

**From:** Goutam Bagchi *NRR*  
**To:** George Hubbard *NRR*  
**Date:** Fri, Aug 11, 2000 10:45 AM  
**Subject:** Seismic Input for TWG Report: Decommissioning

George,  
You wanted the input in whatever form it exists as of Friday morning. I have attached the file which I prepared from Nilesh Chokshi's outline. Then I requested Bob Rothman to add his thoughts to it. Please note that my management's approval is necessary before this writeup goes beyond the TWG and the immediate management. Nilesh also intends to add his remarks. I expect that the version that you can work with will be available by next Tuesday.

Thank you,  
Goutam  
301-415-3305

**CC:** Gene Imbro, Jack Strosnider, Kamal Manoly, Nilesh Chokshi, Richard Wessman, Robert Rothman, Timothy Collins

B/3-11

### Outline- White Paper on Seismic Risk of Spent Fuel Pool in Decommissioning

#### Current Approach:

Because of the need for radiation shielding the spent fuel pool structures at nuclear power reactor facilities are constructed with thick reinforced concrete walls and floor slabs. This construction provides the pool structures considerable strength reserve for resisting seismic loads much beyond its design basis seismic loading (safe shutdown earthquake vibratory motion). The safe shutdown earthquakes for U. S. operating nuclear power plants were based on evaluations to obtain the maximum earthquake potential considering the regional and local geology and seismology and specific characteristics of local subsurface material using deterministic methods. A comparison of the safe shutdown earthquake (SSE) ground motions of eastern U. S. nuclear power plants to the seismic hazard estimates of two currently available studies, one by the Lawrence Livermore National Laboratory (LLNL) and the other by the Electric Power Research Institute (EPRI) indicate that in general the SSE have frequencies of exceedance on the order of  $1 \times 10^{-3}$  to  $1 \times 10^{-4}$  per year.

The study of the failure probability of spent fuel pools was conducted for the NRC by Dr. Robert Kennedy utilizing the seismic hazard estimates of LLNL and the EPRI studies. The result of the Kennedy study shows that the seismic probability of failure of spent fuel pools generally fall below  $1.3 \times 10^{-6}$  using the LLNL hazard, except for about 9 sites where the probability of failure is somewhat higher. This was a generic study using a capacity assumption of 1.2 g spectral acceleration. The capacity at individual spent fuel pools could be higher in many cases and somewhat smaller in cases where out of plane shear failure can be a concern. In order to determine a criteria for generically eliminating the concern for seismic failure, the NRC staff proposed an approach based on a physical verification of the pool structure followed by a confirmation of structural capacity at the levels of  $2 \times \text{SSE}$  and  $3 \times \text{SSE}$  for the west and east coast sites respectively. Considering the conservatism used in determining the SSE for the plants and the regulatory requirement that it represent the maximum earthquake potential for the site the possibility of having ground motions  $2 \times \text{SSE}$  and  $3 \times \text{SSE}$  at operating plants under the current tectonic regime is extremely unlikely. This was a bounding approach and the staff was very confident that the plants screened out by this approach would have a very low failure probability and in a risk informed sense, the seismic failure likelihood for these plants can be ignored as being too small. Although, from a risk point of view the seismic failure probability was assumed to be  $1.3 \times 10^{-6}$ , keeping in mind that there is a factor of conservatism in the capacity of pool structures. Probabilistic ground motion estimates at annual frequencies of exceedance of  $1 \times 10^{-5}$  or less for the eastern U. S. nuclear power plants are very uncertain due to the flat slope of the seismic hazard curves at these frequencies and the lack of tectonic strain, in these areas, large enough to generate the events capable of producing ground motions at these levels.

From the seismic hazard stand point, both the LLNL and the EPRI estimates are equally credible estimates and the divergence in individual site results stem, in part, from the large uncertainty in the ground motion modeling and the differences in the ways the expert judgements were elicited in the two studies. The uncertainty in seismic hazard estimates is well known and they are so large that seismic risk estimates are generally not compared with risk from internal events and are used in a relative sense rather than as an absolute value.

#### Key Assumptions:

1. Site specific SSE values generally correlate with the site seismic hazard and can be used as a basis for a bounding set of criteria.
2. LLNL and EPRI hazard estimates are equally valid. LLNL results being more conservative, should be used for consideration in risk informed decisions.
3. Spent fuel pool capacity can be represented generically by 1.2 g spectral acceleration at a high confidence in low probability of failure.
4. A physical verification of the structural integrity of spent fuel pools, confirmation of structural strength based verification of construction drawings, affirmation of no ongoing age related degradation and verification that there are no sources of seismic interaction between pool structures and the superstructure are to be conducted through the use of a seismic check list.

#### Sources of Conservatism:

1. For some eastern US sites, the SSE frequency is very low or does not correlate to the seismic hazard estimate.
2. Although the 1993 LLNL hazard estimates correlated well with the EPRI results at the SSE levels, the LLNL hazard estimates are significantly more conservative than the EPRI estimates for earthquakes several times larger than the SSE levels.
3. The structural capacity of spent fuel pools is set at a relatively low value that can be readily verified through a peer reviewed data base.

#### Sources of Uncertainty

1. The primary source of uncertainty is in the ground motion estimates at very low frequencies in the 1XE-5 to 1XE-6 range. These uncertainties come from seismogenic source characterizations and the assumptions of ground motion attenuation. The NEI study shows that a change in sigma, the attenuation uncertainty, from 0.4 to 0.5 changes the probability of exceeding 1000 cm/sec<sup>2</sup>, an acceleration value in the range of interest for structural failures, by a factor of about 100.
2. New ground motion modeling would likely reduce uncertainty and reduce level of motion at large return periods.
3. Plant specific fragility values are likely to be higher.

#### Quantification of Uncertainty

1. Ground motion modeling has the largest source of uncertainty. There are two effects here, one is the site specific uncertainty in the hazard estimate where a factor of 3 is in the noise range, and the other results from the bounding effect. In a bounding approach a large number of sites are well below the screening value, because of this approach a margin exists for certain specific sites, but the factor is not quantifiable.
2. The joint NRC and DOE developed methodology for performing seismic hazard studies recently completed can be used to better estimate the seismic hazard. It is expected that in a large majority of cases, the frequency of large ground motions will be reduced. This factor is not readily quantifiable.
3. In the spent fuel pool fragility evaluation, the deep box shape of the pool needs to be taken into account. The available fragility evaluation has not considered the ultimate failure mode. Near the lower part of the pool membrane stretching would be the primary

mode of load transfer and in the upper part of the pool the load would be carried by out of plane shear. Since the out of plane shear in the upper part would be quite a bit less, there will be some margin that is currently not recognized. In the absence of a detailed three dimensional finite element analysis of the pool structures, it is the staff judgement that the additional margin is in the order of a factor of 2.

4. The overall factor of conservatism is judged to be about a factor of 5.

#### Proposed approach

Considering the ACRS comment, detailed NEI comments and reviewing the factors of conservatism as discussed above, the staff finds that in Table 3 of Dr. Kennedy's report the probability of failure of  $4.5 \times 10^{-6}$  provides a convenient line of demarcation between sites with low probability of failure and the four sites with relatively higher probability of failure. This also means that the seismic check list can be used with a 0.5 g capacity screening. Looking at the NEI letter of November 2, 1999 there is a figure which shows a  $1 \times 10^{-6}$  per year failure probability line that covers all but five sites. These two approaches produce essentially the same ranking of plant failure probability. Although the figures of merit in the two cases are different, when used in relative sense they yield the same result. Given the factors of conservatism, one can argue that the seismic screening of plants at 1.2 g spectral acceleration provides assurance of a low probability of failure due to earthquakes on the order of  $1 \times 10^{-6}$  per year and a physical verification of structural adequacy of the pools.

#### Decision-Making Framework for Seismic Risk:

The staff intends to display separate results for both LLNL and EPRI as indicators of range of results and also to display mean and median (may require additional calculations) results to high light uncertainties, sources of uncertainties, and to provide a perspective on seismic risk when compared to other initiators.

#### Conclusions and Recommendations:

Using either the LLNL or the EPRI/NEI results, the plant risk ranking remains essentially the same and the proposed approach ensures, through the seismic check list and walkdown, seismic capacity and low risk. Only three Eastern US plants may require additional analysis. Refinements will not lead to a different conclusion as the results will be very plant specific and factors greater than 5 (for example) will be difficult to obtain because of hazard driven uncertainties.

Considering the above factors (i.e., knowing the bounding range, sources of uncertainties, and nature of uncertainties), the most useful index for risk-informed decision is the capacity measure, that is, if a plant demonstrates or confirms that the plant HCLPF is greater than 1.2g spectral acceleration ( $\sim 0.5$ g peak ground acceleration), the seismic risk is acceptably low and should be treated in the same fashion as other low-risk initiators. The seismic risk should not be a determinant of requirements for EP, insurance, etc.