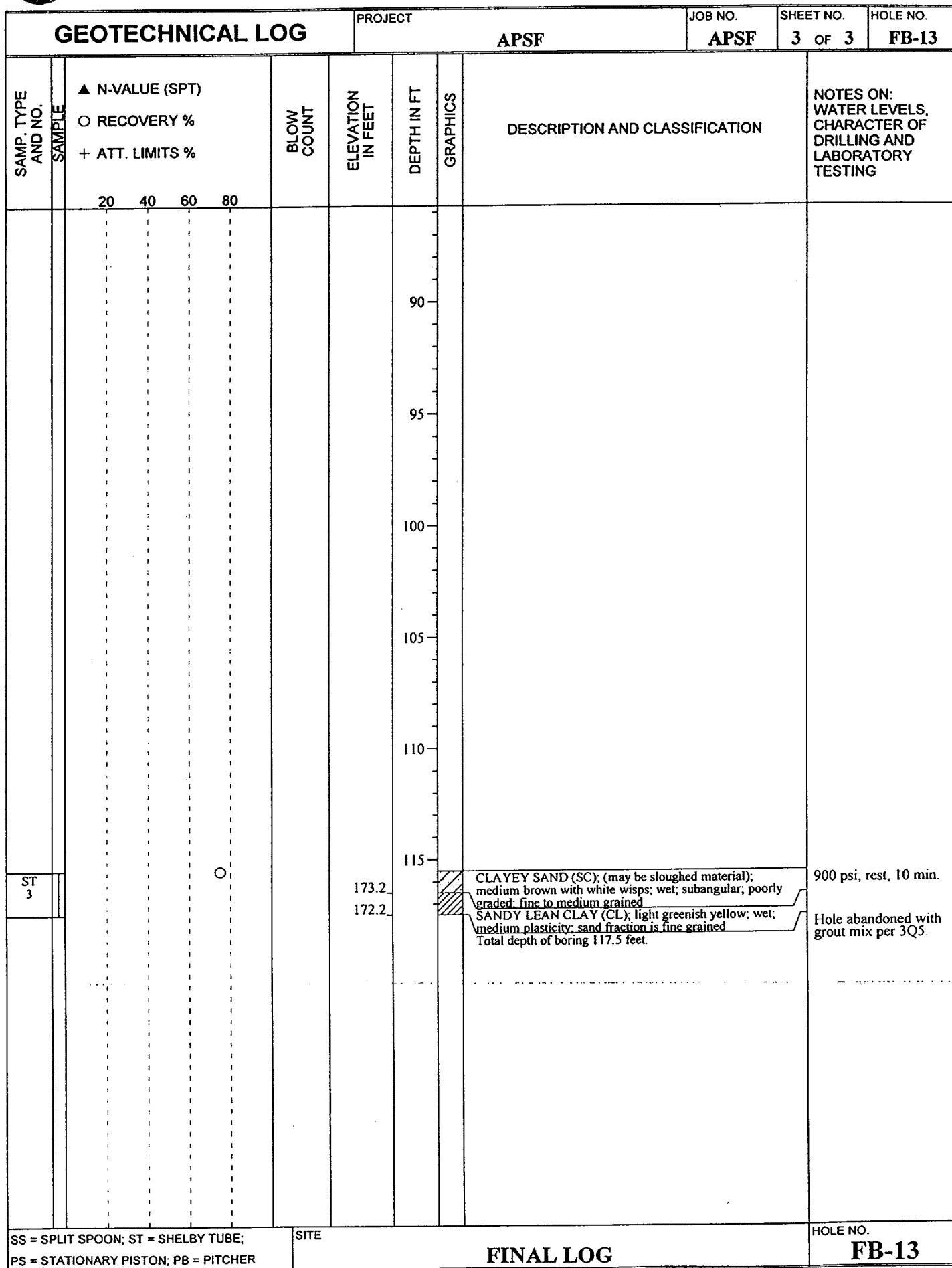




GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.				
				APSF		APSF	2 OF 3	FB-13				
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
		○ RECOVERY %										
		+ ATT. LIMITS %										
		20	40	60	80							
								45				
								50				
								55				
								60				
								65				
								70				
								75				
ST 2								212.7				
								211.7				
								80				
								85				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE				FINAL LOG		HOLE NO. FB-13

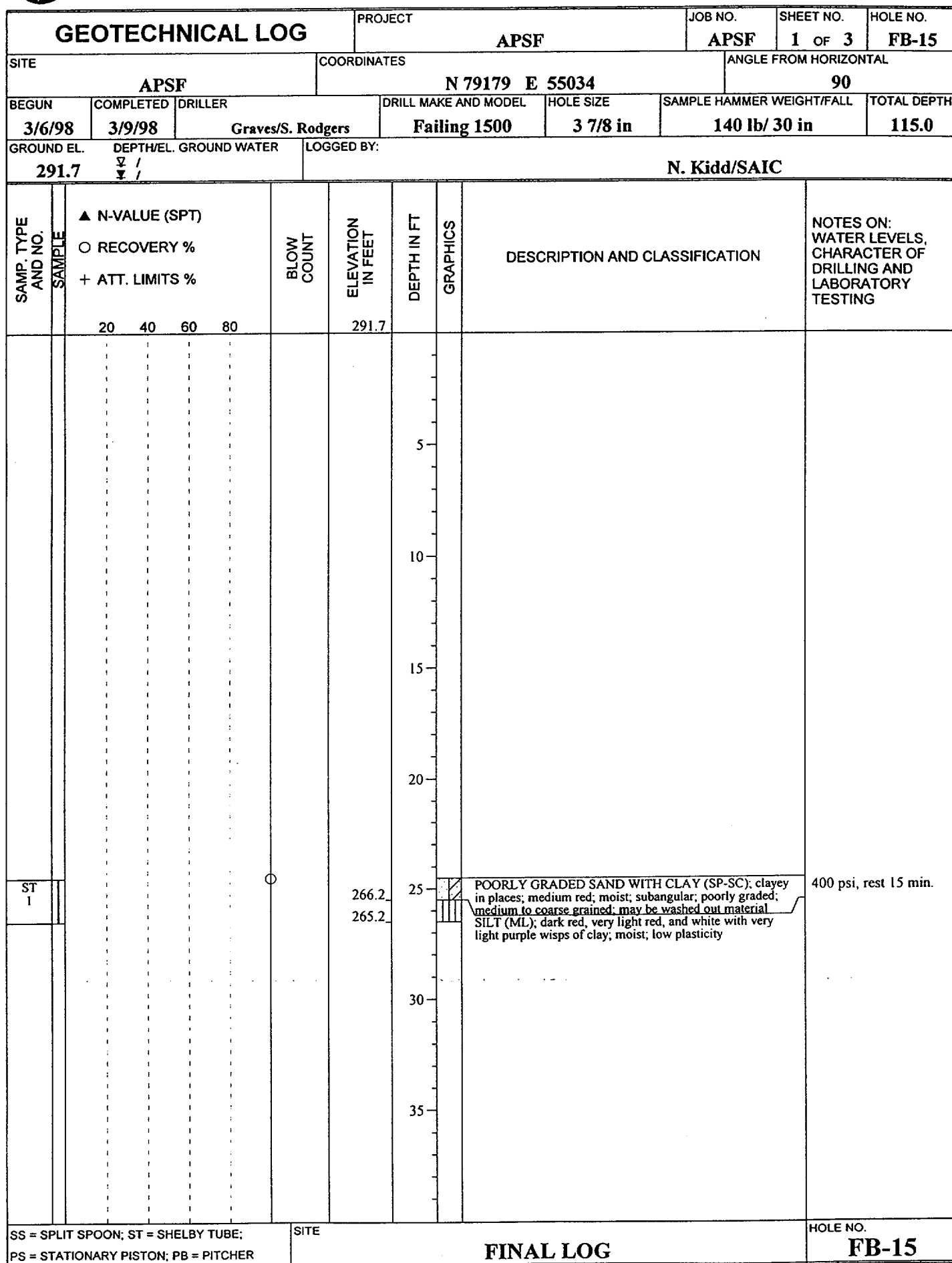
POORLY GRADED SAND WITH CLAY (SP-SC);
medium reddish brown with light yellow, purple, and white
clay stringers; wet; subangular; poorly graded; fine to
medium grained
LEAN CLAY WITH SAND (CL); sandy in places; black,
light tan, white, and brown; wet; medium plasticity; sand
fraction is fine grained; trace heavies

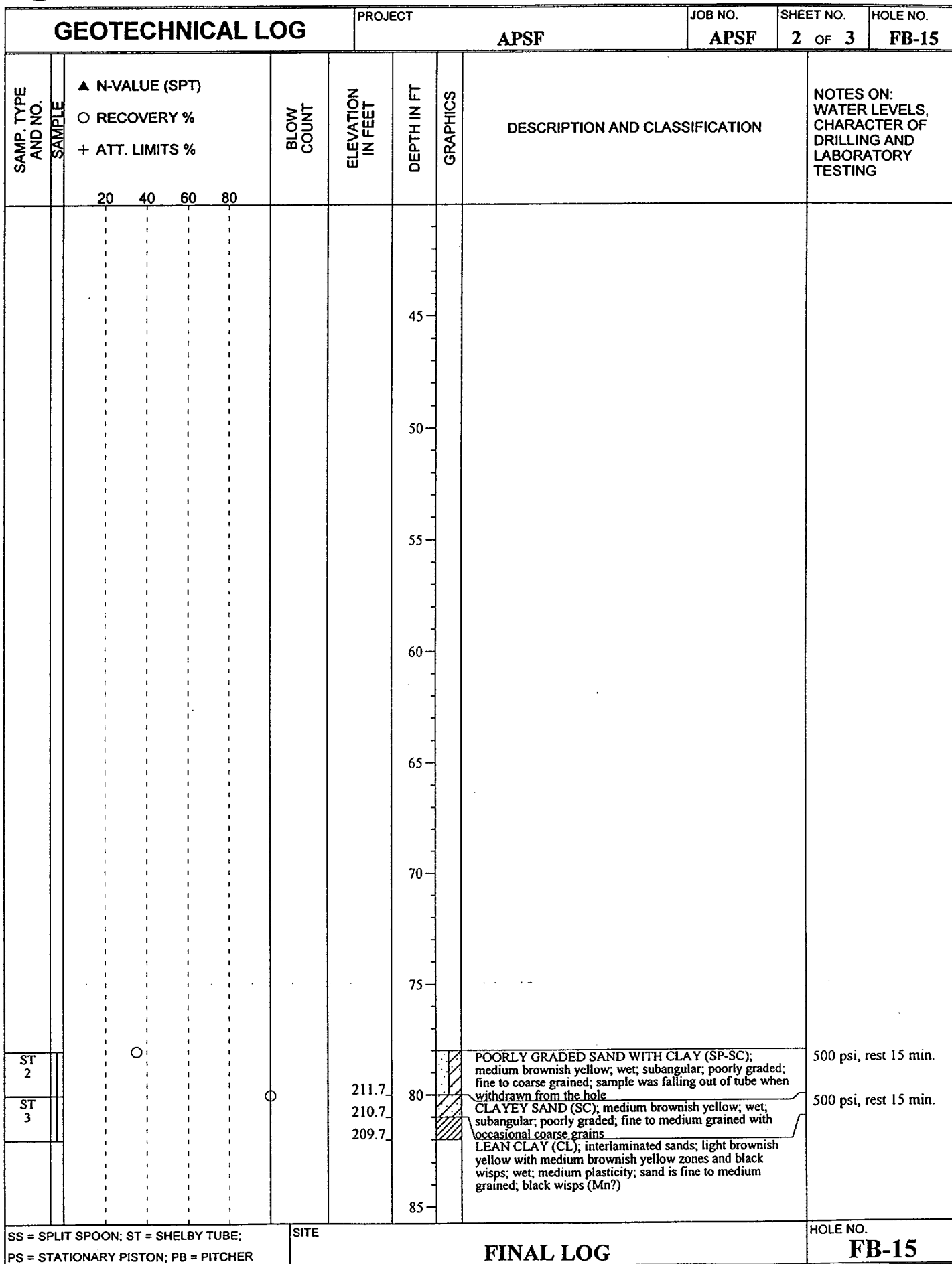
200 psi, rest 15 min.

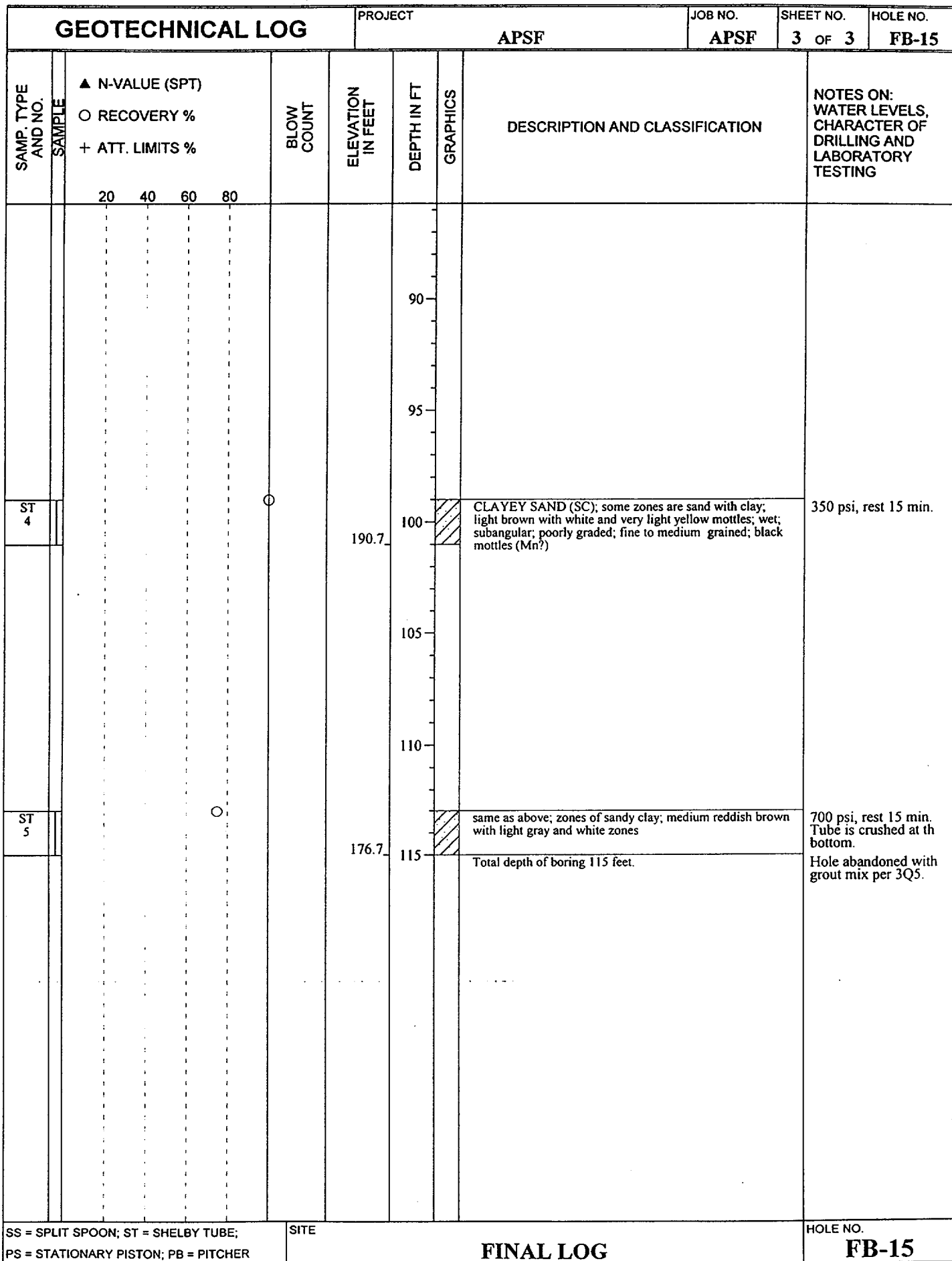




GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.	
SITE				COORDINATES		ANGLE FROM HORIZONTAL			
APSF				N 79437 E 54990		90			
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH			
3/6/98	3/6/98	Graves/S. Rodgers	Failing 1500	3 7/8 in	140 lb/ 30 in	80.0			
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:					
287.2		2 / 2 /		N. Kidd/SAIC					
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80		287.2						
ST			267.2	5 10 15 20 25 30 35 40 45 50 55 60 65 70		SANDY LEAN CLAY (CL); medium red; damp; low plasticity; sand fraction is fine to medium grained		700 psi, rest 15 min.	
ST			214.2	75		POORLY GRADED SAND WITH CLAY (SP-SC); medium red; wet; subangular; poorly graded; medium grained; could be washed out material		300 psi, rest 15 min.	
2			213.2			same as above; some zones are clayey; medium brownish yellow; fine to medium grained			
ST			208.2	80		SANDY LEAN CLAY (CL); with interbedded light yellow clay; medium red, white, and medium brownish yellow; wet; medium plasticity; sand fraction is coarse grained		300 psi, rest 15 min.	
3			207.2			CLAYEY SAND (SC); medium brownish yellow; wet; subangular; poorly graded; fine to medium grained; trace mica; trace heavies		Hole abandoned with grout mix per 3Q5.	
				Total depth of boring 80 feet					
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE				HOLE NO.	
				FINAL LOG				FB-14	










GEOTECHNICAL LOG				PROJECT APSF		JOB NO. APSF	SHEET NO. 1 OF 3	HOLE NO. FB-16	
SITE APSF			COORDINATES N 79212 E 54889			ANGLE FROM HORIZONTAL 90			
BEGUN 3/9/98	COMPLETED 3/11/98	DRILLER Graves/S. Rodgers	DRILL MAKE AND MODEL Failing 1500	HOLE SIZE 3 7/8 in	SAMPLE HAMMER WEIGHT/FALL 140 lb/ 30 in	TOTAL DEPTH 120.0			
GROUND EL. 290.1		DEPTH/EL. GROUND WATER ▽ / ▽ /	LOGGED BY: N. Kidd/SAIC						
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20 40 60 80		290.1					
					5				
					10				
					15				
ST 1			○	274.1 273.1			CLAYEY SAND (SC); medium red; damp; subangular; poorly graded; fine grained same as above; medium brownish yellow; fine to medium grained; trace heavies		500 psi, 15 min.
					20				
					25				
					30				
					35				

SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE FINAL LOG		HOLE NO. FB-16
---	--	--	-------------------	--	-------------------



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				APSF		APSF	2 OF 3	FB-16			
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY % + ATT. LIMITS %									
		20	40	60	80						
								45			
								50			
								55			
								60			Lost 800 gallons of water. See *
								65			
								70			
								75			
								80			
ST 2						○		85		LEAN CLAY WITH SAND (CL); sandy in places; interbedded sand stringers; medium brownish yellow,	350 psi, rest 15 min.

SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER	SITE	FINAL LOG	HOLE NO. FB-16
---	------	-----------	-------------------



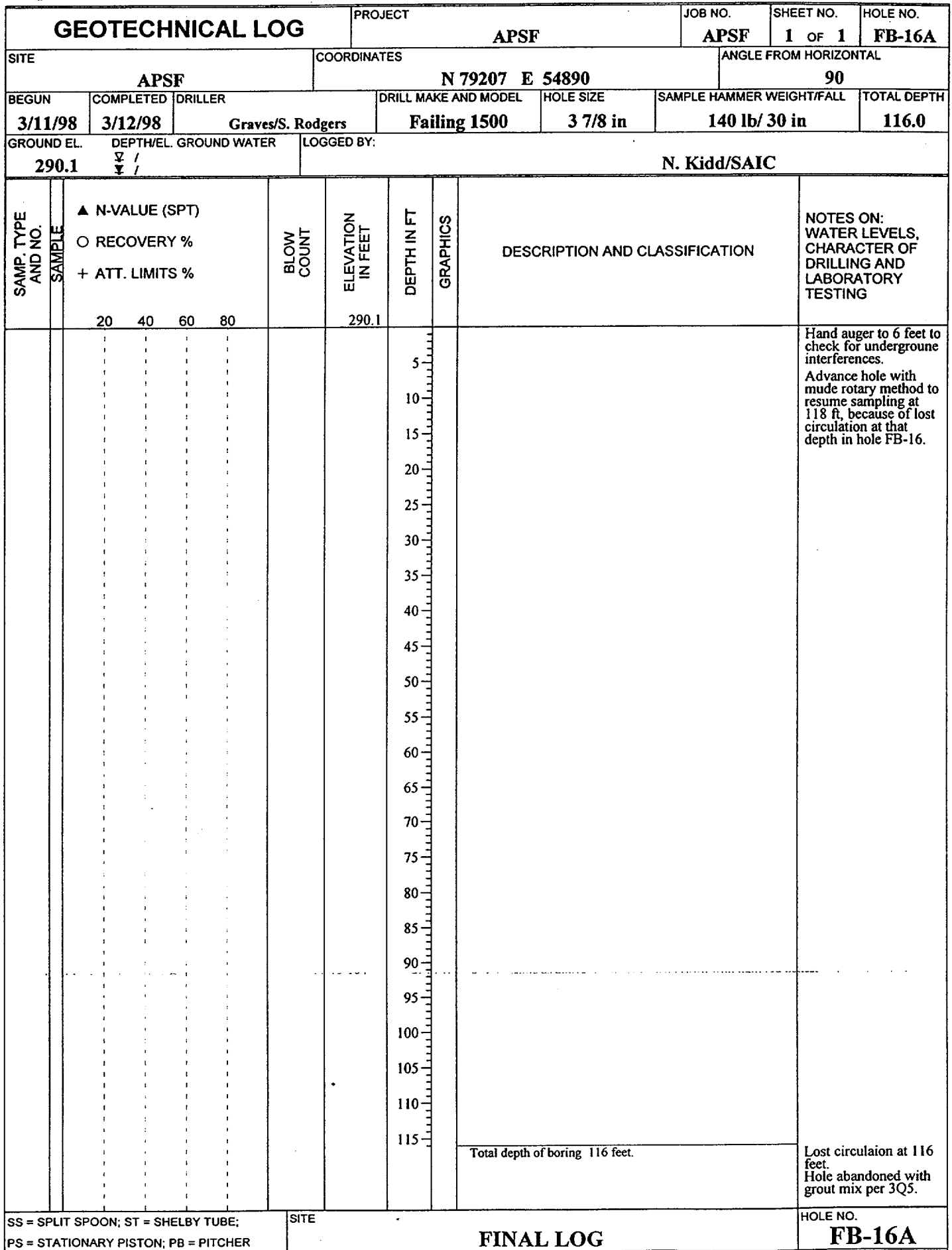
GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				APSF		APSF	3 OF 3	FB-16
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
			204.1			white, and light red; wet; medium plasticity; sand is fine to medium grained; this zone is highly variable with respect to sand content		
				90				
				95				
ST 3		○	191.1			CLAYEY SAND (SC); medium brownish yellow with light yellow and white zones; wet; subangular; poorly graded; fine to medium grained; black wisps (Mn?)	250 psi, rest 15 min.	
				100				
				105				
				110				
				115				
				120		Total depth of boring 120 feet,	Lost circulation at 118 ft., could not recover. Lost over 1000 gallons of water. After loss of circulation two buckets of bentonite pellets were placed in the hole and allowed to hydrate for approximately 1 hour. Tried to resume drilling and after 60 feet, rods got stuck in the hole. The hole took an additional 800 gallons of water. Hole finally abandoned with grout mix per 3Q5.	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-16





GEOTECHNICAL LOG				PROJECT Northeast Expansion		JOB NO. NEX	SHEET NO. 1 OF 4	HOLE NO. FB-17
SITE APSF-NE			COORDINATES N 79244 E 55520			ANGLE FROM HORIZONTAL 90		
BEGUN 4/13/98	COMPLETED 4/20/98	DRILLER B. Cunningham/Graves	DRILL MAKE AND MODEL Failing 1500	HOLE SIZE 3 7/8 in	SAMPLE HAMMER WEIGHT/FALL 140 lb/30 in	TOTAL DEPTH 145.0		
GROUND EL. 283.1		DEPTH/EL. GROUND WATER V / V /	LOGGED BY: N. Kidd/SAIC					
SAMP. TYPE AND NO.	DEPTH IN FEET	ELEVATION IN FEET	BLOW COUNT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING		
	5	283.1				Hand auger to 7 feet to check for underground interferences.		
SS 1			3-4-4		POORLY GRADED SAND (SP) trace clay; light brown; loose; wet; subangular; poorly graded; fine to medium grained; trace heavies			
SS 2		274.6	3-5-9		POORLY GRADED SAND WITH CLAY (SP-SC); medium brown with medium yellowish red mottles; medium dense; moist; subangular; poorly graded; fine to medium grained			
SS 3	10	273.1	9-9-15		CLAYEY SAND (SC); light red with yellowish red, brownish yellow, and gray zones; medium dense; damp; subangular; poorly graded; fine to medium grained			
SS 4		271.6	7-14-13		same as above; medium red with light yellow and white mottles			
SS 5		270.1	10-15-22		LEAN CLAY WITH SAND (CL); light brownish red, brownish yellow, and gray; hard; damp; medium plasticity; sand is fine to medium grained			
SS 6	15	268.6	9-14-19		SANDY LEAN CLAY (CL) with clayey sand zones; medium brownish yellow with medium red zones; hard; damp; medium plasticity; sand is fine to medium grained			
SS 7		267.1	12-15-21		CLAYEY SAND (SC); light brownish yellow with white wisps and medium red zones; dense; damp; subangular; poorly graded; fine to medium grained			
SS 8		265.6	10-14-16		same as above; light red with white flecks and medium yellowish brown zones; medium dense; fine to lower medium grained; trace mica			
SS 9	20	264.1	10-12-17		same as above; light red; no mica observed			
SS 10		262.6	10-13-17		SILTY SAND (SM) some areas are clayey; medium red with brownish yellow, brown, and white bands; medium dense; damp; subangular; poorly graded; fine to lower medium grained			
SS 11		261.1	9-14-16		CLAYEY SAND (SC); medium red; medium dense; damp; subangular; poorly graded; fine to lower medium grained			
SS 12		259.6	10-12-15		same as above; silty in places; medium red with white and brownish yellow zones			
SS 13	25	258.1	11-13-16		same as above; light yellowish brown with light red zones; fine to medium grained			
SS 14		256.6	13-15-17		same as above; with white wisps; dense			
SS 15		255.1	12-19-22		POORLY GRADED SAND WITH CLAY (SP-SC); some areas trace clay; light brownish yellow with light red and light brown zones; dense; wet; subangular; poorly graded; medium grained			
SS 16	30	253.6	17-24-29		POORLY GRADED SAND (SP) trace clay; some areas with clay; light red with light reddish brown zones and dark yellowish brown wisps; very dense; damp; subangular; poorly graded; medium grained			
SS 17		252.1	20-31-21		same as above; light red with very light orange and white zones			
SS 18		250.6	12-17-19		CLAYEY SAND (SC) some zones are less clayey; light reddish yellow with light red zones and white pellets; dense; damp; subangular; poorly graded; fine to medium			
SS		249.1	15-20-21					

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-17



GEOTECHNICAL LOG			PROJECT		JOB NO.	SHEET NO.	HOLE NO.
			Northeast Expansion		NEX	2 OF 4	FB-17
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80						
19							
SS 20		14-25-29	247.6			grained same as above; medium yellowish brown with light yellowish brown zones and white wispy striations; moist	
SS 21		18-23-28	246.1			POORLY GRADED SAND WITH CLAY (SP-SC) some areas trace clay; medium brownish yellow with dark brown pods; very dense; damp; subangular; poorly graded; medium grained	
SS 22		19-22-25	244.6			same as above; clayey in places; medium yellowish brown; moist	
SS 23		15-27-36	243.1	40		same as above; dense; wet	
SS 24		19-20-25	241.6			same as above; clayey in places; very dense; moist	
SS 25		14-14-20	240.1			same as above; borderline clayey sand; dense	
SS 26		14-18-16	238.6	45		CLAYEY SAND (SC); medium brownish red with dark yellowish brown zones and wispy white striations; dense; moist; subangular; poorly graded; fine to medium grained	
SS 27		11-20-26	237.1			same as above; medium red	
SS 28		22-26-27	235.6			POORLY GRADED SAND WITH CLAY (SP-SC) some areas clayey; medium reddish brown; very dense; damp; subangular; poorly graded; fine to medium grained	
SS 29		21-24-31	234.1			same as above; light reddish brown with white zones; moist; poorly graded	
SS 30		23-27-28	232.6	50		same as above	
SS 31		20-24-29	231.1			POORLY GRADED SAND (SP) trace clay; some areas with clay; light brown with light brownish yellow and white mottles; very dense; damp; subangular; poorly graded; fine to medium grained	
SS 32		18-21-25	229.6			same as above; light brownish yellow	
SS 33		12-21-19	228.1	55		same as above; some zones with clay; dense	
SS 34		7-18-15	226.6			POORLY GRADED SAND WITH CLAY (SP-SC) some areas trace clay; medium reddish yellow; dense; moist; subangular; poorly graded; fine to medium grained	
SS 35		14-17-22	225.1			same as above; light brownish yellow; trace heavies	
SS 36		18-27-34	223.6	60		same as above; some zones clayey; medium yellowish brown; fine to lower coarse grained	
SS 37		21-19-25	222.1			same as above; very dense	
SS 38		19-22-26	220.6			same as above; dense	
SS 39		14-17-21	219.1	65		same as above; medium brownish yellow; fine to medium grained; trace lower coarse	
SS 40		13-16-15	217.6			same as above; light brown	
SS 41		4-4-6	216.1			same as above; some zones are clayey sand; medium brownish yellow	
SS 42		3-4-6	214.6	70		SANDY LEAN CLAY (CL) with interlaminated sand and lean clay; lt brownish yellow with med reddish brown zones; stiff; wet; med plasticity; sand is fine to med grained; black staining (Mn?); trace heavies; flattened quartz pebbles	
SS 43		4-4-6	213.1			SANDY FAT CLAY (CH) some zones are lean clay; interlaminated cse sand; med yellowish brown with med reddish brown and white zones; stiff; wet; high plasticity; sand is fine to med grained with some cse sand; black staining (Mn?); trace heavies	
SS 44		5-8-9	211.6			CLAYEY SAND (SC) with lean clay pods; med yellowish brown with lt brownish yellow zones; loose; wet; subangular; poorly graded; fine to lwr med grained; trace heavies; black staining (Mn?)	
SS 45		6-5-6	210.1			same as above; interbedded lt brownish yellow lean clay	
			208.6				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE		FINAL LOG		HOLE NO. FB-17



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 4	FB-17
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 46		5-7-9	207.1			and med brownish yellow sand; med brownish yellow with v lt yellow and dk reddish brown laminae; med dense; fine to med grained; clay content varies widely	
SS 47		8-14-23	205.6			same as above; with lt brownish yellow lean clay laminae; med reddish yellow with dk brown zones; moist; heavy black staining (Mn?)	
SS 48		12-14-15	204.1			LEAN CLAY (CL); lt brownish yellow with dk reddish brown and black fracture coatings (Mn?); v stiff; wet; med plasticity; sand is fine grained and sparse	
SS 49		8-14-16	202.6	80		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; lt brownish yellow clay at top of sample; med yellowish brown with v lt yellow zones; dense; wet; subangular; poorly graded; fine to med grained; black pellets (Mn?); trace heavies	
SS 50		10-18-25	201.1			same as above; med dense	
SS 51		14-20-20	199.6			same as above; lt yellowish brown	
SS 52		12-18-17	198.1	85		same as above; dense	
SS 53		14-22-21	196.6			POORLY GRADED SAND WITH SILT (SP-SM) with v lt yellow clay wisps; lt yellowish brown; dense; wet; subangular; poorly graded; fine to med grained; abundant black (Mn?) and dk brown staining	
SS 54		13-16-21	195.1			same as above with white streaks	
SS 55		17-16-27	193.6			same as above with some clayey zones	
SS 56		13-20-18	192.1	90		same as above	
SS 57		10-10-7	190.6			CLAYEY SAND (SC); lt reddish brown; dense; wet; subangular; well graded; fine to lwr cse grained; black zone within interval is Mn?	
SS 58		4-6-13	189.1			same as above; some zones are sand with clay; lt brown with white zones; Mn? staining	
SS 59		13-20-21	187.6	95		same as above; interbedded white clayey sands; lt reddish brown with white zones; med dense; poorly graded; fine to med grained	
SS 60		16-23-22	186.1			POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey sand; lt reddish brown with white zones; med dense; wet; subangular; poorly graded; fine to med grained; trace lwr cse	
SS 61		9-10-15	184.6			same as above; some zones are sandy clay; lt brown with v lt yellow zones; dense; Mn? staining	
SS 62		12-13-16	183.1	100		POORLY GRADED SAND (SP) trace clay; lt brown; dense; wet; subrounded; poorly graded; med to cse grained; trace heavies	
SS 63		1/2 in-1/15 in-1/1 in	181.6			WELL GRADED SAND WITH CLAY (SW-SC) bottom of interval is sand trace clay; lt brown with white zones; med dense; wet; subrounded; well graded; trace heavies; fine to coarse grained	Possible location of a soft zone @ 100 ft. Losing drilling fluid.
SS 64		1/10 in-1/2 in-4	180.1			same as above; med brown and lt brown with dk reddish brown mottles; subangular; fine to cse grained; black pellets (Mn?)	
SS 65		3-3-2	178.6	105		CLAYEY SAND (SC) clay content varies; med brown with lt brown and white zones and dk brown wisps; v loose; wet; subangular; poorly graded; fine to med grained; black stains (Mn?)	
SS 66		1/21 in	175.4			same as above; few interlaminated lt yellowish brown clay wisps; lt reddish brown with lt brown and white zones; loose; trace heavies	Overdrilled
SS 67		7-6-8	173.6			same as above; med reddish brown with v lt green clay laminae; fine grained	
SS 68		5-9-9	172.1	110		same as above; with silty zones; white, v lt green, and lt brown; v loose; v fine to lwr med grained; weathered shell fragments?; silicified zone at top of interval; trace heavies	
SS 69		19-36-32	170.6			same as above; interlayered with sand with clay; white and lt brown with v lt green zone at top of interval; med dense; fine to med grained; pelecypod fragments	
SS 70		15-26-24	169.1			WELL GRADED SAND WITH CLAY (SW-SC) bands of clayey sand; lt brown and lt yellow with white clayey sand bands; med dense; wet; subangular; well graded; fine to lwr cse grained; trace heavies; pelecypod fragment	
SS		9-14-13				POORLY GRADED SAND (SP) trace clay; lt brown with v lt brown and v lt green zones; v dense; wet; subangular; poorly graded; fine to med grained; abundant heavies; shell fragments; calcareous nodules	
SS						SILTY SAND (SM); lt yellowish brown with white clayey zones; dense; wet; subangular; poorly graded; fine grained;	Circulation good but
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				FINAL LOG			HOLE NO. FB-17



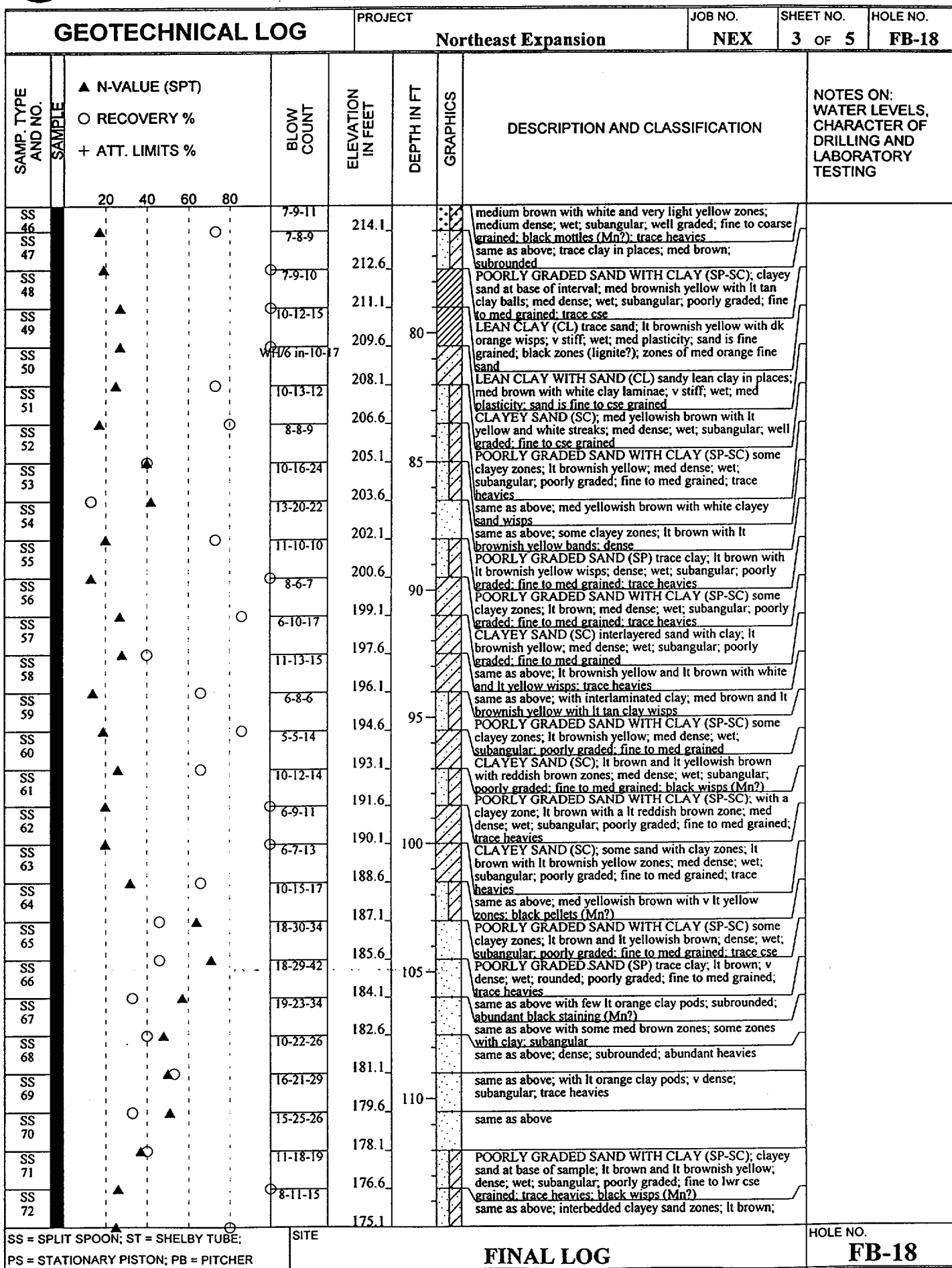
GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	4 OF 4	FB-17
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
71						calcareous nodules; pelecypod fragments	continuing to lose drilling fluid. Rod drop @ 115.5-117.67 ft.
SS 72		3/26 in	167.6			POORLY GRADED SAND WITH SILT (SP-SM); lt brown and lt yellowish brown; med dense; wet; subangular; poorly graded; fine grained; trace heavies same as above; lt brown; v loose; v fine grained; dk purple mottle	
			165.4			SILTY SAND (SM); v lt yellowish brown with orange mottles and lt green zones; dense; wet; subangular; poorly graded; v fine grained; trace heavies	Lost over 1000 gallons of water from 116.67-122 ft.
SS 73		8-14-24				same as above; borderline sand with silt; v lt greenish brown and v lt brown; some zones have lt orange mottles; v dense	
SS 74		10-27-35	163.6	120		same as above; v lt brown to lt brown; dense	Lost 1000 gallons of fluid between 124 and 134.5 feet.
SS 75		12-20-30	162.1			POORLY GRADED SAND (SP) trace silt; v lt brown with white zones; v dense; wet; subangular; poorly graded; v fine grained; trace heavies	
SS 76		13-28-36	160.6			same as above	Continuing to lose fluid.
SS 77		15-30-32	159.1			SILTY SAND (SM); lt greenish brown; med dense; wet; subangular; poorly graded; v fine grained; trace heavies	
SS 78		8-5-4	157.6	125		same as above; with white clay wisps; loose; fine grained	Hole caved in over weekend 4/18, 19. Had to be cleaned out before grouting.
SS 79		19 in-1/3 in-5	156.1			CLAYEY SAND (SC) some zones are silty; lt greenish brown with v lt gray zones; loose; wet; subangular; poorly graded; v fine grained	
SS 80		WR/12 in-7	154.6			same as above	Hole abandoned with grout mix per 3Q5.
SS 81		WR/12 in-WH/3 in-5/3 in	153.1	130		same as above; lt yellowish brown with lt orange mottles; med dense; silicified turritella shells; trace heavies	
SS 82		WR/6 in-4-8	151.6			same as above; lt brown with lt orange mottles	Hole caved in over weekend 4/18, 19. Had to be cleaned out before grouting.
SS 83		3-5-7	150.1			same as above	
SS 84		4-8-9	148.6	135		POORLY GRADED SAND WITH SILT (SP-SM) some zones are same as above; lt greenish brown; dense; wet; subangular; poorly graded; fine grained; silicified turritella shells; trace heavies	Hole caved in over weekend 4/18, 19. Had to be cleaned out before grouting.
SS 85		8-14-25	147.1			CLAYEY SAND (SC) with wispy lt greenish gray clay; lt grayish brown with lt orange mottles; dense; wet; subangular; poorly graded; fine to med grained; silicified chips and shells	
SS 86		5-12-38	145.6			same as above; some zones silty; lt greenish brown; med dense; v fine grained; silicified nodules and pelecypod shells; trace heavies; trace mica	Hole caved in over weekend 4/18, 19. Had to be cleaned out before grouting.
SS 87		5-7-8	144.1	140		same as above; sandy clay in places; white and dk brownish red; dense; fine to med grained; clay content varies; some material calcareous mud?; shell fragments	
SS 88		WR/3 in-2/3 in-8-18	142.6			same as above; with interlaminated lt green clays; med green and dk reddish orange with white pellets and lt green clay laminae and dk green at tip of interval; med dense	Hole caved in over weekend 4/18, 19. Had to be cleaned out before grouting.
SS 89		4-13-12	141.1			same as above; intermixed with lean clay; dk greenish gray; fine grained; abundant mica; green clay	
SS 90		9-12-16	139.6			Total depth of boring 145 feet.	
			138.1	145			
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				FINAL LOG			HOLE NO. FB-17



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.				
APSF-NE				Northeast Expansion		NEX	1 OF 5	FB-18				
SITE		COORDINATES				ANGLE FROM HORIZONTAL						
		N 79331 E 55131				90						
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH						
4/20/98	4/27/98	B Cunningham/Graves	Failing 1500	3 7/8 in	140 lb/30 in	159.3						
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:									
290.1		▽ / ▽ /	N. Kidd/SAIC									
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
		20	40	60	80							
						290.1					Hand auger to 7 feet to check for underground interferences.	
								5				
SS 1		▲	○			5-7-11				POORLY GRADED SAND (SP) trace clay; light reddish brown; medium dense; damp; subangular; poorly graded; fine to medium grained; trace heavies		
SS 2		▲		○		6-13-20	281.6			same as above; light yellowish red with light brown zones; dense		
SS 3		▲			○	11-19-23	280.1	10		CLAYEY SAND (SC); medium red; dense; damp; subangular; poorly graded; fine to medium grained		
SS 4		▲			○	12-14-17	278.6			same as above; wet		
SS 5		▲			○	12-14-23	277.1			POORLY GRADED SAND WITH CLAY (SP-SC); medium reddish brown; dense; moist; subangular; poorly graded; fine to medium grained		
SS 6		▲			○	12-13-16	275.6	15		CLAYEY SAND (SC); medium red; medium dense; moist; subangular; poorly graded; fine to medium grained		
SS 7		▲			○	10-12-10	274.1			same as above; trace mica		
SS 8		▲			○	5-6-7	272.6			SILTY SAND (SM); medium red with light brown wisps; medium dense; moist; subangular; poorly graded; fine grained; trace mica; trace heavies		
SS 9		▲			○	4-7-8	271.1	20		CLAYEY SAND (SC); medium reddish brown; medium dense; moist; subangular; poorly graded; fine to medium grained with lower coarse grains		
SS 10		▲			○	6-8-8	269.6			SANDY LEAN CLAY (CL); medium reddish brown; very stiff; moist; medium plasticity; sand is fine to coarse grained		
SS 11		▲			○	7-10-9	268.1			CLAYEY SAND (SC); medium red; medium dense; moist; subangular; poorly graded; fine to medium grained		
SS 12		▲			○	6-6-7	266.6			same as above; trace coarse grains		
SS 13		▲			○	5-6-6	265.1	25		POORLY GRADED SAND WITH CLAY (SP-SC); medium yellowish brown with dark red mottles; medium dense; moist; subangular; poorly graded; fine to lower medium grained; trace mica		
SS 14		▲	○			3-4-5	263.6			CLAYEY SAND (SC); medium yellowish brown; loose; wet; subangular; poorly graded; fine to lower medium grained; trace mica		
SS 15		▲		○		4-5-8	262.1			same as above; medium brown; medium dense; moist; fine to medium grained		
SS 16		▲		○		6-9-9	260.6	30		SANDY LEAN CLAY (CL); medium reddish brown; very stiff; moist; low plasticity; sand is fine to lower coarse grained		
SS 17		▲		○		6-9-16	259.1			CLAYEY SAND (SC); medium reddish brown; medium dense; moist; subangular; poorly graded; fine to medium grained; trace mica		
SS 18		▲		○		12-16-15	257.6			POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light brownish yellow with light red bands and white wisps; dense; moist; subangular; poorly graded; fine to medium grained		
SS		▲		○		12-14-15	256.1					
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE		FINAL LOG				HOLE NO. FB-18

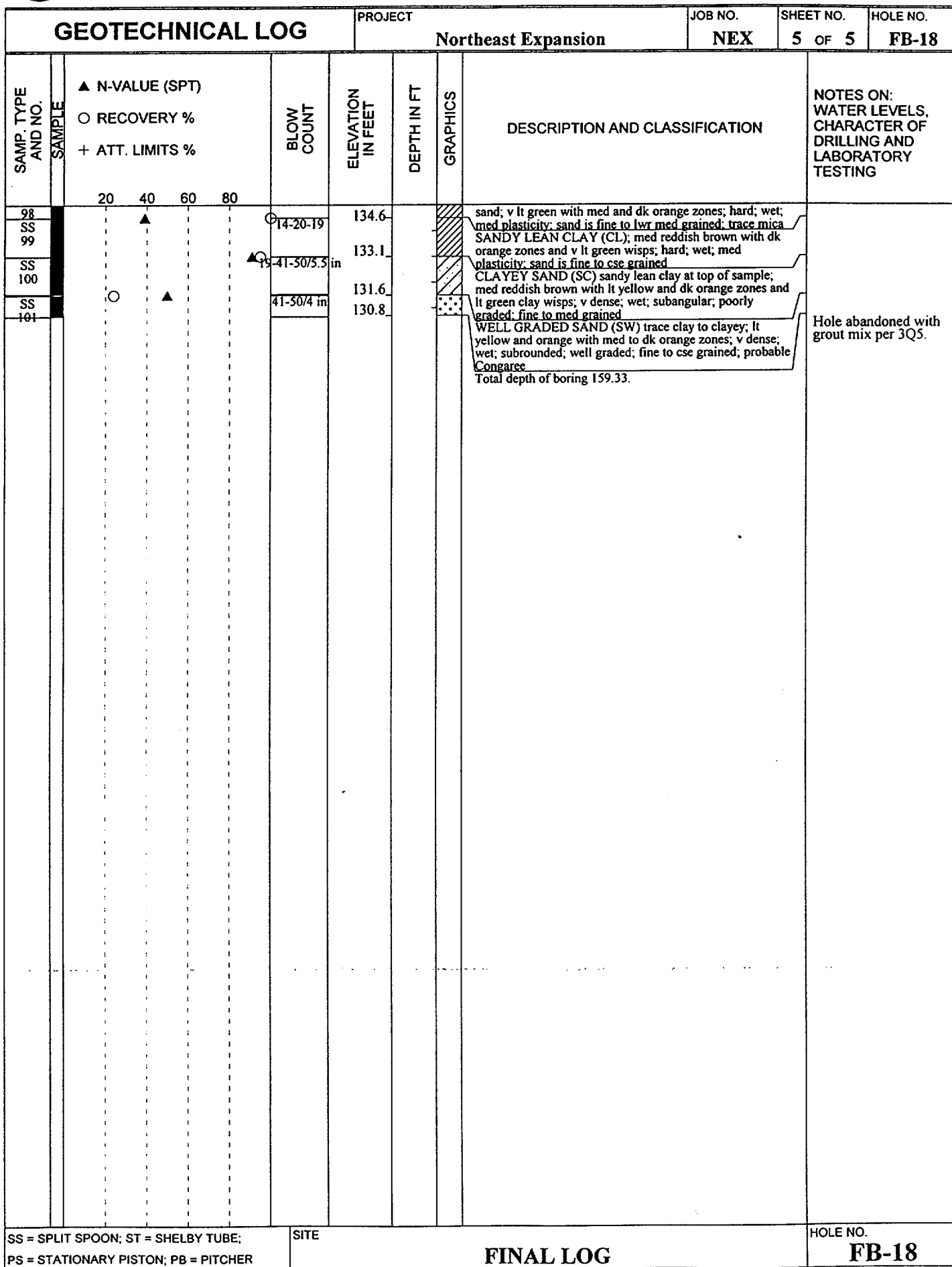


GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion		NEX	2 OF 5	FB-18
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING	
	20 40 60 80							
19	▲	8-14-14	254.6			CLAYEY SAND (SC); light yellowish brown with medium red bands; medium dense; moist; subangular; poorly graded; fine to medium grained		
SS 20			253.1			same as above; light brownish yellow with light red bands and white wisps; trace heavies		
SS 21	▲	10-15-15	251.6			same as above; some zones are sand with clay; light brownish yellow with light orange bands; trace mica		
SS 22	▲	11-19-20	250.1	40		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light yellowish brown with light red zones and white wisps; dense; moist; subangular; poorly graded; fine to medium grained; some zones are wet		
SS 23	○	17-24-29	248.6			same as above; light red and light brownish yellow with white wisps; very dense		
SS 24	○	17-30-27	247.1			POORLY GRADED SAND (SP) trace clay; some zones with clay; medium red with light brownish yellow zones and white wisps; very dense; wet; subangular; poorly graded; fine to medium grained; trace coarse grains		
SS 25	○	20-17-24	245.6			same as above; dense		
SS 26	▲	22-22-31	244.1	45		POORLY GRADED SAND WITH CLAY (SP-SC); light brownish yellow and light red with white spots; very dense; moist; subangular; poorly graded; fine to medium grained; trace mica		
SS 27	▲	13-22-23	242.6			same as above; dense		
SS 28	▲	14-14-20	241.1			CLAYEY SAND (SC) borderline sand with clay; light red and light brownish yellow; dense; moist; subangular; poorly graded; fine to medium grained; trace mica		
SS 29	▲	16-18-18	239.6	50		POORLY GRADED SAND (SP) trace clay; upper part of the interval is sand with clay (as above); medium brown with light red and light brownish yellow zones and white wisps; dense; wet; subrounded; poorly graded; fine to medium grained; trace mica; trace heavies		
SS 30	○	10-13-20	238.1			CLAYEY SAND (SC); light brownish yellow; dense; wet; subangular; poorly graded; fine to medium grained		
SS 31	○	20-26-30	236.6			POORLY GRADED SAND (SP) trace clay; medium brown; very dense; wet; subrounded; poorly graded; medium to lower coarse grained		
SS 32	○	17-14-16	235.1	55		POORLY GRADED SAND WITH CLAY (SP-SC); medium brown with light red zones; medium dense; wet; subrounded; poorly graded; medium to lower coarse grained; trace heavies		
SS 33	▲	11-13-15	233.6			CLAYEY SAND (SC); medium red; medium dense; wet; subangular; poorly graded; fine to medium grained; black spots		
SS 34	▲	10-9-12	232.1			same as above; medium yellowish brown with dark brown zones and white wisps; trace heavies		
SS 35	○	12-16-22	230.6	60		POORLY GRADED SAND WITH CLAY (SP-SC); medium brown; dense; moist; subangular; poorly graded; fine to medium grained; trace heavies		
SS 36	○	14-19-25	229.1			same as above		
SS 37	○	16-20-22	227.6			POORLY GRADED SAND (SP) trace clay; some zones with clay; light brown; dense; wet; subrounded; poorly graded; fine to lower coarse grained		
SS 38	○	16-17-19	226.1			same as above; medium yellowish brown with medium brown bands; moist; subangular; fine to medium grained; trace coarse grains; trace heavies		
SS 39	○	14-15-22	224.6	65		same as above; wet		
SS 40	○	20-22-27	223.1			same as above		
SS 41	▲	16-14-18	221.6			POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish yellow; dense; wet; subangular; poorly graded; fine to medium grained; trace coarse; trace heavies		
SS 42	▲	12-13-18	220.1	70		same as above; light brownish yellow; moist; black mottles (Mn?)		
SS 43	○	16-19-23	218.6			same as above; some zones trace clay; wet		
SS 44	○	16-16-21	217.1			POORLY GRADED SAND (SP) some zones with clay; medium brown with white wisps; dense; wet; subangular; poorly graded; fine to medium grained; trace coarse; trace heavies		
SS 45	○	12-10-13	215.6			WELL GRADED SAND WITH CLAY (SW-SC) clay content varies widely from trace clay to clayey in places;		
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE		FINAL LOG			
					HOLE NO. FB-18			





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.	
				Northeast Expansion	NEX	4 OF 5	FB-18	
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY %						
		20 40 60 80						
SS 73			10-12-13				med dense; fine to med grained; black staining (Mn?)	
SS 74			8-20-14	173.6			CLAYEY SAND (SC) with sandy clay bands; lt brown with v lt greenish brown bands; med dense; wet; subangular; well graded; fine to cse grained; larger grains are subrounded to rounded	
SS 75			7-14-20	172.1			SANDY LEAN CLAY (CL) with clayey sand zones; med reddish brown with v lt brown zones; hard; wet; med plasticity; fine to med grained; trace mica; trace heavies	
SS 76			7-14-22	170.6	120		CLAYEY SAND (SC); lt reddish brown with v lt brown zones; dense; wet; subangular; poorly graded; fine grained	
SS 77			12-16-25	169.1			same as above; med yellowish brown with black Mn spots; fine to lwr med grained with Mn staining	
				167.6			same as above; with some silty sand layers; med reddish brown with lt brown zones; trace clay rip-ups	
SS 78			8-17-26				SILTY SAND (SM); med yellowish brown; dense; wet; subangular; poorly graded; fine to lwr cse grained	
SS 79			18-43-48	164.8	125		POORLY GRADED SAND WITH SILT (SP-SM); med yellowish brown; v dense; wet; subangular; poorly graded; fine to lwr med grained	
SS 80			23-30-43	163.3			CLAYEY SAND (SC); med yellowish brown and lt greenish gray with brownish orange mottles; v dense; moist; subangular; poorly graded; fine to lwr med grained	
SS 81			26-27-31	161.8			POORLY GRADED SAND WITH CLAY (SP-SC); med yellowish brown with some lt yellowish gray zones; v dense; wet; subangular; poorly graded; fine to lwr med grained	
SS 82			13-32-50	160.3	130		CLAYEY SAND (SC); med yellowish brown with some lt greenish gray zones; v dense; wet; subangular; poorly graded; v fine to fine grained	
SS 83			30-32-42	158.8			same as above; with some lt gray zones	
SS 84			19-22-18	157.3			POORLY GRADED SAND WITH SILT (SP-SM); lt greenish brown and lt grayish brown; dense; wet; subangular; poorly graded; v fine to fine grained	
SS 85			9-11-23	155.8	135		SILTY SAND (SM); med yellowish brown and whitish brown; dense; wet; subangular; poorly graded; v fine to fine grained	
SS 86			26-35-47	154.1			same as above; med brown to orangish brown; v dense; v fine to lwr med grained	
SS 87			37-37-42	152.6			same as above; med yellowish brown with some lt gray zones; v fine to fine grained	
SS 88			26-23-30	151.1			POORLY GRADED SAND (SP) trace silt; lt gray clayey sand layer at 140 ft; med yellowish brown; v dense; wet; subangular; poorly graded; fine to lwr med grained	
SS 89			16-23-29	149.6	140		CLAYEY SAND (SC); med yellowish brown; v dense; wet; subangular; poorly graded; v fine to fine grained	
SS 90			19-20-30	148.1			POORLY GRADED SAND WITH SILT (SP-SM) some zones have clay; lt reddish brown with white bands; dense; wet; subangular; poorly graded; fine grained; trace mica; trace heavies	
SS 91			16-18-22	146.6			same as above; lt reddish brown with lt brownish yellow zones; med dense	
SS 92			13-39-43	145.1	145		same as above; some zones are silty; lt reddish brown with lt brown zones; v dense	
SS 93			18-13-25	143.6			CLAYEY SAND (SC) some zones have less clay; lt reddish brown; dense; wet; subangular; poorly graded; fine grained; trace mica; trace heavies	
SS 94			15-21-33	142.1			POORLY GRADED SAND (SP) trace silt; lt yellowish brown; v dense; wet; subangular; poorly graded; fine grained; trace mica; trace heavies	
SS 95			12-8-14	140.6	150		CLAYEY SAND (SC); med brownish red with v lt green wisps; med dense; wet; subangular; poorly graded; fine to lwr med grained; trace mica; trace heavies	
SS 96			6-9-11	139.1			same as above; med brownish yellow with med reddish brown zones and v lt green clay wisps	
SS 97			7-9-13	137.6			same as above; clayey at base of sample; med reddish brown with v lt green wisps; occasional rounded cse grains	
SS			10-16-19	136.1			LEAN CLAY WITH SAND (CL) with interbedded clay	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE		FINAL LOG		
						HOLE NO. FB-18		





GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				Northeast Expansion		NEX	1 OF 4	FB-19			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF-NE			N 79781 E 54979			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
4/27/98	4/29/98	M Rizer/Graves	Failing 1500		3 7/8 in	140 lb/30 in		144.5			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
273.6		▽ / ▽ /	N. Kidd/SAIC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						273.6					
								5			
SS 1		▲			○	6-10-14	265.1			CLAYEY SAND (SC); light reddish yellow; medium dense; damp; subangular; poorly graded; fine to medium grained	
SS 2		▲			○	7-8-13	263.6			same as above; light yellow, light red, and light orange	
SS 3			▲		○	11-16-25	262.1	10		same as above; medium yellowish red with light yellow and white mottles; dense	
SS 4				▲	○	15-31-29	260.6			same as above; medium yellowish red with light reddish purple bands; very dense	
SS 5				▲	○	17-27-25	259.1			same as above; medium yellow with light to medium red zones and light gray mottles	
SS 6		▲			○	12-16-17	257.6	15		same as above; light brownish yellow with white wisps; dense	
SS 7		▲			○	7-14-23	256.1			same as above; with light red	
SS 8		▲			○	11-13-16	254.6			same as above; medium dense; moist	
SS 9		▲			○	11-14-19	253.1	20		same as above; with zones of sand with clay; light brownish yellow; dense	
SS 10		▲			○	9-14-15	248.1	25		POORLY GRADED SAND WITH CLAY (SP-SC) some areas are clayey; light brownish yellow with light brown bands and white wisps; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 11		▲			○	7-11-12	243.1	30		same as above; light brownish yellow with white wisps and light reddish bands; trace mica; some zones are wet	
SS		▲			○	7-10-14				same as above; with two bands of clayey sand; various	

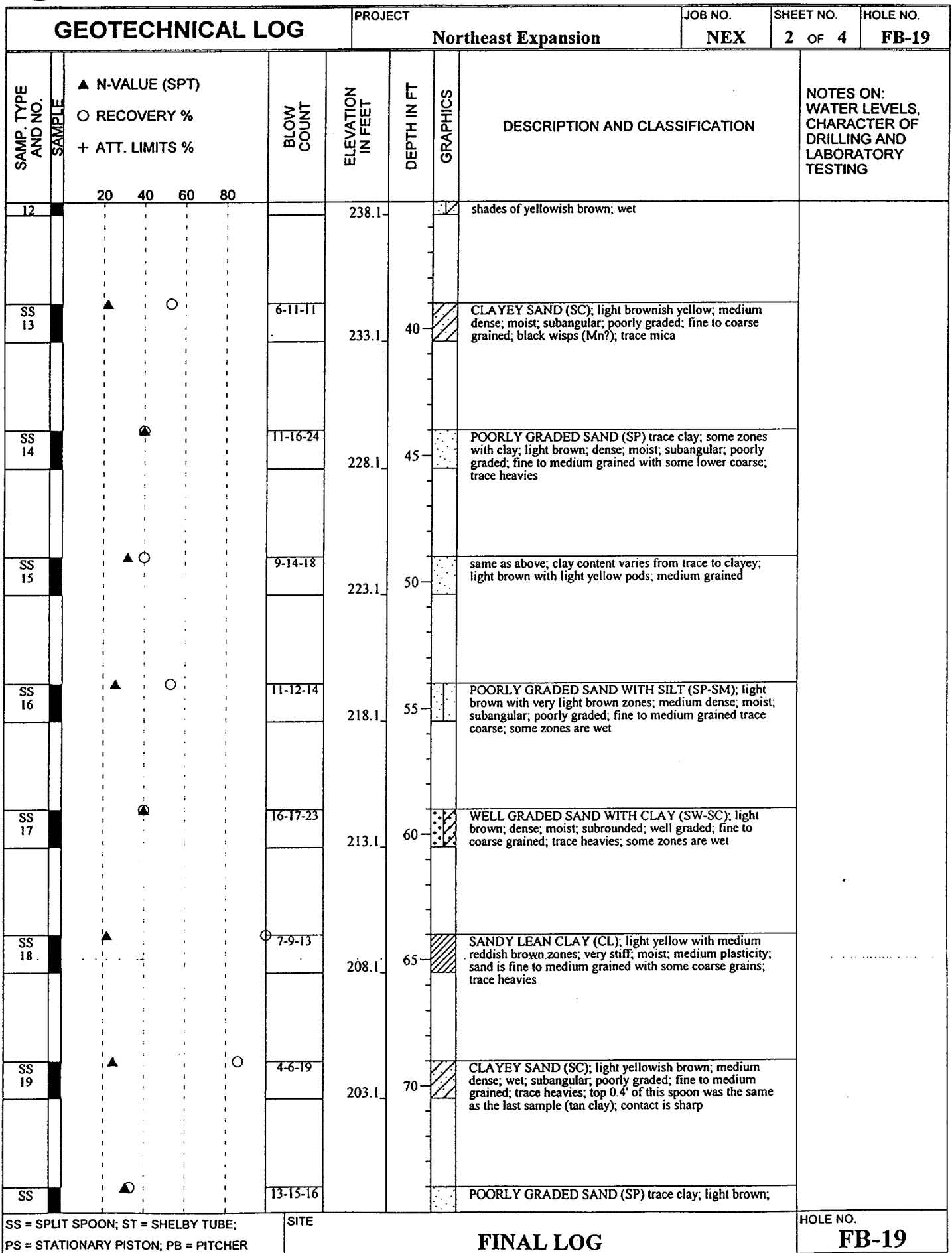
SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

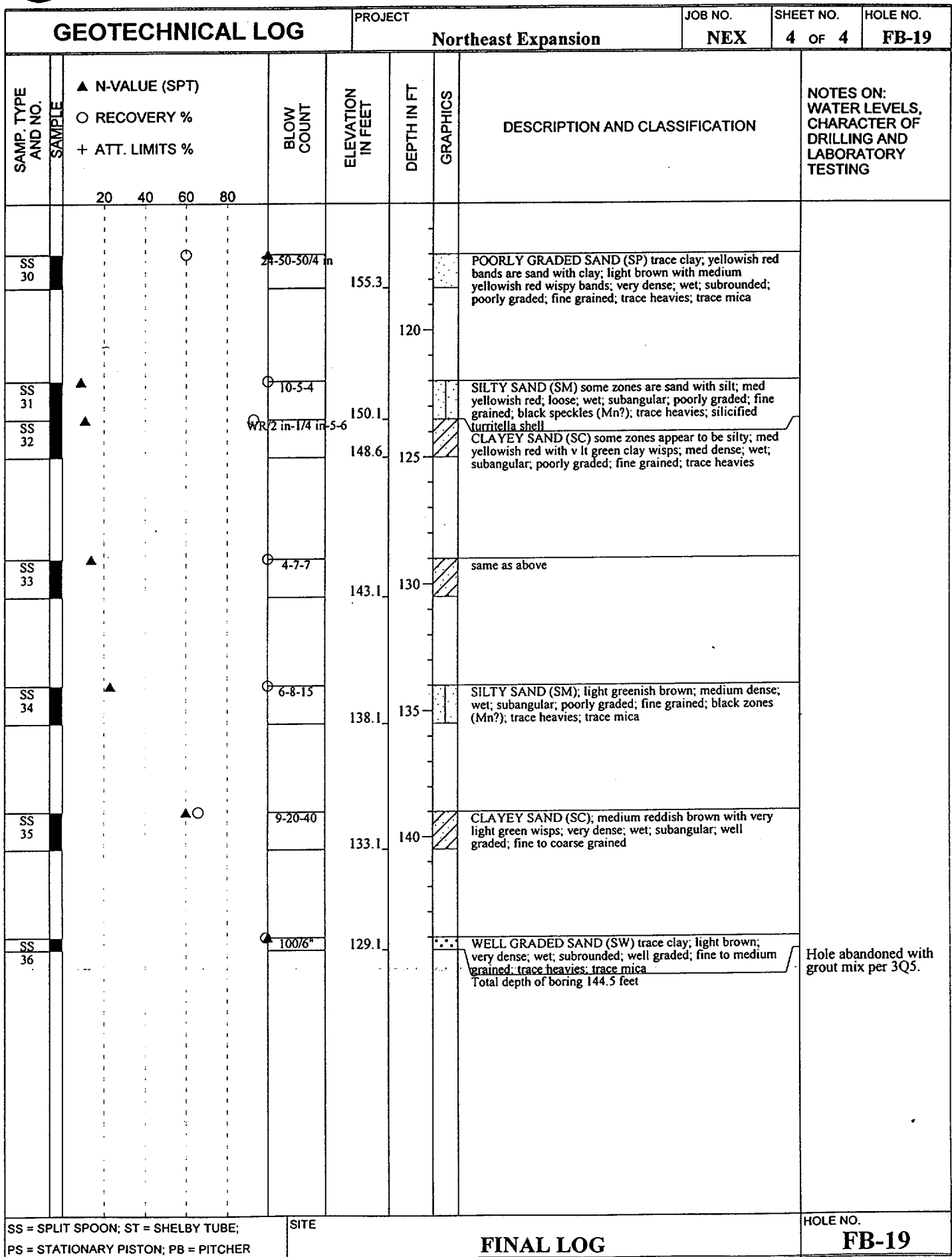
HOLE NO.

FB-19





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 4	FB-19
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
20			198.1			dense; wet; subangular; poorly graded; fine to medium grained; trace heavies	
SS 21	▲	5-6-14	193.1	80		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light yellowish brown with white wisps; medium dense; wet; subangular; poorly graded; fine to medium grained; black zones (Mn?)	
SS 22	▲	4-10 in-1/2 in-2	188.1	85		same as above; white zones are clayey sand; light brownish yellow with light yellow and white zones; very loose; trace heavies	
SS 23	▲	WR/2 in-3/6 in-1/4 in-3	186.6			CLAYEY SAND (SC); light yellow with very light brown zones; very loose; wet; subangular; poorly graded; fine to medium grained; trace heavies	
SS 24	▲	3-4-13	185.1			same as above; interbedded sand with clay and sandy lean clay; light yellow; medium dense; fine to coarse grained; black wisps (Mn?)	
SS 25	○	18-42-50	180.1			POORLY GRADED SAND (SP) trace clay; light brown with a light reddish brown wisp; very dense; wet; subangular; poorly graded; fine to medium grained; trace heavies	
SS 26	▲	9-9-11	175.1			CLAYEY SAND (SC); light reddish brown with very light brown zones; medium dense; wet; subangular; poorly graded; fine to medium grained; trace heavies	
SS 27	▲	10-13-25	170.1			same as above; light reddish brown with very light greenish yellow clay wisps; dense; black pellets (Mn?)	
SS 28	○	9-20-22	165.1			POORLY GRADED SAND (SP) trace clay; yellow zones contain more clay; light brown with light yellow wisps; dense; wet; subrounded; poorly graded; fine to lower medium grained; trace heavies	
SS 29	○	11-9-27	160.1			POORLY GRADED SAND WITH CLAY (SP-SC) intermixed with clayey sand; medium yellowish red with light brown zones and light yellow clay wisps; dense; wet; subangular; poorly graded; fine grained; trace heavies	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				HOLE NO.
			FINAL LOG				FB-19





GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion		NEX	1 OF 3	FB-20
SITE			COORDINATES			ANGLE FROM HORIZONTAL		
APSF-NE			N 78642 E 55518			90		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH
4/29/98	5/13/98	J Corbitt/Graves	Failing 1500		3 7/8 in	140 lb/30 in		112.7
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:					
301.4		▽ /	C. Rothammer/WSRC					
SAMP. TYPE AND NO.	GRAPHICS	DEPTH IN FT	ELEVATION IN FEET	BLOW COUNT	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING		
			301.4					
		5						
SS 1			293.9	9-11-14	SILTY SAND (SM); with clay; medium reddish brown; medium dense; wet; subangular; poorly graded; fine to medium grained, lithic fragments			
SS 2			292.4	7-10-13	CLAYEY SAND (SC); dark yellowish brown and red with tan; medium dense; moist; subangular; poorly graded; fine to medium grained			
SS 3			290.9	9-21-31	CLAYEY SAND WITH GRAVEL (SC); very dense; same as above			
SS 4		10	289.4	18-31-40	CLAYEY SAND (SC); dark brownish yellow with tan; very dense; moist; subangular; poorly graded; fine to medium grained, lithic fragments			
SS 5			287.9	24-17-14	CLAYEY SAND WITH GRAVEL (SC); medium purple, orange, and yellow; dense; moist; subangular; well graded; fine to medium grained			
SS 6			286.4	6-12-16	SANDY FAT CLAY (CH); medium purple with white; very stiff; moist; high plasticity			
SS 7		15	284.9	6-11-19	CLAYEY SAND WITH GRAVEL (SC); medium white, yellow, orange, and purple; medium dense; wet; subangular; well graded; fine to coarse grained			
SS 8			283.4	10-14-30	SILTY SAND (SM) with clay; medium reddish brown; dense; wet; subangular; poorly graded; fine to medium sand			
SS 9			281.9	10-15-17	same as above; reddish brown with yellow			
SS 10		20	280.4	7-12-17	same as above; medium dense			
SS 11			278.9	12-23-20	same as above; dense			
SS 12			277.4	12-18-17	POORLY GRADED SAND WITH SILT (SP-SM); medium brownish red; dense; wet; subangular; poorly graded; fine to medium grained			
SS 13		25	275.9	11-15-22	CLAYEY SAND (SC); medium brownish red with white; dense; wet; subangular; poorly graded; fine to medium grained			
SS 14			274.4	15-19-21	POORLY GRADED SAND WITH SILT (SP-SM); medium brownish red, yellow, and orange; dense; wet; subangular; poorly graded; fine to medium grained			
SS 15			272.9	12-14-18	SILTY SAND (SM); medium brownish red, white, and yellow; banded; dense; moist; subangular; poorly graded; fine to very fine grained			
SS 16			271.4	10-18-21	same as above; light reddish brown and red			
SS 17		30	269.9	17-19-21	same as above; light brown with yellow and orange; subrounded; well graded; fine to medium grained			
SS 18			268.4	11-14-16	same as above; with clay; medium red, white, yellow, purple, and orange; some mottling; medium dense; subangular; poorly graded			
SS 19			266.9	10-17-21	same as above; dense			

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-20



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	2 OF 3	FB-20
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 20		16-18-28	265.4			same as above; dark brownish red with white and yellow	
SS 21		10-16-18	263.9			same as above; with clay; dark red with yellow	
SS 22		14-14-19	262.4			same as above; dark red, white, and yellow	
SS 23		9-19-22	260.9	40		same as above	
SS 24		15-16-16	259.4			same as above; medium brownish yellow with red and white	
SS 25		12-19-25	257.9			CLAYEY SAND (SC); medium brownish yellow with white; dense; moist; subangular; poorly graded; fine to medium grained	
SS 26		9-16-19	256.4	45		SILTY SAND (SM) with clay; medium dark red, white, and yellow; mottled; dense; moist; subangular; poorly graded; fine to medium grained	
SS 27		14-15-26	254.9			CLAYEY SAND (SC); medium dark red, white, and yellow; mottled; dense; moist; subangular; poorly graded; fine to medium grained	
SS 28		20-22-26	253.4			same as above; dark red; wet; trace coarse sand	
SS 29		19-30-26	251.9	50		SILTY SAND (SM); dark red, white, yellow, and purple; very dense; wet; subangular; poorly graded; fine to medium grained; trace coarse sand	
SS 30		12-20-27	250.4			POORLY GRADED GRAVEL WITH SILT (GP-GM); medium red, yellow, and tan; dense; wet; subangular; poorly graded; fine to coarse grained	
SS 31		6-15-25	248.9			POORLY GRADED SAND WITH SILT (SP-SM); medium reddish brown with yellow; dense; wet; subrounded; poorly graded; fine to medium grained, trace gravel	
SS 32		10-10-6	247.4			CLAYEY SAND (SC); medium brownish white and yellow; mottled; medium dense; moist; subangular; well graded; fine to coarse grained	
SS 33		8-10-10	245.9	55		SILTY SAND (SM) with clay; medium brownish red; medium dense; moist; subangular; well graded; fine to medium grained	
SS 34		6-10-12	244.4			same as above; medium brownish yellow	
SS 35		16-20-23	242.9			WELL GRADED SAND WITH SILT (SW-SM); medium orange; dense; wet; subrounded; well graded; fine to coarse grained	
SS 36		6-6-8	241.4	60		CLAYEY SAND (SC); medium brownish orange with white; medium dense; wet; subangular; well graded; fine to coarse grained	
SS 37		3-8-13	239.9			SILTY SAND (SM) with clay; medium brownish yellow; medium dense; wet; subangular; well graded; fine to coarse grained	
SS 38		16-20-18	238.4			POORLY GRADED SAND WITH SILT (SP-SM); medium brownish orange; dense; wet; angular; poorly graded; fine to coarse grained	
SS 39		19-21-26	236.9			no recovery	Catcher broken
SS 40		12-20-22	235.4	65		SILTY SAND (SM); medium orange; dense; moist; subangular; poorly graded; fine to medium grained	
SS 41		10-11-24	233.9			CLAYEY SAND (SC); medium brownish orange; dense; moist; subangular; poorly graded; fine to medium grained	
SS 42		17-28-34	232.4			SILTY SAND (SM) with clay; medium brownish orange with pink and white; very dense; wet; subangular; well graded; fine to coarse grained; lenses of moist clayey sand	
SS 43		10-26-28	230.9	70		POORLY GRADED SAND WITH SILT (SP-SM); medium brownish orange with medium gray; very dense; wet; subangular; poorly graded; medium to coarse grained	
SS 44		10-11-21	229.4			SILTY SAND (SM); medium brownish orange; dense; wet; subangular; poorly graded; fine to medium grained	
SS 45		21-24-12	227.9			no recovery	
SS 46		13-13-13	226.4			CLAYEY SAND (SC); medium brownish orange; medium dense; moist; subangular; well graded; fine to coarse	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-20



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 3	FB-20
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 47		6-10-10				grained, trace gravel	
SS 48		12-21-22	224.9			same as above; with silt; medium brownish yellow with black; wet; lithic fragment; Mn present; lenses of fat clay	
SS 49		7-10-15	223.4			SANDY LEAN CLAY (CL); medium brownish yellow with white, black, and brown; mottled; hard; moist; medium plasticity; Mn present	
SS 50		19-16-10	221.9			same as above; with orange and tan zones; very stiff; lenses of fat clay	
SS 51		10-17-21	220.4	80		FAT CLAY (CH); light yellowish brown with black streaks; very stiff; moist; high plasticity	
SS 52		13-19-22	218.9			CLAYEY SAND (SC); medium yellowish brown with black, white, and brown; mottled; dense; moist; subangular; poorly graded; fine to medium grained, Mn present, clay lenses	
SS 53		17-19-22	217.4			same as above; with silt; wet	
SS 54		10-15-20	215.9	85		SILTY SAND (SM) with clay; medium yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 55		18-7-10	214.4			CLAYEY SAND (SC); medium yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 56		14-24-36	212.9			POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish orange; medium dense; wet; subangular; poorly graded; fine to medium grained, trace gravel	
SS 57		29-15-14	211.4	90		WELL GRADED SAND WITH CLAY (SW-SC); medium brownish orange; very dense; wet; subangular; well graded; fine to coarse grained	
SS 58		10-19-23	209.9			POORLY GRADED SAND WITH SILT (SP-SM); light brown with tan; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 59		10-16-15	208.4			POORLY GRADED SAND (SP) trace silt; light brown; dense; moist; subangular; poorly graded; fine to medium grained, trace coarse sand	
SS 60		19-25-31	206.9	95		SILTY SAND (SM); brownish orange; dense; moist; subangular; poorly graded; fine to medium grained	
SS 61		16-18-21	205.4			same as above (top 0.6 ft.); remaining portion is yellowish brown fat clay with sand (CH), hard, moist, high plasticity	
SS 62		8-13-25	203.9			same as above; medium brownish orange with black; wet; Mn present	
SS 63		4-9-19	202.4	100		CLAYEY SAND WITH GRAVEL (SC); medium brown; dense; moist; subangular; well graded; fine to very coarse grained	
SS 64		15-16-19	200.8			CLAYEY SAND (SC) with 0.4 ft thick grayish brown lean clay layer at 100.0 ft; medium yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	Loas circulation (5/6/98).
SS 65		17-18-19	199.3			same as above; medium yellowish brown to grayish brown; dense; some Mn staining	
SS 66		12-12-15	197.8			POORLY GRADED SAND (SP) trace clay; medium grayish brown; dense; wet; subangular; poorly graded; fine to medium grained	Lost circulation on 5/12/98 while attempting to redrill to 180 feet for geophysical logging.
SS 67		9-14-15	196.3	105		same as above; trace silt; medium dense; with black Mn staining	
SS 68		10-9-9	194.8			POORLY GRADED SAND WITH SILT (SP-SM); medium grayish brown; medium dense; wet; subangular; poorly graded; fine to medium grained; trace Mn staining	
SS 69		1-2-1	193.3			POORLY GRADED SAND WITH CLAY (SP-SC); medium grayish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 70		WR37 in	191.8	110		LEAN CLAY WITH SAND (CL); light grayish white, light brown, and black; soft; wet; medium plasticity; sand fraction is very fine to fine grained	Lost circulation and could not regain. Pumped two mud tubs into hole with 500 gallons water will take one more spoon (5/7/98).
			188.7			CLAYEY SAND (SC); medium grayish brown; very loose; wet; subangular; poorly graded; fine to medium grained	Hole abandoned with grout mix per 3Q5.
						Total depth of boring 112.7 feet.	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-20



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
APSF-NE				Northeast Expansion		NEX	1 OF 3	FB-20A
SITE		COORDINATES				ANGLE FROM HORIZONTAL		
		N 78642 E 55518				90		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL	HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL	TOTAL DEPTH		
4/29/98	5/13/98	J Corbitt/Graves	Failing 1500	3 7/8 in	140 lb/30 in	120.0		
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:				
301.4		7 / 7 /		C. Rothhammer/WSRC				
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
	20 40 60 80		301.4					Hand auger to 6 feet to check for underground interferences. Then switch to mud rotary. Straight drill to 106.5 feet to begin sampling.
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE				HOLE NO.
				FINAL LOG				FB-20A



GEOTECHNICAL LOG					PROJECT Northeast Expansion		JOB NO. NEX	SHEET NO. 2 OF 3	HOLE NO. FB-20A		
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
								45			
								50			
								55			
								60			
								65			
								70			
								75			
								80			
								85			

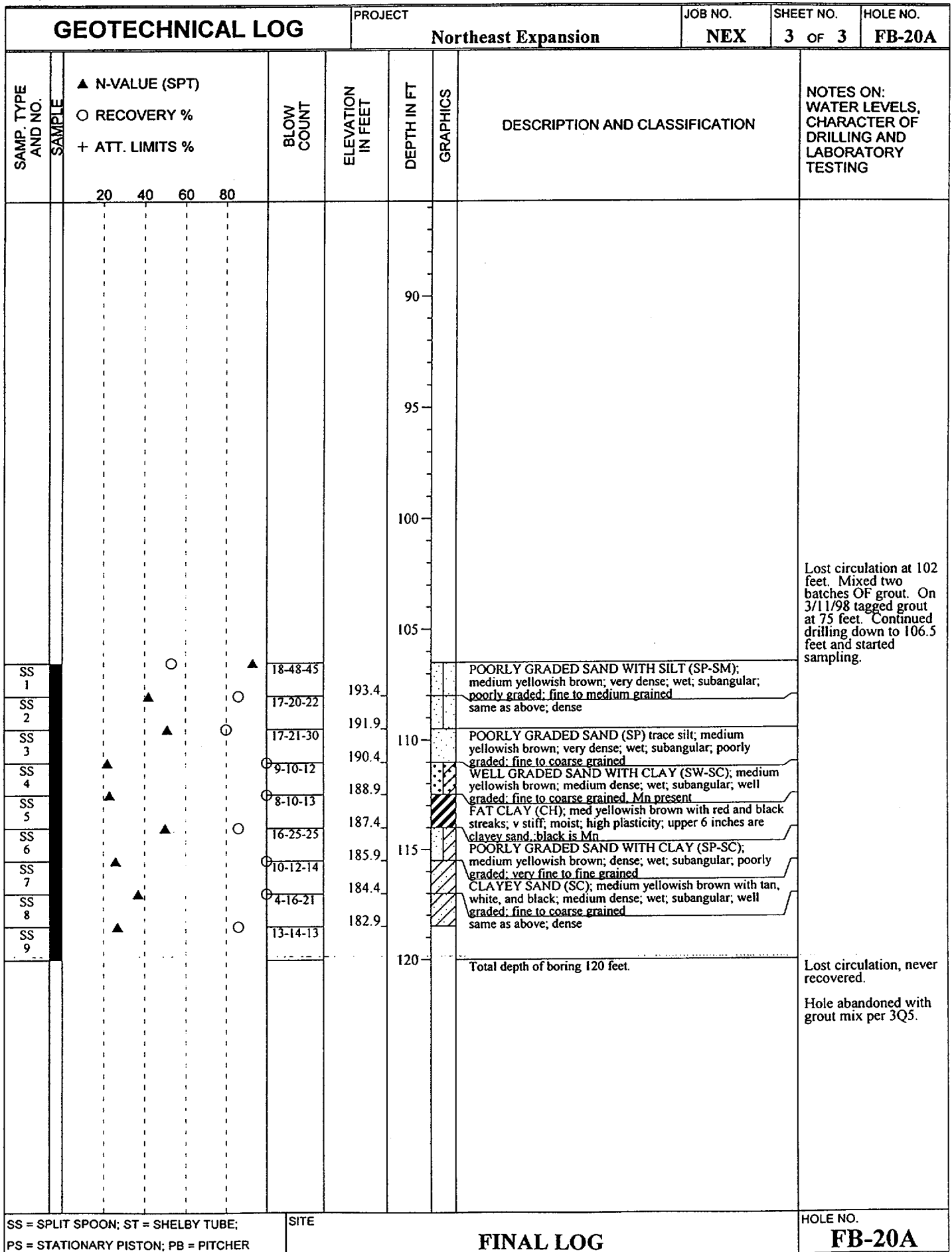
SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

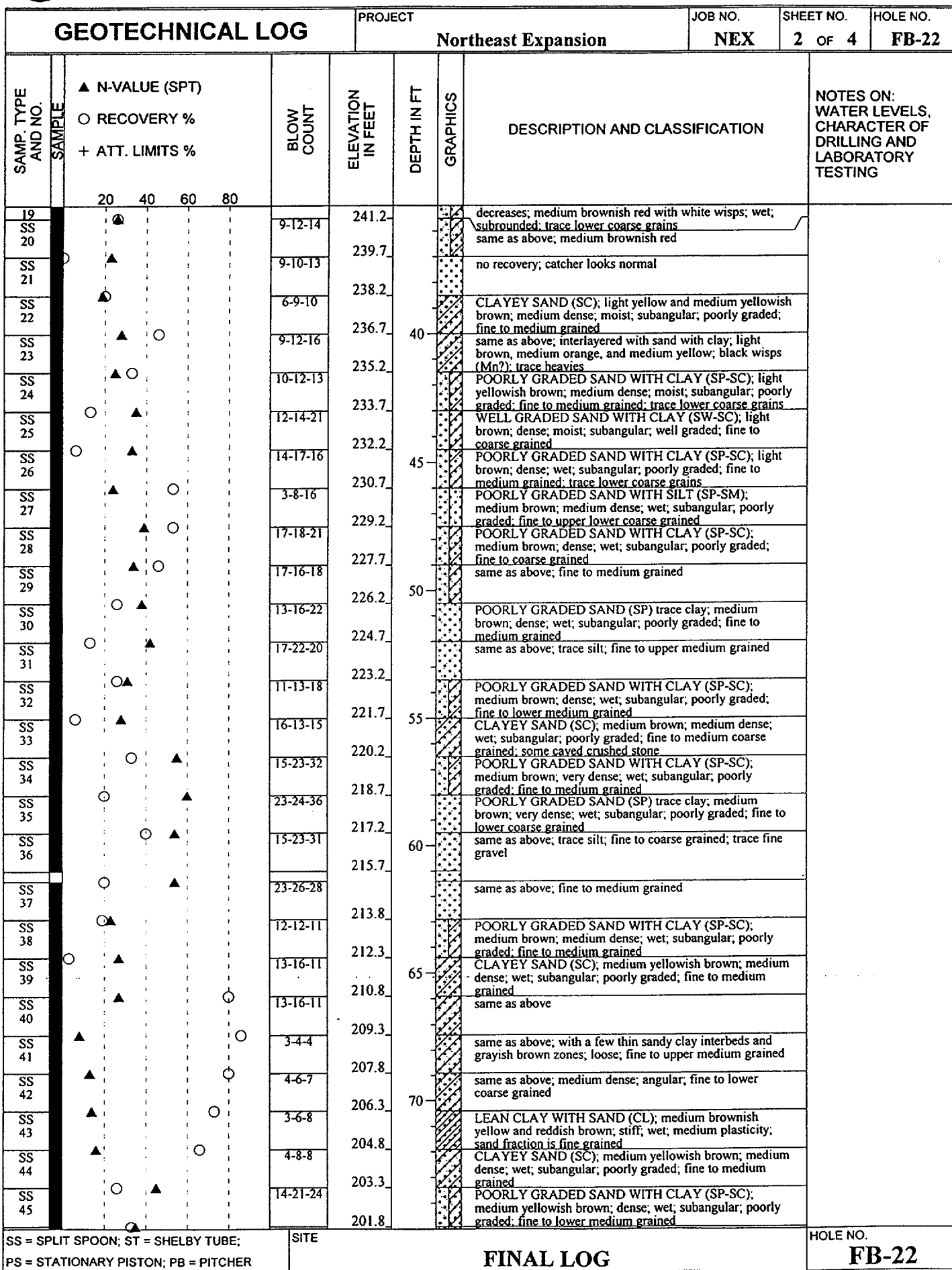
HOLE NO.
FB-20A

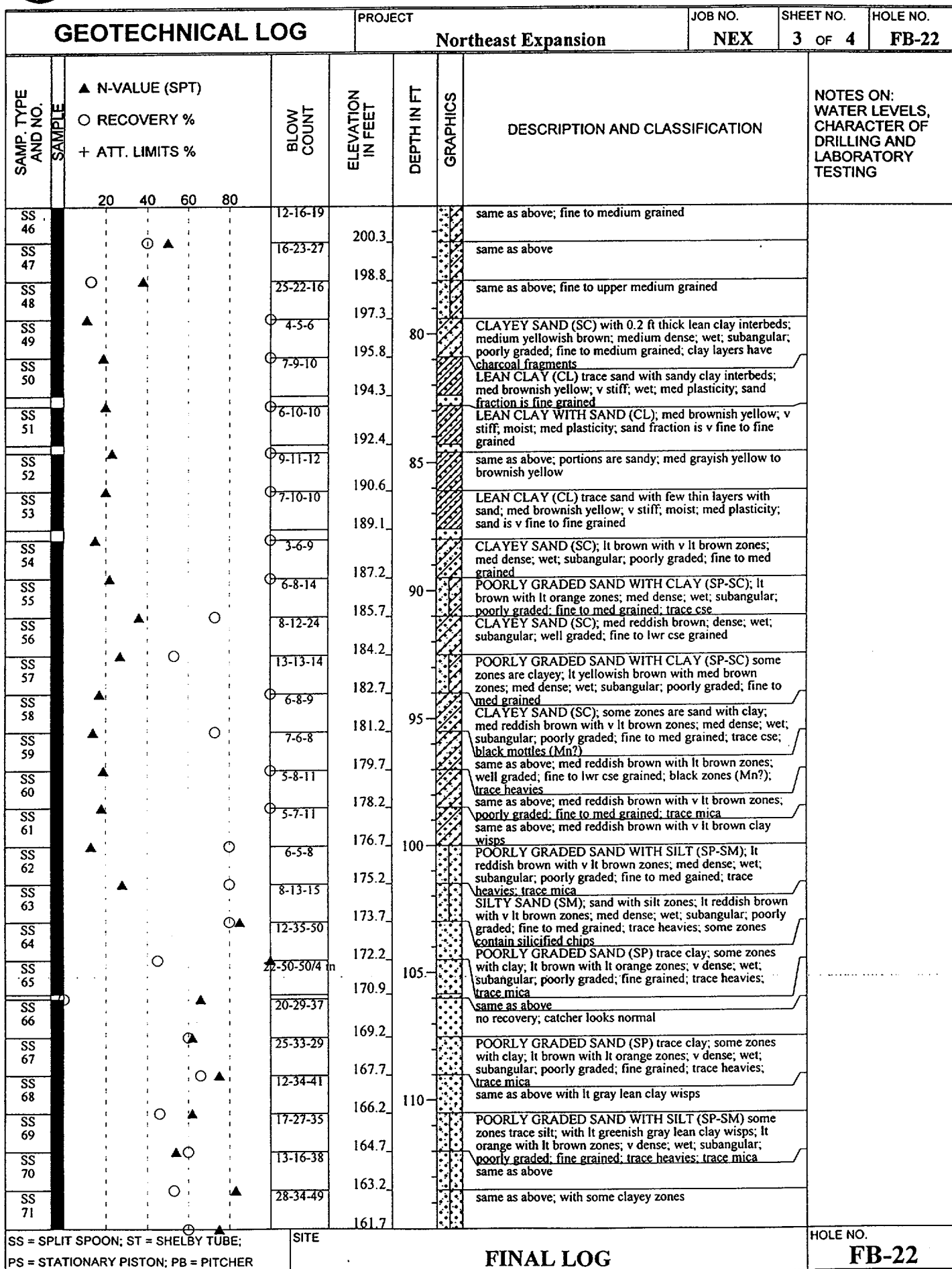
Adding quick gel.

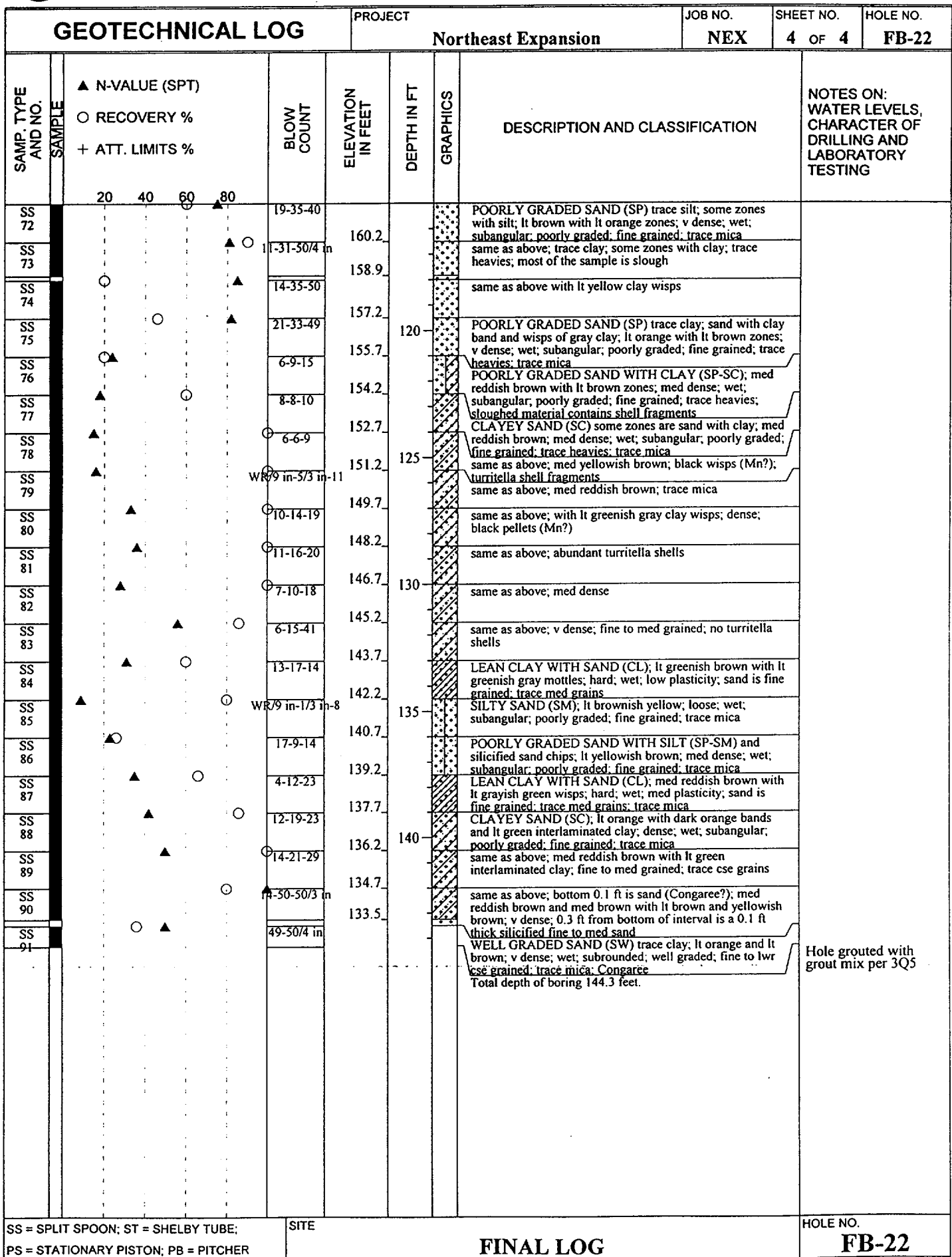




GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
SITE				COORDINATES		ANGLE FROM HORIZONTAL					
APSF-NE				N 79779 E 54828		90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
5/1/98	5/7/98	M Rizer/Graves	Failing 1500		3 7/8 in	140 lb/30 in		144.3			
GROUND EL.		DEPTH/EL. GROUND WATER		LOGGED BY:							
276.7		▽ / ▽ /		N. Kidd/SAIC							
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						276.7					
								5			
SS 1		▲		○		6-7-12	268.2			POORLY GRADED SAND (SP) trace clay; light brown with dark reddish bands; medium dense; moist; subangular; poorly graded; fine to medium grained; trace heavies; probable fill	
SS 2		▲				7-14-14	266.7	10		no recovery	
SS 3		▲		○		6-7-8	265.2			CLAYEY SAND (SC); light yellow with dark red zones; medium dense; damp; subangular; poorly graded; fine to medium grained	
SS 4		▲		○		8-17-23	263.7			LEAN CLAY (CL) with interlayered silt and very fine grained sand zones; medium red, white, and dark yellowish orange; hard; damp; medium plasticity; sand is very fine grained; abundant mica	
SS 5		▲		○		9-14-16	262.4			POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light reddish yellow grading downward to light red; medium dense; damp; subangular; poorly graded; fine to medium grained	
SS 6		▲		○		10-20-22	260.7	15		same as above; light yellowish brown with white wisps; dense; dry	
SS 7		▲		○		12-15-20	259.2			same as above; light brownish yellow with white wisps; trace mica	
SS 8		▲		○		6-11-14	257.7			CLAYEY SAND (SC); light brownish yellow with white wisps; medium dense; dry; subangular; poorly graded; fine to medium grained; trace mica	
SS 9		▲		○		8-11-12	256.2	20		same as above	
SS 10		▲		○		8-12-13	254.7			same as above; light yellow with light red and white bands; damp; trace mica	
SS 11		▲		○		14-19-20	253.2			no recovery	
SS 12		▲		○		14-19-19	251.7			no recovery	
SS 13		▲		○		12-17-17	250.2	25		CLAYEY SAND (SC); light brownish yellow with white wisps; dense; damp; subangular; poorly graded; fine to medium grained; bottom of the interval is less clayey	
SS 14		▲		○		11-14-15	248.7			POORLY GRADED SAND WITH CLAY (SP-SC) some bands of clayey sand; light brownish yellow with white wisps; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 15		▲		○		10-12-13	247.2			CLAYEY SAND (SC); light brownish yellow with white wisps; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 16		▲		○		10-11-14	245.7	30		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light yellowish brown with light red bands and white wisps; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 17		▲		○		13-15-16	244.2			same as above; light brownish yellow with white wisps; dense	
SS 18		▲		○		12-12-14	242.7			same as above; light yellowish brown with white wisps and dark orange bands; medium dense; trace mica	
SS		▲		○		9-10-12				same as above; clay content increases with depth; grain size	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE		FINAL LOG			HOLE NO. FB-22









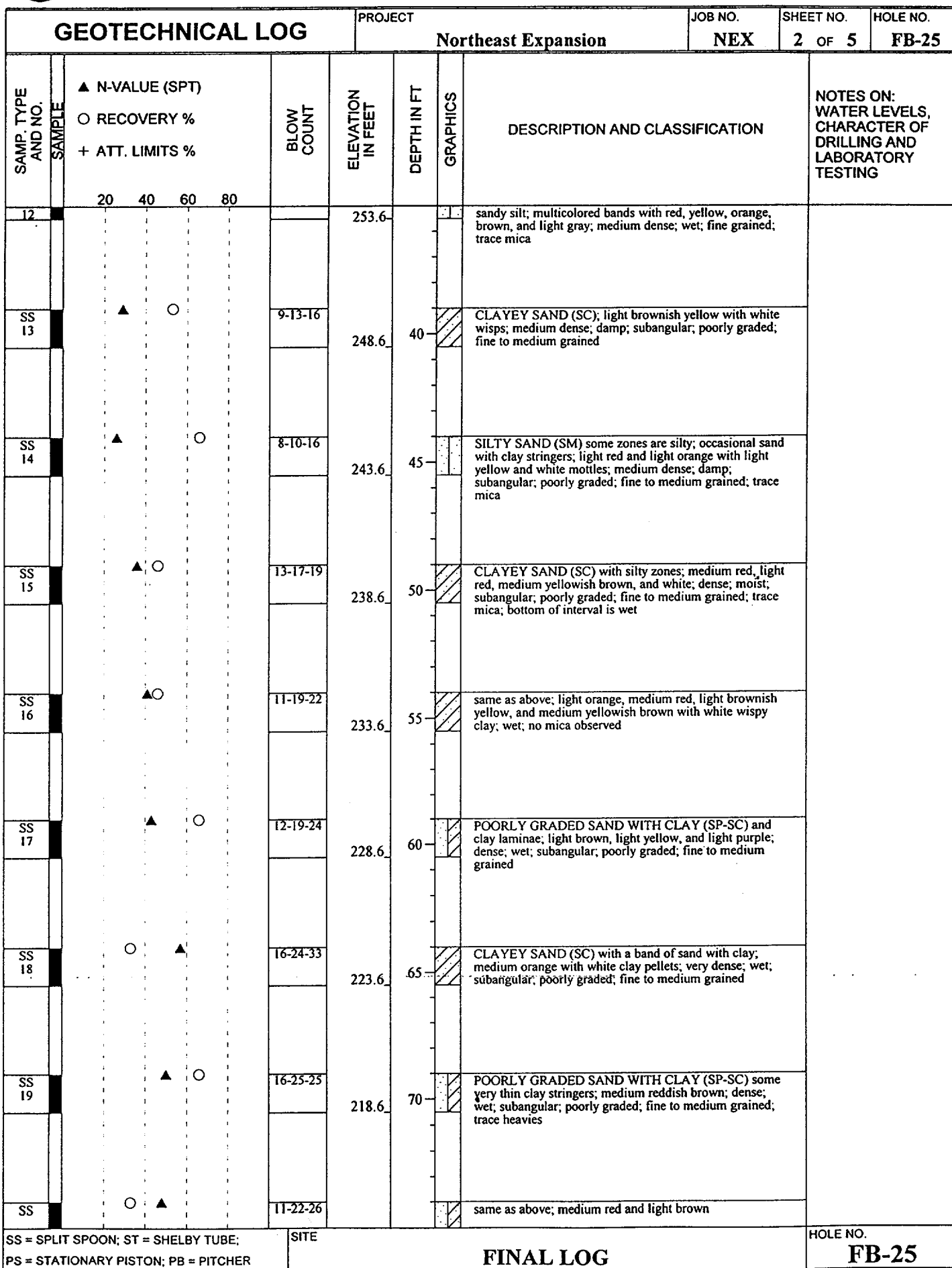
GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion		NEX	1 OF 5	FB-25
SITE			COORDINATES			ANGLE FROM HORIZONTAL		
APSF-NE			N 79242 E 55261			90		
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH
5/26/98	5/28/98	J Corbitt/Graves	Failing 1500		3 7/8 in	140 lb/30 in		165.5
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:					
289.1		▽ /	N. Kidd/SAIC					
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
				289.1				
					5			
SS 1			9-20-34	280.6			CLAYEY SAND (SC); light red and light orange with white mottles; very dense; damp; subangular; poorly graded; <u>fine to medium grained</u>	Hand auger to 7 feet to check for interferences.
SS 2			15-17-21				same as above; light red; dense	
SS 3			11-16-20	279.1	10		POORLY GRADED SAND WITH CLAY (SP-SC); light red and light orange with a white, very clean band of sand; dense; damp; subangular; poorly graded; fine to medium grained; <u>trace heavies</u>	
SS 4			10-16-16	277.6			CLAYEY SAND (SC); with sand with clay zones; light red with very light yellow band; dense; damp; subangular; poorly graded; <u>fine to medium grained</u>	
SS 5			8-10-10	276.1			same as above; some zones are silty; light red; medium dense; <u>fine grained; trace medium; trace mica</u>	
SS 6			5-7-10	274.6	15		POORLY GRADED SAND WITH CLAY (SP-SC); light red with dark brown mottles; medium dense; moist; subangular; poorly graded; <u>fine to medium grained</u>	
SS 7			6-11-17	273.1			CLAYEY SAND (SC) trace gravel; light orange and red with light brownish yellow zones and white wispy clays; medium dense; damp; subangular; poorly graded; medium to coarse grained; <u>trace gravel; silicified chips</u>	
SS 8			10-15-15	271.6			SILTY SAND (SM) with interlaminated clayey sand; light red, light yellow, and light purplish gray; medium dense; moist; subangular; poorly graded; fine to medium grained; <u>trace mica</u>	
SS 9			9-10-15	270.1			same as above; light red with very light pinkish brown zones	
				268.6	20			
SS 10			17-19-23	263.6	25		CLAYEY SAND (SC) with silty sand laminae; light red and light brownish orange; dense; moist; subangular; poorly graded; fine to medium grained; <u>trace mica</u>	
SS 11			12-18-18	258.6	30		SILTY SAND (SM) with clayey sand zones and a layer of light red lean clay with sand; light red, light purple, light grayish yellow, and light orange with light purple clay wisps; dense; damp; subangular; poorly graded; fine to medium grained	
SS			7-7-9				same as above; some zones are clayey; some zones are	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-25





GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 5	FB-25
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
20	20 40 60 80		213.6				
SS 21	▲ 40 ○ 60	16-21-16	208.6	80		WELL GRADED SAND WITH CLAY (SW-SC) some zones are clayey; light brown and light brownish yellow with white clay wisps; dense; wet; subangular; well graded; fine to medium grained; trace heavies	
SS 22	▲ 40 ○ 60	6-12-14	203.6	85		CLAYEY SAND (SC) with light brownish yellow lean clay stringers; medium orange, light brown, and dark yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained; trace coarse grains; trace heavies	
SS 23	▲ 40 ○ 60	7-6-11	198.6	90		same as above; with lean clay laminae; medium brownish yellow with light brownish yellow lean clay wisps; well graded; fine to coarse grained	
SS 24	○ 60 ▲ 80	41-41-45	193.6	95		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are borderline clayey sand; light red and light brown with a white band; very dense; wet; subangular; poorly graded; fine to medium grained	
SS 25	○ 60 ▲ 80	30-47-49	188.6	100		same as above; light red and light brown with light yellow bands	
SS 26	▲ 40 ○ 60	18-28-29	183.6	105		CLAYEY SAND (SC); light brownish yellow with light brown zones; very dense; wet; subangular; well graded; fine to coarse grained	
SS 27	○ 60 ▲ 80	24-47-50	178.6	110		WELL GRADED SAND (SW) trace clay; light brown; very dense; wet; subrounded; well graded; fine to coarse grained; trace heavies	
SS	○ 60 ▲ 80	32-49-50				same as above	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				HOLE NO.
			FINAL LOG				FB-25



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	4 OF 5	FB-25
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
28			173.6				
SS 29	▲	8-15-27	168.6	120		CLAYEY SAND (SC); dark brown with medium yellow zones; dense; wet; subangular; poorly graded; fine to medium grained; lignite present; trace heavies	
SS 30	▲ ○	11-28-50	163.6	125		POORLY GRADED SAND WITH CLAY (SP-SC) borderline clayey sand; light to dark brown and medium orange; very dense; wet; subangular; poorly graded; fine to lower medium grained; lignite present; trace heavies; trace mica	
SS 31	○ ▲	50/4 in	159.8	130		same as above (slough?); not enough sample to jar; light brown and medium orange	
SS 32	▲	24-18-22	153.6	135		CLAYEY SAND (SC) with zones of sand with clay; light brownish yellow and light orange with light green wisps of clay; dense; wet; subangular; poorly graded; fine to lower medium grained; trace heavies	
SS 33	▲	6-7-13	148.6	140		SILTY SAND (SM) some zones are clayey; medium orange with light green wisps; dense; wet; subangular; poorly graded; fine grained; trace heavies; trace mica	
SS 34	▲	8-10-13	143.6	145		CLAYEY SAND (SC); medium orange with light green wispy clays; medium dense; wet; subangular; poorly graded; fine grained	
SS 35	▲	10-30-50/3 in	138.9	150		same as above; light yellowish brown and light brown with light green wispy clays; very dense; very fine grained; trace mica; trace black staining (Mn?)	
SS	▲	8-16-19				LEAN CLAY (CL) with interbedded sand; light green with	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-25



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion		NEX	5 OF 5	FB-25
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
36			133.6			medium orange sand stringers; hard; wet; medium plasticity; sand is fine grained; black mottling (Mn?)		Hole abandoned with grout mix per 3Q5.
SS 37	▲	50/5 in	129.7	160		CLAYEY SAND (SC) with clean sand laminae; medium orange and medium yellow with light brown wisps; very dense; wet; subangular; well graded; fine to coarse grained; clayey slough at top of recovered interval; probable Congaree		
SS 38	○	37-41-41	123.6	165		POORLY GRADED SAND WITH CLAY (SP-SC); medium brown with dark brown wisps; very dense; wet; subangular; poorly graded; fine to medium grained; trace heavies; trace mica; Congaree Total depth of boring 165.5 feet.		

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-25



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
				Northeast Expansion		NEX	1 OF 5	FB-27			
SITE			COORDINATES			ANGLE FROM HORIZONTAL					
APSF-NE			N 78900 E 56660			90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
4/28/98	4/29/98	J Corbitt/Graves	Failing 1500		3 7/8 in	140 lb/30 in		155.5			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
297.6		▽ / ▽ /	B. Gelinas/SAIC; C. Rothammer/WSRC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20	40	60	80						
						297.6					Hand auger to 6 feet to check for interferences.
SS 1						15-25-26	290.1	5		POORLY GRADED SAND WITH SILT (SP-SM); light brown to medium brown; very dense; moist; subangular; poorly graded; fine to lower medium grained	
SS 2						14-12-12	288.6			same as above; light brown with some black zones; medium dense; fine to medium grained	
SS 3						11-9-8	287.1	10		same as above; medium brown; wet	
SS 4						8-5-5	285.6			SILTY SAND (SM); medium brown to light brown; loose; wet; subangular; poorly graded; fine to medium grained	
SS 5						3-8-15	284.1			CLAYEY SAND (SC) trace medium gravel; medium red, light brown, and grayish orange; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 6						4-13-17	282.6	15		same as above; with fine to medium gravel; medium red, light brown, and gray	
SS 7						21-19-9	281.1			same as above; trace fine gravel; medium brownish red and medium grayish brown	
SS 8						10-25-26	279.6			same as above; trace coarse sand and fine gravel; medium brownish red with some light gray to white zones; very dense; wet	
SS 9						21-29-20	278.1			same as above; medium brownish red; dense	
SS 10						12-29-46	276.6	20		same as above; with fine to medium gravel and few sandy clay layers; medium brownish red with light gray to white zones; very dense; fine to upper medium grained; joint at 20 ft depth	
SS 11						21-21-20	271.1	25		POORLY GRADED SAND WITH CLAY (SP-SC); medium brownish red; dense; moist; subangular; poorly graded; fine to upper medium grained	
SS 12						17-22-29	267.3	30		same as above; with some light gray layers; very dense; wet; fine to medium grained	
SS						14-18-20				CLAYEY SAND (SC) with lean clay layer in tip of spoon;	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.

FB-27



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	2 OF 5	FB-27
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
13			262.0			light yellowish brown to reddish brown and light gray to purple clay layer in tip of spoon; dense; wet; subangular; poorly graded; fine to medium grained	
SS 14	▲ 45 ○ 65	13-23-23	258.2	40		same as above; few clay laminae; medium yellowish brown with light gray clay laminae	
SS 15	▲ 55 ○ 65	18-21-30	252.3	45		same as above; portions silty sand; medium yellowish brown and reddish brown; very dense	
SS 16	▲ 35 ○ 65	11-12-11	247.8	50		same as above; medium yellowish brown; medium dense	
SS 17	▲ 55 ○ 65	30-26-24	242.7	55		POORLY GRADED SAND (SP) trace clay; light yellowish brown; dense; wet; subangular; poorly graded; fine to medium grained	
SS 18	▲ 35 ○ 65	10-10-15	238.8	60		CLAYEY SAND (SC); dark yellowish brown; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 19	○ 65 ▲ 75	24-39-40	231.1	65		same as above; medium yellowish brown; very dense	
SS 20	○ 65 ▲ 75	27-29-32	226.1	70		same as above; dark yellowish brown with black specks; well graded; fine to coarse grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-27



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 5	FB-27
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 21	20 40 60 80	30-28-32	221.1			POORLY GRADED SAND WITH SILT (SP-SM); dark orange; very dense; wet; subangular; poorly graded	
SS 22		15-17-18	216.1	80		WELL GRADED SAND WITH SILT AND GRAVEL (SW-SM); dark yellowish brown; dense; wet; subangular; well graded; fine to very coarse grained	
SS 23		3-5-9	211.1	85		SANDY LEAN CLAY (CL); dark yellowish brown; stiff; wet; medium plasticity; trace of coarse sand	
SS 24		6-7-9	206.1	90		SANDY FAT CLAY (CH); dark yellowish brown; mottled with black; very stiff; moist; high plasticity; Mn present	
SS 25		4-7-10	201.1	95		SILTY SAND (SM); medium yellowish brown; medium dense; wet; subangular; well graded; fine to coarse grained	
SS 26		6-8-9	196.1	100		CLAYEY SAND WITH GRAVEL (SC); medium yellowish brown with white mottles; medium dense; wet; subrounded; well graded; fine sand to granule sized grains	
SS 27		25-42-56	191.1	105		WELL GRADED SAND WITH SILT AND GRAVEL (SW-SM); medium yellowish brown; very dense; wet; subangular; well graded; medium to very coarse grained with gravel	
SS 28		6-7-15	186.1	110		CLAYEY SAND WITH GRAVEL (SC); light yellowish brown with tan and white mottles; medium dense; moist; subrounded; well graded; fine to very coarse grained	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE	FINAL LOG		HOLE NO. FB-27

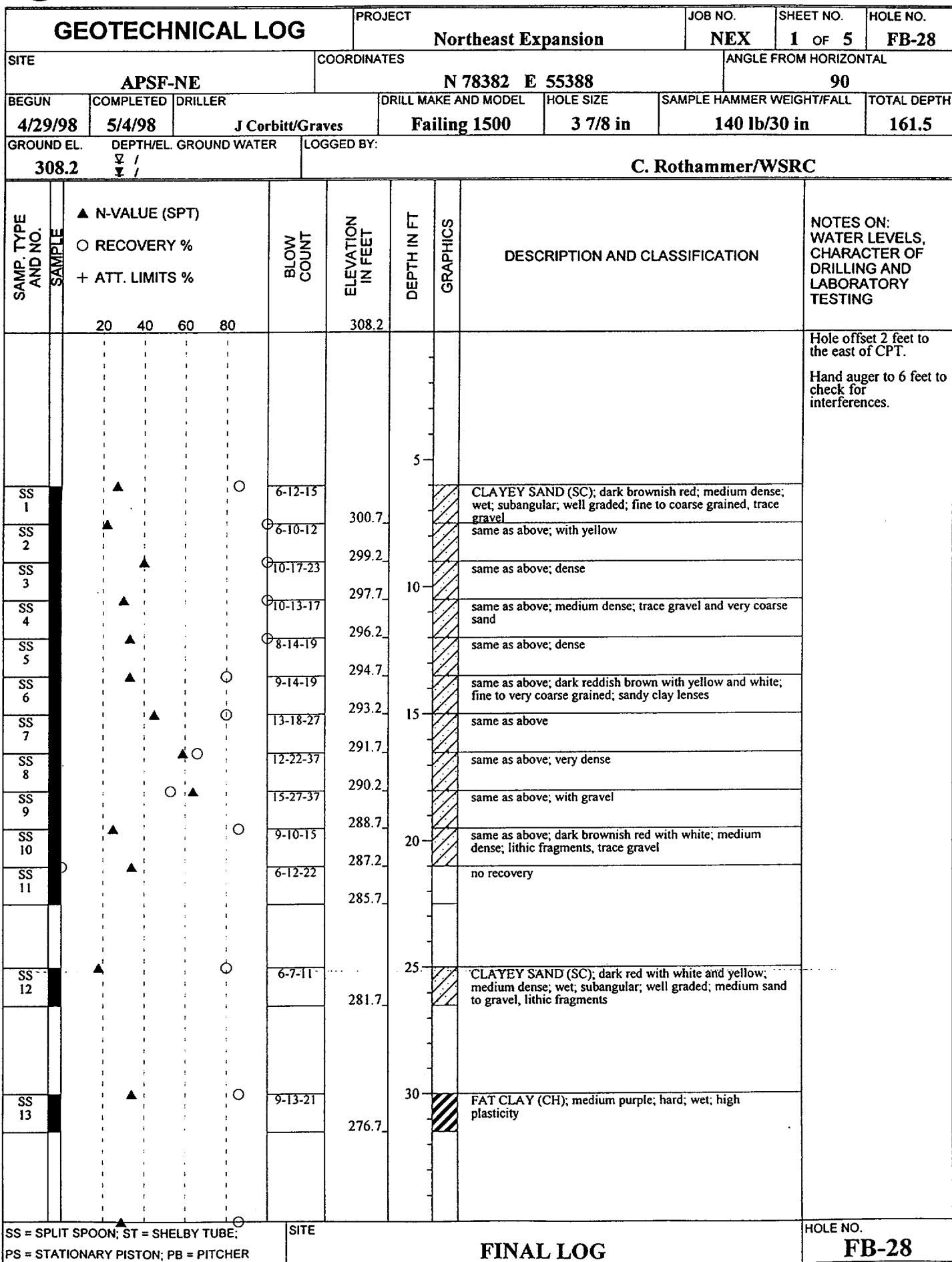


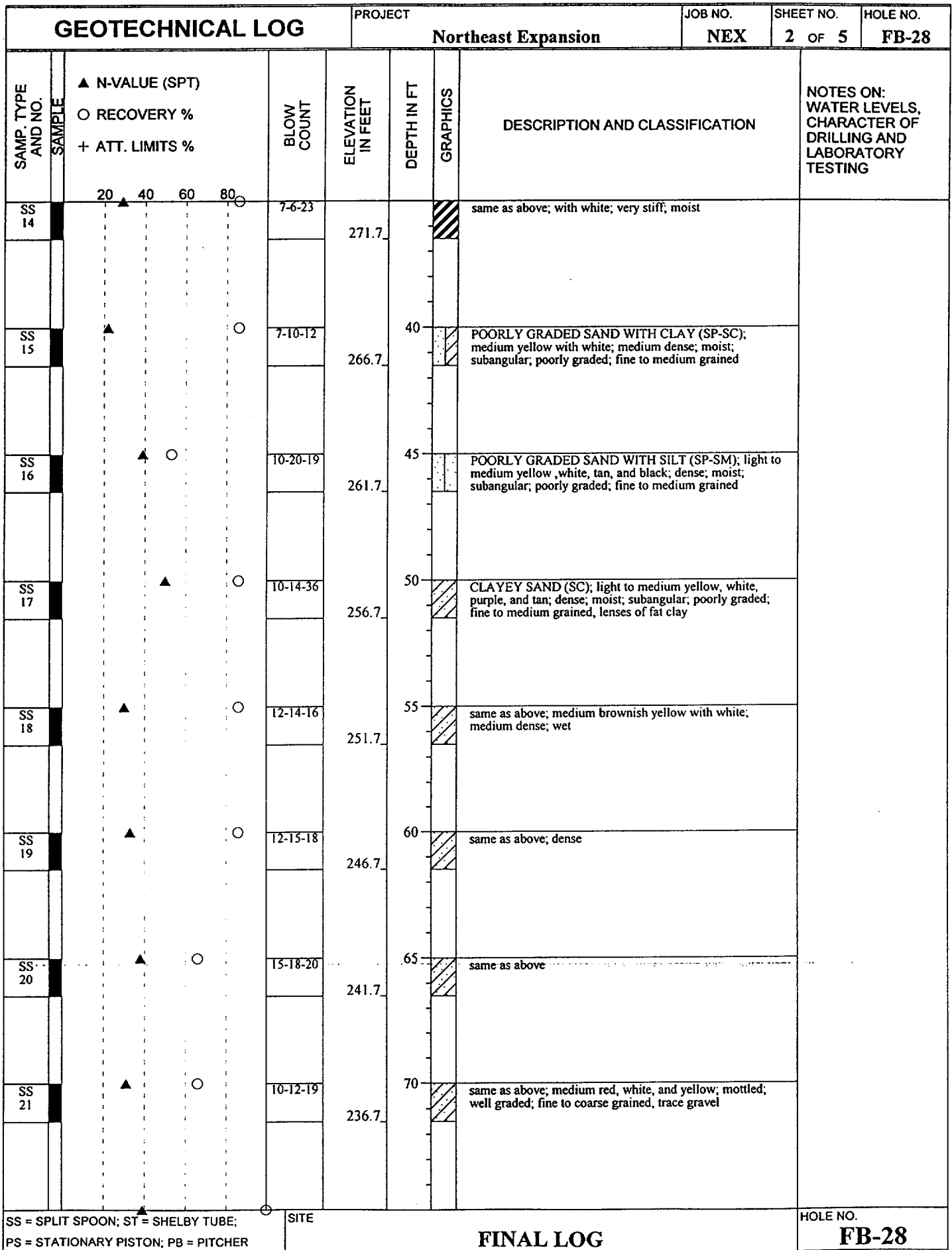
GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	4 OF 5	FB-27
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 29	20 ▲	15-12-17	181.1			SILTY SAND (SM) with clay; medium yellowish brown; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 30	○	25-36-55	176.1	120		POORLY GRADED SAND (SP) trace clay; white; very dense; moist; rounded; poorly graded; fine to very fine grained	
SS 31	○	125-86-70-55	171.1	125		POORLY GRADED SAND WITH SILT (SP-SM); light tannish white; very dense; moist; rounded; poorly graded; fine to very fine grained	
SS 32	▲	10-7-11	166.1	130		same as above; light yellowish brown with white; medium dense	
SS 33	▲	8-11-18	161.1	135		ELASTIC SILT WITH SAND (MH); light yellowish brown with white; very stiff; moist; medium plasticity	
SS 34	○	151-19-81-70	156.1	140		SILTY SAND (SM); medium yellowish brown with white; very dense; moist; subrounded; poorly graded; fine to very fine grained; silts are elastic	
SS 35	○	20-22-23	151.1	145		same as above; with clay; dense	
SS 36	▲	12-14-19	146.1	150		CLAYEY SAND (SC); very dark greenish black; dense; moist; subangular; poorly graded; very fine grained, micaceous, green clay	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-27

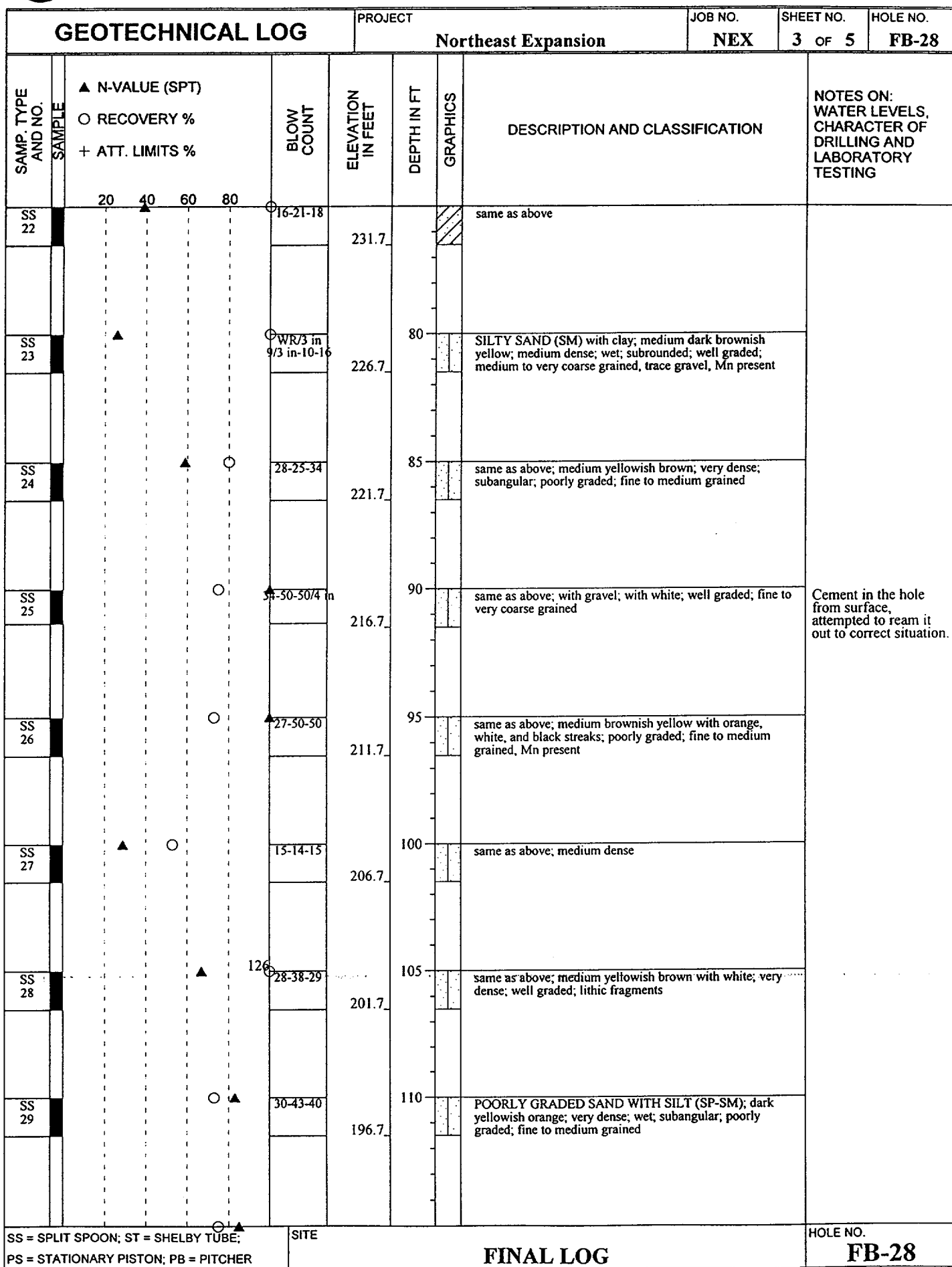


GEOTECHNICAL LOG				PROJECT Northeast Expansion		JOB NO. NEX	SHEET NO. 5 OF 5	HOLE NO. FB-27
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		20 40 60 80	27-34-35				Total depth of boring 155 feet.	Broke down spoon but did not bottle sample. Hole abandoned with grout mix per 3Q5.
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER			SITE				FINAL LOG	

HOLE NO.
FB-27









GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	4 OF 5	FB-28
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 30	20 40 60 80	29-35-50/4 in	191.7			same as above; with coarse sand	
SS 31		22-24-22	186.7	120		SILTY SAND (SM); dark yellowish orange; dense; wet; subangular; poorly graded; fine to medium grained	
SS 32		12-14-16	181.7	125		CLAYEY SAND (SC); dark yellowish orange; medium dense; wet; subangular; poorly graded; fine to medium grained	
SS 33		8-9-10	176.7	130		same as above; medium yellowish brown	
SS 34		WR 8 in-1/4 in-3	171.7	135		ELASTIC SILT (MH); medium yellowish brown with white; soft; moist; medium plasticity	
SS 35		6-16-29	170.2			SILTY SAND (SM); light medium brown with white and red; dense; wet; subrounded; poorly graded; very fine grained, silts are elastic	
SS 36		50/4 in	166.7	140		no recovery	
SS 37		122 35-60-62	161.7	145		WELL-GRADED SAND WITH GRAVEL (SW); light yellowish brown; very dense; wet; subrounded; well graded; medium to very coarse grained	
SS 38		10-12-12	156.7	150		SILTY SAND (SM); light greenish yellow with orange; medium dense; moist; subangular; poorly graded; very fine to fine grained, silts are elastic	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-28



GEOTECHNICAL LOG				PROJECT Northeast Expansion		JOB NO. NEX	SHEET NO. 5 OF 5	HOLE NO. FB-28
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
SS 39		20 40 60 80	19-20-38	151.7			same as above; light yellowish brown; very dense	Hole abandoned with grout mix per 3Q5.
SS 40			19-43-51	146.7	160		same as above	
							Total depth of boring 161.5 feet.	

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-28



GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.			
SITE				COORDINATES		ANGLE FROM HORIZONTAL					
APSF-NE				N 79371 E 55388		90					
BEGUN	COMPLETED	DRILLER	DRILL MAKE AND MODEL		HOLE SIZE	SAMPLE HAMMER WEIGHT/FALL		TOTAL DEPTH			
5/7/98	5/12/98	M Rizer/Graves	Failing 1500		3 7/8 in	140 lb/30 in		179.4			
GROUND EL.		DEPTH/EL. GROUND WATER	LOGGED BY:								
284.1		▽ /	N. Kidd/SAIC								
SAMP. TYPE AND NO.	SAMPLE	▲ N-VALUE (SPT)				BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
		○ RECOVERY %	+ ATT. LIMITS %								
		20	40	60	80		284.1				
								5			
SS 1		▲			○	7-9-9				POORLY GRADED SAND WITH CLAY (SP-SC); light brown; medium dense; wet; subangular; poorly graded; fine to medium grained; trace heavies; trace mica	
SS 2		▲			○	2-3-8	275.6			POORLY GRADED SAND (SP) trace clay; some zones with clay; light reddish brown; medium dense; moist; subangular; poorly graded; fine to medium grained; much bentonite in tube; no useful sample recovered; trace heavies; same as above; light brown; wet; no bentonite observed	
SS 3		▲		○		5-6-6	274.1	10			
SS 4		▲		○		5-6-9	272.6			same as above; moist; some zones are wet	
SS 5		▲		○		7-12-19	271.1			same as above; very light brown with light brown areas; dense; trace mica	
SS 6		▲		○		13-15-15	269.6	15		CLAYEY SAND (SC); light orange and light red with light brown bands; medium dense; moist; subangular; poorly graded; fine to medium grained	
SS 7		▲		○		7-12-19	268.1			same as above; with sandy lean clay zones; medium red and medium orange with light yellow and white zones; dense; trace lower coarse	
SS 8		▲		○		7-9-11	266.6			same as above; medium red with light yellow and white zones; medium dense	
SS 9		▲		○		6-8-10	265.1			same as above; with sandy lean clay zones; medium brownish red; fine to lower coarse grained; trace mica	
							263.6	20			
SS 10			▲		○	16-16-26				same as above; medium brownish yellow with medium red zones; dense; fine to medium grained with coarse grains	
							258.6	25			
SS 11		▲			○	13-15-21				same as above; medium red with white wisps; fine to medium grained	
							253.6	30			
SS			▲		○	47-50/4 in				same as above; medium brownish red with white wisps;	
							249.3				
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER						SITE			HOLE NO.		
						FINAL LOG			FB-30		



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	2 OF 5	FB-30
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
12						very dense	
SS 13	▲ 40 ○ 60	17-21-22	243.6	40		same as above; with interlaminated clean sands; medium brownish yellow with white wispy clays; dark yellowish brown zones and light red zones; dense	
SS 14	○ 60 ▲ 80	27-42-42					
			238.6	45		same as above; dark red; very dense; wet; trace coarse grains	
SS 15	○ 40 ▲ 40	17-21-22	233.6	50		WELL GRADED SAND WITH CLAY (SW-SC) borderline clayey sand; medium brown; dense; moist; subangular; well graded; fine to coarse grained	
SS 16	▲ 60 ○ 60	18-27-27	228.6	55		same as above; some zones are clayey sand; medium brown and medium purplish brown with light brown zones; very dense; some zones are wet	
SS 17	▲ 40 ○ 60	14-22-25	223.6	60		same as above; medium brownish yellow with occasional white wisps; dense; wet; trace heavies	
SS 18	▲ 40 ○ 60	8-14-20	218.6	65		CLAYEY SAND (SC) with interlayered sand with clay; medium brownish yellow; dense; wet; subangular; well graded; fine to coarse grained	
SS 19	○ 40 ▲ 40	14-18-22	213.6	70		WELL GRADED SAND WITH CLAY (SW-SC) some zones are clayey; medium brownish yellow; dense; wet; subangular; well graded; fine to coarse grained	
SS	○ 40	12-16-14				CLAYEY SAND (SC); medium brownish yellow; medium	

SS = SPLIT SPOON; ST = SHELBY TUBE;
 PS = STATIONARY PISTON; PB = PITCHER

SITE

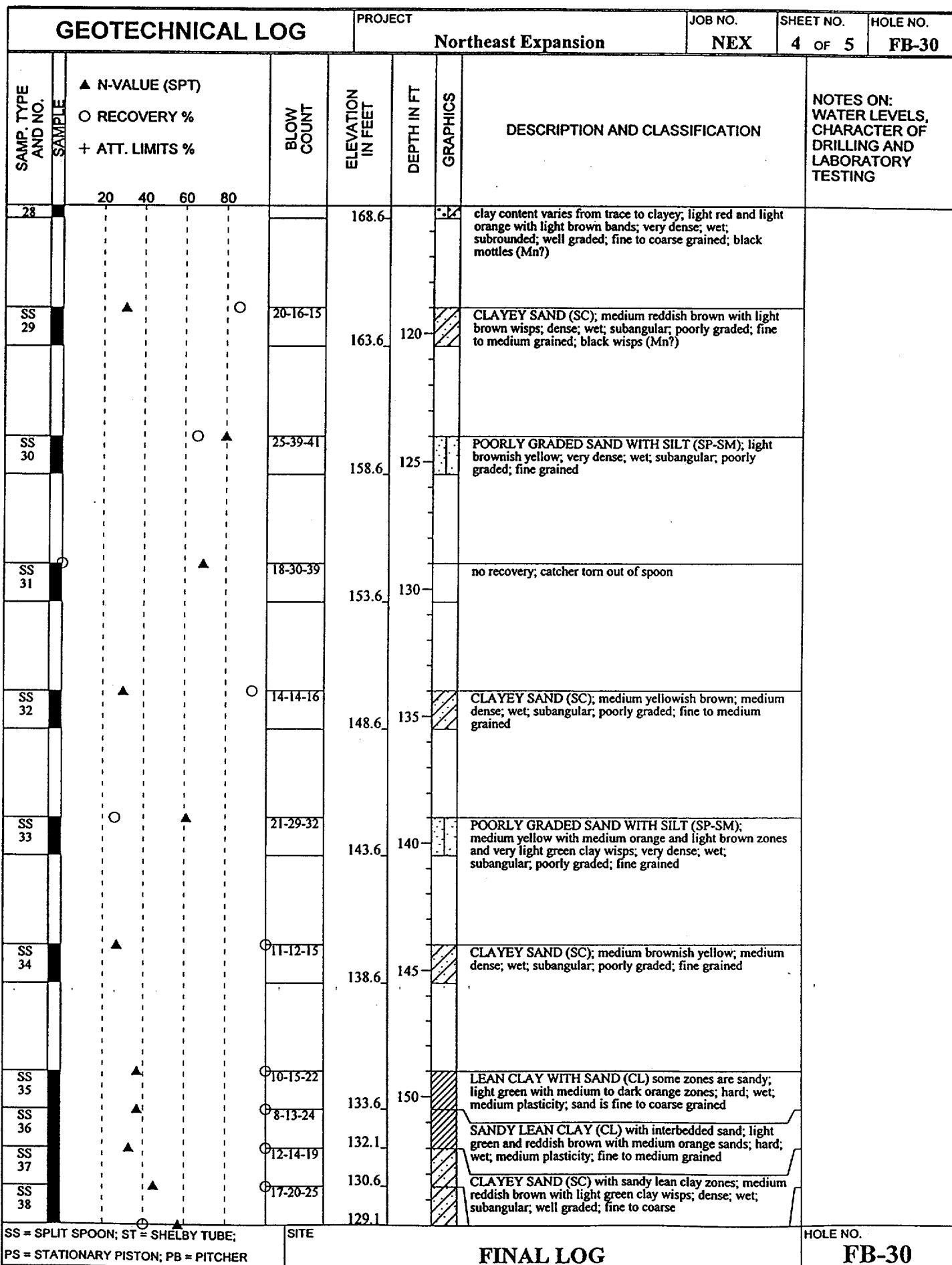
HOLE NO.

FINAL LOG

FB-30



GEOTECHNICAL LOG				PROJECT	JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion	NEX	3 OF 5	FB-30
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING
20			208.6			medium dense; wet; subangular; well graded; fine to coarse grained; trace heavies	
SS 21	▲	5-8-9	203.6	80		same as above; bottom 0.3' have tan lean clay interbeds; light yellowish brown with light yellow (tan) clay at the bottom of the interval; poorly graded; fine to medium grained; top of tan clay is about 0.3 ft above bottom of interval	
SS 22	▲	8-16-24	198.6	85		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey; light brownish yellow; dense; wet; subangular; poorly graded; fine to medium grained; black stains (Mn?)	
SS 23	▲	18-28-27	193.6	90		same as above; very dense; no black stains	
SS 24	▲	18-23-24	188.6	95		same as above; medium yellowish brown clayey sand zones; dense; moist	
SS 25	▲	17-27-32	183.6	100		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are clayey sand; light yellowish brown and light brown with occasional white wisps; very dense; wet; subangular; poorly graded; fine to medium grained; black mottles (Mn?)	
SS 26	○	34-48-50/5 in	178.7	105		POORLY GRADED SAND (SP) trace clay; light brown with very light brown and light red zones; very dense; wet; subangular; poorly graded; fine to medium grained; trace heavies	
SS 27	▲	39-50/5 in	174.2	110		POORLY GRADED SAND WITH CLAY (SP-SC) some zones are trace clay; light brown with very light brown and light reddish brown zones; very dense; wet; subangular; poorly graded; fine to medium grained; black mottles (Mn?); trace heavies	
SS	▲	30-34-23				WELL GRADED SAND WITH CLAY (SW-SC)	
SS = SPLIT SPOON; ST = SHELBY TUBE; PS = STATIONARY PISTON; PB = PITCHER				SITE			HOLE NO.
				FINAL LOG			FB-30





GEOTECHNICAL LOG				PROJECT		JOB NO.	SHEET NO.	HOLE NO.
				Northeast Expansion		NEX	5 OF 5	FB-30
SAMP. TYPE AND NO.	▲ N-VALUE (SPT) ○ RECOVERY % + ATT. LIMITS %	BLOW COUNT	ELEVATION IN FEET	DEPTH IN FT	GRAPHICS	DESCRIPTION AND CLASSIFICATION		
						NOTES ON: WATER LEVELS, CHARACTER OF DRILLING AND LABORATORY TESTING		
SS 39	20 40 60 80	18-21-36	127.6			grained same as above; dark brown; poorly graded; fine to medium		
SS 40	○ ▲	25-30 1/4 in	126.8			grained WELL GRADED SAND WITH CLAY (SW-SC) clay content varies widely with depth; light brown, light gray, and light orange; very dense; wet; subangular; well graded; fine to coarse grained; larger grains are subrounded; probable Congaree at bottom of spoon		
				160		same as above; some zones are trace clay; light brown with dark red zones; subrounded; fine to lower coarse grained; trace heavies; red zones are iron cemented; Congaree		
				165				
				170				
				175				

SS = SPLIT SPOON; ST = SHELBY TUBE;
PS = STATIONARY PISTON; PB = PITCHER

SITE

FINAL LOG

HOLE NO.
FB-30